



April 27, 2023

El Paso County Planning and Community Development  
2880 International Circle, Suite 110  
Colorado Springs, CO 80910

RE: Lorson Ranch East Filing No. 1 (SF 18-008)  
Certification Letter

Dear El Paso County PCD,

Based upon information gathered from as-built surveys and periodic visits to the project, Core Engineering Group is of the opinion that the subdivision improvements have been constructed in general conformance with the approved design plans as filed with El Paso County.

The site and adjacent properties (as affected by work performed under the County permit) appear to be stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and the improvements (public improvements, common development improvements, site grading and paving) visually appear to meet or exceed the minimum design requirements.

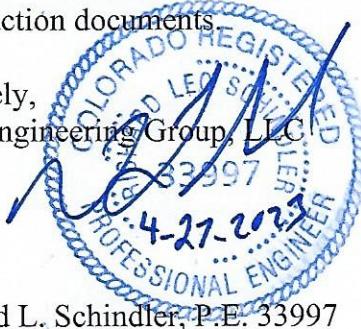
The sanitary and watermain located in the public ROW has also been completed in accordance with Widefield Water and Sanitation Districts criteria.

In addition, Core Engineering Group has verified that the Extended Detention Basin/WQ Pond C5 and D2 meet the volume and elevation requirements and are constructed in general compliance with the approved construction plans. The pond "as-built" documents are attached to this letter. The tributary area draining to Pond D2 has changed and the updated spreadsheet with the corresponding spreadsheet printouts is attached.

Based on information gathered during construction and post-construction, Core Engineering Group is of the opinion that the public streets, storm sewer, and Detention Pond C5 and D2 have been constructed in general accordance with the approved construction documents.

Sincerely,  
Core Engineering Group, LLC

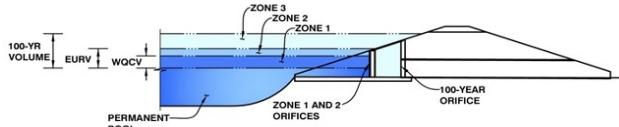
Richard L. Schindler, P.E. 33997



Attachments: Pond As-built elevations

## Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Lorson Ranch East MDDP Basin ID: Pond D2 - As-built																																																																																																																																																																			
																																																																																																																																																																			
<b>Example Zone Configuration (Retention Pond)</b> <table border="1" style="margin-left: auto; margin-right: auto; border-collapse: collapse; width: fit-content;"> <thead> <tr> <th style="text-align: left;">Stage (ft)</th> <th style="text-align: left;">Zone Volume (ac-ft)</th> <th style="text-align: left;">Outlet Type</th> </tr> </thead> <tbody> <tr> <td>Zone 1 (WQCV)</td> <td>3.08</td> <td>Orifice Plate</td> </tr> <tr> <td>Zone 2 (EURV)</td> <td>4.77</td> <td>Rectangular Orifice</td> </tr> <tr> <td>Zone 3 (100-year)</td> <td>6.75</td> <td>Weir&amp;Pipe (Restrict)</td> </tr> <tr> <td></td> <td style="border-top: none;">8.168</td> <td style="border-top: none;">Total</td> </tr> </tbody> </table>		Stage (ft)	Zone Volume (ac-ft)	Outlet Type	Zone 1 (WQCV)	3.08	Orifice Plate	Zone 2 (EURV)	4.77	Rectangular Orifice	Zone 3 (100-year)	6.75	Weir&Pipe (Restrict)		8.168	Total																																																																																																																																																			
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<b>User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)</b> Underdrain Orifice Invert Depth = <span style="border: 1px solid black; padding: 2px;">N/A</span> ft (distance below the filtration media surface) Underdrain Orifice Diameter = <span style="border: 1px solid black; padding: 2px;">N/A</span> inches																																																																																																																																																																			
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<b>User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)</b> Invert of Lowest Orifice = <span style="border: 1px solid black; padding: 2px;">0.00</span> ft (relative to basin bottom at Stage = 0 ft) Depth at top of Zone using Orifice Plate = <span style="border: 1px solid black; padding: 2px;">3.08</span> ft (relative to basin bottom at Stage = 0 ft) Orifice Plate: Orifice Vertical Spacing = <span style="border: 1px solid black; padding: 2px;">9.00</span> inches Orifice Plate: Orifice Area per Row = <span style="border: 1px solid black; padding: 2px;">4.00</span> sq. inches (use rectangular openings)																																																																																																																																																																			
		<b>Calculated Parameters for Plate</b> WQ Orifice Area per Row = <span style="border: 1px solid black; padding: 2px;">2.778E-02</span> ft <sup>2</sup> Elliptical Half-Width = <span style="border: 1px solid black; padding: 2px;">N/A</span> feet Elliptical Slot Centroid = <span style="border: 1px solid black; padding: 2px;">N/A</span> feet Elliptical Slot Area = <span style="border: 1px solid black; padding: 2px;">N/A</span> ft <sup>2</sup>																																																																																																																																																																	
<b>User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)</b> <table border="1" style="margin-left: auto; margin-right: auto; border-collapse: collapse; width: fit-content;"> <tbody> <tr> <td style="text-align: center;">Row 1 (required)</td> <td style="text-align: center;">Row 2 (optional)</td> <td style="text-align: center;">Row 3 (optional)</td> <td style="text-align: center;">Row 4 (optional)</td> <td style="text-align: center;">Row 5 (optional)</td> <td style="text-align: center;">Row 6 (optional)</td> <td style="text-align: center;">Row 7 (optional)</td> <td style="text-align: center;">Row 8 (optional)</td> </tr> <tr> <td>Stage of Orifice Centroid (ft)</td> <td>0.00</td> <td>0.60</td> <td>1.20</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Orifice Area (sq. inches)</td> <td>4.00</td> <td>4.00</td> <td>4.00</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td style="text-align: center;">Row 9 (optional)</td> <td style="text-align: center;">Row 10 (optional)</td> <td style="text-align: center;">Row 11 (optional)</td> <td style="text-align: center;">Row 12 (optional)</td> <td style="text-align: center;">Row 13 (optional)</td> <td style="text-align: center;">Row 14 (optional)</td> <td style="text-align: center;">Row 15 (optional)</td> <td style="text-align: center;">Row 16 (optional)</td> </tr> <tr> <td>Stage of Orifice Centroid (ft)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Orifice Area (sq. inches)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>		Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)	Stage of Orifice Centroid (ft)	0.00	0.60	1.20					Orifice Area (sq. inches)	4.00	4.00	4.00					Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)	Stage of Orifice Centroid (ft)								Orifice Area (sq. inches)																																																																																																																									
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		<b>Calculated Parameters for Outlet Pipe w/ Flow Restriction Plat</b> Zone 3 Restrictor = <span style="border: 1px solid black; padding: 2px;">10.91</span> ft (distance below basin bottom at Stage = 0 ft) Outlet Orifice Area = <span style="border: 1px solid black; padding: 2px;">1.64</span> ft <sup>2</sup> Outlet Orifice Centroid = <span style="border: 1px solid black; padding: 2px;">1.64</span> feet Half-Central Angle of Restrictor Plate on Pipe = <span style="border: 1px solid black; padding: 2px;">1.87</span> N/A																																																																																																																																																																	
<b>User Input: Emergency Spillway (Rectangular or Trapezoidal)</b> <table border="1" style="margin-left: auto; margin-right: auto; border-collapse: collapse; width: fit-content;"> <tbody> <tr> <td>Spillway Invert Stage</td> <td><span style="border: 1px solid black; padding: 2px;">7.00</span></td> <td>ft (relative to basin bottom at Stage = 0 ft)</td> </tr> <tr> <td>Spillway Crest Length</td> <td><span style="border: 1px solid black; padding: 2px;">30.00</span></td> <td>feet</td> </tr> <tr> <td>Spillway End Slopes</td> <td><span style="border: 1px solid black; padding: 2px;">4.00</span></td> <td>H:V</td> </tr> <tr> <td>Freeboard above Max Water Surface</td> <td><span style="border: 1px solid black; padding: 2px;">3.00</span></td> <td>feet</td> </tr> </tbody> </table>		Spillway Invert Stage	<span style="border: 1px solid black; padding: 2px;">7.00</span>	ft (relative to basin bottom at Stage = 0 ft)	Spillway Crest Length	<span style="border: 1px solid black; padding: 2px;">30.00</span>	feet	Spillway End Slopes	<span style="border: 1px solid black; padding: 2px;">4.00</span>	H:V	Freeboard above Max Water Surface	<span style="border: 1px solid black; padding: 2px;">3.00</span>	feet																																																																																																																																																						
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		<b>Calculated Parameters for Spillway</b> Spillway Design Flow Depth = <span style="border: 1px solid black; padding: 2px;">1.64</span> feet Stage at Top of Freeboard = <span style="border: 1px solid black; padding: 2px;">11.64</span> feet Basin Area at Top of Freeboard = <span style="border: 1px solid black; padding: 2px;">2.43</span> acres bottom orifice=95.00																																																																																																																																																																	
<b>Routed Hydrograph Results</b> <table border="1" style="margin-left: auto; margin-right: auto; border-collapse: collapse; width: fit-content;"> <thead> <tr> <th>Design Storm Return Period</th> <th>WQCV</th> <th>EURV</th> <th>2 Year</th> <th>5 Year</th> <th>10 Year</th> <th>25 Year</th> <th>50 Year</th> <th>100 Year</th> </tr> </thead> <tbody> <tr> <td>One-Hour Rainfall Depth (in)</td> <td>0.53</td> <td>1.07</td> <td>1.19</td> <td>1.50</td> <td>1.75</td> <td>2.00</td> <td>2.25</td> <td>2.52</td> </tr> <tr> <td>Calculated Runoff Volume (acre-ft) =</td> <td>1.552</td> <td>4.430</td> <td>4.191</td> <td>6.097</td> <td>7.712</td> <td>10.275</td> <td>12.244</td> <td>14.693</td> </tr> <tr> <td>OPTIONAL Override Runoff Volume (acre-ft) =</td> <td>1.551</td> <td>4.428</td> <td>4.189</td> <td>6.089</td> <td>7.707</td> <td>10.263</td> <td>12.238</td> <td>14.680</td> </tr> <tr> <td>Inflow Hydrograph Volume (acre-ft) =</td> <td>1.551</td> <td>4.428</td> <td>4.189</td> <td>6.089</td> <td>7.707</td> <td>10.263</td> <td>12.238</td> <td>14.680</td> </tr> <tr> <td>Predevelopment Unit Peak Flow, q (cfs/acre) =</td> <td>0.00</td> <td>0.00</td> <td>0.02</td> <td>0.15</td> <td>0.42</td> <td>0.95</td> <td>1.26</td> <td>1.63</td> </tr> <tr> <td>Predevelopment Peak Q (cfs) =</td> <td>0.0</td> <td>0.0</td> <td>1.5</td> <td>13.0</td> <td>35.3</td> <td>80.5</td> <td>106.2</td> <td>137.4</td> </tr> <tr> <td>Peak Inflow Q (cfs) =</td> <td>30.0</td> <td>84.3</td> <td>79.8</td> <td>115.1</td> <td>144.8</td> <td>191.1</td> <td>226.6</td> <td>269.8</td> </tr> <tr> <td>Peak Outflow Q (cfs) =</td> <td>0.7</td> <td>2.1</td> <td>1.6</td> <td>14.8</td> <td>36.6</td> <td>76.7</td> <td>108.8</td> <td>119.1</td> </tr> <tr> <td>Ratio Peak Outflow to Predevelopment Q =</td> <td>N/A</td> <td>N/A</td> <td>N/A</td> <td>1.1</td> <td>1.0</td> <td>1.0</td> <td>1.0</td> <td>0.9</td> </tr> <tr> <td>Structure Controlling Flow =</td> <td>Vertical Orifice 1</td> <td>Overflow Grade 1</td> <td>Overflow Plate 1</td> </tr> <tr> <td>Max Velocity through Grate 1 (fps) =</td> <td>N/A</td> <td>0.01</td> <td>0.00</td> <td>0.2</td> <td>0.6</td> <td>1.3</td> <td>1.9</td> <td>2.1</td> </tr> <tr> <td>Max Velocity through Grate 2 (fps) =</td> <td>N/A</td> <td>N/A</td> <td>N/A</td> <td>N/A</td> <td>N/A</td> <td>N/A</td> <td>N/A</td> <td>N/A</td> </tr> <tr> <td>Time to Drain 97% of Inflow Volume (hours) =</td> <td>35</td> <td>62</td> <td>60</td> <td>62</td> <td>61</td> <td>59</td> <td>58</td> <td>56</td> </tr> <tr> <td>Time to Drain 99% of Inflow Volume (hours) =</td> <td>37</td> <td>65</td> <td>63</td> <td>66</td> <td>66</td> <td>65</td> <td>65</td> <td>64</td> </tr> <tr> <td>Maximum Ponding Depth (ft) =</td> <td>3.02</td> <td>4.64</td> <td>4.52</td> <td>5.22</td> <td>5.61</td> <td>6.05</td> <td>6.31</td> <td>6.78</td> </tr> <tr> <td>Area at Maximum Ponding Depth (acres) =</td> <td>1.59</td> <td>1.78</td> <td>1.77</td> <td>1.84</td> <td>1.88</td> <td>1.92</td> <td>1.95</td> <td>2.00</td> </tr> <tr> <td>Maximum Volume Stored (acre-ft) =</td> <td>1.461</td> <td>4.200</td> <td>3.988</td> <td>5.230</td> <td>5.973</td> <td>6.809</td> <td>7.312</td> <td>8.239</td> </tr> </tbody> </table>		Design Storm Return Period	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	One-Hour Rainfall Depth (in)	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	Calculated Runoff Volume (acre-ft) =	1.552	4.430	4.191	6.097	7.712	10.275	12.244	14.693	OPTIONAL Override Runoff Volume (acre-ft) =	1.551	4.428	4.189	6.089	7.707	10.263	12.238	14.680	Inflow Hydrograph Volume (acre-ft) =	1.551	4.428	4.189	6.089	7.707	10.263	12.238	14.680	Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.02	0.15	0.42	0.95	1.26	1.63	Predevelopment Peak Q (cfs) =	0.0	0.0	1.5	13.0	35.3	80.5	106.2	137.4	Peak Inflow Q (cfs) =	30.0	84.3	79.8	115.1	144.8	191.1	226.6	269.8	Peak Outflow Q (cfs) =	0.7	2.1	1.6	14.8	36.6	76.7	108.8	119.1	Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.1	1.0	1.0	1.0	0.9	Structure Controlling Flow =	Vertical Orifice 1	Overflow Grade 1	Overflow Plate 1	Max Velocity through Grate 1 (fps) =	N/A	0.01	0.00	0.2	0.6	1.3	1.9	2.1	Max Velocity through Grate 2 (fps) =	N/A	Time to Drain 97% of Inflow Volume (hours) =	35	62	60	62	61	59	58	56	Time to Drain 99% of Inflow Volume (hours) =	37	65	63	66	66	65	65	64	Maximum Ponding Depth (ft) =	3.02	4.64	4.52	5.22	5.61	6.05	6.31	6.78	Area at Maximum Ponding Depth (acres) =	1.59	1.78	1.77	1.84	1.88	1.92	1.95	2.00	Maximum Volume Stored (acre-ft) =	1.461	4.200	3.988	5.230	5.973	6.809	7.312	8.239												
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## DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

**Project:** LRE Filing No. 1  
**Basin ID:** Pond D2 - As-built

**Example Zone Configuration (Retention Pond)**

**Required Volume Calculation**

Selected BMP Type =	EDE
Watershed Area =	84.50 acres
Watershed Length =	2,200 ft
Watershed Slope =	0.025 ft/ft
Watershed Imperviousness =	55.00% percent
Percentage Hydrologic Soil Group A =	0.0% percent
Percentage Hydrologic Soil Group B =	0.0% percent
Percentage Hydrologic Soil Groups C/D =	100.0% percent
Desired WQCF Drain Time =	40.0 hours
Location for 1-hr Rainfall Depths =	User Input
Water Quality Capture Volume (WQCV) =	1.552 acre-feet
Excess Urban Runoff Volume (EURV) =	4.430 acre-feet
2-yr Runoff Volume ( $P_1 = 1.19 \text{ in.}$ ) =	4.191 acre-feet
5-yr Runoff Volume ( $P_1 = 1.5 \text{ in.}$ ) =	6.097 acre-feet
10-yr Runoff Volume ( $P_1 = 1.75 \text{ in.}$ ) =	7.712 acre-feet
25-yr Runoff Volume ( $P_1 = 2 \text{ in.}$ ) =	10.275 acre-feet
50-yr Runoff Volume ( $P_1 = 2.25 \text{ in.}$ ) =	12.244 acre-feet
100-yr Runoff Volume ( $P_1 = 2.52 \text{ in.}$ ) =	14.693 acre-feet
500-yr Runoff Volume ( $P_1 = 0 \text{ in.}$ ) =	0.000 acre-feet
Approximate 2-yr Detention Volume =	3.931 acre-feet
Approximate 5-yr Detention Volume =	5.745 acre-feet
Approximate 10-yr Detention Volume =	6.551 acre-feet
Approximate 25-yr Detention Volume =	7.048 acre-feet
Approximate 50-yr Detention Volume =	7.284 acre-feet
Approximate 100-yr Detention Volume =	8.168 acre-feet

**Stage-Storage Calculation**

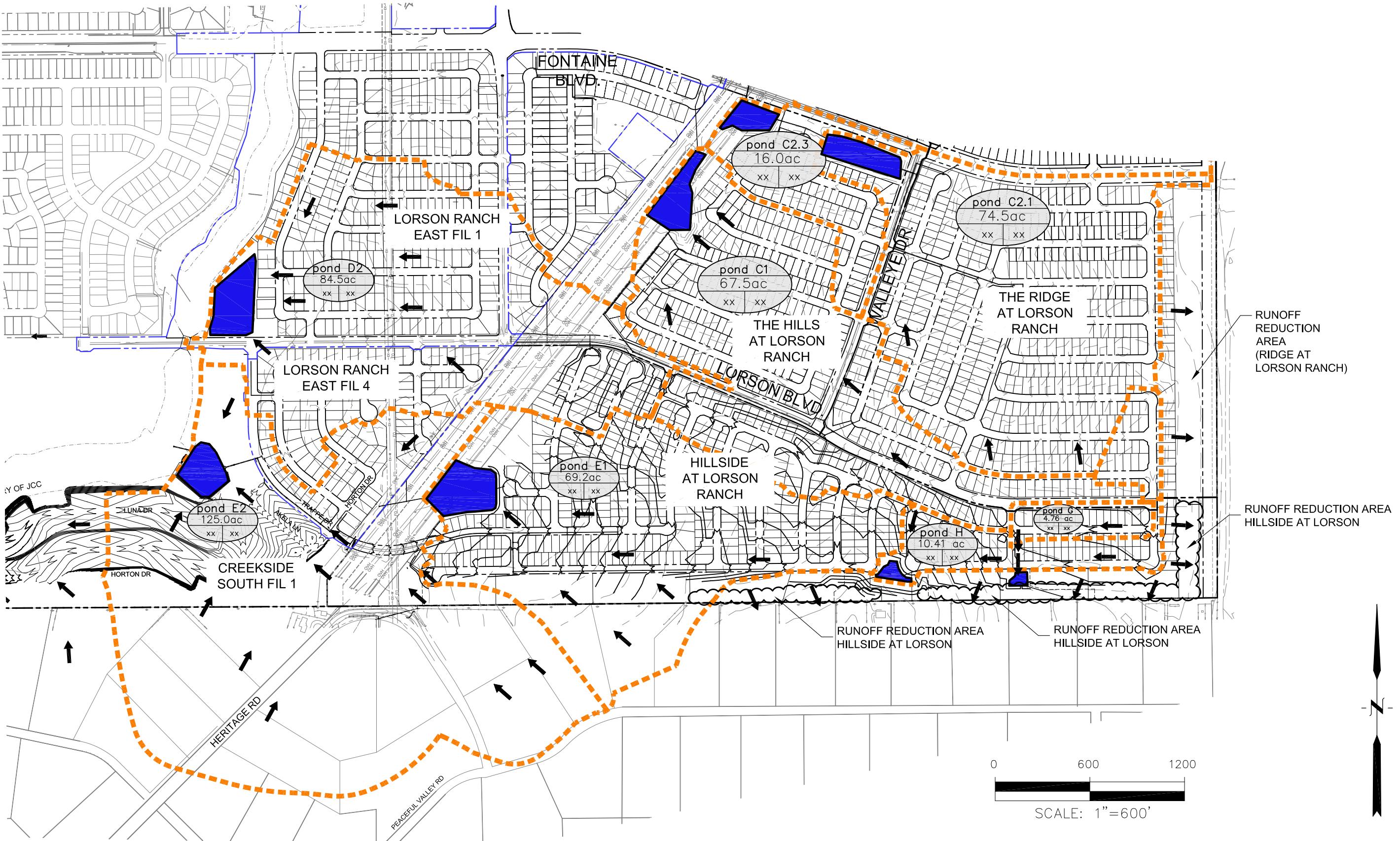
Zone 1 Volume (WQCV) =	1.552 acre-feet
Zone 2 Volume (EURV - Zone 1) =	2.878 acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	3.737 acre-feet
Total Detention Basin Volume =	8.168 acre-feet
Initial Surcharge Volume (ISV) =	user ft <sup>3</sup>
Initial Surcharge Depth (ISD) =	user ft
Total Available Detention Depth ( $H_{total}$ ) =	user ft
Depth of Trickle Channel ( $H_{TC}$ ) =	user ft
Slope of Trickle Channel ( $S_{TC}$ ) =	user ft/ft
Slopes of Main Basin Sides ( $S_{main}$ ) =	user H:V
Basin Length-to-Width Ratio ( $R_{L/W}$ ) =	user
Initial Surcharge Area ( $A_{ISV}$ ) =	user ft <sup>2</sup>
Surcharge Volume Length ( $L_{ISV}$ ) =	user ft
Surcharge Volume Width ( $W_{ISV}$ ) =	user ft
Depth of Basin Floor ( $H_{fLOOR}$ ) =	user ft
Length of Basin Floor ( $L_{fLOOR}$ ) =	user ft
Width of Basin Floor ( $W_{fLOOR}$ ) =	user ft
Area of Basin Floor ( $A_{fLOOR}$ ) =	user ft <sup>2</sup>
Volume of Basin Floor ( $V_{fLOOR}$ ) =	user ft <sup>3</sup>
Depth of Main Basin ( $H_{MAN}$ ) =	user ft
Length of Main Basin ( $L_{MAN}$ ) =	user ft
Width of Main Basin ( $W_{MAN}$ ) =	user ft
Area of Main Basin ( $A_{MAN}$ ) =	user ft <sup>2</sup>
Volume of Main Basin ( $V_{MAN}$ ) =	user ft <sup>3</sup>
Calculated Total Basin Volume ( $V_{total}$ ) =	user acre-feet

**bottom orifice - 95.00**

Depth Increment =	0.2 ft	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft <sup>2</sup> )	Optional Override Area (ft <sup>2</sup> )	Area (acre)	Volume (ft <sup>3</sup> )	Volume (ac-ft)
Top of Micropool	-	0.00	--	--	--	56	0.001	18	0.000
5695.33	-	0.33	--	--	--	56	0.001	590	0.014
5696	-	1.00	--	--	--	1,701	0.039		
5697	-	2.00	--	--	--	26,100	0.599	14,247	0.327
5698	-	3.00	--	--	--	69,360	1.592	62,237	1.429
5699	-	4.00	--	--	--	74,702	1.715	134,268	3.082
5700	-	5.00	--	--	--	79,028	1.814	211,133	4.847
5701	-	6.00	--	--	--	83,502	1.917	292,398	6.713
5702	-	7.00	--	--	--	88,029	2.021	378,164	8.681
5703	-	8.00	--	--	--	92,327	2.120	468,342	10.752
5704	-	9.00	--	--	--	96,850	2.223	562,930	12.923
5705	-	10.00	--	--	--	100,980	2.318	661,845	15.194
5706	-	11.00	--	--	--	105,820	2.429	765,245	17.568

pond D2-asbl.xlsx, Basin

4/27/2023, 6:52 AM



**CORE**  
ENGINEERING GROUP

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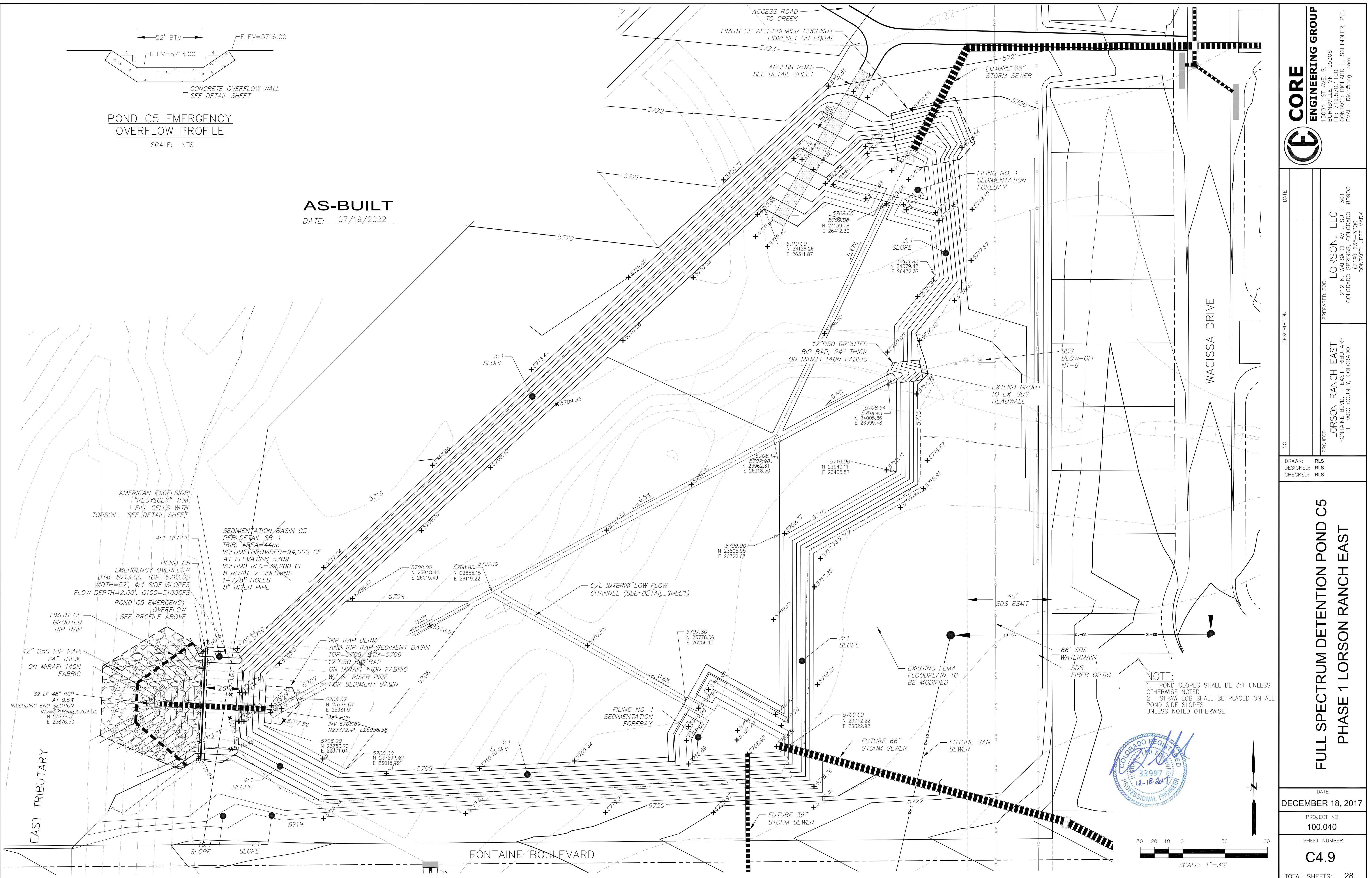
HILLSIDE AT LORSON RANCH  
WATER QUALITY & POND TRIBUTARY AREAS

SCALE: NTS	DATE: APRIL, 2022	FIGURE NO. 1
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FULL SPECTRUM DETENTION POND C5  
PHASE 1 LORSON RANCH EAST

DATE: DECEMBER 18, 2017  
PROJECT NO. 100.040

SHEET NUMBER C4.9  
TOTAL SHEETS: 28



**CORE**  
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**LORSON, LLC**  
212 N. WAHATCH AVE, SUITE 301  
COLORADO SPRINGS, COLORADO  
(719) 635-3200  
CONTACT: JEFF MARK

NO. \_\_\_\_\_  
PROJECT: **LORSON RANCH EAST**  
FILING NO. 1  
FONTAINE BLVD - OLD GLORY DR  
COLORADO SPRINGS, COLORADO

NO. \_\_\_\_\_  
DRAWN: RLS  
DESIGNED: RLS  
CHECKED: RLS

**POND C5**  
TRICKLE CHANNEL  
LAYOUT AND DETAILS

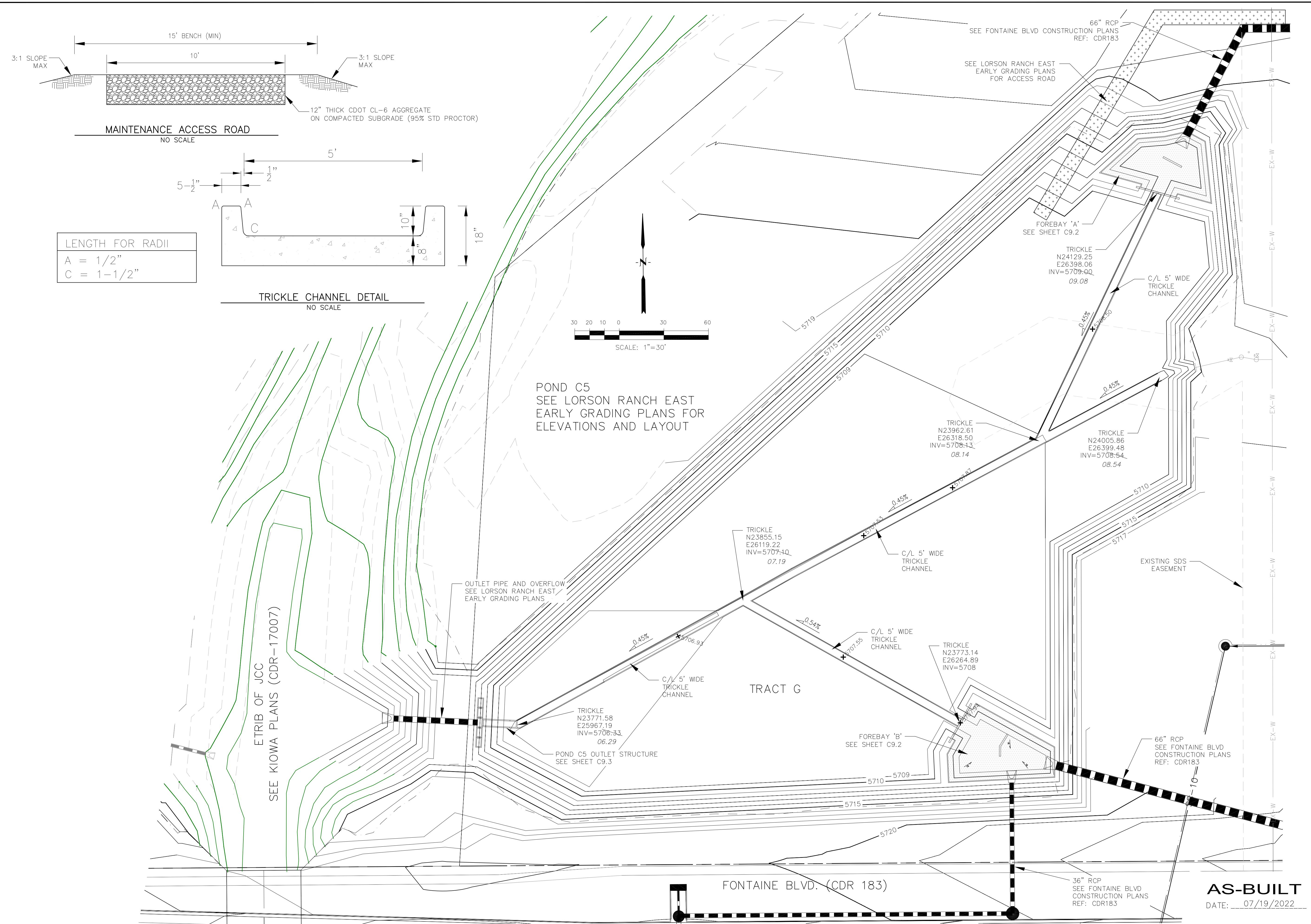


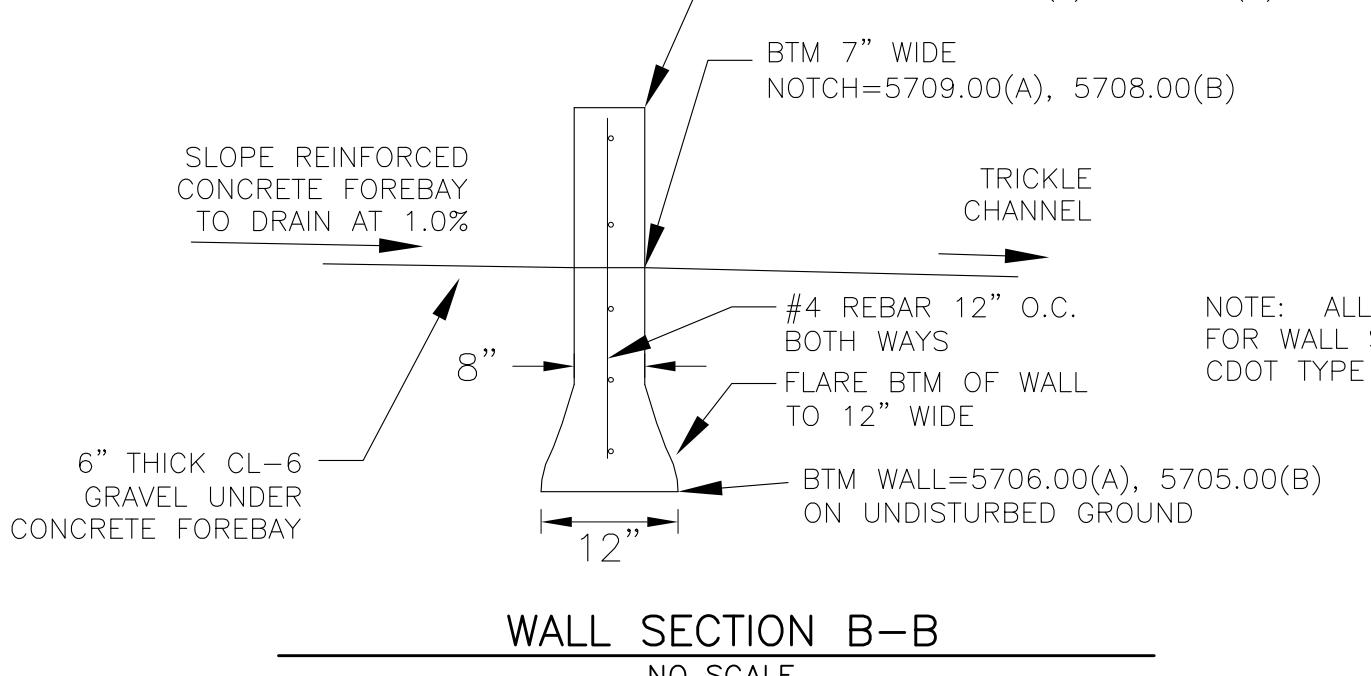
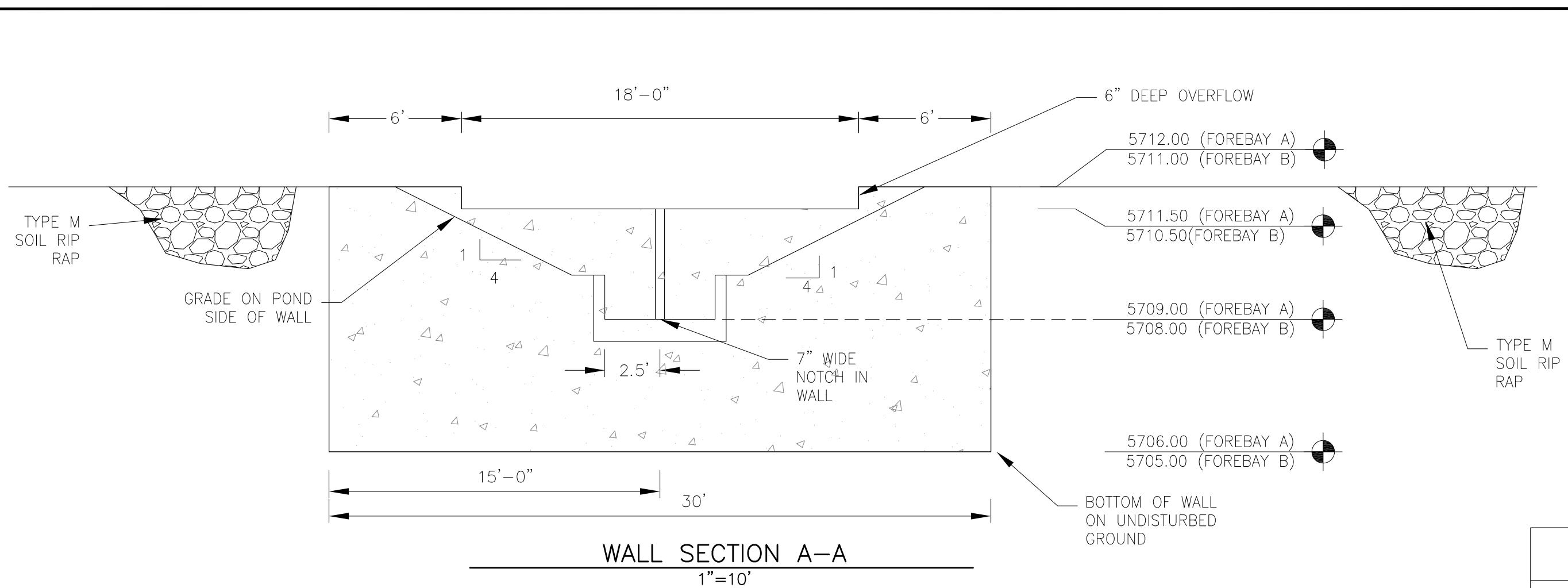
DATE:  
JUNE 12, 2018

PROJECT NO.  
100.042

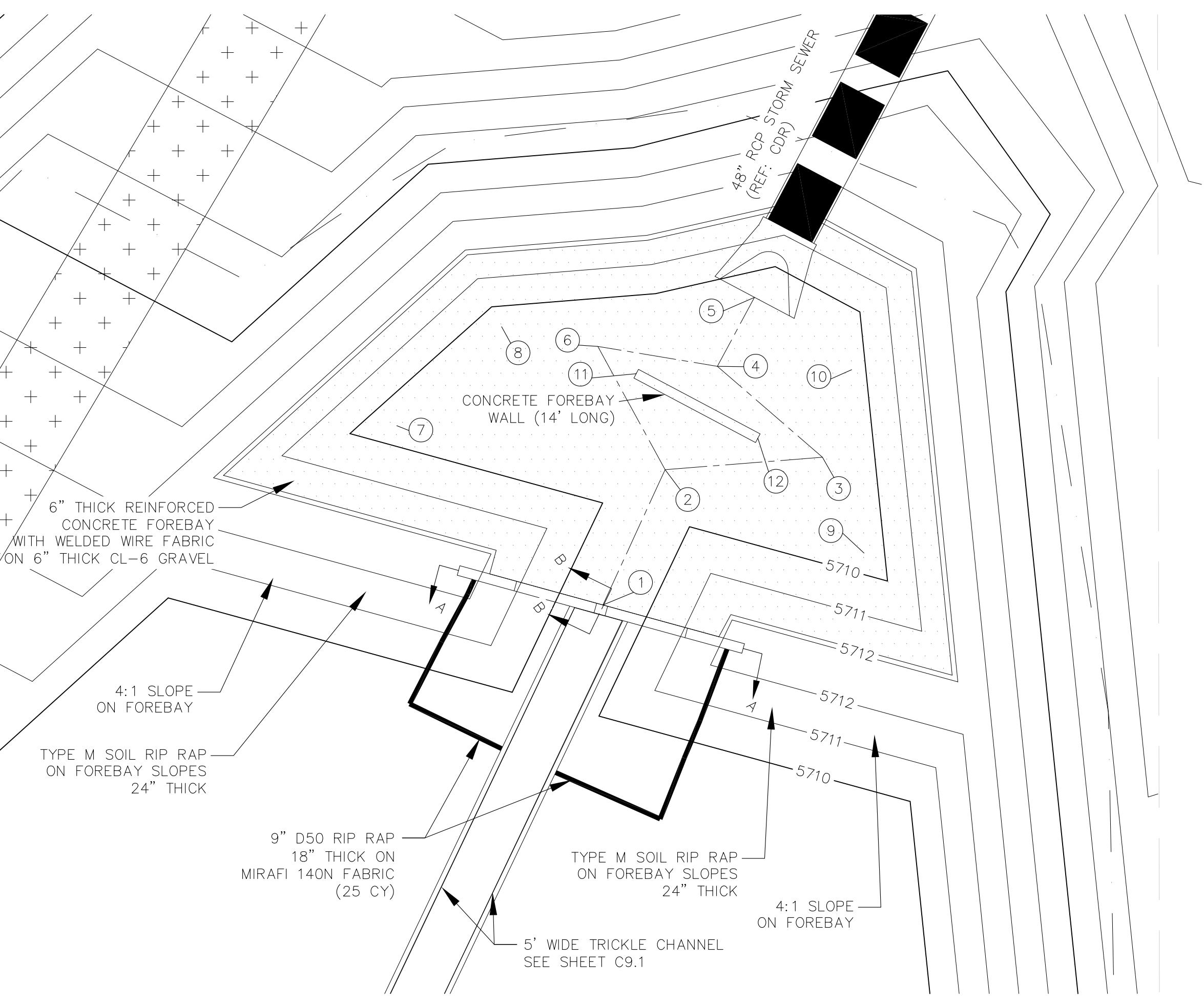
SHEET NUMBER  
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TOTAL SHEETS: 45

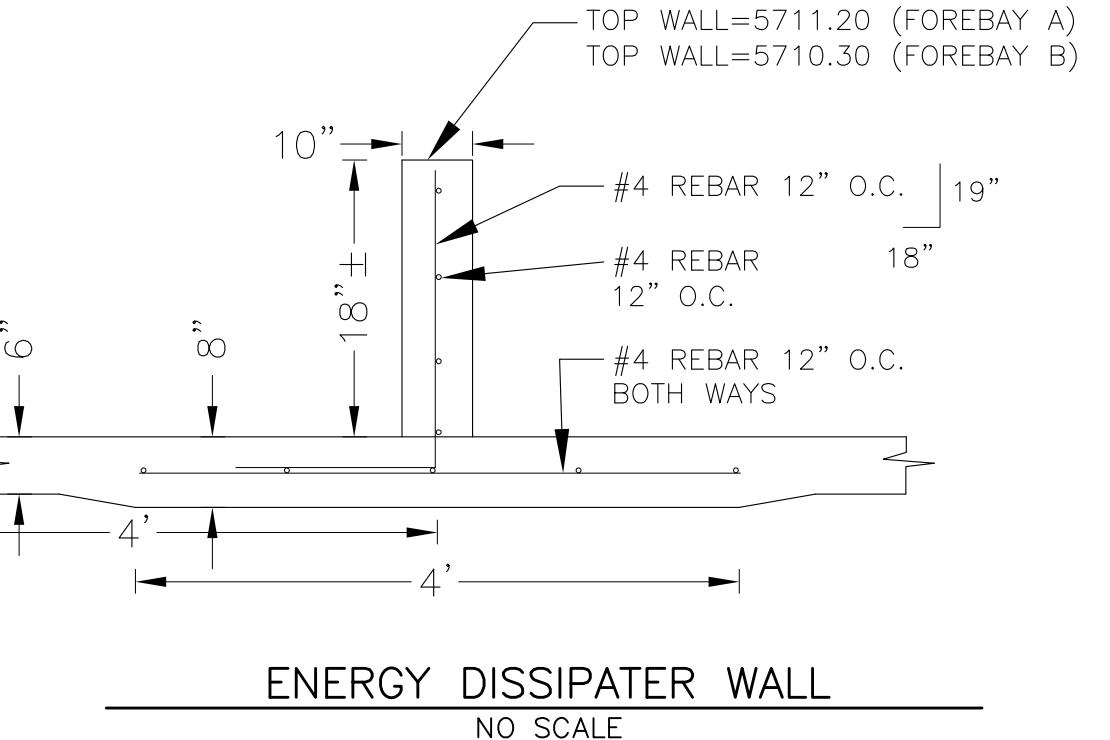
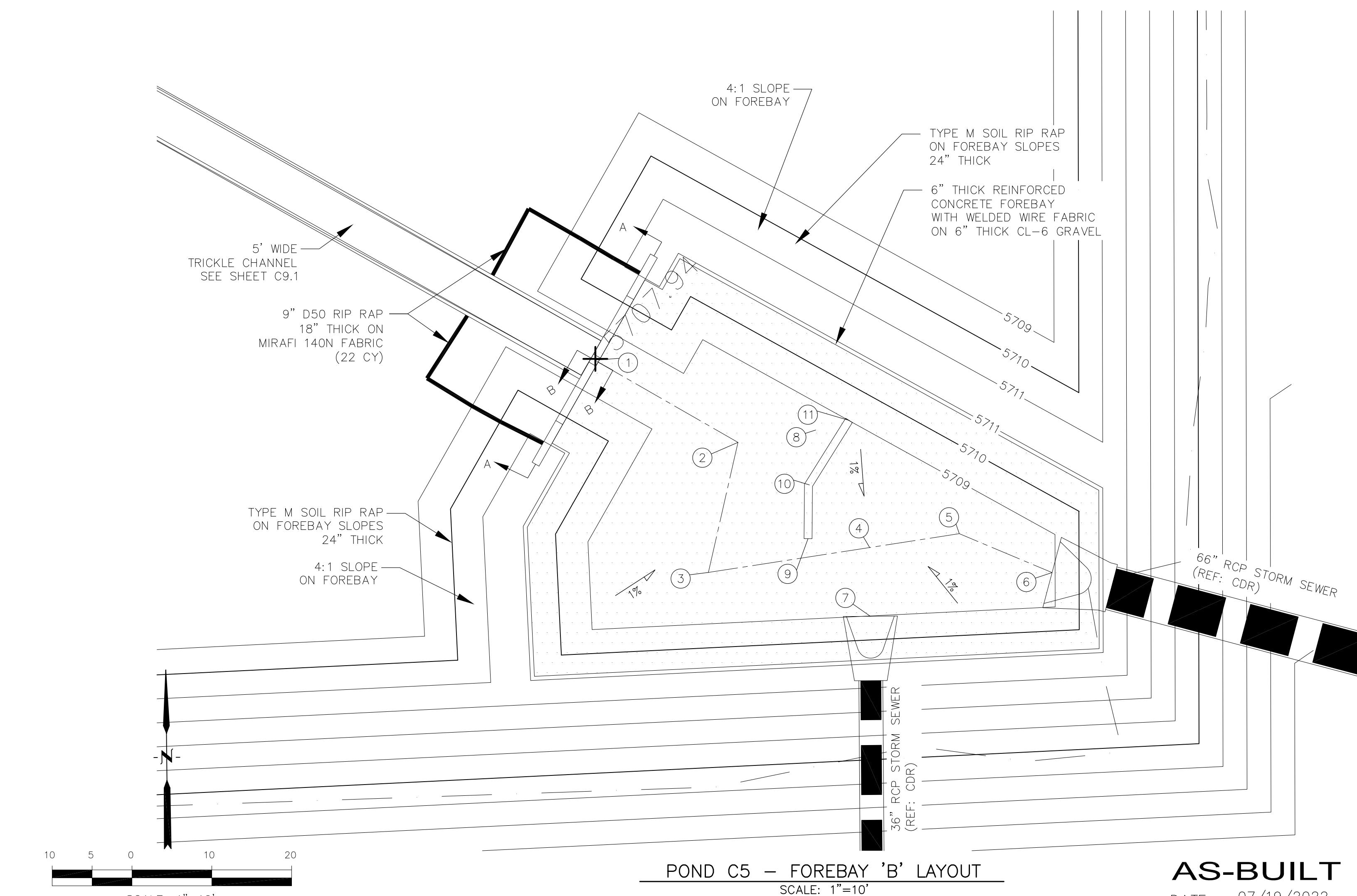




WALL SECTION B-B  
NO SCALE



POND C5 - FOREBAY 'A' LAYOUT  
SCALE: 1"=10'



NOTE: ALL CONCRETE FOR WALL SHALL BE CDOT TYPE D

ENERGY DISSIPATER WALL  
NO SCALE

POINT TABLE (FOREBAY A)				
NUMBER	NORTHING	EASTING	ELEVATION	NOTES
1	24130.17	26398.49	5709.10	FOREBAY BOTTOM
2	24143.93	26404.81	5709.25	FOREBAY BOTTOM
3	24145.19	26420.81	5709.41	FOREBAY BOTTOM
4	24154.44	26410.13	5709.55	FOREBAY BOTTOM
5	24161.46	26413.90	5709.70	FOREBAY BOTTOM
6	24156.46	26397.97	5709.41	FOREBAY BOTTOM
7	24148.40	26377.53	5709.60	FOREBAY BOTTOM
8	24158.47	26388.19	5709.55	FOREBAY BOTTOM
9	24135.44	26425.04	5709.60	FOREBAY BOTTOM
10	24154.10	26423.78	5709.70	FOREBAY BOTTOM
11	24153.68	26401.87	5709.70	ENERGY DISSIPATER WALL
12	24147.13	26414.24	5709.70	ENERGY DISSIPATER WALL

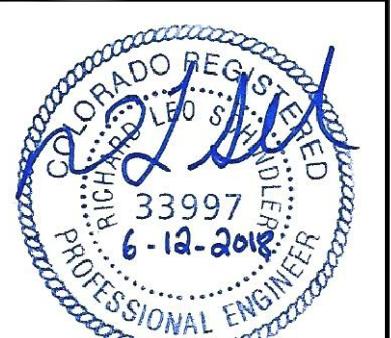
POINT TABLE (FOREBAY B)				
NUMBER	NORTHING	EASTING	ELEVATION	NOTES
1	23772.63	26265.75	5708.10	FOREBAY BOTTOM
2	23762.58	26283.27	5708.30	FOREBAY BOTTOM
3	23746.29	26279.56	5708.50	FOREBAY BOTTOM
4	23749.41	26299.77	5708.70	FOREBAY BOTTOM
5	23751.14	26310.99	5708.70	FOREBAY BOTTOM
6	23746.27	26322.62	5708.95	FOREBAY BOTTOM
7	23740.80	26299.85	5708.80	FOREBAY BOTTOM
8	23765.15	26295.04	5708.40	FOREBAY BOTTOM
9	23750.52	26292.05	5708.80	ENERGY DISSIPATER WALL
10	23757.25	26292.22		ENERGY DISSIPATER WALL
11	23765.34	26297.18		ENERGY DISSIPATER WALL

**CORE**  
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PREPARED FOR:  
PROJECT:  
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FILING NO. 1  
FONTEA BLVD - OLD GLORY DR  
COLORADO SPRINGS, COLORADO  
DRAWN: RLS  
DESIGNED: RLS  
CHECKED: RLS

**POND C5**  
**FOREBAY 'A' AND 'B'  
LAYOUT AND DETAILS**

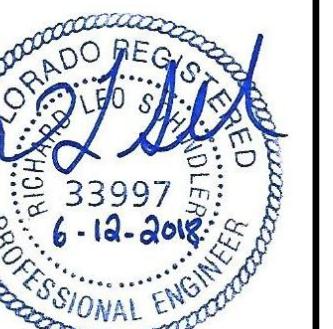


DATE:  
JUNE 12, 2018  
PROJECT NO.  
100.042  
SHEET NUMBER  
C9.2  
TOTAL SHEETS: 45  
DATE: 07/19/2022

**CORE**  
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DATE: \_\_\_\_\_  
NO. \_\_\_\_\_  
PROJECT: RLS  
DRAWN: RLS  
DESIGNED: RLS  
CHECKED: RLS  
PREPARED FOR: LORSON, LLC  
FILING NO. 1  
FONTEINE BLVD.-OLD GLORY DR  
COLORADO SPRINGS, COLORADO  
CONTACT: JEFF MARK

**LORSON RANCH EAST FILING NO. 1**  
**FULL SPECTRUM POND C5**  
**OUTLET STRUCTURE**

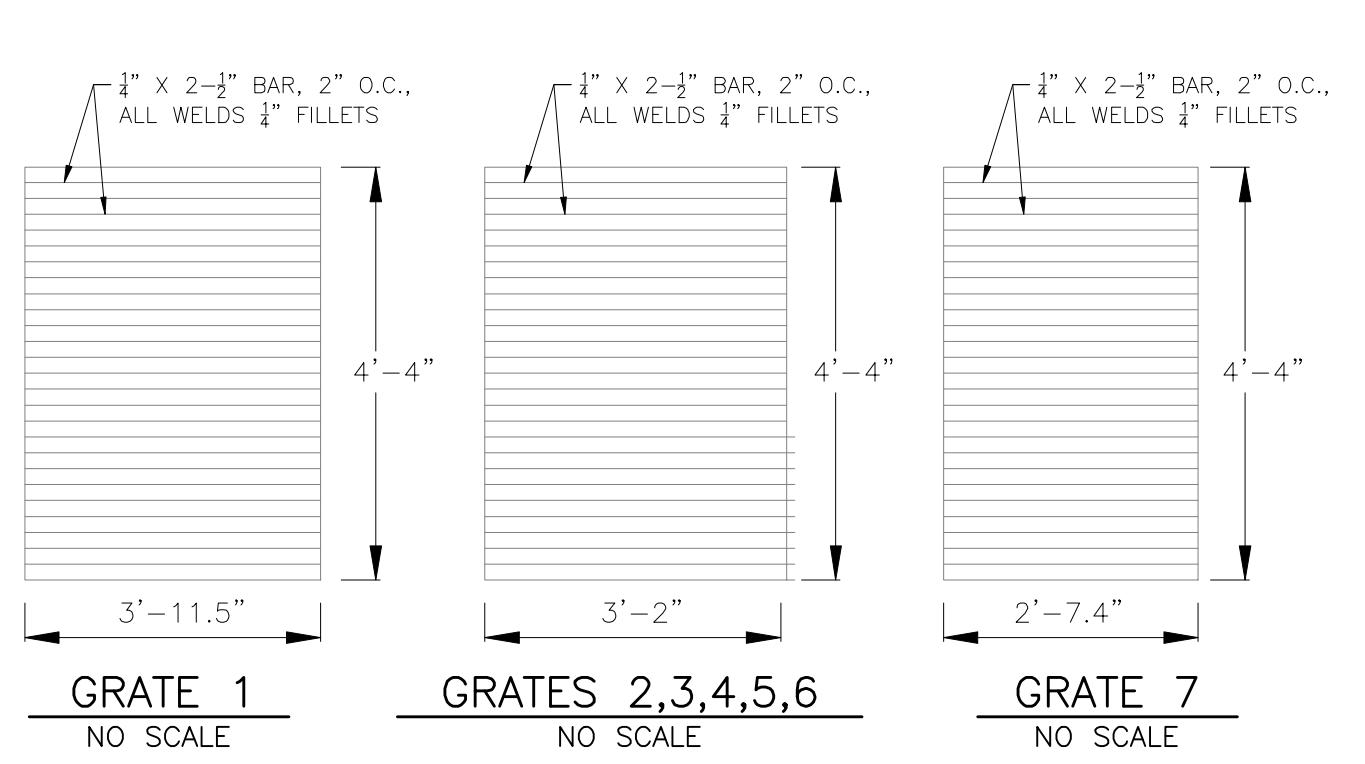


DATE:  
JUNE 12, 2018

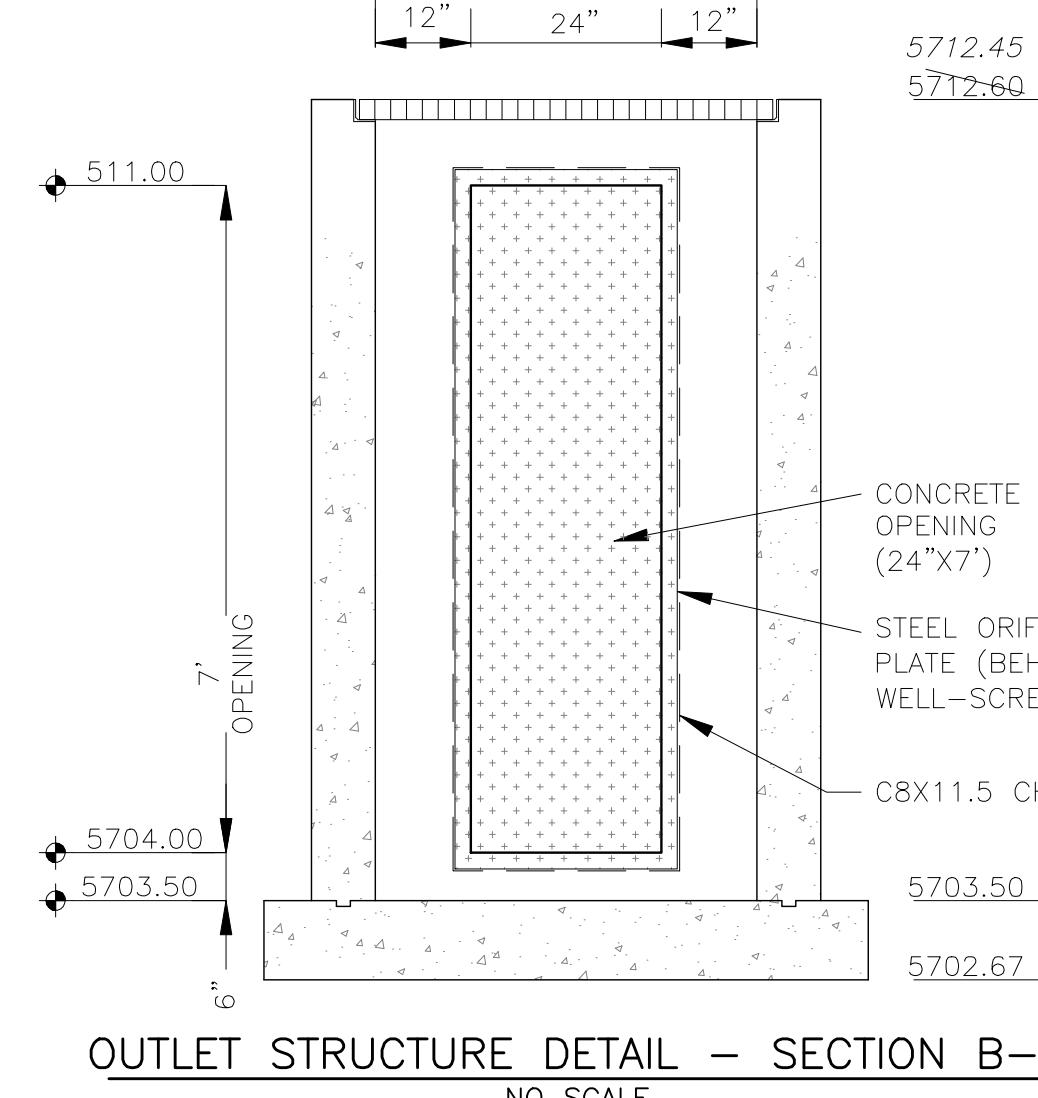
PROJECT NO.  
100.042

SHEET NUMBER  
C9.3  
TOTAL SHEETS: 45  
DATE: 07/19/2022

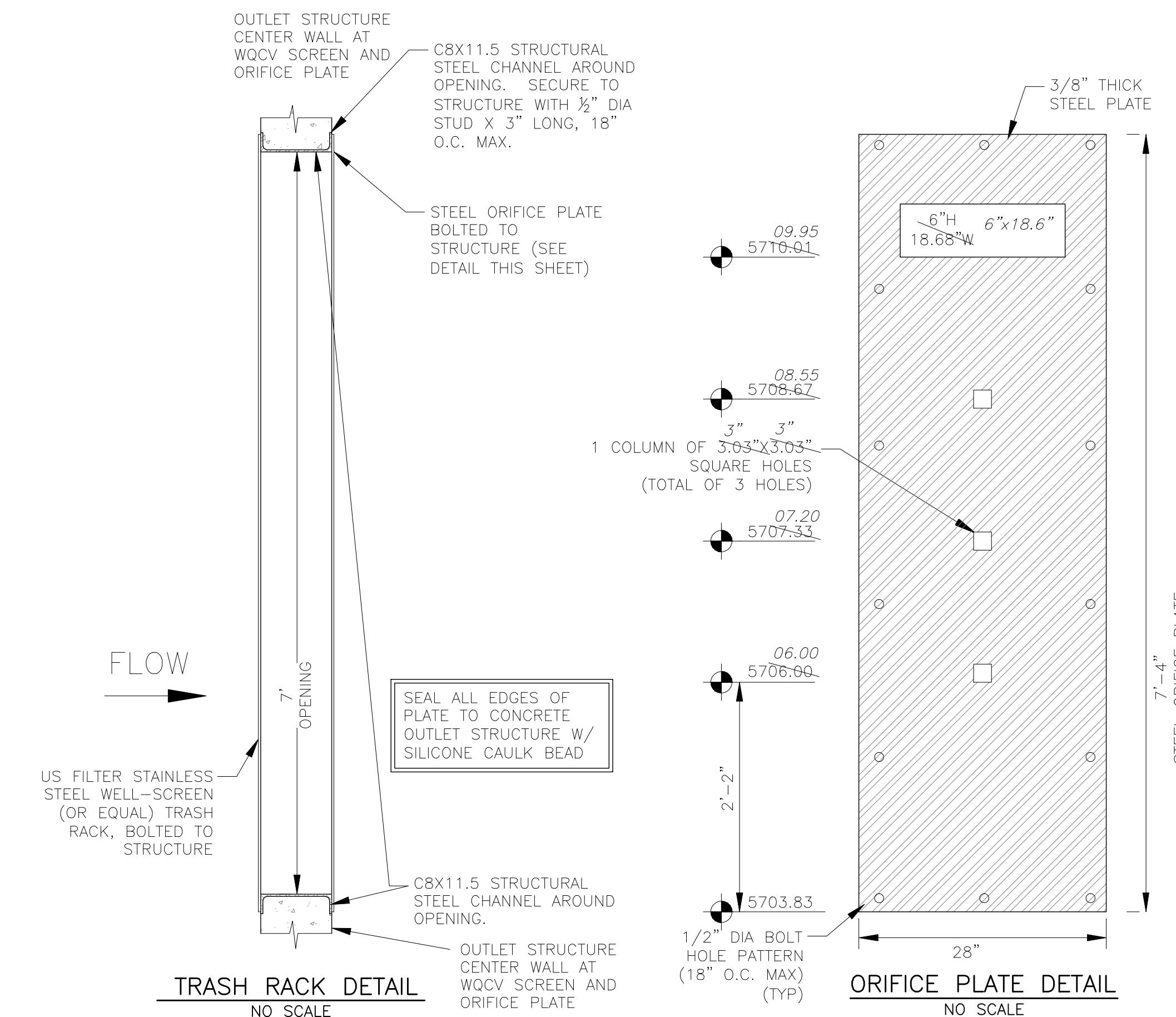
NOTE:  
AFTER CONCRETE STRUCTURE HAS BEEN POURED  
ALL GRATE DIMENSIONS SHALL BE FIELD VERIFIED  
PRIOR TO GRATE CONSTRUCTION



GRATE 1 NO SCALE      GRATES 2,3,4,5,6 NO SCALE      GRATE 7 NO SCALE



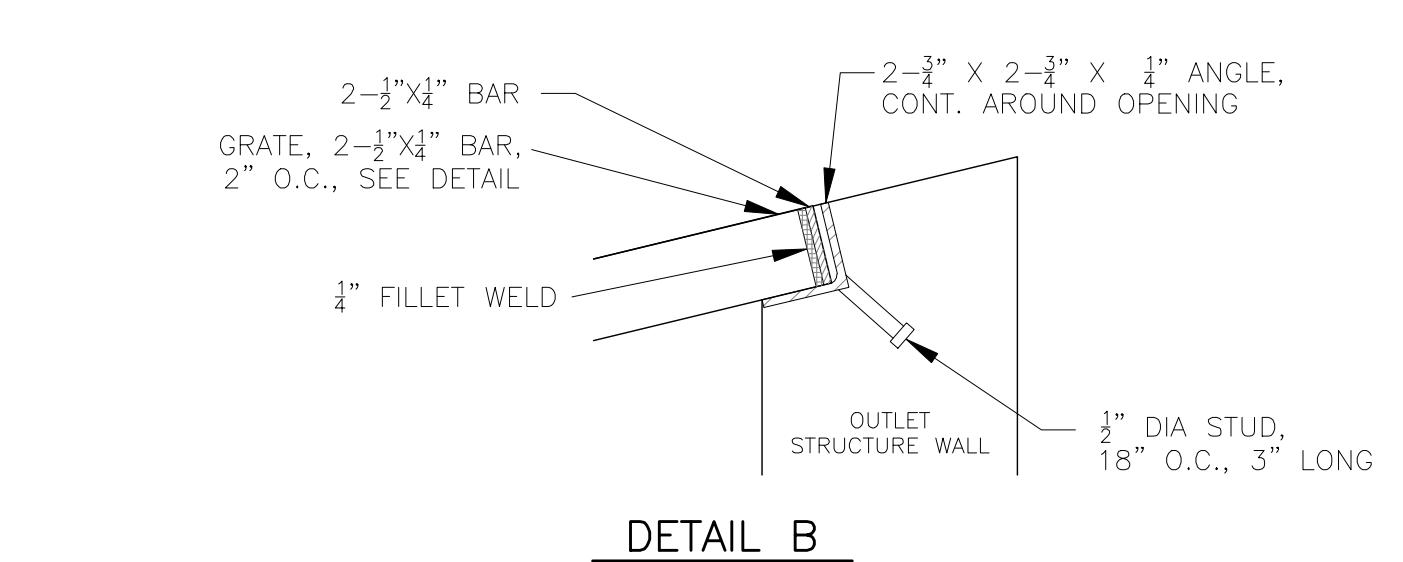
OUTLET STRUCTURE DETAIL - SECTION B-B  
NO SCALE



TRASH RACK DETAIL  
NO SCALE

OUTLET STRUCTURE CENTER WALL AT WQCV SCREEN AND ORIFICE PLATE

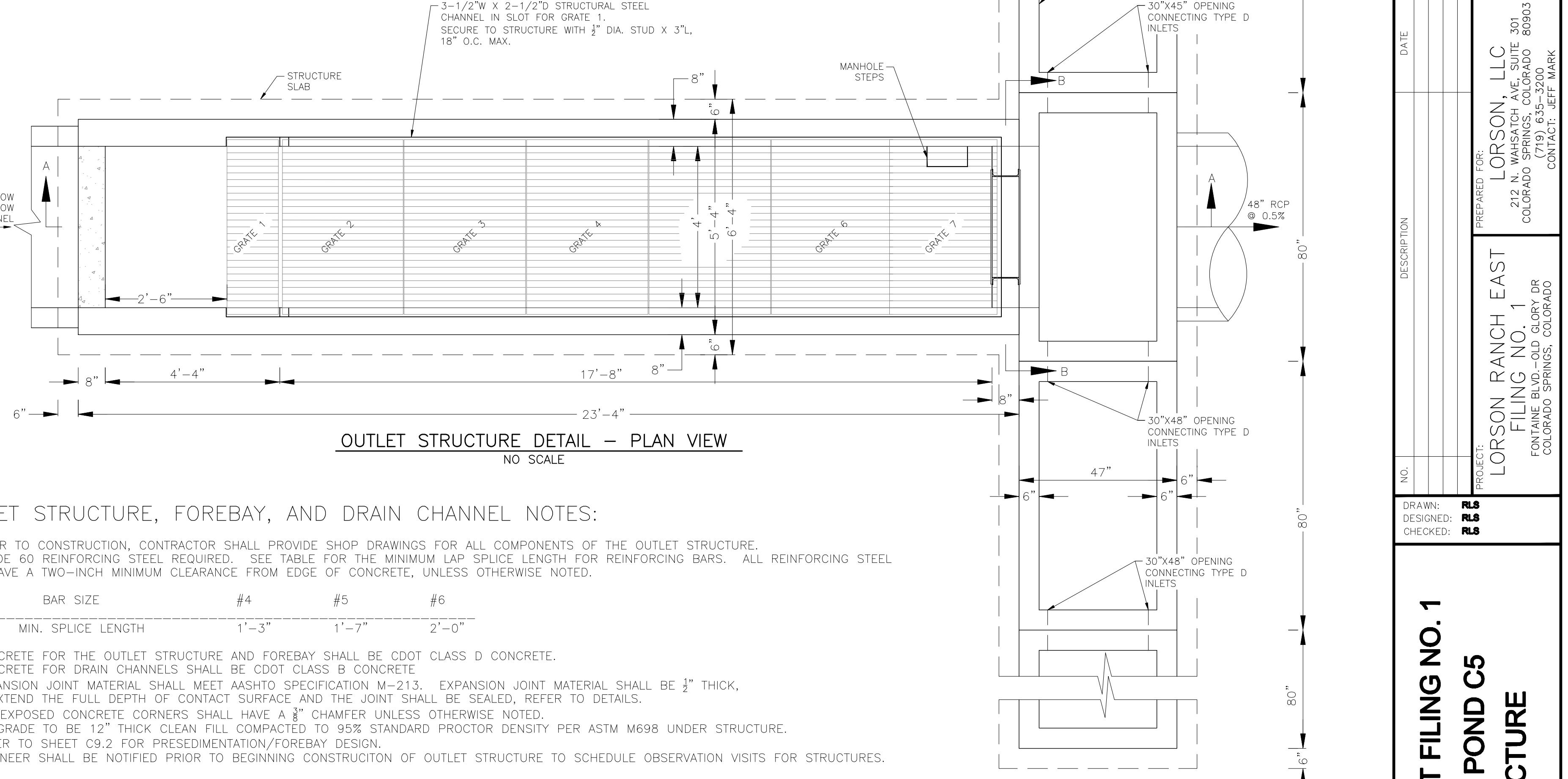
ORIFICE PLATE DETAIL  
NO SCALE



DETAIL B  
NO SCALE

WQCV WELL-SCREEN NOTES:

- Well-Screen shall be stainless steel and attached by stainless steel bolts along edge of the mounting frame.
- WQCV Well Screen
  - Type of Screen: Stainless steel #93 Vee Wire (Johnson Vee Wire (tm) Stainless Steel Screen or equivalent with 60% open area)
  - Screen slot opening dimension: 0.139"
  - Type and Size of Support Rod: TE 0.074"X0.50"
  - Spacing of Support Rod (O.C.): 1.0 Inch
  - Total Screen Thickness: 0.655"
  - Carbon Steel Holding Frame Type: 3/4" x 1.0" angle



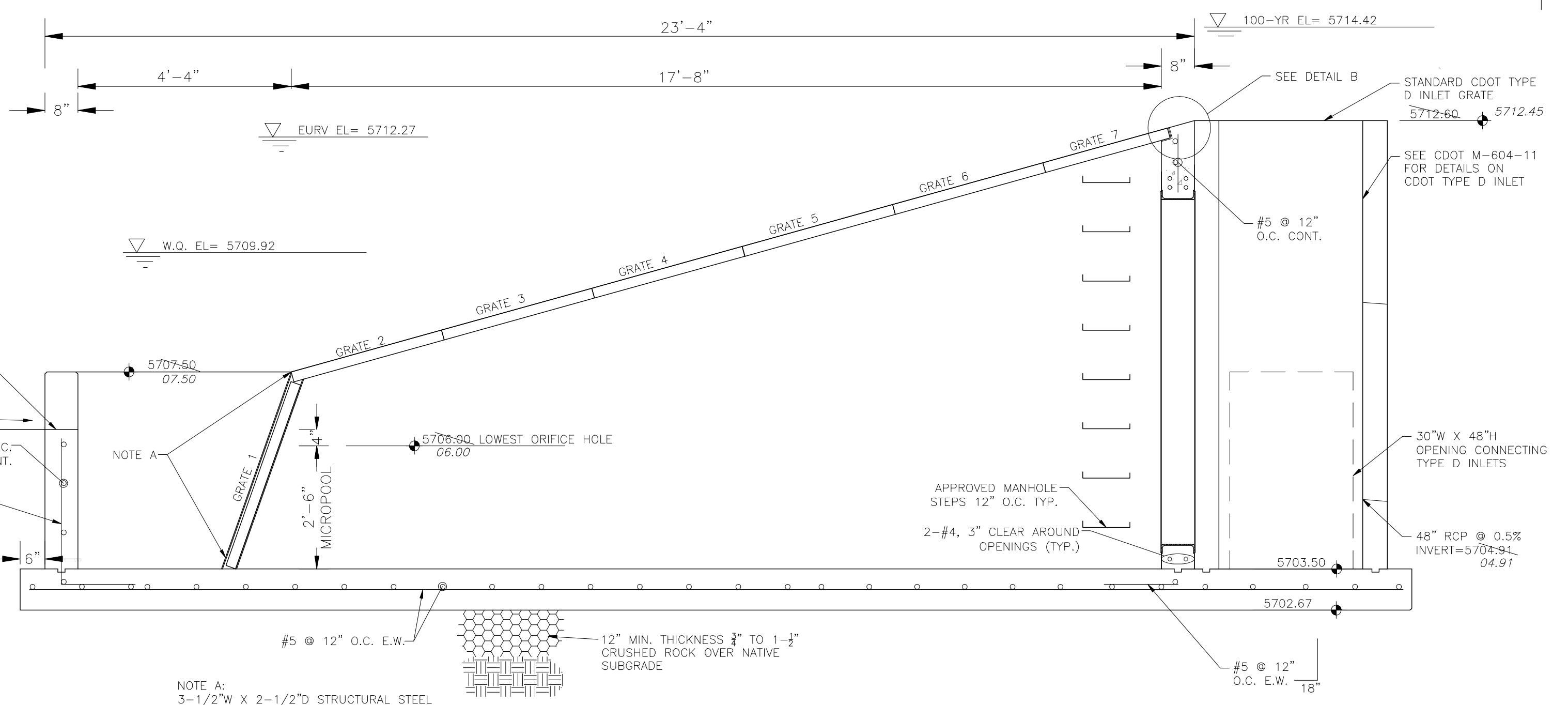
OUTLET STRUCTURE DETAIL - PLAN VIEW  
NO SCALE

OUTLET STRUCTURE, FOREBAY, AND DRAIN CHANNEL NOTES:

- PRIOR TO CONSTRUCTION, CONTRACTOR SHALL PROVIDE SHOP DRAWINGS FOR ALL COMPONENTS OF THE OUTLET STRUCTURE.
- GRADE 60 REINFORCING STEEL REQUIRED. SEE TABLE FOR THE MINIMUM LAP SPLICE LENGTH FOR REINFORCING BARS. ALL REINFORCING STEEL SHALL HAVE A TWO-INCH MINIMUM CLEARANCE FROM EDGE OF CONCRETE, UNLESS OTHERWISE NOTED.

BAR SIZE	#4	#5	#6
	MIN. SPLICE LENGTH	1'-3"	1'-7"

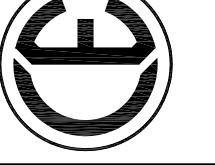
- CONCRETE FOR THE OUTLET STRUCTURE AND FOREBAY SHALL BE CDOT CLASS D CONCRETE.
- CONCRETE FOR DRAIN CHANNELS SHALL BE CDOT CLASS B CONCRETE
- EXPANSION JOINT MATERIAL SHALL MEET AASHTO SPECIFICATION M-213. EXPANSION JOINT MATERIAL SHALL BE  $\frac{1}{2}$ " THICK, SHALL EXTEND THE FULL DEPTH OF CONTACT SURFACE AND THE JOINT SHALL BE SEALED, REFER TO DETAILS.
- ALL EXPOSED CONCRETE CORNERS SHALL HAVE A  $\frac{1}{8}$ " CHAMFER UNLESS OTHERWISE NOTED.
- SUBGRADE TO BE 12" THICK CLEAN FILL COMPACTED TO 95% STANDARD PROCTOR DENSITY PER ASTM M698 UNDER STRUCTURE.
- REFER TO SHEET C9.2 FOR PRESEDIMENTATION/FOREBAY DESIGN.
- ENGINEER SHALL BE NOTIFIED PRIOR TO BEGINNING CONSTRUCTION OF OUTLET STRUCTURE TO SCHEDULE OBSERVATION VISITS FOR STRUCTURES.



OUTLET STRUCTURE DETAIL - SECTION A-A  
NO SCALE

AS-BUILT  
DATE: 07/19/2022

SHEET NUMBER  
C9.3  
TOTAL SHEETS: 45



PROJECT	NO.	DESCRIPTION	DATE
LORSON RANCH EAST FONTAINE BLVD. - EAST TRIBUTARY EL PASO COUNTY, COLORADO			

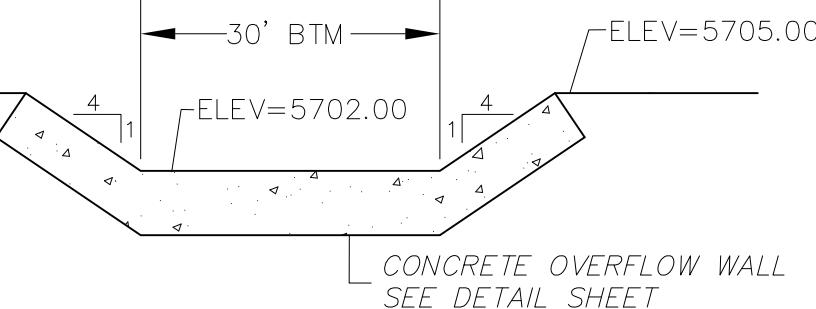
DRAWN: RLS	DESIGNED: RLS	CHECKED: RLS
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FULL SPECTRUM DETENTION POND D2  
LORSON RANCH EAST

DATE: DECEMBER 18, 2017  
PROJECT NO. 100.040  
SHEET NUMBER C4.10  
TOTAL SHEETS: 28  
SCALE: 1"=30'

**AS-BUILT**

DATE: 07/19/2022



POND D2 EMERGENCY  
OVERFLOW PROFILE

SCALE: NTS

NOTE:

1. POND SLOPES SHALL BE 3:1 UNLESS OTHERWISE NOTED
2. STRAW ECB SHALL BE PLACED ON ALL POND SIDE SLOPES UNLESS NOTED OTHERWISE



EAST TRIBUTARY

AMERICAN EXCELSIOR  
"RECYLCEX" TRM  
FILL CELLS WITH  
TOPSOIL

POND D2  
EMERGENCY OVERFLOW  
BTM=5702.00, TOP=5705.00  
WIDTH=30', 4:1 SIDE SLOPES  
FLOW DEPTH=1.64'; Q100=277CFS

CONCRETE OVERFLOW  
WALL. SEE SHEET C4.7

48" RCP END SECTION  
INV 5694.40 5694.00  
N 21950.12  
E 25316.82

102LF OF 48" RCP  
INCLUDING END SECTION

12" D50 RIP RAP,  
24" THICK  
ON MIRAFI 140N FABRIC

LIMITS OF  
GROUTED RIP RAP

4:1 SLOPE

3:1 SLOPE

3:1 SLOPE

4:1 SLOPE

3:1 SLOPE

3:1 SLOPE

3:1 SLOPE

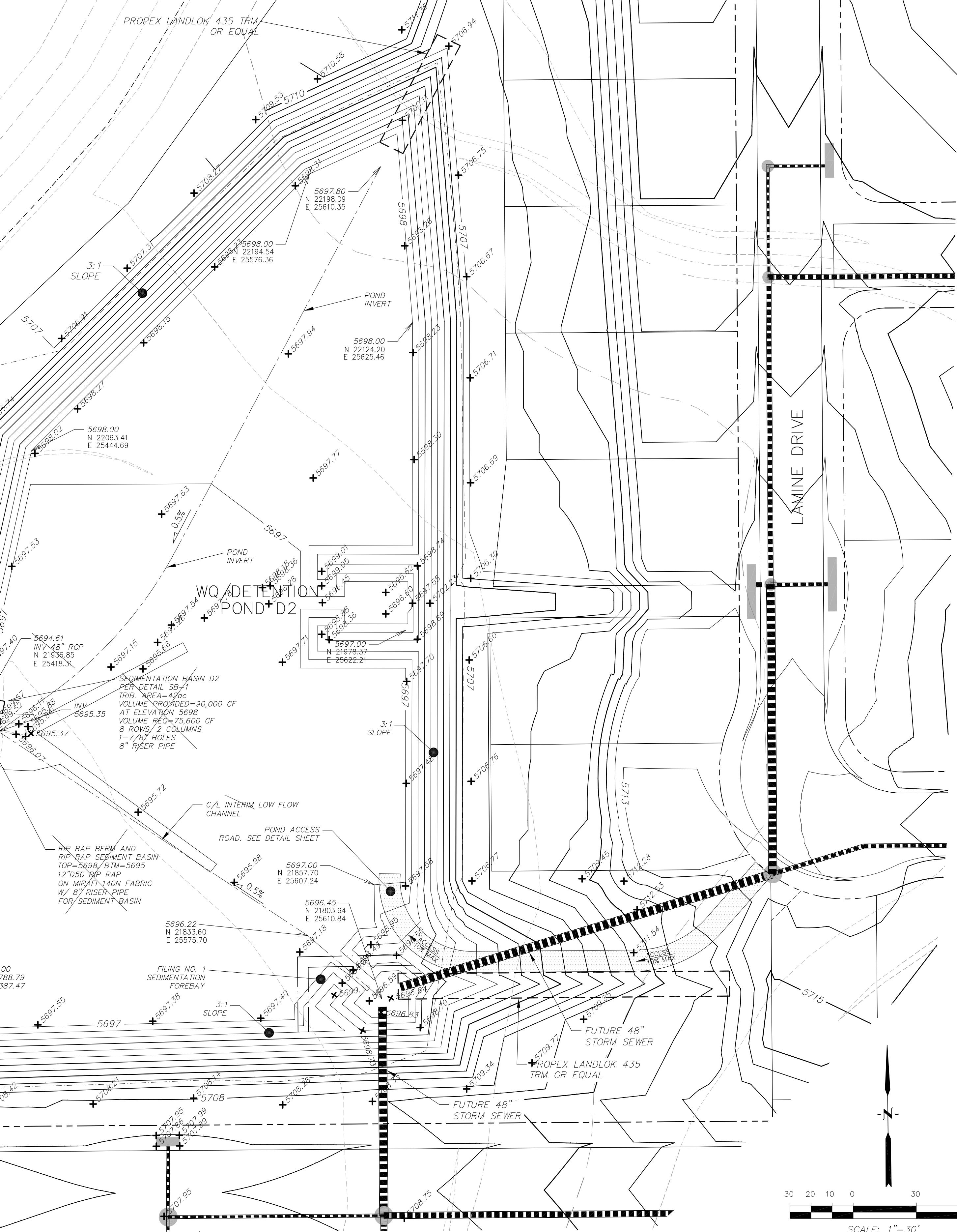
3:1 SLOPE

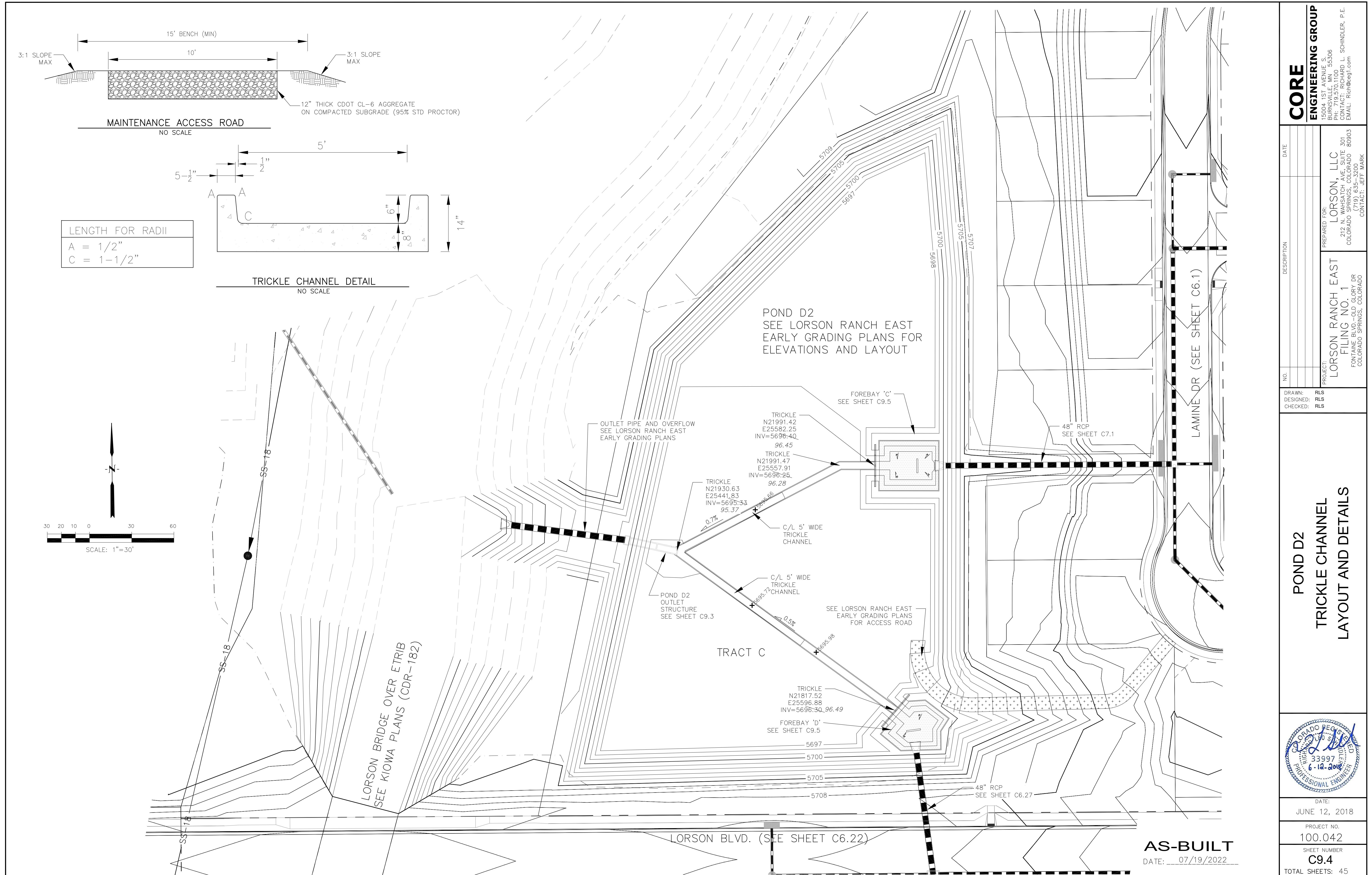
3:1 SLOPE

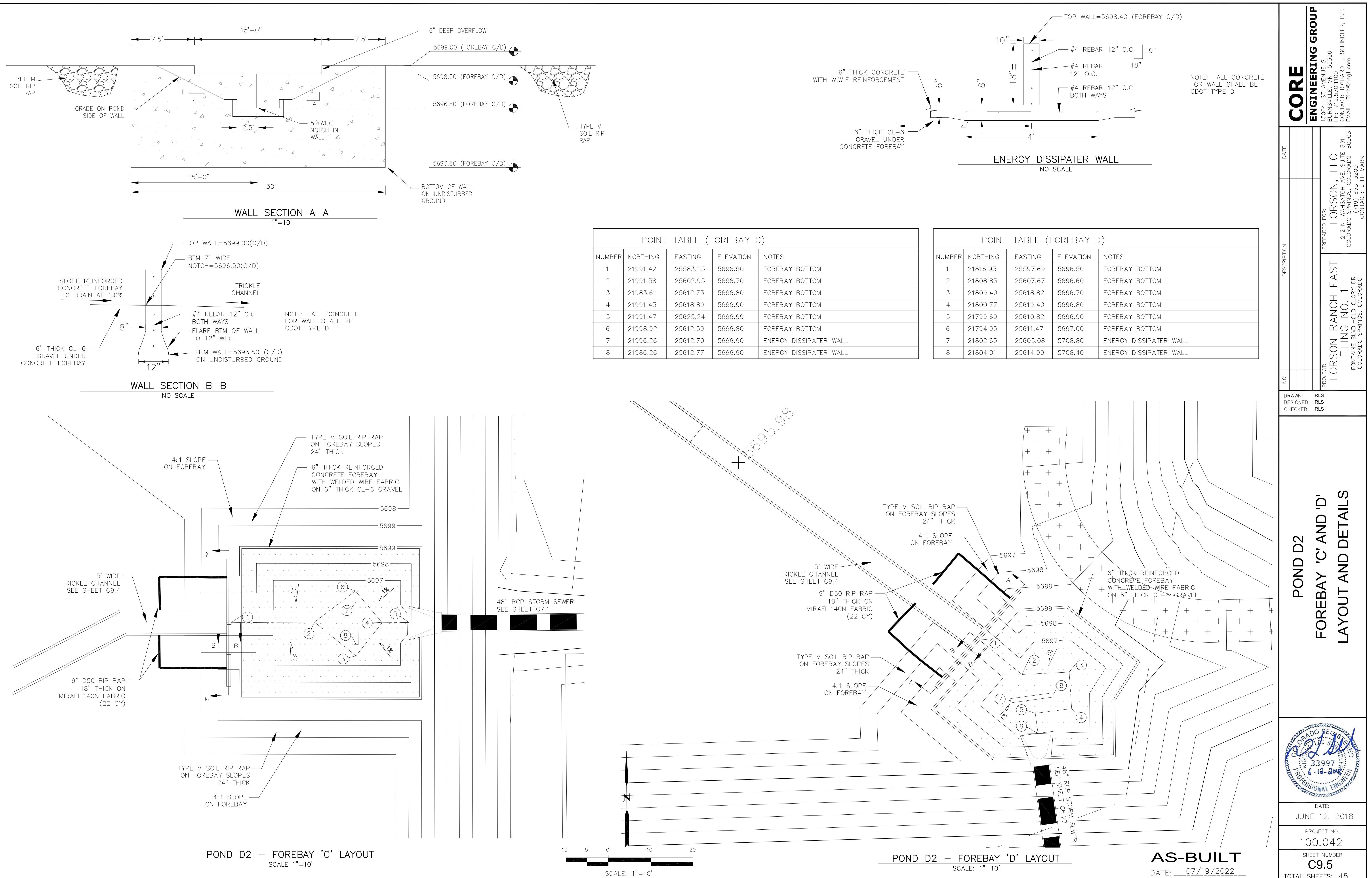
3:1 SLOPE

LORSON BOULEVARD

LORSON BOULEVARD



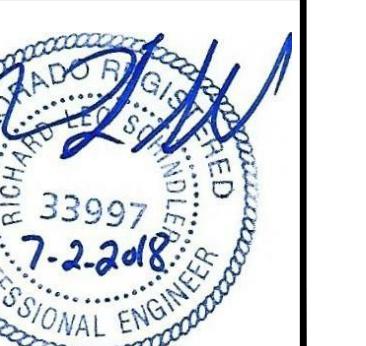




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**LORSON RANCH EAST FILING NO. 1**  
**FULL SPECTRUM POND D2**  
**OUTLET STRUCTURE**



DATE:  
JUNE 12, 2018

PROJECT NO.  
100.042

SHEET NUMBER  
C9.6

TOTAL SHEETS: 45

**AS-BUILT**  
DATE: 07/19/2022

