

ACCEPTED for FILE Engineering Review

08/09/2023 3:42:37 PM Jeff Rice - EPC Engineering EPC Department of Public Works

July 7, 2023

El Paso County Planning & Community Development 2880 International Circle, Suite 110 Colorado Springs, CO 80910

Attn.: Project Manager

RE: Timberline Storage Yard Private Detention/Stormwater Quality Pond

Dear Project Manager:

Per the approved construction drawings for "Timberline Storage Yard" improvements were made to construct a water quality facility in compliance with the current El Paso County Drainage Criteria and the approved Final Drainage Report for this project.

Based upon this information and periodic site visits to the project during significant/key phases of the stormwater BMP installation, M&S Civil Consultants, Inc. is of the opinion that the stormwater BMPs have been constructed in general compliance with the approved design plans, and specifications as filed with El Paso County.

Statement Of Engineer In Responsible Charge

To the best of my knowledge, information and belief, for the referenced project above, the improvements have been constructed in general compliance with the approved design plans and specifications as filed with El Paso County to provide the required storage volume and meet the required release rates documented by the SDI design form, the stage areas, elevations and outlet dimensions. In addition, to the best of my knowledge, information and belief, for the referenced project above, the site and adjacent properties (as affected by work performed under the County permit) are stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and that the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements.

Virgil A. Sanchez Colorado P.E. No.37160 For and on behalf of M&S Civil Consultants, Inc.





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11-13,16 OF 16	TIMBERLINE FULL SPECTRUM DET. POND DETAILS
14-15 OF 16	DWIRE FULL SPECTRUM DET. POND & DETAILS







ORAGE YARD		ON CONTROL PLAN		DATE: 03/30/2020	SHEET 2 OF 16 GR02
TIMBERI INF STO		GRADING AND EROSIC			DESIGNED BY: GW 1"=40' DRAWN BY: GW VERTICAL: CHECKED BY: VAS N/A
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					NEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTH THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE AF PLANS.
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Temporary and Permanent Seeding (TS/PS) EC-2

soil amendments and rototill them into the soil to a depth of 6 inches or more.

Topsoil should be salvaged during grading operations for use and spread on areas to be revegetated later. Topsoil should be viewed as an important resource to be utilized for vegetation establishment, due to its water-holding capacity, structure, texture, organic matter content, biological activity, and nutrient content. The rooting depth of most native grasses in the semi-arid Denver metropolitan area is 6 to 18 inches. At a minimum, the upper 6 inches of topsoil should be stripped, stockpiled, and ultimately respread across areas that will be revegetated.

Where topsoil is not available, subsoils should be amended to provide an appropriate plant-growth medium. Organic matter, such as well digested compost, can be added to improve soil characteristics conducive to plant growth. Other treatments can be used to adjust soil pH conditions when needed. Soil testing, which is typically inexpensive, should be completed to determine and optimize the types and amounts of amendments that are required.

If the disturbed ground surface is compacted, rip or rototill the surface prior to placing topsoil. If adding compost to the existing soil surface, rototilling is necessary. Surface roughening will assist in placement of a stable topsoil layer on steeper slopes, and allow infiltration and root penetration to greater depth.

Prior to seeding, the soil surface should be rough and the seedbed should be firm, but neither too loose nor compacted. The upper layer of soil should be in a condition suitable for seeding at the proper depth and conducive to plant growth. Seed-to-soil contact is the key to good germination.

Seed Mix for Temporary Vegetation

To provide temporary vegetative cover on disturbed areas which will not be paved, built upon, or fully landscaped or worked for an extended period (typically 30 days or more), plant an annual grass appropriate for the time of planting and mulch the planted areas. Annual grasses suitable for the Denver metropolitan area are listed in Table TS/PS-1. These are to be considered only as general recommendations when specific design guidance for a particular site is not available. Local governments typically specify seed mixes appropriate for their jurisdiction.

Seed Mix for Permanent Revegetation

To provide vegetative cover on disturbed areas that have reached final grade, a perennial grass mix should be established. Permanent seeding should be performed promptly (typically within 14 days) after reaching final grade. Each site will have different characteristics and a landscape professional or the local jurisdiction should be contacted to determine the most suitable seed mix for a specific site. In lieu of a specific recommendation, one of the perennial grass mixes appropriate for site conditions and growth season listed in Table TS/PS-2 can be used. The pure live seed (PLS) rates of application recommended in these tables are considered to be absolute minimum rates for seed applied using proper drill-seeding equipment.

If desired for wildlife habitat or landscape diversity, shrubs such as rubber rabbitbrush (*Chrysothamnus* nauseosus), fourwing saltbush (Atriplex canescens) and skunkbrush sumac (Rhus trilobata) could be added to the upland seedmixes at 0.25, 0.5 and 1 pound PLS/acre, respectively. In riparian zones, planting root stock of such species as American plum (Prunus americana), woods rose (Rosa woodsii), plains cottonwood (Populus sargentii), and willow (Populus spp.) may be considered. On non-topsoiled upland sites, a legume such as Ladak alfalfa at 1 pound PLS/acre can be included as a source of nitrogen for perennial grasses.

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Table TS/PS-2. Minimum Drill Seeding Rates for Perennial Grasses (cont.)

Common Name	Botanical Name	Growth Season ^b	Growth Form	Seeds/ Pound	Pounds of PLS/acre
Sandy Soil Seed Mix					
Blue grama	Bouteloua gracilis	Warm	Sod-forming bunchgrass	825,000	0.5
Camper little bluestem	Schizachyrium scoparium 'Camper'	Warm	Bunch	240,000	1.0
Prairie sandreed	Calamovilfa longifolia	Warm	Open sod	274,000	1.0
Sand dropseed	Sporobolus cryptandrus	Cool	Bunch	5,298,000	0.25
Vaughn sideoats grama	Bouteloua curtipendula 'Vaughn'	Warm	Sod	191,000	2.0
Arriba western wheatgrass	Agropyron smithii 'Arriba'	Cool	Sod	110,000	5.5
Total					10.25
Heavy Clay, Rocky Foothill Seed M	Лix				
Ephriam crested wheatgrass ^d	Agropyron cristatum 'Ephriam'	Cool	Sod	175,000	1.5
Oahe Intermediate wheatgrass	Agropyron intermedium 'Oahe'	Cool	Sod	115,000	5.5
Vaughn sideoats grama ^e	Bouteloua curtipendula 'Vaughn'	Warm	Sod	191,000	2.0
Lincoln smooth brome	Bromus inermis leyss 'Lincoln'	Cool	Sod	130,000	3.0
Arriba western wheatgrass	Agropyron smithii 'Arriba'	Cool	Sod	110,000	5.5
Total					17.5
^a All of the above seeding mixes an doubled if seed is broadcast and s	d rates are based on drill seedin hould be increased by 50 percen	g followed by at if the seeding	crimped straw m s is done using a	ulch. These rat Brillion Drill o	tes should be r is applied

through hydraulic seeding. Hydraulic seeding may be substituted for drilling only where slopes are steeper than 3:1. If hydraulic seeding is used, hydraulic mulching should be done as a separate operation.

See Table TS/PS-3 for seeding dates.

If site is to be irrigated, the transition turf seed rates should be doubled.

¹ Crested wheatgrass should not be used on slopes steeper than 6H to 1V.

Can substitute 0.5 lbs PLS of blue grama for the 2.0 lbs PLS of Vaughn sideoats grama.

Seeding dates for the highest success probability of perennial species along the Front Range are generally in the spring from April through early May and in the fall after the first of September until the ground freezes. If the area is irrigated, seeding may occur in summer months, as well. See Table TS/PS-3 for appropriate seeding dates.

Species ^a (Common name)	Growth Season ^b	Pounds of Pure Live Seed (PLS)/acre [°]	Planting Depth (inches)
1. Oats	Cool	35 - 50	1 - 2
2. Spring wheat	Cool	25 - 35	1 - 2
3. Spring barley	Cool	25 - 35	1 - 2
4. Annual ryegrass	Cool	10 - 15	1/2
5. Millet	Warm	3 - 15	1/2 - 3/4
6. Sudangrass	Warm	5–10	1/2 - 3/4
7. Sorghum	Warm	5–10	1/2 - 3/4
8. Winter wheat	Cool	20–35	1 - 2
9. Winter barley	Cool	20–35	1 - 2
10. Winter rye	Cool	20–35	1 - 2
11. Triticale	Cool	25–40	1 - 2
 Successful seeding of usually produce enoug wind and water erosic is not disturbed or mon Hydraulic seeding ma 	annual grass resu gh dead-plant resi on for an additiona wed closer than 8 whe substituted f	Iting in adequate plant due to provide protect al year. This assumes inches.	growth will ion from that the cove
steeper than 3:1 or wh	ere access limitat	ions exist. When hydr	slopes are
seeding is used, hydra operation, when pract the mulch.	ulic mulching sho ical, to prevent th	ould be applied as a se e seeds from being end	raulic parate capsulated in
 seeding is used, hydra operation, when pract the mulch. ^b See Table TS/PS-3 fo may extend the use of 	ulic mulching sho ical, to prevent th r seeding dates. I E cool season spec	ould be applied as a se e seeds from being end rrigation, if consistent ies during the summer	raulic parate capsulated ir ly applied, months.

AS-BUILT ENGINEERING RECORD DRAWINGS

June 2012	

EC-2	Т

	Annual (Numbers in species in T	l Grasses table reference able TS/PS-1)	Perennia	l Grasses
Seeding Dates	Warm	Cool	Warm	Cool
January 1–March 15			✓	✓
March 16–April 30	4	1,2,3	✓	\checkmark
May 1–May 15	4		✓	
May 16–June 30	4,5,6,7			
July 1–July 15	5,6,7			
July 16–August 31				
September 1–September 30		8,9,10,11		
October 1–December 31			✓	\checkmark

Mulch

Cover seeded areas with mulch or an appropriate rolled erosion control product to promote establishment of vegetation. Anchor mulch by crimping, netting or use of a non-toxic tackifier. See the Mulching BMP Fact Sheet for additional guidance.

Maintenance and Removal

Monitor and observe seeded areas to identify areas of poor growth or areas that fail to germinate. Reseed and mulch these areas, as needed.

An area that has been permanently seeded should have a good stand of vegetation within one growing season if irrigated and within three growing seasons without irrigation in Colorado. Reseed portions of the site that fail to germinate or remain bare after the first growing season.

also be necessary.

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TS/PS-5

TS/PS-6

Temporary and Permanent Seeding (TS/PS) EC-2

Table TS/PS-1. Minimum Drill Seeding Rates for Various Temporary Annual Grasses

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Cemporary and Permanent Seeding (TS/PS)

Table TS/PS-3. Seeding Dates for Annual and Perennial Grasses

Seeded areas may require irrigation, particularly during extended dry periods. Targeted weed control may

Protect seeded areas from construction equipment and vehicle access.

Temporary and Permanent Seeding (TS/PS) EC-2

 Table TS/PS-2.
 Minimum Drill Seeding Rates for Perennial Grasses

Common ^a Name	Botanical Name	Growth Season ^b	Growth Form	Seeds/ Pound	Pounds of PLS/acre
Alakali Soil Seed Mix			1		
Alkali sacaton	Sporobolus airoides	Cool	Bunch	1,750,000	0.25
Basin wildrye	Elymus cinereus	Cool	Bunch	165,000	2.5
Sodar streambank wheatgrass	Agropyron riparium 'Sodar'	Cool	Sod	170,000	2.5
Jose tall wheatgrass	Agropyron elongatum 'Jose'	Cool	Bunch	79,000	7.0
Arriba western wheatgrass	Agropyron smithii 'Arriba'	Cool	Sod	110,000	5.5
Total					17.75
Fertile Loamy Soil Seed Mix					
Ephriam crested wheatgrass	Agropyron cristatum 'Ephriam'	Cool	Sod	175,000	2.0
Dural hard fescue	Festuca ovina 'duriuscula'	Cool	Bunch	565,000	1.0
Lincoln smooth brome	Bromus inermis leyss 'Lincoln'	Cool	Sod	130,000	3.0
Sodar streambank wheatgrass	Agropyron riparium 'Sodar'	Cool	Sod	170,000	2.5
Arriba western wheatgrass	Agropyron smithii 'Arriba'	Cool	Sod	110,000	7.0
Total					15.5
High Water Table Soil Seed Mix	ĸ				
Meadow foxtail	Alopecurus pratensis	Cool	Sod	900,000	0.5
Redtop	Agrostis alba	Warm	Open sod	5,000,000	0.25
Reed canarygrass	Phalaris arundinacea	Cool	Sod	68,000	0.5
Lincoln smooth brome	Bromus inermis leyss 'Lincoln'	Cool	Sod	130,000	3.0
Pathfinder switchgrass	Panicum virgatum 'Pathfinder'	Warm	Sod	389,000	1.0
Alkar tall wheatgrass	Agropyron elongatum 'Alkar'	Cool	Bunch	79,000	5.5
Total					10.75
Transition Turf Seed Mix ^c					
Ruebens Canadian bluegrass	Poa compressa 'Ruebens'	Cool	Sod	2,500,000	0.5
Dural hard fescue	Festuca ovina 'duriuscula'	Cool	Bunch	565,000	1.0
Citation perennial ryegrass	Lolium perenne 'Citation'	Cool	Sod	247,000	3.0
Lincoln smooth brome	Bromus inermis leyss 'Lincoln'	Cool	Sod	130,000	3.0
Total					7.5

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- Clean, weed-free and seed-free cereal grain straw should be applied evenly at a rate of 2 tons per acre and must be tacked or fastened by a method suitable for the condition of the site. Straw mulch must be anchored (and not merely placed) on the surface. This can be accomplished mechanically by crimping or with the aid of tackifiers or nets. Anchoring with a crimping implement is preferred, and is the recommended method for areas flatter than 3:1. Mechanical crimpers must be capable of tucking the long mulch fibers into the soil to a depth of 3 inches without cutting them. An agricultural disk, while not an ideal substitute, may work if the disk blades are dull or blunted and set vertically; however, the frame may have to be weighted to afford proper soil penetration.
- Grass hay may be used in place of straw; however, because hay is comprised of the entire plant including seed, mulching with hay may seed the site with non-native grass species which might in turn out-compete the native seed. Alternatively, native species of grass hay may be purchased, but can be difficult to find and are more expensive than straw. Purchasing and utilizing a certified weed-free straw is an easier and less costly mulching method. When using grass hay, follow the same guidelines as for straw (provided above).
- On small areas sheltered from the wind and heavy runoff, spraying a tackifier on the mulch is satisfactory for holding it in place. For steep slopes and special situations where greater control is needed, erosion control blankets anchored with stakes should be used instead of mulch.
- Hydraulic mulching consists of wood cellulose fibers mixed with water and a tackifying agent and should be applied at a rate of no less than 1,500 pounds per acre (1,425 lbs of fibers mixed with at least 75 lbs of tackifier) with a hydraulic mulcher. For steeper slopes, up to 2000 pounds per acre may be required for effective hydroseeding. Hydromulch typically requires up to 24 hours to dry; therefore, it should not be applied immediately prior to inclement weather. Application to roads, waterways and existing vegetation should be avoided.
- Erosion control mats, blankets, or nets are recommended to help stabilize steep slopes (generally 3:1 and steeper) and waterways. Depending on the product, these may be used alone or in conjunction with grass or straw mulch. Normally, use of these products will be restricted to relatively small areas. Biodegradable mats made of straw and jute, straw-coconut, coconut fiber, or excelsior can be used instead of mulch. (See the ECM/TRM BMP for more information.)
- Some tackifiers or binders may be used to anchor mulch. Check with the local jurisdiction for allowed tackifiers. Manufacturer's recommendations should be followed at all times. (See the Soil Binder BMP for more information on general types of tackifiers.)
- Rock can also be used as mulch. It provides protection of exposed soils to wind and water erosion and allows infiltration of precipitation. An aggregate base course can be spread on disturbed areas for temporary or permanent stabilization. The rock mulch layer should be thick enough to provide full coverage of exposed soil on the area it is applied.

Maintenance and Removal

After mulching, the bare ground surface should not be more than 10 percent exposed. Reapply mulch, as needed, to cover bare areas.

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Mulching (MU)

June 2012

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EROSION CONTROL CRITERIA:

EROSION CONTROL MEASURES SHALL BE IMPLEMENTED IN A MANNER THAT WILL PROTECT PROPERTIES AND PUBLIC FACILITIES FROM THE ADVERSE EFFECTS OF EROSION AND SEDIMENTATION AS A RESULT OF CONSTRUCTION AND EARTHWORK ACTIVITIES WITHIN THE PROJECT SITE.

- I. PRIOR TO START OF GRADING OPERATIONS, LOCATE AND SET THE SEDIMENT BERM AND VEHICLE TRACKING CONTROL AS SHOWN ON THE EROSION CONTROL PLAN.
- 2. THE SILT FENCE SHALL BE KEPT IN PLACE AND MAINTAINED UNTIL EROSION AND SEDIMENTATION POTENTIAL IS MITIGATED. REMOVAL OF SILT AND SEDIMENT COLLECTED BY THE SILT FENCE IS REQUIRED ONCE IT REACHES HALF THE HEIGHT OF THE SILT FENCE.
- 3. EROSION CONTROL DEVICES SHOULD BE CHECKED AFTER EVERY STORM OR NOT MORE THAN EVERY 14 DAYS. REPAIRS OR REPLACEMENT SHOULD BE MADE AS NECESSARY TO MAINTAIN PROPER PROTECTION.

SOIL EROSION CONTROL MEASURES FOR ALL SLOPES, CHANNELS, DITCHES, OR ANY DISTURBED LAND AREA SHALL BE COMPLETED WITHIN TWENTY-ONE (21) CALENDAR DAYS AFTER FINAL GRADING, OR FINAL EARTH DISTURBANCE HAS BEEN COMPLETED. DISTURBED AREAS AND STOCKPILES WHICH ARE NOT AT THE FINAL GRADE BUT WILL REMAIN DORMANT FOR LONGER THAN 30 DAYS SHALL ALSO BE MULCHED WITHIN 21 DAYS AFTER INTERIM GRADING. AN AREA THAT IS GOING TO REMAIN IN AN INTERIM STATE FOR MORE THAN 60 DAYS SHALL ALSO BE SEEDED. ALL TEMPORARY SOIL EROSION CONTROL MEASURES AND BMP'S SHALL BE MAINTAINED UNTIL PERMANENT SOIL EROSION CONTROL MEASURES ARE IMPLEMENTED.

STANDARD CONSTRUCTION NOTES:

- . ALL DRAINAGE AND ROADWAY CONSTRUCTION SHALL MEET THE STANDARDS AND SPECIFICATIONS OF THE CITY OF COLORADO SPRINGS/EL PASO COUNTY DRAINAGE CRITERIA MANUAL VOLUMES 1 AND 2, AND THE EL PASO COUNTY ENGINEERING CRITERIA MANUAL.
- 2. CONTRACTOR SHALL BE RESPONSIBLE FOR THE NOTIFICATION AND FIELD LOCATION OF ALL EXISTING UTILITIES, WHETHER SHOWN ON THE PLANS OR NOT, BEFORE BEGINNING CONSTRUCTION. LOCATION OF EXISTING UTILITIES SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. CALL 811 TO CONTACT THE UTILITY NOTIFICATION CENTER OF COLORADO
- 3. CONTRACTOR SHALL KEEP A COPY OF THESE APPROVED PLANS. THE GRADING AND EROSION CONTROL PLAN, THE STORMWATER MANAGEMENT PLAN (SWMP), THE SOILS AND GEOTECHNICAL REPORT AND THE APPROPRIATÉ DESIGN AND CONSTRUCTION STANDARDS AND SPECIFICATIONS AT THE JOB SITE AT ALL TIME INCLUDING THE FOLLOWING: 3.1 EL PASO COUNTY ENGINEERING CRITERIA MANUAL (ECM) 3.2 CITY OF COLORADO SPRINGS/EL PASO COUNTY ENGINEERING CRITERIA MANUAL VOLUMES 1 AND 2. 3.3 COLORADO DEPARTMENT OF TRANSPORTATION (CDOT) STANDARDS SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION. 3.4 CDOT M&S STANDARDS.
- 4. IT IS THE DESIGN ENGINEERS RESPONSIBILITY TO ACCURACY SHOW EXISTING CONDITION BOTH ONSITE AND OFFSITE ON THE CONSTRUCTION PLANS. ANY MODIFICATION NECESSARY DUE TO CONFLICT OMISSIONS OR CHANGED CONDITIONS WILL BE ENTIRELY THE DEVELOPERS RESPONSIBILITY TO RECTIFY.
- 5. ONCE THE ESQCP HAS BEEN ISSUED, THE CONTRACTOR MAY INSTALL THE INITIAL STAGE EROSION AND SEDIMENT CONTROL BMPS AS INDICATED ON THE GEC. A PRECONSTRUCTION MEETING BETWEEN THE CONTRACTOR. ENGINEER, AND EL PASO COUNTY WILL BE HELD PRIOR TO ANY CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE APPLICANT TO COORDINATE THE MEETING TIME AND PLACE WITH COUNTY PCD INSPECTIONS STAFF.
- 6. IT IS THE CONTRACTORS RESPONSIBILITY TO UNDERSTAND THE REQUIREMENTS OF ALL JURISDICTIONAL AGENCIES AND TO OBTAIN ALL REQUIRED PERMITS, INCLUDING BUT NOT LIMITED TO EL PASO COUNTY EROSION AND STORM WATER QUALITY CONTROL PERMIT (ESQCP), US ARMY CORPS OF ENGINEER ISSUED 401 AND/OR 404 PERMITS AND COUNTY AND STATE FUGITIVE DUST PERMITS.
- 7. ALL CONSTRUCTION TRAFFIC MUST ENTER/EXIT THE CONSTRUCTION SITE AT APPROVED CONSTRUCTION ACCESS POINTS.
- 8. ANY TEMPORARY SIGNAGE AND STRIPING SHALL COMPLY WITH EL PASO COUNTY DPW AND MUTCD CRITERIA.
- 9. CONTRACTOR SHALL OBTAIN ANY PERMITS REQUIRE BY EL PASO COUNTY DPW INCLUDING WORK WITHIN THE RIGHT-OF-WAY AND SPECIAL TRANSPORT PERMITS.
- 10. THE LIMITS OF CONSTRUCTION SHALL REMAIN WITHIN THE PROPERTY LINE UNLESS OTHERWISE NOTED. THE OWNER/DEVELOPER SHALL OBTAIN WRITTEN PERMISSION AND EASEMENTS, WHERE REQUIRED, FROM ADJOINING PROPERTY OWNER(S) PRIOR TO ANY OFFSITE DISTURBANCE GRADING, OR CONSTRUCTION.
- 11. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT, NOR DOES IT AFFIRM THAT SUFFICIENT EASEMENTS OR OTHER PERMISSIONS EXIST FOR ANY OFFSITE WORK.

	STORAGE YARD	ON CONTROL DETAILS	DATE: 03/30/2020	SHEET 4 OF 16 GR04
	ERLINE	D EROS	SCALE:	N/A VERTICAL: N/A
	BMIT	GRADING AN	PROJECT NO. 43-095	DESIGNED BY: GW DRAWN BY: GW CHECKED BY: VAS
	10 E DIKES DEAK AVE STH FLOOD	20 L. T. M.C. J. L. M. C. M. C		
				CIVIL CONSULTANTS, INC.
	VIRGIL A. SANCHEZ, COLORADO P.E. NO. 37160	FOR AND ON BEHALF OF BEHALF OF	CONSULTANTS,	ONAL ENGNESS
	PTION: APRV'D. BY: DATE:			THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER
-2	REVISIONS: NO. DATE: BY: DESCRII			THE ENGINEER PREPARING USES OF THESE PLANS. OF THESE PLANS.

EL PASO COUNTY FILE NO. PPR 19-04









SF-4



11.

GRADING AND EROSION CONTROL NOTES

STORMWATER DISCHARGES FROM CONSTRUCTION SITES SHALL NOT CAUSE OR THREATEN TO CAUSE POLLUTION, CONTAMINATION. OR DEGRADATION OF STATE WATERS. ALL WORK AND EARTH DISTURBANCE SHALL BE DONE IN A MANNER THAT MINIMIZES POLLUTION OF ANY ON-SITE OR OFF-SITE WATERS, INCLUDING WETLANDS.

NOTWITHSTANDING ANYTHING DEPICTED IN THESE PLANS IN WORDS OR GRAPHIC REPRESENTATION, ALL DESIGN AND CONSTRUCTION RELATED TO ROADS, STORM DRAINAGE AND EROSION CONTROL SHALL CONFORM TO THE STANDARDS AND REQUIREMENTS OF THE MOST RECENT VERSION OF THE RELEVANT ADOPTED EL PASO COUNTY STANDARDS, INCLUDING THE LAND DEVELOPMENT CODE, THE ENGINEERING CRITERIA MANUAL, THE DRAINAGE CRITERIA MANUAL, AND THE DRAINAGE CRITERIA MANUAL VOLUME 2. ANY DEVIATIONS FROM REGULATIONS AND STANDARDS MUST BE REQUESTED, AND APPROVED, IN WRITING.

A SEPARATE STORMWATER MANAGEMENT PLAN (SMWP) FOR THIS PROJECT SHALL BE COMPLETED AND AN EROSION AND STORMWATER QUALITY CONTROL PERMIT (ESQCP) ISSUED PRIOR TO COMMENCING CONSTRUCTION. MANAGEMENT OF THE SWMP DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE DESIGNATED QUALIFIED STORMWATER MANAGER OR CERTIFIED EROSION CONTROL INSPECTOR. THE SWMP SHALL BE LOCATED ON SITE AT ALL TIMES DURING CONSTRUCTION AND SHALL BE KEPT UP TO DATE WITH WORK PROGRESS AND CHANGES IN THE FIELD.

ONCE THE ESQCP IS APPROVED AND A 'NOTICE TO PROCEED" HAS BEEN ISSUED, THE CONTRACTOR MAY INSTALL THE INITIAL STAGE EROSION AND SEDIMENT CONTROL MEASURES AS INDICATED ON THE APPROVED GEC. A PRECONSTRUCTION MEETING BETWEEN THE CONTRACTOR, ENGINEER, AND EL PASO COUNTY WILL BE HELD PRIOR TO ANY CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE APPLICANT TO COORDINATE THE MEETING TIME AND PLACE WITH COUNTY STAFF.

CONTROL MEASURES MUST BE INSTALLED PRIOR TO COMMENCEMENT OF ACTIVITIES THAT COULD CONTRIBUTE POLLUTANTS TO STORMWATER. CONTROL MEASURES FOR ALL SLOPES, CHANNELS, DITCHES, AND DISTURBED LAND AREAS SHALL BE INSTALLED IMMEDIATELY UPON COMPLETION OF THE DISTURBANCE.

ALL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES SHALL BE MAINTAINED AND REMAIN IN EFFECTIVE OPERATING CONDITION UNTIL PERMANENT SOIL EROSION CONTROL MEASURES ARE IMPLEMENTED AND FINAL STABILIZATION IS ESTABLISHED. ALL PERSONS ENGAGED IN LAND DISTURBANCE ACTIVITIES SHALL ASSESS THE ADEQUACY OF CONTROL MEASURES AT THE SITE AND IDENTIFY IF CHANGES TO THOSE CONTROL MEASURES ARE NEEDED TO ENSURE THE CONTINUED EFFECTIVE PERFORMANCE OF THE CONTROL MEASURES. ALL CHANGES TO TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES MUST BE INCORPORATED INTO THE STORMWATER MANAGEMENT PLAN.

TEMPORARY STABILIZATION SHALL BE IMPLEMENTED ON DISTURBED AREAS AND STOCKPILES WHERE GROUND DISTURBING CONSTRUCTION ACTIVITY HAS PERMANENTLY CEASED OR TEMPORARILY CEASED FOR LONGER THAN 14 DAYS.

FINAL STABILIZATION MUST BE IMPLEMENTED AT ALL APPLICABLE CONSTRUCTION SITES. FINAL STABILIZATION IS ACHIEVED WHEN ALL GROUND DISTURBING ACTIVITIES ARE COMPLETE AND ALL DISTURBED AREAS EITHER HAVE A UNIFORM VEGETATIVE COVER WITH INDIVIDUAL PLANT DENSITY OF 70 PERCENT OF PRE-DISTURBANCE LEVELS ESTABLISHED OR EQUIVALENT PERMANENT ALTERNATIVE STABILIZATION METHOD IS IMPLEMENTED. ALL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES SHALL BE REMOVED UPON FINAL STABILIZATION AND BEFORE PERMIT CLOSURE.

ALL PERMANENT STORMWATER MANAGEMENT FACILITIES SHALL BE INSTALLED AS DESIGNED IN THE APPROVED PLANS. ANY PROPOSED CHANGES THAT EFFECT THE DESIGN OR FUNCTION OF PERMANENT STORMWATER MANAGEMENT STRUCTURES MUST BE APPROVED BY THE ECM ADMINISTRATOR PRIOR TO IMPLEMENTATION.

10. EARTH DISTURBANCES SHALL BE CONDUCTED IN SUCH A MANNER SO AS TO EFFECTIVELY MINIMIZE ACCELERATED SOIL EROSION AND RESULTING SEDIMENTATION. ALL DISTURBANCES SHALL BE DESIGNED, CONSTRUCTED, AND COMPLETED SO THAT THE EXPOSED AREA OF ANY DISTURBED LAND SHALL BE LIMITED TO THE SHORTEST PRACTICAL PERIOD OF TIME. PRE-EXISTING VEGETATION SHALL BE PROTECTED AND MAINTAINED WITHIN 50 HORIZONTAL FEET OF A WATERS OF THE STATE UNLESS SHOWN TO BE INFEASIBLE AND SPECIFICALLY REQUESTED AND APPROVED.

COMPACTION OF SOIL MUST BE PREVENTED IN AREAS DESIGNATED FOR INFILTRATION CONTROL MEASURES OR WHERE FINAL STABILIZATION WILL BE ACHIEVED BY VEGETATIVE COVER. AREAS DESIGNATED FOR INFILTRATION CONTROL MEASURES SHALL ALSO BE PROTECTED FROM SEDIMENTATION DURING CONSTRUCTION UNTIL FINAL STABILIZATION IS ACHIEVED. IF COMPACTION PREVENTION IS NOT FEASIBLE DUE TO SITE CONSTRAINTS, ALL AREAS DESIGNATED FOR INFILTRATION AND VEGETATION CONTROL MEASURES MUST BE LOOSENED PRIOR TO INSTALLATION OF THE CONTROL MEASURE(S).

12. ANY TEMPORARY OR PERMANENT FACILITY DESIGNED AND CONSTRUCTED FOR THE CONVEYANCE OF STORMWATER AROUND, THROUGH, OR FROM THE EARTH DISTURBANCE AREA SHALL BE A STABILIZED CONVEYANCE DESIGNED TO MINIMIZE EROSION AND THE DISCHARGE OF SEDIMENT OFF SITE.

CONCRETE WASH WATER SHALL BE CONTAINED AND DISPOSED OF IN ACCORDANCE WITH THE SWMP. NO WASH WATER SHALL BE DISCHARGED TO OR ALLOWED TO ENTER STATE WATERS, INCLUDING ANY SURFACE OR SUBSURFACE STORM DRAINAGE SYSTEM OR FACILITIES. CONCRETE WASHOUTS SHALL NOT BE LOCATED IN AN AREA WHERE SHALLOW GROUNDWATER MAY BE PRESENT, OR WITHIN 50 FEET OF A SURFACE WATER BODY, CREEK OR STREAM.

14. DURING DEWATERING OPERATIONS OF UNCONTAMINATED GROUND WATER MAY BE DISCHARGED ON SITE, BUT SHALL NOT LEAVE THE SITE IN THE FORM OF SURFACE RUNOFF UNLESS AN APPROVED STATE DEWATERING PERMIT IS IN PLACE. 15. EROSION CONTROL BLANKETING OR OTHER PROTECTIVE COVERING SHALL BE USED ON SLOPES STEEPER THAN 3:1.

CONTRACTOR SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL WASTES FROM THE CONSTRUCTION SITE FOR DISPOSAL IN ACCORDANCE WITH LOCAL AND STATE REGULATORY REQUIREMENTS. NO CONSTRUCTION DEBRIS, TREE SLASH, BUILDING MATERIAL WASTES OR UNUSED BUILDING MATERIALS SHALL BE BURIED, DUMPED, OR DISCHARGED AT THE SITE.

17. WASTE MATERIALS SHALL NOT BE TEMPORARILY PLACED OR STORED IN THE STREET, ALLEY, OR OTHER PUBLIC WAY, UNLESS IN ACCORDANCE WITH AN APPROVED TRAFFIC CONTROL PLAN. CONTROL MEASURES MAY BE REQUIRED BY EL PASO COUNTY ENGINEERING IF DEEMED NECESSARY, BASED ON SPECIFIC CONDITIONS AND CIRCUMSTANCES.

18. TRACKING OF SOILS AND CONSTRUCTION DEBRIS OFF-SITE SHALL BE MINIMIZED. MATERIALS TRACKED OFF-SITE SHALL BE CLEANED UP AND PROPERLY DISPOSED OF IMMEDIATELY.

19. THE OWNER/DEVELOPER SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL CONSTRUCTION DEBRIS, DIRT, TRASH, ROCK, SEDIMENT, SOIL, AND SAND THAT MAY ACCUMULATE IN ROADS, STORM DRAINS AND OTHER DRAINAGE CONVEYANCE SYSTEMS AND STORMWATER APPURTENANCES AS A RESULT OF SITE DEVELOPMENT.

20. THE QUANTITY OF MATERIALS STORED ON THE PROJECT SITE SHALL BE LIMITED, AS MUCH AS PRACTICAL, TO THAT QUANTITY REQUIRED TO PERFORM THE WORK IN AN ORDERLY SEQUENCE. ALL MATERIALS STORED ON-SITE SHALL BE STORED IN A NEAT, ORDERLY MANNER, IN THEIR ORIGINAL CONTAINERS, WITH ORIGINAL MANUFACTURER'S LABELS.

21. NO CHEMICAL(S) HAVING THE POTENTIAL TO BE RELEASED IN STORMWATER ARE TO BE STORED OR USED ONSITE UNLESS PERMISSION FOR THE USE OF SUCH CHEMICAL(S) IS GRANTED IN WRITING BY THE ECM ADMINISTRATOR. IN GRANTING APPROVAL FOR THE USE OF SUCH CHEMICAL(S), SPECIAL CONDITIONS AND MONITORING MAY BE REQUIRED.

22. BULK STORAGE OF ALLOWED PETROLEUM PRODUCTS OR OTHER ALLOWED LIQUID CHEMICALS IN EXCESS OF 55 GALLONS SHALL REQUIRE ADEQUATE SECONDARY CONTAINMENT PROTECTION TO CONTAIN ALL SPILLS ONSITE AND TO PREVENT ANY SPILLED MATERIALS FROM ENTERING STATE WATERS, ANY SURFACE OR SUBSURFACE STORM DRAINAGE SYSTEM OR OTHER

23. NO PERSON SHALL CAUSE THE IMPEDIMENT OF STORMWATER FLOW IN THE CURB AND GUTTER OR DITCH EXCEPT WITH APPROVED SEDIMENT CONTROL MEASURES.

24. OWNER/DEVELOPER AND THEIR AGENTS SHALL COMPLY WITH THE "COLORADO WATER QUALITY CONTROL ACT" (TITLE 25, ARTICLE 8, CRS), AND THE "CLEAN WATER ACT" (33 USC 1344), IN ADDITION TO THE REQUIREMENTS OF THE LAND DEVELOPMENT CODE, DCM VOLUME II AND THE ECM APPENDIX I. ALL APPROPRIATE PERMITS MUST BE OBTAINED BY THE CONTRACTOR PRIOR TO CONSTRUCTION (1041, NPDES, FLOODPLAIN, 404, FUGITIVE DUST, ETC.). IN THE EVENT OF CONFLICTS BETWEEN THESE REQUIREMENTS AND OTHER LAWS, RULES, OR REGULATIONS OF OTHER FEDERAL, STATE, LOCAL, OR COUNTY AGENCIES, THE MOST RESTRICTIVE LAWS, RULES, OR REGULATIONS SHALL APPLY.

25. ALL CONSTRUCTION TRAFFIC MUST ENTER/EXIT THE SITE ONLY AT APPROVED CONSTRUCTION ACCESS POINTS. 26. PRIOR TO CONSTRUCTION THE PERMITTEE SHALL VERIFY THE LOCATION OF EXISTING UTILITIES.

27. A WATER SOURCE SHALL BE AVAILABLE ON SITE DURING EARTHWORK OPERATIONS AND SHALL BE UTILIZED AS REQUIRED TO MINIMIZE DUST FROM EARTHWORK EQUIPMENT AND WIND.

THE SOILS REPORT FOR THIS SITE HAS BEEN PREPARED BY CTL THOMPSON, INC., ENTITLED GEOTECHNICAL INVESTIGATION TIMBERLINE LANDSCAPING OFFICE AND WAREHOUSE, DATED MAY 5, 2017, AND SHALL BE CONSIDERED A PART OF THESE PLANS

29. AT LEAST TEN (10) DAYS PRIOR TO THE ANTICIPATED START OF CONSTRUCTION, FOR PROJECTS THAT WILL DISTURB ONE (1) ACRE OR MORE, THE OWNER OR OPERATOR OF CONSTRUCTION ACTIVITY SHALL SUBMIT A PERMIT APPLICATION FOR STORMWATER DISCHARGE TO THE COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT, WATER QUALITY DIVISION. THE APPLICATION CONTAINS CERTIFICATION OF COMPLETION OF A STORMWATER MANAGEMENT PLAN (SWMP), OF WHICH THIS GRADING AND EROSION CONTROL PLAN MAY BE A PART. FOR INFORMATION OR APPLICATION MATERIALS CONTACT:

COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT WATER QUALITY CONTROL DIVISION WQCD - PERMITS 4300 CHERRY CREEK DRIVE SOUTH DENVER, CO 80246-1530

ATTN: PERMITS UNIT

28.

REVISIONS: Incl. a. Sanchez, colorado P.E. No. 37160 No. Date: Br. Description: Incl. a. Sanchez, colorado P.E. No. 37160 Incl. a. Sanchez, colorado P.E. No. 37160 Incl. a. Sanchez, colorado P.E. No. 37160 No. Date: Br. Description: Incl. a. Sanchez, colorado P.E. No. 37160 Incl. a. Sanchez, colorado P.E. No. 37160 No. Date: Br. Description: Incl. a. Sanchez, colorado P.E. No. 37160 Incl. a. Sanchez, colorado P.E. No. 37160 Incl. a. Sanchez, colorado P.E. No. 37160 No. Date: Br. Description: Incl. a. Sanchez, colorado P.E. No. 37160 Incl. a. Sanchez, colorado P.E. No. 37160 No. Date: Date:	
NO. DATE: BY: DATE: DAT	
FOR AND ON BEHALF OF MAS CIVIL	IIMBERLINE SIURAGE TARD
	NDING AND EROSION CONTROL DETAILS
PROJECT NO. 43–095 SCALE:	NO. 43-095 SCALE: DATE: 03/30/2020
THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER N/A	D BY: GW HORLZONIAL: 3Y: GW VERTICAL: SHEET 6 OF 16 GROG

EL PASO COUNTY FILE NO. PPR 19-042

synthetic fibers, filame	Mat (TRM) : A rolled erosion control product composed of non-degradable nents, nets, wire mesh, and/or other elements, processed into a permanent, three-	Table RE	CP-1. EC
dimensional matrix of components, are desig and provide long-term maturation. Note: TR	f sufficient thickness. TRMs, which may be supplemented with degradable gned to impart immediate erosion protection, enhance vegetation establishment a functionality by permanently reinforcing vegetation during and after RMs are typically used in hydraulic applications, such as high flow ditches and	Product De	escription
channels, steep slopes, natural, unreinforced v	s, stream banks, and shorelines, where erosive forces may exceed the limits of vegetation or in areas where limited vegetation establishment is anticipated.		
Tables RECP-1 and RECP to site conditions and desin will provide permanent ero	P-2 provide guidelines for selecting rolled erosion control products appropriate ired longevity. Table RECP-1 is for conditions where natural vegetation alone cosion control, whereas Table RECP-2 is for conditions where vegetation alone	Mulch Contr	rol Nets
will not be adequately stab	ble to provide long-term erosion protection due to flow or other conditions.	Netless Roll Erosion Con Blankets	ed itrol
		Single-net E Control Blar Open Weave	brosion nkets & e Textiles
		Double-net I Control Blar	Erosion nkets
		Mulch Contr	rol Nets
		Erosion Con Blankets & G Weave Texti (slowly degr	ıtrol Open iles rading)
		Erosion Con Blankets & O Weave Text	ıtrol Open iles
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		² C Factor ca gradient, H. ¹ ³ Required n erosion (> 12 ⁴ The permis experience v ⁵ Acceptable acceptable b ⁶ Per the eng D 6460, or o	alculated a V) to ratic ninimum s 2.7 mm (0 ssible shea with produ e large-sca by the enging gineer's di other indep
RECP-2	Urban Drainage and Flood Control DistrictNovember 2010Urban Storm Drainage Criteria Manual Volume 3	November 2	2010
SM-6	Stabilized Staging Area (SSA)	Stabili	ized S
STABILIZED_STAG 5. STABILIZED S STORAGE, AND 1	<u>GING AREA MAINTENANCE NOTES</u> STAGING AREA SHALL BE ENLARGED IF NECESSARY TO CONTAIN PARKING, UNLOADING/LOADING OPERATIONS.		
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ntrol Products (RECP)

EC-6

d Specification for Temporary Rolled Erosion Control Products m Erosion Control Technology Council 2005)

ica	pe itions*	* Channel Applications* Minimum Tensile Strength ¹		Expected Longevity
ı	C Factor ^{2,5}	Max. Shear Stress ^{3,4,6}		
	≤0.10 @ 5:1	0.25 lbs/ft ² (12 Pa)	5 lbs/ft (0.073 kN/m)	
)	≤0.10 @ 4:1	0.5 lbs/ft ² (24 Pa)	5 lbs/ft (0.073 kN/m)	Up to 12
	≤0.15 @ 3:1	1.5 lbs/ft ² (72 Pa)	50 lbs/ft (0.73 kN/m)	months
)	≤0.20 @ 2:1	1.75 lbs/ft ² (84 Pa)	75 lbs/ft (1.09 kN/m)	
)	≤0.10 @ 5:1	0.25 lbs/ft ² (12 Pa)	25 lbs/ft (0.36 kN/m)	24 months
)	≤0.25 @ 1.5:1	2.00 lbs/ft ² (96 Pa)	100 lbs/ft (1.45 kN/m)	24 months
	≤0.25 @ 1:1	2.25 lbs/ft ² (108 Pa)	125 lbs/ft (1.82 kN/m)	36 months

h control nettings must be obtained with netting used in conjunction *ee Section 5.3 of Chapter 7 Construction BMPs for more information*

achine direction using ECTC Mod. ASTM D 5035.

loss from RECP protected slope (tested at specified or greater

om unprotected (control) plot in large-scale testing. CCP (unvegetated) can sustain without physical damage or excess

) during a 30-minute flow event in large-scale testing.

established for each performance category are based on historical zed by Manning's roughness coefficients in the range of 0.01 - 0.05.

s may include ASTM D 6459, or other independent testing deemed

ommended acceptable large-scale testing protocol may include ASTM g deemed acceptable by the engineer.

rainage and Flood Control District RECP-3 n Drainage Criteria Manual Volume 3

PERM

SM-6

Area (SSA)

— SF/CF — SF/CF — SSA ONSITE CONSTRUCTION VEHICLE PARKING (1F CONSTRUCTION **FRAILERS** NEEDED) 3" MIN. THICKNESS MATERIAL STORAGE AREA GRANULAR MATERIAL _____ SILT FENCE OR CONSTRUCTION FENCING AS NEEDED — SF/CF — SF/CF – TING ROADWAY STABILIZED STAGING AREA STALLATION NOTES AREA(S) JUST LOCATION AND SIZE OF STAGING AREA WITH APPROVAL

EA SHOULD BE APPROPRIATE FOR THE NEEDS OF THE SITE. LARGER AREA TO STABILIZE FOLLOWING CONSTRUCTION. E STABILIZED PRIOR TO OTHER OPERATIONS ON THE SITE.

G AREA SHALL CONSIST OF A MINIMUM 3" THICK GRANULAR ECIFIED BY LOCAL JURISDICTION, ROCK SHALL CONSIST OF DOT COARSE AGGREGATE OR 6" (MINUS) ROCK. BMPs MAY BE REQUIRED INCLUDING BUT NOT LIMITED TO SILT I FENCING.

MAINTENANCE NOTES ЮRKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. HOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPS AS SOON AS ITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE

IECESSARY MAINTENANCE. IS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN DITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE

LED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON RE.

PLIED OR REGRADED AS NECESSARY IF RUTTING OCCURS OR ECOMES EXPOSED.

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INITIAL

SSA-2

SSA-3

EC-6

Rolled Erosion Control Products (RECP)

 Table RECP-2. ECTC Standard Specification for Permanent¹ Rolled Erosion Control Products (Adapted from: Erosion Control Technology Council 2005)

Product Type	Slope Applications	Channel Applications	
	Maximum Gradient	Maximum Shear Stress ^{4,5}	Minimum Tensile Strength ^{2,3}
TRMs with a minimum thickness of 0.25 inches (6.35 mm) per ASTM D	0.5:1 (H:V)	6.0 lbs/ft ² (288 Pa)	125 lbs/ft (1.82 kN/m)
ASTM D 4355 (500 hours exposure).	0.5:1 (H:V)	8.0 lbs/ft ² (384 Pa)	150 lbs/ft (2.19 kN/m)
	0.5:1 (H:V)	10.0 lbs/ft ² (480 Pa)	175 lbs/ft (2.55 kN/m)

¹ For TRMs containing degradable components, all property values must be obtained on the nondegradable portion of the matting alone.

² Minimum Average Roll Values, machine direction only for tensile strength determination using <u>ASTM</u> <u>D 6818</u> (Supersedes Mod. <u>ASTM D 5035</u> for RECPs)

³ Field conditions with high loading and/or high survivability requirements may warrant the use of a TRM with a tensile strength of 44 kN/m (3,000 lb/ft) or greater.

⁴ Required minimum shear stress TRM (fully vegetated) can sustain without physical damage or excess erosion (> 12.7 mm (0.5 in.) soil loss) during a 30-minute flow event in large scale testing.
 ⁵ Acceptable large-scale testing protocols may include <u>ASTM D 6460</u>, or other independent testing deemed acceptable by the engineer.

Design and Installation

RECPs should be installed according to manufacturer's specifications and guidelines. Regardless of the type of product used, it is important to ensure no gaps or voids exist under the material and that all corners of the material are secured using stakes and trenching. Continuous contact between the product and the soil is necessary to avoid failure. Never use metal stakes to secure temporary erosion control products. Often wooden stakes are used to anchor RECPs; however, wood stakes may present installation and maintenance challenges and generally take a long time to biodegrade. Some local jurisdictions have had favorable experiences using biodegradable stakes.

This BMP Fact Sheet provides design details for several commonly used ECB applications, including:

ECB-1 Pipe Outlet to Drainageway

ECB-2 Small Ditch or Drainageway

ECB-3 Outside of Drainageway

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SM-6

Stabilized Staging Area (SSA)

Minimizing Long-Term Stabilization Requirements

Utilize off-site parking and restrict vehicle access to the site.

- Use construction mats in lieu of rock when staging is provided in an area that will not be disturbed otherwise.
- Consider use of a bermed contained area for materials and equipment that do not require a stabilized surface.
- Consider phasing of staging areas to avoid disturbance in an area that will not be otherwise disturbed.

See Detail SSA-1 for a typical stabilized staging area and SSA-2 for a stabilized staging area when materials staging in roadways is required.

Maintenance and Removal

Maintenance of stabilized staging areas includes maintaining a stable surface cover of gravel, repairing perimeter controls, and following good housekeeping practices.

When construction is complete, debris, unused stockpiles and materials should be recycled or properly disposed. In some cases, this will require disposal of contaminated soil from equipment leaks in an appropriate landfill. Staging areas should then be permanently stabilized with vegetation or other surface cover planned for the development.

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STANDARD CONSTRUCTION NOTES:

- 1. ALL DRAINAGE AND ROADWAY CONSTRUCTION SHALL MEET THE STANDARDS AND SPECIFICATIONS OF THE CITY OF COLORADO SPRINGS/EL PASO COUNTY DRAINAGE CRITERIA MANUAL VOLUMES 1 AND 2, AND THE EL PASO COUNTY ENGINEERING CRITERIA MANUAL.
- REPORT AND THE APPROPRIATE DESIGN AND CONSTRUCTION STANDARDS AND SPECIFICATIONS AT THE JOB SITE AT ALL TIME INCLUDING THE FOLLOWING: 3.1 EL PASO COUNTY ENGINEERING CRITERIA MANUAL (ECM)
- 3.2 CITY OF COLORADO SPRINGS/EL PASO COUNTY ENGINEERING CRITERA MANUAL VOLUMES 1 AND 2. 3.3 COLORADO DEPARTMENT OF TRANSPORTATION (CDOT) STANDARDS SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION. 3.4 CDOT M&S STANDARDS.
- CONFLICT OMISSIONS OR CHANGED CONDITIONS WILL BE ENTIRELY THE DEVELOPERS RESPONSIBILITY TO RECTIFY.
- EL PASO COUNTY EROSION AND STORM WATER QUALITY CONTROL PERMIT (ESQCP), REGIONAL BUILDING FLOODPLAIN DEVELOPMENT PERMIT, US ARMY CORPS OF ENGINEER ISSUED 401 AND/OR 404 PERMITS AND COUNTY AND STATE FUGITIVE DUST PERMITS.
- 6. ANY TEMPORARY SIGNAGE AND STRIPING SHALL COMPLY WITH EL PASO COUNTY PCD AND MUTCD CRITERIA.
- 7. CONTRACTOR SHALL OBTAIN ANY PERMITS REQUIRED BY EL PASO COUNTY DPW INCLUDING WORK WITHIN THE RIGHT-OF-WAY AND SPECIAL TRANSPORT PERMITS.
- 8. THE LIMITS OF CONSTRUCTION SHALL REMAIN WITHIN THE PROPERTY LINE UNLESS OTHERWISE NOTED. THE OWNER/DEVELOPER SHALL OBTAIN WRITTEN PERMISSION AND EASEMENTS, WHERE REQUIRED, FROM ADJOINING PROPERTY OWNER(S) PRIOR TO ANY OFFSITE DISTURBANCE GRADING, OR CONSTRUCTION.

STORM SEWER GENERAL NOTES

- 1. ALL STATIONING IS ALONG STORM SEWER CENTERLINE UNLESS OTHERWISE INDICATED. ALL ELEVATIONS ARE INVERT UNLESS OTHERWISE INDICATED.
- 2. ALL STORM SEWER BENDS AND WYES SHOWN ON THE PLAN SHALL BE PREFABRICATED.
- 3. HORIZONTAL AND VERTICAL BENDS ARE INDICATED ON THE PLANS.
- SHALL THE MAXIMUM JOINT OPENING FOR STRAIGHT ALIGNMENT EXCEED 1 INCH OR ONE AND ONE-HALF INCH ON CURVED ALIGNMENT.
- 5. INLET DIMENSIONS SHOWN ON PLANS REFER TO DISTANCES FROM INSIDE FACES OF BOX BETWEEN THE WIDTHS AND LENGTHS.
- 7. SINCE ALL PIPE ENTRIES INTO THE BASE ARE VARIABLE, THE DIMENSIONS SHOWN ARE TYPICAL. ACTUAL DIMENSIONS AND QUANTITIES FOR CONCRETE AND REINFORCEMENT SHALL BE AS REQUIRED IN THE WORK.
- 8. STEPS SHALL BE REQUIRED WHEN THE MANHOLE DEPTH EXCEEDS 3'-6" AND SHALL BE IN ACCORDANCE WITH AASHTO M 199.
- 10. FLOW CHANNELS AND INVERTS SHALL BE FORMED BY SHAPING WITH CLASS B CONCRETE OR APPROVED GROUT.
- 11. CHECK WITH THE LOCAL GOVERNMENT AUTHORITY FOR ANY ADDITIONAL STORM SEWER SPECIFICATIONS, DETAILS, OR REGULATIONS.
- 12. THE CONTRACTOR SHALL PROVIDE SHOP DRAWINGS OF ALL PREFABRICATED STRUCTURES TO THE ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

STRUCTURAL CONCRETE NOTES:

- COLORADO DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROADWAY AND BRIDGE CONSTRUCTION.
- 2. STEEL REINFORCING SHALL BE GRADE 60 FOR ALL REINFORCING STEEL GREATER THAN #4. SPLICING, LAP SPLICING SHALL BE MINIMUM IN THE FOLLOWING TABLE UNLESS OTHERWISE SPECIFIED: bar size #4 #5 #6 #7
- SPLICE LENGTH 1["]-9" 2["]-2" 2["]-7" 3["]-4" 4^{"-}3" ALL REINFORCING SHALL HAVE A 2-INCH MINIMUM COVER UNLESS OTHERWISE SPECIFIED. ALL REINFORCED STEEL TO BE EPOXY COATED.
- 4. EXPANSION JOINT MATERIAL SHALL MEET AASHTO SPECIFICATION M-213.
- SHALL BE PLACED EQUALLY ON EACH SIDE OF RETAINING WALL STRUCTURES AND CUTOFF WALLS UNTIL THE FINAL GRADE IS REACHED.
- IN THE ABSENCE OF TESTING SHALL BE COMPLETED AT THE SOLE RISK OF THE CONTRACTOR.
- 7. PRIOR TO THE PLACEMENT OF CONCRETE IN AREAS WHERE SOIL IS PRESENT, THE SOIL SHALL BE SCARIFIED TO A MINIMUM DEPTH OF 6-INCHES. THE MOISTURE CONTENT SHALL BE

ABBREVIATIONS EC -- EPOXY COATED O.F. -- OUTSIDE FACE E.F. -- EACH FACE E.W. -- EACH WAY I.F. -- INSIDE FACE N.F. -- NEAR FACE T.O.C. -- TOP OF CONCRETE B.O.C. -- BOTTOM OF CONCRETE CONT. -- CONTINUOUS



2. CONTRACTOR SHALL BE RESPONSIBLE FOR THE NOTIFICATION AND FIELD LOCATION OF ALL EXISTING UTILITIES, WHETHER SHOWN ON THE PLANS OR NOT, BEFORE BEGINNING CONSTRUCTION. LOCATION OF EXISTING UTILITIES SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. CALL 811 TO CONTACT THE UTILITY NOTIFICATION CENTER OF COLORADO (UNCC). 3. CONTRACTOR SHALL KEEP A COPY OF THESE APPROVED PLANS, THE GRADING AND EROSION CONTROL PLAN, THE STORMWATER MANAGEMENT PLAN (SWMP), THE SOILS AND GEOTECHNICAL

4. IT IS THE DESIGN ENGINEERS RESPONSIBILITY TO ACCURACY SHOW EXISTING CONDITION BOTH ONSITE AND OFFSITE ON THE CONSTRUCTION PLANS. ANY MODIFICATION NECESSARY DUE TO 5. IT IS THE CONTRACTORS RESPONSIBILITY TO UNDERSTAND THE REQUIREMENTS OF ALL JURISDICTIONAL AGENCIES AND TO OBTAIN ALL REQUIRED PERMITS, INCLUDING BUT NOT LIMITED TO

4. JOINTS SHALL BE IN ACCORDANCE WITH ASTM C443 "STANDARD SPECIFICATIONS FOR JOINTS FOR CIRCULAR CONCRETE SEWER AND CULVERT PIPE USING RUBBER GASKET." IN NO CASE

6. ALL STORM SEWER SHALL BE A MINIMUM OF CLASS III REINFORCED CONCRETE PIPE. SPECIFIC SEGMENTS OF STORM SEWER SHALL BE REQUIRED TO BE CONSTRUCTED OF A MINIMUM OF 5000 PSI CONCRETE DUE TO EXCESSIVE VELOCITIES. REFER TO ADDITIONAL NOTES WITHIN CONSTRUCTION PLANS.

9. ALL REINFORCING STEEL SHALL HAVE A MINIMUM YIELD STRENGTH OF 60,000 PSI. VERTICAL STEEL SHALL BE PLACED AT 🖗 OF WALL. ALL BARS SHALL HAVE A 2" MINIMUM CLEARANCE.

1. ALL CONSTRUCTION INVOLVING THE PLACEMENT OF STRUCTURAL CONCRETE SHALL BE COMPLETED IN ACCORDANCE WITH STANDARD SPECIFICATIONS, AND AS SUPPLEMENTED BY THE

3. CAST-IN-PLACE CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (fc) OF 4,000 PSI AT 28 DAYS. ALL CONCRETE PLACED AGAINST SOIL SHALL BE TYPE II PORTLAND CEMENT. ALL EXPOSED CORNERS SHALL BE FORMED WITH A 3/4" CHAMFER UNLESS OTHERWISE SPECIFIED.

5. BACKFILL AGAINST STRUCTURES SHALL NOT COMMENCE UNTIL ALL SUPPORTING DIAPHRAGMS ARE IN PLACE AND CONCRETE HAS OBTAINED ITS FULL SEVEN DAY STRENGTH. BACKFILL 6. FOOTING EXCAVATIONS SHALL BE EXAMINED BY THE GEOTECHNICAL ENGINEER WITH A 24-HOUR MINIMUM NOTIFICATION FOR SOIL AND/OR CONCRETE TESTING. PLACEMENT OF CONCRETE

ADJUSTED TO WITHIN PLUS OR MINUS 2 PERCENT OF THE OPTIMUM MOISTURE CONTENT AND RECOMPACTED TO AT LEAST 95 PERCENT RELATIVE COMPACTION (AASHTO-T-180).

			TING NG NE ON DIG 37
REVISIONS:	VIRGIL A. SANCHEZ, COLORADO P.E. NO. 37160		
NO. DATE: BY: DESCRIPTION: APRV'D. BY: DESCRIPTION: APRV'D. BY: DATE:	annual and an annual and an annual and an	102 E. PIKES PEAK AVE. 5TH FLOOR	IIMBERLINE SIORAGE YARD
	FOR AND ON	COLORADO SPRINGS, CO 80903	CENERAL NOTES AND DETAILS
	BEHALF OF	PHONE: /19.955.5485	
			PROJECT NO. 43-095 SCALE. STATE AT / TA / TA / TA
THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.	A CONSTRUCTION OF CONSTRUCTURE	CIVIL CONSULTANTS, INC.	DESIGNED BY: ET N/A DRAWN BY: ELY VERTICAL: SHEET 8 OF 16 STO1 CHECKED BY: VAS N/A SHEET 8 OF 16 STO1









EXISTING

ASPHALT DEMO PLAN **OPPORTUNITY VIEW** EAST ENTRANCE

50							
15							
40							
35							
30							
25							
20							
5							
- 6534 7/	1						
UT (42")	+ = 6529.96						











THE DESIGN; THE CITY HAS LIMITED ITS SCOPE OF REVIEW ACCORDINGLY. RESUBMITTAL REQUIRED IF CONSTRUCTION HAS NOT COMMENCED WITHIN 180 DAYS AFTER APPROVAL DATE.

FOR BURIED UTILITY INFORMATION 48 HRS BEFORE YOU DIG CALL 1-800-922-1987

AS-BUILT ENGINEERING RECORD DRAWINGS

07/24/2023

TEMP DWIRE POND 4:1 RUNDOWN

NOT TO SCALE

				ST07
	NURAGE TARL	WER PLANS	DATE: 03-30-20	SHEET 15 OF 16
		STORM SEV	PROJECT NO. 43-095 SCALE:	DESIGNED BY: GT HURLONIAL: DRAWN BY: JWP VERTICAL: CHECKED BY: GT 1"=5'
	20 BOULDER CRESCENT, SUITE 110	COLORADO SPRINGS, CO 80903 PHONE: 719.955.5485		
				CIVIL CONSULTANTS, INC
. 37160		FOR AND ON BEHALF OF	M&S CIVIL CONSULTANTS, INF	
VIRGIL A. SANCHEZ, COLORADO P.E. NC	Appendent and	MARTIN CONCERNING		PROFESSION CONTRACTOR
/ISIONS:	DATE: BY: DESCRIPTION: APRV'D. BY: DATE:			THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.
RE	NO.			CAUTION

CAUTIO

		CATION	3	PC01
	ORAGE YARD	D 1 CERTIFIC	DATE: 05/12/2023	SHEET 1 OF 1
<u>END</u>	IBERLINE ST	UALITY PONI	5 SCALE: HORIZONTAL	U 1"=20" VETTICAL: 1"=5'
Z FENCE POST STORM SEWER MANHOLE EXIST MAJ CONT EXIST MIN CONT STSTUNDERGROUND STORM SEWER LINE(S)	NIL	WATER Q	PROJECT NO. 43-09	DESIGNED BY: TAU DRAWN BY: TAU CHECKED BY: DLIN
CHAIN-LINK FENCE CHAIN-LINK FENCE BARBED WIRE FENCE DECIDUOUS TREE CONIFEROUS TREE EMERGENCY OVERFLOW/SPILLWAY	212 N WAHSATCH AVE STE 305	COLORADO SPRINGS, CO 80903 PHONE: 719,955,5485		
D CERTIFICATION BAY INLETS				CIVIL CONSULTANTS, INC.
ENED INVERT IN (SW) = 6530.96', SURVEY INVERT IN (SW) = 6531.03' ET STRUCTURE ENED INVERT IN = 6530.51', SURVEY INVERT IN = 6530.34' INVED 42" INVERT OUT = 6529.93', SURVEY 42" INVERT OUT = 6529.96' INED TOP CONC. GRATE = 6534.51', SURVEY TOP CONC. GRATE = 6534.74' WAY (EDB)	37160	FOR AND ON BEHALF OF	M&S CIVIL CONSULTANTS, INC.	
NED SPILLWAY ELEVATION = 6538.5', SURVEY SPILLWAY ELEVATION = 6538.5' ME NED VOLUME = 2.721 ACRE FEET © 6536.31' (100 YR SURFACE) IDED VOLUME = 2.785 ACRE FEET © 6536.38' (100 YR SURFACE)	IRGIL A. SANCHEZ, COLORADO P.E. NO.			
	APRVD. BY: DATE:			ANS WILL NOT BE RESPONSIBLE, OR LUABLE FOR, S OF THESE PLANS. ALL CHANGES TO THE PLANS APPROVED BY THE PREPARER OF THESE PLANS.
1" = 20' $10 \ 20 \ 40$ Scole in Feet	REVISIONS: NO. DATE: BY: DESCRIPTION:			DI THE ENGINEER PREPARING THESE PL INAUTHORIZED CHARGES TO OR USE UNAUTHORIZED CHARGES TO OR USE MUST BE IN WRITING AND MUST BE.

		Dete	ention Basin C	Outlet Struct	ure Design				
Project:	Timberline Storage	9	UD-Detention, Ve	rsion 3.07 (Februar	ry 2017)				
Basin ID:	FSD Pond 1								
ZONE 2 ZONE 2 ZONE 1				(han //h)	7	Outlat Trans			
			7000 1 (10/00)	Stage (ft)	Zone volume (ac-ft)	Outlet Type	1		
T Market			Zone 1 (WQCV)	3.29	0.668	Orifice Plate			
ZONE 1 AND 2	ORIFICE	н I	Zone Z (EURV)	4.41	0.737	Orifice Plate			
POOL Example Zone	Configuration (Re	etention Pond)	20ne 3 (100-year)	6.84	1.928	weir&Pipe (Restrict)	l		
Iser Input: Orifice at Underdrain Outlet (tynically u	ed to drain WOCV ir	, a Filtration BMP)			3.333	Calculate	ad Parameters for Lin	derdrain	
Underdrain Orifice Invert Depth =	N/A	ft (distance below th	ne filtration media sur	face)	Unde	rdrain Orifice Area =	N/A	ft ²	
Underdrain Orifice Diameter =	N/A	inches			Underdra	in Orifice Centroid =	N/A	feet	
		-						-	
User Input: Orifice Plate with one or more orifices of	r Elliptical Slot Weir	(typically used to dra	in WQCV and/or EUR	V in a sedimentation	n BMP)	Calcu	lated Parameters for	Plate	
Invert of Lowest Orifice =	0.00	ft (relative to basin t	oottom at Stage = 0 ft)		WQO	rifice Area per Row =	N/A	ft ²	
Orifice Plate: Orifice Vertical Spacing =	19.60	inches	octom at Stage = 0 it)		E	niplical Hall-Width =	N/A N/A	feet	
Orifice Plate: Orifice Area per Row =	N/A	inches			2.00	Elliptical Slot Area =	N/A	ft ²	
		1				·		1	
User Input: Stage and Total Area of Each Orifice	Row (numbered fro	m lowest to highest)		1		1	1	1
	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)	
Stage of Orifice Centroid (ft)	0.00	1.47	2.94						
Orifice Area (sq. inches)	2.51	2.30	1.50						I
	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)	
Stage of Orifice Centroid (ft)									
Orifice Area (sq. inches)									
User Input: Vertical Orifice (Cire	cular or Rectangular)	Not Colocted	1			Calculated	Parameters for Vert	Not Selected	l
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin b	ottom at Stage = 0 ft) V	ertical Orifice Area =	N/A	N/A	ft ²
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin b	ottom at Stage = 0 ft) Verti	cal Orifice Centroid =	N/A	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches	0	,		· · · ·		
			-						
User Input: Overflow Weir (Dropbox) and G	Topo 2 Woir	Not Selected	1			Calculated	Parameters for Ove	rflow Weir	
Overflow Weir Front Edge Height, Ho =	4.41	N/A	ft (relative to basin bot	ttom at Stage = 0 ft)	Height of Gr	ate Upper Edge. H. =	4.41	Not Selected	feet
Overflow Weir Front Edge Length =	17.00	N/A	feet	,	Over Flow	Weir Slope Length =	4.00	N/A	feet
Overflow Weir Slope =	0.00	N/A	H:V (enter zero for fl	at grate)	Grate Open Area /	100-yr Orifice Area =	7.24	N/A	should be <u>></u> 4
Horiz. Length of Weir Sides =	4.00	N/A	feet		Overflow Grate Ope	en Area w/o Debris =	47.60	N/A	ft²
Overflow Grate Open Area % =	70%	N/A	%, grate open area/t	otal area	Overflow Grate O	pen Area w/ Debris =	23.80	N/A	ft ²
Debris Clogging % =	50%	N/A	%						
User Input: Outlet Pine w/ Flow Restriction Plate (Ci	rcular Orifice Restric	tor Plate or Rectan	ular Orifice)		(alculated Parameter	rs for Outlet Pine w/	Flow Restriction Plat	P
	Zone 3 Restrictor	Not Selected	,ulai erinee,				Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.25	N/A	ft (distance below basi	n bottom at Stage = 0	ft)	Outlet Orifice Area =	6.58	N/A	ft ²
Outlet Pipe Diameter =	42.00	N/A	inches		Out	let Orifice Centroid =	1.28	N/A	feet
Restrictor Plate Height Above Pipe Invert =	27.15	J	inches	Half-	Central Angle of Rest	rictor Plate on Pipe =	1.87	N/A	radians
licor input: Emergency Calibury (Daster	ular or Transsid-N					Calcula	tod Daramators f C	nillway	
Spillway Invert Stage	6.85	ft (relative to basin h	nottom at Stage = 0 ft)		Snillway	Design Flow Denth=	0.65	feet	
Spillway Crest Length =	72.50	feet			Stage a	it Top of Freeboard =	8.17	feet	
Spillway End Slopes =	10.00	H:V			Basin Area a	It Top of Freeboard =	0.98	acres	
Freeboard above Max Water Surface =	0.67	feet							
Devided Understand Devide									
Routed Hydrograph Results	WOCV	FLIRV/	2 Year	5 Year	10 Vear	25 Vear	50 Vear	100 Vear	500 Vear
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	0.00
Calculated Runoff Volume (acre-ft) =	0.668	1.406	1.016	1.450	2.269	4.027	5.340	7.102	0.000
OPTIONAL Override Runoff Volume (acre-ft) =	0.659	1.406	1.016	1 451	2 270	4 030	5 226	7 109	#NI/A
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.008	0.00	0.01	0.02	0.16	0.53	0.78	1.12	#IN/A 0.00
Predevelopment Peak Q (cfs) =	0.0	0.0	0.6	1.2	10.5	33.6	49.9	71.5	0.0
Peak Inflow Q (cfs) =	12.1	25.2	18.3	26.0	40.4	71.2	93.7	123.9	#N/A
Peak Outflow Q (cfs) = Ratio Peak Outflow to Predevelopment O =	0.3 N/A	0.4 N/A	0.3 N/A	0.4	15.9	47.0	65.5	/1.5	#N/A #N/A
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Grate 1	Overflow Grate 1	Outlet Plate 1	Outlet Plate 1	#N/A
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.3	1.0	1.4	1.5	#N/A
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	#N/A
Time to Drain 97% of Inflow Volume (nours) =	40 42	68	52	69	69	67	65	54 64	#N/A #N/A
Maximum Ponding Depth (ft) =	3.22	4.33	3.77	4.39	4.72	5.05	5.31	6.13	#N/A
Area at Maximum Ponding Depth (acres) =	0.55	0.71	0.66	0.71	0.73	0.75	0.77	0.83	#N/A
Maximum Volume Stored (acre-ft) =	0.623	1.350	0.965	1.392	1.623	1.875	2.066	2.721	#N/A

Maximum Volume Stored (acre-ft) = 0.623

Basin ID:	FSD Pond 1								
ZONE 3									
				Stage (ft)	Zone Volume (ac-ft)	Outlet Type			
	a I		Zone 1 (WQCV)	3.04	0.668	Orifice Plate			
	100-YEAF	1	Zone 2 (EURV)	4.17	0.737	Orifice Plate			
PERMANENT ZONE 1 AND 2 ORIFICES	/ Unition		lone 3 (100-year)	6.60	1.928	Weir&Pipe (Restrict)			
POOL Example Zone	Configuration (Ref	tention Pond)	· •		3.333	Total			
User Input: Orifice at Underdrain Outlet (typically u	sed to drain WQCV ii	n a Filtration BMP)			·,	Calculate	d Parameters for Un	derdrain	
Underdrain Orifice Invert Depth =	N/A	ft (distance below th	e filtration media sur	rface)	Unde	rdrain Orifice Area =	N/A	ft ²	
Underdrain Orifice Diameter =	N/A	inches			Underdra	in Orifice Centroid =	N/A	feet	
User Input: Orifice Plate with one or more orifices	or Elliptical Slot Weir	(typically used to dr	ain WQCV and/or EU	JRV in a sedimentati	on BMP)	Calcu	ated Parameters for	Plate	
Invert of Lowest Orifice =	0.00	ft (relative to basin b	oottom at Stage = 0 ft	:)	WQ Or	ifice Area per Row =	N/A	ft ²	
Depth at top of Zone using Orifice Plate =	4.32	ft (relative to basin b	oottom at Stage = 0 ft	:)	El	liptical Half-Width =	N/A	feet	
Orifice Plate: Orifice Vertical Spacing =	N/A	inches			Ellip	tical Slot Centroid =	N/A	feet	
Orifice Plate: Orifice Area per Row =	N/A	inches				Elliptical Slot Area =	N/A	ft ²	
User Input: Stage and Total Area of Each Orifice	Row (numbered fror	n lowest to highest)	1						
	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)	
Stage of Orifice Centroid (ft)	0.00	1.47	2.87						
Orifice Area (sq. inches)	2.54	2.32	1.54						
									1
	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)	
Stage of Orifice Centroid (ft)									
Orifice Area (sq. inches)									
• • • •									1
User Input: Vertical Orifice (Cire	ular or Rectangular)					Calculated	Parameters for Vert	ical Orifice	
	Not Selected	Not Selected	l				Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin b	oottom at Stage = 0 f	t) V	ertical Orifice Area =	N/A	N/A	ft ²
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin b	ottom at Stage = 0 f	t) Vertic	al Orifice Centroid =	N/A	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches						
User Input: Overflow Weir (Dropbox) and G	rate (Flat or Sloped)					Calculated	Parameters for Over	rflow Weir	
User Input: Overflow Weir (Dropbox) and C	rate (Flat or Sloped) Zone 3 Weir	Not Selected				Calculated	Parameters for Over Zone 3 Weir	rflow Weir Not Selected	
User Input: Overflow Weir (Dropbox) and G	rate (Flat or Sloped) Zone 3 Weir 4.32	Not Selected	ft (relative to basin bot	ttom at Stage = 0 ft)	Height of Gra	Calculated ite Upper Edge, H _t =	Parameters for Over Zone 3 Weir 4.32	rflow Weir Not Selected N/A	feet
User Input: Overflow Weir (Dropbox) and G Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length =	irate (Flat or Sloped) Zone 3 Weir 4.32 	Not Selected N/A N/A	ft (relative to basin bot	ttom at Stage = 0 ft)	Height of Gra Over Flow	Calculated ate Upper Edge, H _t = Weir Slope Length =	Parameters for Over Zone 3 Weir 4.32 4.00	rflow Weir Not Selected N/A N/A	feet feet
User Input: Overflow Weir (Dropbox) and O Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope =	rate (Flat or Sloped) Zone 3 Weir 4.32 17.00 0.00	Not Selected N/A N/A N/A	ft (relative to basin bot feet H:V (enter zero for fla	ttom at Stage = 0 ft) at grate)	Height of Gra Over Flow Grate Open Area / 2	Calculated ate Upper Edge, H _t = Weir Slope Length = .00-yr Orifice Area =	Parameters for Over Zone 3 Weir 4.32 4.00 7.24	rflow Weir Not Selected N/A N/A N/A	feet feet should be \geq 4
User Input: Overflow Weir (Dropbox) and C Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sldes =	rate (Flat or Sloped) Zone 3 Weir 4.32 17.00 0.00 4.00	Not Selected N/A N/A N/A N/A N/A	ft (relative to basin bot feet H:V (enter zero for fla feet	ttom at Stage = 0 ft) at grate)	Height of Gra Over Flow Grate Open Area / 3 Overflow Grate Ope	Calculated ate Upper Edge, H _t = Weir Slope Length = L00-yr Orifice Area = n Area w/o Debris =	Parameters for Over Zone 3 Weir 4.32 4.00 7.24 47.60	rflow Weir Not Selected N/A N/A N/A N/A	feet feet should be ≥ 4 ft^2
User Input: Overflow Weir (Dropbox) and G Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sides = Overflow Grate Open Area % =	rate (Flat or Sloped) Zone 3 Weir 4.32 17.00 0.00 4.00 70%	Not Selected N/A N/A N/A N/A N/A N/A	ft (relative to basin bol feet H:V (enter zero for fla feet %, grate open area/t	ttom at Stage = 0 ft) at grate) otal area	Height of Gr. Over Flow Grate Open Area / : Overflow Grate Ope Overflow Grate Op	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area = n Area w/o Debris = en Area w/ Debris =	Parameters for Over Zone 3 Weir 4.32 4.00 7.24 47.60 23.80	rflow Weir Not Selected N/A N/A N/A N/A N/A	feet feet should be ≥ 4 ft ² ft ²
User Input: Overflow Weir (Dropbox) and G Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Sides Overflow Grate Open Area % = Debris Clogging % =	rate (Flat or Sloped) Zone 3 Weir 4.32 17.00 0.00 4.00 70% 50%	Not Selected N/A N/A N/A N/A N/A N/A	ft (relative to basin bor feet H:V (enter zero for fla feet %, grate open area/t %	ttom at Stage = 0 ft) at grate) otal area	Height of Gr. Over Flow Grate Open Area / : Overflow Grate Ope Overflow Grate Op	Calculated ate Upper Edge, H _t = Weir Slope Length = 100-yr Orifice Area = n Area w/o Debris = en Area w/ Debris =	Parameters for Over Zone 3 Weir 4.32 4.00 7.24 47.60 23.80	flow Weir N/A N/A N/A N/A N/A N/A	feet feet should be ≥ 4 ft^2
User Input: Overflow Weir (Dropbox) and C Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Slope = Horiz. Length of Weir Slodes = Overflow Grate Open Area % = Debris Clogging % =	rate (Flat or Sloped) Zone 3 Weir 4.32 17.00 0.00 4.00 70% 50%	Not Selected N/A N/A N/A N/A N/A	ft (relative to basin bor feet H:V (enter zero for fla feet %, grate open area/tr %	ttom at Stage = 0 ft) at grate) otal area	Height of Gr. Over Flow Grate Open Area / : Overflow Grate Ope Overflow Grate Op	Calculated ate Upper Edge, H, = Weir Slope Length = L00-yr Orifice Area = n Area w/o Debris = en Area w/ Debris =	Parameters for Ove. Zone 3 Weir 4.32 4.00 7.24 47.60 23.80	rflow Weir N/A N/A N/A N/A N/A N/A	feet feet should be \geq 4 ft ² ft ²
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User Input: Overflow Weir (Dropbox) and C Overflow Weir Front Edge Height, Ho = Overflow Weir Front Edge Length = Overflow Weir Stope = Horiz. Length of Weir Sides = Overflow Grate Open Area % = Debris Clogging % = User Input: Outlet Pipe w/ Flow Restriction Plate (C Depth to Invert of Outlet Pipe = Outlet Pipe Diameter = Restrictor Plate Height Above Pipe Invert = Restrictor Plate Height Above Pipe Invert = Spillway (Crest Length = Spillway Crest Length = Calculated Runoff Volume (acre-ft) = Predevelopment Unit Peak Flow, q (cfs/acre) = Peak Uniflow Q (cfs) = Ratio Peak Outflow to Predevelopment Q = Structure Controlling Flow = Max Velocity through Grate 1 (fps) = Max Velocity through Grate 2 (fps) = Time to Drain 97% of Inflow Volume (barret) =	irate (Flat or Sloped) Zone 3 Weir 4.32 17.00 0.00 4.00 70% 50% 3 3 3 3 3 3 3 3 3 3 3 3 3	Not Selected N/A N/A N/A N/A N/A N/A N/A N/A ctor Plate, or Rectan Not Selected N/A N/A ft (relative to basin b feet H:V feet 1.07 1.406 0.0 25.2 0.4 N/A Plate N/A	ft (relative to basin bo feet H:V (enter zero for fl feet %, grate open area/t % gular Orifice) ft (distance below basi inches i	ttom at Stage = 0 ft) at grate) otal area in bottom at Stage = 0 Half-C 1.50 1.450 1.450 1.451 0.02 1.2 2.6.0 0.4 0.3 Plate N/A N/A N/A	Height of Gr Over Flow Grate Open Area / Overflow Grate Ope Overflow Grate Ope Overflow Grate Ope Overflow Grate Ope Overflow Grate of Basin Area at Basin Area at 2.269 2.270 0.16 10.5 40.4 13.5 1.3 Overflow Grate 1 0.3 N/A 73	Calculated ate Upper Edge, H, = Weir Slope Length = 100-yr Orifice Area = in Area w/o Debris = en Area w/o Debris = alculated Parameter Dutlet Orifice Area = et Orifice Centroid = ictor Plate on Pipe = Calcula Design Flow Depth= Top of Freeboard = Cop of Freeboard = 25 Year 2.00 4.027 4.027 4.030 0.53 33.6 71.2 45.3 1.3 Overflow Grate 1 1.0 N/A 67	Parameters for Ove Zone 3 Weir 4.32 4.00 7.24 47.60 23.80 s for Outlet Pipe w/ / Zone 3 Restrictor 6.58 1.28 1.87 ted Parameters for S 0.65 9.40 1.14 5.0 Year 2.25 5.340 5.336 0.78 49.9 93.7 66.1 1.3 Outlet Plate 1 1.4 N/A N/A	Image: fillow Weir N/A 100 Year 2.52 7.102 7.108 1.12 7.1.5 1.23.9 71.8 1.0 Outlet Plate 1 1.5 N/A	feet feet should be \geq 4 ft ² ft ² feet radians $\frac{500 \text{ Year}}{0.00}$ 0.000 0.000 $\frac{100}{0}$ $\frac{100}{100$

61 3.53

0.66

75 4.15

0.71 1.391

4.60

0.74 1.717

4.95

0.76

5.17

0.78

5.96

0.83

#N/A

#N/A #N/A

Project: Timberline Storage (Amended)

AS-BUILT

Time to Drain 99% of Inflow Volume (hours) =

Maximum Ponding Depth (ft) = Area at Maximum Ponding Depth (acres) = Maximum Volume Stored (acre-ft) =

2.97

0.55

4.09

0.71

UD-Detention, Version 3.07 (February 2017)

Detention Basin Outlet Structure Design

