



June 8, 2026

Revised: August 19, 2026

Mayberry Communities
5155 North Academy Boulevard
Colorado Springs, Colorado 80918

Attention: Scott Souders, P.E.

Subject: Response to Review Comments
Geologic Hazards Evaluation and Preliminary Geotechnical Investigation
Mayberry, 540 Acre Site, PUD2
Highway 94 and Log Road
El Paso County, Colorado
CTL|T Project No. CS18969.004-105-L1 Rev

CTL|Thompson, Inc. (CTL|T) prepared a Geologic Hazard Evaluation and Preliminary Geotechnical Investigation (CTL|T Project No. CS18969.004-105, dated March 13, 2023), for the Mayberry, 540 Acre Site. Our firm has performed two design level geotechnical investigations that cover the area within PUD 2, (CTL|T Project No. CS19587.000-120, dated February 20, 2025 AND CTL|T Project No. CS19587.001-125, dated February 18, 2025).

The site is located west of Ellicott, Colorado, in El Paso County. We have been provided with a copy of the review comments from El Paso County (EPC) from October 2025. The review comments we received were markups on a pdf of our report. We understand that a revised submittal was provided to EPC in June 2026, to which we have been provided updated EPC comments and Colorado Geologic Survey (CGS) comments dated July 2026.

The following are a summary of the review comments and our responses.

October 2025 Review Comment in blue text box on Page 1 of our report:

"Should discuss and delineate Mayberry PUD #2"

And October 2025 Review Comment in blue text box on Fig. 1 of our report:

"Call out the specific project area for Mayberry PUD 2 with a boundary"

CTL|T Response:

PUD2 is a part of the 540-Acre Site, which is covered by this report. We have revised Figure 1 to include a boundary for PUD2 and we have added a Figure 2, which is a detail of the PUD2 area from Figure 1, since the PUD2 area is relatively small compared to the overall 540 Acre site.

October 2025 Review Comment in green text box on the cover page of our report:

"Provide a site-specific soils & geology report for phase 2. It is not possible from this report to discern the geologic hazards and constraints that may impact the phase 2 site."



Please review Section 8.4.9 of the Land Development Code to ensure the proper information is included in the report.”

And July 2026 CGS Comment:

“The CTL Thompson report (March 13, 2023) was prepared for the entire Mayberry site, not specifically for Phase 2, and the report did not include an Engineering Geology Map.”

CTL|T Response:

We assume the reference to “*phase 2*” is a reference to PUD2 which has been misstated. PUD2 is a part of the 540-Acre Site which is covered by this report.

We believe our report was prepared in conformance with **Section 8.4.9 — Geology and Soils Standards and Reports** of the Land Development Code. If there is an issue with a topic EPC would like identified in more detail, please specifically state the issue.

PUD2 is bracketed by borings TH-1, TH-3, and TH-6. As stated in the Limitations section of our report on page 15 *“The recommendations and conclusions presented in this report were prepared based on conditions disclosed by our exploratory borings, geologic reconnaissance, engineering analyses, and our experience.”* In this case, our experience includes two additional design level investigations conducted within PUD2. We have revised Figure 1 to include an indication of the locations of the borings for the PUD2 investigations, and the laboratory testing is included in Appendices A and B of this letter. This information has been included to demonstrate that the subsurface conditions are consistent with the conditions discussed in our Geologic Hazard Evaluation and Preliminary Geotechnical Investigation (CTL|T Project No. CS18969.004-105, dated March 13, 2023) which includes PUD2.

Conditions we identified that may pose hazards or constraints to development include expansive soil lenses and collapse prone soils with low to moderate collapse potential. One subsurface sample from a boring in the proposed park development, south of Tract K, classified as having moderate collapse potential, and samples from the borings within the Tract K residential site were identified as loose, surficial sands. These conditions are sporadic and may exist anywhere in the site, therefore a map is not appropriate. Attempting to map where expansive or collapse prone soils may exist at the site would falsely suggest that we believe these conditions could occur in some areas of the site and not in others.

October 2025 Review Comment in blue text box on Page 1 of our report:

“Summarization should also be in terms of notable geological hazards and constraints.”

CTL|T Response:

We did not identify geologic hazards that we believe preclude the development of the project. Conditions we identified that may pose hazards or constraints to development include expansive soil lenses and collapse prone soils with low to moderate collapse potential. The design level Geotechnical Investigation reports for PUD2 included one subsurface sample in the proposed park development south of Tract K which classified as having a moderate collapse potential, and the residential report identified areas of loose material.



October 2025 Review Comments in overlapping blue text boxes on Page 2 of our report:

“Recommendations and Mitigation Plans. The report shall include recommendations and mitigation concerning but not limited to the following:

- *Allowable soil pressure*

Include discussion note if basements are or are not recommended.”

CTL|T Response:

The design level Geotechnical Investigation reports for PUD2 included a 2-foot sub-excavation for proposed Concessions Building in the park development which will remediate materials which may have collapse potential, and the residential report includes individual structure recommendations which includes a 3-foot sub-excavation for identified structures which will mitigate loose material.

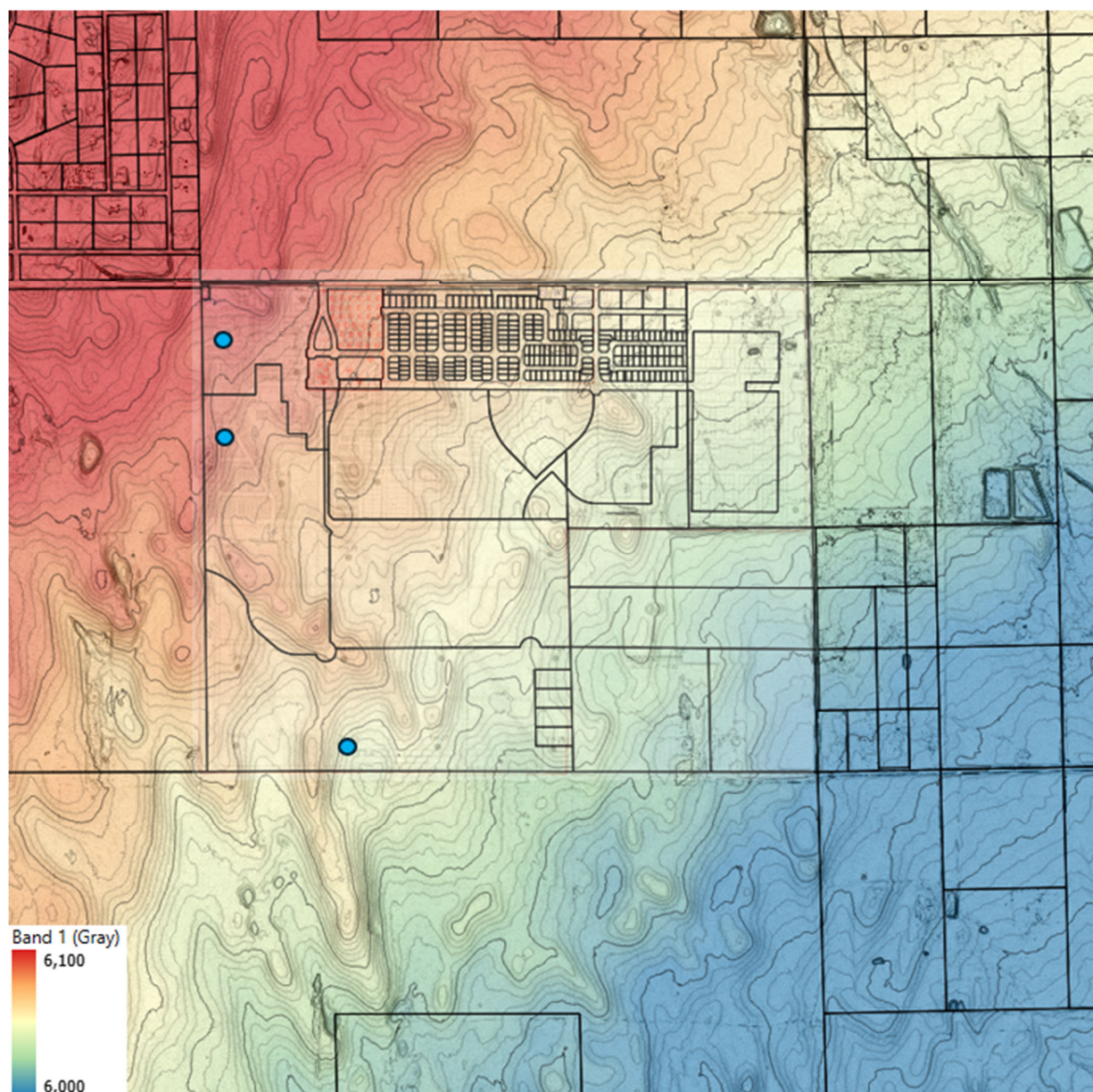
The design level Geotechnical Investigations both recommend an allowable soil pressure of 3,000 psf for structures. Retaining walls within the park area have a recommended allowable soil pressure of 2,000 psf.

July 2026 CGS Comment:

“As stated in our previous review (October 22, 2025), if basements are planned, CGS recommends that the [C]ounty require groundwater monitoring/observation to verify that proposed floor levels are at least three feet above maximum anticipated groundwater levels and maintained year-round. This monitoring/observation program should be conducted immediately and/or before the installation of public infrastructure to determine if basements are feasible. CGS recommends no below-grade construction unless a groundwater monitoring/observation program is implemented.”

CTL|T Response:

The CGS recommendation for a groundwater monitoring/observation program is excessive and unwarranted. Our firm has advanced over 100 borings at the Mayberry site during the past 6 years. Shallow groundwater has not been identified as a geologic hazard or engineering constraint. We have reviewed historic aerial imagery of the site and conducted numerous site visits. No surface evidence of springs or seeps were found. No channelized drainages were observed within the site. Localized groundwater greater than 10 feet below the surface and outside of the PUD2 project limits does not justify a costly groundwater monitoring program that would delay development for over a year. An elevation shaded DEM overlaying a slope shaded DEM of the Mayberry Site with 2-foot contours generated from the 2018 3DEP East CO El Paso 1-meter resolution Lidar is provided below. The three borings where groundwater was encountered are indicated with blue dots.



Topography of Mayberry Site and Surrounding Area



July 2026 CGS Comment:

Regarding the CTL Thompson report (March 13, 2023):

“Additionally groundwater was encountered in CTL’s borings at depths of 10.5 to 15 feet within the western portion of the Mayberry development.”

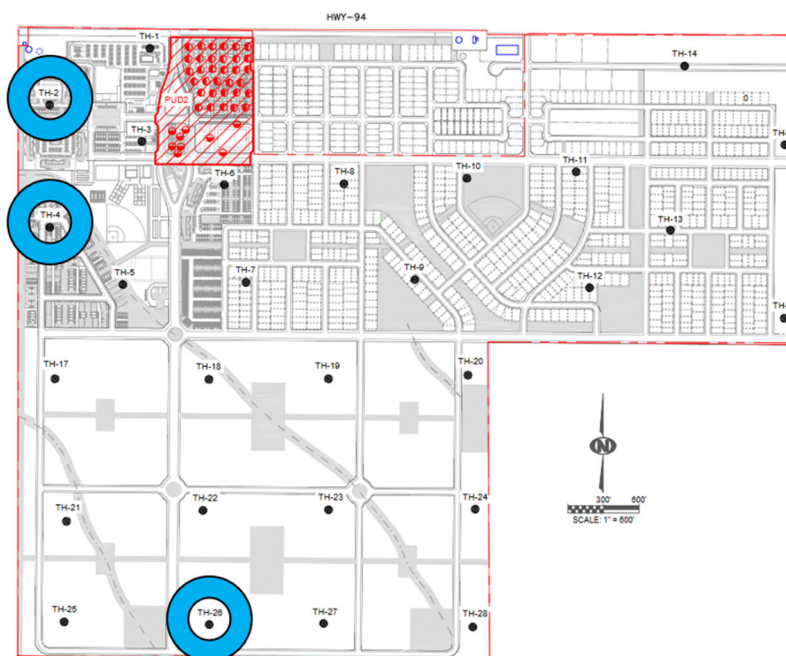
And:

“The PUD development plan (LJA, June 15, 2026) includes a Soils Note, stating: ‘Soil report dated June 8, 2026 (CTL [P]roject [N]o. [CS]18969.001-115-L1) contains revised figures and development guidance CTL did not identify geologic hazards that preclude the development of the project. Conditions identified that may pose hazards or constraints to development include expansive soil lenses and collapse-prone soils with low to moderate collapse potential. Design level guidelines are included in the report.’ The note includes no mention of shallow groundwater or mitigation measures.

CTL|T Response:

CTL|T has conducted two additional design level investigations within PUD2. We have revised Figure 1 to include an indication of the locations of the borings for the PUD2 investigations, and the laboratory testing is included in Appendices A and B of this letter. No groundwater was encountered in these investigations.

This investigation (CTL|T Project No. CS18969.00-105) that covers the 540-acre site, including PUD2, had three borings in which we observed groundwater. The boring numbers are TH-2, TH-4, and TH-26. These three borings lie along the western edge (TH-2 and TH-4) and southern edge (TH-26) of the overall development, and are well outside of the boundary of PUD2. A screenshot of the revised Fig. 1 (attached) with the locations of borings TH-2, TH-4, and TH-26 (circled in light blue) relative to the location of PUD2 (outlined in red) within the overall development is included below:





Shallow groundwater was not identified within PUD2, therefore, it does not require mitigation measures.

Limitations

This letter is based upon our observations, soil conditions found during our investigation, and our experience. We believe this investigation was conducted with the level of care and skill normally used by geotechnical engineers practicing under similar conditions. No warranty, express or implied, is made. If we can be of further service in discussing the contents of this letter or in the analysis of the influence of soils on the performance of the pavement section, please contact the undersigned.

If we can be of further service in discussing the contents of this report, please let us know.

Respectfully submitted,

CTL|THOMPSON, INC.

Gwendolyn Eberhart, P.E.
Senior Associate Engineer



Reviewed by:

Jeffrey M. Jones, P.E.
Principal Engineer

GE:JMJ:cw

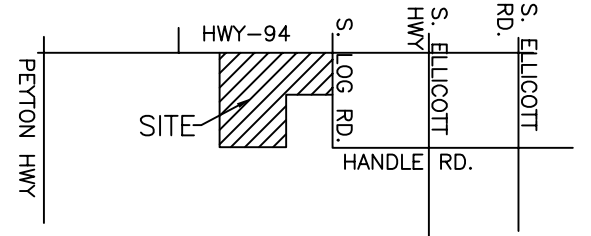
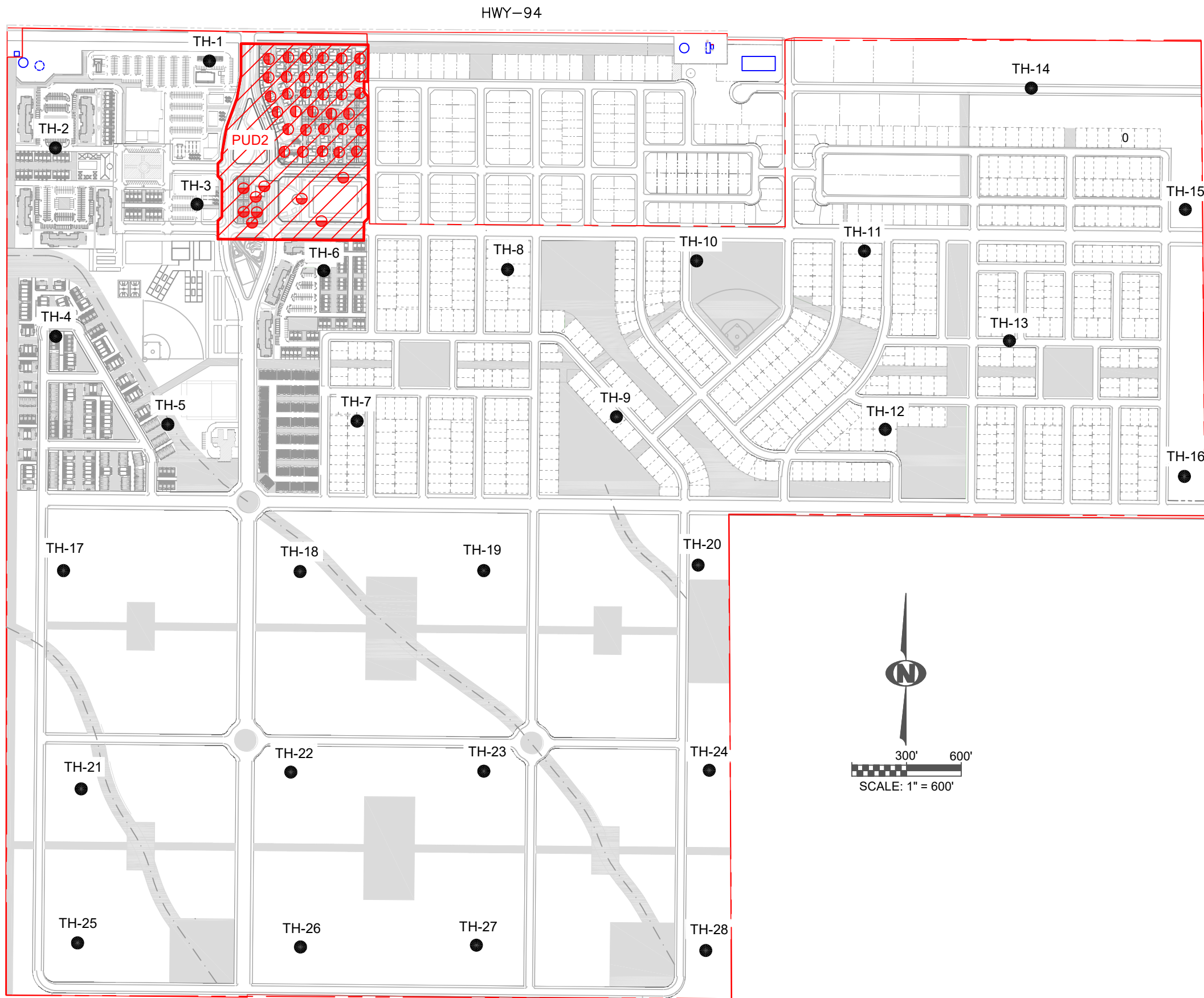
Via Email: scottssouders@mayberrycoloradosprings.com;

Attachments: Revised Figure 1

Figure 2

Appendix A — select pages from CS19587.000-120, dated February 20, 2025

Appendix B — select pages from CS19587.001-125, dated February 18, 2025

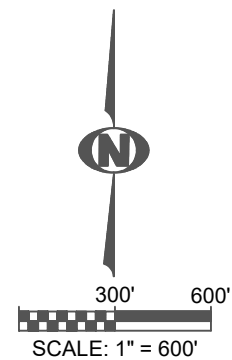


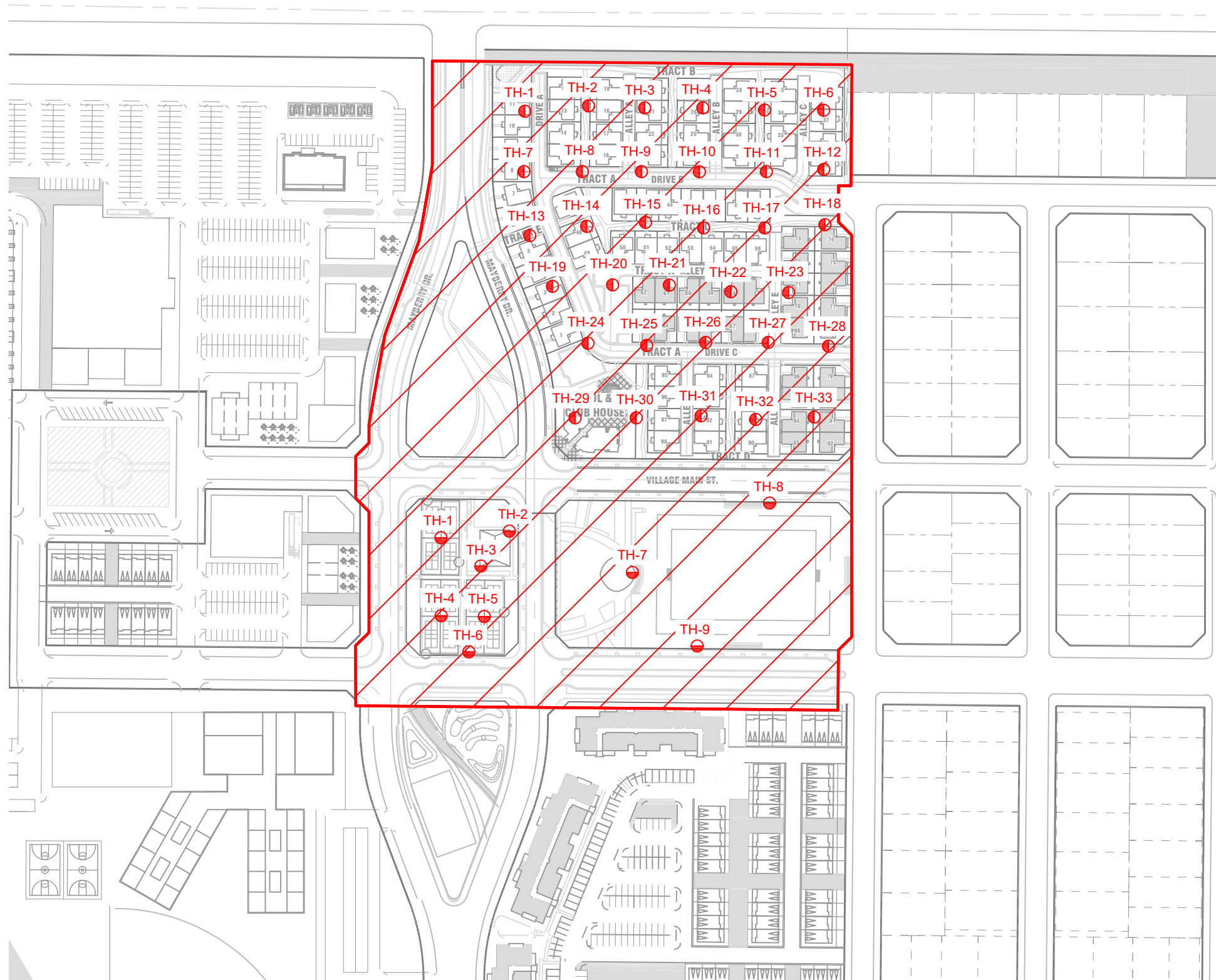
VICINITY MAP

(NOT TO SCALE)

LEGEND:

- TH-1
● APPROXIMATE LOCATION OF EXPLORATORY BORING.
- APPROXIMATE LOCATION OF EXPLORATORY BORING (CTL/T PROJECT NO. CS19587.000-120).
- APPROXIMATE LOCATION OF EXPLORATORY BORING (CTL/T PROJECT NO. CS19587.001-125).
- ▨ APPROXIMATE BOUNDARY PUD2
- - - APPROXIMATE SITE BOUNDARY



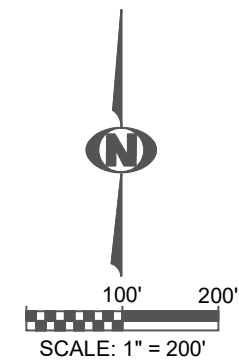


LEGEND:

-  APPROXIMATE LOCATION OF EXPLORATORY BORING (CTL/T PROJECT NO. CS19587.000-120).
-  APPROXIMATE LOCATION OF EXPLORATORY BORING (CTL/T PROJECT NO. CS19587.001-125).
-  APPROXIMATE BOUNDARY PUD2

NOTES:

1. BASE DRAWING OF PUD2 WAS PROVIDED BY LJA (DATED JUNE 3, 2026).

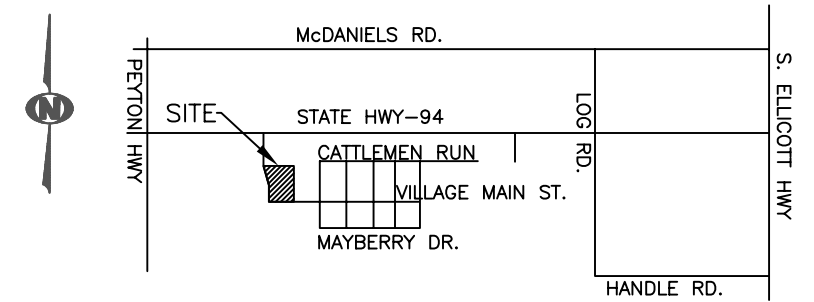


Location of Exploratory Borings - PUD2



APPENDIX A

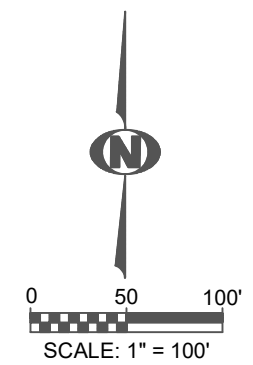
Select Pages from CS19587.000-120, dated February 20, 2025



VICINITY MAP
(NOT TO SCALE)

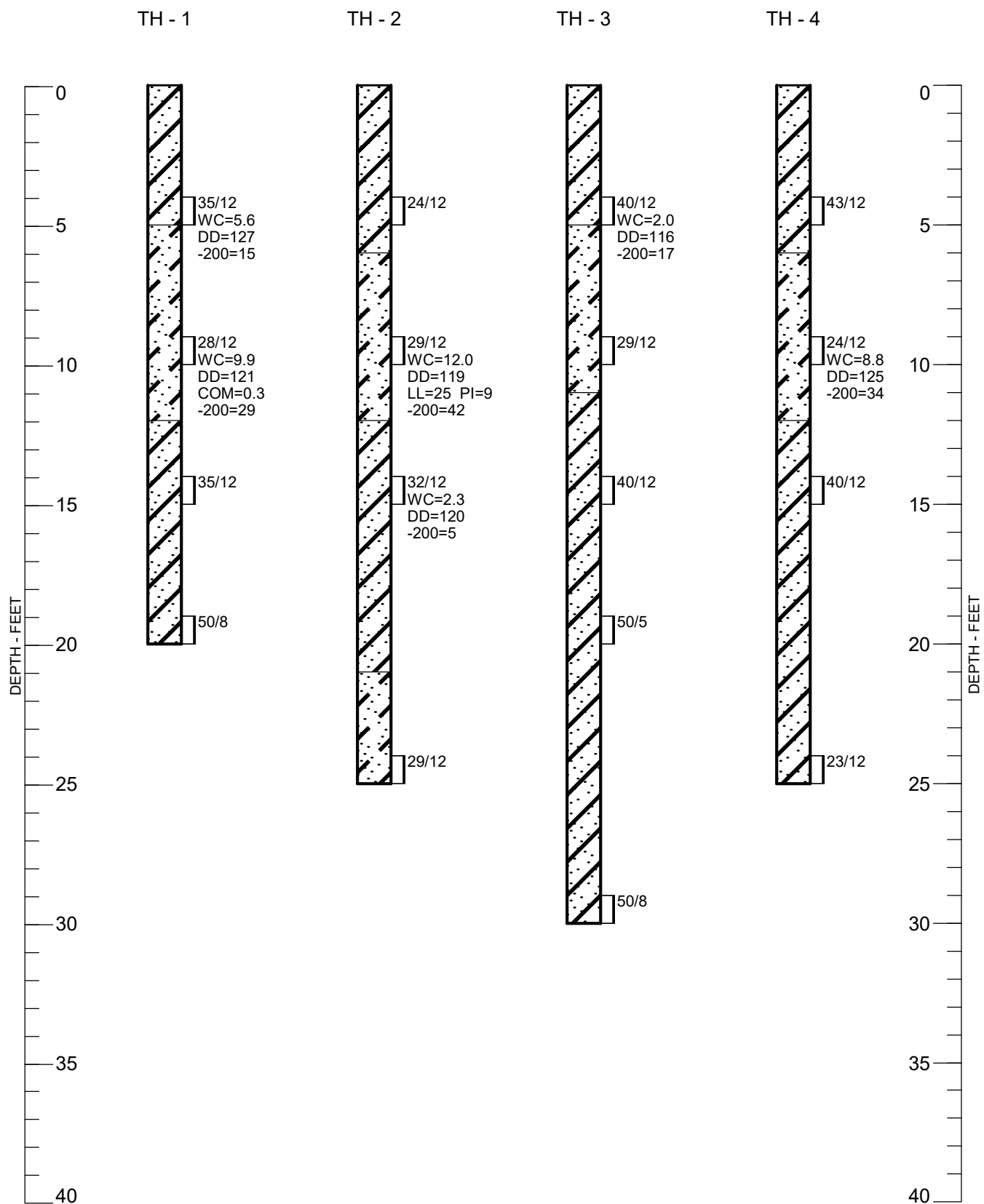
LEGEND:

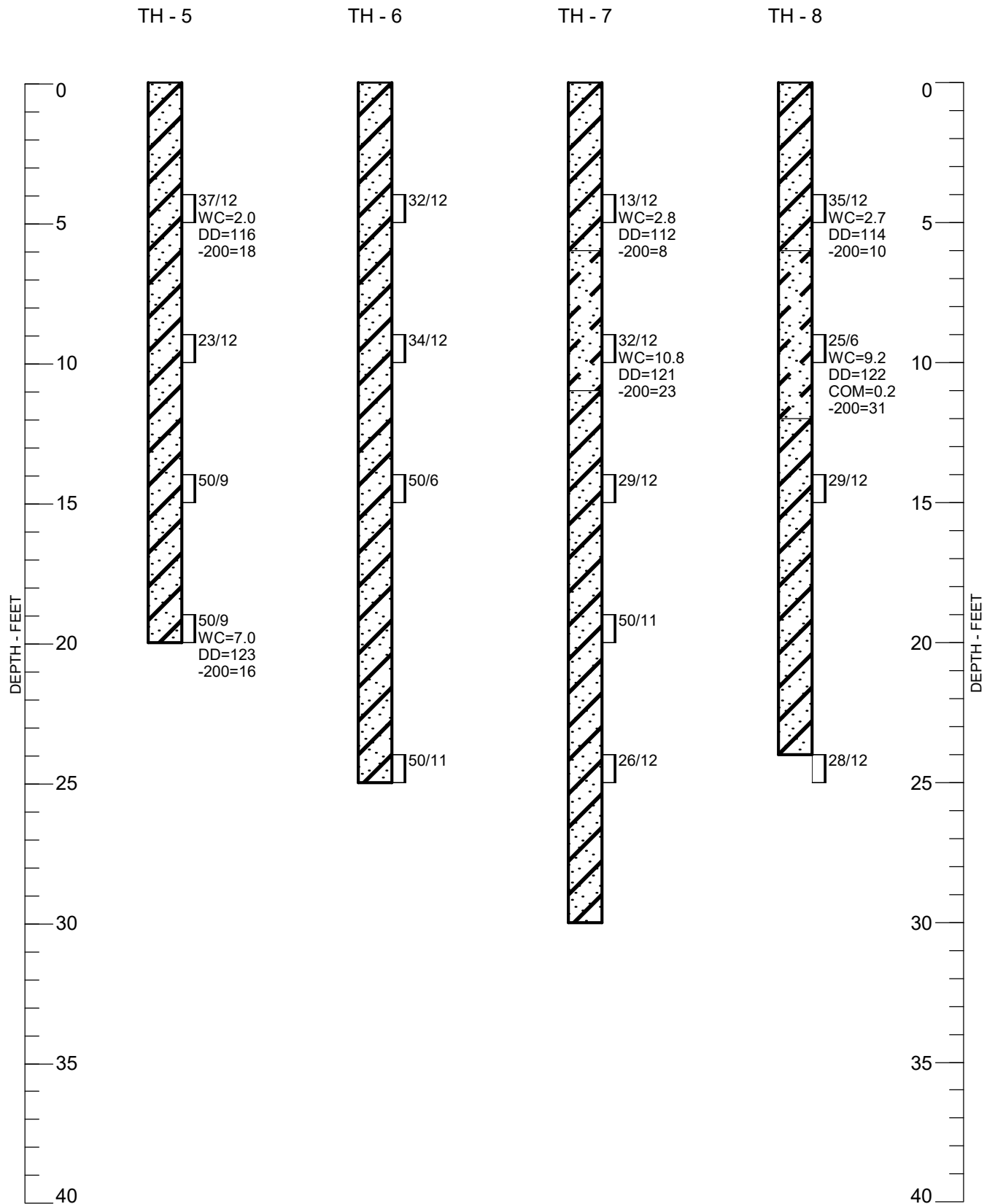
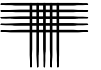
TH-1 APPROXIMATE LOCATION OF EXPLORATORY BORING.

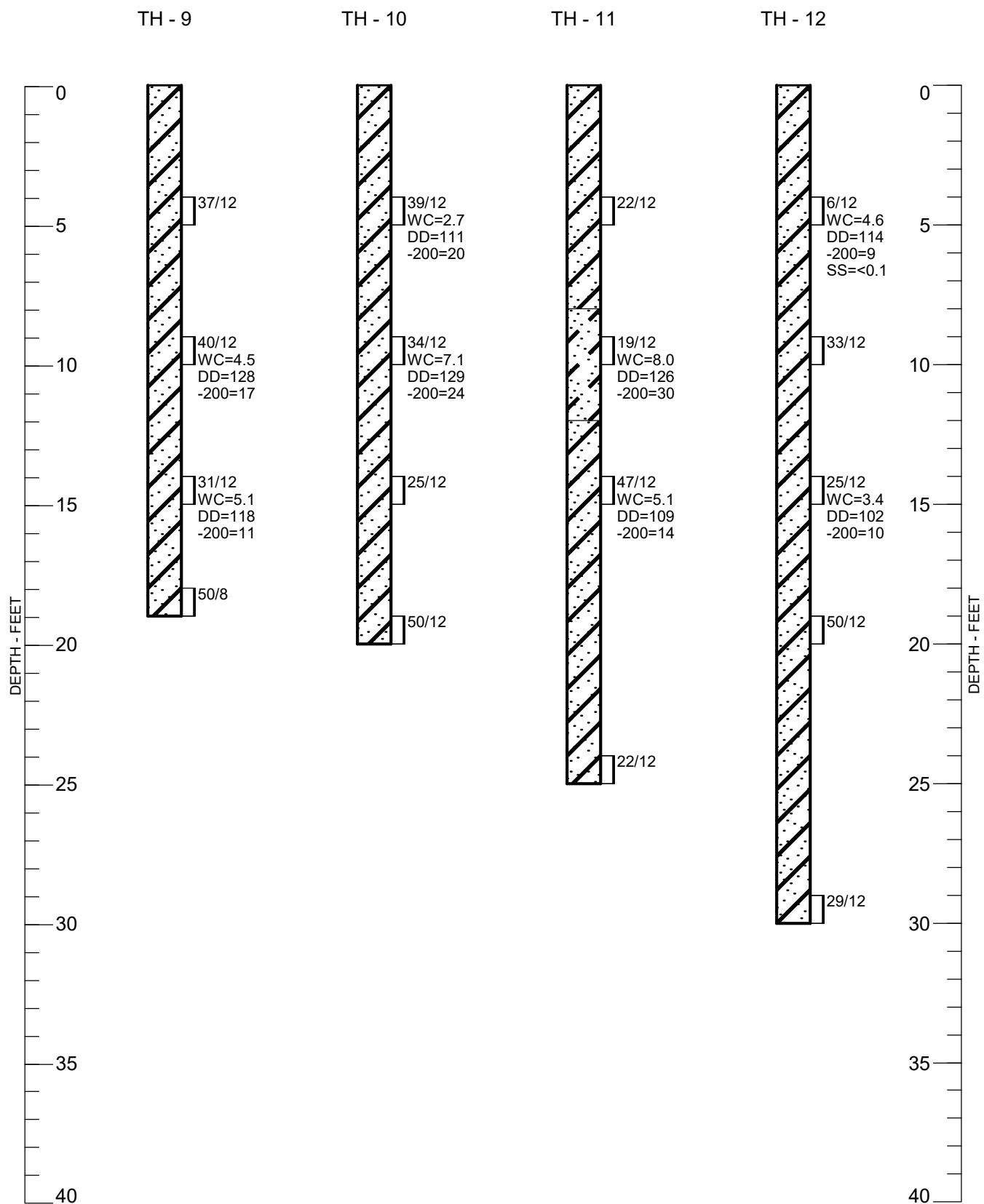


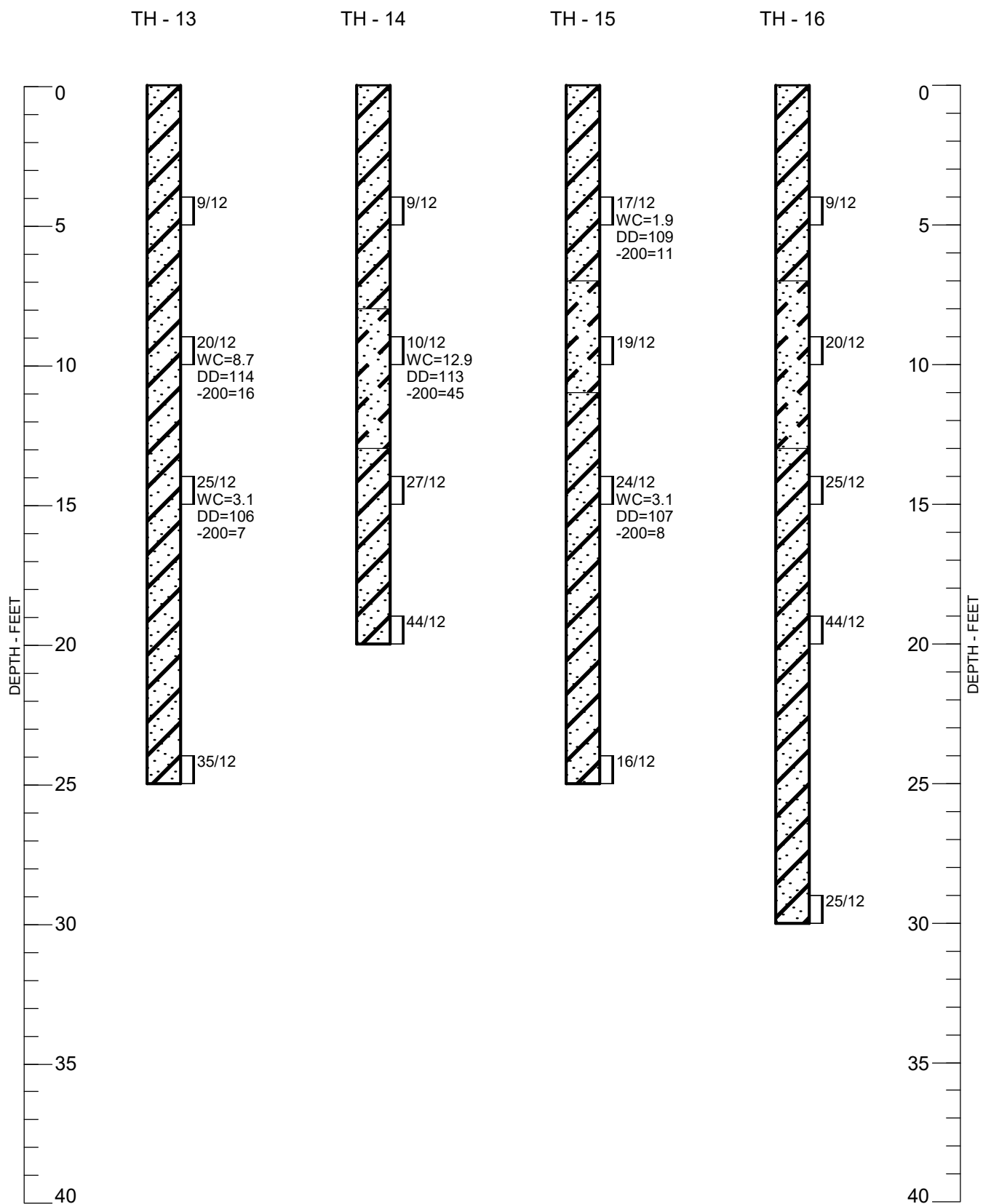
Location of
Exploratory
Borings

FIG. 1

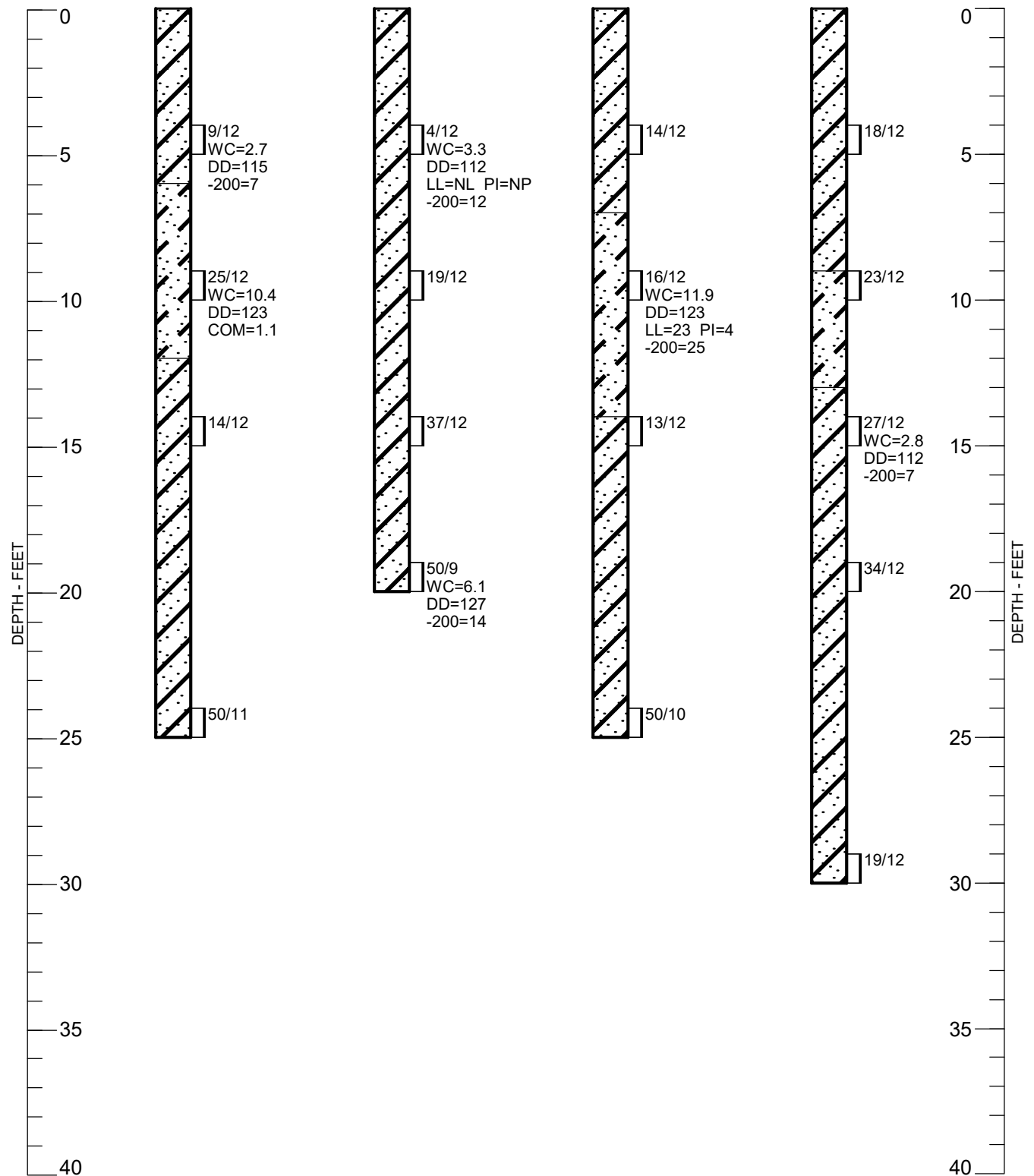


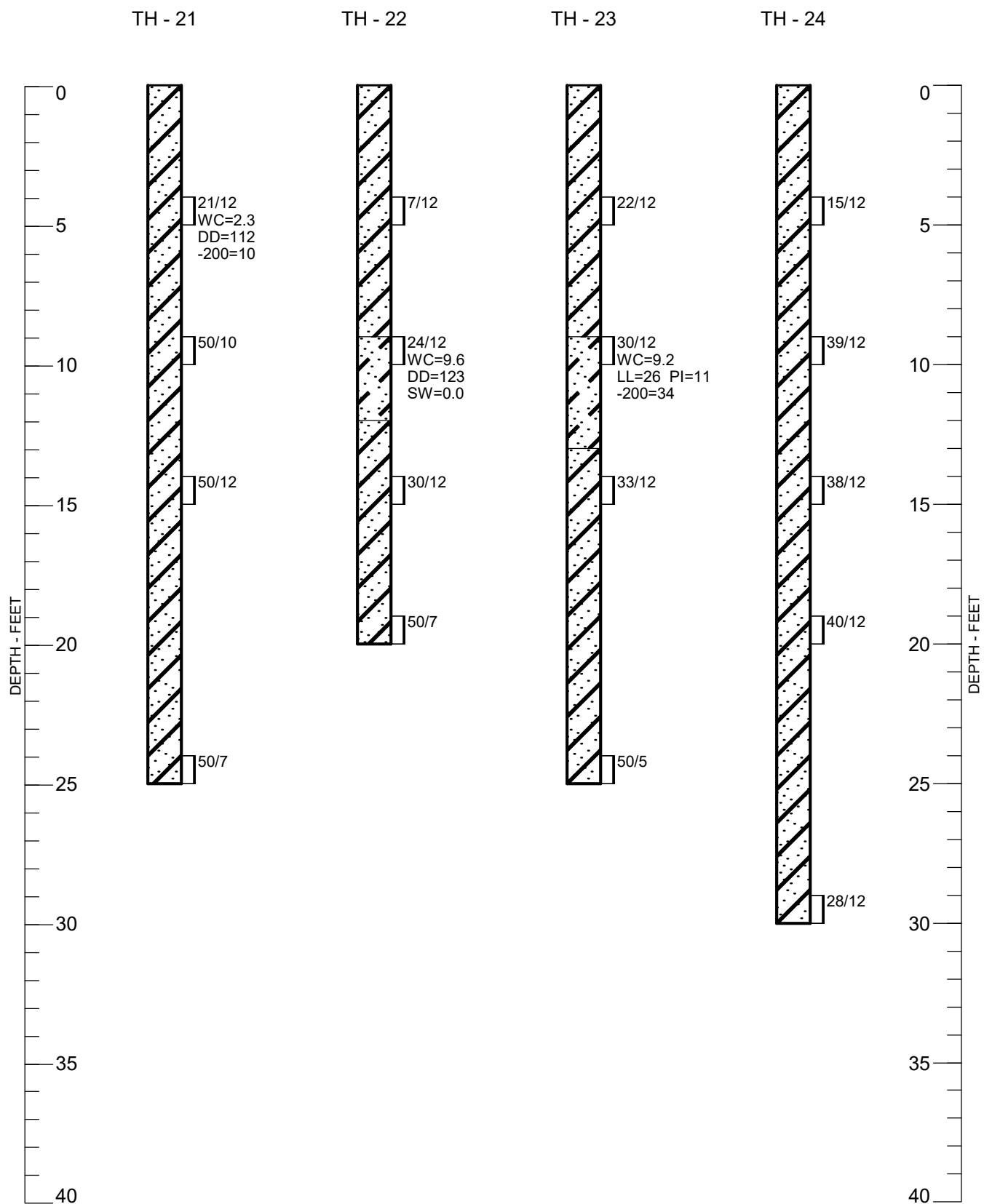


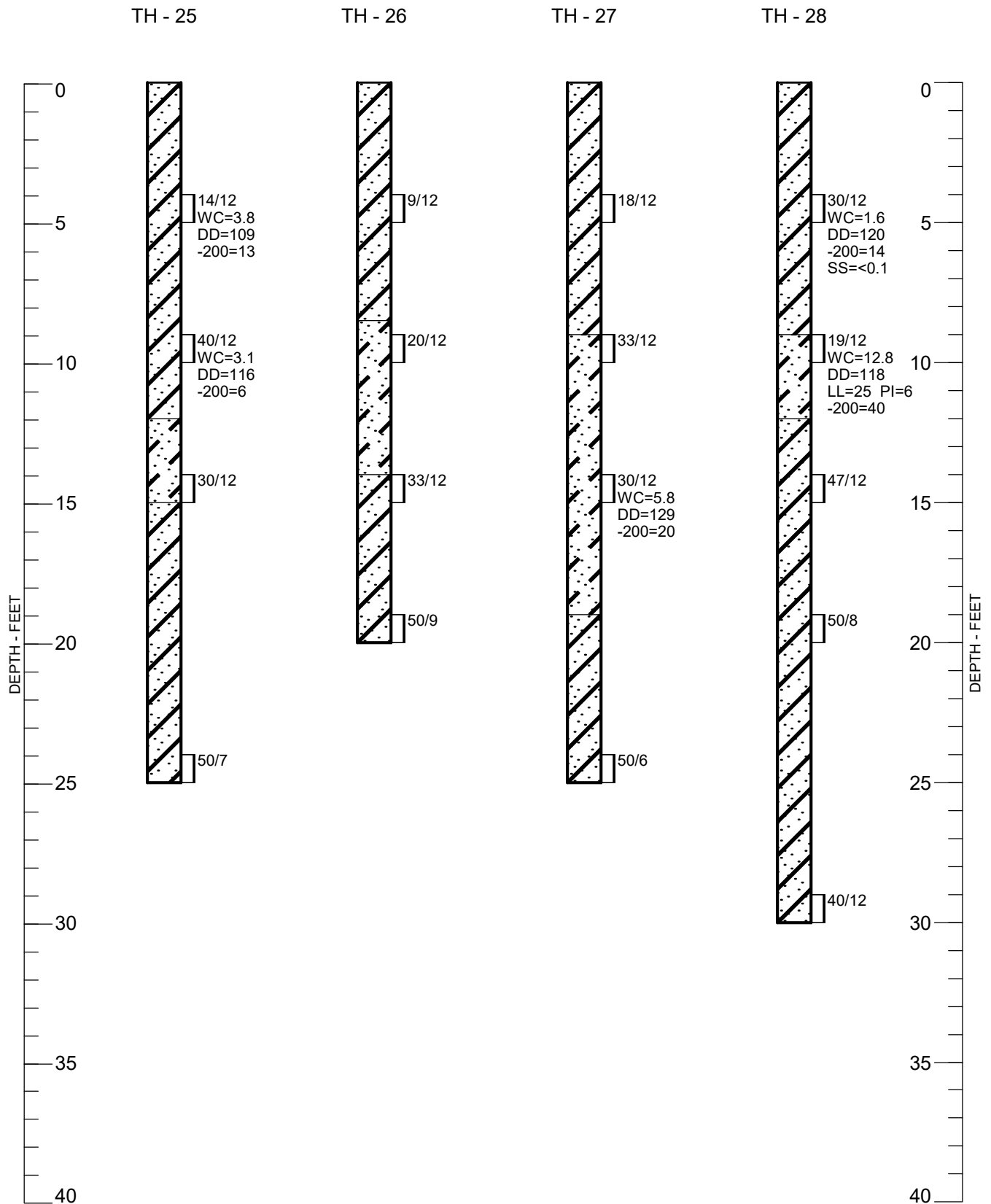


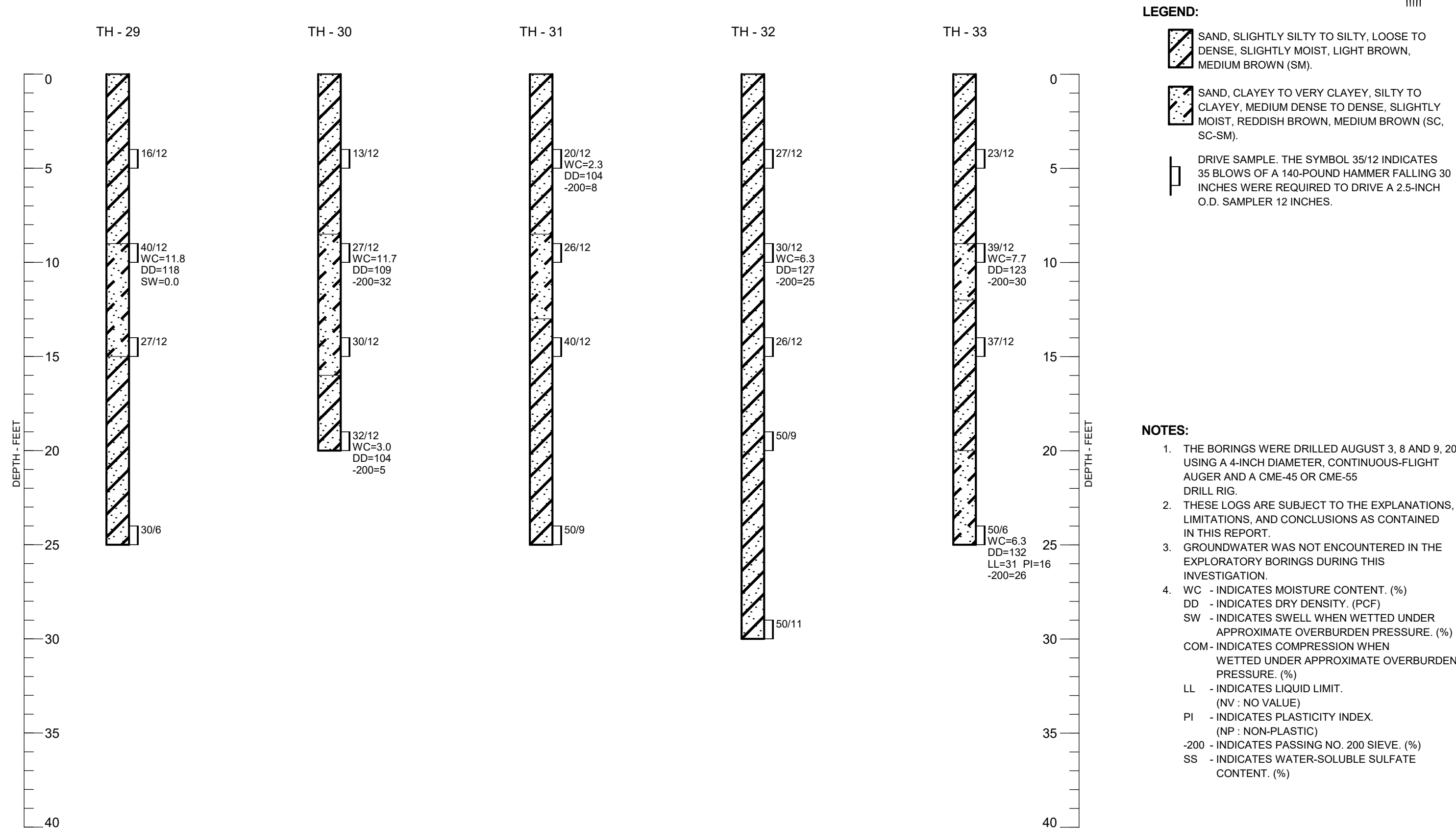


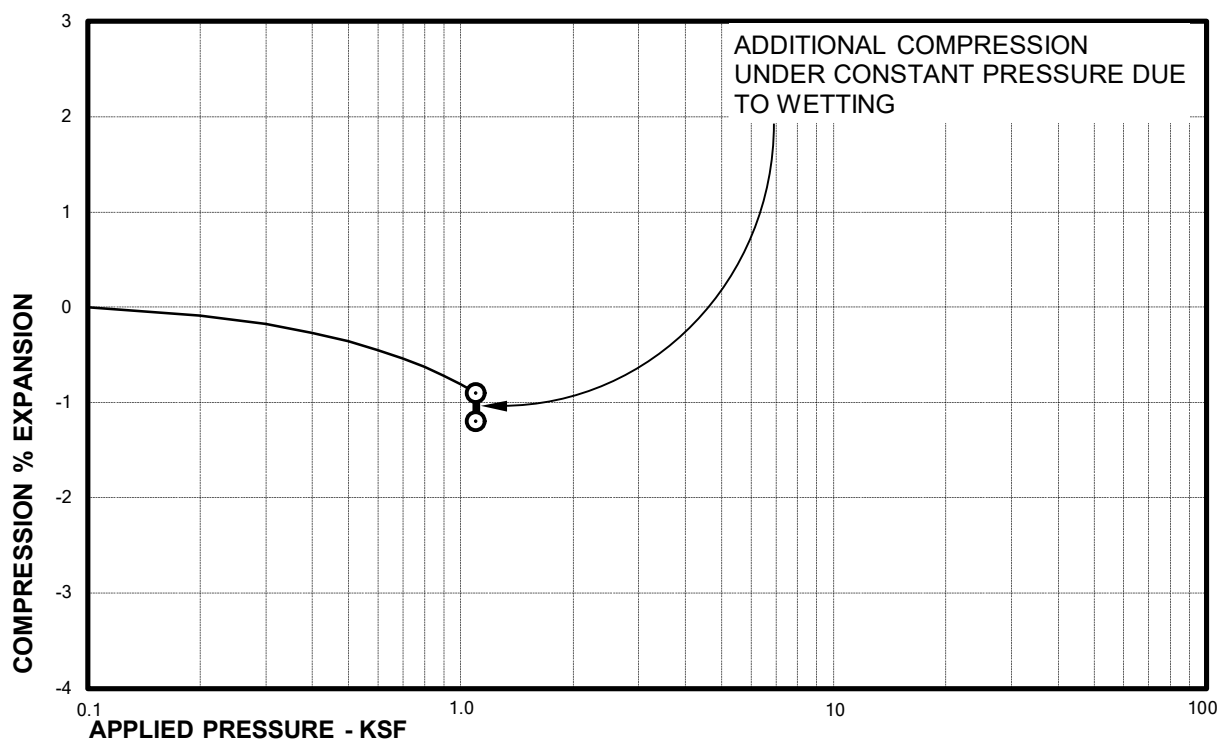
TH - 20





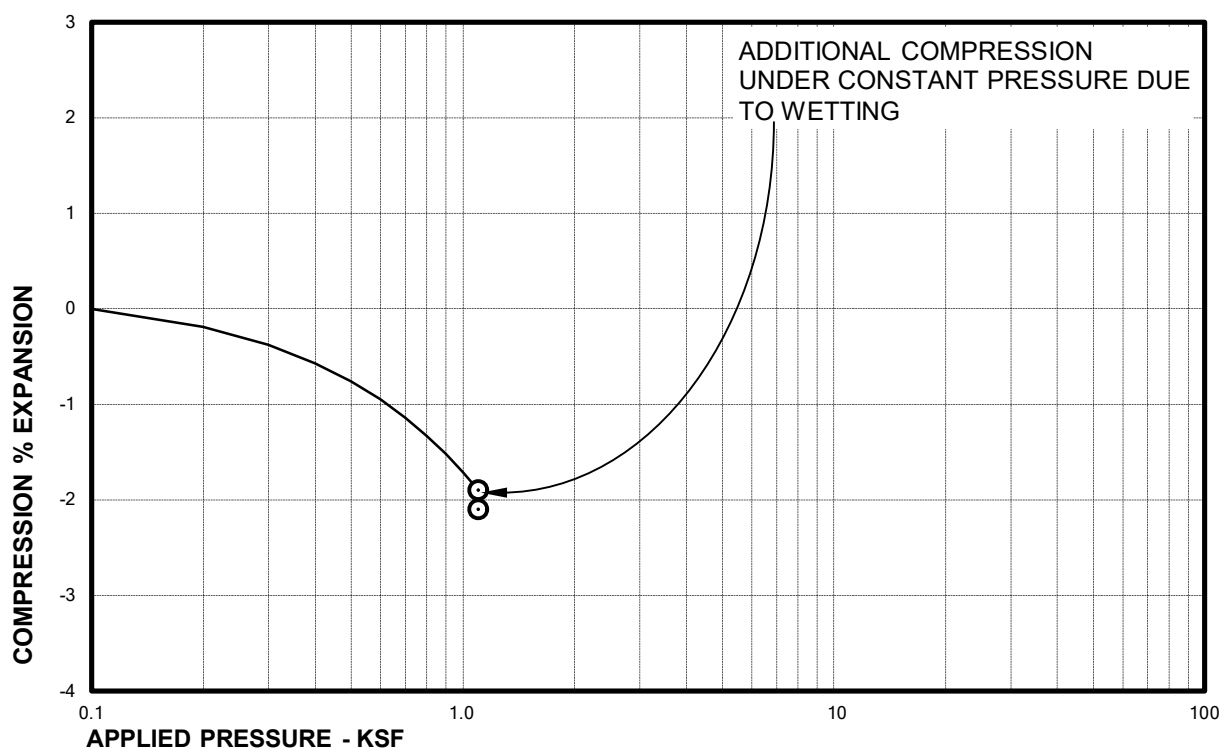






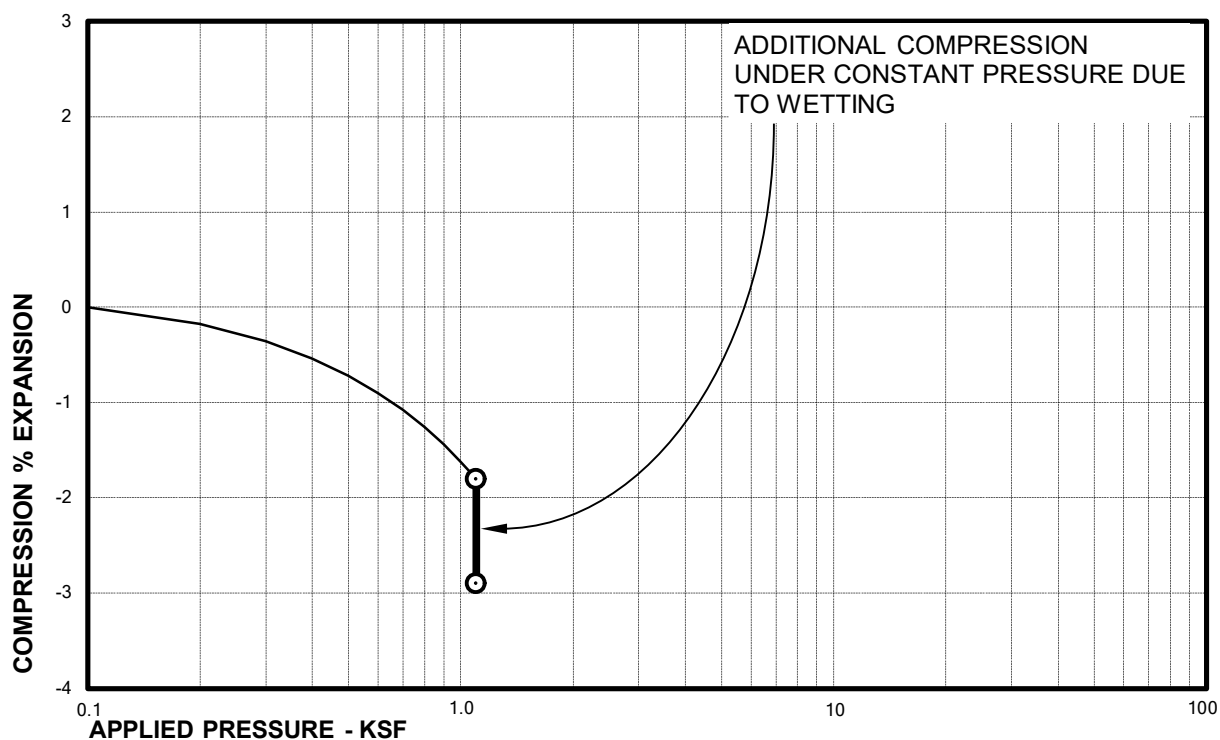
Sample of SAND, CLAYEY (SC)
From TH-1 AT 9 FEET

DRY UNIT WEIGHT= 121 PCF
MOISTURE CONTENT= 9.9 %



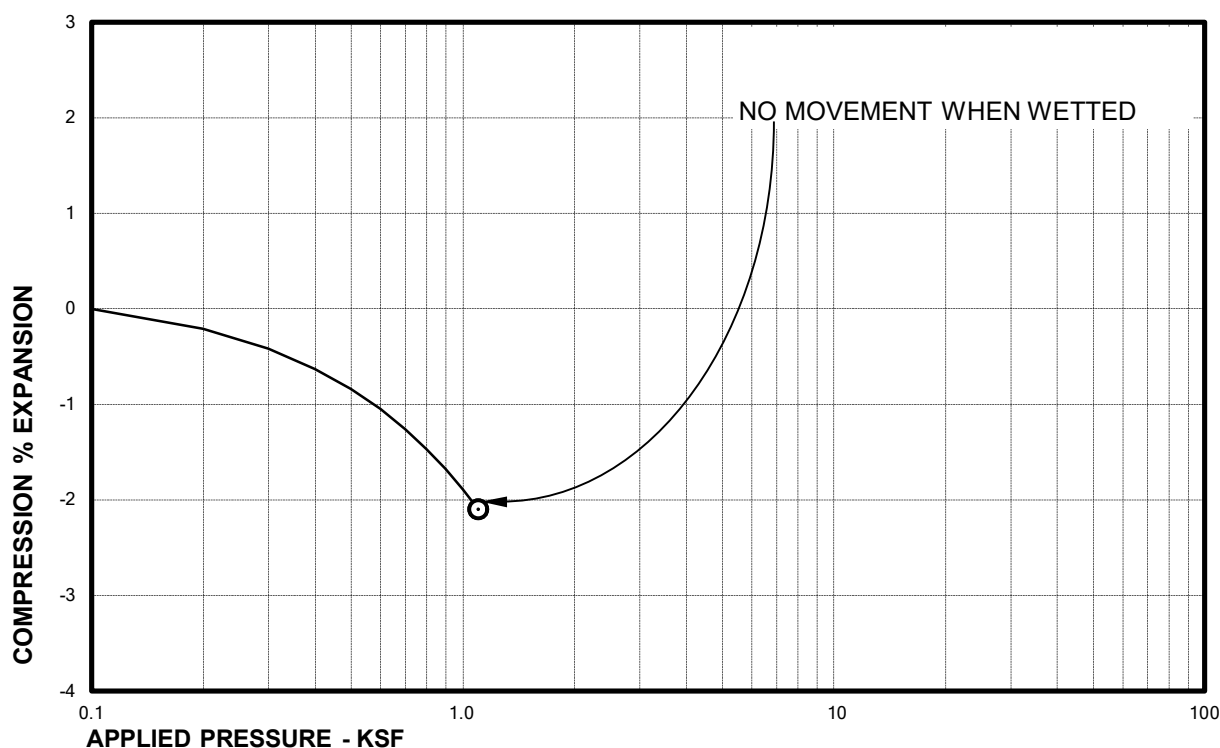
Sample of SAND, CLAYEY (SC)
From TH-8 AT 9 FEET

DRY UNIT WEIGHT= 120 PCF
MOISTURE CONTENT= 9.2 %



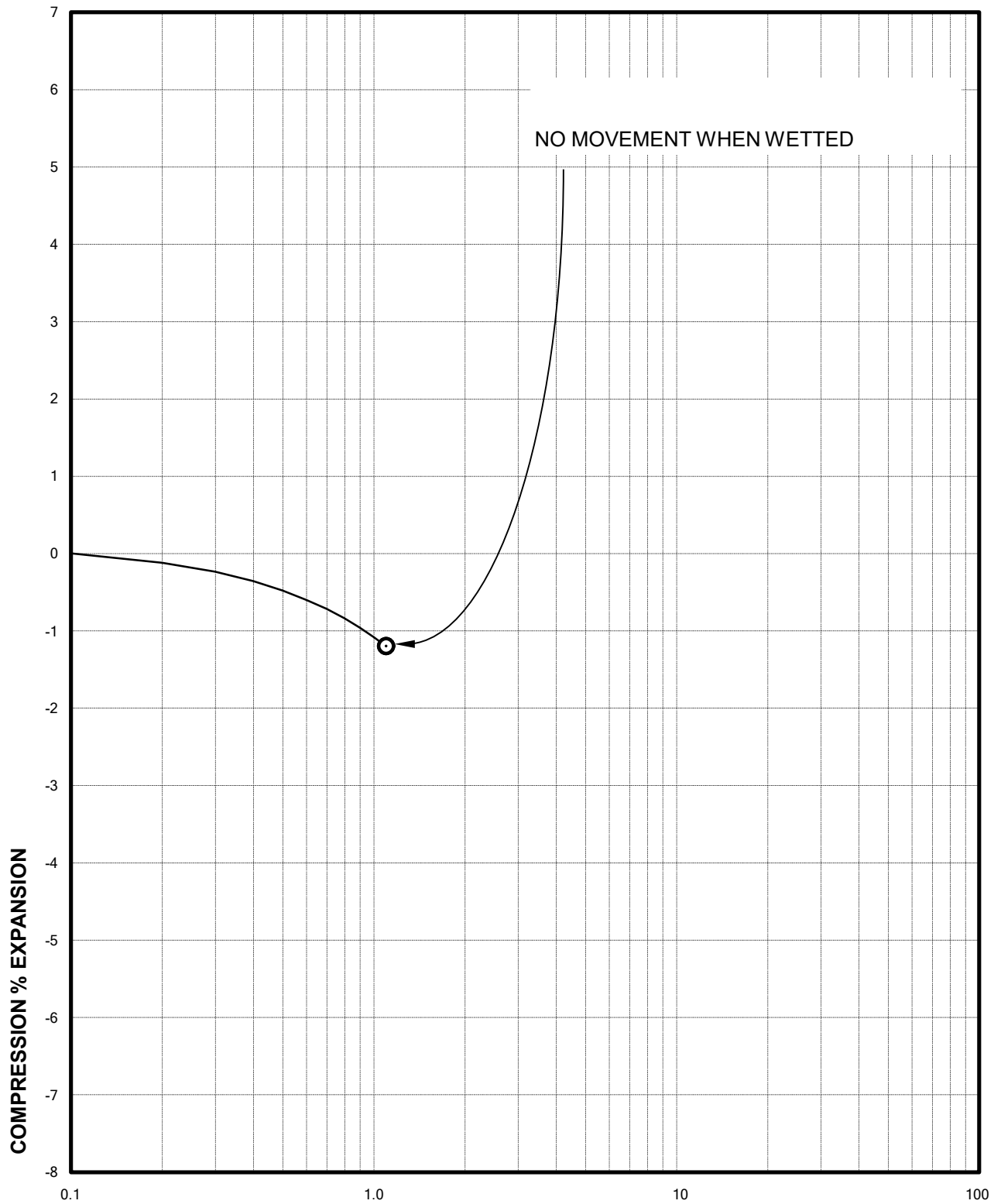
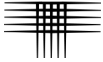
Sample of SAND, CLAYEY (SC)
From TH-17 AT 9 FEET

DRY UNIT WEIGHT= 123 PCF
MOISTURE CONTENT= 10.4 %



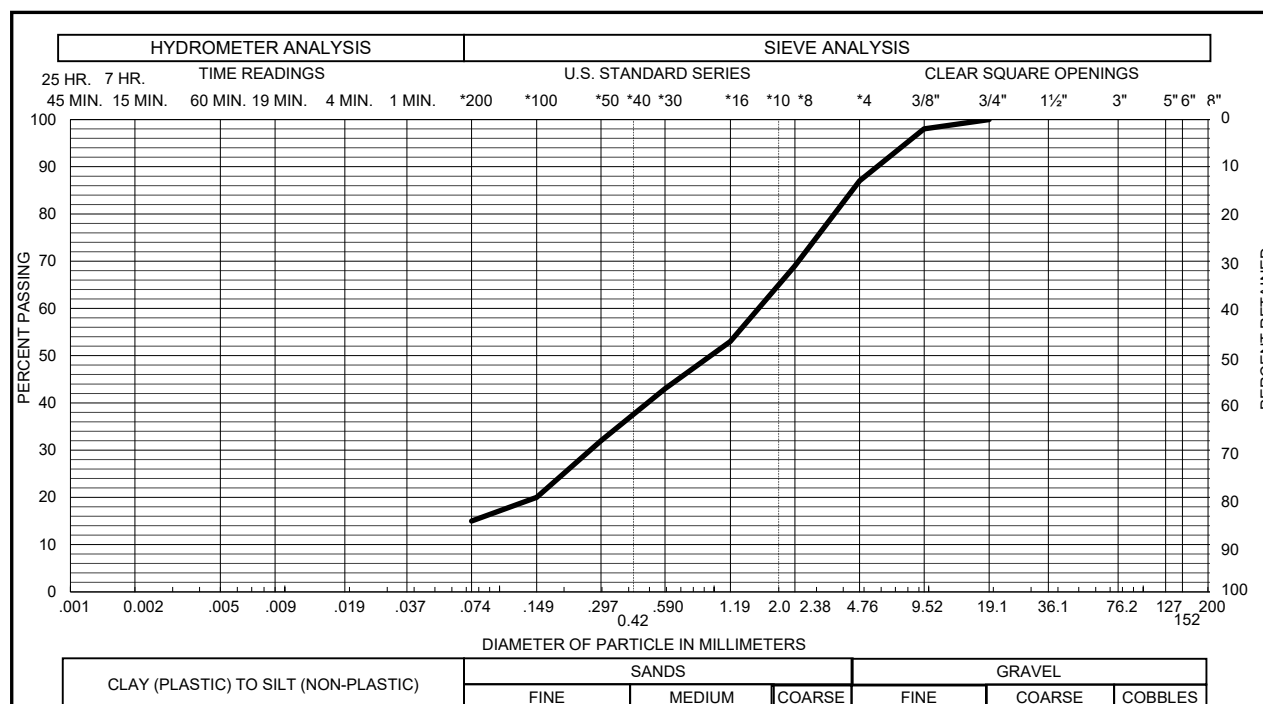
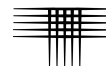
Sample of SAND, CLAYEY (SC)
From TH-22 AT 9 FEET

DRY UNIT WEIGHT= 123 PCF
MOISTURE CONTENT= 9.6 %

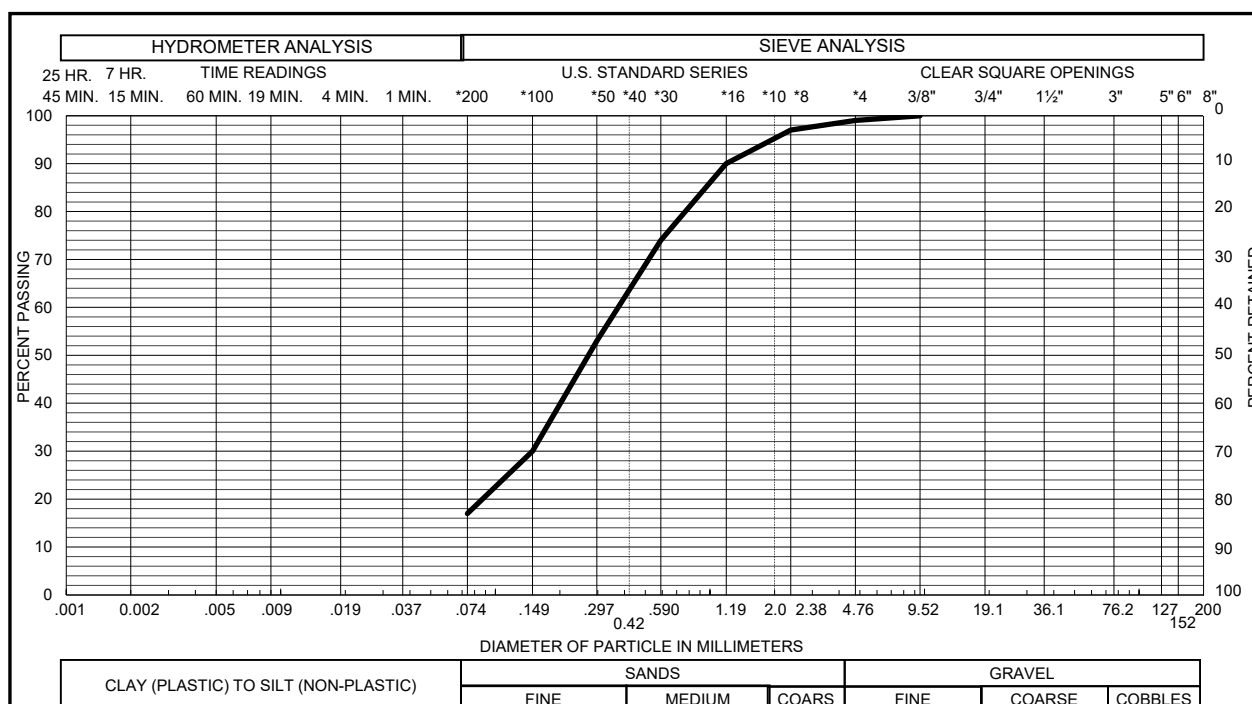


Sample of SAND, CLAYEY (SC)
From TH-29 AT 9 FEET

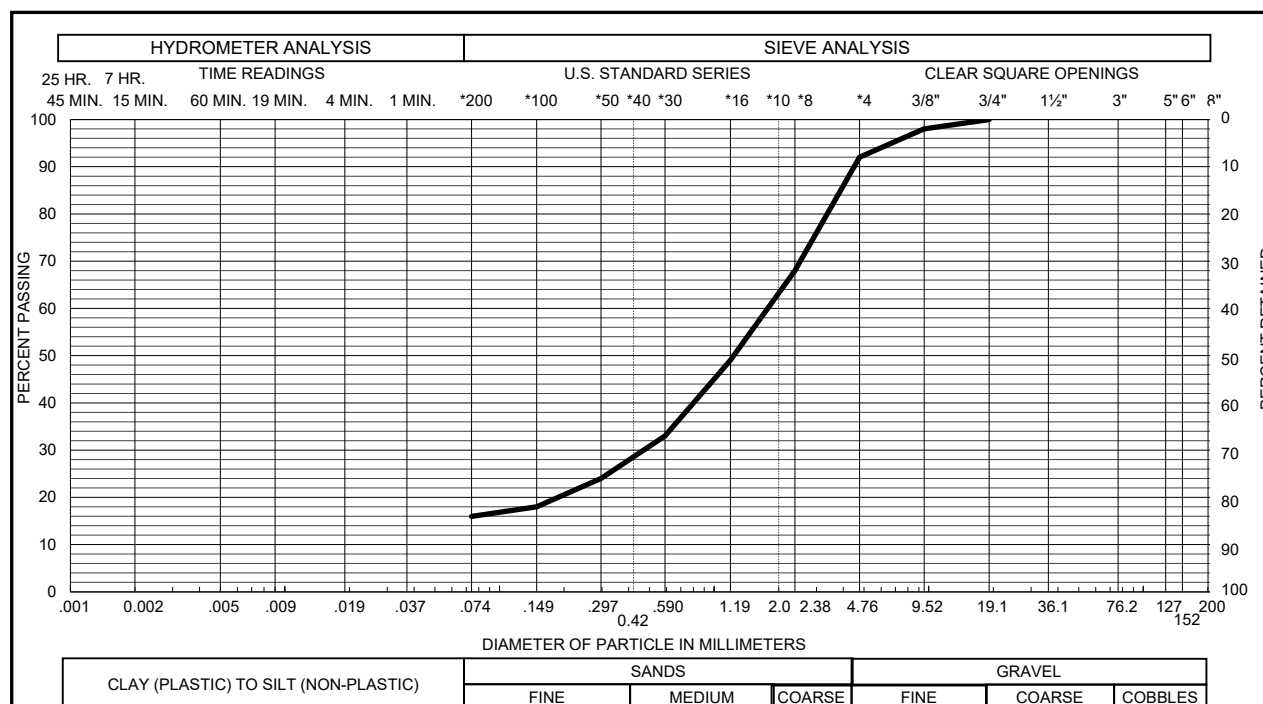
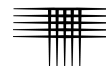
DRY UNIT WEIGHT= 118 PCF
MOISTURE CONTENT= 11.8 %



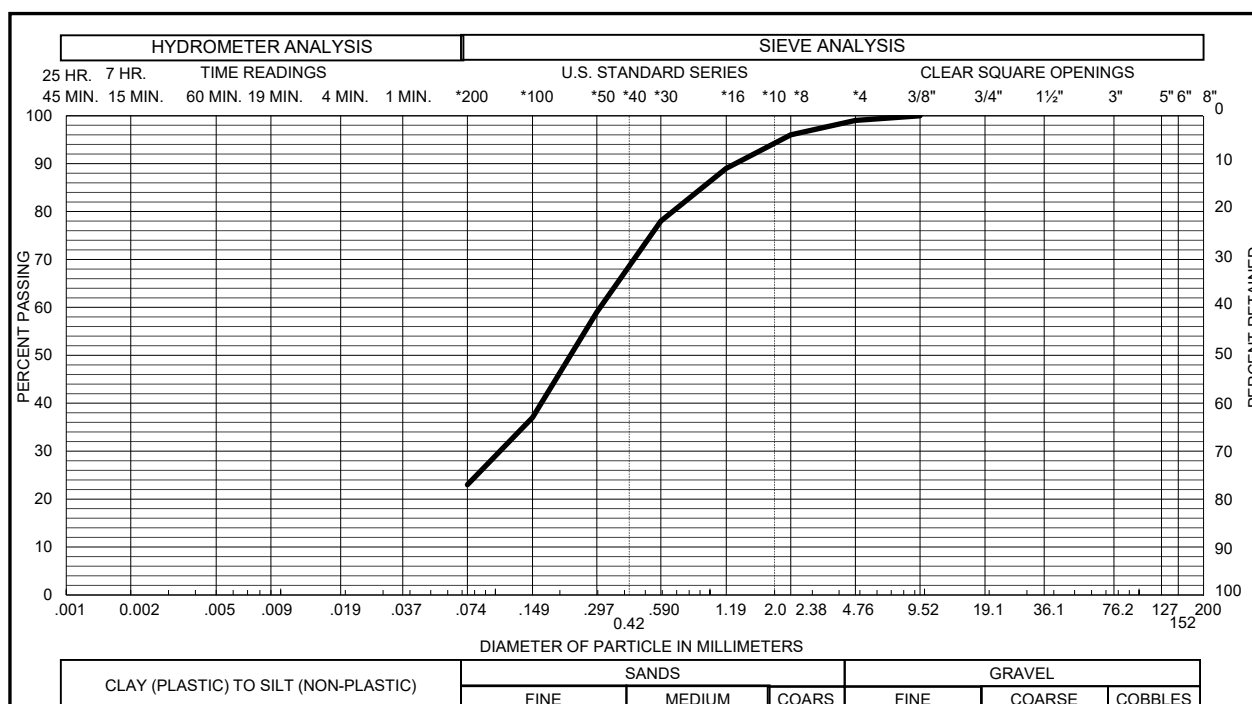
Sample of **SAND, SILTY (SM)** GRAVEL 13 % SAND 72 %
From TH - 1 AT 4 FEET SILT & CLAY 15 % LIQUID LIMIT %
PLASTICITY INDEX %



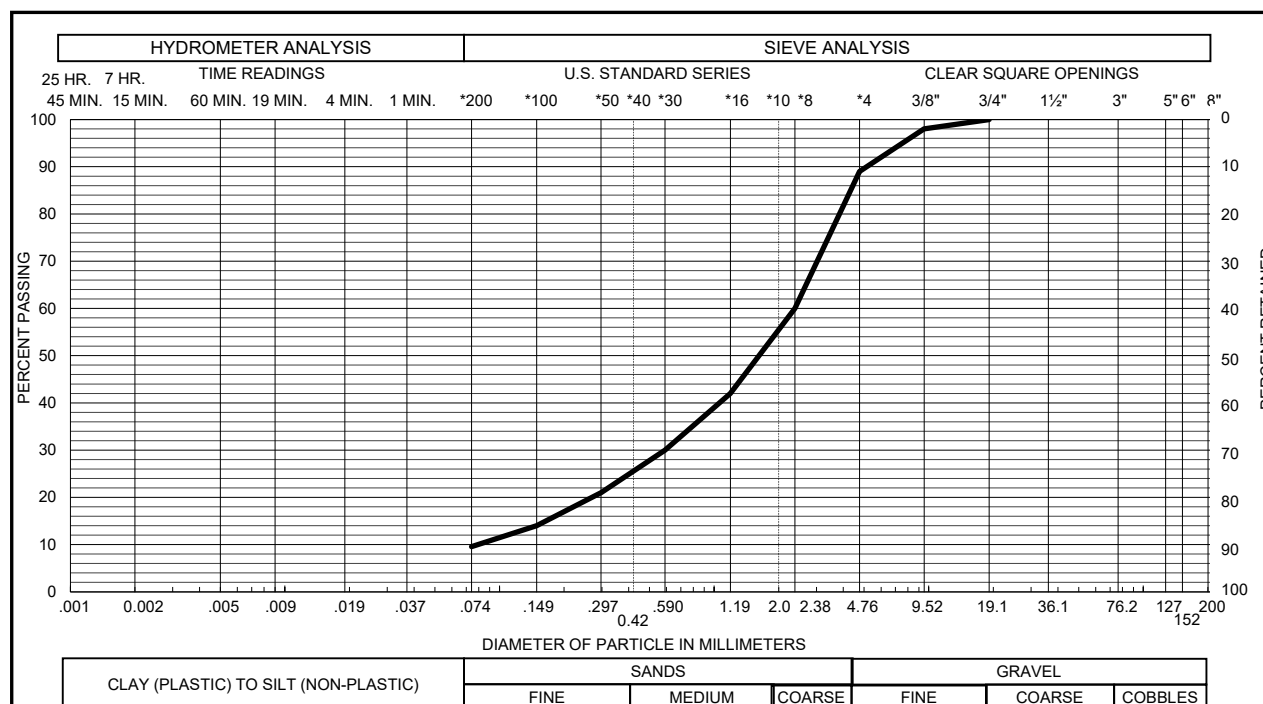
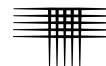
Sample of **SAND, SILTY (SM)** GRAVEL 1 % SAND 82 %
From TH - 3 AT 4 FEET SILT & CLAY 17 % LIQUID LIMIT %
PLASTICITY INDEX %



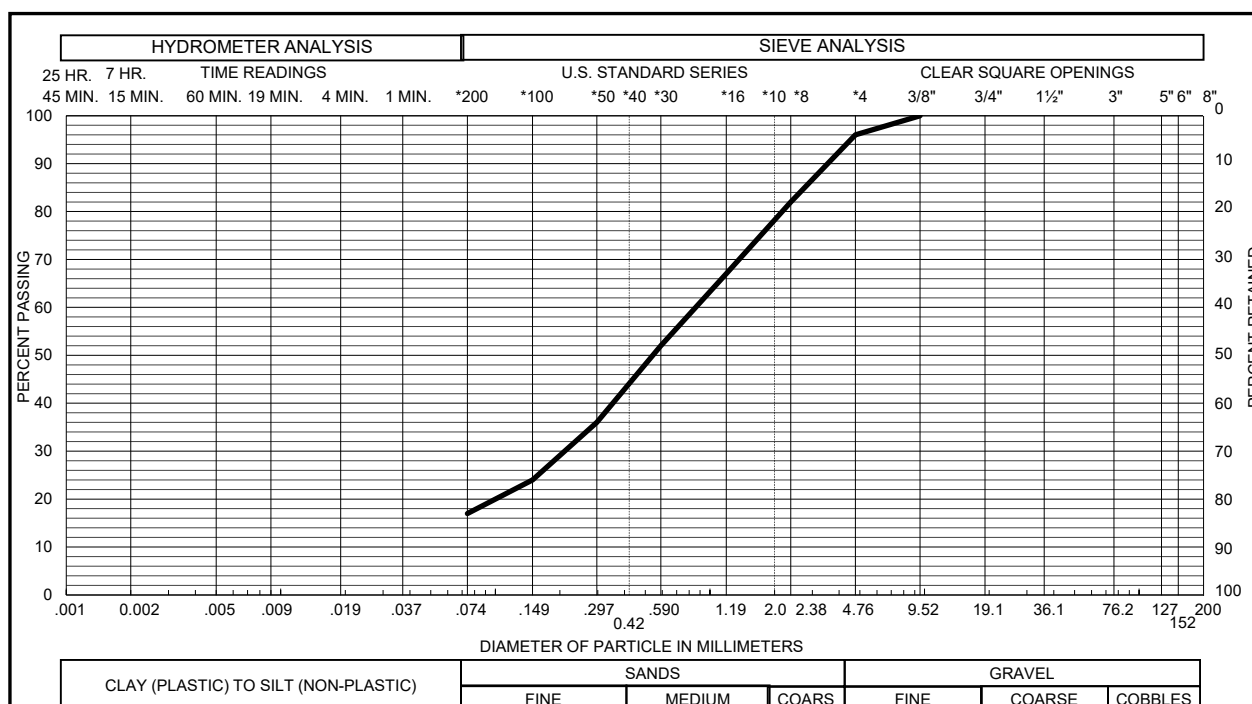
Sample of **SAND, SILTY (SM)** GRAVEL 8 % SAND 76 %
From TH - 5 AT 19 FEET SILT & CLAY 16 % LIQUID LIMIT %
PLASTICITY INDEX %



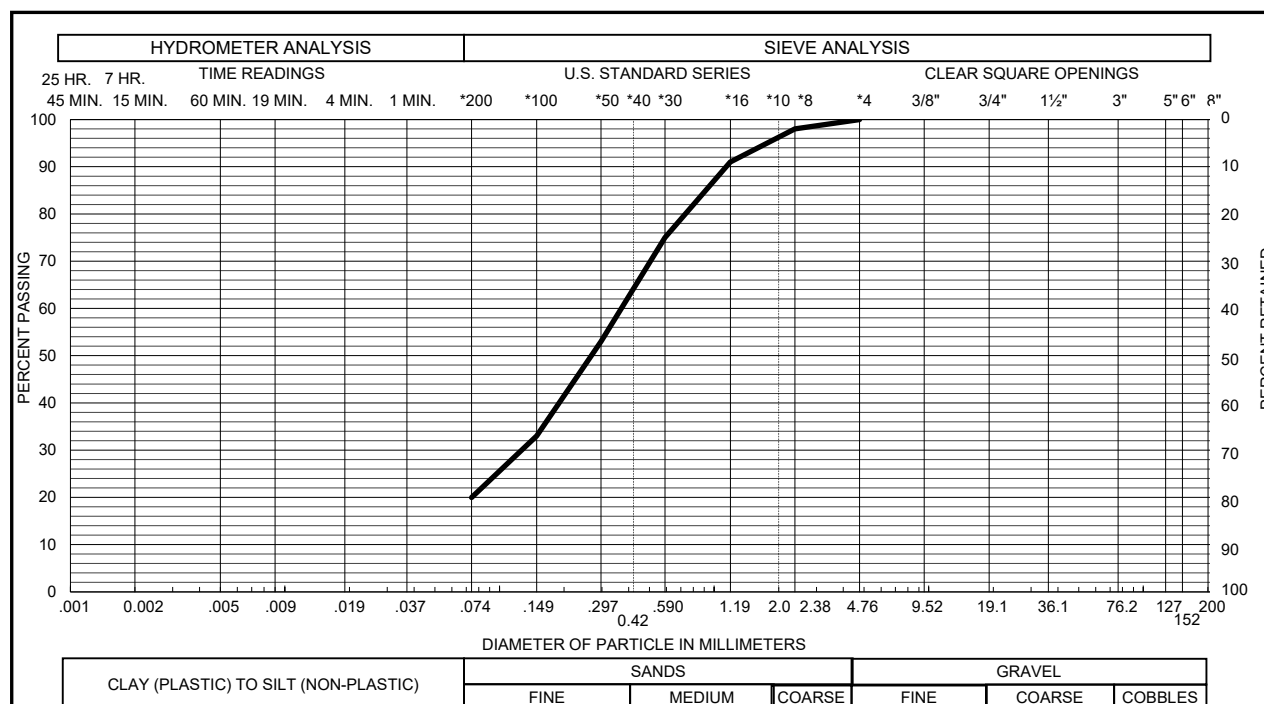
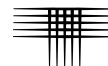
Sample of **SAND, SILTY (SM)** GRAVEL 1 % SAND 76 %
From TH - 7 AT 9 FEET SILT & CLAY 23 % LIQUID LIMIT %
PLASTICITY INDEX %



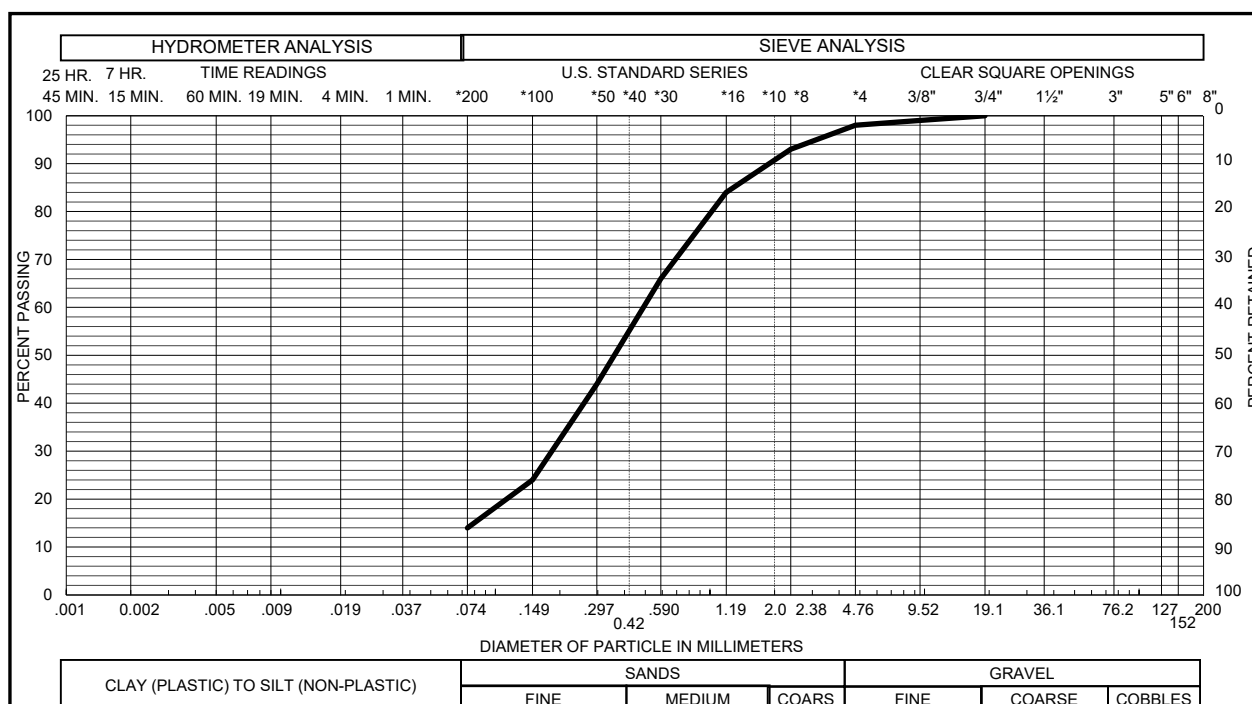
Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 11 % SAND 79 %
From TH - 8 AT 4 FEET SILT & CLAY 10 % LIQUID LIMIT %
PLASTICITY INDEX %



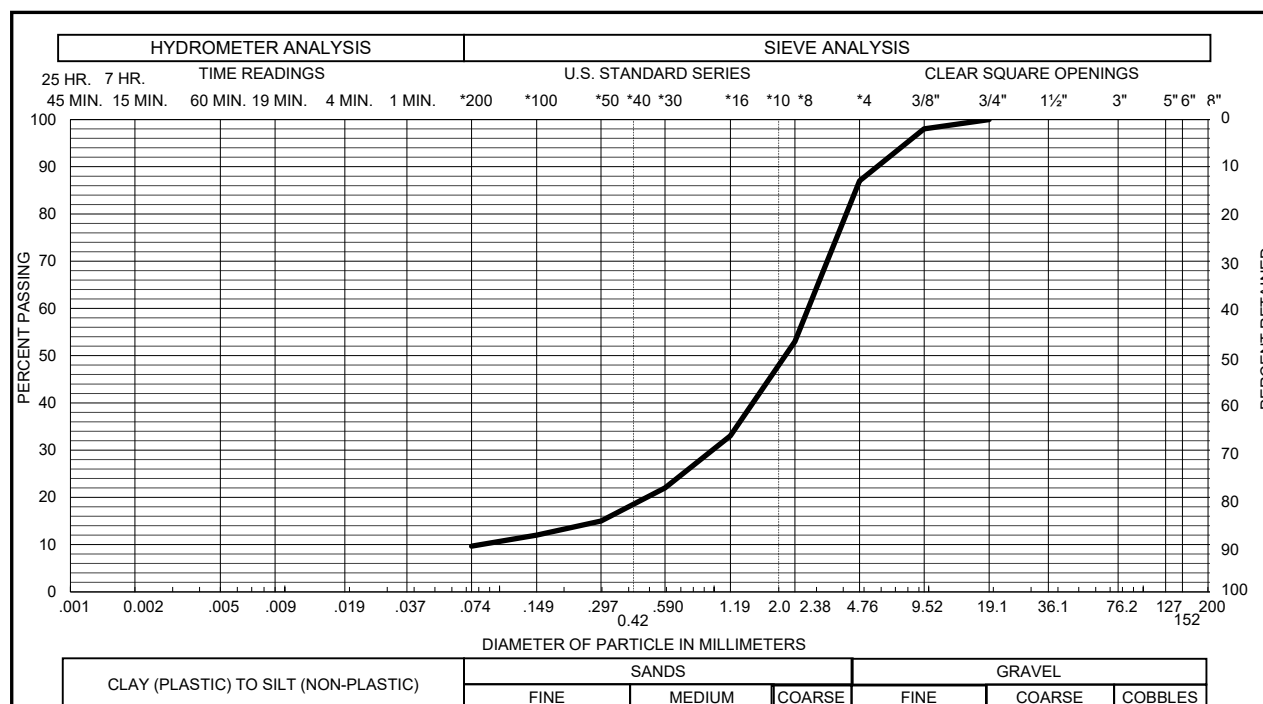
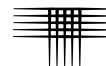
Sample of SAND, SILTY (SM) GRAVEL 4 % SAND 79 %
From TH - 9 AT 9 FEET SILT & CLAY 17 % LIQUID LIMIT %
PLASTICITY INDEX %



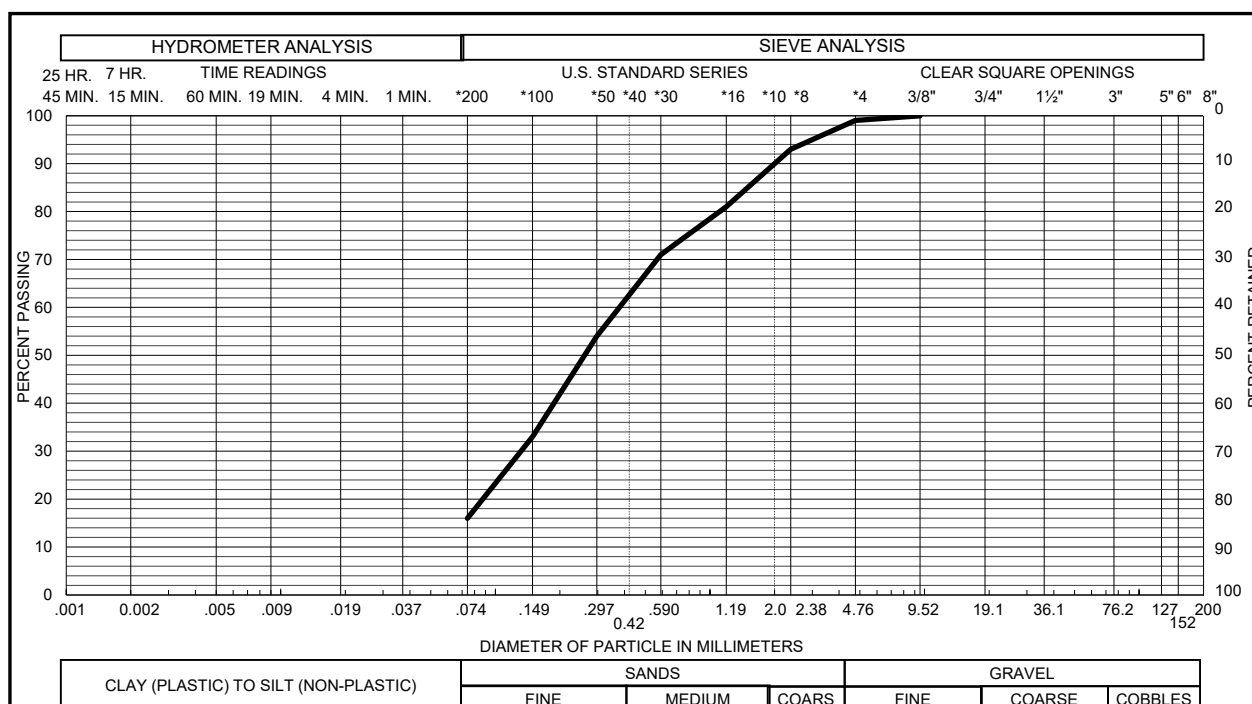
Sample of SAND, SILTY (SM) GRAVEL 0 % SAND 80 %
From TH - 10 AT 4 FEET SILT & CLAY 20 % LIQUID LIMIT %
PLASTICITY INDEX %



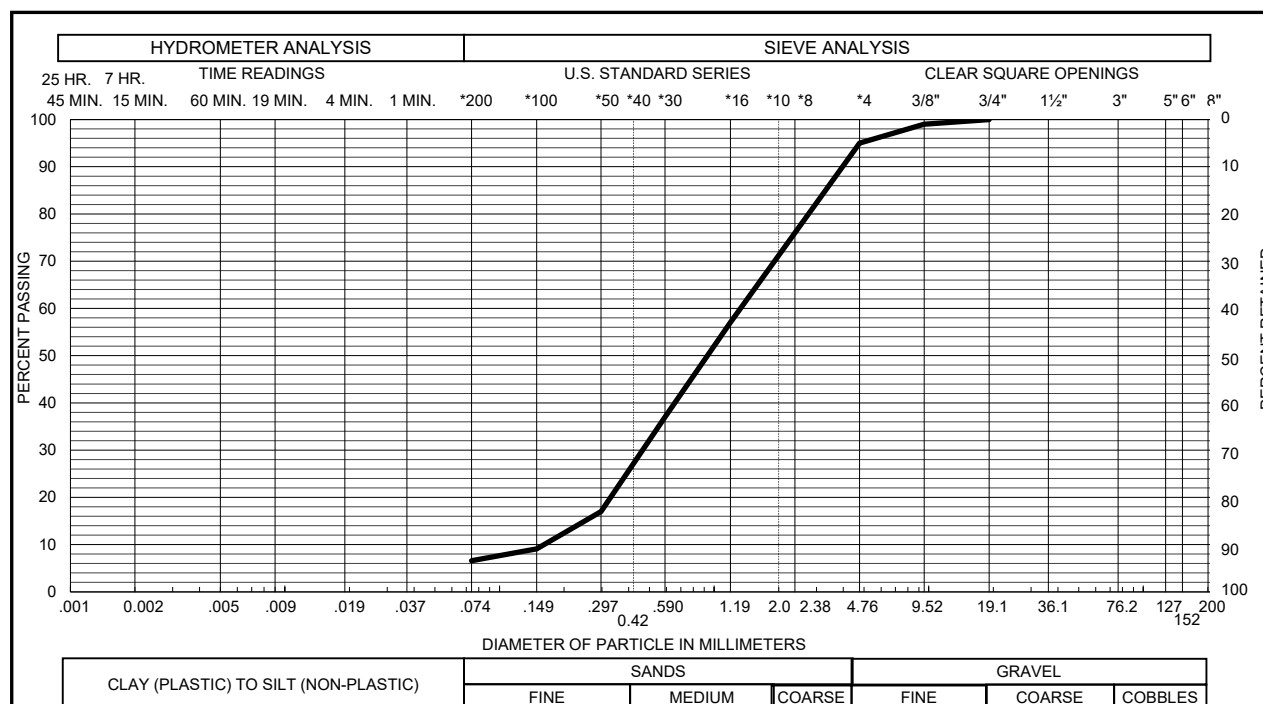
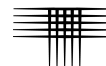
Sample of SAND, SILTY (SM) GRAVEL 2 % SAND 84 %
From TH - 11 AT 14 FEET SILT & CLAY 14 % LIQUID LIMIT %
PLASTICITY INDEX %



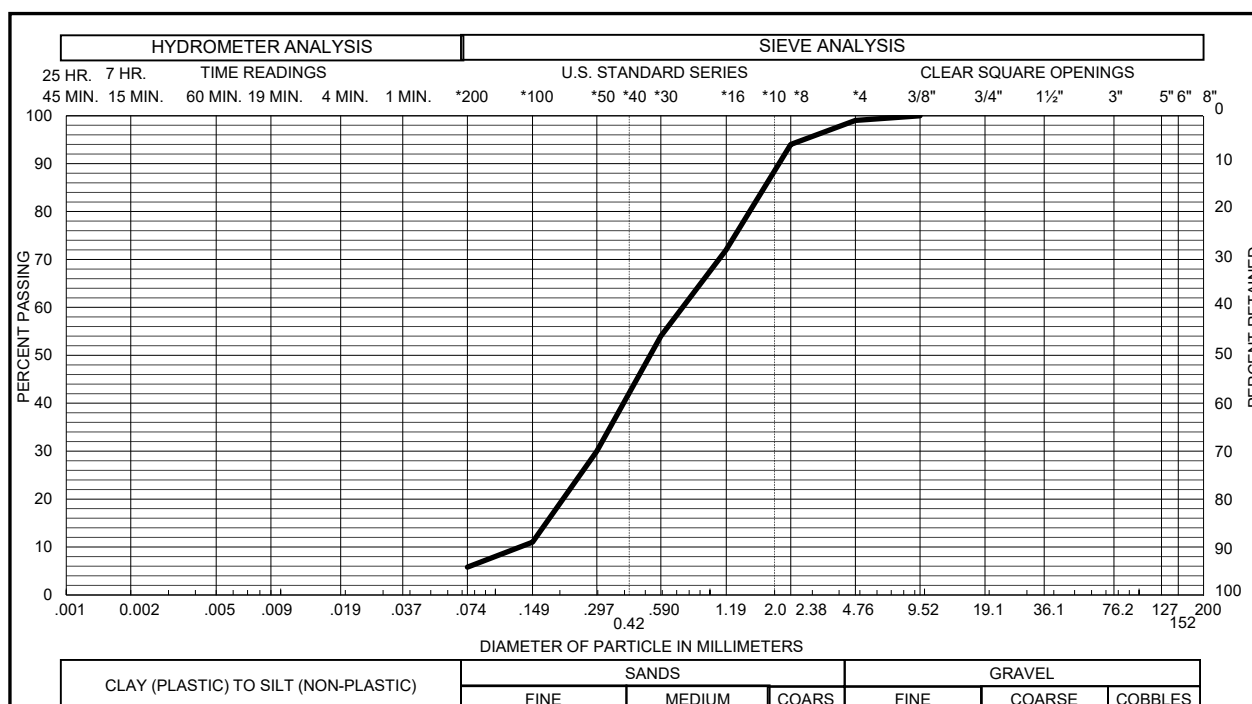
Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 13 % SAND 77 %
From TH - 12 AT 14 FEET SILT & CLAY 10 % LIQUID LIMIT %
PLASTICITY INDEX %



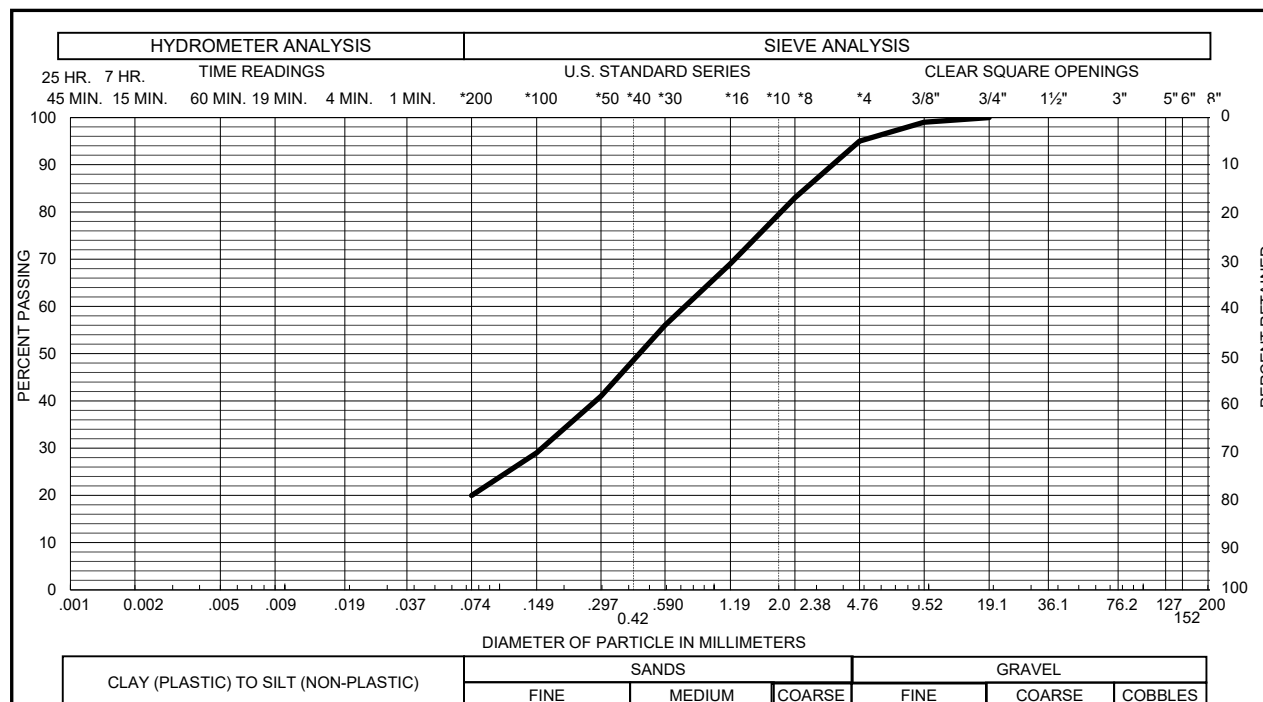
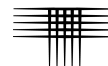
Sample of SAND, SILTY (SM) GRAVEL 1 % SAND 83 %
From TH - 13 AT 9 FEET SILT & CLAY 16 % LIQUID LIMIT %
PLASTICITY INDEX %



Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 5 % SAND 88 %
From TH - 20 AT 14 FEET SILT & CLAY 7 % LIQUID LIMIT %
PLASTICITY INDEX %

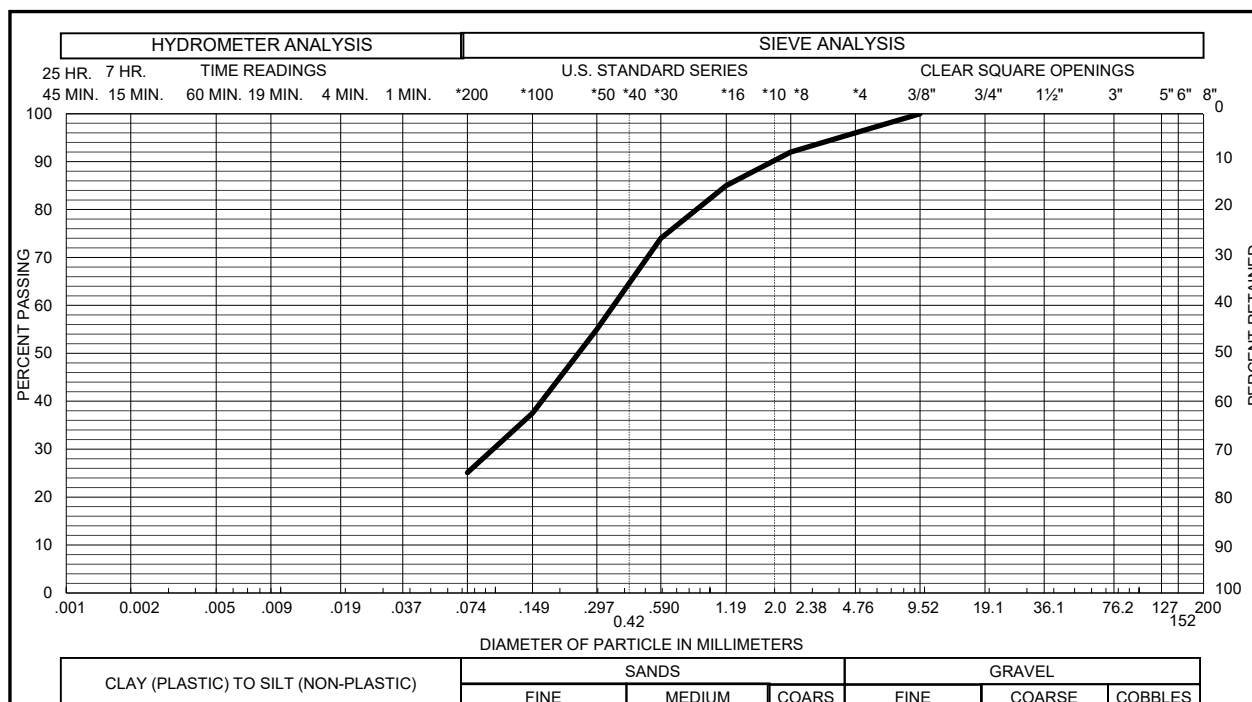


Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 1 % SAND 93 %
From TH - 25 AT 9 FEET SILT & CLAY 6 % LIQUID LIMIT %
PLASTICITY INDEX %



Sample of SAND, CLAYEY (SC)
From TH - 27 AT 14 FEET

GRAVEL 5 % SAND 75 %
SILT & CLAY 20 % LIQUID LIMIT %
PLASTICITY INDEX %



Sample of SAND, SILTY (SM)
From TH - 32 AT 9 FEET

GRAVEL 4 % SAND 71 %
SILT & CLAY 25 % LIQUID LIMIT %
PLASTICITY INDEX %

TABLE B-1

SUMMARY OF LABORATORY TESTING
CTLJT PROJECT NO. CS19587.000-120

BORING	DEPTH (FEET)	MOISTURE CONTENT (%)	DRY DENSITY (PCF)	ATTERBERG LIMITS		SWELL TEST RESULTS*			PASSING NO. 200 SIEVE (%)	WATER SOLUBLE SULFATES (%)	DESCRIPTION
				LIQUID LIMIT (%)	PLASTICITY INDEX (%)	SWELL (%)	APPLIED PRESSURE (PSF)	SWELL PRESSURE (PSF)			
TH-1	4	5.6	127						15		SAND, SILTY (SM)
TH-1	9	9.9	121			-0.3	1100		29		SAND, CLAYEY (SC)
TH-2	9	12.0	119	25	9				42		SAND, VERY CLAYEY (SC)
TH-2	14	2.3	120						5		SAND, SLIGHTLY SILTY (SP-SM)
TH-3	4	2.0	116						17		SAND, SILTY (SM)
TH-4	9	8.8	125						34		SAND, CLAYEY (SC)
TH-5	4	2.0	116						18		SAND, SILTY (SM)
TH-5	19	7.0	123						16		SAND, SILTY (SM)
TH-7	4	2.8	112						8		SAND, SLIGHTLY SILTY (SP-SM)
TH-7	9	10.8	121						23		SAND, SILTY (SM)
TH-8	4	2.7	114						10		SAND, SLIGHTLY SILTY (SP-SM)
TH-8	9	9.2	122			-0.2	1100		31		SAND, CLAYEY (SC)
TH-9	9	4.5	128						17		SAND, SILTY (SM)
TH-9	14	5.1	118						11		SAND, SLIGHTLY SILTY (SP-SM)
TH-10	4	2.7	111						20		SAND, SILTY (SM)
TH-10	9	7.1	129						24		SAND, SILTY (SM)
TH-11	9	8.0	126						30		SAND, CLAYEY (SC)
TH-11	14	5.1	109						14		SAND, SILTY (SM)
TH-12	4	4.6	114						9	<0.1	SAND, SLIGHTLY SILTY (SP-SM)
TH-12	14	3.4	102						10		SAND, SLIGHTLY SILTY (SP-SM)
TH-13	9	8.7	114						16		SAND, SILTY (SM)
TH-13	14	3.1	106						7		SAND, SLIGHTLY SILTY (SP-SM)
TH-14	9	12.9	113						45		SAND, VERY CLAYEY (SC)
TH-15	4	1.9	109						11		SAND, SLIGHTLY SILTY (SP-SM)
TH-15	14	3.1	107						8		SAND, SLIGHTLY SILTY (SP-SM)
TH-17	4	2.7	115						7		SAND, SLIGHTLY SILTY (SP-SM)
TH-17	9	10.4	123			-1.1	1100				SAND, CLAYEY (SC)
TH-18	4	3.3	112	NL	NP				12		SAND, SLIGHTLY SILTY (SP-SM)
TH-18	19	6.1	127						14		SAND, SILTY (SM)
TH-19	9	11.9	123	23	4				25		SAND, SILTY-CLAYEY (SC-SM)
TH-20	14	2.8	112						7		SAND, SLIGHTLY SILTY (SP-SM)
TH-21	4	2.3	112						10		SAND, SLIGHTLY SILTY (SP-SM)
TH-22	9	9.6	123			0.0	1100				SAND, CLAYEY (SC)
TH-23	9	9.2		26	11				34		SAND, CLAYEY (SC)
TH-25	4	3.8	109						13		SAND, SILTY (SM)
TH-25	9	3.1	116						6		SAND, SLIGHTLY SILTY (SP-SM)

* SWELL MEASURED UNDER ESTIMATED IN-SITU OVERBURDEN PRESSURE.
NEGATIVE VALUE INDICATES COMPRESSION.

SUMMARY OF LABORATORY TESTING
CTL/T PROJECT NO. CS19587.000-120

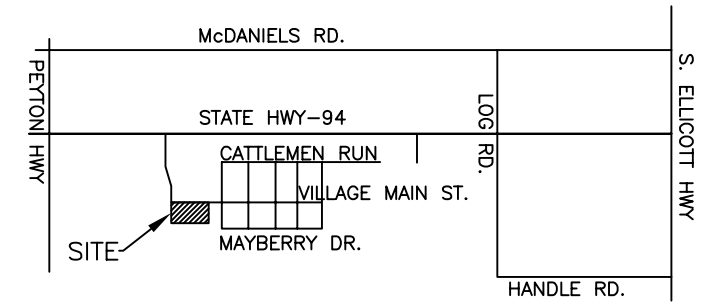
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* SWELL MEASURED UNDER ESTIMATED IN-SITU OVERBURDEN PRESSURE.
NEGATIVE VALUE INDICATES COMPRESSION.



APPENDIX B

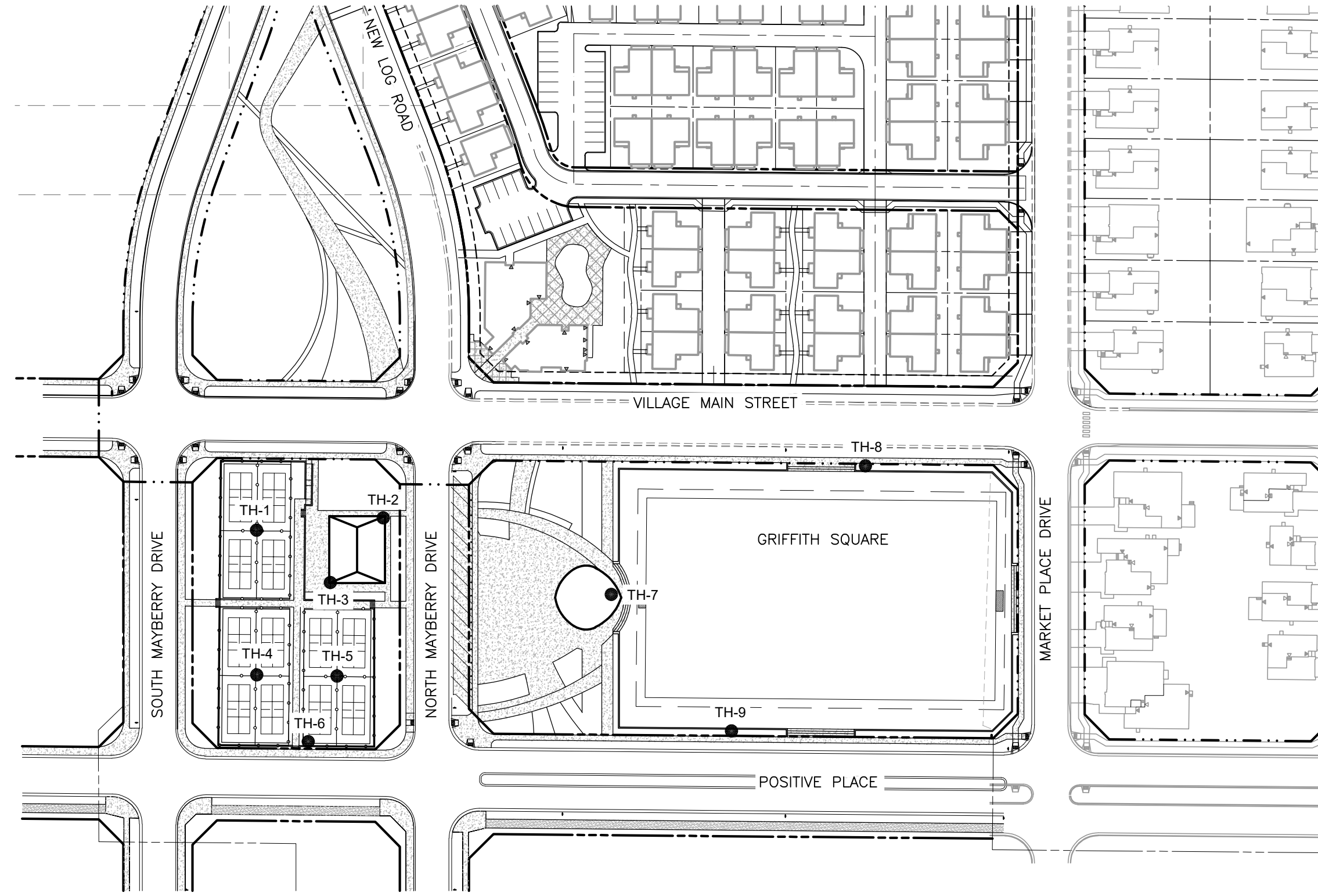
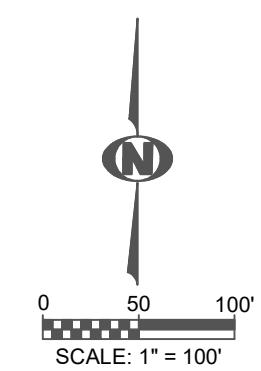
Select Pages from CS19587.001-125, dated February 18, 2025



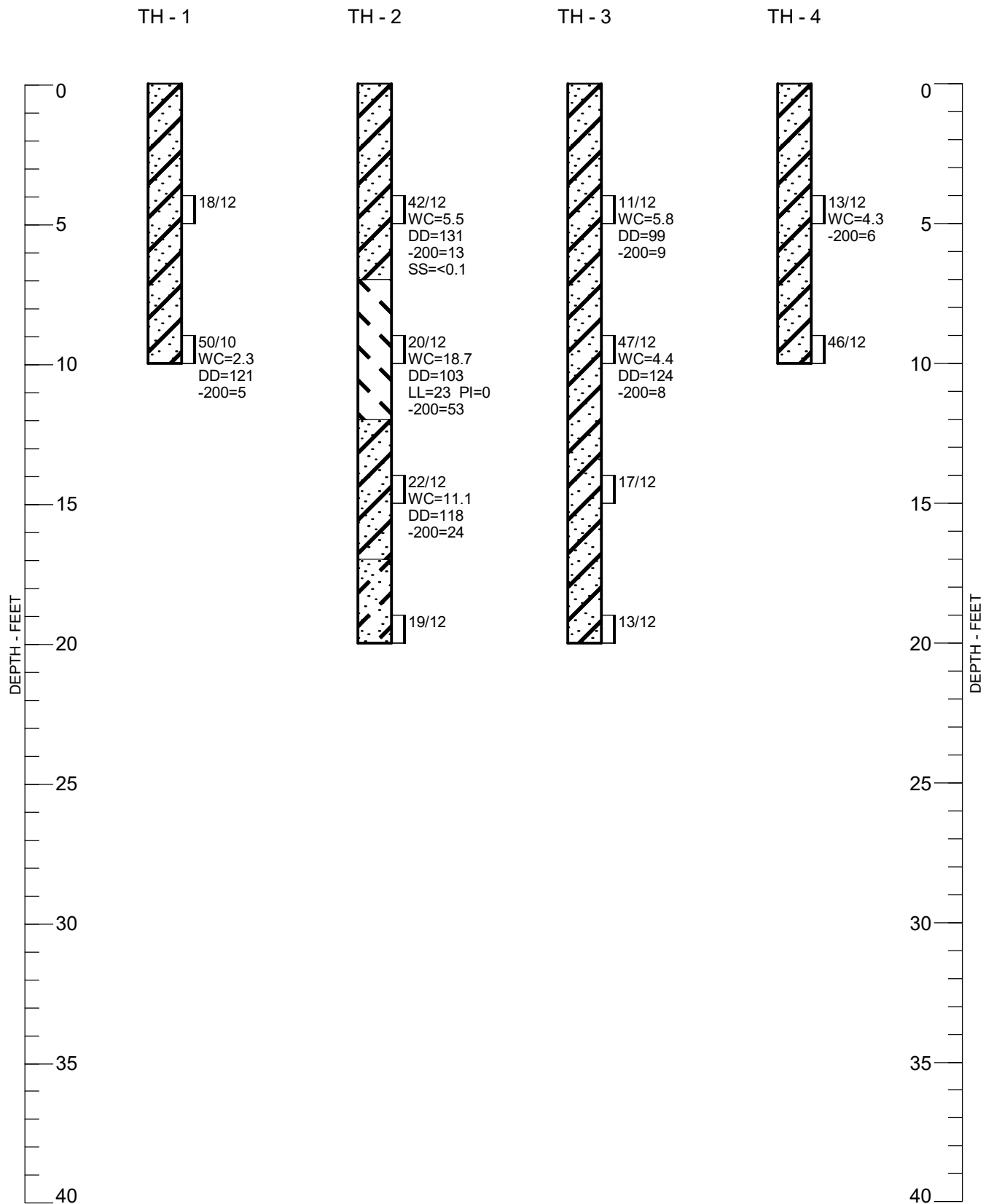
VICINITY MAP
(NOT TO SCALE)

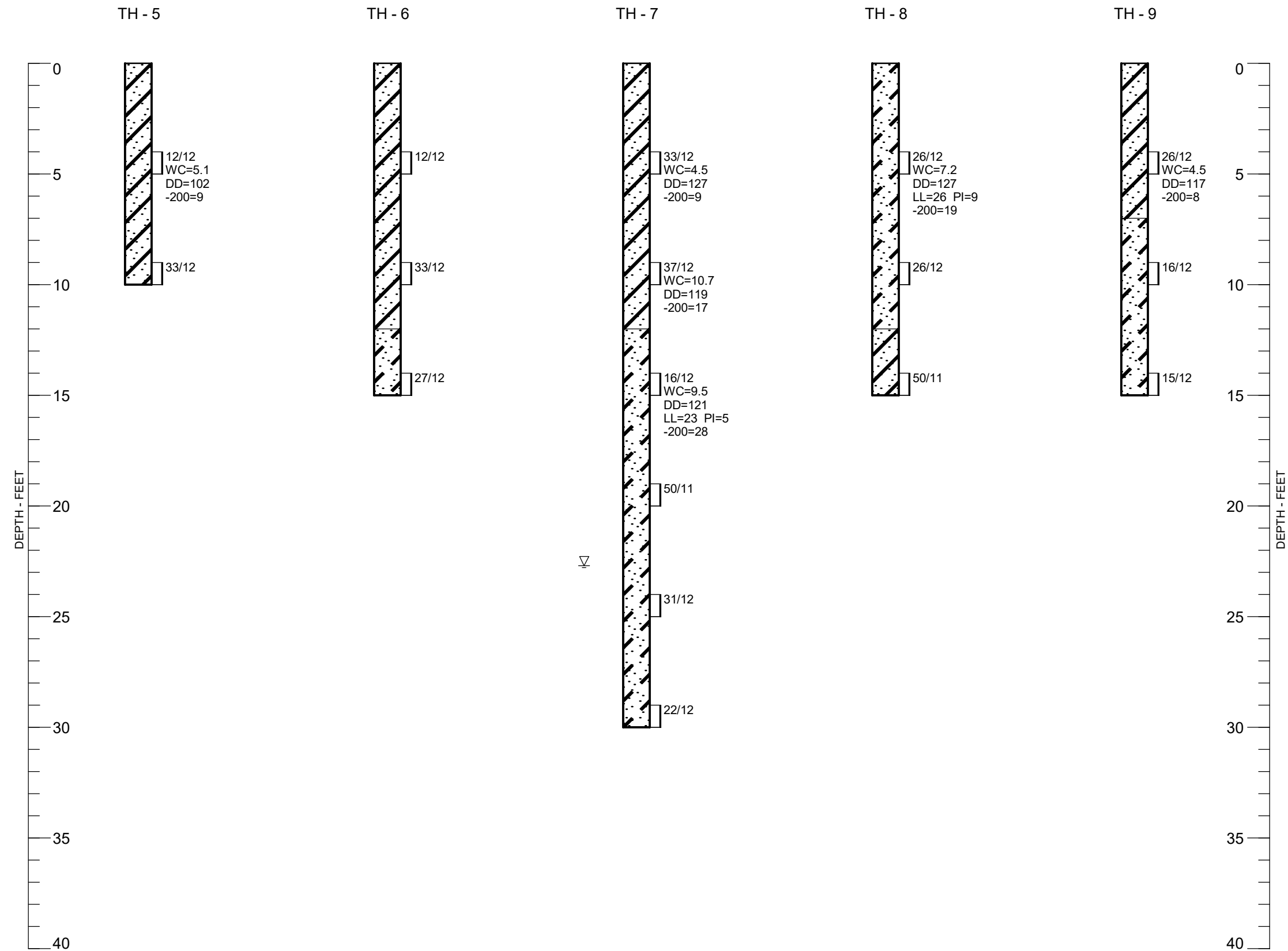
LEGEND:

- TH-1 ● APPROXIMATE LOCATION OF EXPLORATORY BORING.



Location of
Exploratory
Borings



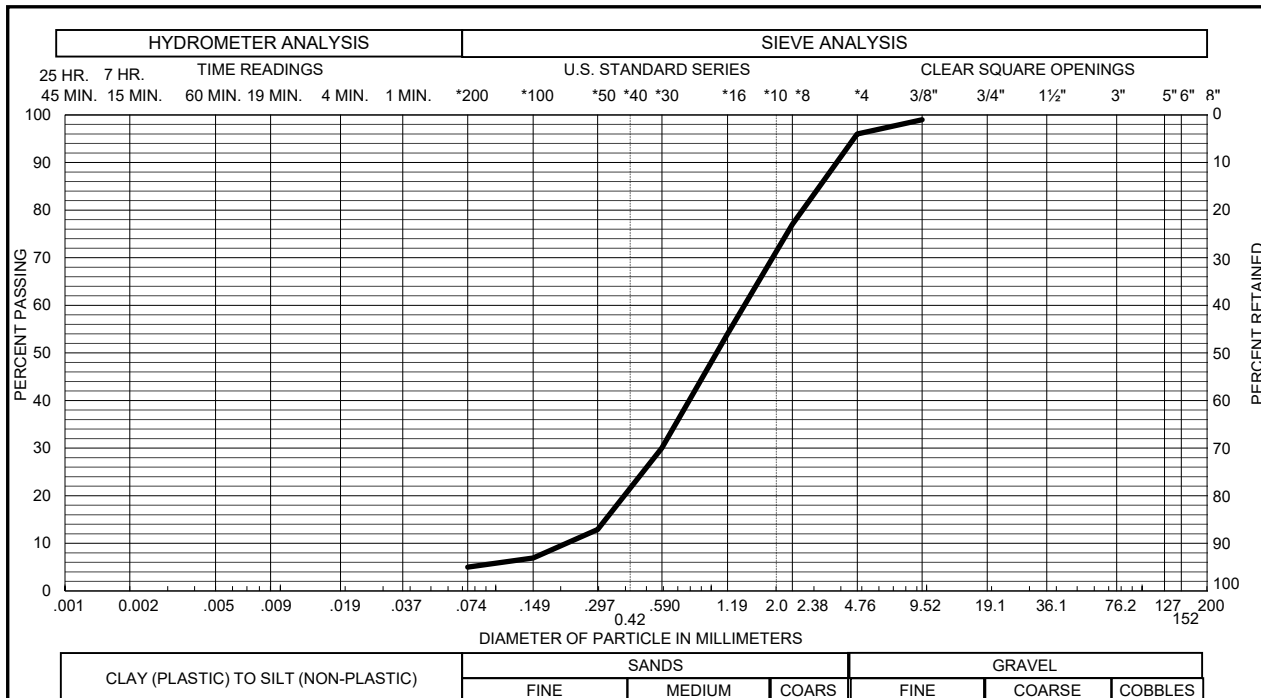
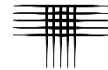


LEGEND:

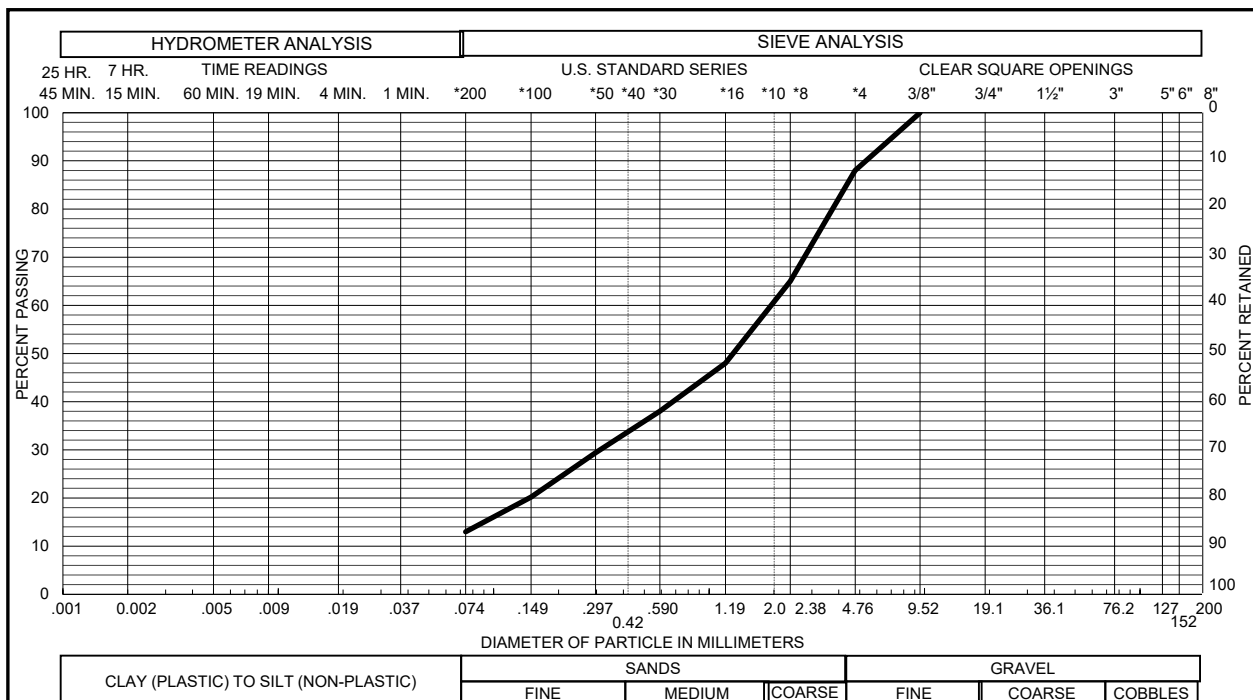
- SAND, CLAYEY, SLIGHTLY MOIST TO MOIST, MEDIUM DENSE TO VERY DENSE, BROWN, LIGHT RUSTIC BROWN (SC).
- SAND, SILTY, SLIGHTLY MOIST, MEDIUM DENSE, BROWN, LIGHT YELLOWISH BROWN, LIGHT RUSTIC BROWN (SM, SW-SM, SP-SM).
- SILT, VERY SANDY, MOIST, VERY STIFF, YELLOWISH-BROWN (ML).
- DRIVE SAMPLE. THE SYMBOL 18/12 INDICATES 18 BLOWS OF A 140-POUND HAMMER FALLING 30 INCHES WERE REQUIRED TO DRIVE A 2.5-INCH O.D. SAMPLER 12 INCHES.
- GROUNDWATER LEVEL MEASURED AT TIME OF DRILLING.

NOTES:

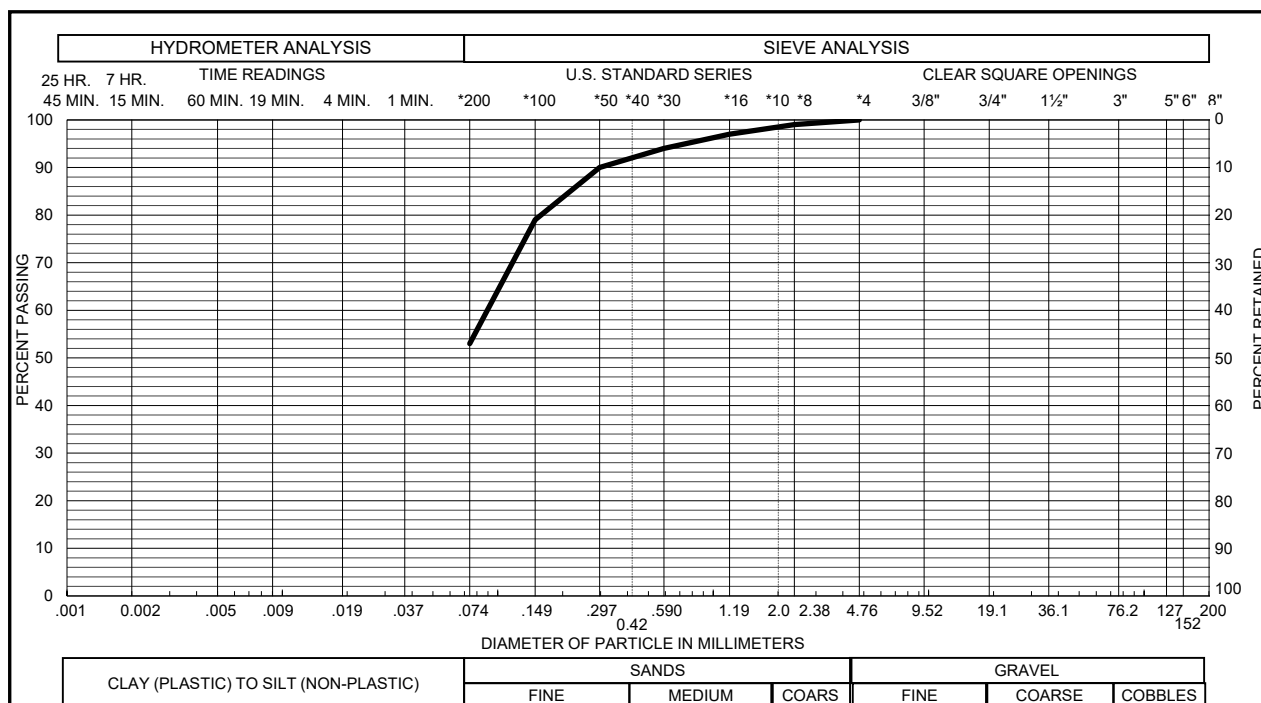
- THE BORINGS WERE DRILLED JANUARY 24TH, 2025 USING A 4-INCH DIAMETER, CONTINUOUS-FLIGHT AUGER AND A CME-45, TRUCK-MOUNTED DRILL RIG.
- WC - INDICATES MOISTURE CONTENT. (%)
DD - INDICATES DRY DENSITY. (PCF)
LL - INDICATES LIQUID LIMIT.
(NV : NO VALUE)
PI - INDICATES PLASTICITY INDEX.
(NP : NON-PLASTIC)
-200 - INDICATES PASSING NO. 200 SIEVE. (%)
SS - INDICATES WATER-SOLUBLE SULFATE CONTENT. (%)
- THESE LOGS ARE SUBJECT TO THE EXPLANATIONS, LIMITATIONS, AND CONCLUSIONS AS CONTAINED IN THIS REPORT.



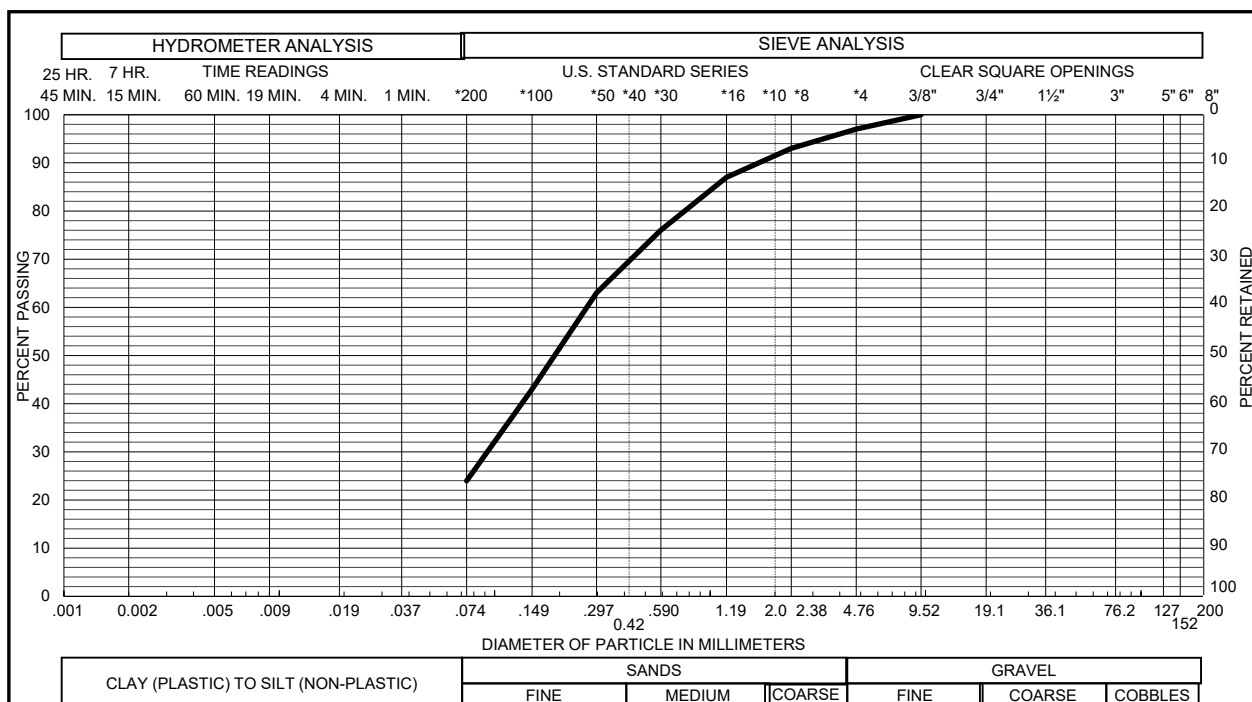
Sample of SAND, SLIGHTLY SILTY (SW-SM) GRAVEL 4 % SAND 91 %
From TH - 1 AT 9 FEET SILT & CLAY 5 % LIQUID LIMIT _____
PLASTICITY INDEX _____



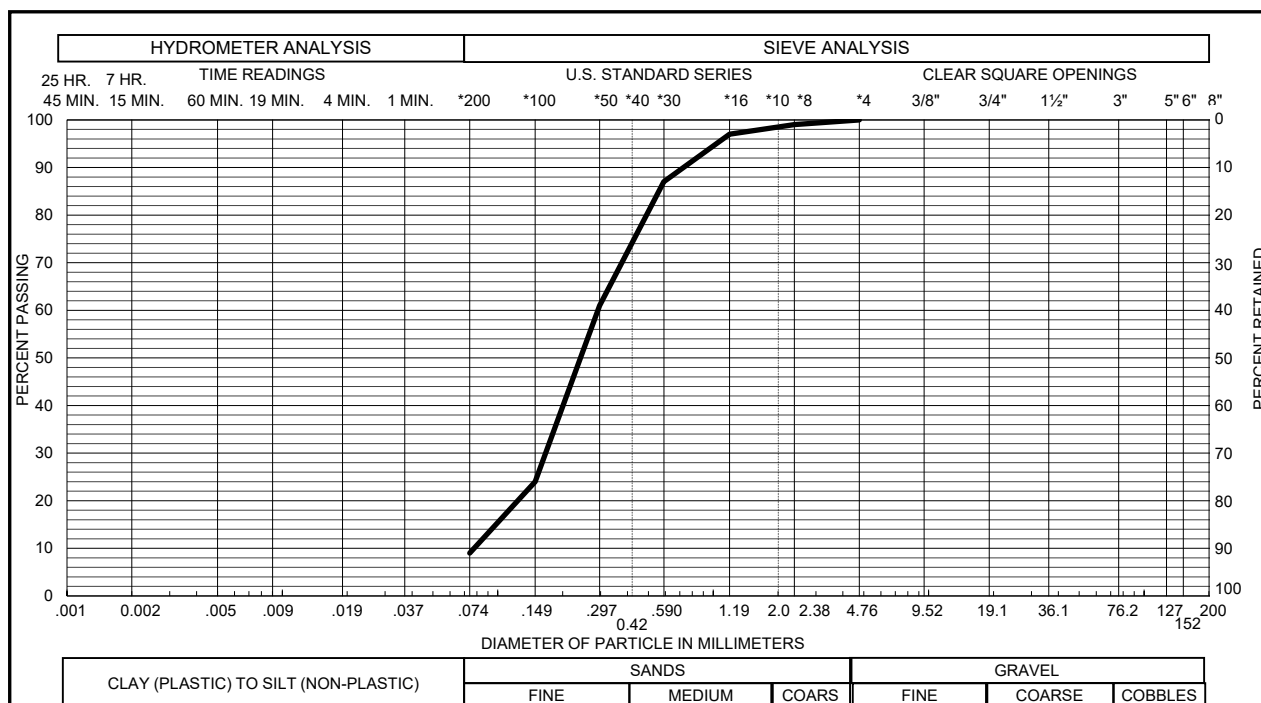
Sample of SAND, SILTY (SM) GRAVEL 12 % SAND 75 %
From TH - 2 AT 4 FEET SILT & CLAY 13 % LIQUID LIMIT _____
PLASTICITY INDEX _____



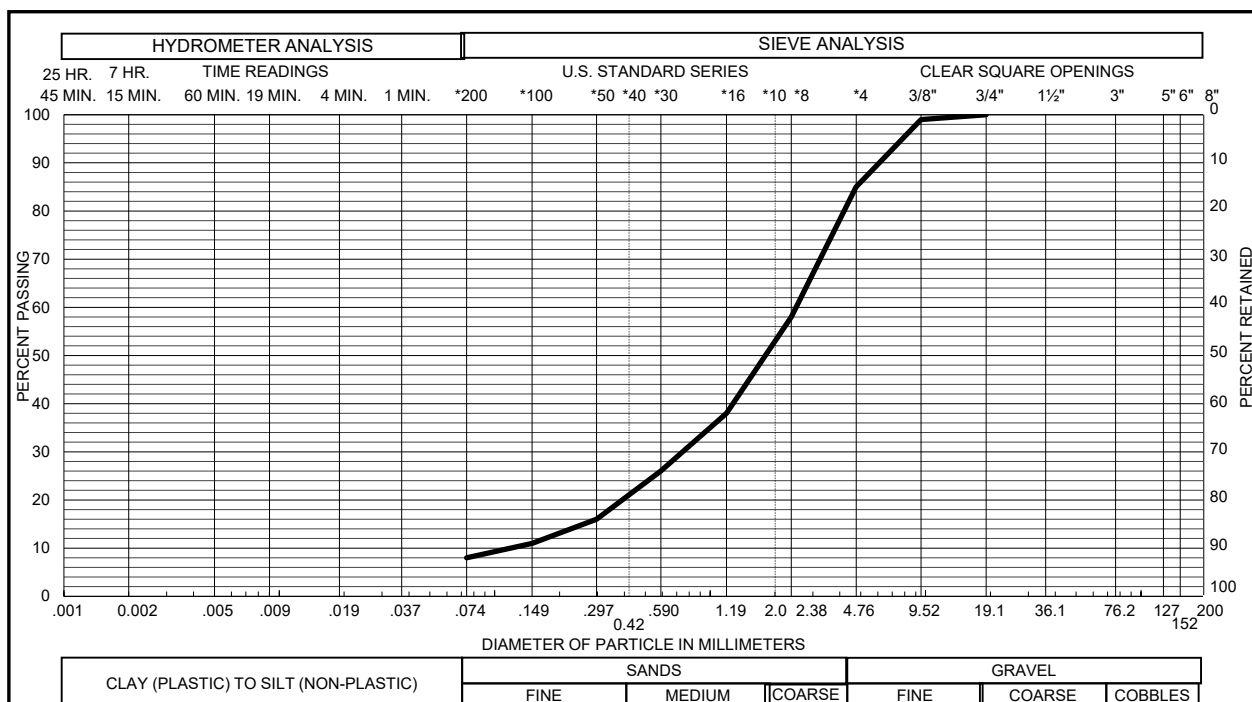
Sample of FILL, SAND, CLAYEY GRAVEL 0 % SAND 47 %
From TH - 2 AT 9 FEET SILT & CLAY 53 % LIQUID LIMIT 23
PLASTICITY INDEX 0



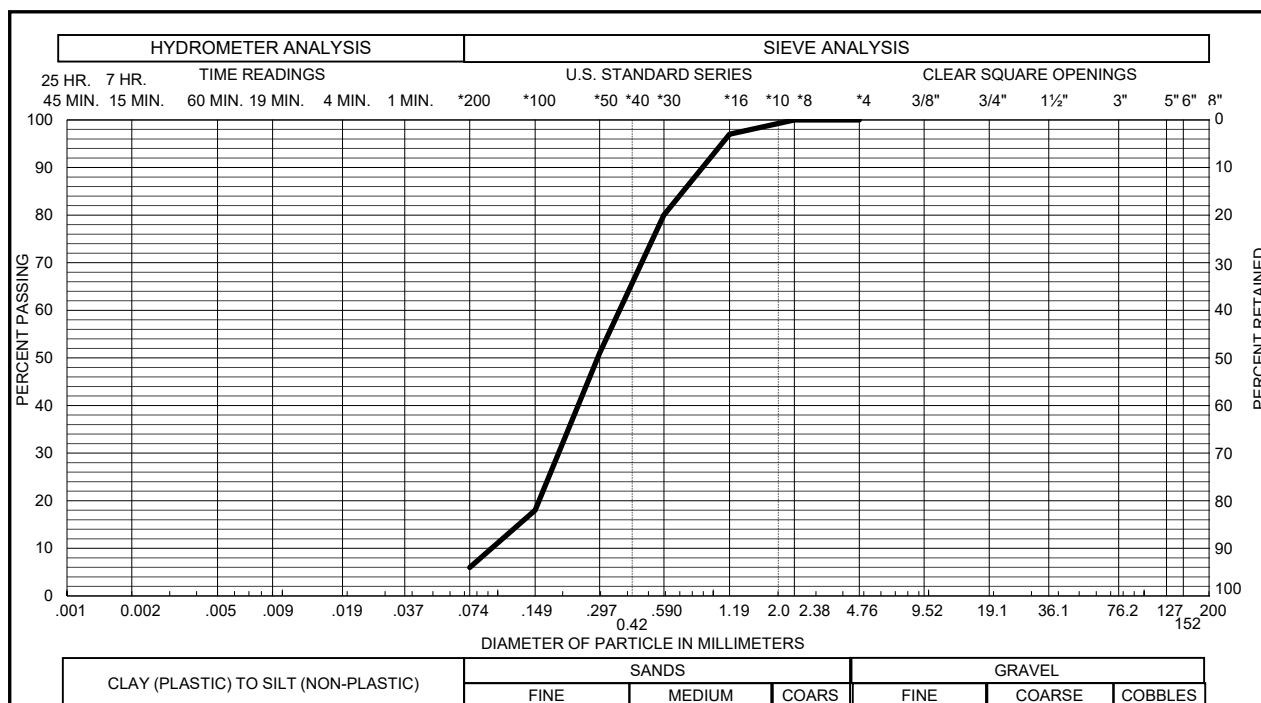
Sample of SAND, SILTY (SM) GRAVEL 3 % SAND 73 %
From TH - 2 AT 14 FEET SILT & CLAY 24 % LIQUID LIMIT
PLASTICITY INDEX



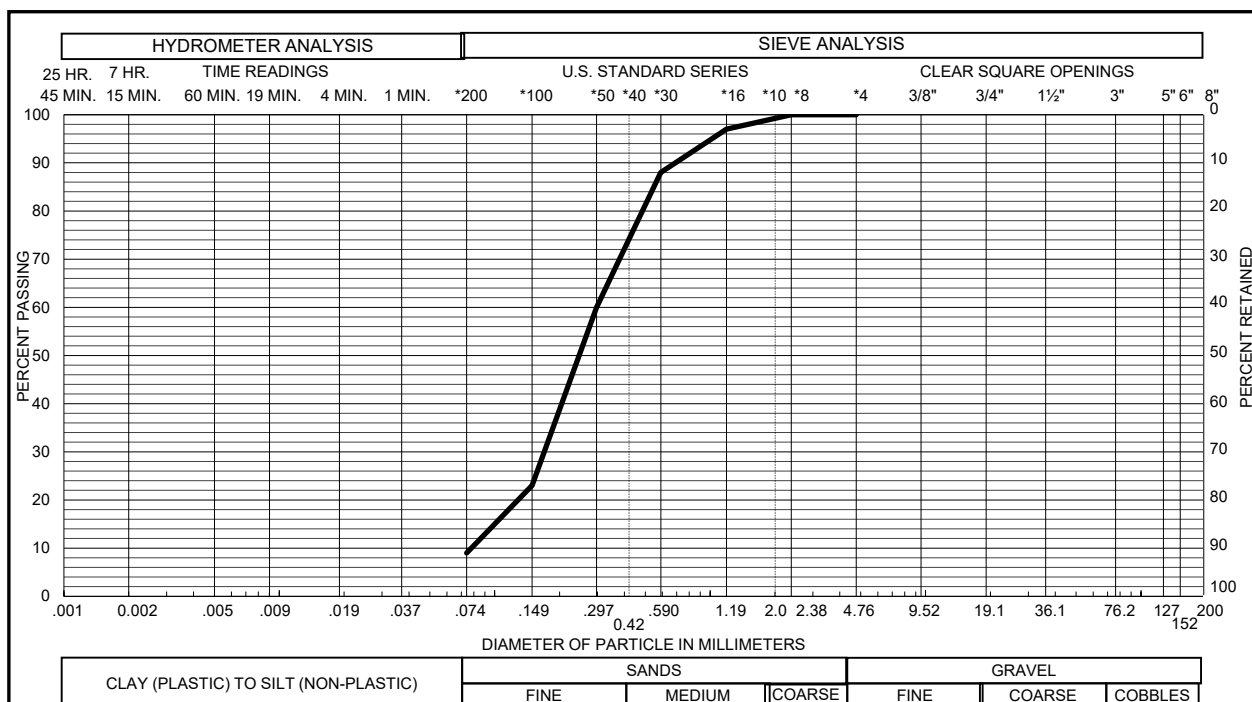
Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 0 % SAND 91 %
From TH - 3 AT 4 FEET SILT & CLAY 9 % LIQUID LIMIT _____
PLASTICITY INDEX _____



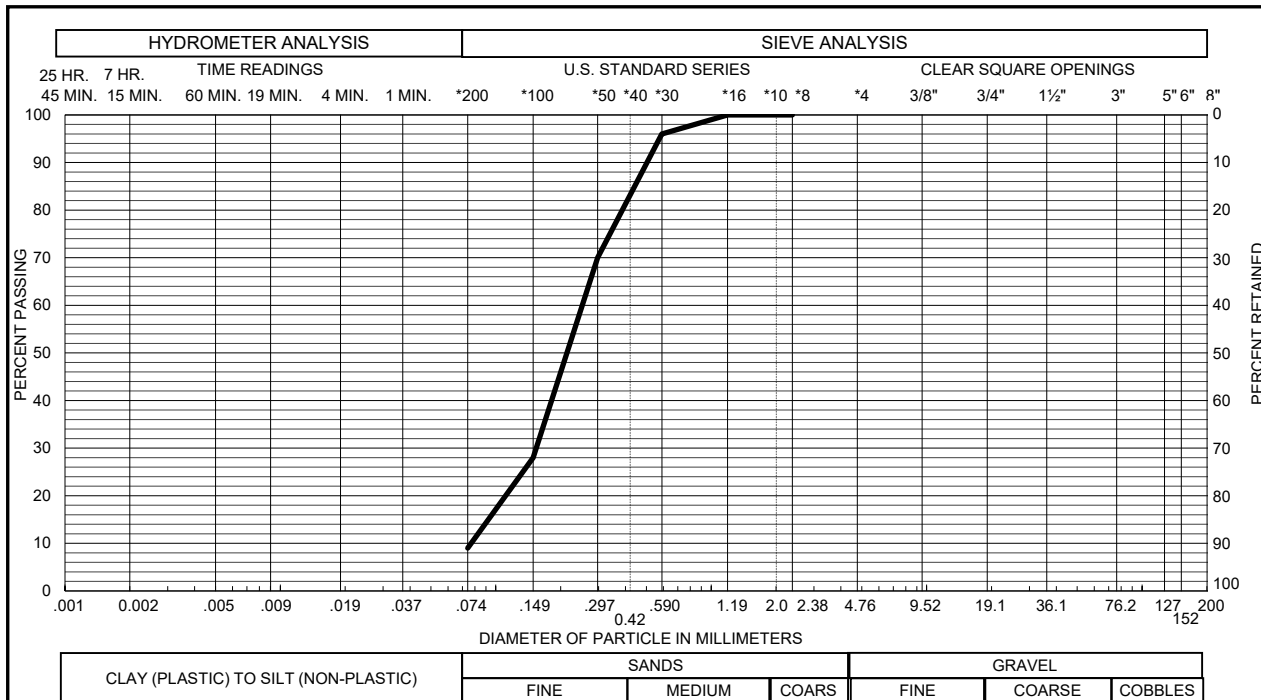
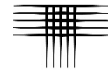
Sample of SAND, SLIGHTLY SILTY (SW-SM) GRAVEL 15 % SAND 77 %
From TH - 3 AT 9 FEET SILT & CLAY 8 % LIQUID LIMIT _____
PLASTICITY INDEX _____



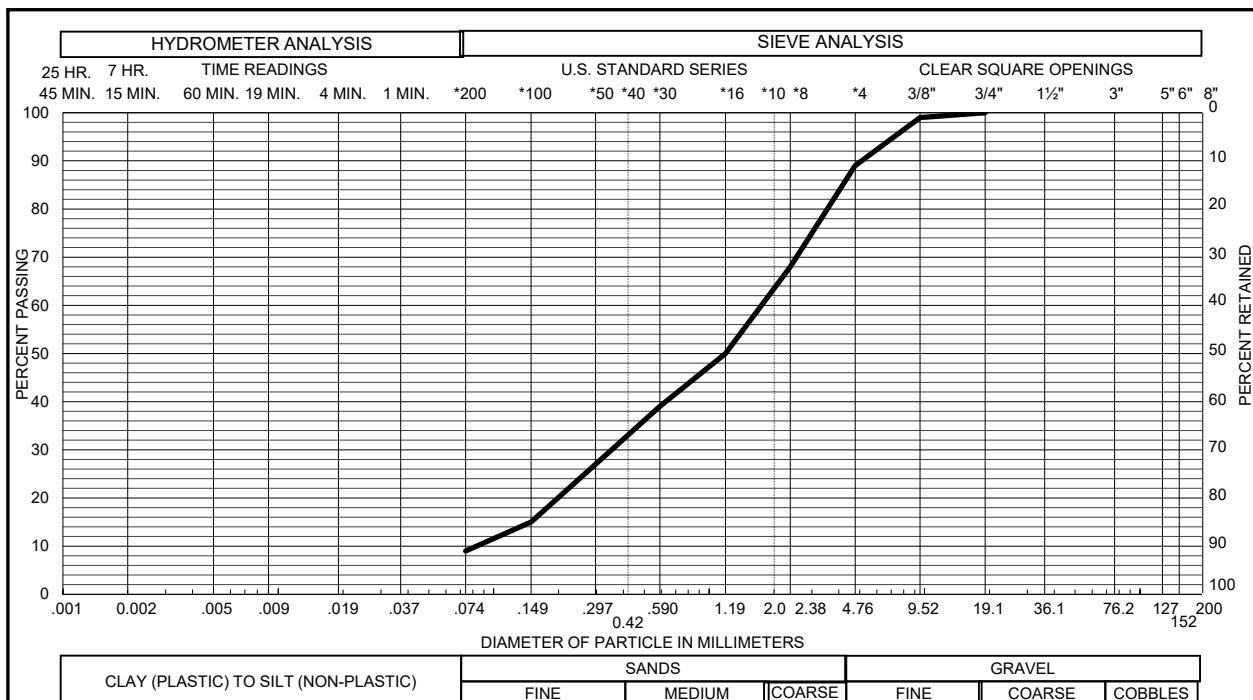
Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 0 % SAND 94 %
From TH - 4 AT 4 FEET SILT & CLAY 6 % LIQUID LIMIT _____
PLASTICITY INDEX _____



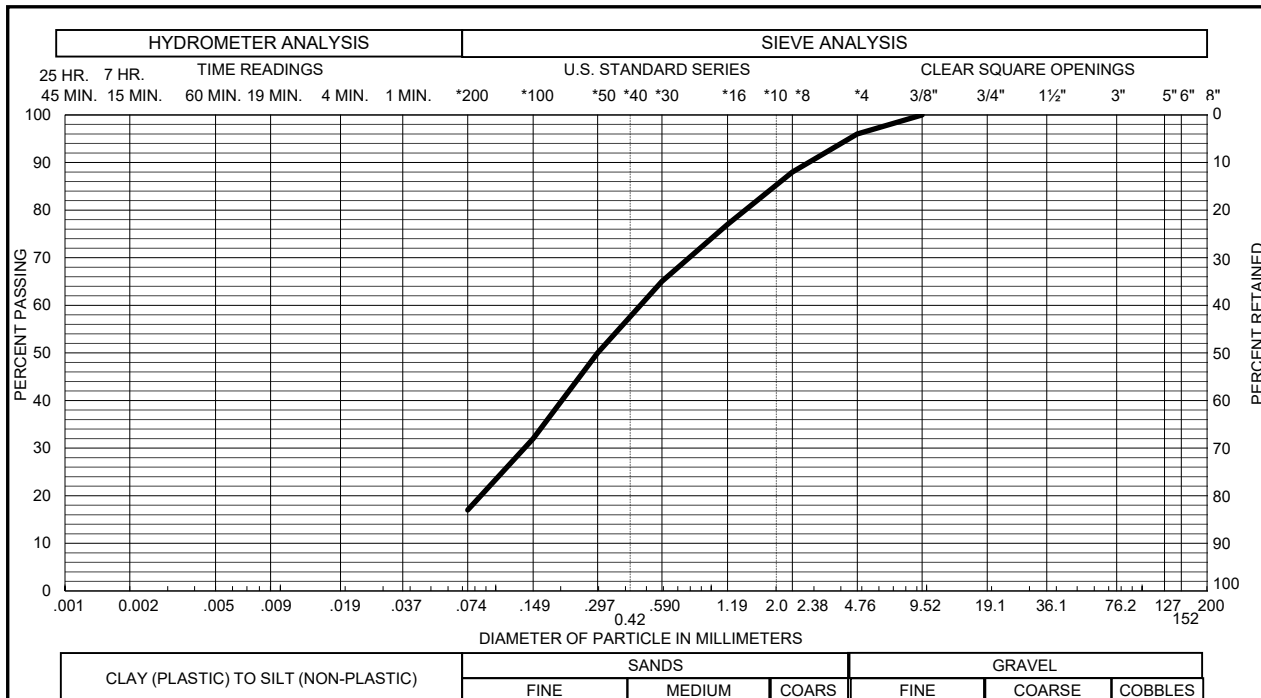
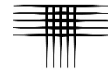
Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 0 % SAND 91 %
From TH - 5 AT 4 FEET SILT & CLAY 9 % LIQUID LIMIT _____
PLASTICITY INDEX _____



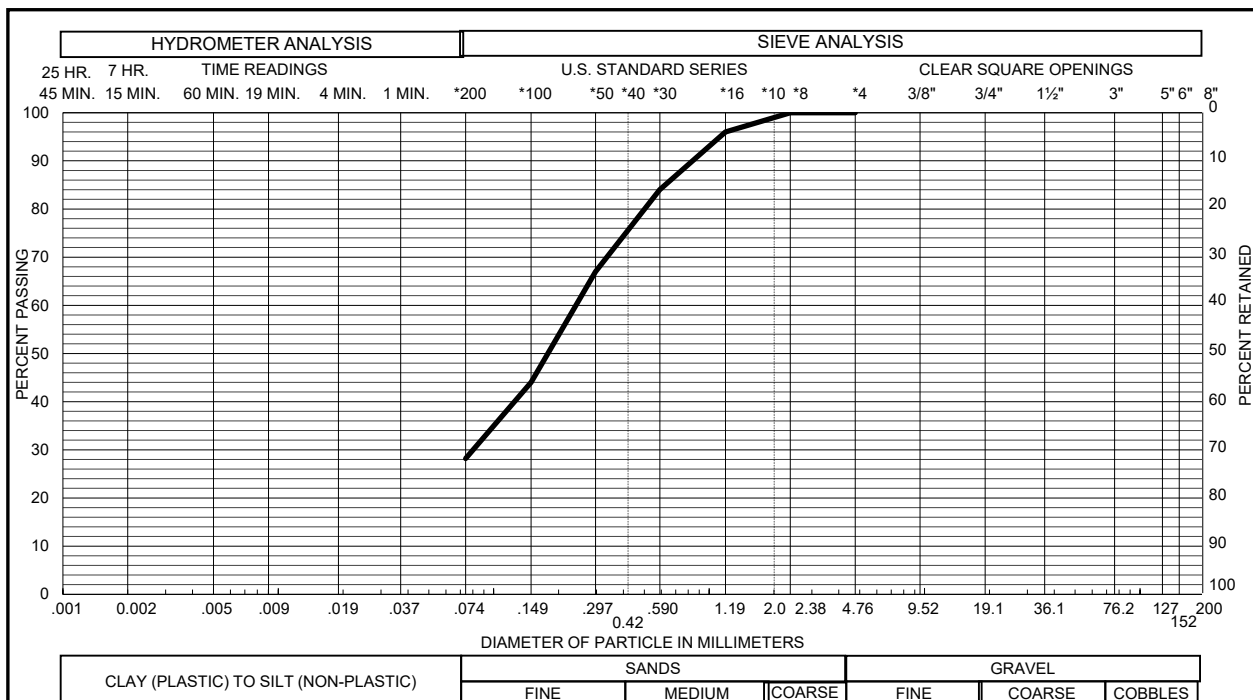
Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 0 % SAND 91 %
From TH - 6 AT 4 FEET SILT & CLAY 9 % LIQUID LIMIT _____
PLASTICITY INDEX _____



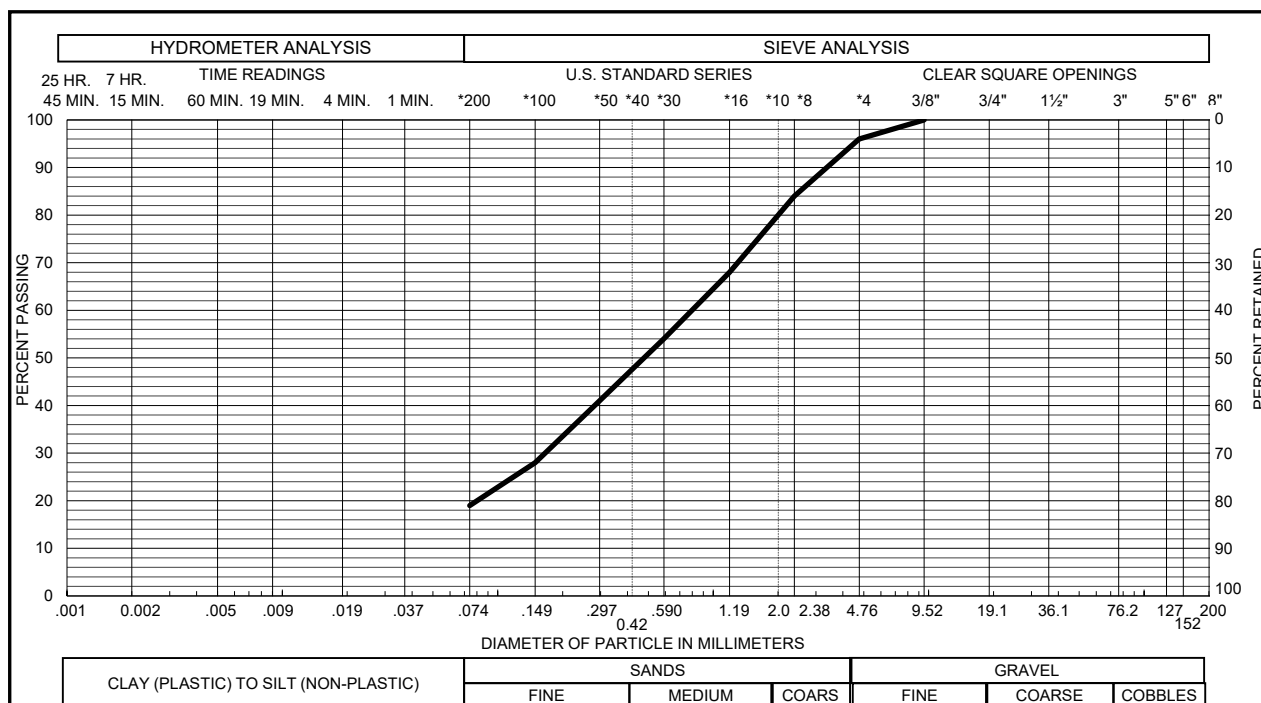
Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 11 % SAND 80 %
From TH - 7 AT 4 FEET SILT & CLAY 9 % LIQUID LIMIT _____
PLASTICITY INDEX _____



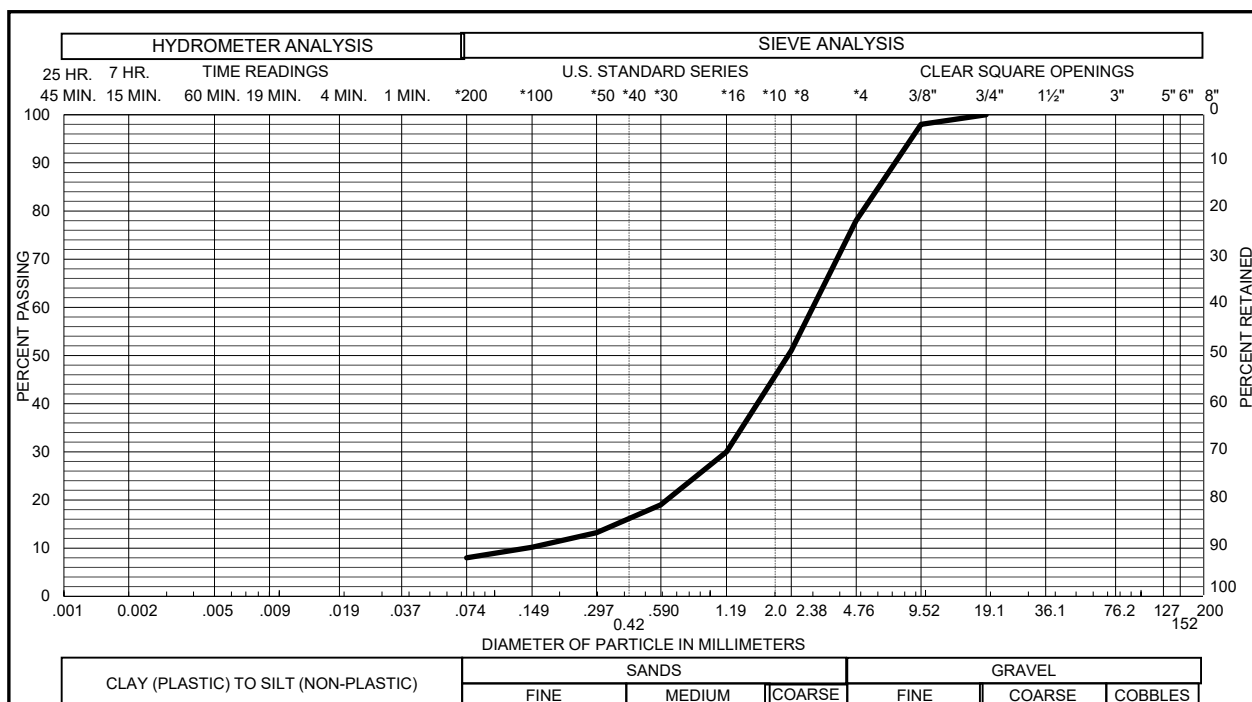
Sample of SAND, SILTY (SM) GRAVEL 4 % SAND 79 %
From TH - 7 AT 9 FEET SILT & CLAY 17 % LIQUID LIMIT _____
PLASTICITY INDEX _____



Sample of FILL, SAND, SLIGHTLY SILTY GRAVEL 0 % SAND 72 %
From TH - 7 AT 14 FEET SILT & CLAY 28 % LIQUID LIMIT 23
PLASTICITY INDEX 5



Sample of SAND, CLAYEY (SC) GRAVEL 4 % SAND 77 %
From TH - 8 AT 4 FEET SILT & CLAY 19 % LIQUID LIMIT 26
PLASTICITY INDEX 9



Sample of SAND, SLIGHTLY SILTY (SP-SM) GRAVEL 22 % SAND 70 %
From TH - 9 AT 4 FEET SILT & CLAY 8 % LIQUID LIMIT
PLASTICITY INDEX

SUMMARY OF LABORATORY TESTING
CTL|T PROJECT NO. CS19587.001-125

[illegible]

* SWELL MEASURED UNDER ESTIMATED IN-SITU OVERBURDEN PRESSURE.
NEGATIVE VALUE INDICATES COMPRESSION.