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# Hannah Ridge at Feathergrass <br> Filings 3 and 4 <br> (LSC \#174210) Add filifing number(s) <br> August 9, 2017 

## Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


## Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

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August 9, 2017
Feathergrass Investments, LLC
c/o David R. Gorman, P.E.
M.V.E. Inc.

1903 Lelaray Street, Suite 200
Colorado Springs, CO 80909

RE: Hannah Ridge at Feathergrass<br>Filings 3 and 4<br>El Paso County, CO<br>Traffic Impact Analysis<br>LSC \#174210

Dear Mr. Gorman:
In response to your request, LSC Transportation Consultants, Inc. has prepared this traffic impact analysis for Filings 3 and 4 of the Hannah Ridge at Feathergrass development located north and south of Constitution Avenue and west of Marksheffel Road in El Paso County, Colorado. The site location is shown in Figure 1. A previous traffic impact study for this development was completed by LSC dated October 23, 2007.

## REPORT CONTENTS

This report is being prepared as part of a submittal to El Paso County. It identifies the traffic impacts of this development. The report contains the following:

- The traffic count data and street conditions.
- Projections of short-term (2020) and long-term (2040) baseline/background traffic volumes.
- The projected average weekday and peak-hour vehicle-trips to be generated by the site by phase.
- The assignment of the site's projected traffic volumes to the key area streets and intersections for the short and long term and the resulting total traffic volumes for the short and long term.
- The resulting traffic impacts including level of service analysis at key intersections and average daily traffic volumes on key street sections in the vicinity of the site.
- Recommended lane configuration for Constitution Avenue.
- Recommendations for contributions for a future traffic signal at Hannah Ridge/Constitution.


## LAND USE AND ACCESS

The Hannah Ridge at Feathergrass development is planned to contain a mix of residential and commercial uses. The proposed preliminary plan is attached. The preliminary plan also shows the proposed phasing of the development.

The Hannah Ridge at Feathergrass development is planned to contain 353 lots for single-family homes. The 2007 study assumed the site would be developed with 354 lots for single-family homes. Since completion of that report 45 lots have been platted as part of Filing No. 1 and 39 lots have been platted as Filing No. 2. At the time traffic counts were conducted 44 homes had been constructed within Filing No. 1 and five homes had been constructed within Filing 2.

Thirty-nine lots for single-family homes are currently proposed to be platted as Filing No. 3. With this filing, Hunter Jumper Drive and Winslow Park Drive would be extended east to Grand Prix Court. Shawnee Drive would not be extended north of Constitution as part of this filing.

Fifty-two lots for single-family homes are currently proposed to be platted as Filing No. 4. With this filing Shawnee Drive would be extended north of Constitution Avenue. The intersection of Constitution/ Shawnee is planned to be restricted to three-quarter movements (left-in/right-in/right-out only) with Filing No. 4. Center raised median modification to a standard three-quarter channelizing median would also restrict the south side of Constitution (Shawnee) to a three-quarter movement intersection. The traffic volume projections in this report reflect this.

The remaining 178 lots for single-family homes are proposed for future Filings 5 through 8. Future Filing 9 is located west of Hannah Ridge Drive between Constitution Avenue and Hunter Jumper Drive. The 2007 traffic impact study assumed this area would be developed with about 205 multi-family dwelling units. Future Filing 10 is located in three tracts on the northwest, northeast, and southeast corners of the intersection of Akers/Constitution. The 2007 traffic impact study assumed these parcels would be developed with about 400,000 square feet of retail floor space including a 70,0000 supermarket. Access to Filing 10 is proposed via Akers Drive. An additional right-in/right-out-only access is proposed on the north side of Constitution east of Akers Drive and a right-in-only access on the south side on Constitution. No changes are currently planned for Filings 9 and 10.

## ROADWAY AND TRAFFIC CONDITIONS

## Area Roadways

The area roadways in the site's vicinity are shown on Figure 1 and are described below.

- Constitution Avenue is a Principal Arterial adjacent to the site extending from Paseo Road east to US Highway (US) 24. In the vicinity of the site, Constitution Avenue has two through lanes in each direction plus continuous right-turn lanes and left-turn lanes. The posted speed limit on Constitution Avenue adjacent to the site is 50 miles per hour ( mph ). Constitution Avenue is shown as a future sixlane street on the County Major Transportation Corridor Plan (MTCP).
- Hannah Ridge Drive is a Collector that extends north from just south of Palmer Park Boulevard to just north of Constitution Avenue. The Constitution Avenue/Hannah Ridge Drive intersection is planned to be signalized in the future.
- Akers Drive is a Non-Residential Collector extending between Constitution Avenue and North Carefree Circle west of Marksheffel Road. The posted speed limit on Akers Drive is 35 mph . The Constitution Avenue/Akers Drive intersection is planned to be signalized in the future.


## Existing Traffic Volumes

Figure 2 shows the existing peak-hour traffic volumes and existing lane geometries and traffic controls at the intersections of Constitution Avenue/Shawnee Drive, Constitution Avenue/Hannah Ridge Drive and Constitution Avenue/Akers Drive. The traffic volumes are based on counts by LSC in March and July 2017. The traffic count reports are attached.

## Existing Level of Service

Level of service (LOS) is a quantitative measure of the level of delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A represents control delay of less than 10 seconds for unsignalized and signalized intersections. LOS F represents control delay of more than 50 seconds for unsignalized intersections and more than 80 seconds for signalized intersections. Table 1 shows the level of service delay ranges.

| Intersection Levels of Service Delay Ranges |  |  |
| :---: | :---: | :---: |
|  | Signalized <br> Intersections | Unsignalized <br> Intersections |
|  | Control Delay (seconds per vehicle) |  |
| A | 10 sec or less | 10 sec or less |
| B | $10-20$ sec | $10-15 \mathrm{sec}$ |
| C | $20-35 \mathrm{sec}$ | $15-25 \mathrm{sec}$ |
| D | $35-55 \mathrm{sec}$ | $25-35 \mathrm{sec}$ |
| E | $55-80 \mathrm{sec}$ | $35-50 \mathrm{sec}$ |
| F | 80 sec or more | 50 sec or more |

The intersections of Constitution Avenue/Shawnee Drive, Constitution Avenue/Hannah Ridge Drive, and Constitution Avenue/Akers Drive have been analyzed to determine the existing levels of service based on the unsignalized method of analysis procedures found in the Highway Capacity Manual, 2010 Edition by the Transportation Research Board. The results of the analysis are shown in Figure 2. The level of service reports are attached.

All movements at the intersections of Constitution Avenue/Shawnee Drive and Constitution Avenue/ Akers Drive are currently operating at a satisfactory level of service (LOS D or better) during the peak hours as stop-sign-controlled intersections.

The northbound left-turn movement at the intersection of Constitution Avenue/Hannah Ridge Drive is currently operating at LOS D during the morning peak hour and LOS E during the afternoon peak hour.

## TRIP GENERATION

Estimates of the traffic volumes expected to be generated by the proposed land were made using the nationally published trip generation rates found in Trip Generation, 9th Edition, 2012 by the Institute of Transportation Engineers (ITE). Table 2 shows the trip generation estimates for the Hannah Ridge at Feathergrass development by phase. The trip generation estimate includes vehicle-trips estimated to be generated by homes that have already been constructed in Filing 1. Table 2 also shows the trip generation estimate from the 2007 traffic impact study. Although the anticipated land uses for Phases 9 and 10 have not changed the ITE trip generation rates for "Shopping Center" and "Supermarket" have changed between the $7^{\text {th }}$ edition published in 2003 and the current $9^{\text {th }}$ edition resulting in a slightly lower trip generation estimate for these parcels.

The total number of external new impact vehicle-trips generated by the retail land uses has been reduced to take into account the "pass-by" phenomena. A pass-by trip is made by a motorist who would already be on the adjacent roadways regardless of the proposed development, but who stops in at the site while passing by. The motorist would then continue on his or her way to a final destination in the original direction. The pass-by percentages shown on Table 2 are from the Trip Generation Handbook - An ITE Proposed Recommended Practice, 3rd Edition, 2014 by ITE.

Filing No. 3 of the Hannah Ridge at Feathergrass development is expected to generate about 371 new vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 -hour period. During the morning peak hour, which generally occurs for one hour between 6:30 and 8:30 a.m., about seven vehicles would enter and 22 vehicles would exit the site. During the afternoon peak hour of the adjacent street traffic, which generally occurs for one hour between $4: 15$ and 6:15 p.m., about 25 vehicles would enter and 14 vehicles would exit the site.

Filing No. 4 of the Hannah Ridge at Feathergrass development is expected to generate about 495 new vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 -hour period. During the morning peak hour about 10 vehicles would enter and 29 vehicles would exit the site. During the afternoon peak hour of the adjacent street traffic about 33 vehicles would enter and 19 vehicles would exit the site.

At buildout the site is expected to generate about 17,547 new vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 -hour period. During the morning peak hour about 414 vehicles would enter and 477 vehicles would exit the site. During the afternoon peak hour of the adjacent street traffic about 1,201 vehicles would enter and 1,095 vehicles would exit the site.

## SHORT-TERM BACKGROUND TRAFFIC

Background traffic is the traffic estimated to be on the adjacent roadways and at adjacent intersections without the proposed development's trip generation of site-generated traffic volumes. Background traffic includes increases in the through traffic and the traffic generated by nearby developments, but assumes no additional traffic generated by development within the Hannah Ridge at Feathergrass Sketch Plan
area including the 35 lots for single-family homes within Hannah Ridge at Feathergrass Filings 1 and 2 that have been platted but were not constructed at the time of the traffic counts. Figure 3 shows the background traffic for the short term (year 2020) assuming the intersection of Shawnee/Constitution remains a full-movement intersection. Figure 4 shows the background traffic for the short term (year 2020) assuming the intersection of Shawnee/Constitution is restricted to three-quarter movement (left-in/right-in/right-out only). The short-term background traffic volumes were based on the existing volumes shown in Figure 2 with some growth in existing through volumes on Constitution Avenue.

## 2040 BACKGROUND TRAFFIC

Figure 5 shows the projected background traffic volumes for the year 2040. These volumes are estimates by LSC based on previous work completed in the area including The Sands located east of Marksheffel and north of Constitution Avenue and The Wilshire Subdivision located south and west of the site.

## DIRECTIONAL DISTRIBUTION

The directional distribution of the site-generated traffic volumes on the area roadways is an important factor in determining the site's traffic impacts. Figure 6 shows the directional distribution estimates for the site-generated traffic volumes. The estimates have been based on the following factors: the site's location with respect to the City of Colorado Springs, Peterson Air Force Base to the south, and the developing area to the east; the roadway system serving the site; the site's proposed land uses; and the existing traffic counts. The pass-by trips were assigned based, in large part, on the magnitude and direction of the existing traffic volumes on the adjacent roadways.

## SITE-GENERATED TRAFFIC

The site-generated traffic volumes were calculated by applying the directional distribution percentages (from Figure 6) to the trip generation estimates from Table 2. Figures 7 through 13 show the projected site-generated traffic volumes by filing.

Figure 7 shows the additional traffic volumes estimated to be generated by the 35 homes that had not been constructed within Filing Nos. 1 and 2 at the time traffic counts were conducted. The short-term volumes assume Shawnee Drive has not been extended north of Constitution Avenue. The long-term estimates assume Shawnee Drive has been extended north of Constitution and the intersection of Shawnee/Constitution has been restricted to three-quarter movement only (left-in/right-in/right-out only).

Figure 8 shows traffic volumes estimated to be generated by the 39 homes currently proposed for Filing 3. The short-term volumes assume Shawnee Drive has not been extended north of Constitution Avenue. The long-term estimates assume Shawnee Drive has been extended north of Constitution and the intersection of Shawnee/Constitution has been restricted to three-quarter movement only (left-in/right-in/right-out only).

Figure 9 shows traffic volumes estimated to be generated by the 52 homes currently proposed for Filing 4. The estimates assume Shawnee Drive has been extended north of Constitution and the intersection of Shawnee/Constitution has been restricted to three-quarter movement only (left-in/right-in/right-out only).

Figure 10 shows traffic volumes estimated to be generated by the remaining 178 single-family homes planned for the future Filings 5 through 8. The estimates assume buildout of the Hannah Ridge at Feathergrass street system.

Figures 11 and 12 show the site-generated traffic volumes for the future multi-family development assumed for Filing 9 and the future commercial development assumed for Filing 10. Figure 13 shows site-generated traffic at buildout of the Hannah Ridge at Feathergrass development. These volumes are the sum of the long-term volumes shown in Figures 7 through 12.

## SHORT-TERM TOTAL TRAFFIC

Figure 14 shows the projected short-term total traffic volumes assuming buildout of Filings Nos. 1 to 3 only. These volumes are the sum of the short-term background traffic volumes (from Figure 3) plus the additional traffic projected to be generated by the lots left to be developed within Filings Nos. 1 and 2 (from Figure 7) plus the Filing No. 3-generated short-term generated traffic volumes (from Figure 8). These volumes assume Shawnee Drive has not been extended north of Constitution Avenue.

Figure 15 shows the projected short-term total traffic volumes assuming buildout of Filing Nos. 1 to 4 only. These volumes are the sum of the short-term background traffic volumes assuming the intersection of Shawnee/Constitution has been restricted to three-quarter movement (from Figure 4) plus the additional traffic projected to be generated by the lots left to be developed Filing Nos. 1 and 2 (from Figure 7) plus the Filing No. 3-generated long-term traffic volumes (from Figure 8) plus the Filing No. 4-generated traffic volumes (from Figure 9). These volumes assume Shawnee Drive has been extended north of Constitution Avenue and the intersection of Shawnee/Constitution has been restricted to threequarter movement only (left-in/right-in/right-out only).

## 2040 TOTAL TRAFFIC

Figure 16 shows the projected 2040 total traffic volumes. The 2040 total traffic volumes are the sum of the 2040 background traffic volumes (from Figure 5) plus the buildout site-generated traffic volumes from Figure 13.

## PROJECTED LEVELS OF SERVICE

All of the intersections along Constitution Avenue adjacent to the site were analyzed to determine the projected levels of service for the 2020 and 2040 background and total traffic volumes based on the unsignalized method of analysis procedures from the Highway Capacity Manual, 2010 Edition by the Transportation Research Board and the Synchro signalized intersection procedures. Figures 3, 4, 5, 14, 15 , and 16 show the level of service analysis results. The laneage and traffic control assumed in the analysis are depicted on the figures. The level of service reports are attached.

## Constitution/Akers

All movements at the intersection of Constitution/Akers are projected to continue to operate at a satisfactory level of service (LOS D or better) during the peak hours based on the 2020 total traffic volumes following buildout of Filing No. 3 and following buildout of Filing No. 4 assuming this intersection remains stop-sign controlled. By 2040, this intersection was assumed to be signalized. All
movements are projected to operate at a satisfactory level of service based on the projected 2040 total traffic volumes.

## Constitution/Hannah Ridge

The northbound left-turn movement at the intersection of Constitution/Hannah Ridge is currently operating at LOS E during the afternoon peak hour. By 2020 the southbound left-turn movement is also projected to operate at LOS E during the peak hours assuming no additional development within the Hannah Ridge at Feathergrass Sketch Plan area. Following buildout of Filings 1 to 3 the southbound through movement is projected to operate at LOS E during the peak hours and the northbound left-turn movement is projected to operate at LOS F during the afternoon peak hour.

If the intersection of Shawnee/Constitution is restricted to three-quarter movement (left-in/right-in/rightout only) the northbound left-turn movement at the intersection of Hannah Ridge/Constitution is projected to operate at LOS E during the morning peak hour based on both the short-term background traffic and the short-term total traffic volumes assuming buildout of Filing Nos. 1 through 4. During the afternoon peak hour the northbound left-turn movement is projected to operate at LOS E based on shortterm background traffic and LOS F based on short-term total traffic assuming buildout through Filing 4.

This intersection is planned to be signalized in the future. It is common for the minor/side-street intersection approaches to operate at LOS E or F as the intersection volumes approach the volumes needed to satisfy a traffic signal warrant. As a signalized intersection, all movements are projected to operate at a satisfactory level of service during the peak hours based on the projected 2040 total traffic volumes.

## Constitution/Shawnee

It was assumed that the intersection of Constitution/Shawnee would be restricted to three-quarter movement (left-in/right-in/right-out only) once Shawnee is extended north of Constitution. This is planned to occur with development of Hannah Ridge at Feathergrass Filing 4. Based on the projected 2020 total traffic volumes following buildout of Filing No. 4 and the 2040 total traffic volumes all movements at this intersection are projected to operate at LOS C or better during the peak hours.

## Future Constitution Site Access

All movements at the future right-in/right-out-only access to Constitution Avenue (right-in only on the south side) are projected to operate at LOS C or better during the peak hours based on the projected 2040 total traffic volumes.

## VEHICLE QUEUING ANALYSIS

A queuing analysis was performed using Synchro/SimTraffic to determine if the 300-foot eastbound leftturn lane on Constitution Avenue approaching Hannah Ridge Drive Road will be sufficient to accommodate the projected total traffic volumes. The 2040 total afternoon peak-hour traffic volumes were entered into the Synchro model. The simulation was run five times. The projected eastbound leftturn queue on Constitution Avenue approaching Hannah Ridge Drive is about 85 feet long. The queuing reports are attached.

## TRAFFIC SIGNAL WARRANT ANALYSIS

The intersection of Constitution Avenue and Hannah Ridge Drive was analyzed to determine if a FourHour Vehicular Volume Traffic Signal Warrant threshold would be reached or exceeded based on the projected 2020 total traffic volumes following buildout of Filing Nos. 1 through 3 (shown in Figure 14) and following buildout of Filing 4 (shown in Figure 15). The results of the analysis are shown in Figures 17 and 18.

As shown in Figure 17, following buildout of Filings 1 through 3 the thresholds for a Four-Hour Vehicular Volume Traffic Signal Warrant are not projected to be met for either peak hour. As shown in Figure 18, following buildout of Filings 1 through 4 the thresholds are projected to be just met during the afternoon peak hour but not met during the morning peak hour. This analysis using the peak hours is intended to provide an indication that a warrant may be met or is close to being met. In order for a FourHour Traffic Signal Warrant to be satisfied, the volume threshold would need to be met for two additional hours of the day. For example, the four-hour warrant would be satisfied with the volume thresholds met for one hour in the morning, two hours (instead of the one-hour peak) during the afternoon peak period, and an hour during the mid-afternoon. Based on this analysis and our experience, it is likely that the volume thresholds would not be met or exceeded until additional development occurs south of Constitution Avenue. Therefore, it is expected that a traffic signal would not meet a Four-Hour Volume Warrant in the short term.

## PEDESTRIAN AND BICYCLE IMPACT EVALUATIONS

A school pedestrian route evaluation between the site and both Remington Elementary School and Horizon Middle School is required per ECM criteria B.4.1.C Pedestrian and Bicycle Impact Evaluations. The routes for both schools are established from the Peterson/Constitution intersection south to Horizon M.S. and north to Remington Elementary School. The route for these subdivisions would connect to these established routes via Hannah Ridge Drive and the north side of Constitution Avenue west of Hannah Ridge Drive. The pedestrian route would include the subdivision streets connecting to the intersection of Hunter Jumper Drive/Hannah Ridge Drive, Hannah Ridge Drive between Constitution and Hunter Jumper Drive and the north side of Constitution between Hannah Ridge Drive and Peterson Road. A sidewalk has been constructed adjacent to the Charter school west of the Rock Island Trail right-of-way. A future county-standard concrete sidewalk would be constructed along the north side of Constitution with development of the parcel adjacent to Constitution Avenue on the northwest corner of Hannah Ridge Drive/Constitution Avenue ${ }^{\text {The short section of sidewalk across the Rock Island Trail }}$ right-of-way will presumably be installed in the future with the Rock Island Trail construction.

## ESCROW FOR FUTURE TRAFFIC SIGNAL

An estimate of the fair contribution to the cost of a future traffic signal at the intersection of Constitution Avenue and Hannah Ridge Drive has been calculated for the currently proposed Hannah Ridge at Feathergrass Filing Nos. 3 and 4.

The cost of this signal is approximately $\$ 300,000$. Table 3 shows a projection of the key peak-hour intersection turning movement traffic volumes at the intersection of Hannah Ridge/Constitution by filing. These volumes were used to determine a fair share contribution to this signal for Filing Nos. 3 and 4. As
traffic signal percentages have been based on the minor street approach volumes, only the northbound and southbound left-turn and through movements were included in determining the fair share contribution. The site-generated traffic for Filing 3 would be 1.60 percent of the total. The Filing 3 contribution toward the signal would be $\$ 4,800$ ( 1.60 percent of a $\$ 300,000$ signal cost). The sitegenerated traffic for Filing 4 would be 2.08 percent of the total. The Filing 4 contribution toward the signal would be $\$ 6,420$.

## ROADWAY IMPROVEMENT FEE PROGRAM

This development is subject to fees established by the El Paso County Road Impact Fee Program per El Paso County Resolution Number 12-389. Based on a per-lot up-front building permit fee of $\$ 923$ per dwelling unit, the total building permit fee amount would be $\$ 35,997$ for Filing 3 and $\$ 47,996$ for Filing 4.

## CONCLUSIONS AND RECOMMENDATIONS

## Trip Generation

- Filing 3 of the Feathergrass at Hannah Ridge development is expected to generate about 371 new vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 hour period. During the morning peak hour about seven vehicles would enter and 22 vehicles would exit the site. During the afternoon peak hour of the adjacent street traffic about 25 vehicles would enter and 14 vehicles would exit the site.
- Filing 4 of the Feathergrass at Hannah Ridge development is expected to generate about 495 new vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 hour period. During the morning peak hour about 10 vehicles would enter and 29 vehicles would exit the site. During the afternoon peak hour of the adjacent street traffic about 33 vehicles would enter and 19 vehicles would exit the site.


## Level of Service

- All movements at the intersection of Constitution/Akers are projected to continue to operate at a satisfactory level of service (LOS D or better) during the peak hours based on the 2020 total traffic volumes following buildout of Filing No. 3 and buildout of Filing No. 4 assuming this intersection remains stop-sign controlled. By 2040, this intersection was assumed to be signalized. All movements are projected to operate at a satisfactory level of service based on the projected 2040 total traffic volumes.
- The northbound left-turn movement at the intersection of Constitution/Hannah Ridge is projected to operate at LOS F and southbound left-turn and through movements at the intersection of Constitution/Hannah Ridge are projected to operate at LOS E during the afternoon peak hour based on the projected 2020 total traffic volumes following buildout of Filing No. 4 assuming this intersection remains stop-sign controlled. Nearby signalized intersections at Peterson/Constitution and Marksheffel/Constitution create gaps in east-west through traffic and side street traffic utilizes these gaps to enter the intersection. This intersection is planned to be signalized in the future. It is common for the minor/side-street intersection approaches to operate at LOS E or F as the intersection
volumes approach the volumes needed to satisfy a traffic signal warrant. As a signalized intersection, all movements are projected to operate at a satisfactory level of service during the peak hours based on the projected 2040 total traffic volumes.
- The intersection of Constitution/Shawnee will be restricted to three-quarter movement (left-in/rightin /right-out only) once Shawnee is extended north of Constitution. This is planned to occur with development of Hannah Ridge at Feathergrass Filing No. 4. This report accounts for this change. Based on the projected 2020 total traffic volumes following buildout of Filing No. 4 and the 2040 total traffic volumes, all movements at this intersection are projected to operate at LOS C or better during the peak hours.
- All movements at the future right-in/right-out-only access to Constitution Avenue (right-in only on the south side) are projected to operate at LOS C or better during the peak hours based on the projected 2040 total traffic volumes.


## Recommended Improvements

- Table 4 shows the recommended roadway improvements recommended for the overall Hannah Ridge at Feathergrass sketch plan.

Please contact me if you have any questions or need further assistance.
Sincerely,


## JCH:KDF:bjwb

Enclosures: Table 2-4
Figures 1-18
Hannah Ridge at Feathergrass Preliminary Plan Traffic Count Reports
Level of Service Reports

| Table 2 <br> Trip Generation Estimate <br> Hannah Ridge at Feathergrass Filing Nos. 3 and 4 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Phase | Land <br> Use <br> Code | Land <br> Use <br> Description | Trip Generation Units | Trip Generation Rates ${ }^{(1)}$ |  |  |  |  | Total Trips Generated |  |  |  |  | $\begin{aligned} & \text { Pass-By } \\ & \text { Trips }{ }^{(2)} \\ & \hline \end{aligned}$ | New External Trips Generated <br> Average New Weekday Traffic |
|  |  |  |  | Average Weekday Traffic | Morning Peak Hour |  | Afternoon <br> Peak Hour |  | Average Weekday Traffic | Morning <br> Peak Hour |  | Afternoon <br> Peak Hour |  |  |  |
|  |  |  |  |  | In | Out | In | Out |  | In | Out | In | Out |  |  |
| Trip Generation Estimate Based on the Currently Proposed Sketch Plan |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Existing Filings |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | 210 | Single-Family Detached Housing | $45 \mathrm{DU}{ }^{(3)}$ | 9.52 | 0.19 | 0.56 | 0.63 | 0.37 | 428 | 8 | 25 | 28 | 17 | 0\% | 428 |
| 2 | 210 | Single-Family Detached Housing | 39 DU | 9.52 | 0.19 | 0.56 | 0.63 | 0.37 | 371 | 7 | 22 | 25 | 14 | 0\% | 371 |
|  |  | Total Filings 1-3 | 84 DU |  |  |  |  |  | 799 | 15 | 47 | 53 | 31 |  | 799 |
| Currently Proposed Filings |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | 210 | Single-Family Detached Housing | 39 DU | 9.52 | 0.19 | 0.56 | 0.63 | 0.37 | 371 | 7 | 22 | 25 | 14 | 0\% | 371 |
| 4 | 210 | Single-Family Detached Housing | 52 DU | 9.52 | 0.19 | 0.56 | 0.63 | 0.37 | 495 | 10 | 29 | 33 | 19 | 0\% | 495 |
|  |  | Total Filings 3-4 | 91 DU |  |  |  |  |  | 866 | 17 | 51 | 58 | 33 |  | 866 |
| Future Filings |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | 210 | Single-Family Detached Housing | 39 DU | 9.52 | 0.19 | 0.56 | 0.63 | 0.37 | 371 | 7 | 22 | 25 | 14 | 0\% | 371 |
| 6 | 210 | Single-Family Detached Housing | 16 DU | 9.52 | 0.19 | 0.56 | 0.63 | 0.37 | 152 | 3 | 9 | 10 | 6 | 0\% | 152 |
| 7 | 210 | Single-Family Detached Housing | 82 DU | 9.52 | 0.19 | 0.56 | 0.63 | 0.37 | 781 | 15 | 46 | 52 | 30 | 0\% | 781 |
| 8 | 210 | Single-Family Detached Housing | 41 DU | 9.52 | 0.19 | 0.56 | 0.63 | 0.37 | 390 | 8 | 23 | 26 | 15 | 0\% | 390 |
|  |  | Total Filings 5-8 | 178 DU |  |  |  |  |  | 1,694 | 33 | 100 | 113 | 65 |  | 1,694 |
| 9 | 230 | Residential Condominium/Townhouse | 205 DU | 5.81 | 0.07 | 0.37 | 0.35 | 0.17 | 1,191 | 15 | 75 | 71 | 35 | 0\% | 1,191 |
|  |  | Total Filings 1-9 |  |  |  |  |  |  | 4,550 | 80 | 273 | 295 | 164 |  | 4,550 |
| 10 | 820 | Shopping Center ${ }^{(4)}$ | $330 \mathrm{KSF}^{(5)}$ | 41.80 | 0.56 | 0.34 | 1.82 | 1.93 | 13,795 | 186 | 114 | 601 | 638 | 34\% | 9,105 |
|  |  | Supermarket | 70 KSF | 86.83 | 2.11 | 1.29 | 4.36 | 4.19 | 6,078 | 148 | 90 | 305 | 293 | 36\% | 3,890 |
|  |  | Total Filing 10 |  |  |  |  |  |  | 19,873 | 334 | 204 | 906 | 931 |  | 12,995 |
|  |  | Total Filings 1-10 |  |  |  |  |  |  | 24,423 | 414 | 477 | 1,201 | 1,095 |  | 17,545 |
| Notes: <br> (1) Source: <br> (2) Source: <br> (3) DU = dw <br> (4) Shoppin <br> (5) $\mathrm{KSF}=1$ | Gener <br> Gener <br> g unit <br> nter rat <br> square | ation, 9th Edition, 2012" by the Institute of Transpo ation Handbook - An ITE Proposed Recommended <br> sere based on the ITE best-fit curve formula us feet of floor space | ation Engineers Practice" 3rd E <br> g the square fo | (ITE) <br> tion, 2014 <br> tage of the | ire com | rcial | $\text { a }(400 \mathrm{k}$ |  |  |  |  |  |  |  |  |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Do we have escrow for these already? If not, how do we get up to the required $\$ 300,000$ ?

|  | Table 3Traffic Signal Escrow Calculation TableConstitution Avenue/Hannah Ridge Drive IntersectionHannah Ridge at Feathergrass Filing Nos. 3 and 4 |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Filing | 2040 Minor Approach Volumes (vehicles per hour) |  |  |  |  |  |  |  |  |  |  |  |
|  | AM Peak |  |  |  | PM Peak |  |  |  |  | AM+PM |  |  |
|  | SB LT | SB TH | NB LT | NB TH |  | SB LT | SB TH | NB LT | NB TH | Total |  |  |
| Existing | 3 | 0 | 25 | 1 |  | 3 | 0 | 36 | 3 | 71 | 11.36\% | \} P \% |
| Additional Traffic With Shawnee Restriction | 0 | 0 | 15 | 0 |  | 0 | 0 | 17 | 0 | 32 | 5.12\% | $\} \$ 15,360\}$ |
| Hannah Ridge Fil. 1 \& 2 | 3 | 1 | 0 | 0 |  | 2 | 1 | 0 | 1 | 8 | 1.28\% | \$3,840 |
| Hannah Ridge Fil. 3 | 4 | 1 | 0 | 0 |  | 3 | 1 | 0 | 1 | 10 | 1.60\% | , \$4,800 |
| Hannah Ridge Fil. 4 | 5 | 1 | 0 | 1 |  | 3 | 1 | 0 | 2 | 13 | 2.08\% | \$6,240 |
| Hannah Ridge Fil. 5-8 | 3 | 5 | 0 | 2 |  | 2 | 3 | 0 | 6 | 21 | 3.36\% | \$10,080 |
| Hannah Ridge Fil. 9 | 22 | 8 | 0 | 2 |  | 10 | 4 | 0 | 7 | 53 | 8.48\% | \$25,440 |
| Hannah Ridge Fil. 10 | 5 | 4 | 0 | 11 |  | 29 | 22 | 0 | 29 | 100 | 16.00\% | \$48,000 |
| Other Background | 0 | 0 | 164 | 0 |  | 0 | 0 | 153 | 0 | 317 | 50.72\% | \$152,160 |
|  | 45 | 20 | 204 | 17 | 0 | 52 | 32 | 206 | 49 | 625 |  | \$300,000 |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |  |  |  |  |  |  |  |  |


| Table 4Hannah Ridge at Feathergrass Sketch Plan Amendment/Preliminary PlanRoadway Improvements |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Improvement | Type | Timing | Phase | Responsibility |
| Re-stripe outside westbound through lane on Constitution Avenue for acceleration/deceleration lanes approaching Shawnee Drive | Re-striping | When Shawnee Drive is extended north of Constitution Avenue | Phase 4 | Applicant and/or Metro District |
| Lengthen eastbound left-turn lane on Constitution Avenue approaching Shawnee Drive and reconstruct median to restrict the access to threequarter movement (left-in/right-in/right-out only) | Auxiliary lane and median modification | When Shawnee Drive is extended north of Constitution Avenue | Phase 4 | Applicant and/or Metro District |
| Lengthen westbound right-turn acceleration lane on Constitution Avenue from Shawnee Drive | Auxiliary Lane | When Shawnee Drive is extended north of Constitution Avenue | Phase 4 | Applicant and/or Metro District |
| Signalization of Constitution Avenue/Akers Drive intersection | Traffic Signal | When warranted | - | El Paso County |
| Signalization of Constitution Avenue/Hannah Ridge Drive intersection | Traffic Signal | When warranted | - | Developments north and south of Constitution Avenue |
| Signalization of Akers Drive/Hunter Jumper Drive intersection | Traffic Signal | When warranted | Phase 10 | Applicant and/or Metro District |
| Construct dual eastbound left-turn lanes on Constitution Avenue approaching Akers Drive | Auxiliary lane | When needed | Phase 10 | Developments and other trip generators along Akers Drive north of Constitution Avenue |
| Widen Constitution Avenue to six-lane cross section | Roadway segment | Shown on MTCP by 2040 | - | Master planned |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |











Figure 9

LEGEND:
$\frac{X X}{X X}=\frac{\text { AM Weekday Peak-Hour Traffic (vehicles per hour) }}{\text { PM }}$ PM Weekday Peak-Hour Traffic (vehicles per hour)
$X X X=$ Average Weekday Traffic (vehicles per day)

Assignment of
Filing 4 Only Site-Generated Traffic


Figure 10

LEGEND:
$\frac{X X}{X X}=\underline{\text { AM Weekday Peak-Hour Traffic (vehicles per hour) }}$ PM Weekday Peak-Hour Traffic (vehicles per hour)
$X X X=$ Average Weekday Traffic (vehicles per day)

Assignment of Filings 5-8 Only Site-Generated Traffic







Figure 4C-2. Warrant 2 Four-Hour Vehicular Volume ( $70 \%$ Factor)
(Community Lees than 10,000 population or above $\mathbf{4 0}$ mph on Major Street)


* Note: 80 vph applies as the lower threshold volumes for a minor-street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor-street approach with one lane.
- Northbound Minor Approach: (northbound left-turn and through volume plus $50 \%$ of right-turn volume).
- Southbound Minor Approach: (southbound left-turn and through volume).

Short-Term Traffic Signal Warrant Constitution/Hannah Ridge Following Filing 3

Figure 4C-2. Warrant 2 Four-Hour Vehicular Volume ( $70 \%$ Factor) (Community Lees than $\mathbf{1 0 , 0 0 0}$ population or above 40 mph on Malor Street)


* Note: 80 vph applies as the lower threshold volumes for a minor-street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor-street approach with one lane.
- Northbound Minor Approach: (northbound left-turn and through volume plus $50 \%$ of right-turn volume).
- Southbound Minor Approach: (southbound left-turn and through volume).

Short-Term Traffic Signal Warrant Constitution/Hannah Ridge Following Filing 4


LSC Transportation Consultants, Inc.

## 545 E. Pikes Peak Ave., \#210

LSC Transportation Consultants, Inc. Colorado Springs, CO 8090円le Name : Akers Dr - Constitution AM

| (719) 633-2868 | Site Code :00174210 |
| :--- | :--- |
|  | Start Date :03/15/2017 |
|  | Page No : 1 |

Groups Printed- Unshifted

|  | Akers Dr From North |  |  |  | Constitution Ave From East |  |  |  | From South |  |  |  | Constitution Ave From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 06:30 AM | 7 | 0 | 6 | 0 | 12 | 93 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 113 | 4 | 1 | 236 |
| 06:45 AM | 8 | 0 | 7 | 0 | 19 | 114 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 123 | 18 | 0 | 290 |
| Total | 15 | 0 | 13 | 0 | 31 | 207 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 236 | 22 | 1 | 526 |


| 07:00 AM | 18 | 0 | 10 | 0 | 6 | 130 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 102 | 8 | 0 | 276 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $07: 15 \mathrm{AM}$ | 24 | 0 | 13 | 0 | 3 | 171 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 121 | 27 | 0 | 359 |
| 07:30 AM | 32 | 0 | 12 | 0 | 8 | 190 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 108 | 36 | 0 | 386 |
| $07: 45 \mathrm{AM}$ | 33 | 0 | 9 | 0 | 13 | 166 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 100 | 23 | 0 | 344 |
| Total | 107 | 0 | 44 | 0 | 30 | 657 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 431 | 94 | 0 | 1365 |


| 08:00 AM | 23 | 0 | 7 | 0 | 10 | 131 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 132 | 13 | 0 | 316 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 08:15 AM | 5 | 0 | 11 | 0 | 5 | 104 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 103 | 8 | 0 | 236 |
| Grand Total | 150 | 0 | 75 | 0 | 76 | 1099 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 902 | 137 | 1 | 2443 |
| Apprch \% | 66.7 | 0.0 | 33.3 | 0.0 | 6.5 | 93.3 | 0.0 | 0.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 86.7 | 13.2 | 0.1 |  |
| Total \% | 6.1 | 0.0 | 3.1 | 0.0 | 3.1 | 45.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 36.9 | 5.6 | 0.0 |  |

LSC Transportation Consultants, Inc.
545 E. Pikes Peak Ave., \#210
Colorado Springs, CO 80908le Name : Akers Dr - Constitution AM
$\begin{array}{ll}\text { (719) 633-2868 } & \text { Site Code :00174210 } \\ & \text { Start Date :03/15/2017 } \\ & \text { Page No }: 2\end{array}$

|  | Akers Dr From North |  |  |  |  | Constitution Ave From East |  |  |  |  | From South |  |  |  |  | Constitution Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | $\begin{gathered} \text { Rig } \\ \text { ht } \\ \hline \end{gathered}$ | Thr u | Lef t | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \\ \hline \end{gathered}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r\|} \hline \text { Lef } \\ \mathrm{t} \\ \hline \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \\ \hline \end{gathered}$ | $\begin{array}{r}\text { Thr } \\ \text { u } \\ \hline\end{array}$ | Lef t | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \\ \hline \end{gathered}$ | Thr | Lef t | $\begin{aligned} & \mathrm{Pe} \\ & \text { ds } \end{aligned}$ | App. <br> Total | $\begin{aligned} & \text { Int. } \\ & \text { Total } \end{aligned}$ |




LSC Transportation Consultants, Inc.
545 E. Pikes Peak Ave., \#210
LSC Transportation Consultants, Inc. Colorado Springs, CO 8090Bile Name : Akers Dr - Constitution PM

| (719) 633-2868 | Site Code :00174210 |
| :--- | :--- |
|  | Start Date :03/15/2017 |
|  | Page No $: 1$ |

Groups Printed- Unshifted

|  | Akers Dr From North |  |  |  | Constitution Ave From East |  |  |  | From South |  |  |  | Constitution Ave From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \hline \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 04:00 PM | 30 | 0 | 14 | 0 | 22 | 127 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 174 | 21 | 0 | 389 |
| 04:15 PM | 7 | 0 | 9 | 0 | 16 | 163 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 151 | 16 | 0 | 362 |
| 04:30 PM | 7 | 0 | 3 | 0 | 7 | 161 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 151 | 8 | 0 | 337 |
| 04:45 PM | 6 | 0 | 5 | 0 | 6 | 138 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 151 | 15 | 0 | 321 |
| Total | 50 | 0 | 31 | 0 | 51 | 589 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 627 | 60 | 0 | 1409 |
| 05:00 PM | 35 | 0 | 18 | 0 | 7 | 156 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 156 | 12 | 0 | 385 |
| 05:15 PM | 12 | 0 | 9 | 0 | 10 | 156 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 175 | 12 | 0 | 374 |
| 05:30 PM | 13 | 0 | 5 | 0 | 3 | 127 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 169 | 11 | 0 | 329 |
| 05:45 PM | 3 | 0 | 3 | 0 | 3 | 126 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 146 | 9 | 0 | 291 |
| Total | 63 | 0 | 35 | 0 | 23 | 565 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 646 | 44 | 0 | 1379 |
| Grand Total | 113 | 0 | 66 | 0 | 74 | 1154 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 1273 | 104 | 0 | 2788 |
| Apprch \% | 63.1 | 0.0 | 36.9 | 0.0 | 6.0 | 93.7 | 0.0 | 0.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 92.4 | 7.6 | 0.0 |  |
| Total \% | 4.1 | 0.0 | 2.4 | 0.0 | 2.7 | 41.4 | 0.0 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 45.7 | 3.7 | 0.0 |  |

LSC Transportation Consultants, Inc.
545 E. Pikes Peak Ave., \#210
Colorado Springs, CO 8090Bile Name : Akers Dr - Constitution PM

| (719) 633-2868 | Site Code :00174210 |
| :--- | :--- |
|  | Start Date $: 03 / 15 / 2017$ |
|  | Page No $: 2$ |


|  | Akers Dr From North |  |  |  |  | Constitution Ave From East |  |  |  |  | From South |  |  |  |  | Constitution Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r} \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \\ \hline \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \text { ds } \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \\ \hline \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \\ \hline \end{gathered}$ | $\begin{array}{r} \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r\|} \hline \text { Lef } \\ \mathrm{t} \\ \hline \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \\ \hline \end{gathered}$ | Thr u | Lef | Pe ds | App. <br> Total | Int. Total |




## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Hanna Ridge - Constitution Ave AM
Site Code : 00174210
Start Date : 07/25/2017
Page No : 1
Groups Printed- Unshifted

|  | Hanna Ridge Dr From North |  |  |  | Constitution Ave From East |  |  |  | Hanna Ridge Dr From South |  |  |  | Constitution Ave From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 06:30 AM | 3 | 1 | 0 | 0 | 1 | 75 | 7 | 0 | 20 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 111 |
| 06:45 AM | 1 | 0 | 1 | 0 | 1 | 118 | 12 | 0 | 12 | 0 | 6 | 0 | 1 | 0 | 1 | 0 | 153 |
| Total | 4 | 1 | 1 | 0 | 2 | 193 | 19 | 0 | 32 | 0 | 8 | 0 | 3 | 0 | 1 | 0 | 264 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 07:00 AM | 4 | 0 | 1 | 0 | 2 | 86 | 5 | 0 | 12 | 0 | 15 | 0 | 2 | 0 | 3 | 2 | 132 |
| $07: 15 \mathrm{AM}$ | 3 | 0 | 0 | 0 | 3 | 143 | 2 | 0 | 15 | 0 | 10 | 0 | 0 | 0 | 4 | 0 | 180 |
| 07:30 AM | 3 | 0 | 1 | 0 | 4 | 117 | 5 | 0 | 19 | 0 | 7 | 0 | 0 | 0 | 5 | 1 | 162 |
| $07: 45 \mathrm{AM}$ | 2 | 0 | 1 | 0 | 0 | 119 | 2 | 0 | 8 | 1 | 5 | 0 | 4 | 0 | 2 | 0 | 144 |
| Total | 12 | 0 | 3 | 0 | 9 | 465 | 14 | 0 | 54 | 1 | 37 | 0 | 6 | 0 | 14 | 3 | 618 |


| 08:00 AM | 2 | 0 | 1 | 0 | 3 | 112 | 6 | 0 | 10 | 0 | 3 | 0 | 3 | 0 | 4 | 0 | 144 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 08:15 AM | 2 | 0 | 1 | 0 | 1 | 79 | 4 | 1 | 13 | 1 | 2 | 0 | 2 | 0 | 0 | 0 | 106 |
| Grand Total | 20 | 1 | 6 | 0 | 15 | 849 | 43 | 1 | 109 | 2 | 50 | 0 | 14 | 0 | 19 | 3 | 1132 |
| Apprch \% | 74.1 | 3.7 | 22.2 | 0.0 | 1.7 | 93.5 | 4.7 | 0.1 | 67.7 | 1.2 | 31.1 | 0.0 | 38.9 | 0.0 | 52.8 | 8.3 |  |
| Total \% | 1.8 | 0.1 | 0.5 | 0.0 | 1.3 | 75.0 | 3.8 | 0.1 | 9.6 | 0.2 | 4.4 | 0.0 | 1.2 | 0.0 | 1.7 | 0.3 |  |

File Name : Hanna Ridge - Constitution Ave AM
Site Code : 00174210
Start Date : 07/25/2017
Page No : 2

|  | Hanna Ridge Dr From North |  |  |  |  | Constitution Ave From East |  |  |  |  | Hanna Ridge Dr From South |  |  |  |  | Constitution Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | $\begin{array}{\|r\|} \hline \text { Rig } \\ \mathrm{ht} \\ \hline \end{array}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \hline \mathrm{Pe} \\ & \mathrm{ds} \\ & \hline \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{\|r\|} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \hline \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r\|} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \hline \mathrm{Pe} \\ & \mathrm{ds} \\ & \hline \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{\|r\|} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{array}{\|c} \hline \text { Int. } \\ \text { Total } \end{array}$ |
| Peak Hour From 06:30 AM to 08:15 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersecti on | 07:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 10 | 0 | 3 | 0 | 13 | 10 | 49 1 | 15 | 0 | 516 | 52 | 1 | 25 | 0 | 78 | 7 | 0 | 15 | 1 | 23 | 630 |
| Percent | $\begin{array}{r} 76 . \\ 9 \end{array}$ | 0.0 | $\begin{array}{r} 23 . \\ 1 \end{array}$ | 0.0 |  | 1.9 | 95. | 2.9 | 0.0 |  | 66. 7 | 1.3 | 32. | 0.0 |  | 30. | 0.0 | 65. | 4.3 |  |  |
| 07:15 Volume | 3 | 0 | 0 | 0 | 3 | 3 | 14 3 | 2 | 0 | 148 | 15 | 0 | 10 | 0 | 25 | 0 | 0 | 4 | 0 | 4 | 180 |
| Peak | 07:30 AM |  |  |  |  | 07:15 AM |  |  |  |  | 07:30 AM |  |  |  |  | 08:00 AM |  |  |  |  | 0.875 |
| Factor |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| High Int. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 3 | 0 | 1 | 0 | 4 |  |  |  |  |  | 3 | 14 3 |  | 0 | 148 | 19 | 0 | 7 | 0 | 26 | 3 | 0 | 4 | 0 | 7 |  |
| Peak |  |  |  |  | 0.81 |  |  |  |  |  |  |  |  |  | 0.87 |  |  |  |  | 0.75 |  |  |  |  | 0.82 |  |
| Factor |  |  |  |  | 3 |  |  |  |  | 2 |  |  |  |  | 0 |  |  |  |  | 1 |  |



## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Hanna Ridge - Constitution Ave PM
Site Code $: 00174210$
Start Date $: 07 / 24 / 2017$
Page No $: 1$
Groups Printed- Unshifted

|  | Hanna Ridge Dr From North |  |  |  | Constitution Ave From East |  |  |  | Hanna Ridge Dr From South |  |  |  | Constitution Ave From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \hline \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 04:00 PM | 4 | 0 | 2 | 0 | 2 | 130 | 16 | 0 | 12 | 3 | 10 | 0 | 13 | 0 | 2 | 0 | 194 |
| 04:15 PM | 1 | 0 | 0 | 0 | 2 | 108 | 15 | 0 | 17 | 5 | 10 | 0 | 7 | 0 | 5 | 0 | 170 |
| 04:30 PM | 3 | 0 | 0 | 0 | 2 | 122 | 11 | 0 | 6 | 7 | 7 | 0 | 8 | 0 | 4 | 0 | 170 |
| 04:45 PM | 2 | 0 | 1 | 0 | 1 | 135 | 21 | 0 | 13 | 2 | 8 | 0 | 7 | 0 | 1 | 0 | 191 |
| Total | 10 | 0 | 3 | 0 | 7 | 495 | 63 | 0 | 48 | 17 | 35 | 0 | 35 | 0 | 12 | 0 | 725 |
| 05:00 PM | 6 | 0 | 0 | 0 | 1 | 167 | 17 | 0 | 18 | 0 | 8 | 0 | 13 | 0 | 2 | 0 | 232 |
| 05:15 PM | 3 | 0 | 1 | 1 | 0 | 152 | 14 | 0 | 9 | 1 | 10 | 0 | 12 | 0 | 8 | 0 | 211 |
| 05:30 PM | 1 | 0 | 1 | 0 | 1 | 118 | 17 | 0 | 18 | 0 | 10 | 0 | 13 | 0 | 2 | 0 | 181 |
| 05:45 PM | 5 | 0 | 0 | 0 | 0 | 95 | 12 | 0 | 11 | 1 | 10 | 0 | 10 | 0 | 4 | 0 | 148 |
| Total | 15 | 0 | 2 | 1 | 2 | 532 | 60 | 0 | 56 | 2 | 38 | 0 | 48 | 0 | 16 | 0 | 772 |
| Grand Total | 25 | 0 | 5 | 1 | 9 | 1027 | 123 | 0 | 104 | 19 | 73 | 0 | 83 | 0 | 28 | 0 | 1497 |
| Apprch \% | 80.6 | 0.0 | 16.1 | 3.2 | 0.8 | 88.6 | 10.6 | 0.0 | 53.1 | 9.7 | 37.2 | 0.0 | 74.8 | 0.0 | 25.2 | 0.0 |  |
| Total \% | 1.7 | 0.0 | 0.3 | 0.1 | 0.6 | 68.6 | 8.2 | 0.0 | 6.9 | 1.3 | 4.9 | 0.0 | 5.5 | 0.0 | 1.9 | 0.0 |  |

```
File Name : Hanna Ridge - Constitution Ave PM
Site Code : 00174210
Start Date :07/24/2017
Page No : 2
```



## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Shawnee Dr - Constitution Ave AM
Site Code : 00174210
Start Date : 07/25/2017
Page No : 1
Groups Printed- Bank 1

|  | From North |  |  |  | Constitution Ave From East |  |  |  | Shawnee Dr From South |  |  |  | Constitution Ave From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 06:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 12 | 0 | 10 | 0 | 3 | 0 | 0 | 0 | 29 |
| 06:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 15 | 0 | 16 | 0 | 3 | 0 | 0 | 0 | 38 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 27 | 0 | 26 | 0 | 6 | 0 | 0 | 0 | 67 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $07: 00 \mathrm{AM}$ | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 13 | 0 | 12 | 0 | 4 | 0 | 0 | 0 | 36 |
| $07: 15 \mathrm{AM}$ | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 11 | 0 | 20 | 0 | 4 | 0 | 0 | 0 | 48 |
| 07:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 12 | 0 | 13 | 0 | 5 | 0 | 0 | 0 | 33 |
| $07: 45 \mathrm{AM}$ | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 14 | 0 | 9 | 0 | 4 | 0 | 0 | 0 | 38 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 34 | 0 | 50 | 0 | 54 | 0 | 17 | 0 | 0 | 0 | 155 |


| 08:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 5 | 0 | 17 | 0 | 6 | 0 | 0 | 0 | 38 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 08:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 6 | 0 | 11 | 0 | 4 | 0 | 0 | 0 | 24 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 0 | 55 | 0 | 88 | 0 | 108 | 0 | 33 | 0 | 0 | 0 | 284 |
| Apprch \% | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 100. | 0.0 | 44.9 | 0.0 | 55.1 | 0.0 | 100. | 0.0 | 0.0 | 0.0 |  |
| Total \% | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 19.4 | 0.0 | 31.0 | 0.0 | 38.0 | 0.0 | 11.6 | 0.0 | 0.0 | 0.0 |  |

File Name : Shawnee Dr-Constitution Ave AM
Site Code : 00174210
Start Date : 07/25/2017
Page No : 2


## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Shawnee Dr - Constitution Ave PM
Site Code : 00174210
Start Date : 07/24/2017
Page No : 1
Groups Printed- Bank 1

|  | From North |  |  |  | Constitution Ave From East |  |  |  | Shawnee Dr From South |  |  |  | Constitution Ave From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 04:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 11 | 0 | 8 | 0 | 11 | 0 | 0 | 0 | 33 |
| 04:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 9 | 0 | 8 | 0 | 19 | 0 | 0 | 1 | 48 |
| 04:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 14 | 0 | 10 | 0 | 17 | 0 | 0 | 0 | 53 |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 12 | 0 | 17 | 0 | 19 | 0 | 0 | 0 | 53 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 31 | 0 | 46 | 0 | 43 | 0 | 66 | 0 | 0 | 1 | 187 |
| 05:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 15 | 0 | 14 | 0 | 17 | 0 | 0 | 0 | 57 |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 0 | 12 | 0 | 14 | 0 | 16 | 0 | 0 | 0 | 58 |
| 05:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 9 | 0 | 21 | 0 | 19 | 0 | 0 | 0 | 59 |
| 05:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 7 | 0 | 14 | 0 | 20 | 0 | 0 | 0 | 51 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 47 | 0 | 43 | 0 | 63 | 0 | 72 | 0 | 0 | 0 | 225 |


| Grand Total | 0 | 0 | 0 | 0 | 0 | 0 | 78 | 0 | 89 | 0 | 106 | 0 | 138 | 0 | 0 | 1 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Apprch \% | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 100. | 0.0 | 45.6 | 0.0 | 54.4 | 0.0 | 99.3 | 0.0 | 0.0 | 0.7 |
| Total \% | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 18.9 | 0.0 | 21.6 | 0.0 | 25.7 | 0.0 | 33.5 | 0.0 | 0.0 | 0.2 |

File Name : Shawnee Dr - Constitution Ave PM
Site Code : 00174210
Start Date : 07/24/2017
Page No : 2


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | 44 | 「 | ${ }^{7}$ | 中4 | 「 | ${ }^{7}$ | $\dagger$ |  | ${ }^{7}$ | 4 | 7 |
| Traffic Vol，veh／h | 15 | 505 | 7 | 15 | 745 | 10 | 25 | 1 | 52 | 3 | 0 | 10 |
| Future Vol，veh／h | 15 | 505 | 7 | 15 | 745 | 10 | 25 | 1 | 52 | 3 | 0 | 10 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 300 | － | 0 | 185 | － | 0 | 0 | － | － | 0 | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 100 | 100 | 100 | 87 | 87 | 87 | 78 | 78 | 78 | 100 | 100 | 100 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 15 | 505 | 7 | 17 | 856 | 11 | 32 | 1 | 67 | 3 | 0 | 10 |







| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 44 | 「 | ${ }^{7}$ | 44 | 「 | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | 「 |
| Traffic Vol，veh／h | 13 | 619 | 45 | 69 | 599 | 3 | 36 | 3 | 58 | 3 | 0 | 12 |
| Future Vol，veh／h | 13 | 619 | 45 | 69 | 599 | 3 | 36 | 3 | 58 | 3 | 0 | 12 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 300 | － | 0 | 185 | － | 0 | 0 | － | － | 0 | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 92 | 93 | 93 | 100 | 100 | 100 | 93 | 93 | 93 | 63 | 63 | 63 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 14 | 666 | 48 | 69 | 599 | 3 | 39 | 3 | 62 | 5 | 0 | 19 |





| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1．6 |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{1}$ | 4个4 | 个 $\uparrow$ | 「 | ＊ | 「 |
| Traffic Vol，veh／h | 47 | 633 | 611 | 30 | 35 | 60 |
| Future Vol，veh／h | 47 | 633 | 611 | 30 | 35 | 60 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | － | None | － | None | － | Free |
| Storage Length | 0 | － | － | 0 | 0 | 0 |
| Veh in Median Storage，\＃ | － | 0 | 0 | － | 0 | － |
| Grade，\％ | － | 0 | 0 | － | 0 | － |
| Peak Hour Factor | 100 | 100 | 98 | 98 | 45 | 45 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 47 | 633 | 623 | 31 | 78 | 133 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1.4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 44 | 「 | ${ }^{*}$ | 中4 | 「 | ${ }^{7}$ | $\uparrow$ |  | ${ }^{1}$ | 4 | 「 |
| Traffic Vol，veh／h | 15 | 544 | 7 | 15 | 787 | 10 | 25 | 1 | 52 | 3 | 0 | 10 |
| Future Vol，veh／h | 15 | 544 | 7 | 15 | 787 | 10 | 25 | 1 | 52 | 3 | 0 | 10 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 300 | － | 0 | 185 | － | 0 | 0 | － | － | 0 | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 100 | 100 | 100 | 87 | 87 | 87 | 78 | 78 | 78 | 100 | 100 | 100 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 15 | 544 | 7 | 17 | 905 | 11 | 32 | 1 | 67 | 3 | 0 | 10 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1.8 |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL |  | NBL | NBR |
| Lane Configurations | 个4 | 「 | ${ }^{7}$ | 个个4 | M |  |
| Traffic Vol，veh／h | 524 | 19 | 37 | 785 | 59 | 42 |
| Future Vol，veh／h | 524 | 19 | 37 | 785 | 59 | 42 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | － | None | － | None | － | None |
| Storage Length | － | 275 | 215 | － | 0 | － |
| Veh in Median Storage，\＃ | 0 | － | － | 0 | 0 | － |
| Grade，\％ | 0 | － | － | 0 | 0 | － |
| Peak Hour Factor | 97 | 97 | 87 | 87 | 81 | 81 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 540 | 20 | 43 | 902 | 73 | 52 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1.7 |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{1}$ | 444 | 44 | 「 | ${ }^{1}$ | F' |
| Traffic Vol, veh/h | 99 | 500 | 700 | 34 | 41 | 112 |
| Future Vol, veh/h | 99 | 500 | 700 | 34 | 41 | 112 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Free |
| Storage Length | 0 | - | - | 0 | 0 | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 97 | 97 | 87 | 87 | 87 | 87 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 102 | 515 | 805 | 39 | 47 | 129 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 44 | 7 | ${ }^{7}$ | 44 | F | ${ }^{7}$ | $\uparrow$ |  | ${ }^{1 /}$ | 4 | 「 |
| Traffic Vol, veh/h | 13 | 661 | 45 | 69 | 638 | 3 | 36 | 3 | 58 | 3 | 0 | 12 |
| Future Vol, veh/h | 13 | 661 | 45 | 69 | 638 | 3 | 36 | 3 | 58 | 3 | 0 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 300 | - | 0 | 185 | - | 0 | 0 | - | - | 0 | - | 0 |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 92 | 93 | 93 | 100 | 100 | 100 | 93 | 93 | 93 | 63 | 63 | 63 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 14 | 711 | 48 | 69 | 638 | 3 | 39 | 3 | 62 | 5 | 0 | 19 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 2 |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL |  | NBL | NBR |
| Lane Configurations | 个4 | 「 | \% | 444 | M |  |
| Traffic Vol, veh/h | 671 | 71 | 42 | 644 | 66 | 48 |
| Future Vol, veh/h | 671 | 71 | 42 | 644 | 66 | 48 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 275 | 215 | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 93 | 93 | 98 | 98 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 722 | 76 | 43 | 657 | 69 | 51 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{*}$ | 444 | 44 | 「 | ${ }^{1}$ | 「 |
| Traffic Vol, veh/h | 47 | 675 | 650 | 30 | 35 | 60 |
| Future Vol, veh/h | 47 | 675 | 650 | 30 | 35 | 60 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Free |
| Storage Length | 0 | - | - | 0 | 0 | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 98 | 98 | 45 | 45 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 675 | 663 | 31 | 78 | 133 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 个4 | 「 | ${ }^{7}$ | 个4 | 「 | ${ }^{7}$ | $\hat{F}$ |  | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Vol，veh／h | 15 | 544 | 7 | 15 | 787 | 10 | 40 | 1 | 52 | 3 | 1 | 10 |
| Future Vol，veh／h | 15 | 544 | 7 | 15 | 787 | 10 | 40 | 1 | 52 | 3 | 1 | 10 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 300 | － | 0 | 185 | － | 0 | 0 | － | － | 0 | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 |  |
| Peak Hour Factor | 100 | 100 | 100 | 87 | 87 | 87 | 78 | 78 | 78 | 100 | 100 | 100 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 15 | 544 | 7 | 17 | 905 | 11 | 51 | 1 | 67 | 3 | 1 | 10 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 0．6 |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 中4 | 「 | ${ }^{*}$ | 坐乐 |  | 「 |
| Traffic Vol，veh／h | 524 | 19 | 37 | 802 | 0 | 42 |
| Future Vol，veh／h | 524 | 19 | 37 | 802 | 0 | 42 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | － | None | － | None | － | None |
| Storage Length | － | 275 | 215 | － | － | 0 |
| Veh in Median Storage，\＃ | 0 | － | － | 0 | 0 | － |
| Grade，\％ | 0 | － | － | 0 | 0 | － |
| Peak Hour Factor | 97 | 97 | 87 | 87 | 81 | 81 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 540 | 20 | 43 | 922 | 0 | 52 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 44 | 「 | ${ }^{7}$ | 44 | 「 | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | F |
| Traffic Vol, veh/h | 13 | 661 | 45 | 69 | 638 | 3 | 43 | 3 | 58 | 3 | 1 | 12 |
| Future Vol, veh/h | 13 | 661 | 45 | 69 | 638 | 3 | 43 | 3 | 58 | 3 | 1 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 300 | - | 0 | 185 | - | 0 | 0 | - | - | 0 | - | 0 |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 92 | 93 | 93 | 100 | 100 | 100 | 93 | 93 | 93 | 63 | 63 | 63 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 14 | 711 | 48 | 69 | 638 | 3 | 46 | 3 | 62 | 5 | 2 | 19 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 0.6 |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 44 | 「 | ${ }^{7}$ | 种4 |  | F |
| Traffic Vol, veh/h | 671 | 71 | 42 | 661 | 0 | 48 |
| Future Vol, veh/h | 671 | 71 | 42 | 661 | 0 | 48 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 275 | 215 | - | - | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 93 | 93 | 98 | 98 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 722 | 76 | 43 | 674 | 0 | 51 |



|  | 4 | $\rightarrow$ | $\checkmark$ | 7 | 4 | 4 | 4 | $\dagger$ | $\pm$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 4坐 | F | ${ }^{7}$ | 4坐 | 「 | \％ | F | ${ }^{7}$ | 4 | F |
| Traffic Volume（vph） | 15 | 1257 | 75 | 157 | 1031 | 10 | 204 | 1 | 3 | 1 | 10 |
| Future Volume（vph） | 15 | 1257 | 75 | 157 | 1031 | 10 | 204 | 1 | 3 | 1 | 10 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Prot | NA | pm＋pt | NA | Perm |
| Protected Phases | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 1 | 6 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 |  |  | 6 |  | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | 8 | 5 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 22.0 | 22.0 | 9.0 | 22.0 | 22.0 | 9.0 | 13.0 | 9.0 | 13.0 | 13.0 |
| Total Split（s） | 10.0 | 70.0 | 70.0 | 15.0 | 75.0 | 75.0 | 20.0 | 25.0 | 10.0 | 15.0 | 15.0 |
| Total Split（\％） | 8．3\％ | 58．3\％ | 58．3\％ | 12．5\％ | 62．5\％ | 62．5\％ | 16．7\％ | 20．8\％ | 8．3\％ | 12．5\％ | 12．5\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Min | Min | None | Min | Min | None | None | None | None | None |
| Act Effct Green（s） | 32.8 | 25.5 | 25.5 | 40.7 | 37.2 | 37.2 | 9.9 | 10.0 | 6.1 | 5.8 | 5.8 |
| Actuated g／C Ratio | 0.52 | 0.41 | 0.41 | 0.65 | 0.59 | 0.59 | 0.16 | 0.16 | 0.10 | 0.09 | 0.09 |
| v／c Ratio | 0.05 | 0.64 | 0.11 | 0.48 | 0.36 | 0.01 | 0.40 | 0.53 | 0.02 | 0.01 | 0.04 |
| Control Delay | 6.0 | 16.9 | 0.3 | 11.5 | 8.3 | 0.0 | 28.7 | 12.9 | 26.3 | 34.0 | 0.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 6.0 | 16.9 | 0.3 | 11.5 | 8.3 | 0.0 | 28.7 | 12.9 | 26.3 | 34.0 | 0.2 |
| LOS | A | B | A | B | A | A | C | B | C | C | A |
| Approach Delay |  | 15.9 |  |  | 8.6 |  |  | 20.9 |  | 7.7 |  |
| Approach LOS |  | B |  |  | A |  |  | C |  | A |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 62.7
Natural Cycle： 60
Control Type：Actuated－Uncoordinated
Maximum v／c Ratio： 0.64
Intersection Signal Delay： 13.6
Intersection LOS：B
Intersection Capacity Utilization 59．6\％
ICU Level of Service B
Analysis Period（min） 15
Splits and Phases：2：Hannah Ridge \＆Constitution


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 0.6 |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 444 | 「 | ${ }^{1}$ | 444 |  | 「 |
| Traffic Vol, veh/h | 1305 | 19 | 37 | 1208 | 0 | 42 |
| Future Vol, veh/h | 1305 | 19 | 37 | 1208 | 0 | 42 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 285 | 420 | - | - | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1374 | 20 | 39 | 1272 | 0 | 44 |



|  | 4 | $\rightarrow$ | \％ | $\checkmark$ | 4 | 4 | 4 | 4 |  | 1 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | SBL | SBT | SBR |
| Lane Configurations | ＊＊ | 忡4 | F | ${ }^{1}$ | 坐坐 | 「 | ${ }^{7}$ | $\uparrow$ | ${ }^{7} 1$ | 4 | 「 |
| Traffic Volume（vph） | 291 | 1113 | 56 | 19 | 1023 | 216 | 24 | 4 | 78 | 5 | 151 |
| Future Volume（vph） | 291 | 1113 | 56 | 19 | 1023 | 216 | 24 | 4 | 78 | 5 | 151 |
| Turn Type | Prot | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | pm＋pt | NA | Perm |
| Protected Phases | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 1 | 6 |  |
| Permitted Phases |  |  | 4 | 8 |  | 8 | 2 |  | 6 |  | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | 8 | 5 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 23.0 | 23.0 | 9.0 | 23.0 | 23.0 | 9.0 | 15.0 | 9.0 | 20.0 | 20.0 |
| Total Split（s） | 25.0 | 75.0 | 75.0 | 10.0 | 60.0 | 60.0 | 15.0 | 20.0 | 15.0 | 20.0 | 20.0 |
| Total Split（\％） | 20．8\％ | 62．5\％ | 62．5\％ | 8．3\％ | 50．0\％ | 50．0\％ | 12．5\％ | 16．7\％ | 12．5\％ | 16．7\％ | 16．7\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Min | Min | None | Min | Min | None | None | None | None | None |
| Act Effct Green（s） | 12.2 | 37.8 | 37.8 | 31.0 | 23.6 | 23.6 | 10.5 | 6.7 | 12.1 | 9.5 | 9.5 |
| Actuated g／C Ratio | 0.18 | 0.56 | 0.56 | 0.46 | 0.35 | 0.35 | 0.16 | 0.10 | 0.18 | 0.14 | 0.14 |
| v／c Ratio | 0.49 | 0.41 | 0.06 | 0.07 | 0.60 | 0.32 | 0.10 | 0.28 | 0.17 | 0.02 | 0.43 |
| Control Delay | 30.4 | 10.7 | 0.6 | 7.9 | 20.6 | 4.3 | 24.4 | 15.9 | 24.0 | 32.8 | 9.4 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 30.4 | 10.7 | 0.6 | 7.9 | 20.6 | 4.3 | 24.4 | 15.9 | 24.0 | 32.8 | 9.4 |
| LOS | C | B | A | A | C | A | C | B | C | C | A |
| Approach Delay |  | 14.3 |  |  | 17.6 |  |  | 18.5 |  | 14.8 |  |
| Approach LOS |  | B |  |  | B |  |  | B |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 67.3
Natural Cycle： 65
Control Type：Actuated－Uncoordinated
Maximum v／c Ratio： 0.60
Intersection Signal Delay： 15.8
Intersection LOS：B
Intersection Capacity Utilization 51．1\％
ICU Level of Service A
Analysis Period（min） 15
Splits and Phases：5：Akers \＆Constitution


2：Hannah Ridge \＆Constitution

|  | $\rangle$ |  |  | 7 |  |  | 4 | 4 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 快 | F | ${ }^{*}$ | 个个中 | F | \％${ }^{1+1}$ | $\hat{\beta}$ | ${ }^{7}$ | $\uparrow$ | 7 |
| Traffic Volume（vph） | 13 | 949 | 213 | 256 | 854 | 3 | 206 | 3 | 3 | 1 | 12 |
| Future Volume（vph） | 13 | 949 | 213 | 256 | 854 | 3 | 206 | 3 | 3 | 1 | 12 |
| Turn Type | pm＋pt | NA | Perm | pm＋pt | NA | Perm | Prot | NA | $\mathrm{pm}+\mathrm{pt}$ | NA | Perm |
| Protected Phases | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 1 | 6 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 |  |  | 6 |  | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | 8 | 5 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 22.0 | 22.0 | 9.0 | 22.0 | 22.0 | 9.0 | 13.0 | 9.0 | 13.0 | 13.0 |
| Total Split（s） | 10.0 | 60.0 | 60.0 | 25.0 | 75.0 | 75.0 | 20.0 | 25.0 | 10.0 | 15.0 | 15.0 |
| Total Split（\％） | 8．3\％ | 50．0\％ | 50．0\％ | 20．8\％ | 62．5\％ | 62．5\％ | 16．7\％ | 20．8\％ | 8．3\％ | 12．5\％ | 12．5\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Min | Min | None | Min | Min | None | None | None | None | None |
| Act Effct Green（s） | 27.6 | 20.2 | 20.2 | 37.7 | 34.2 | 34.2 | 9.9 | 10.0 | 6.1 | 5.9 | 5.9 |
| Actuated g／C Ratio | 0.46 | 0.34 | 0.34 | 0.63 | 0.57 | 0.57 | 0.17 | 0.17 | 0.10 | 0.10 | 0.10 |
| v／c Ratio | 0.04 | 0.58 | 0.33 | 0.58 | 0.31 | 0.00 | 0.38 | 0.48 | 0.02 | 0.01 | 0.04 |
| Control Delay | 6.7 | 18.3 | 4.3 | 11.7 | 8.2 | 0.0 | 27.1 | 9.2 | 25.0 | 33.0 | 0.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 6.7 | 18.3 | 4.3 | 11.7 | 8.2 | 0.0 | 27.1 | 9.2 | 25.0 | 33.0 | 0.2 |
| LOS | A | B | A | B | A | A | C | A | C | C | A |
| Approach Delay |  | 15.6 |  |  | 8.9 |  |  | 18.3 |  | 6.5 |  |
| Approach LOS |  | B |  |  | A |  |  | B |  | A |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 59.6
Natural Cycle： 60
Control Type：Actuated－Uncoordinated
Maximum v／c Ratio： 0.58
Intersection Signal Delay： 13.2
Intersection LOS：B
Intersection Capacity Utilization 59．2\％
ICU Level of Service B
Analysis Period（min） 15

Splits and Phases：2：Hannah Ridge \＆Constitution


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 0．7 |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL |  | NBL | NBR |
| Lane Configurations | 个种 | 「 | ${ }^{7}$ | 个个中 |  | 7 |
| Traffic Vol，veh／h | 1127 | 71 | 42 | 1030 | 0 | 48 |
| Future Vol，veh／h | 1127 | 71 | 42 | 1030 | 0 | 48 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | － | None | － | None | － | None |
| Storage Length | － | 285 | 420 | － | － | 0 |
| Veh in Median Storage，\＃ | 0 | － | － | 0 | 0 | － |
| Grade，\％ | 0 | － | － | 0 | 0 | － |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1186 | 75 | 44 | 1084 | 0 | 51 |



|  | 4 |  |  | 7 |  |  | 4 | 4 | $\downarrow$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | SBL | SBT | SBR |
| Lane Configurations | \％${ }^{1+1}$ | 个禹 | 「 | 7 | 恌 | F | \％ | $\uparrow$ | \％＊ | $\uparrow$ | 「 |
| Traffic Volume（vph） | 291 | 1113 | 56 | 19 | 1023 | 216 | 24 | 4 | 78 | 5 | 151 |
| Future Volume（vph） | 291 | 1113 | 56 | 19 | 1023 | 216 | 24 | 4 | 78 | 5 | 151 |
| Turn Type | Prot | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Prot | NA | Perm |
| Protected Phases | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 1 | 6 |  |
| Permitted Phases |  |  | 4 | 8 |  | 8 | 2 |  |  |  | 6 |
| Detector Phase | 7 | 4 | ， | 3 | 8 |  | 5 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 23.0 | 23.0 | 9.0 | 23.0 | 23.0 | 9.0 | 15.0 | 9.0 | 20.0 | 20.0 |
| Total Split（s） | 21.0 | 60.0 | 60.0 | 12.0 | 51.0 | 51.0 | 10.0 | 18.0 | 30.0 | 38.0 | 38.0 |
| Total Split（\％） | 17．5\％ | 50．0\％ | 50．0\％ | 10．0\％ | 42．5\％ | 42．5\％ | 8．3\％ | 15．0\％ | 25．0\％ | 31．7\％ | 31．7\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Min | Min | None | Min | Min | None | None | None | None | None |
| Act Effct Green（s） | 12.2 | 36.8 | 36.8 | 31.4 | 23.2 | 23.2 | 10.3 | 6.6 | 7.5 | 12.7 | 12.7 |
| Actuated g／C Ratio | 0.18 | 0.53 | 0.53 | 0.45 | 0.33 | 0.33 | 0.15 | 0.10 | 0.11 | 0.18 | 0.18 |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.51 | 0.43 | 0.06 | 0.06 | 0.63 | 0.33 | 0.11 | 0.28 | 0.22 | 0.01 | 0.37 |
| Control Delay | 31.4 | 12.2 | 0.1 | 7.7 | 22.1 | 4.4 | 24.1 | 16.1 | 34.0 | 30.0 | 7.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 31.4 | 12.2 | 0.1 | 7.7 | 22.1 | 4.4 | 24.1 | 16.1 | 34.0 | 30.0 | 7.7 |
| LOS | C | B | A | A | C | A | C | B | C | C | A |
| Approach Delay |  | 15.5 |  |  | 18.8 |  |  | 18.5 |  | 16.9 |  |
| Approach LOS |  | B |  |  | B |  |  | B |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 69.3
Natural Cycle： 65
Control Type：Actuated－Uncoordinated
Maximum v／c Ratio： 0.63
Intersection Signal Delay： 17.1
Intersection LOS：B
Intersection Capacity Utilization 51．1\％
ICU Level of Service A
Analysis Period（min） 15

Splits and Phases：5：Akers \＆Constitution


| Intersection |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1.8 |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 44 | 「 | ${ }^{7}$ | 种4 | * ${ }^{\text {F }}$ |  |
| Traffic Vol, veh/h | 530 | 19 | 37 | 804 | 59 | 42 |
| Future Vol, veh/h | 530 | 19 | 37 | 804 | 59 | 42 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 275 | 215 | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 97 | 97 | 87 | 87 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 546 | 20 | 43 | 924 | 73 | 52 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1.8 |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{7}$ | 种4 | 中4 | 「 | ${ }^{*}$ | 「 |
| Traffic Vol，veh／h | 99 | 507 | 705 | 35 | 44 | 112 |
| Future Vol，veh／h | 99 | 507 | 705 | 35 | 44 | 112 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | － | None | － | None | － | Free |
| Storage Length | 0 | － | － | 0 | 0 | 0 |
| Veh in Median Storage，\＃ | － | 0 | 0 | － | 0 | － |
| Grade，\％ | － | 0 | 0 | － | 0 | － |
| Peak Hour Factor | 97 | 97 | 87 | 87 | 87 | 87 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 102 | 523 | 810 | 40 | 51 | 129 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 44 | 「 | ${ }^{7}$ | 中4 | 「 | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | 「 |
| Traffic Vol，veh／h | 35 | 661 | 45 | 69 | 638 | 17 | 36 | 5 | 58 | 8 | 2 | 24 |
| Future Vol，veh／h | 35 | 661 | 45 | 69 | 638 | 17 | 36 | 5 | 58 | 8 | 2 | 24 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 300 | － | 0 | 185 | － | 0 | 0 | － | － | 0 | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 92 | 93 | 93 | 100 | 100 | 92 | 93 | 93 | 93 | 92 | 92 | 92 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 38 | 711 | 48 | 69 | 638 | 18 | 39 | 5 | 62 | 9 | 2 | 26 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 2 |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 44 | 「 | ${ }^{1}$ | 种4 | * |  |
| Traffic Vol, veh/h | 693 | 71 | 42 | 656 | 66 | 48 |
| Future Vol, veh/h | 693 | 71 | 42 | 656 | 66 | 48 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 275 | 215 | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 93 | 93 | 98 | 98 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 745 | 76 | 43 | 669 | 69 | 51 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1.8 |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{*}$ | 种中 | 中4 | 「 | ${ }^{7}$ | 「＇ |
| Traffic Vol，veh／h | 47 | 680 | 664 | 33 | 37 | 60 |
| Future Vol，veh／h | 47 | 680 | 664 | 33 | 37 | 60 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | － | None | － | None | － | Free |
| Storage Length | 0 | － | － | 0 | 0 | 0 |
| Veh in Median Storage，\＃ | － | 0 | 0 | － | 0 | － |
| Grade，\％ | － | 0 | 0 | － | 0 | － |
| Peak Hour Factor | 100 | 100 | 98 | 98 | 45 | 45 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 680 | 678 | 34 | 82 | 133 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 44 | 「 | ${ }^{*}$ | 中4 | 「 | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | 4 | 7 |
| Traffic Vol，veh／h | 17 | 544 | 7 | 15 | 791 | 14 | 40 | 2 | 52 | 15 | 3 | 16 |
| Future Vol，veh／h | 17 | 544 | 7 | 15 | 791 | 14 | 40 | 2 | 52 | 15 | 3 | 16 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 300 | － | 0 | 185 | － | 0 | 0 | － | － | 0 | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 100 | 100 | 100 | 87 | 87 | 92 | 78 | 78 | 78 | 92 | 92 | 92 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 17 | 544 | 7 | 17 | 909 | 15 | 51 | 3 | 67 | 16 | 3 | 17 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 0．8 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 4 4 | 「 | \％ | 个4 | 「 |  |  | 「 |  |  | F |
| Traffic Vol，veh／h | 9 | 526 | 19 | 37 | 806 | 4 | 0 | 0 | 42 | 0 | 0 | 26 |
| Future Vol，veh／h | 9 | 526 | 19 | 37 | 806 | 4 | 0 | 0 | 42 | 0 | 0 | 26 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 0 | － | 275 | 215 | － | 0 | － | － | 0 | － | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － |  | 0 | － |  | 0 |  |
| Grade，\％ | － | 0 |  | － | 0 | － |  | 0 |  |  | 0 |  |
| Peak Hour Factor | 92 | 97 | 97 | 87 | 87 | 92 | 81 | 81 | 81 | 92 | 92 | 92 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 10 | 542 | 20 | 43 | 926 | 4 | 0 | 0 | 52 | 0 | 0 | 28 |


| Major／Minor | Major1 |  | Major2 |  |  |  |  | Minor1 |  |  | Minor2 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 930 | 0 | 0 |  | 562 | 0 |  | 0 | － | － | 271 | － | － | 463 |
| Stage 1 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Stage 2 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Critical Hdwy | 4.14 | － | － |  | 4.14 | － |  | － | － | － | 6.94 | － | － | 6.94 |
| Critical Hdwy Stg 1 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Critical Hdwy Stg 2 | － | － | － |  | － | － |  | － | － | － |  | － | － |  |
| Follow－up Hdwy | 2.22 | － | － |  | 2.22 | － |  | － | － | － | 3.32 | － | － | 3.32 |
| Pot Cap－1 Maneuver | 731 | － | － |  | 1005 | － |  | － | 0 | 0 | 727 | 0 | 0 | 546 |
| Stage 1 | － | － | － |  | － | － |  | － | 0 | 0 | － | 0 | 0 |  |
| Stage 2 | － | － | － |  | － | － |  | － | 0 | 0 | － | 0 | 0 |  |
| Platoon blocked，\％ |  | － | － |  |  | － |  | － |  |  |  |  |  |  |
| Mov Cap－1 Maneuver | 731 | － | － |  | 1005 | － |  | － | － | － | 727 | － | － | 546 |
| Mov Cap－2 Maneuver | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Stage 1 | － | － | － |  | － | － |  | － | － | － |  | － | － |  |
| Stage 2 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  |  | WB |  |  |  | NB |  |  | SB |  |  |
| HCM Control Delay，s | 0.2 |  |  |  | 0.4 |  |  |  | 10.3 |  |  | 12 |  |  |
| HCM LOS |  |  |  |  |  |  |  |  | B |  |  | B |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | R SBLn1 |  |  |  |  |  |  |
| Capacity（veh／h） | 727 | 731 | － | － | 1005 | － |  | － 546 |  |  |  |  |  |  |
| HCM Lane V／C Ratio | 0.071 | 0.013 | － |  | 0.042 | － | － | － 0.052 |  |  |  |  |  |  |
| HCM Control Delay（s） | 10.3 | 10 | － | － | 8.7 | － | － | 12 |  |  |  |  |  |  |
| HCM Lane LOS | B | A | － | － | A | － | － | B |  |  |  |  |  |  |
| HCM 95th \％tile Q（veh） | 0.2 | 0 | － | － | 0.1 | － | － | － 0.2 |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1.9 |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{7}$ | 种中 | 中4 | 「 | ${ }^{*}$ | 「 |
| Traffic Vol，veh／h | 99 | 512 | 707 | 35 | 47 | 112 |
| Future Vol，veh／h | 99 | 512 | 707 | 35 | 47 | 112 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | － | None | － | None | － | Free |
| Storage Length | 0 | － | － | 0 | 0 | 0 |
| Veh in Median Storage，\＃ | － | 0 | 0 | － | 0 | － |
| Grade，\％ | － | 0 | 0 | － | 0 | － |
| Peak Hour Factor | 97 | 97 | 87 | 87 | 87 | 87 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 102 | 528 | 813 | 40 | 54 | 129 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | 44 | 「 | ${ }^{1}$ | 中4 | 「 | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | 7 |
| Traffic Vol，veh／h | 20 | 661 | 45 | 69 | 651 | 14 | 53 | 7 | 58 | 11 | 3 | 16 |
| Future Vol，veh／h | 20 | 661 | 45 | 69 | 651 | 14 | 53 | 7 | 58 | 11 | 3 | 16 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 300 | － | 0 | 185 | － | 0 | 0 | － | － | 0 | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 92 | 93 | 93 | 100 | 100 | 92 | 93 | 93 | 93 | 92 | 92 | 92 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 22 | 711 | 48 | 69 | 651 | 15 | 57 | 8 | 62 | 12 | 3 | 17 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh | 0.9 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 4 4 | 「 | \％ | 个4 | 「 |  |  | F＇ |  |  | F |
| Traffic Vol，veh／h | 30 | 678 | 71 | 42 | 665 | 13 | 0 | 0 | 48 | 0 | 0 | 17 |
| Future Vol，veh／h | 30 | 678 | 71 | 42 | 665 | 13 | 0 | 0 | 48 | 0 | 0 | 17 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 0 | － | 275 | 215 | － | 0 | － | － | 0 | － | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 |  |
| Peak Hour Factor | 92 | 93 | 93 | 98 | 98 | 92 | 95 | 92 | 95 | 92 | 92 | 92 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 33 | 729 | 76 | 43 | 679 | 14 | 0 | 0 | 51 | 0 | 0 | 18 |


| Major／Minor | Major1 |  | Major2 |  |  |  |  | Minor1 |  |  | Minor2 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 693 | 0 | 0 |  | 805 | 0 | 0 | 0 | － | － | 365 | － | － | 340 |
| Stage 1 | － | － | － |  | － | － | － | － | － | － | － | － | － |  |
| Stage 2 | － | － | － |  | － | － | － | － | － | － | － | － | － |  |
| Critical Hdwy | 4.14 | － | － |  | 4.14 | － |  | － | － |  | 6.94 | － | － | 6.94 |
| Critical Hdwy Stg 1 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Critical Hdwy Stg 2 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Follow－up Hdwy | 2.22 | － | － |  | 2.22 | － | － | － | － | － | 3.32 | － | － | 3.32 |
| Pot Cap－1 Maneuver | 898 | － | － |  | 815 | － | － | － | 0 | 0 | 632 | 0 | 0 | 656 |
| Stage 1 | － | － | － |  | － | － |  | － | 0 | 0 | － | 0 | 0 |  |
| Stage 2 | － | － | － |  | － | － |  | － | 0 | 0 | － | 0 | 0 |  |
| Platoon blocked，\％ |  | － | － |  |  | － | － | － |  |  |  |  |  |  |
| Mov Cap－1 Maneuver | 898 | － | － |  | 815 | － | － |  | － | － | 632 | － | － | 656 |
| Mov Cap－2 Maneuver | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Stage 1 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Stage 2 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  |  | WB |  |  |  | NB |  |  | SB |  |  |
| HCM Control Delay，s | 0.4 |  |  |  | 0.6 |  |  |  | 11.2 |  |  | 10.6 |  |  |
| HCM LOS |  |  |  |  |  |  |  |  | B |  |  | B |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |  |  |  |  |  |  |
| Capacity（veh／h） | 632 | 898 | － | － | 815 | － |  | － 656 |  |  |  |  |  |  |
| HCM Lane V／C Ratio | 0.08 | 0.036 | － |  | 0.053 | － | － | － 0.028 |  |  |  |  |  |  |
| HCM Control Delay（s） | 11.2 | 9.2 | － | － | 9.7 | － | － | － 10.6 |  |  |  |  |  |  |
| HCM Lane LOS | B | A | － | － | A | － | － | B |  |  |  |  |  |  |
| HCM 95th \％tile Q（veh） | 0.3 | 0.1 | － | － | 0.2 | － | － | － 0.1 |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1.9 |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{7}$ | 种中 | 中4 | 「 | ${ }^{7}$ | 「＇ |
| Traffic Vol，veh／h | 47 | 683 | 674 | 34 | 39 | 60 |
| Future Vol，veh／h | 47 | 683 | 674 | 34 | 39 | 60 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | － | None | － | None | － | Free |
| Storage Length | 0 | － | － | 0 | 0 | 0 |
| Veh in Median Storage，\＃ | － | 0 | 0 | － | 0 | － |
| Grade，\％ | － | 0 | 0 | － | 0 | － |
| Peak Hour Factor | 100 | 100 | 98 | 98 | 45 | 45 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 683 | 688 | 35 | 87 | 133 |



|  | 4 | $\rightarrow$ | $\checkmark$ | 6 | 4 | 4 | 4 | $\dagger$ | * | $\frac{1}{7}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ |  | 7 | ${ }^{1}$ | 坐4 | 7 | 7 | $\uparrow$ | ${ }^{1 /}$ | 4 | 「 |
| Traffic Volume (vph) | 25 | 1337 | 75 | 164 | 1079 | 25 | 204 | 17 | 46 | 20 | 33 |
| Future Volume (vph) | 25 | 1337 | 75 | 164 | 1079 | 25 | 204 | 17 | 46 | 20 | 33 |
| Turn Type | pm+pt | NA | Perm | pm+pt | NA | Perm | Prot | NA | pm+pt | NA | Perm |
| Protected Phases | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 1 | 6 |  |
| Permitted Phases | 4 |  | 4 | 8 |  | 8 |  |  | 6 |  | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | 8 | 5 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 9.0 | 22.0 | 22.0 | 9.0 | 22.0 | 22.0 | 9.0 | 13.0 | 9.0 | 13.0 | 13.0 |
| Total Split (s) | 10.0 | 70.0 | 70.0 | 15.0 | 75.0 | 75.0 | 20.0 | 25.0 | 10.0 | 15.0 | 15.0 |
| Total Split (\%) | 8.3\% | 58.3\% | 58.3\% | 12.5\% | 62.5\% | 62.5\% | 16.7\% | 20.8\% | 8.3\% | 12.5\% | 12.5\% |
| Yellow Time (s) | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead/Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Min | Min | None | Min | Min | None | None | None | None | None |
| Act Effct Green (s) | 37.9 | 30.4 | 30.4 | 46.5 | 41.1 | 41.1 | 10.6 | 12.0 | 9.3 | 7.1 | 7.1 |
| Actuated g/C Ratio | 0.51 | 0.41 | 0.41 | 0.63 | 0.55 | 0.55 | 0.14 | 0.16 | 0.13 | 0.10 | 0.10 |
| v/c Ratio | 0.08 | 0.68 | 0.11 | 0.57 | 0.40 | 0.03 | 0.44 | 0.64 | 0.21 | 0.12 | 0.11 |
| Control Delay | 8.4 | 20.6 | 0.3 | 19.1 | 12.1 | 0.0 | 35.7 | 22.3 | 26.9 | 39.3 | 0.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 8.4 | 20.6 | 0.3 | 19.1 | 12.1 | 0.0 | 35.7 | 22.3 | 26.9 | 39.3 | 0.8 |
| LOS | A | C | A | B | B | A | D | C | C | D | A |
| Approach Delay |  | 19.3 |  |  | 12.8 |  |  | 28.6 |  | 20.6 |  |
| Approach LOS |  | B |  |  | B |  |  | C |  | C |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length: 120
Actuated Cycle Length: 74.3
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.68
Intersection Signal Delay: 18.0
Intersection LOS: B
Intersection Capacity Utilization 70.6\%
ICU Level of Service C
Analysis Period (min) 15

Splits and Phases: 2: Hannah Ridge \& Constitution


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | 种中 | F | ${ }^{7}$ | 个个4 | 「 |  |  | F |  |  | F |
| Traffic Vol，veh／h | 25 | 1394 | 19 | 37 | 1272 | 7 | 0 | 0 | 42 | 0 | 0 | 78 |
| Future Vol，veh／h | 25 | 1394 | 19 | 37 | 1272 | 7 | 0 | 0 | 42 | 0 | 0 | 78 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 335 | － | 285 | 420 | － | 300 | － | － | 0 | － | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 |  |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 |  |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 26 | 1467 | 20 | 39 | 1339 | 7 | 0 | 0 | 44 | 0 | 0 | 82 |


| Major／Minor | Major1 |  | Major2 |  |  |  |  | Minor1 |  |  | Minor2 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1346 | 0 | 0 |  | 1487 | 0 |  | 0 | － | － | 734 | － | － | 670 |
| Stage 1 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Stage 2 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Critical Hdwy | 5.34 | － | － |  | 5.34 | － |  | － | － | － | 7.14 | － | － | 7.14 |
| Critical Hdwy Stg 1 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Critical Hdwy Stg 2 | － | － | － |  | － | － |  | － | － | － |  | － | － |  |
| Follow－up Hdwy | 3.12 | － | － |  | 3.12 | － |  | － | － | － | 3.92 | － | － | 3.92 |
| Pot Cap－1 Maneuver | 265 | － | － |  | 226 | － |  | － | 0 | 0 | 311 | 0 | 0 | 343 |
| Stage 1 | － | － | － |  | － | － |  | － | 0 | 0 | － | 0 | 0 |  |
| Stage 2 | － | － | － |  | － | － |  | － | 0 | 0 | － | 0 | 0 |  |
| Platoon blocked，\％ |  | － | － |  |  | － |  | － |  |  |  |  |  |  |
| Mov Cap－1 Maneuver | 265 | － | － |  | 226 | － |  | － | － | － | 311 | － | － | 343 |
| Mov Cap－2 Maneuver | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Stage 1 | － | － | － |  | － | － |  | － | － | － |  | － | － |  |
| Stage 2 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  |  | WB |  |  |  | NB |  |  | SB |  |  |
| HCM Control Delay，s | 0.3 |  |  |  | 0.7 |  |  |  | 18.5 |  |  | 18.8 |  |  |
| HCM LOS |  |  |  |  |  |  |  |  | C |  |  | C |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | R SBLn1 |  |  |  |  |  |  |
| Capacity（veh／h） | 311 | 265 | － | － | 226 | － | － | － 343 |  |  |  |  |  |  |
| HCM Lane V／C Ratio | 0.142 | 0.099 | － |  | 0.172 | － | － | － 0.239 |  |  |  |  |  |  |
| HCM Control Delay（s） | 18.5 | 20.1 | － | － |  | － | － | 18.8 |  |  |  |  |  |  |
| HCM Lane LOS | C | C | － | － | C | － | － | C |  |  |  |  |  |  |
| HCM 95th \％tile Q（veh） | 0.5 | 0.3 | － | － | 0.6 | － | － | － 0.9 |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | \％ | 7 | $\downarrow$ |  | 4 | $\dagger$ |  | $\dagger$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | SBL | SBT | SBR |
| Lane Configurations | 71 | 蚛 | 「 | ${ }^{1}$ | 坐坐 | 「＇ | ${ }^{7}$ | $\uparrow$ | 7\％ | 4 | 「 |
| Traffic Volume（vph） | 369 | 1154 | 73 | 54 | 1055 | 248 | 41 | 11 | 179 | 15 | 171 |
| Future Volume（vph） | 369 | 1154 | 73 | 54 | 1055 | 248 | 41 | 11 | 179 | 15 | 171 |
| Turn Type | Prot | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | pm＋pt | NA | Perm |
| Protected Phases | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 1 | 6 |  |
| Permitted Phases |  |  | 4 | 8 |  | 8 | 2 |  | 6 |  | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | 8 | 5 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split（s） | 9.0 | 23.0 | 23.0 | 9.0 | 23.0 | 23.0 | 9.0 | 15.0 | 9.0 | 20.0 | 20.0 |
| Total Split（s） | 25.0 | 75.0 | 75.0 | 10.0 | 60.0 | 60.0 | 15.0 | 20.0 | 15.0 | 20.0 | 20.0 |
| Total Split（\％） | 20．8\％ | 62．5\％ | 62．5\％ | 8．3\％ | 50．0\％ | 50．0\％ | 12．5\％ | 16．7\％ | 12．5\％ | 16．7\％ | 16．7\％ |
| Yellow Time（s） | 3.0 | 5.0 | 5.0 | 3.0 | 5.0 | 5.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 7.0 | 7.0 | 5.0 | 7.0 | 7.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Min | Min | None | Min | Min | None | None | None | None | None |
| Act Effct Green（s） | 14.4 | 38.1 | 38.1 | 33.2 | 25.8 | 25.8 | 12.9 | 7.3 | 16.5 | 11.4 | 11.4 |
| Actuated g／C Ratio | 0.19 | 0.50 | 0.50 | 0.43 | 0.34 | 0.34 | 0.17 | 0.10 | 0.22 | 0.15 | 0.15 |
| v／c Ratio | 0.60 | 0.48 | 0.09 | 0.21 | 0.65 | 0.37 | 0.16 | 0.39 | 0.34 | 0.06 | 0.46 |
| Control Delay | 34.8 | 14.7 | 1.4 | 10.5 | 24.3 | 4.6 | 26.5 | 18.3 | 26.5 | 36.6 | 10.9 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 34.8 | 14.7 | 1.4 | 10.5 | 24.3 | 4.6 | 26.5 | 18.3 | 26.5 | 36.6 | 10.9 |
| LOS | C | B | A | B | C | A | C | B | C | D | B |
| Approach Delay |  | 18.8 |  |  | 20.2 |  |  | 21.1 |  | 19.6 |  |
| Approach LOS |  | B |  |  | C |  |  | C |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 76.6
Natural Cycle： 65
Control Type：Actuated－Uncoordinated
Maximum v／c Ratio： 0.65
Intersection Signal Delay： 19.5
Intersection LOS：B
Intersection Capacity Utilization 56．9\％
ICU Level of Service B
Analysis Period（min） 15
Splits and Phases：5：Akers \＆Constitution


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 0．2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 种中 | 「 |  | 快 | 「 |  |  |  |  |  | F |
| Traffic Vol，veh／h | 0 | 1389 | 13 | 0 | 1326 | 102 | 0 | 0 | 0 | 0 | 0 | 31 |
| Future Vol，veh／h | 0 | 1389 | 13 | 0 | 1326 | 102 | 0 | 0 | 0 | 0 | 0 | 31 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | － | － | 200 | － | － | 0 | － | － | － | － | － | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － |  |  | － | 0 |  |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 |  |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 0 | 1462 | 14 | 0 | 1396 | 107 | 0 | 0 | 0 | 0 | 0 | 33 |


| Major／Minor | Major1 | Major2 |  |  |  |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | － | 0 | 0 | － | － | 0 | － | － | 698 |
| Stage 1 | － | － | － | － | － | － | － |  |  |
| Stage 2 | － | － | － | － | － | － | － | － |  |
| Critical Hdwy | － | － | － | － | － | － | － | － | 7.14 |
| Critical Hdwy Stg 1 | － | － | － | － | － | － | － | － |  |
| Critical Hdwy Stg 2 | － | － | － | － | － | － | － | － |  |
| Follow－up Hdwy | － | － | － | － | － | － | － | － | 3.92 |
| Pot Cap－1 Maneuver | 0 | － | － | 0 | － | － | 0 | 0 | 328 |
| Stage 1 | 0 | － | － | 0 | － | － | 0 | 0 |  |
| Stage 2 | 0 | － | － | 0 | － | － | 0 | 0 |  |
| Platoon blocked，\％ |  | － | － |  | － | － |  |  |  |
| Mov Cap－1 Maneuver | － | － | － | － | － | － | － | 0 | 328 |
| Mov Cap－2 Maneuver | － | － | － | － | － | － | － | 0 |  |
| Stage 1 | － | － | － | － | － | － | － | 0 |  |
| Stage 2 | － | － | － | － | － | － | － | 0 |  |


| Approach | EB | WB | SB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay，$s$ | 0 | 0 | 17.2 |

HCM LOS
C

| Minor Lane／Major Mvmt | EBT | EBR | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity（veh／h） | - | - | - | -328 |
| HCM Lane V／C Ratio | - | - | - | -0.099 |
| HCM Control Delay（s） | - | - | - | -17.2 |
| HCM Lane LOS | - | - | - | - |
| HCM 95th \％tile Q（veh） | - | - | - | - |
| H． |  |  |  |  |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh 1.8 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4虫 | 「 | \％ | 个4中 | 「 |  |  | 「 |  |  | \％ |
| Traffic Vol，veh／h | 92 | 1352 | 71 | 42 | 1260 | 25 | 0 | 0 | 48 | 0 | 0 | 47 |
| Future Vol，veh／h | 92 | 1352 | 71 | 42 | 1260 | 25 | 0 | 0 | 48 | 0 | 0 | 47 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 335 | － | 285 | 420 | － | 300 | － | － | 0 | － | － | 0 |
| Veh in Median Storage，\＃ | － | 0 |  |  | 0 | － | － | 0 | － |  | 0 |  |
| Grade，\％ | － | 0 |  | － | 0 |  |  | 0 | － |  | 0 |  |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 97 | 1423 | 75 | 44 | 1326 | 26 | 0 | 0 | 51 | 0 | 0 | 49 |


| Major／Minor | Major1 |  | Major2 |  |  |  |  | Minor1 |  |  | Minor2 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1352 | 0 | 0 |  | 1498 | 0 |  | 0 | － | － | 712 | － | － | 663 |
| Stage 1 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Stage 2 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Critical Hdwy | 5.34 | － | － |  | 5.34 | － |  | － | － | － | 7.14 | － | － | 7.14 |
| Critical Hdwy Stg 1 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Critical Hdwy Stg 2 | － | － | － |  | － | － |  | － | － | － |  | － | － |  |
| Follow－up Hdwy | 3.12 | － | － |  | 3.12 | － |  | － | － | － | 3.92 | － | － | 3.92 |
| Pot Cap－1 Maneuver | 264 | － | － |  | 223 | － |  | － | 0 | 0 | 322 | 0 | 0 | 346 |
| Stage 1 | － | － | － |  | － | － |  | － | 0 | 0 | － | 0 | 0 |  |
| Stage 2 | － | － | － |  | － | － |  | － | 0 | 0 | － | 0 | 0 |  |
| Platoon blocked，\％ |  | － | － |  |  | － |  | － |  |  |  |  |  |  |
| Mov Cap－1 Maneuver | 264 | － | － |  | 223 | － |  | － | － | － | 322 | － | － | 346 |
| Mov Cap－2 Maneuver | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
| Stage 1 | － | － | － |  | － | － |  | － | － | － |  | － | － |  |
| Stage 2 | － | － | － |  | － | － |  | － | － | － | － | － | － |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  |  | WB |  |  |  | NB |  |  | SB |  |  |
| HCM Control Delay，s | 1.6 |  |  |  | 0.8 |  |  |  | 18.3 |  |  | 17.1 |  |  |
| HCM LOS |  |  |  |  |  |  |  |  | C |  |  | C |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | R SBLn1 |  |  |  |  |  |  |
| Capacity（veh／h） | 322 | 264 | － | － | 223 | － | － | － 346 |  |  |  |  |  |  |
| HCM Lane V／C Ratio | 0.157 | 0.367 | － |  | 0.198 | － | － | － 0.143 |  |  |  |  |  |  |
| HCM Control Delay（s） | 18.3 | 26.3 | － | － |  | － | － | － 17.1 |  |  |  |  |  |  |
| HCM Lane LOS | C | D | － | － | D | － | － | C |  |  |  |  |  |  |
| HCM 95th \％tile Q（veh） | 0.5 | 1.6 | － | － | 0.7 | － | － | － 0.5 |  |  |  |  |  |  |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh | 0.9 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 个性 |  | 「 | 恌 |  | 「 | 0 | 0 |  |  | 0 | 「 |
| Traffic Vol，veh／h | 0 | 1593 | 40 | 0 | 1229 | 263 |  |  | 0 | 0 |  |  |
| Future Vol，veh／h | 0 | 1593 | 40 | 0 | 1229 | 263 | 0 | 0 | 0 | 0 | 0 | 137 137 |
| Conflicting Peds，\＃hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | None | － | － | None | － |  | None | － | － | None |
| Storage Length | － | － | 200 | － | － | 0 | － | － | － | － |  | 0 |
| Veh in Median Storage，\＃ | － | 0 | － | － | 0 | － | － | － | － | － | 0 |  |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 |  |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 1677 | 42 | 0 | 1294 | 277 | 0 | 0 | 0 | 0 | 0 | 144 |


| Major／Minor | Major1 | Major2 |  |  |  |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | － | 0 | 0 | － | － | 0 | － | － | 647 |
| Stage 1 | － | － | － | － | － | － | － | － |  |
| Stage 2 | － | － | － | － | － | － | － | － |  |
| Critical Hdwy | － | － | － | － | － | － | － | － | 7.14 |
| Critical Hdwy Stg 1 | － | － | － | － | － | － | － | － |  |
| Critical Hdwy Stg 2 | － | － | － | － | － | － | － | － |  |
| Follow－up Hdwy | － | － | － | － | － | － | － | － | 3.92 |
| Pot Cap－1 Maneuver | 0 | － | － | 0 | － | － | 0 | 0 | 355 |
| Stage 1 | 0 | － | － | 0 | － | － | 0 | 0 |  |
| Stage 2 | 0 | － | － | 0 | － | － | 0 | 0 |  |
| Platoon blocked，\％ |  | － | － |  | － | － |  |  |  |
| Mov Cap－1 Maneuver | － | － | － | － | － | － | － | 0 | 355 |
| Mov Cap－2 Maneuver | － | － | － | － | － | － | － | 0 |  |
| Stage 1 | － | － | － | － | － | － | － | 0 |  |
| Stage 2 | － | － | － | － | － | － | － | 0 |  |
|  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  | WB |  |  | SB |  |  |
| HCM Control Delay，s | 0 |  |  | 0 |  |  | 21.9 |  |  |
| HCM LOS |  |  |  |  |  |  | C |  |  |
|  |  |  |  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt | EBT | EBR | WBT | WBR SBLn1 |  |  |  |  |  |
| Capacity（veh／h） | － | － | － | － 355 |  |  |  |  |  |
| HCM Lane V／C Ratio | － | － | － | － 0.406 |  |  |  |  |  |
| HCM Control Delay（s） | － | － | － | － 21.9 |  |  |  |  |  |
| HCM Lane LOS | － | － | － | －C |  |  |  |  |  |
| HCM 95th \％tile Q（veh） | － | － | － | － 1.9 |  |  |  |  |  |

Intersection: 2: Hannah Ridge \& Constitution

| Movement | EB | EB | EB | EB | EB | WB | WB | WB | WB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | T | R | L | T | T | T | R | L | L |
| Maximum Queue (ft) | 85 | 317 | 279 | 373 | 116 | 261 | 226 | 264 | 257 | 48 | 184 | 152 |
| Average Queue (ft) | 28 | 128 | 105 | 145 | 46 | 128 | 71 | 98 | 112 | 15 | 101 | 43 |
| 95th Queue (ft) | 61 | 277 | 244 | 321 | 93 | 210 | 168 | 208 | 220 | 41 | 163 | 113 |
| Link Distance (ft) |  | 1111 | 1111 | 1111 |  |  | 772 | 772 | 772 |  | 513 | 513 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 300 |  |  |  | 300 | 275 |  |  |  | 250 |  |  |
| Storage Blk Time (\%) |  | 1 |  | 1 |  | 0 | 0 |  | 0 |  |  |  |
| Queuing Penalty (veh) |  | 0 |  | 3 |  | 2 | 0 |  | 0 |  |  |  |

Intersection: 2: Hannah Ridge \& Constitution

| Movement | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | TR | L | T | R |
| Maximum Queue (ft) | 280 | 85 | 84 | 64 |
| Average Queue (ft) | 129 | 30 | 20 | 18 |
| 95th Queue (ft) | 235 | 62 | 56 | 44 |
| Link Distance (ft) | 513 |  | 378 |  |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  | 75 |  | 75 |
| Storage Blk Time (\%) |  | 1 | 0 | 0 |
| Queuing Penalty (veh) |  | 1 | 0 | 0 |

## Markup Summary

| dsdrice (6) |  |  |
| :---: | :---: | :---: |
|  | Subject: Resolved <br> Page Label: 1 <br> Lock: Unlocked <br> Status: <br> Checkmark: Unchecked <br> Author: dsdrice <br> Date: 8/25/2017 11:52:30 AM <br> Color: |  |
| $\begin{aligned} & \text { Feathergass } \\ & \text { and } \\ & \text { and Ad Ad filing number(s) } \\ & \text { 2017 } \end{aligned}$ | Subject: Text Box <br> Page Label: 1 <br> Lock: Unlocked <br> Status: <br> Checkmark: Unchecked <br> Author: dsdrice <br> Date: 8/25/2017 11:52:07 AM <br> Color: | Add filing number(s) |
|  | Subject: Callout <br> Page Label: 9 <br> Lock: Unlocked <br> Status: <br> Checkmark: Unchecked <br> Author: dsdrice <br> Date: 8/25/2017 1:35:55 PM <br> Color: | Has a decision been made regarding constructing the reach between Shawnee and the trail with Filing 4? |
| $\begin{aligned} & \text { ser } \\ & \text { for } \end{aligned}$ | Subject: Highlight <br> Page Label: 10 <br> Lock: Unlocked <br> Status: <br> Checkmark: Unchecked <br> Author: dsdrice <br> Date: 8/25/2017 1:39:24 PM <br> Color: |  |
|  | Subject: Cloud+ <br> Page Label: 13 <br> Lock: Unlocked <br> Status: <br> Checkmark: Unchecked <br> Author: dsdrice <br> Date: 8/25/2017 1:57:34 PM <br> Color: | Do we have escrow for these already? If not, how do we get up to the required $\$ 300,000$ ? |
|  | Subject: Cloud+ <br> Page Label: 19 <br> Lock: Unlocked <br> Status: <br> Checkmark: Unchecked <br> Author: dsdrice <br> Date: 8/25/2017 2:10:18 PM <br> Color: | 3 through lanes? |

