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## **Grandview Reserve Phase 3 Preliminary Drainage Report**

March 2024

HR Green Project No: 201662.203

**Prepared For:**

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### Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

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Ken Huhn, P.E.

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Date

State of Colorado No. 54022

For and on behalf of HR Green Development, LLC

### Owner/Developer's Statement:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

By: \_\_\_\_\_

Authorized Signature

\_\_\_\_\_ Date

Address: D.R. Horton  
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Englewood, CO

### El Paso County Statement

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development code, as amended.

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Joshua Palmer, P.E.

Date

County Engineer/ECM Administrator

Conditions:



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## I. General Purpose, Location and Description

### a. Purpose

The purpose of this Preliminary Drainage Report (PDR) for the Grandview Reserve Subdivision Phase 3 is to describe the onsite and offsite drainage patterns, size drainage infrastructure to safely capture and convey developed runoff to water quality and detention facilities, and to safely route detained stormwater to adequate outfalls.

### b. Location

The Grandview Reserve Phase 3 site is located in unincorporated El Paso County, Colorado. The Phase 3 location (referred to as the site herein) is located west of Grandview Reserve Filings 1-4 and MST2, south of Grandview Reserve Phase 2, and east of Rex Road.

The site lies within a tract of land within Sections 21, 22, 27 and 28, Township 12 South, Range 64 West of the 6<sup>th</sup> Principal Meridian, in El Paso County, State of Colorado. A Vicinity Map is included in **Appendix A**.

The site is bound by a segment of Rex Road to be developed with this project to the east and undeveloped land that has historically been used as ranching lands. The northwest of the site is bound by Phase 2 of the subdivision. The west of the site is bound by Grandview Reserve Filings 1-4 and MST2. A vicinity map is presented in Appendix A.

The Gieck Ranch Tributary #2 "MST2" (Channel B) is a part of the Gieck Ranch Drainage Basin tributary to Black Squirrel Creek. The channel draining through the site is an ongoing project with associated CLOMR Report and the PCD File No. is CDR228 with El Paso County. The Grandview Reserve improvements will follow any requirements of that report. There is another floodplain channel to the north of Rex Road that will not be disturbed by this phase of development and studies as a future project.

The existing surrounding platted developments include the Grandview Reserve Phase 1 Filings 1-4 and Grandview Reserve Phase 2.

### c. Description of Property

The site is approximately 102.82 acres of proposed residential development with associated right of way, open space tracts, public improvements, and stormwater treatment infrastructure.

The existing groundcover and topography of the site is native grasses/weeds and exposed soil on gently rolling hillside with slopes ranging from 2% to 4%.

Per a NRCS soil survey, the site is made up of Type A Columbine gravelly sandy loam. The NRCS soil survey is presented in **Appendix A**.

There is one major drainageway through the site. The Gieck Ranch Tributary #2 (MST2 in the MDDP) traverses through the site and divides it in two. The drainageway generally flows from the northwest to the southeast towards Highway 24, before crossing through existing drainage infrastructure. The CLOMR report by HR Green for MST2 is ongoing and pending approval for this channel. Refer to the CLOMR report included in **Appendix E** for more specific design information regarding the MST2 channel. Gieck Ranch Tributary #3 (future Channel C) traverses the site along its northeastern boundary and forms the northeast boundary for Phase 3 along Rex Road. The initial analysis of this drainage way has been performed by HR Green in

conjunction with Phase 2. The analysis delineated the 100-yr floodplain and ensures the construction of Rex Road will not impact the floodplain. No fill will be placed in this channel in this phase of development. A CLOMR report is not required by the County at this time.

There are no known irrigation facilities in the area.

There are no known existing utilities or other encumbrances on site.

**d. Floodplain Statement**

Based on FEMA Firm map 08041C0552G & 08041C0556G (eff. 12/7/2018), the site contains flood Zone A through the site which is part of the Gieck Ranch Tributary #2. See FEMA Firm Maps in **Appendix A**. This floodplain is being studied and revised in the Gieck Ranch Tributary # 2 CLOMR report. A copy of the current revised floodplain map is also provided in **Appendix A**. There is a Zone A floodplain northeast of the site which will not be altered with this project’s improvements.

## II. Drainage Design Criteria

**a. Drainage Criteria**

Hydrologic data and calculations were performed using Drainage Criteria Manual Volume 1 of El Paso County (EPCDCM), with County adopted Chapter 6 and Section 3.2.1 of Chapter 13 of the City of Colorado Springs Drainage Criteria Manual (CCSDCM), May 2014 revised January 2021.

Onsite drainage improvements are designed for the 5-year storm (minor event) and 100-year storm (major event) using rainfall values from the NOAA Atlas 14 Point Precipitation Frequency Data Server. Runoff was calculated per CCSDCM Section 6.3.0 - Rational Method. Private, full spectrum pond design was completed using the latest version of Mile High Flood District’s (MHFD) UD-Detention per CCSDCM Section 13.3.2.1 – Private, full spectrum Detention. Detention pond allowable release rate will be limited to less than historic rates.

Rainfall Depths per NOAA Atlas 14		
Return Period (yr)	5	100
1-hr Rainfall Depth (in)	1.21	2.49

Storm sewer and inlet sizing shown is preliminary at this stage of the project. Calculations for the storm sewer system on site will be provided with the Final Drainage Report (FDR) for the project. The sizing methodology that will be used is per the methods described in EPCDCM Section III Chapter 7 – Street Drainage and Storm Water Inlets. Storm sewer sizing was performed per the methods described in EPCDCM Section III Chapter 8 – Storm Drains and Appurtenances.

This preliminary drainage report follows any recommendations and is in conformance with the previously approved MDDP for the site prepared by HR Green, “*Grandview Reserve Master Development Drainage Plan*”, HR Green, November 2020 (MDDP).

### III. Drainage Basins and Subbasins

#### a. Major Basin Description

The site is located within the Gieck Ranch Drainage Basin. The site's drainage characteristics were previously studied in the following reports:

1. "Gieck Ranch Drainage Basin Planning Study" prepared by Drexel, Barrel & Co, February 2010.
2. "Grandview Reserve Master Development Drainage Plan" prepared by HR Green, August 2021.
3. "Grandview Reserve Filing No. 1 Preliminary Drainage Report" prepared by Galloway & Company, Inc., September 2022.
4. "Grandview Reserve CLOMR REPORT" prepared by HR Green, March 2023

Gieck Ranch Drainage Basin is a 22.05 square mile watershed located in El Paso County, Colorado. Gieck Ranch Drainage Basin is tributary to Black Squirrel Creek which drains to the Arkansas River. The majority of the basin is undeveloped and rolling range land of 2% - 4% slopes.

The Grandview Reserve MDDP divided the site into 8 major drainage basins (A-H), where each basin is tributary to a full spectrum detention pond facility. The Grandview Reserve Phase 3 improvements are located in subbasins B3 and D1 of the MDDP.

There are no known existing irrigation facilities or other obstructions that could influence or will be influenced by local drainage characteristics. Proposed local drainage characteristics will continue to follow historic patterns. The offsite flows entering Phase 3 from Phase 2 have been accounted for in the drainage calculations.

#### b. Existing Subbasin Description

The Grandview Reserve Phase 3 site drains from the northwest to the southeast slopes ranging from 2% - 4%. The majority of the site has historically drained into the Gieck Ranch Tributary #2 (existing MST2), with portions draining to East Fork Tributary (EFT).

The existing subbasins for the Grandview Reserve Phase 3 site were studied in the approved MDDP for Grandview Reserve. This site is located within subbasins B3 and D1 of this report and are described as follows.

"Subbasin B3 is located between MS and EF and to the northeast of east of basin B2. The existing MST2 tributary runs through the basin. The site drains towards the southeast and towards Detention Pond B. Current planning documents call for high, medium-high, and medium density dwelling units along with a pocket park. The basin is 118.90 acres, with a composite impervious value of 49.42% and runoff rates for the 5 and 100 year of 92.76 cfs and 295.27 cfs respectively."

"Subbasin D1 is located between MS and MST2 to the east of Basin B3 and adjacent to the MST2 channel. The basin drains towards the southeast and towards drainage basin D2. Current planning documents call for medium density dwelling units along with a pocket park. The basin is 24.33 acres, with a composite impervious value of 53.89% and runoff rates for the 5 and 100 year of 24.15 cfs and 70.07 cfs respectively."

A copy of the approved MDDP has been included in **Appendix E** of this report. The proposed drainage conditions for this development will follow historic drainage patterns as described in the MDDP.

## c. Proposed Subbasin Description

### Description of Proposed Project

The proposed drainage conditions for the site generally follow historic drainage patterns. The site drains from the northwest to the southeast at slopes between 0.6% - 4%, into proposed public storm sewer systems via sheet flow/curb & gutter/channel flow which drain to proposed private extended detention basins for treatment and flood attenuation.

The northeastern corner of the site will drain to and be treated by "Pond R", which will outfall in the EFT, henceforth being referred to as "Channel C". Although the tributary area of Pond R would historically drain to MST2, it will now outfall in Channel C. The increase in discharge volume to Channel C will be negligible as a significant tributary area of Phase 2 Pond A and Pond B historically drained to Channel C and will now outfall into MST2. A comparison of the drainage areas is overlaid on the existing drainage map. Any increase in volume entering the channel will be mitigated with slope stabilization measures.

The west and south portions of the site will drain to and be treated by two separate detention ponds, "Pond F," and "Pond C." Both Pond F and Pond C will outfall into the rerouted channel MST2, henceforth being referred to as "Channel B." Drainage from both of these ponds has been accounted for in the channel re-alignment design and is detailed in the CLOMR report.

The 25.0 acre future school site will remain undeveloped and will not be accounted for in this report.

There are minor offsite flows from Phase 2 which are accounted for.

### Subbasins Tributary to and Treated by Pond R

Basin R-A1 is 2.16 acres of landscaped area and 50' lots area. Stormwater ( $Q_5 = 1.7$  cfs  $Q_{100} = 6.0$  cfs) is conveyed via grass swales in Tracts E and D to a private type-C inlet at DP R-1, and ultimately draining to Pond R via the proposed public storm sewer network.

Basin R-A2 is 1.58 acres of El Paso County (EPC) Local right-of-way (ROW), landscaped area and 50' lots area. Stormwater ( $Q_5 = 2.8$  cfs  $Q_{100} = 6.0$  cfs) is conveyed via curb and gutter to a type R inlet at DP R-2 and ultimately drains to Pond R via the proposed public storm sewer network. Flowby from DP R-2 ( $Q_5 = 0.0$  cfs  $Q_{100} = 1.0$  cfs) will be captured by future development.

Basin R-B is 4.19 acres of EPC Local ROW area, landscaped area, and 35' lots area. Stormwater ( $Q_5 = 6.1$  cfs  $Q_{100} = 14.7$  cfs) is conveyed via curb and gutter to a type R inlet at DP R-3 and ultimately drains to Pond R via the proposed public storm sewer network. Flowby from DP R-3 is captured at DP R-6 sump inlet.

Basin R-C is 2.90 acres of EPC Local ROW area, and 35' lots area. Stormwater ( $Q_5 = 5.4$  cfs  $Q_{100} = 12.0$  cfs) is conveyed via curb and gutter to a type R sump inlet at DP R-4 and ultimately drains to Pond R via the proposed public storm sewer network. In the event of inlet clogging, stormwater will overtop the curb and flow via an overflow swale to Rex Road.

Basin R-D is 5.53 acres of EPC Minor Arterial ROW area, landscaped area, and 35' lots area. Stormwater ( $Q_5 = 7.2$  cfs  $Q_{100} = 17.7$  cfs) is conveyed via curb and gutter to a type R inlet at DP R-5. Flowby stormwater from Grandview Phase 2 Basin G-B, H-B is also captured at DP R-5 and ultimately drains to Pond R via the proposed public storm sewer network. Flowby from DP R-5 is captured at DP R-6 sump inlet.

Basin R-E is 1.81 acres of EPC Local ROW area, 35' lots area. Stormwater ( $Q_5 = 3.3$  cfs  $Q_{100} = 7.4$  cfs) is conveyed via curb and gutter to a type R sump inlet at DP R-6 and ultimately drains to Pond R via the proposed public storm sewer network.

Basin R-F is 0.81 acres of EPC Local ROW area and landscaped area. Stormwater ( $Q_5 = 1.8$  cfs  $Q_{100} = 4.0$  cfs) is conveyed via curb and gutter to a type R inlet at DP R-7 and ultimately drains to Pond R via the proposed public storm sewer network.

Basin R-G is 1.80 acres of landscaped detention facility area. Stormwater ( $Q_5 = 0.6$  cfs  $Q_{100} = 4.3$  cfs) is conveyed via sheet flow in Tract C and flows directly into Pond R at DP R-8.

#### **Subbasins Tributary to and Treated by Pond F**

Basin F-A is 4.58 acres of EPC Local ROW area, landscaped area, and 35' lots area. Stormwater ( $Q_5 = 7.0$  cfs  $Q_{100} = 15.8$  cfs) is conveyed via curb and gutter to a type R sump inlet at DP F-1 and ultimately drains to Pond F via the proposed public storm sewer network.

Basin F-B is 3.76 acres of EPC Local ROW area and 40' lots area. Stormwater ( $Q_5 = 7.0$  cfs  $Q_{100} = 15.2$  cfs) is conveyed via curb and gutter to a type R sump inlet at DP F-2 and ultimately drains to Pond F via the proposed public storm sewer network.

Basin F-C1 is 2.08 acres of EPC Local ROW area, landscaped area, and 40' lots area. Stormwater ( $Q_5 = 4.0$  cfs  $Q_{100} = 8.9$  cfs) is conveyed via curb and gutter to a type R inlet at DP F-3 and ultimately drains to Pond F via the proposed public storm sewer network. Flowby from DP F-3 will flow to DP F-5 via curb and gutter.

Basin F-C2 is 3.64 acres of EPC Local ROW area and 40' lots area. Stormwater ( $Q_5 = 6.9$  cfs  $Q_{100} = 14.6$  cfs) is conveyed via curb and gutter to a type R inlet at DP F-5 and ultimately drains to Pond F via the proposed public storm sewer network. Flowby from DP F-5 is captured in DP F-7 sump inlet.

Basin F-C3 is 1.33 acres of EPC Local ROW, landscaped area, and 40' lots area. Stormwater ( $Q_5 = 2.9$  cfs  $Q_{100} = 6.0$  cfs) is conveyed via curb and gutter to a type R inlet at DP F-4 and ultimately drains to Pond F via the proposed public storm sewer network. Flowby from DP F-4 is routed to DP F-5.

Basin F-D is 3.13 acres of EPC Local ROW, landscaped area, and 50' lots area. Stormwater ( $Q_5 = 4.2$  cfs  $Q_{100} = 10.4$  cfs) is conveyed via curb and gutter to a type R inlet at DP F-6 and ultimately drains to Pond F via the proposed public storm sewer network. Flowby from DP F-6 is captured in DP F-7 sump inlet.

Basin F-E is 4.42 acres of EPC Local ROW, landscaped area, 35' lots area, 40' lots area, and 50' lots area. Stormwater ( $Q_5 = 7.7$  cfs  $Q_{100} = 17.4$  cfs) is conveyed via curb and gutter to a type R sump inlet at DP F-7 and ultimately drains to Pond F via the proposed public storm sewer network. In the event of inlet clogging, stormwater from DP F-7 is permitted to overtop the crown of the road and be captured at DP R-8 sump inlet.

Basin F-F is 0.94 acres of EPC Local ROW, landscaped area, and 50' lots area. Stormwater ( $Q_5 = 1.9$  cfs  $Q_{100} = 4.1$  cfs) is conveyed via curb and gutter to a type R sump inlet at DP F-8 and ultimately drains to Pond F via the proposed public storm sewer network. In the event of inlet clogging, stormwater at DP F-8 will overtop the curb and follow an overflow swale to Pond F.

Basin F-G is 0.80 acres of landscaped area and 50' lots area. Stormwater ( $Q_5 = 0.9$  cfs  $Q_{100} = 2.6$  cfs) is conveyed via sheet flow and drains directly to Pond F at DP F-9.

Basin F-H is 1.95 acres of landscaped detention facility area. Stormwater ( $Q_5 = 0.6$  cfs  $Q_{100} = 4.5$  cfs) is conveyed via sheet flow and drains directly to Pond F at DP F-10.



Basin F-I is 1.03 acres of undeveloped area. Stormwater ( $Q_5 = 0.4$  cfs  $Q_{100} = 2.8$  cfs) follows historic drainage patterns and sheet flows offsite at DP F-11.

### **Subbasins Tributary to and Treated by Pond C**

Basin C-A is 3.43 acres of EPC Local ROW area, landscaped area, and 50' lots area. Stormwater ( $Q_5 = 3.7$  cfs  $Q_{100} = 10.5$  cfs) is conveyed via curb and gutter to a type R inlet at DP C-1 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-1 will flow to DP C-3 via curb and gutter.

Basin C-B is 4.54 acres of EPC Local ROW area, landscaped area, 40' lots area, and 50' lots area. Stormwater ( $Q_5 = 6.1$  cfs  $Q_{100} = 15.6$  cfs) is conveyed via curb and gutter to a type R inlet at DP C-2 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-2 will flow to DP C-4 via curb and gutter.

Basin C-C is 2.30 acres of EPC Local ROW area, landscaped area, and 50' lots area. Stormwater ( $Q_5 = 3.6$  cfs  $Q_{100} = 8.4$  cfs) is conveyed via curb and gutter to a type R inlet at DP C-3 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-3 is captured by CP C-7 sump inlet.

Basin C-D is 1.88 acres of EPC Local ROW and 40' lots area. Stormwater ( $Q_5 = 3.8$  cfs  $Q_{100} = 8.1$  cfs) is conveyed via curb and gutter to a type R inlet at DP C-4 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-4 is captured by CP C-7 sump inlet.

Basin C-E is 2.30 acres of EPC Local ROW and 40' lots area. Stormwater ( $Q_5 = 4.7$  cfs  $Q_{100} = 10.2$  cfs) is conveyed via curb and gutter to a type R inlet at DP C-5 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-5 is captured by CP C-7 sump inlet.

Basin C-F is 2.85 acres of EPC Local ROW, landscaped area, and 40' lots area. Stormwater ( $Q_5 = 5.2$  cfs  $Q_{100} = 11.6$  cfs) is conveyed via curb and gutter to a type R inlet at DP C-6 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-6 is captured by CP C-7 sump inlet.

Basin C-G is 1.64 acres of EPC Local ROW, landscaped area, 40' lots area, and 50' lots area. Stormwater ( $Q_5 = 3.1$  cfs  $Q_{100} = 7.1$  cfs) is conveyed via curb and gutter to a type R sump inlet at DP C-7 and ultimately drains to Pond C via the proposed public storm sewer network. In the event of inlet clogging, stormwater from DP C-7 is permitted to overtop the crown of the road and be captured at DP C-8 sump inlet.

Basin C-H is 0.49 acres of EPC Local ROW, landscaped area, and 40' lots area. Stormwater ( $Q_5 = 1.2$  cfs  $Q_{100} = 2.6$  cfs) is conveyed via curb and gutter to a type R sump inlet at DP C-8 and ultimately drains to Pond C via the proposed public storm sewer network. In the event of inlet clogging, stormwater at DP C-8 will overtop the curb and follow an overflow swale to Pond F.

Basin C-I is 0.65 acres of landscaped area. Stormwater ( $Q_5 = 0.2$  cfs  $Q_{100} = 1.4$  cfs) is conveyed via swale flow and sheet flow at DP C-9 and drains directly into Pond C.

Basin C-J is 0.81 acres of landscaped area and 40' lots area. Stormwater ( $Q_5 = 1.0$  cfs  $Q_{100} = 2.9$  cfs) is conveyed via sheet flow at DP C-10 and drains directly into Pond C.

Offsite Basins C1 - C14 is 43.50 acres of Grandview Filing 4 subdivision. Stormwater ( $Q_5 = 50.5$  cfs  $Q_{100} = 107.4$  cfs) is conveyed via offsite storm network and trickle channel flow at DP C-11 and flows directly into Pond C.

Basin C-K is 2.77 acres of landscaped detention facility area. Stormwater ( $Q_5 = 0.8$  cfs  $Q_{100} = 6.1$  cfs) is conveyed via trickle channel flow at DP C-12 and drains directly into Pond C.

Basin C-L is 0.50 acres of undeveloped area. Stormwater ( $Q_5 = 0.2$  cfs  $Q_{100} = 1.3$  cfs) follows historic drainage patterns and sheet flows offsite at DP C-13.

## IV. Drainage Facility Design

### a. General Concept

The proposed improvements will generally follow historic drainage patterns. Inlets will be placed at low points and in the public ROW where the street capacity would be exceeded. Stormwater from the development will be routed via a proposed public storm sewer system to a full spectrum detention pond which release runoff into Channel B and Channel C. All ponds and water quality features will discharge the design storms at less than rates specified in the Grandview reserve MDDP.

### b. Water Quality & Detention

#### **Pond R (Full Spectrum Detention Basin)**

Water quality and detention for Basins RA-1 through R-G is provided in Pond R; a private, full spectrum extended detention basin within Phase 3 of Grandview Reserve. A total of 19.29 acres at 42% composite imperviousness will be detained. The pond has been sized to provide water quality treatment, and detention for up to the 100-yr storm volume to be released at or below historic rates. The WQCV is 0.298 ac-ft, the EURV is 0.897 ac-ft, and the 100-year detention volume is 1.373 ac-ft. The WQCV, EURV and 100-year storms are released in 40, 72 and 77 hours, respectively. A forebay is located at the outfall into the pond and a 6.0' trickle channel conveys flow towards the outlet structure. A 15' access and maintenance road is provided to the bottom of the pond to facilitate maintenance of the pond facilities. A 9' emergency overflow spillway is provided that conveys the developed, peak 100-yr flow rate with 1.0' of freeboard towards Channel C.

#### **Pond F (Full Spectrum Detention Basin)**

Water quality and detention for Basins F-A through F-I is provided in Pond F; a private, full spectrum extended detention basin within Phase 3 of Grandview Reserve. A total of 26.63 acres at 49% composite imperviousness will be detained. The pond has been sized to provide water quality treatment, and detention for up to the 100-yr storm volume to be released at or below historic rates. The WQCV is 0.461 ac-ft, the EURV is 1.540 ac-ft, and the 100-year detention volume is 2.353 ac-ft. The WQCV, EURV and 100-year storms are released in 40, 71 and 74 hours, respectively. A forebay is located at the outfall into the pond and a 6.0' trickle channel conveys flow towards the outlet structure. A 15' access and maintenance road is provided to the bottom of the pond to facilitate maintenance of the pond facilities. A 30.0' emergency overflow spillway is provided that conveys the developed, peak 100-yr flow rate with 1.0' of freeboard towards Channel B.

#### **Pond C (Full Spectrum Detention Basin)**

Water quality and detention for offsite Basins C1- C14, and onsite Basins C-A through C-K is provided in Pond F; a private, full spectrum extended detention basin majority within Phase 3 of Grandview Reserve, and partially within Filing 4 of Grandview Reserve. A total of 67.16 acres at 53% composite imperviousness will be detained. The pond has been sized to provide water quality treatment, and detention for up to the 100-yr storm volume to be released at or below historic rates. The WQCV is 1.209 ac-ft, the EURV is 4.205 ac-ft, and the 100-year detention volume is 6.247 ac-ft. The WQCV, EURV and 100-year storms are released in 40, 71 and 72 hours, respectively. A forebay is located at the outfall into the pond and a 6.0' trickle channel

conveys flow towards the outlet structure. A 15' access and maintenance road is provided to the bottom of the pond to facilitate maintenance of the pond facilities. A 75.0' emergency overflow spillway is provided that conveys the developed, peak 100-yr flow rate with 1.0' of freeboard towards Channel B.

### c. Channel Improvements

The Gieck Ranch Tributary #2 is proposed to be rerouted. As part of this rerouting of the channel, offsite upstream tributary flows will be captured upstream from the proposed Rex Road extension and be conveyed via culvert to the rerouted channel along the Grandview Reserve Phase 2 western boundary. An analysis has been done for the channel with both existing and future condition flows as described within the Grandview Reserve CLOMR Report, HR Green; September 2021; revised January 2022 (CLOMR). Both scenarios, throughout the channel fall within the channel stability criteria. Channel improvement construction plans have been submitted to El Paso County for review as a separate project (#CDR228).

### d. Inspection and Maintenance

After completion of construction and upon the Board of County Commissioners acceptance, it is anticipated that all drainage facilities within the public Right-of-Way are to be owned and maintained by El Paso County.

All private detention ponds are to be owned and maintained by the Grandview Reserve Metropolitan District NO. 2 (DISTRICT), once established, unless an agreement is reached stating otherwise. Maintenance access for all full spectrum detention facilities will be provided from public Right-of-Way. Maintenance access for the drainageways will be provided through the proposed tracts.

## V. Wetlands Mitigation

There is one existing wetlands on site associated with the Gieck Ranch Tributary #2. The wetlands are contained within the existing channel and classified as non-jurisdictional. The wetlands USACE determination will be provided with the Grandview Reserve CLOMR Report, HR Green; April 2022, which can be found in **Appendix E**. Wetlands maintenance will be the responsibility of the Grandview Reserve Metropolitan District No. 2.

## VI. Four Step Method to Minimize Adverse Impacts of Urbanization

Step 1 – Reducing Runoff Volumes: Low impact development (LID) practices are utilized to reduce runoff at the source. In general, stormwater discharges are routed across pervious areas prior to capture in storm sewer. This practice promotes infiltration and reduces peak runoff rates. Rear and side yard swales are also being utilized in the design to promote infiltration from individual lot impervious areas. The Impervious Reduction Factor (IRF) method will be used in the final design and calculations provided with the FDR.

Step 2 – Treat and slowly release the WQCV: This step utilizes full spectrum water quality and detention to capture the WQCV and slowly release runoff from the site. Onsite full spectrum detention pond provides water quality treatment for the site. The WQCV is released over a period of 40 hours while the EURV is released over a period of 68-72 hours.

Step 3 – Stabilize stream channels: This step establishes practices to stabilize drainageways and provide scour protection at stormwater outfalls. Erosion protection is provided at all concentrated stormwater discharge points in the form of riprap pads.

Step 4 – Consider the need for source controls: No industrial or commercial uses are proposed within this development and therefore no source controls are proposed.

## VII. Drainage and Bridge Fees

Gieck Ranch drainage basin has not been established as a fee basin within El Paso County. Therefore, no drainage basin fees are due at time of platting.

## VIII. Opinion of Probable Cost

An engineer's opinion of probable cost will be provided with the Final Drainage Report (FDR) for the site.

## IX. Hydraulic Grade Line Analysis

Hydraulic grade line analysis and final pipe sizes will be provided with the FDR for the site.

## X. Summary

The Grandview Reserve Phase 2 site lies within the Gieck Ranch Drainage Basin. Water quality and detention for the site is provided in full spectrum water quality and detention ponds. There is one major drainageway that traverses the site: Gieck Ranch Tributary #2. The water quality and detention features ponds will be maintained by the Grandview Reserve Metropolitan District No. 2 (DISTRICT). All drainage facilities were sized per the El Paso County Drainage Criteria Manuals.

The development of this project will not adversely affect adjacent or downstream properties.

## XI. Drawings

Refer to the appendices for vicinity and drainage basin maps.

## XII. References

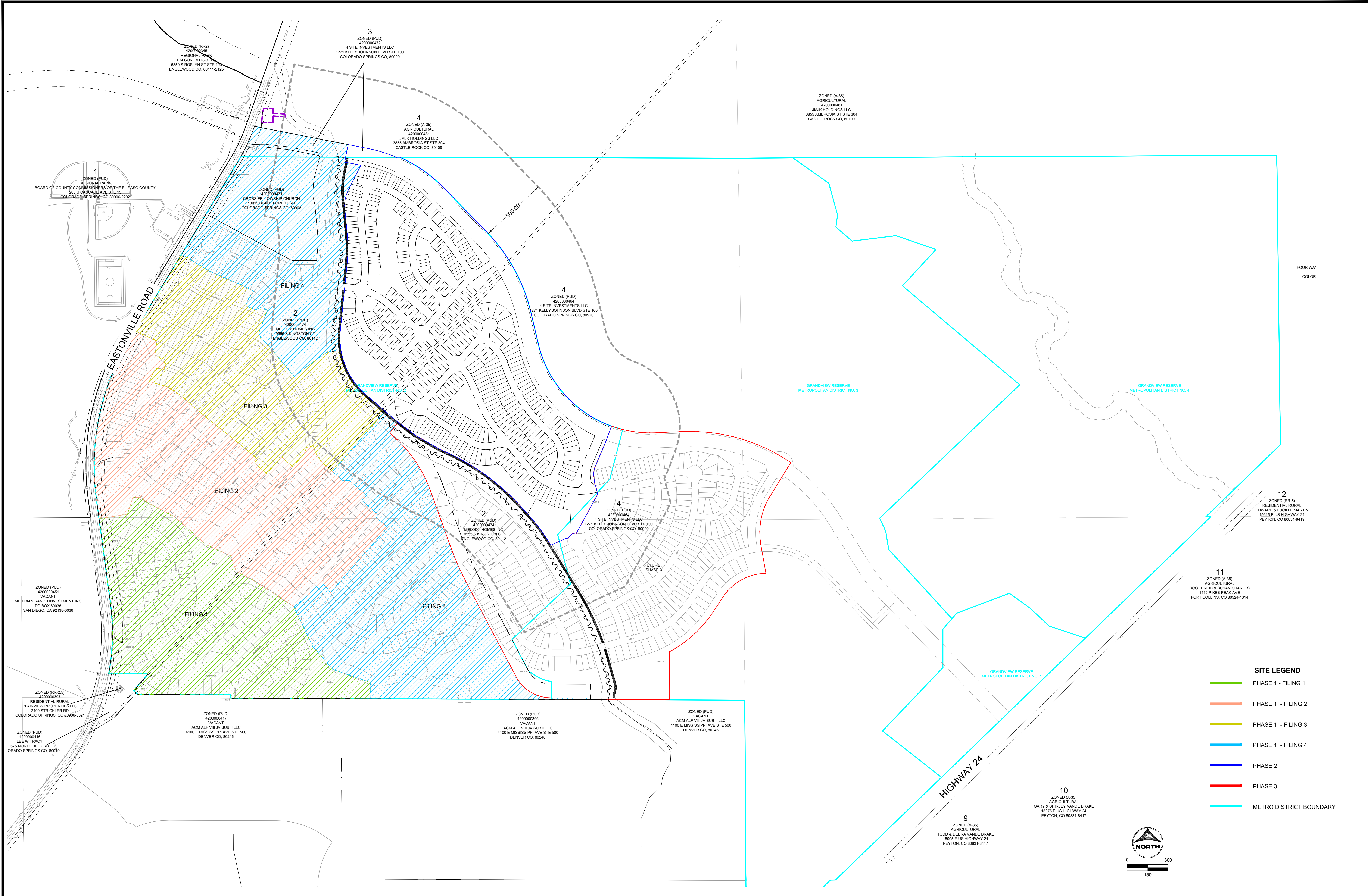
1. City of Colorado Springs – Drainage Criteria Manual, May 2014, Revised January 2021.
2. Drainage Criteria Manual of El Paso, Colorado, October 2018.
3. Urban Storm Drainage Criteria Manual, Urban Drainage Flood Control District, January 2018.
4. “Gieck Ranch Drainage Basin Planning Study” prepared by Drexel, Barrel & Co, February 2010.
5. “Grandview Reserve Master Development Drainage Plan” prepared by HR Green, August 2021.
6. “Grandview Reserve Filing No. 1 Preliminary Drainage Report” prepared by Galloway & Company, Inc., September 2022.
7. “Grandview Reserve CLOMR REPORT” prepared by HR Green, March 2023



## **APPENDIX A – VICINITY MAP, PHOTOS, SOIL MAP, FEMA MAP**

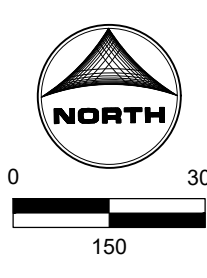
HERGER, DANIEL L, 12/15/2023 9:06 AM

HR GREEN \xrefs\p12\_15\_2023\12\_15\_2023\CAD\Drawings\CIPUD\_Phase\_2\_662.202\Exhibit\Vicinity\_Map.dwg



**SITE LEGEND**

- PHASE 1 - FILING 1
- PHASE 1 - FILING 2
- PHASE 1 - FILING 3
- PHASE 1 - FILING 4
- PHASE 2
- PHASE 3
- METRO DISTRICT BOUNDARY



DRAWN BY: DLH      JOB DATE: ----

APPROVED: KMH      JOB NUMBER: 201662.2

CAD DATE: 12/15/2023

CAD FILE: J:\2020\201662\CAD\Drawings\CIPUD\_Phase\_2\_662.202\Exhibit\Vicinity\_Map

BAR IS ONE INCH ON OFFICIAL DRAWINGS.  
0" = 11'

IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

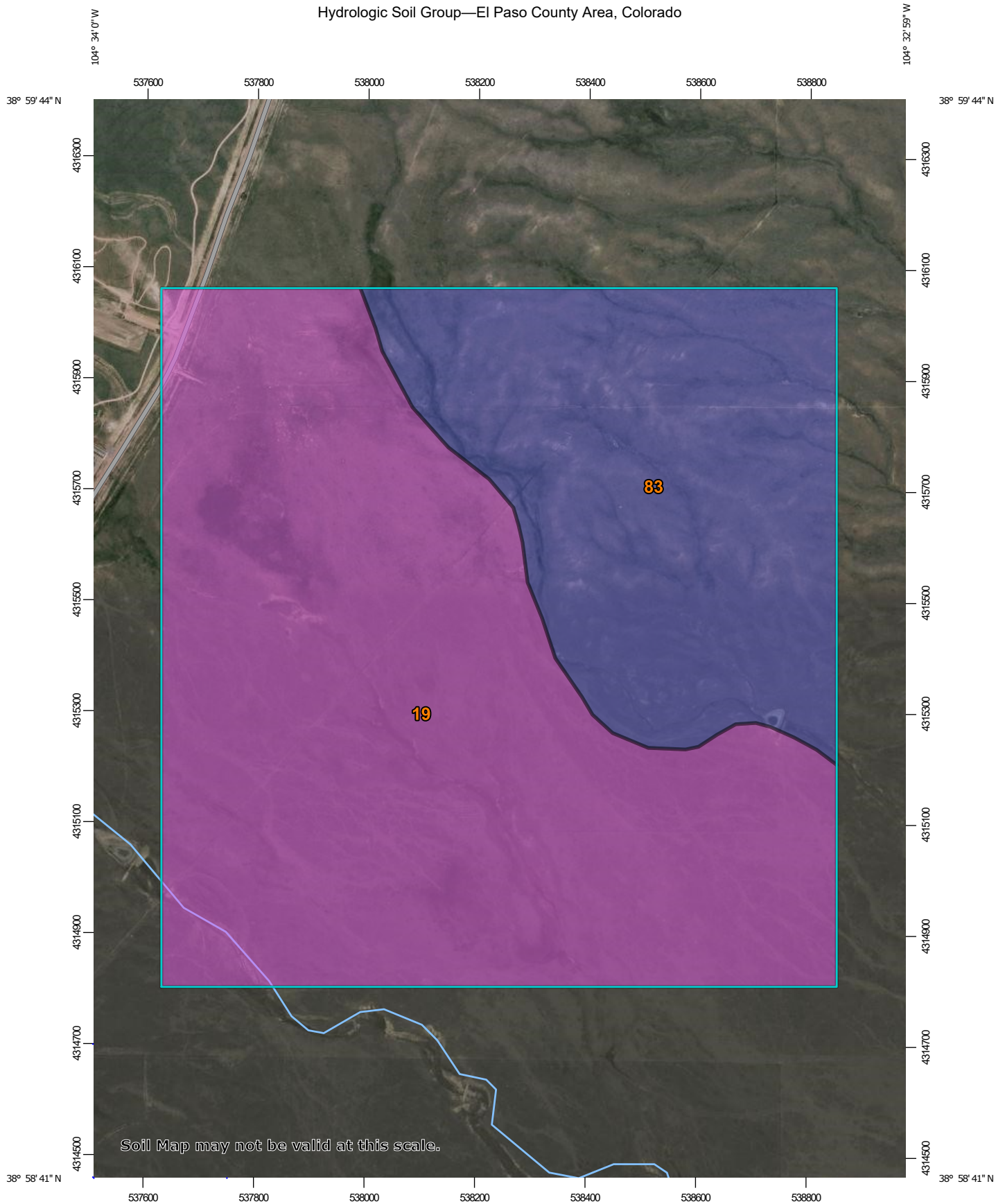
**HRGreen**      HR GREEN - COLORADO SPRINGS  
1975 RESEARCH PKWY SUITE 230  
COLORADO SPRINGS CO 80920  
PHONE: 719.300.4140  
FAX: 713.965.0044

GRANDVIEW RESERVE - PHASE 2  
D.R. HORTON  
EL PASO COUNTY, CO

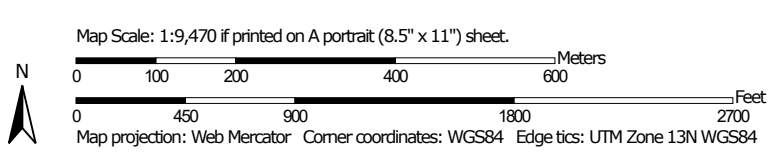
GRANDVIEW PHASE 2  
VICINITY MAP

SHEET MAP 1

































Hydrologic Soil Group—El Paso County Area, Colorado



Soil Map may not be valid at this scale.



### MAP LEGEND

- Area of Interest (AOI)**
  -  Area of Interest (AOI)
- Soils**
  - Soil Rating Polygons**
    -  A
    -  A/D
    -  B
    -  B/D
    -  C
    -  C/D
    -  D
    -  Not rated or not available
  - Soil Rating Lines**
    -  A
    -  A/D
    -  B
    -  B/D
    -  C
    -  C/D
    -  D
    -  Not rated or not available
  - Soil Rating Points**
    -  A
    -  A/D
    -  B
    -  B/D
- Water Features**
  -  Streams and Canals
- Transportation**
  -  Rails
  -  Interstate Highways
  -  US Routes
  -  Major Roads
  -  Local Roads
- Background**
  -  Aerial Photography
- Other**
  -  C
  -  C/D
  -  D
  -  Not rated or not available

### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

**Warning:** Soil Map may not be valid at this scale.  
 Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado  
 Survey Area Data: Version 20, Sep 2, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 11, 2018—Jun 12, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	254.0	66.5%
83	Stapleton sandy loam, 3 to 8 percent slopes	B	127.8	33.5%
<b>Totals for Area of Interest</b>			<b>381.8</b>	<b>100.0%</b>

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## Rating Options

*Aggregation Method:* Dominant Condition

*Component Percent Cutoff:* None Specified

*Tie-break Rule:* Higher



**FLOOD HAZARD INFORMATION**

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR DRAFT FIRM PANEL LAYOUT

<b>SPECIAL FLOOD HAZARD AREAS</b>		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
<b>OTHER AREAS OF FLOOD HAZARD</b>		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee See Notes Zone X
		Area with Flood Risk due to Levee Zone D
<b>OTHER AREAS</b>		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D
<b>GENERAL STRUCTURES</b>		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
		20.2 Cross Sections with 1% Annual Chance
		17.5 Water Surface Elevation
		8 Coastal Transect
		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
<b>OTHER FEATURES</b>		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary

**NOTES TO USERS**

For information and questions about this Flood Insurance Rate Map (FIRM), available products associated with this FIRM, including historic versions, the current map date for each FIRM panel, how to order products, or the National Flood Insurance Program (NFIP) in general, please call the FEMA Map Information eXchange at 1-877-FEMA-MAP (1-877-336-6627) or visit the FEMA Flood Map Service Center website at <https://msc.fema.gov>. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website.

Communities annexing land on adjacent FIRM panels must obtain a current copy of the adjacent panel as well as the current FIRM Index. These may be ordered directly from the Flood Map Service Center at the number listed above.

For community and countywide map dates, refer to the Flood Insurance Study Report for this jurisdiction.

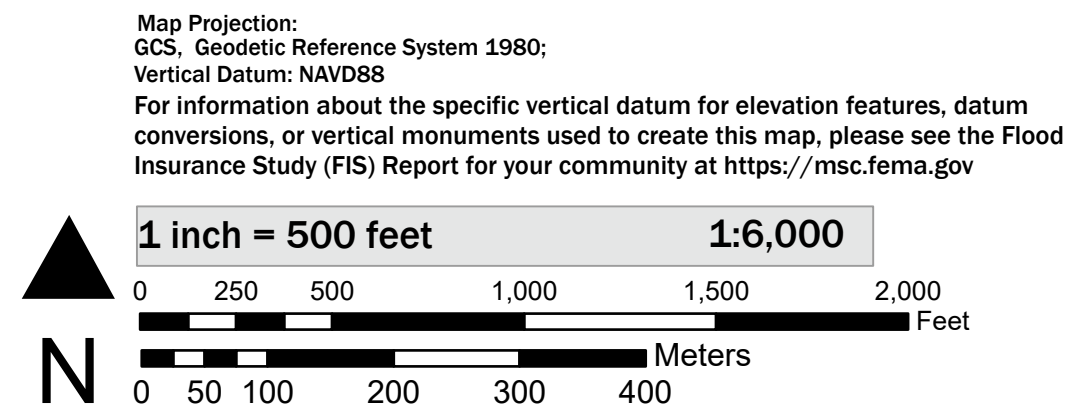
To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

Basemap information shown on this FIRM was provided in digital format by the United States Geological Survey (USGS). The basemap shown is the USGS National Map: Orthoimagery, Last refreshed October, 2020.

This map was exported from FEMA's National Flood Hazard Layer (NFHL) on 12/14/2023 3:20 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. For additional information, please see the Flood Hazard Mapping Updates Overview Fact Sheet at <https://www.fema.gov/media-library/assets/documents/118418>

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date.

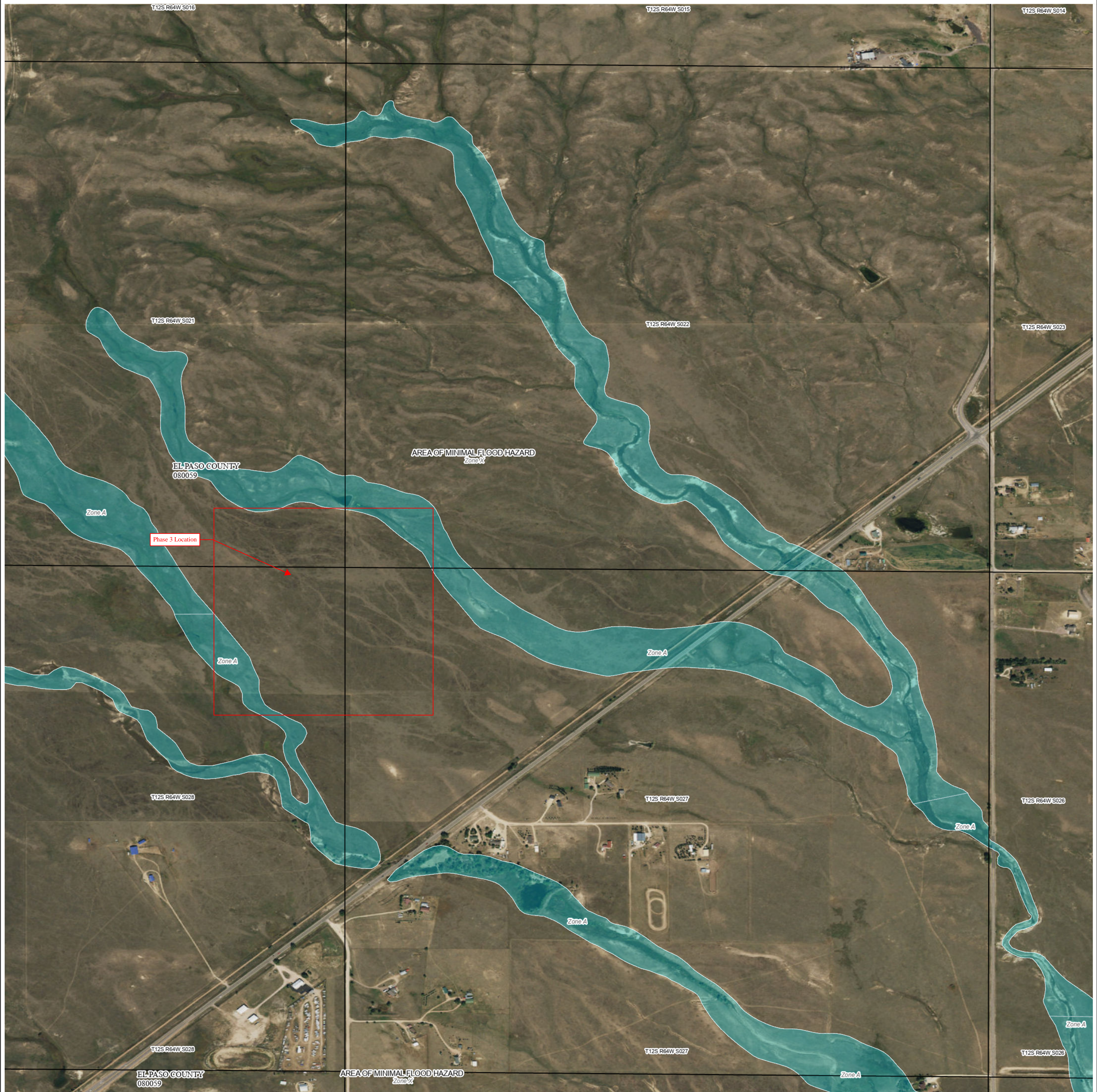
**SCALE**



**NATIONAL FLOOD INSURANCE PROGRAM**  
FLOOD INSURANCE RATE MAP

PANEL 552 OF 1275

Panel Contains:	NUMBER	PANEL
COMMUNITY	080059	0552
EL PASO COUNTY		



**FLOOD HAZARD INFORMATION**

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR DRAFT FIRM PANEL LAYOUT

<b>SPECIAL FLOOD HAZARD AREAS</b>		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
<b>OTHER AREAS OF FLOOD HAZARD</b>		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee See Notes Zone X
		Area with Flood Risk due to Levee Zone D
<b>OTHER AREAS</b>		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D
<b>GENERAL STRUCTURES</b>		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
		20.2 Cross Sections with 1% Annual Chance
		17.5 Water Surface Elevation
		8 Coastal Transect
		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
<b>OTHER FEATURES</b>		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary

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For community and countywide map dates, refer to the Flood Insurance Study Report for this jurisdiction.

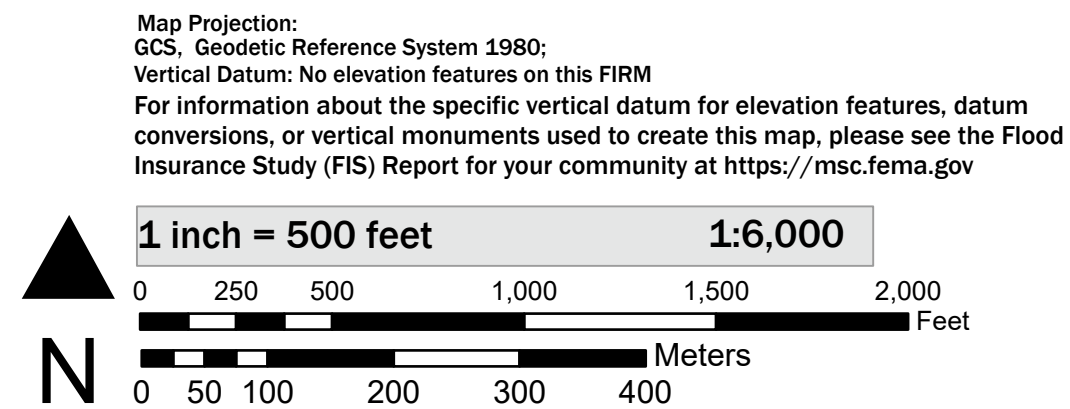
To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

Basemap information shown on this FIRM was provided in digital format by the United States Geological Survey (USGS). The basemap shown is the USGS National Map: Orthoimagery. Last refreshed October, 2020.

This map was exported from FEMA's National Flood Hazard Layer (NFHL) on 12/14/2023 3:22 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. For additional information, please see the Flood Hazard Mapping Updates Overview Fact Sheet at <https://www.fema.gov/media-library/assets/documents/118418>

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date.

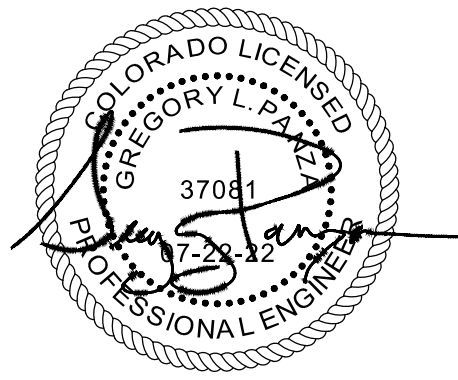
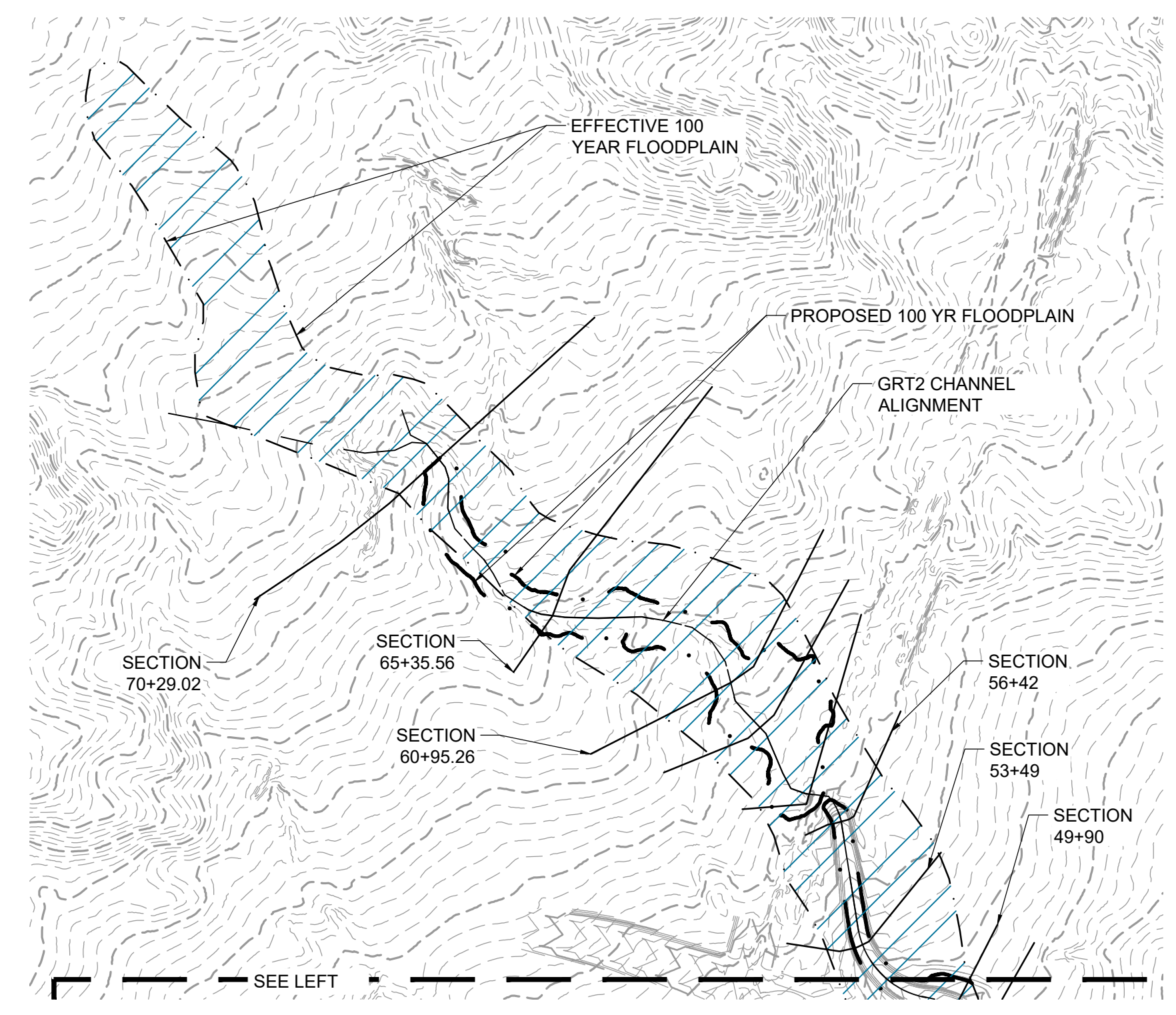
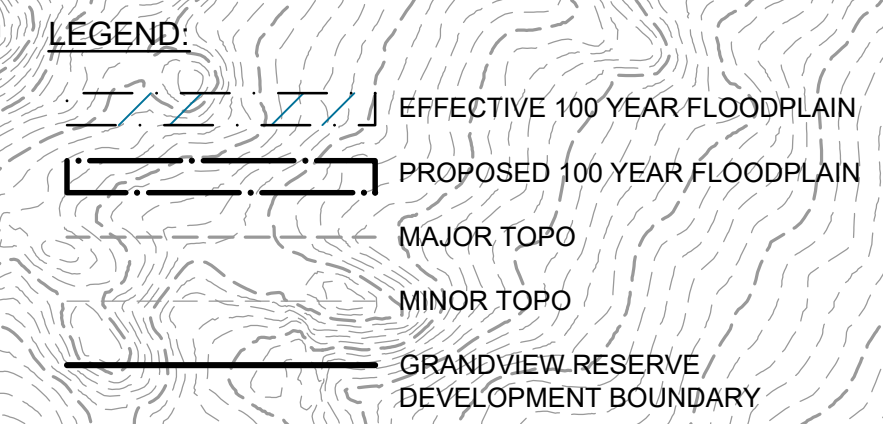
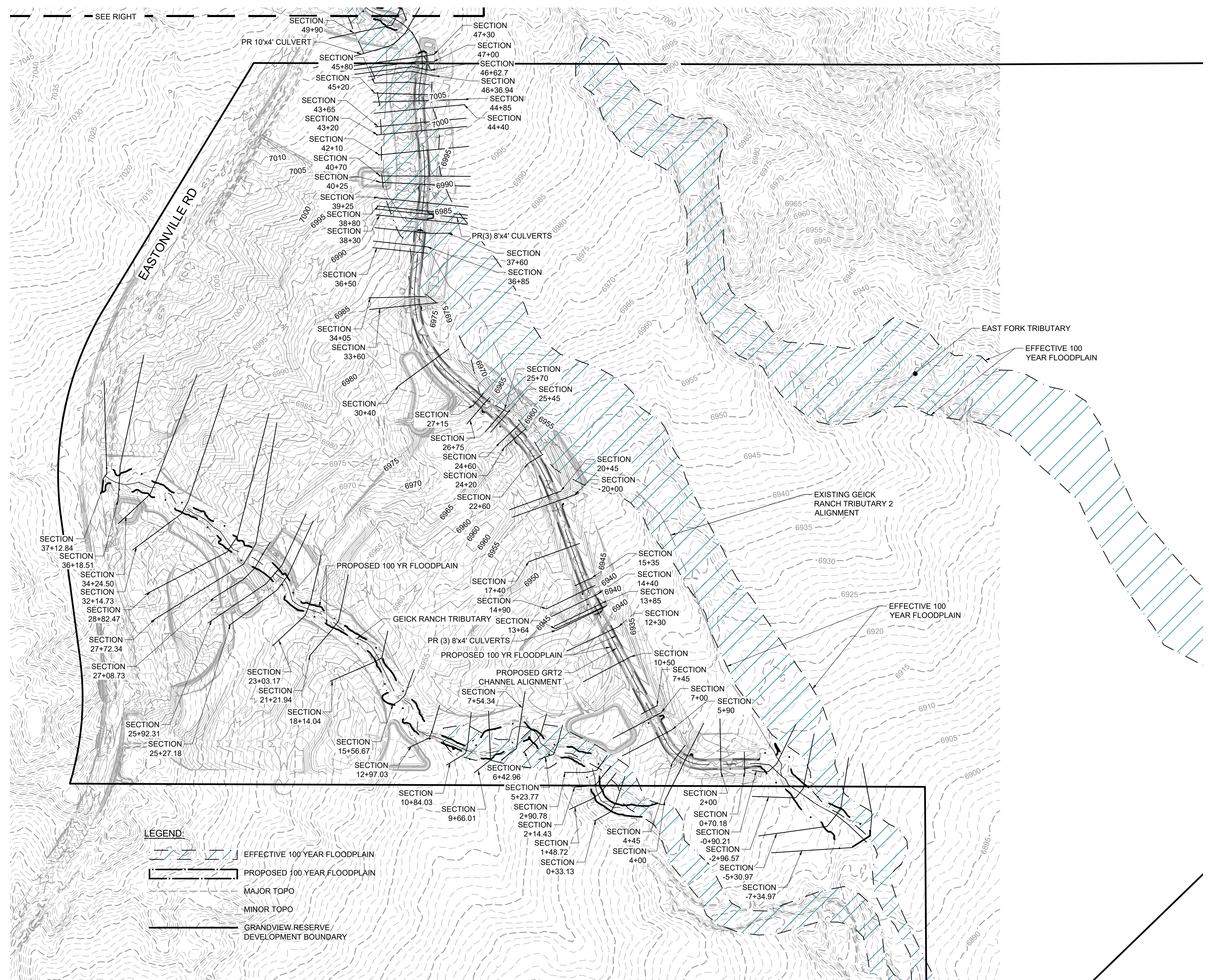
**SCALE**



**NATIONAL FLOOD INSURANCE PROGRAM**  
FLOOD INSURANCE RATE MAP

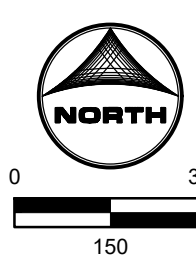
PANEL 556 OF 1275

Panel Contains:  
**COMMUNITY** EL PASO COUNTY      **NUMBER** 080059      **PANEL** 0556



**NOTES:**  
 1. BASIS OF BEARINGS: THE EAST LINE OF SECTION 21, BEING MONUMENTED AT THE SOUTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1998", BEING APPROPRIATELY MARKED, AND BEING MONUMENTED AT THE NORTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1998", BEING APPROPRIATELY MARKED, BEING ASSUMED TO BEAR NORTH 00 DEGREES 52 MINUTES 26 SECONDS WEST, A DISTANCE OF 5290.17 FEET.

NAVD88



Job No.:	201662
Prepared By:	SJF
Date:	3/21/2023

FLOODPLAIN EXHIBIT

## **APPENDIX B – HYDROLOGIC CALCULATIONS**



**NOAA Atlas 14, Volume 8, Version 2**  
**Location name: Peyton, Colorado, USA\***  
**Latitude: 38.9877°, Longitude: -104.5596°**  
**Elevation: 6971 ft\*\***  
 \* source: ESRI Maps  
 \*\* source: USGS



**POINT PRECIPITATION FREQUENCY ESTIMATES**

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

[PF\\_tabular](#) | [PF\\_graphical](#) | [Maps\\_&\\_aerials](#)

**PF tabular**

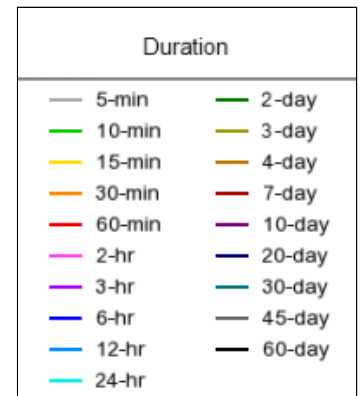
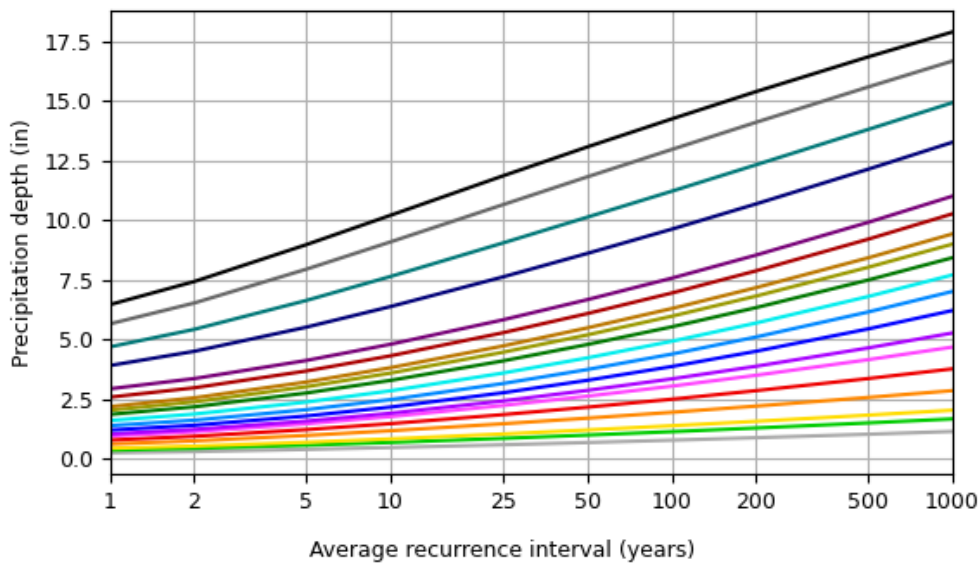
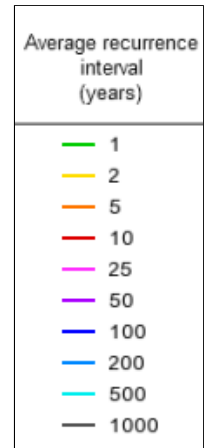
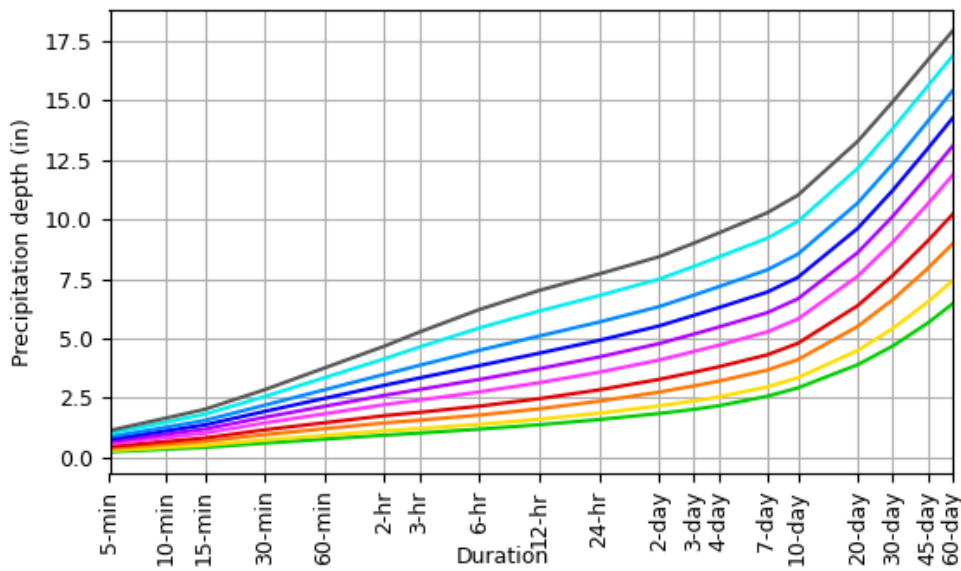
<b>PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)<sup>1</sup></b>										
<b>Duration</b>	<b>Average recurrence interval (years)</b>									
	<b>1</b>	<b>2</b>	<b>5</b>	<b>10</b>	<b>25</b>	<b>50</b>	<b>100</b>	<b>200</b>	<b>500</b>	<b>1000</b>
<b>5-min</b>	<b>0.239</b> (0.189-0.304)	<b>0.291</b> (0.230-0.371)	<b>0.382</b> (0.300-0.487)	<b>0.461</b> (0.360-0.591)	<b>0.576</b> (0.438-0.771)	<b>0.670</b> (0.497-0.906)	<b>0.769</b> (0.552-1.06)	<b>0.874</b> (0.602-1.24)	<b>1.02</b> (0.675-1.48)	<b>1.14</b> (0.731-1.67)
<b>10-min</b>	<b>0.350</b> (0.276-0.446)	<b>0.427</b> (0.337-0.544)	<b>0.559</b> (0.439-0.714)	<b>0.675</b> (0.528-0.866)	<b>0.844</b> (0.642-1.13)	<b>0.982</b> (0.728-1.33)	<b>1.13</b> (0.808-1.56)	<b>1.28</b> (0.881-1.82)	<b>1.49</b> (0.989-2.17)	<b>1.66</b> (1.07-2.44)
<b>15-min</b>	<b>0.427</b> (0.337-0.543)	<b>0.520</b> (0.410-0.663)	<b>0.681</b> (0.536-0.870)	<b>0.823</b> (0.643-1.06)	<b>1.03</b> (0.783-1.38)	<b>1.20</b> (0.888-1.62)	<b>1.37</b> (0.985-1.90)	<b>1.56</b> (1.07-2.22)	<b>1.82</b> (1.21-2.65)	<b>2.03</b> (1.30-2.98)
<b>30-min</b>	<b>0.607</b> (0.480-0.773)	<b>0.740</b> (0.583-0.942)	<b>0.967</b> (0.761-1.24)	<b>1.17</b> (0.912-1.50)	<b>1.46</b> (1.11-1.95)	<b>1.69</b> (1.26-2.29)	<b>1.94</b> (1.39-2.68)	<b>2.20</b> (1.51-3.12)	<b>2.56</b> (1.70-3.73)	<b>2.85</b> (1.83-4.19)
<b>60-min</b>	<b>0.774</b> (0.611-0.985)	<b>0.932</b> (0.735-1.19)	<b>1.21</b> (0.952-1.55)	<b>1.46</b> (1.14-1.88)	<b>1.84</b> (1.40-2.47)	<b>2.15</b> (1.60-2.92)	<b>2.49</b> (1.79-3.45)	<b>2.85</b> (1.96-4.05)	<b>3.35</b> (2.22-4.90)	<b>3.76</b> (2.42-5.54)
<b>2-hr</b>	<b>0.941</b> (0.749-1.19)	<b>1.12</b> (0.894-1.42)	<b>1.46</b> (1.15-1.84)	<b>1.76</b> (1.39-2.24)	<b>2.22</b> (1.71-2.97)	<b>2.61</b> (1.96-3.52)	<b>3.03</b> (2.20-4.19)	<b>3.49</b> (2.43-4.94)	<b>4.14</b> (2.78-6.02)	<b>4.68</b> (3.04-6.84)
<b>3-hr</b>	<b>1.03</b> (0.824-1.29)	<b>1.22</b> (0.973-1.53)	<b>1.57</b> (1.25-1.98)	<b>1.90</b> (1.50-2.40)	<b>2.41</b> (1.88-3.23)	<b>2.86</b> (2.16-3.85)	<b>3.34</b> (2.44-4.60)	<b>3.87</b> (2.72-5.47)	<b>4.64</b> (3.13-6.72)	<b>5.27</b> (3.44-7.67)
<b>6-hr</b>	<b>1.19</b> (0.961-1.48)	<b>1.40</b> (1.12-1.74)	<b>1.78</b> (1.43-2.23)	<b>2.16</b> (1.72-2.71)	<b>2.76</b> (2.17-3.67)	<b>3.28</b> (2.50-4.40)	<b>3.86</b> (2.85-5.29)	<b>4.50</b> (3.19-6.33)	<b>5.44</b> (3.70-7.84)	<b>6.21</b> (4.10-8.98)
<b>12-hr</b>	<b>1.38</b> (1.12-1.70)	<b>1.61</b> (1.30-1.98)	<b>2.05</b> (1.66-2.53)	<b>2.47</b> (1.99-3.07)	<b>3.14</b> (2.49-4.15)	<b>3.73</b> (2.87-4.96)	<b>4.38</b> (3.26-5.96)	<b>5.10</b> (3.64-7.12)	<b>6.14</b> (4.23-8.80)	<b>7.01</b> (4.67-10.1)
<b>24-hr</b>	<b>1.59</b> (1.30-1.95)	<b>1.86</b> (1.52-2.28)	<b>2.37</b> (1.93-2.90)	<b>2.84</b> (2.30-3.50)	<b>3.58</b> (2.86-4.66)	<b>4.22</b> (3.27-5.55)	<b>4.92</b> (3.69-6.62)	<b>5.68</b> (4.09-7.86)	<b>6.79</b> (4.71-9.65)	<b>7.70</b> (5.17-11.0)
<b>2-day</b>	<b>1.85</b> (1.53-2.24)	<b>2.17</b> (1.79-2.63)	<b>2.75</b> (2.26-3.34)	<b>3.28</b> (2.68-4.00)	<b>4.09</b> (3.28-5.26)	<b>4.78</b> (3.73-6.21)	<b>5.52</b> (4.17-7.36)	<b>6.33</b> (4.59-8.67)	<b>7.48</b> (5.23-10.5)	<b>8.42</b> (5.71-12.0)
<b>3-day</b>	<b>2.02</b> (1.68-2.44)	<b>2.38</b> (1.97-2.86)	<b>3.01</b> (2.48-3.64)	<b>3.58</b> (2.94-4.35)	<b>4.45</b> (3.58-5.68)	<b>5.18</b> (4.06-6.69)	<b>5.97</b> (4.52-7.90)	<b>6.81</b> (4.97-9.28)	<b>8.02</b> (5.63-11.2)	<b>8.99</b> (6.13-12.7)
<b>4-day</b>	<b>2.17</b> (1.81-2.61)	<b>2.55</b> (2.12-3.06)	<b>3.21</b> (2.66-3.86)	<b>3.81</b> (3.14-4.61)	<b>4.72</b> (3.80-6.00)	<b>5.48</b> (4.31-7.04)	<b>6.29</b> (4.79-8.30)	<b>7.17</b> (5.24-9.73)	<b>8.42</b> (5.93-11.8)	<b>9.42</b> (6.45-13.3)
<b>7-day</b>	<b>2.57</b> (2.16-3.06)	<b>2.97</b> (2.48-3.54)	<b>3.67</b> (3.06-4.39)	<b>4.31</b> (3.58-5.17)	<b>5.27</b> (4.28-6.64)	<b>6.08</b> (4.81-7.76)	<b>6.94</b> (5.32-9.09)	<b>7.87</b> (5.80-10.6)	<b>9.20</b> (6.53-12.8)	<b>10.3</b> (7.08-14.4)
<b>10-day</b>	<b>2.92</b> (2.46-3.46)	<b>3.35</b> (2.82-3.98)	<b>4.11</b> (3.44-4.89)	<b>4.79</b> (3.99-5.73)	<b>5.81</b> (4.73-7.28)	<b>6.66</b> (5.29-8.45)	<b>7.56</b> (5.82-9.85)	<b>8.53</b> (6.32-11.4)	<b>9.90</b> (7.06-13.7)	<b>11.0</b> (7.63-15.4)
<b>20-day</b>	<b>3.90</b> (3.31-4.57)	<b>4.50</b> (3.81-5.28)	<b>5.51</b> (4.65-6.49)	<b>6.37</b> (5.36-7.55)	<b>7.61</b> (6.22-9.37)	<b>8.60</b> (6.87-10.8)	<b>9.62</b> (7.44-12.4)	<b>10.7</b> (7.95-14.1)	<b>12.1</b> (8.71-16.6)	<b>13.3</b> (9.28-18.4)
<b>30-day</b>	<b>4.68</b> (3.99-5.46)	<b>5.42</b> (4.61-6.33)	<b>6.63</b> (5.63-7.76)	<b>7.64</b> (6.45-8.99)	<b>9.03</b> (7.39-11.0)	<b>10.1</b> (8.11-12.5)	<b>11.2</b> (8.70-14.3)	<b>12.3</b> (9.20-16.2)	<b>13.8</b> (9.95-18.7)	<b>14.9</b> (10.5-20.6)
<b>45-day</b>	<b>5.64</b> (4.84-6.55)	<b>6.52</b> (5.58-7.58)	<b>7.94</b> (6.77-9.25)	<b>9.09</b> (7.71-10.6)	<b>10.6</b> (8.73-12.8)	<b>11.8</b> (9.49-14.5)	<b>13.0</b> (10.1-16.4)	<b>14.1</b> (10.6-18.4)	<b>15.6</b> (11.3-21.0)	<b>16.7</b> (11.8-22.9)
<b>60-day</b>	<b>6.45</b> (5.55-7.46)	<b>7.42</b> (6.37-8.59)	<b>8.96</b> (7.68-10.4)	<b>10.2</b> (8.69-11.9)	<b>11.8</b> (9.74-14.2)	<b>13.1</b> (10.5-16.0)	<b>14.2</b> (11.1-17.9)	<b>15.4</b> (11.6-20.0)	<b>16.8</b> (12.2-22.6)	<b>17.9</b> (12.7-24.6)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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**PF graphical**

PDS-based depth-duration-frequency (DDF) curves  
 Latitude: 38.9877°, Longitude: -104.5596°

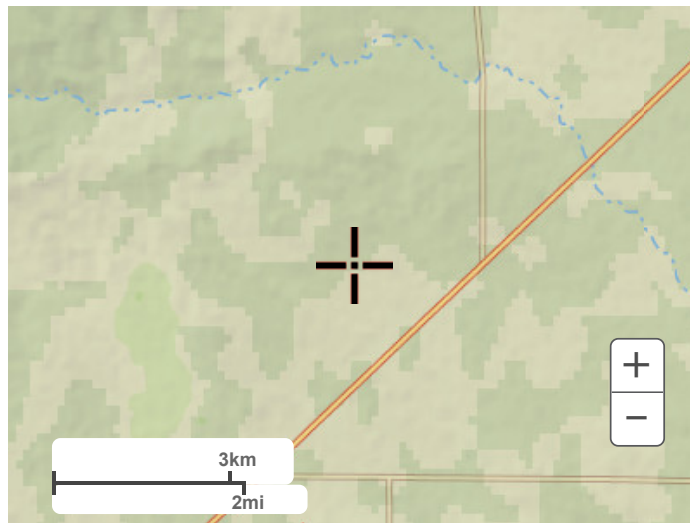


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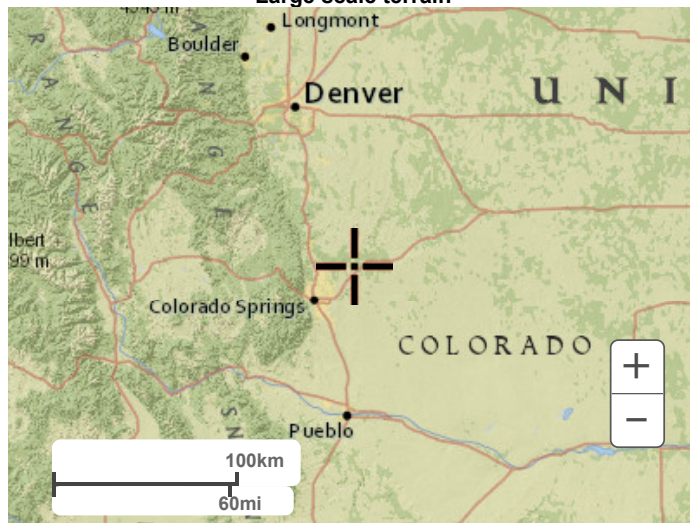
**Maps & aeriels**

**Small scale terrain**





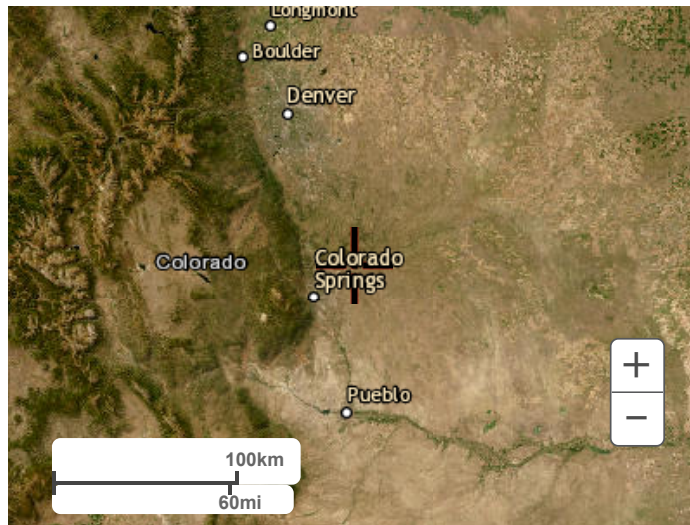
Large scale terrain



Large scale map



Large scale aerial



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Questions?: [HDSC.Questions@noaa.gov](mailto:HDSC.Questions@noaa.gov)

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**APPENDIX C – HYDRAULIC CALCULATIONS  
(TO BE PROVIDED WITH FDR)**

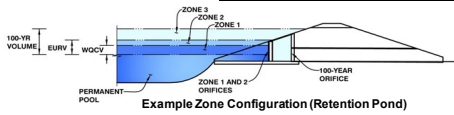
## **APPENDIX D – WATER QUALITY & DETENTION**

**DETENTION BASIN STAGE-STORAGE TABLE BUILDER**

*MHFD-Detention, Version 4.06 (July 2022)*

**Project:** Grandview-PHASE 3

**Basin ID:** POND C



**Watershed Information**

Selected BMP Type =	<b>EDB</b>
Watershed Area =	67.53 acres
Watershed Length =	1,890 ft
Watershed Length to Centroid =	1,050 ft
Watershed Slope =	0.020 ft/ft
Watershed Imperviousness =	53.00% percent
Percentage Hydrologic Soil Group A =	100.0% percent
Percentage Hydrologic Soil Group B =	0.0% percent
Percentage Hydrologic Soil Groups C/D =	0.0% percent
Target WQC Drain Time =	40.0 hours
Location for 1-hr Rainfall Depths =	User Input

After providing required inputs above including 1-hour rainfall depths, click "Run CUHP" to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WQCV) =	1,208	acre-feet
Excess Urban Runoff Volume (EURV) =	4,195	acre-feet
2-yr Runoff Volume (P1 = 0.93 in.) =	2,379	acre-feet
5-yr Runoff Volume (P1 = 1.21 in.) =	3,190	acre-feet
10-yr Runoff Volume (P1 = 1.46 in.) =	4,011	acre-feet
25-yr Runoff Volume (P1 = 1.84 in.) =	5,462	acre-feet
50-yr Runoff Volume (P1 = 2.15 in.) =	6,932	acre-feet
100-yr Runoff Volume (P1 = 2.49 in.) =	8,780	acre-feet
500-yr Runoff Volume (P1 = 3.35 in.) =	13,395	acre-feet
Approximate 2-yr Detention Volume =	2,114	acre-feet
Approximate 5-yr Detention Volume =	2,867	acre-feet
Approximate 10-yr Detention Volume =	3,609	acre-feet
Approximate 25-yr Detention Volume =	4,851	acre-feet
Approximate 50-yr Detention Volume =	5,606	acre-feet
Approximate 100-yr Detention Volume =	6,488	acre-feet

**Define Zones and Basin Geometry**

Zone 1 Volume (WQCV) =	1,208	acre-feet
Zone 2 Volume (EURV - Zone 1) =	2,987	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	2,293	acre-feet
Total Detention Basin Volume =	6,488	acre-feet
Initial Surge Volume (ISV) =	user	ft <sup>3</sup>
Initial Surge Depth (ISD) =	user	ft
Total Available Detention Depth (H <sub>total</sub> ) =	user	ft
Depth of Trickle Channel (H <sub>TC</sub> ) =	user	ft
Slope of Trickle Channel (S <sub>TC</sub> ) =	user	ft/ft
Slopes of Main Basin Sides (S <sub>main</sub> ) =	user	H:V
Basin Length-to-Width Ratio (R <sub>L/W</sub> ) =	user	
Initial Surge Area (A <sub>ISV</sub> ) =	user	ft <sup>2</sup>
Surge Volume Length (L <sub>LSV</sub> ) =	user	ft
Surge Volume Width (W <sub>LSV</sub> ) =	user	ft
Depth of Basin Floor (H <sub>FLOOR</sub> ) =	user	ft
Length of Basin Floor (L <sub>FLOOR</sub> ) =	user	ft
Width of Basin Floor (W <sub>FLOOR</sub> ) =	user	ft
Area of Basin Floor (A <sub>FLOOR</sub> ) =	user	ft <sup>2</sup>
Volume of Basin Floor (V <sub>FLOOR</sub> ) =	user	ft <sup>3</sup>
Depth of Main Basin (H <sub>MAN</sub> ) =	user	ft
Length of Main Basin (L <sub>MAN</sub> ) =	user	ft
Width of Main Basin (W <sub>MAN</sub> ) =	user	ft
Area of Main Basin (A <sub>MAN</sub> ) =	user	ft <sup>2</sup>
Volume of Main Basin (V <sub>MAN</sub> ) =	user	ft <sup>3</sup>
Calculated Total Basin Volume (V <sub>total</sub> ) =	user	acre-feet

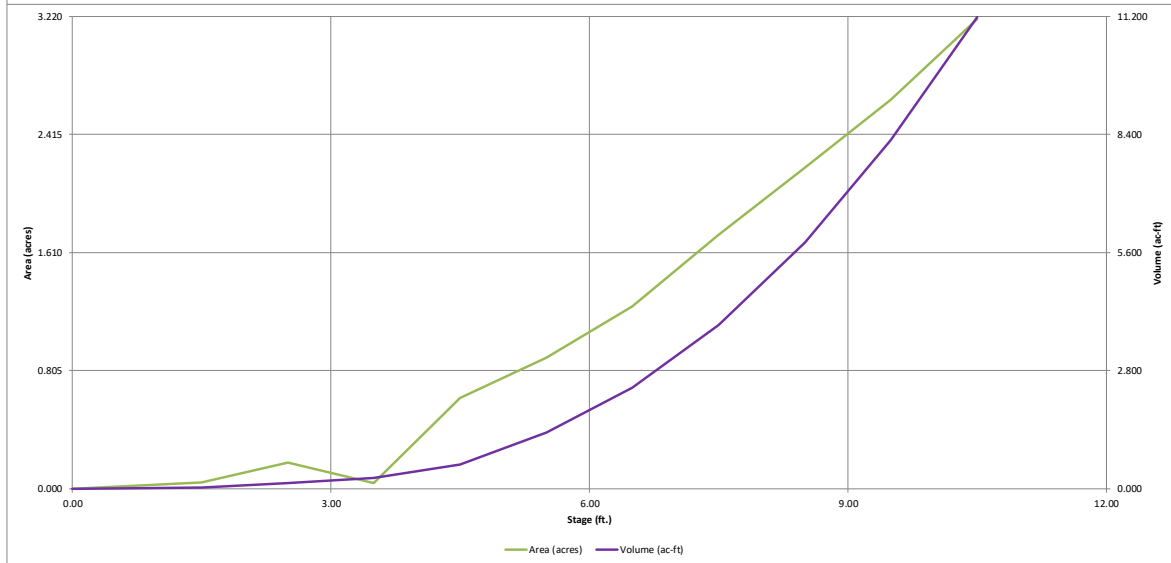
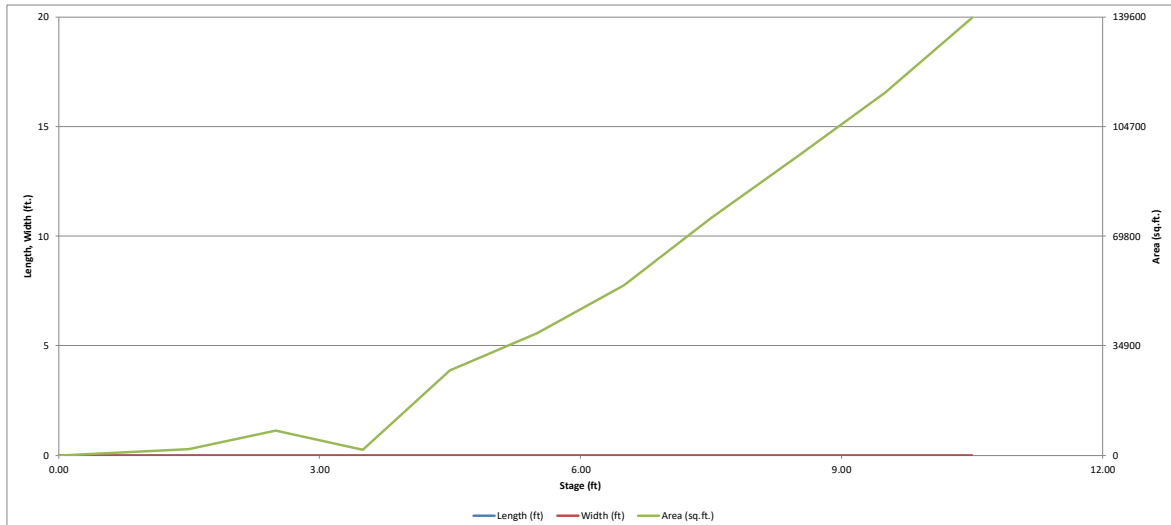
**Optional User Overrides**

	acre-feet
	acre-feet
0.93	inches
1.21	inches
1.46	inches
1.84	inches
2.15	inches
2.49	inches
3.35	inches

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft <sup>2</sup> )	Optional Override Area (ft <sup>2</sup> )	Area (acre)	Volume (ft <sup>3</sup> )	Volume (ac-ft)
6910.5 Top of Micropool	--	0.00	--	--	--	0	0.00		
6912	--	1.50	--	--	--	1,805	0.041	1,354	0.031
6913	--	2.50	--	--	--	7,751	0.178	6,132	0.141
6914	--	3.50	--	--	--	1,650	0.038	10,832	0.249
6915	--	4.50	--	--	--	26,906	0.618	25,110	0.576
6916	--	5.50	--	--	--	38,893	0.893	58,009	1.332
6917	--	6.50	--	--	--	54,061	1.241	104,486	2.399
6918	--	7.50	--	--	--	75,492	1.733	169,263	3.886
6919	--	8.50	--	--	--	95,279	2.187	254,648	5.846
6920	--	9.50	--	--	--	115,443	2.650	360,009	8.265
6921	--	10.50	--	--	--	139,514	3.203	487,488	11.191

# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

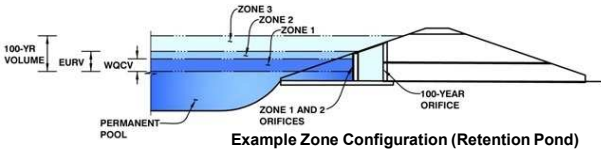
MHFD-Detention, Version 4.06 (July 2022)



# DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

**Project: Grandview-PHASE 3**  
**Basin ID: POND C**



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	5.36	1.208	Orifice Plate
Zone 2 (EURV)	7.68	2.987	Circular Orifice
Zone 3 (100-year)	8.79	2.293	Weir&Pipe (Restrict)
<b>Total (all zones)</b>		<b>6.488</b>	

**User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)**

Underdrain Orifice Invert Depth =  ft (distance below the filtration media surface)  
 Underdrain Orifice Diameter =  inches

**Calculated Parameters for Underdrain**

Underdrain Orifice Area =  ft<sup>2</sup>  
 Underdrain Orifice Centroid =  feet

**User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)**

Centroid of Lowest Orifice =  0.00 ft (relative to basin bottom at Stage = 0 ft)  
 Depth at top of Zone using Orifice Plate =  7.68 ft (relative to basin bottom at Stage = 0 ft)  
 Orifice Plate: Orifice Vertical Spacing =  N/A inches  
 Orifice Plate: Orifice Area per Row =  2.85 sq. inches (diameter = 1-7/8 inches)

**Calculated Parameters for Plate**

WQ Orifice Area per Row =  1.979E-02 ft<sup>2</sup>  
 Elliptical Half-Width =  N/A feet  
 Elliptical Slot Centroid =  N/A feet  
 Elliptical Slot Area =  N/A ft<sup>2</sup>

**User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)**

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.79	3.57					
Orifice Area (sq. inches)	2.85	2.85	2.85					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

**User Input: Vertical Orifice (Circular or Rectangular)**

	Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	5.36	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	7.68	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	4.50	N/A	inches

**Calculated Parameters for Vertical Orifice**

	Zone 2 Circular	Not Selected	
Vertical Orifice Area =	0.11	N/A	ft <sup>2</sup>
Vertical Orifice Centroid =	0.19	N/A	feet

**User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)**

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	7.70	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	8.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	8.00	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	50%	N/A	%

**Calculated Parameters for Overflow Weir**

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H <sub>1</sub> =	7.70	N/A	feet
Overflow Weir Slope Length =	8.00	N/A	feet
Grate Open Area / 100-yr Orifice Area =	16.29	N/A	
Overflow Grate Open Area w/o Debris =	44.54	N/A	ft <sup>2</sup>
Overflow Grate Open Area w/ Debris =	22.27	N/A	ft <sup>2</sup>

**User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)**

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	1.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	24.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	19.50	N/A	inches

**Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate**

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	2.73	N/A	ft <sup>2</sup>
Outlet Orifice Centroid =	0.88	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	2.25	N/A	radians

**User Input: Emergency Spillway (Rectangular or Trapezoidal)**

Spillway Invert Stage =	8.70	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	75.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

**Calculated Parameters for Spillway**

Spillway Design Flow Depth =	0.79	feet
Stage at Top of Freeboard =	10.49	feet
Basin Area at Top of Freeboard =	3.19	acres
Basin Volume at Top of Freeboard =	11.13	acre-ft

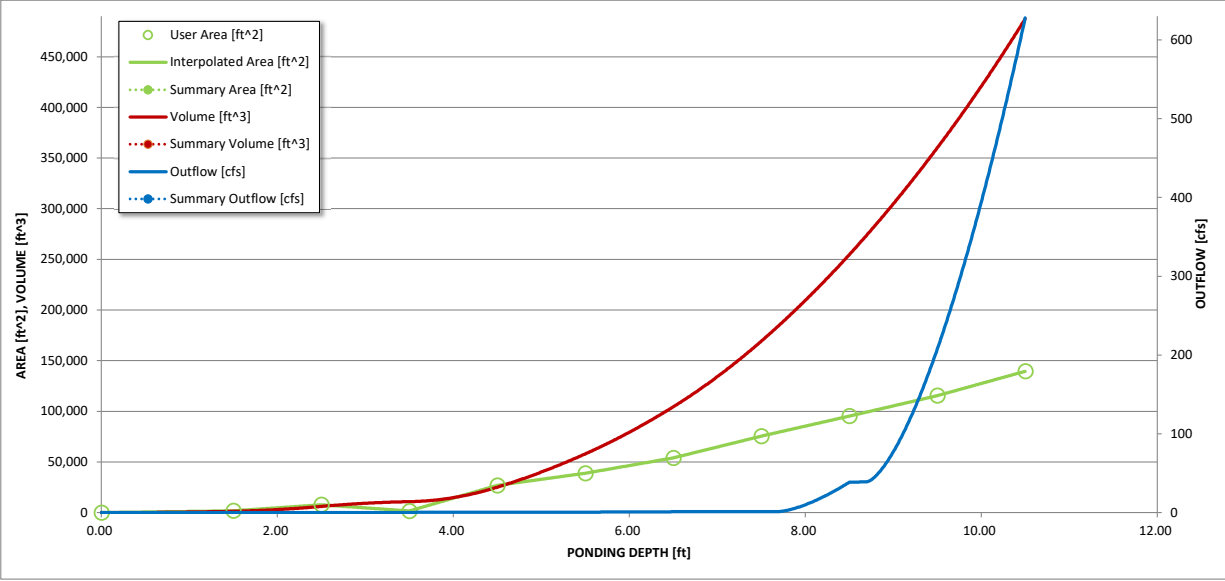
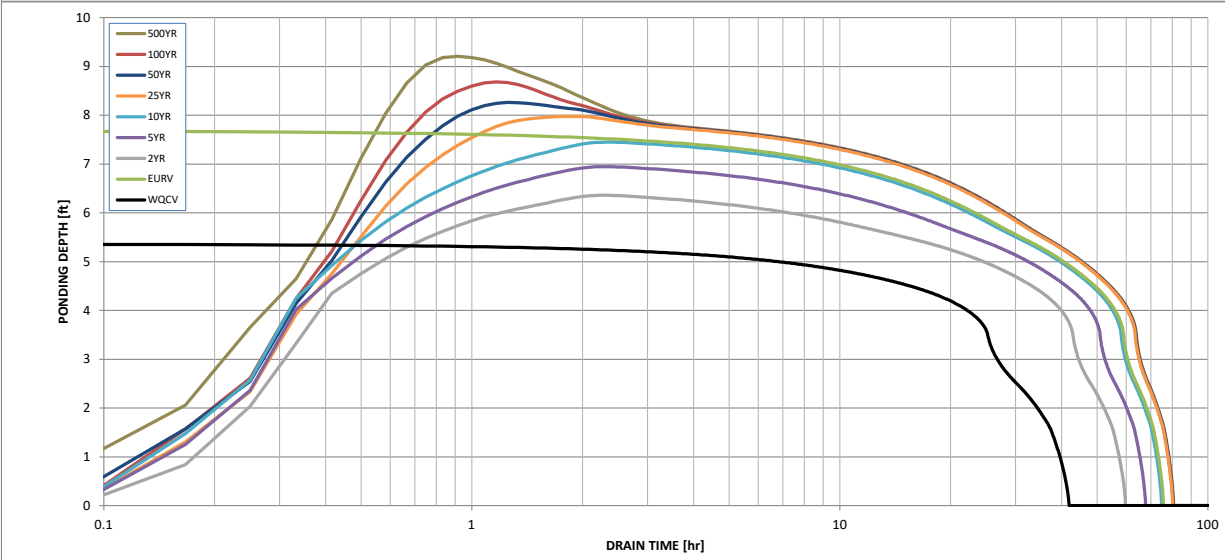
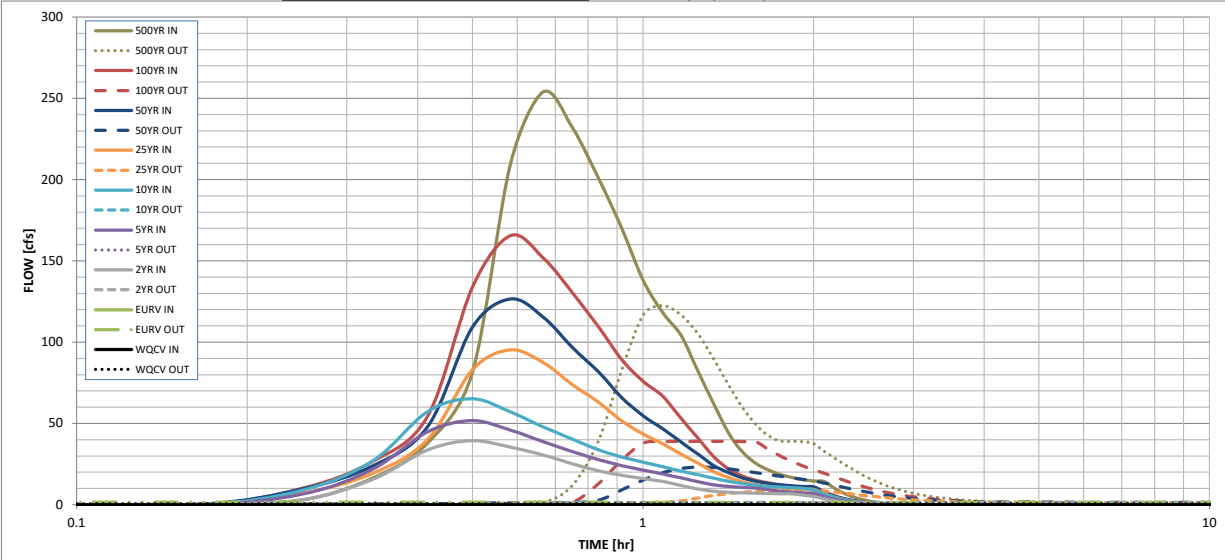
**Routed Hydrograph Results**

*The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).*

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	N/A	N/A	0.93	1.21	1.46	1.84	2.15	2.49	3.35
CUHP Runoff Volume (acre-ft) =	1.208	4.195	2.379	3.190	4.011	5.462	6.932	8.780	13.395
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	2.379	3.190	4.011	5.462	6.932	8.780	13.395
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.0	0.5	1.0	5.6	22.9	44.9	96.8
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.01	0.01	0.08	0.34	0.66	1.43
Peak Inflow Q (cfs) =	N/A	N/A	39.4	51.7	65.1	95.2	126.5	165.5	253.9
Peak Outflow Q (cfs) =	0.5	1.5	1.1	1.3	1.4	8.9	23.1	39.0	122.4
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	2.4	1.5	1.6	1.0	0.9	1.3
Structure Controlling Flow =	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.2	0.5	0.8	0.9
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	37	65	52	59	64	67	66	63	59
Time to Drain 99% of Inflow Volume (hours) =	40	71	56	63	70	74	73	72	69
Maximum Ponding Depth (ft) =	5.36	7.68	6.36	6.94	7.44	7.97	8.26	8.68	9.20
Area at Maximum Ponding Depth (acres) =	0.85	1.81	1.19	1.46	1.70	1.95	2.07	2.27	2.51
Maximum Volume Stored (acre-ft) =	1.209	4.205	2.216	2.992	3.783	4.750	5.313	6.247	7.490

# DETENTION BASIN OUTLET STRUCTURE DESIGN

*MHFD-Detention, Version 4.06 (July 2022)*



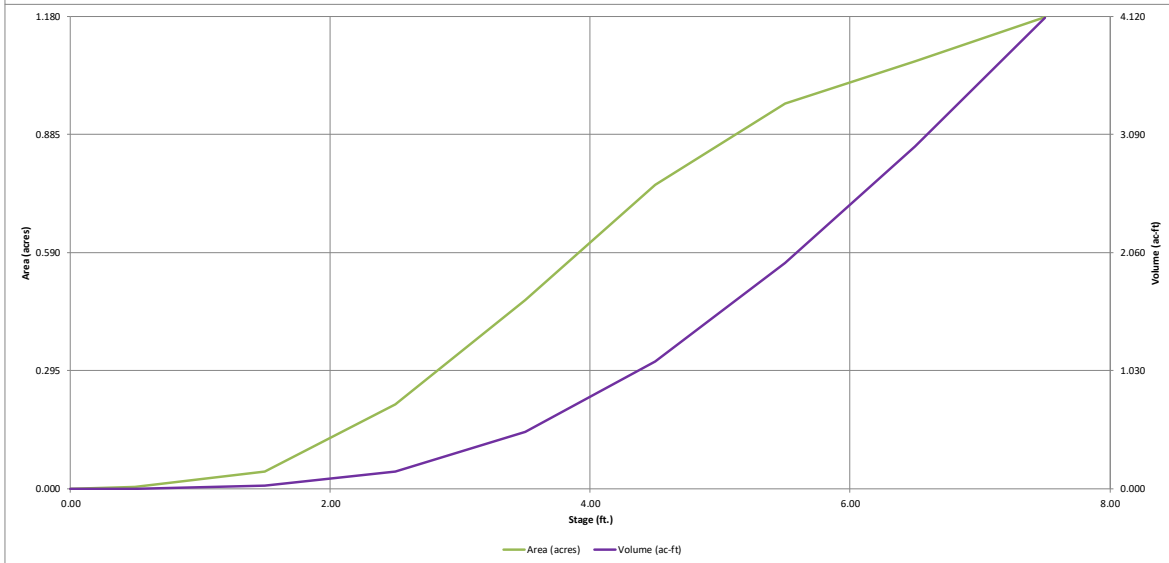
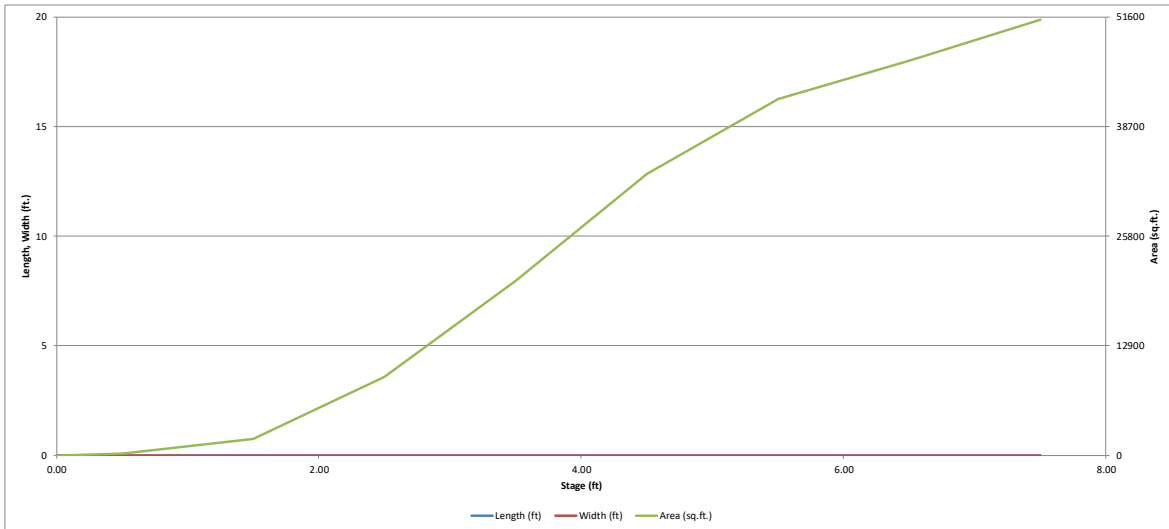
S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			





# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

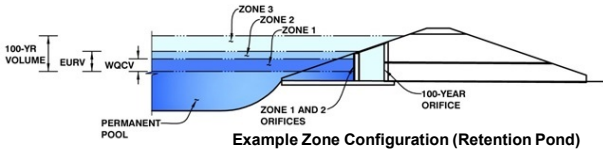
*MHFD-Detention, Version 4.06 (July 2022)*



# DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-*Detention, Version 4.06 (July 2022)*

**Project: GRANDVIEW - PHASE 3**  
**Basin ID: POND F (SOUTHEAST PHASE 3)**



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.43	0.458	Orifice Plate
Zone 2 (EURV)	5.03	1.078	Circular Orifice
Zone 3 (100-year)	5.95	0.876	Weir&Pipe (Restrict)
<b>Total (all zones)</b>		<b>2.411</b>	

**User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)**

Underdrain Orifice Invert Depth =  ft (distance below the filtration media surface)  
 Underdrain Orifice Diameter =  inches

**Calculated Parameters for Underdrain**  
 Underdrain Orifice Area =  ft<sup>2</sup>  
 Underdrain Orifice Centroid =  feet

**User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)**

Centroid of Lowest Orifice =  ft (relative to basin bottom at Stage = 0 ft)  
 Depth at top of Zone using Orifice Plate =  ft (relative to basin bottom at Stage = 0 ft)  
 Orifice Plate: Orifice Vertical Spacing =  inches  
 Orifice Plate: Orifice Area per Row =  sq. inches (diameter = 1-1/4 inches)

**Calculated Parameters for Plate**  
 WQ Orifice Area per Row =  ft<sup>2</sup>  
 Elliptical Half-Width =  feet  
 Elliptical Slot Centroid =  feet  
 Elliptical Slot Area =  ft<sup>2</sup>

**User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)**

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.14	2.29					
Orifice Area (sq. inches)	1.31	1.31	1.31					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

**User Input: Vertical Orifice (Circular or Rectangular)**

	Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	3.43	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	5.03	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	3.00	N/A	inches

**Calculated Parameters for Vertical Orifice**

	Zone 2 Circular	Not Selected	
Vertical Orifice Area =	0.05	N/A	ft <sup>2</sup>
Vertical Orifice Centroid =	0.13	N/A	feet

**User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)**

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	5.10	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	5.67	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	2.92	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	50%	N/A	%

**Calculated Parameters for Overflow Weir**

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H <sub>1</sub> =	5.10	N/A	feet
Overflow Weir Slope Length =	2.92	N/A	feet
Grate Open Area / 100-yr Orifice Area =	12.62	N/A	
Overflow Grate Open Area w/o Debris =	11.52	N/A	ft <sup>2</sup>
Overflow Grate Open Area w/ Debris =	5.76	N/A	ft <sup>2</sup>

**User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)**

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	1.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	21.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	8.50	N/A	inches

**Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate**

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	0.91	N/A	ft <sup>2</sup>
Outlet Orifice Centroid =	0.41	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	1.38	N/A	radians

**User Input: Emergency Spillway (Rectangular or Trapezoidal)**

Spillway Invert Stage =	5.90	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	30.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

**Calculated Parameters for Spillway**

Spillway Design Flow Depth =	0.60	feet
Stage at Top of Freeboard =	7.50	feet
Basin Area at Top of Freeboard =	1.18	acres
Basin Volume at Top of Freeboard =	4.11	acre-ft

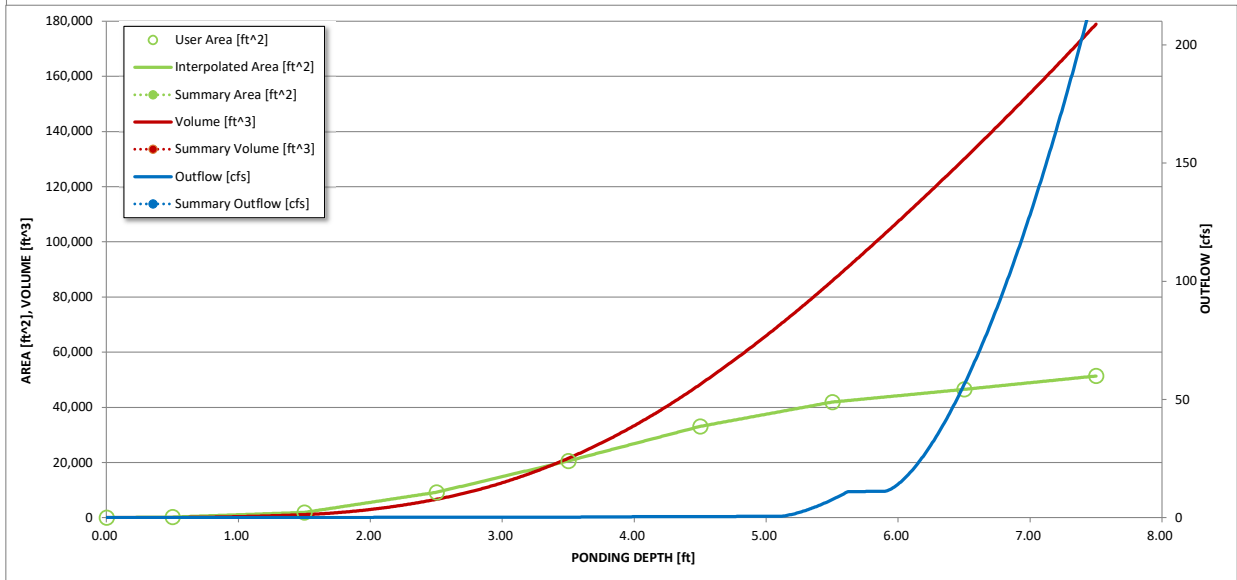
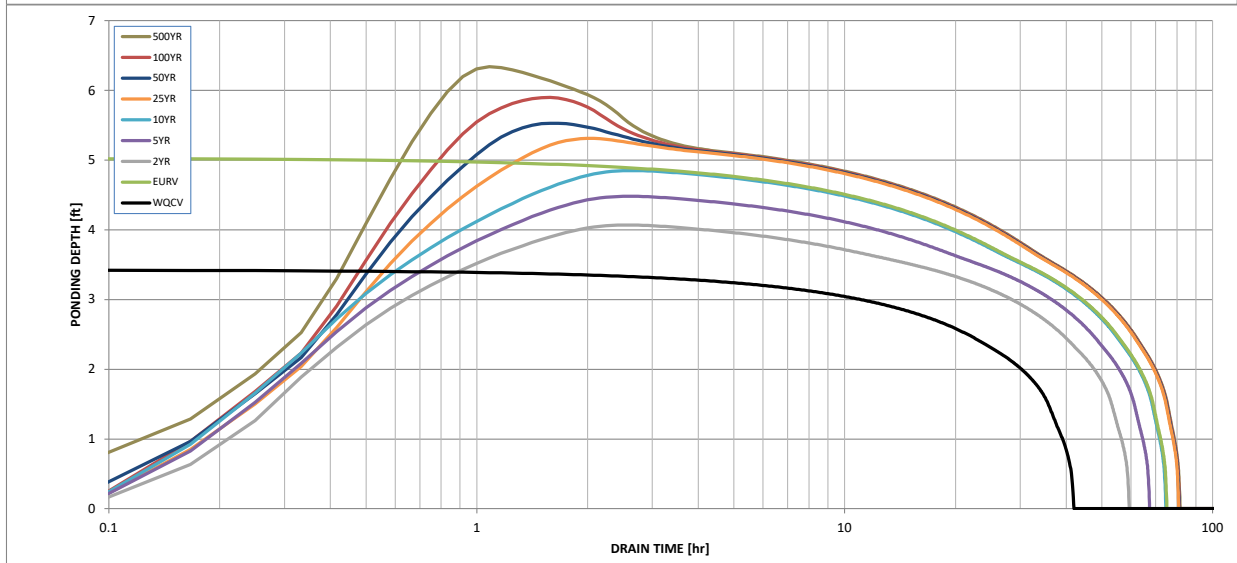
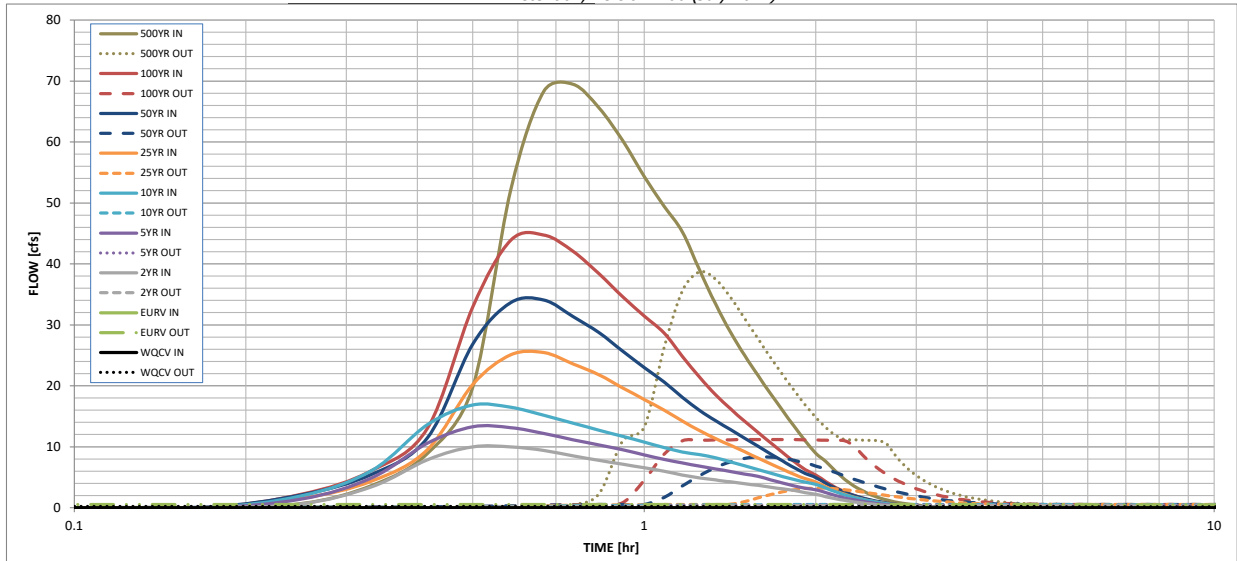
**Routed Hydrograph Results**

*The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).*

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	0.93	1.21	1.46	1.84	2.15	2.49	3.35
One-Hour Rainfall Depth (in) =	N/A	N/A	0.93	1.21	1.46	1.84	2.15	2.49	3.35
CUHP Runoff Volume (acre-ft) =	0.458	1.535	0.871	1.172	1.478	2.029	2.601	3.327	5.141
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.871	1.172	1.478	2.029	2.601	3.327	5.141
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.0	0.1	0.3	1.4	5.9	11.9	26.3
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.01	0.01	0.05	0.22	0.45	0.99
Peak Inflow Q (cfs) =	N/A	N/A	10.0	13.3	16.9	25.4	34.1	44.7	69.5
Peak Outflow Q (cfs) =	0.2	0.5	0.4	0.5	0.5	3.2	8.3	11.2	38.7
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	3.4	2.1	2.3	1.4	0.9	1.5
Structure Controlling Flow =	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Spillway	Spillway
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.2	0.7	0.9	0.9
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	66	53	60	66	70	68	66	61
Time to Drain 99% of Inflow Volume (hours) =	40	71	57	64	70	76	75	74	72
Maximum Ponding Depth (ft) =	3.43	5.03	4.07	4.48	4.85	5.31	5.53	5.90	6.34
Area at Maximum Ponding Depth (acres) =	0.45	0.87	0.63	0.75	0.83	0.92	0.96	1.00	1.05
Maximum Volume Stored (acre-ft) =	0.461	1.540	0.803	1.087	1.379	1.782	1.989	2.353	2.805

# DETENTION BASIN OUTLET STRUCTURE DESIGN

*MHFD-Detention, Version 4.06 (July 2022)*



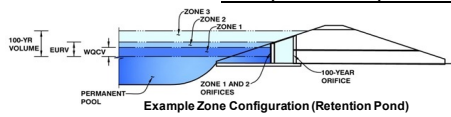
S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: GRANDVIEW - PHASE 3

Basin ID: POND R (REX RD ENTRANCE)



Example Zone Configuration (Retention Pond)

Watershed Information

Table with watershed parameters: Selected BMP Type = EDB, Watershed Area = 19.29 acres, Watershed Length = 1,500 ft, etc.

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Table with water quality and detention parameters: Water Quality Capture Volume (WQCV) = 0.298 acre-feet, Excess Urban Runoff Volume (EURV) = 0.890 acre-feet, etc.

Optional User Overrides

Table for optional user overrides with columns for parameter name and value in different units (acre-feet, inches).

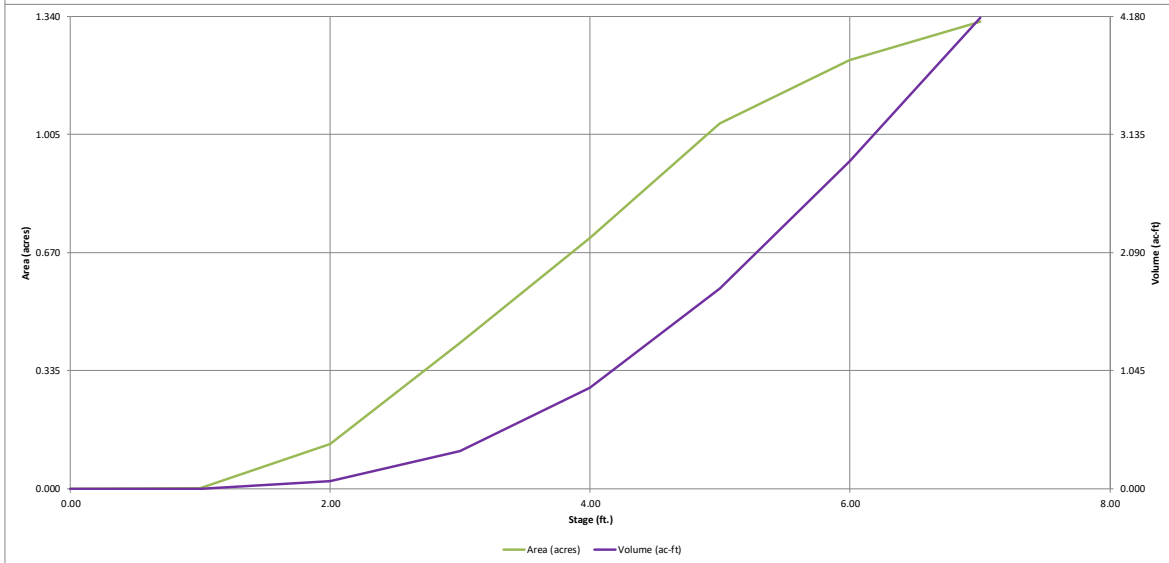
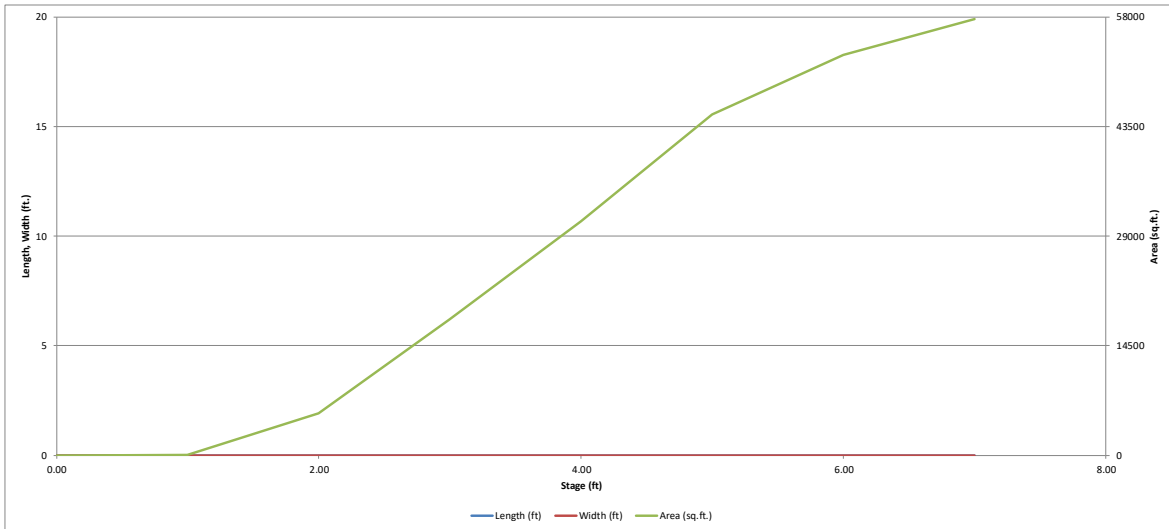
Define Zones and Basin Geometry

Table for defining zones and basin geometry: Zone 1 Volume (WQCV) = 0.298 acre-feet, Zone 2 Volume (EURV - Zone 1) = 0.592 acre-feet, etc.

Main stage-storage table with columns: Stage - Storage Description, Stage (ft), Optional Override Stage (ft), Length (ft), Width (ft), Area (ft^2), Optional Override Area (ft^2), Area (acre), Volume (ft^3), Volume (ac-ft). Includes row 6909 'Top of Micropool'.

# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

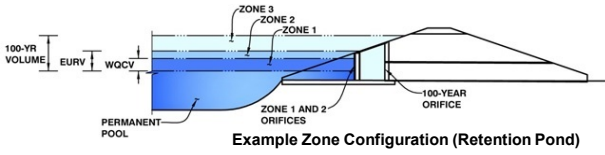
*MHFD-Detention, Version 4.06 (July 2022)*



# DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-*Detention, Version 4.06 (July 2022)*

**Project: GRANDVIEW - PHASE 3**  
**Basin ID: POND R (REX RD ENTRANCE)**



**Example Zone Configuration (Retention Pond)**

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.91	0.298	Orifice Plate
Zone 2 (EURV)	4.00	0.592	Orifice Plate
Zone 3 (100-year)	4.72	0.596	Weir&Pipe (Restrict)
<b>Total (all zones)</b>		<b>1.485</b>	

**User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)**

Underdrain Orifice Invert Depth =  ft (distance below the filtration media surface)  
 Underdrain Orifice Diameter =  inches

**Calculated Parameters for Underdrain**

Underdrain Orifice Area =  ft<sup>2</sup>  
 Underdrain Orifice Centroid =  feet

**User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)**

Centroid of Lowest Orifice =  0.00 ft (relative to basin bottom at Stage = 0 ft)  
 Depth at top of Zone using Orifice Plate =  4.00 ft (relative to basin bottom at Stage = 0 ft)  
 Orifice Plate: Orifice Vertical Spacing =  N/A inches  
 Orifice Plate: Orifice Area per Row =  N/A sq. inches

**Calculated Parameters for Plate**

WQ Orifice Area per Row =  N/A ft<sup>2</sup>  
 Elliptical Half-Width =  N/A feet  
 Elliptical Slot Centroid =  N/A feet  
 Elliptical Slot Area =  N/A ft<sup>2</sup>

**User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)**

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.33	2.67	3.00				
Orifice Area (sq. inches)	1.08	1.08	1.08	3.15				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

**User Input: Vertical Orifice (Circular or Rectangular)**

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

**Calculated Parameters for Vertical Orifice**

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft <sup>2</sup>
Vertical Orifice Centroid =	N/A	N/A	feet

**User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)**

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, H <sub>o</sub> =	4.10	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	4.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	4.00	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	0%	N/A	%

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H <sub>1</sub> =	4.10	N/A	feet
Overflow Weir Slope Length =	4.00	N/A	feet
Grate Open Area / 100-yr Orifice Area =	13.56	N/A	
Overflow Grate Open Area w/o Debris =	11.14	N/A	ft <sup>2</sup>
Overflow Grate Open Area w/ Debris =	11.14	N/A	ft <sup>2</sup>

**User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)**

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	1.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	8.50		inches

**Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate**

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	0.82	N/A	ft <sup>2</sup>
Outlet Orifice Centroid =	0.41	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	1.52	N/A	radians

**User Input: Emergency Spillway (Rectangular or Trapezoidal)**

Spillway Invert Stage =  4.60 ft (relative to basin bottom at Stage = 0 ft)  
 Spillway Crest Length =  9.00 feet  
 Spillway End Slopes =  4.00 H:V  
 Freeboard above Max Water Surface =  1.00 feet

**Calculated Parameters for Spillway**

Spillway Design Flow Depth =  0.85 feet  
 Stage at Top of Freeboard =  6.45 feet  
 Basin Area at Top of Freeboard =  1.26 acres  
 Basin Volume at Top of Freeboard =  3.44 acre-ft

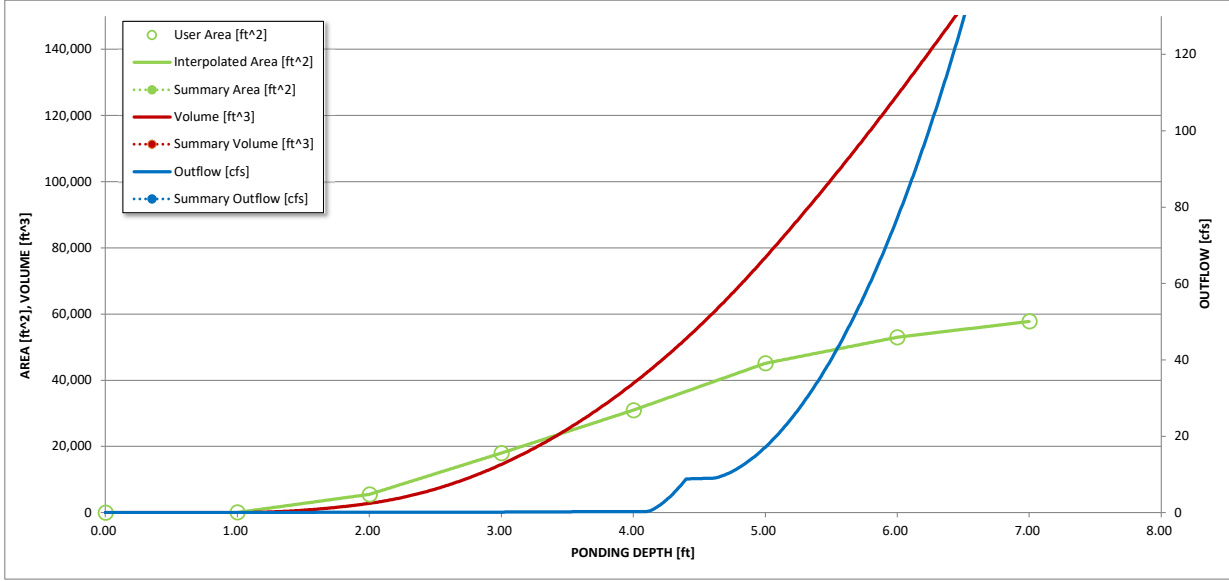
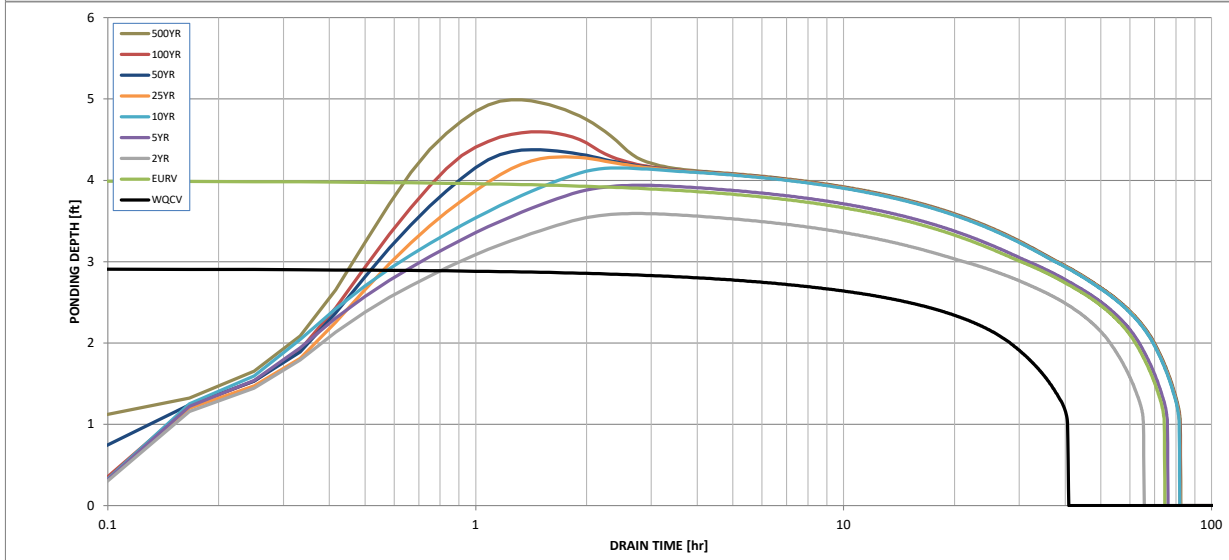
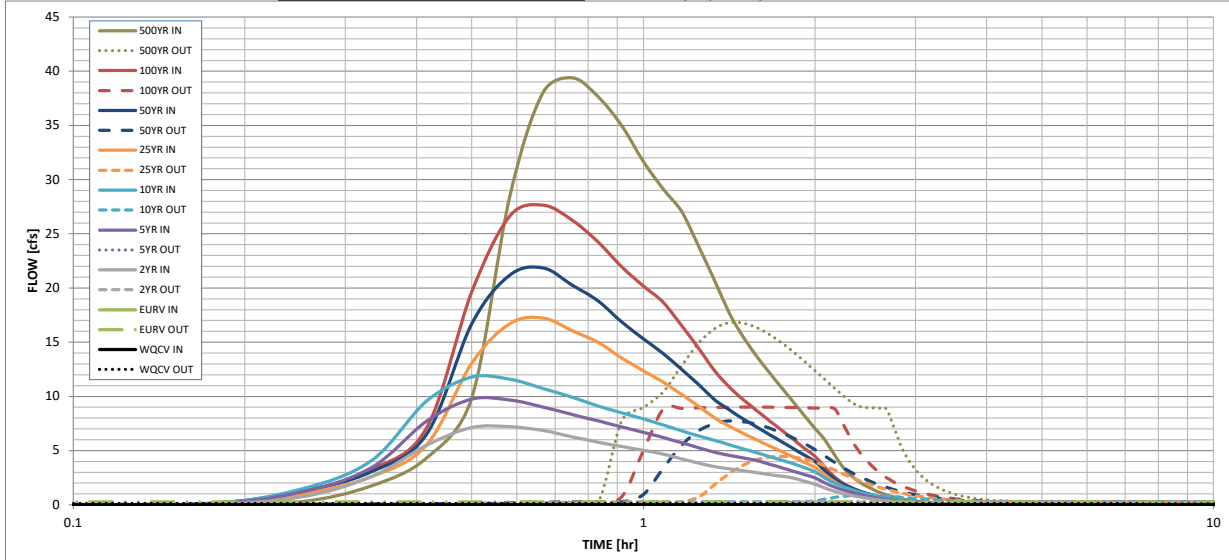
**Routed Hydrograph Results**

*The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).*

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.14
CUHP Runoff Volume (acre-ft) =	0.298	0.890	0.675	0.905	1.089	1.420	1.743	2.164	3.074
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.675	0.905	1.089	1.420	1.743	2.164	3.074
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.1	0.2	0.3	2.7	5.4	9.1	16.6
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.01	0.01	0.02	0.14	0.28	0.47	0.86
Peak Inflow Q (cfs) =	N/A	N/A	7.2	9.7	11.8	17.2	21.9	27.6	39.4
Peak Outflow Q (cfs) =	0.1	0.3	0.2	0.3	0.9	4.5	7.7	9.0	16.8
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.3	2.9	1.6	1.4	1.0	1.0
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	N/A	0.1	0.4	0.6	0.8	0.8
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	68	61	70	75	73	72	70	66
Time to Drain 99% of Inflow Volume (hours) =	<b>40</b>	72	64	73	79	78	78	77	75
Maximum Ponding Depth (ft) =	2.91	4.00	3.59	3.94	4.15	4.29	4.37	4.59	4.99
Area at Maximum Ponding Depth (acres) =	0.39	0.71	0.59	0.69	0.76	0.80	0.83	0.90	1.03
Maximum Volume Stored (acre-ft) =	0.298	0.897	0.624	0.848	1.007	1.108	1.182	1.373	1.749

# DETENTION BASIN OUTLET STRUCTURE DESIGN

*MHFD-Detention, Version 4.06 (July 2022)*



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			



## **APPENDIX E – REFERENCES**



# Grandview Reserve Master Development Drainage Plan

August 2021

HR Green Project No: 191850

**Prepared For:**

4 SITE INVESTMENTS, LLC  
Mr. Peter Martz or Paul Howard  
1271 Kelly Johnson Blvd., Ste. 100  
Colorado Springs, CO 80920  
719-499-8416

**Prepared By:**

HR Green Development, LLC  
Contact: Chris McFarland, PE  
[cmcfarland@hrgreen.com](mailto:cmcfarland@hrgreen.com)  
720-602-4956

## Subbasin Description

The entire site drains in a south easterly direction and is divided into 8 major drainage basins and a total of 18 subbasins together as described below.

- Subbasin A1 is located in the southwestern corner of the site, to the south and west of MS. The basin drains towards the southeast to proposed detention pond A. Current planning documents call for medium density dwelling units and a small pocket park. The basin is 37.00 acres, with a composite impervious value of 35.22% and runoff rates for the 5 and 100 year of 30.72 cfs and 100.64 cfs respectively. The pond will discharge at predevelopment rates and into MS via the ponds outlet structure.
- Subbasin B1 is located between MS and MST2 to the east of subbasin A1. The basin drains towards the southeast and towards subbasin B2. Current planning documents call for medium density dwelling units and some parkland area. The basin is 37.00 acres, with a composite impervious value of 45.00% and runoff rates for the 5 and 100 year of 29.46 cfs and 97.08 cfs respectively.
- Subbasin B2 is located between MS and MST2 to the northeast of subbasin A1. The basin drains towards the southeast and towards Detention Pond B. Current planning documents call for medium density dwelling units. The basin is 24.89 acres, with a composite impervious value of 43.26% and runoff rates for the 5 and 100 year of 12.02 cfs and 42.26 cfs respectively.
- Subbasin B3 is located between MS and EF and to the northeast of east of basin B2. The existing MST2 tributary runs through the basin. The site drains towards the southeast and towards Detention Pond B. Current planning documents call for high, medium-high, and medium density dwelling units along with a pocket park. The basin is 118.90 acres, with a composite impervious value of 49.42% and runoff rates for the 5 and 100 year of 92.76 cfs and 295.27 cfs respectively.
- Subbasin C1 is located to the northeast of east of basin B1 and the existing MST2 tributary runs through the middle of the basin. The basin drains towards the southeast and towards Detention Pond C. Current planning documents call for an institutional parcel, medium and high density dwelling units and a pocket park. The basin is 77.83 acres, with a composite impervious value of 51.20% and runoff rates for the 5 and 100 year of 77.99 cfs and 238.03 cfs respectively.
- Subbasin D1 is located between MS and MST2 to the east of Basin B3 and adjacent to the MST2 channel. The basin drains towards the southeast and towards drainage basin D2. Current planning documents call for medium density dwelling units along with a pocket park. The basin is 24.33 acres, with a composite impervious value of 53.89% and runoff rates for the 5 and 100 year of 24.15 cfs and 70.07 cfs respectively.
- Subbasin D2 is located between MS and MST2 to the south of basins D1 and B3. The basin drains towards the southwest and towards detention pond D. Current planning documents call for high density dwelling units along with a pocket park and a commercial parcel. The basin is 77.90 acres, with a composite impervious value of 62.10% and runoff rates for the 5 and 100 year of 98.47 cfs and 252.18 cfs respectively.
- Subbasin E1 is located just east of EFT along the northern portion of the site. The basin drains towards the southeast and towards basins F3 and F4. Current planning documents call for low

These basins along with the offsite basins which lie east of Eastonville Road contribute flows onto the site through the major tributaries. Estimate oncoming flows for each tributary are as follows:

Offsite Flow Summary		
Tributary	5 Year Peak Runoff (cfs)	100 Year Peak Runoff (cfs)
Main Stem	36	628
Main Stem Tributary 2	8	122
East Fork Tributary*	56	116
East Fork*	175	357

\*Flows from Gieck Ranch  
 DBPS, Oct 2010

As hydraulic analysis continues for the channels, these offsite flows will be used to size the channels for proper conveyance of the flow however it should be noted that the flows mentioned for the Main Stem and Main Stem Tributary 2 assume proper conveyance of the flow through (below or above) Eastonville Road. Due to the unknown nature of these conditions at the time of buildout, a probable scenario of the split flows will require analysis and agreed upon flow rates to each channel will be required. Currently some of the flow shown going to the Main Stem Tributary 2 may be diverted into the Main Stem. Previous analysis done by JR Engineering assumed approximately 160 additional cfs going to the Main Stem Tributary #2 during the 100-year event and as such it is recommended the following flows be used for analysis of the oncoming offsite flows:

Revised Offsite Flow Summary		
Tributary	5 Year Peak Runoff (cfs)	100 Year Peak Runoff (cfs)
Main Stem**	67	413
Main Stem Tributary 2**	59	280
East Fork Tributary*	61	217
East Fork*	180	595

\*Flows from Gieck Ranch  
 DBPS, Oct 2010

\*\*Flows from 4 Way Ranch LOMR, Mar 2004

Please note that the preliminary drainage reports will be required to reconcile any differences between the various reports done for these channels.

## b. Methodology

Design rainfall was determined utilizing figures from the NOAA Atlas 14, Volume 8, Version 2 to determine the 5-year and 100-year rainfall values for 1, 6 and 24-hour events. The 1-hour rainfall depths are 1.22 and 2.50 in/hr respectively, 6 hour 1.79 and 3.87 in/hr respectively and 2.36 and 4.90 in/hr for the 24 hour event. The rainfall values were then used as inputs into the Colorado Urban Hydrograph Procedure (CUHP) spreadsheets to determine runoff values for both pre-development and post-development site.

CUHP is an evolution of the Snyder unit hydrograph and is calibrated for use along the Colorado Front Range. 1 Hour rainfall amounts are input into the program to produce a storm hyetograph that is then used to calculate a storm hydrograph for each basin depending on the subbasins properties including

slope, length, shape, impervious area, pervious depression storage area, and various infiltration rates. Tabular hydrographs are then computed and can be used in EPA SWMM. The CUHP results are included within Appendix B.

EPA SWMM was used to determine flow routing via the kinematic wave method. Subbasins were routed to their respective design points and detention ponds for both the developed and predeveloped condition to determine peak runoff amounts for the 5-year and 100-year storm events. Information from these models along with information and calculations performed in the Colorado Springs BMP spreadsheets was used to determine pond sizing calculations and release rates.

### c. Basin Hydrology

A summary of the flows for both the predeveloped and developed cases for each basin, subbasin and Pond are found on next page along with the full computation found in Appendix B.

SWMM Basin and Pond Summary						
Basin Description	Basin Area (ac)	% Impervious	5 Year Peak Runoff (cfs)	100 Year Peak Runoff (cfs)	5 Year Pond Volume (ac-ft)	100 Year Pond Volume (ac-ft)
A1	45.38	35.22%	30.72	100.64		
<b>Pond A</b>					1.83	3.50
B1	37.00	45.00%	29.46	97.08		
B2	24.89	43.26%	12.02	42.26		
B3	118.90	49.42%	92.76	295.27		
<b>Pond B</b>					5.90	19.00
C1	77.83	51.20%	77.99	238.03		
<b>Pond C</b>					3.91	6.87
D1	24.33	44.14%	24.15	70.07		
D2	77.90	62.10%	98.47	252.18		
<b>Pond D</b>					6.61	10.19
E1	88.60	19.54%	46.88	178.04		
<b>Pond E</b>					1.96	2.44
F1	33.73	25.00%	16.28	58.95		
F2	67.64	51.39%	60.11	170.90		
F3	12.84	45.00%	11.36	32.93		
F4	51.81	46.54%	42.32	124.89		
<b>Pond F</b>					7.38	12.62
G1	20.13	36.52%	13.78	43.95		
G2	15.14	25.00%	6.55	23.95		
<b>Pond G</b>					0.72	2.03
H1	20.71	24.49%	5.68	27.62		
H2	18.55	43.68%	16.24	47.62		
H3	6.01	40.57%	5.21	15.60		
H4	27.65	38.24%	20.93	64.71		
<b>Pond H</b>					2.93	6.17

## IV. Hydraulic Analysis

### a. Major Drainageways

In general, the site runoff flows towards the 4 major drainageways and in a southeasterly direction. These basins are described in more detail below:

#### **Main Stem**

The Main Stem (MS) is in the southwestern portion of the site. Offsite flows collect and are conveyed under Eastonville Road via a culvert. MS travels in a southeasterly direction and combines with the Main Stem Tributary #2 (MST2) just off site where it is then conveyed past Highway 24 via a culvert. An existing breached stock pond exists in the approximate center point of the channel within the site. Jurisdictional wetlands exist within this channel and the area is within a Zone A floodplain towards the southern portion of the site. This channel sees only intermittent flows at this time however once development occurs there may be a more constant baseflow.

Proposed improvements for MS include the removal of the breached stock pond berm and regrading of the affected stretch of channel to restore its historic state. Proposed flow rates through MS are not to exceed historic flowrates and as such, the remainder of the channel is to remain in its current state sans any preemptive check structures; modeling indicates the channel shall remain stable despite the removal of the existing berm.

#### **Main Stem Tributary #2**

MST2 crosses Eastonville road via an existing culvert and flows through the site in a southeasterly direction. Portions of this channel are within a mapped floodplain as shown in the existing FIRM Panel. Per a July email from the USACE this drainage channel was preliminarily determined to be a non-jurisdictional waters/wetland.

Proposed improvements for MST2 include the realignment of the channel, generally shifting the channel towards the west to accommodate the proposed land plan. There is to be a dedicated 100' corridor in which the valley will meander. Preliminary analysis indicates the valley will have an average width of approximately 63' at the elevation necessary to meet freeboard requirements; initial sizing approximates the bankfull width to be 6.8'. The valley and channel thalweg will generally follow the same profile, with some deviation as the bankfull channel meanders through the valley in turn decreasing the low flow channels average slope. The average valley profile is to be approximately 1% with a series of grade control structures to both decrease elevation and dissipate energy to meet natural channel criteria as outline in El Paso County criteria and agreed upon channel parameters.

#### **East Fork Tributary**

The East Fork tributary (EFT) crosses the north property line and flows are conveyed through the site via a natural channel. The channel has been mapped as a Zone A floodplain per the existing FIRM panel; it appears any hydraulic effects of the crossing at Eastonville Road was not accounted for in the floodplain delineation. While the current floodplain delineation shows the channel continuing through Highway 24, there is no existing crossing for this section of the drainage channel below Highway 24 and instead the flows are conveyed to the northeast towards the East Fork Upper (EF). Per a July email from the USACE this drainage channel was preliminarily determined to be a non-jurisdictional waters/wetland.

Per SWMM modeling the current velocities will require channel stabilization. The EFT channel is to be engineered later in the design which will likely include a combination of channel widening, lowering of slope facilitated by the implementation of drop structures to meet non erosive velocity requirements. Bank stabilization, should it be necessary, may include coir rolls, erosion control blankets, live willow staking, soil lifts and/or other measures to ensure successful bank stabilization. The drainageway will require further analysis and design which will be completed as the project progresses.

### **East Fork Upper**

The EF crosses the north property line approximately 1500' east of the EFT crossing. The flow through the site is via a natural channel and travels in a southeasterly direction. The channel is mapped as a Zone A floodplain, and the channel crosses Highway 24 via an existing shallow bridge. The current floodplain delineation shows EF and EFT eventually merging approximately 1750' southeast of the site, however, as mentioned above Highway 24 blocks the flow of the EFT and flows are conveyed northeast to the EF bridge crossing.

Per SWMM modeling the current velocities will require channel stabilization. The EF channel is to be engineered later in the design which will likely include a combination of channel widening, lowering of slope facilitated by the implementation of drop structures to meet non erosive velocity requirements. Bank stabilization, should it be necessary, may include coir rolls, erosion control blankets, live willow staking, soil lifts and/or other measures to ensure successful bank stabilization. The drainageway will require further analysis and design which will be completed as the project progresses.

## **V. Environmental Evaluations**

### **a. Significant existing or potential wetland and riparian areas impacts**

As part of this work, the developer has engaged Ecosystem Services, LLC (ECOS) to perform environmental studies of the site that will be submitted with the planning documents. Major information from this report related to the wetlands shows that two of the tributaries through the site, the Main Stem and the East Fork contain jurisdictional wetlands and the other two tributaries, the East Fork Tributary and the Main Stem Tributary #2 are non-jurisdictional wetlands.

At this time, only minor improvements to the jurisdictional channels are proposed. These stream improvements will be made with keeping the natural habitat intact and the natural function of these channels as it is to maintain the wetland habitat. The non-jurisdictional channels will be modified, and the design of those channels is forthcoming.

### **b. Stormwater quality considerations and proposed practices**

As part of the development, full spectrum detention facilities will be installed to provide water quality for the development. The facilities will be designed using El Paso County criteria and provide stormwater quality by slowing the release of stormwater captured by the ponds and allowing solids to settle out. Additionally, when possible, the revised drainage channels, which were not jurisdictional wetlands, will be used to convey stormwater via a natural channel. Stormwater must be treated before entering the natural channels. The natural channel will provide a pervious means to transport stormwater and provide some water quality benefits as well.

On site practices for the homes, schools, churches, and other buildings should use means such that impervious areas drain across pervious area to allow for infiltration during the minor events. This would

# Grandview Reserve CLOMR Report

## Introduction

This report was prepared by HR Green to support the submission of MT-2 forms and documents in a request for a Conditional Letter of Map Revision (CLOMR) for channel improvements along Geick Ranch Tributary 2. This request impacts the current delineation of the 100-year boundary on Flood Insurance Rate Maps (FIRMs) 08041C0552G and 08041C0556G.

Grandview Reserve is located in Falcon, Colorado within El Paso County and contains approximately 776 acres within the south half of section 21 and 22 and the north half of section 27 and 28, Township 12 South, and Range 66 West of the Sixth Principal Meridian in El Paso County, Colorado.

Grandview Reserve (GVR) falls within the Geick Ranch Drainage Basin which covers approximately 22 square miles. This drainage basin is tributary to Black Squirrel Creek and joins said creek just to the south of Ellicott, CO about 18 miles to the south. Black Squirrel Creek eventually drains to the Arkansas River in Pueblo Colorado. Much of the Geick Ranch Drainage basin is undeveloped and consists of rural farmland. The Geick Ranch Drainage basin lies north of the Haegler Ranch drainage basin. The channels through the Grandview property can all be described as gently sloping drainages that roll through the site towards the creeks to which they are tributary.

Per the NRCS web soil survey, the site is made up entirely of Type A and B soils. The majority of which are Type B soils. The vegetation found within Grandview Reserve consists of wetland communities in the floodplain with a transitional area to shortgrass prairie communities that dominate the site. The primary species found in the shortgrass prairie regions include little bluestem, blue grama, and buffalograss. The transitional area between the wetlands and shortgrass prairie includes patches of snowberry, and wood's rose. There are a few plains cottonwoods along the main channels. The area has historically been heavily grazed and there are weeds throughout the site. Weeds found onsite include Canada thistle, Russian thistle, common mullein and yellow toadflax spp.

Observations of the existing channels suggest that they are at equilibrium with their watershed flows; evidence including relatively stable bank full channels, adequate floodplain (above bank full channel elevations) and in-tact plant communities that would be expected in this type of reach support the notion that the reach is in equilibrium.

At present, the preliminary analysis and design of Geick Ranch Tributary 2 (GRT2) has been completed. Proposed improvements for Geick Ranch Tributary 2 include refinement of the existing channel alignment and a stabilizing natural stream design that will allow a more predictable floodplain. There is to be a dedicated 100' wide corridor in which the channel valley will meander. The valley is the area needed to fully contain the 100-year event. Preliminary analysis indicates the valley will have an average width of approximately 63'; initial sizing approximates the bank full width to be 8.8' – 13.8'. The valley and channel thalweg will generally follow the same profile, with some deviation as the bank full channel meanders through the valley in turn decreasing the low flow channels average slope. The average valley profile is to be approximately 0.9% with a series of grade control structures to both decrease elevation and dissipate energy to meet natural channel criteria as outlined in El Paso County criteria.

## Hydrology

El Paso County criteria states that all developments are required to detain storm flows down to their historic peaks. For this reason GRT2 has been designed using the flows that drain to it in the existing conditions.



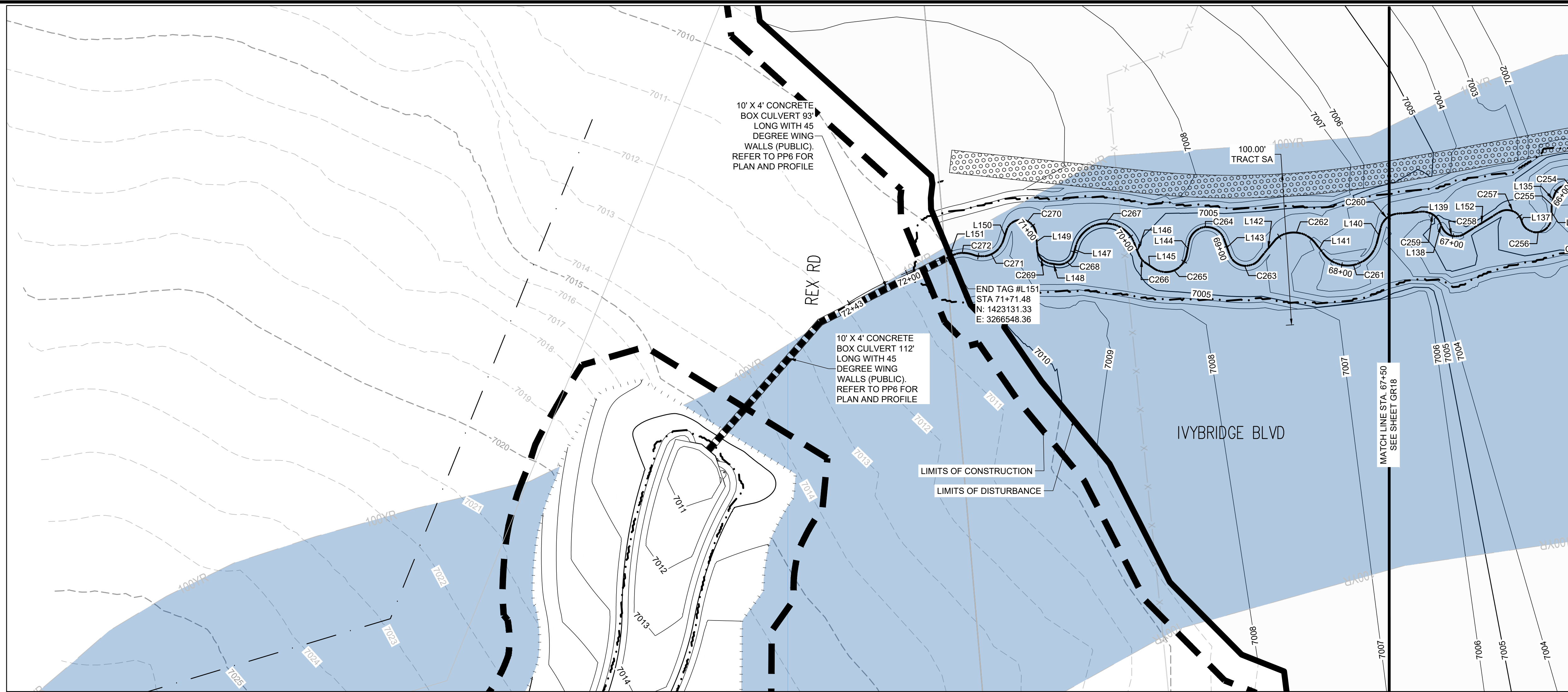








HR GREEN KEY: xg-c-dh01; GRADING KEY: HRG1; 20X; EBase; xv-dgn; 662; xc-dgn; 662; 10; xc-row; F1; 662; 10; xc-row; F2; xv-til; 662; xv-row; 662; xc-dgn; F3; xc-row; F3; xc-row; F4; 01-XC-INITIAL-EC; xc-row; PHG; xc-dgn; PHG; 01-XC-channel; 01\_Xv-Row; 01-XC-PP; 100YR\_FP\_DELINEATION



10' X 4' CONCRETE BOX CULVERT 93 LONG WITH 45 DEGREE WING WALLS (PUBLIC). REFER TO PP6 FOR PLAN AND PROFILE

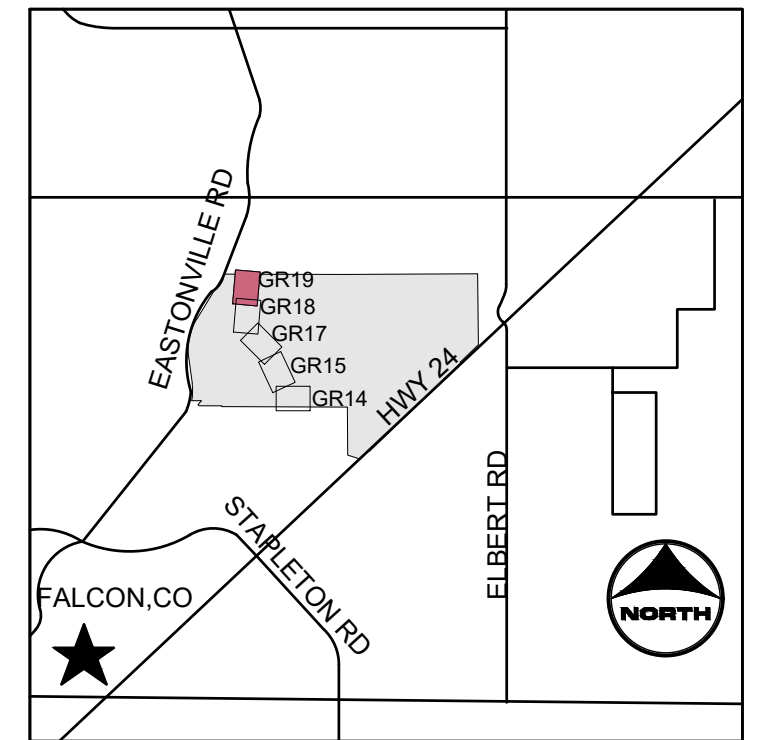
10' X 4' CONCRETE BOX CULVERT 112 LONG WITH 45 DEGREE WING WALLS (PUBLIC). REFER TO PP6 FOR PLAN AND PROFILE

END TAG #L151  
STA 71+71.48  
N: 1423131.33  
E: 3266548.36

MATCH LINE STA. 67+50  
SEE SHEET GR18

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LINE/CHORD DIRECTION	CHORD LENGTH
C260	14.80	12.00	N34° 56' 39.85"W	13.88
L140	10.77		N70° 15' 59.85"W	
C261	44.55	20.00	N6° 27' 33.85"W	35.89
L141	8.98		N57° 20' 52.15"E	
C262	29.24	18.00	N10° 48' 15.97"E	26.13
L142	16.23		N45° 31' 56.12"W	
C263	24.34	11.44	N9° 01' 06.38"E	20.00
L143	11.94		N74° 50' 42.15"E	
C264	31.22	12.00	N0° 18' 44.36"E	23.13
L144	8.82		N74° 13' 13.43"W	
C265	27.28	14.00	N18° 24' 07.71"W	23.16
L145	7.63		N38° 57' 54.40"E	
C266	5.69	10.00	N55° 15' 07.16"E	5.61
L146	6.73		N71° 32' 18.85"E	
C267	53.21	20.40	N5° 38' 58.70"E	39.35
L147	5.41		N68° 39' 59.99"W	
C268	14.28	10.00	N27° 44' 59.82"W	13.10
L148	4.50		N13° 10' 00.36"E	
C269	13.50	10.00	N51° 50' 51.43"E	12.50
L149	10.57		S89° 28' 17.51"E	
C270	27.53	10.00	N11° 39' 24.20"E	19.62

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LINE/CHORD DIRECTION	CHORD LENGTH
L150	7.19		N67° 12' 54.10"W	
C271	24.50	15.00	N20° 25' 45.98"W	21.86
C272	14.98	18.00	N2° 31' 05.79"E	14.55
L151	3.20		N21° 19' 11.62"W	



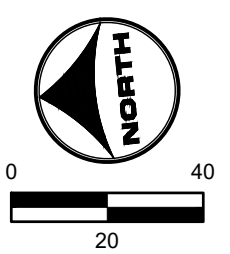
**KEYMAP**

**PROJECT LEGEND:**

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
- SECTION LINE
- EXISTING EASEMENT
- X-X-X- EXISTING FENCE
- 5250--- PROPOSED MAJOR CONTOUR
- 5250--- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- EXISTING MINOR CONTOUR
- CONSTRUCTION EASEMENT
- FLOW ARROW
- LIMITS OF CONSTRUCTION
- LIMITS OF DISTURBANCE
- PROPOSED 100-YR FLOODPLAIN
- EFFECTIVE 100-YR FLOODPLAIN
- EXISTING WETLANDS - JURISDICTIONAL
- EXISTING WETLANDS - NONJURISDICTIONAL
- PROPOSED MAINTENANCE TRAIL

**NOTES:**

- BASIS OF BEARINGS:** THE EAST LINE OF SECTION 21, BEING MONUMENTED AT THE SOUTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED AND BEING MONUMENTED AT THE NORTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, BEING ASSUMED TO BEAR NORTH 00 DEGREES 52 MINUTES 26 SECONDS WEST, A DISTANCE OF 5290.17 FEET.
- BENCHMARK:**  
DESIGNATION = F 24  
PID = JK0240  
DESCRIPTION = DISK ON TOP OF CONCRETE MONUMENT  
CONTROL POINT COORDINATE SYSTEM:  
NAVD88  
NORTHING: 1421049.80  
EASTING: 3273631.55  
ELEVATION: 6866.33



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811 OR 1-800-922-1987  
Utility Notification Center of Colorado

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NO.	DATE	BY	REVISION DESCRIPTION

**HRGreen**  
HR GREEN - DENVER  
5613 DTC PARKWAY SUITE 950  
DENVER CO 80111  
PHONE: 720.602.4999  
FAX: 713.965.0044

**GRANDVIEW RESERVE**  
**DR HORTON**  
FALCON, COLORADO

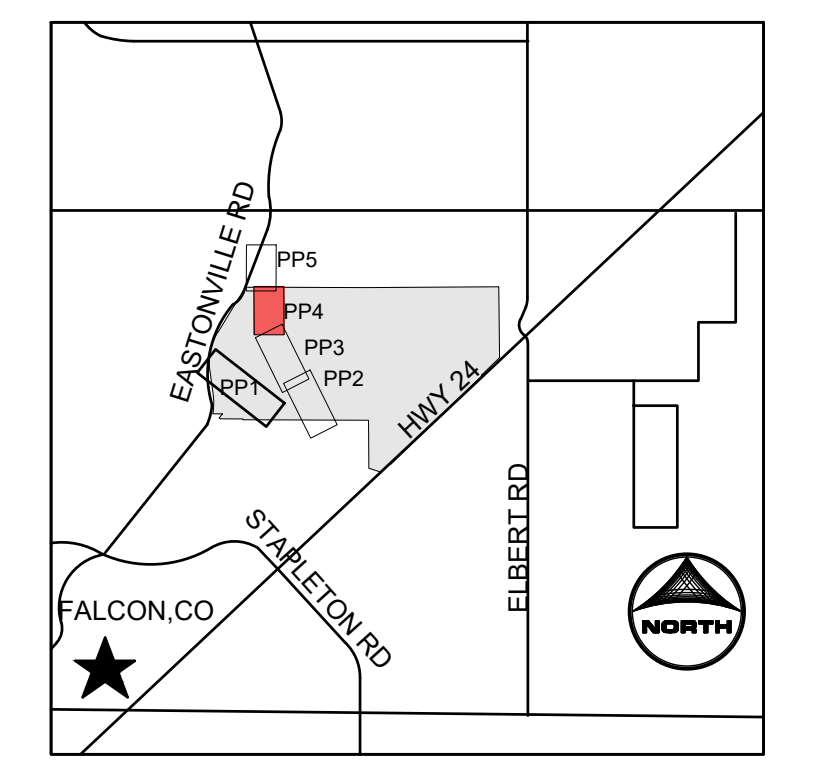
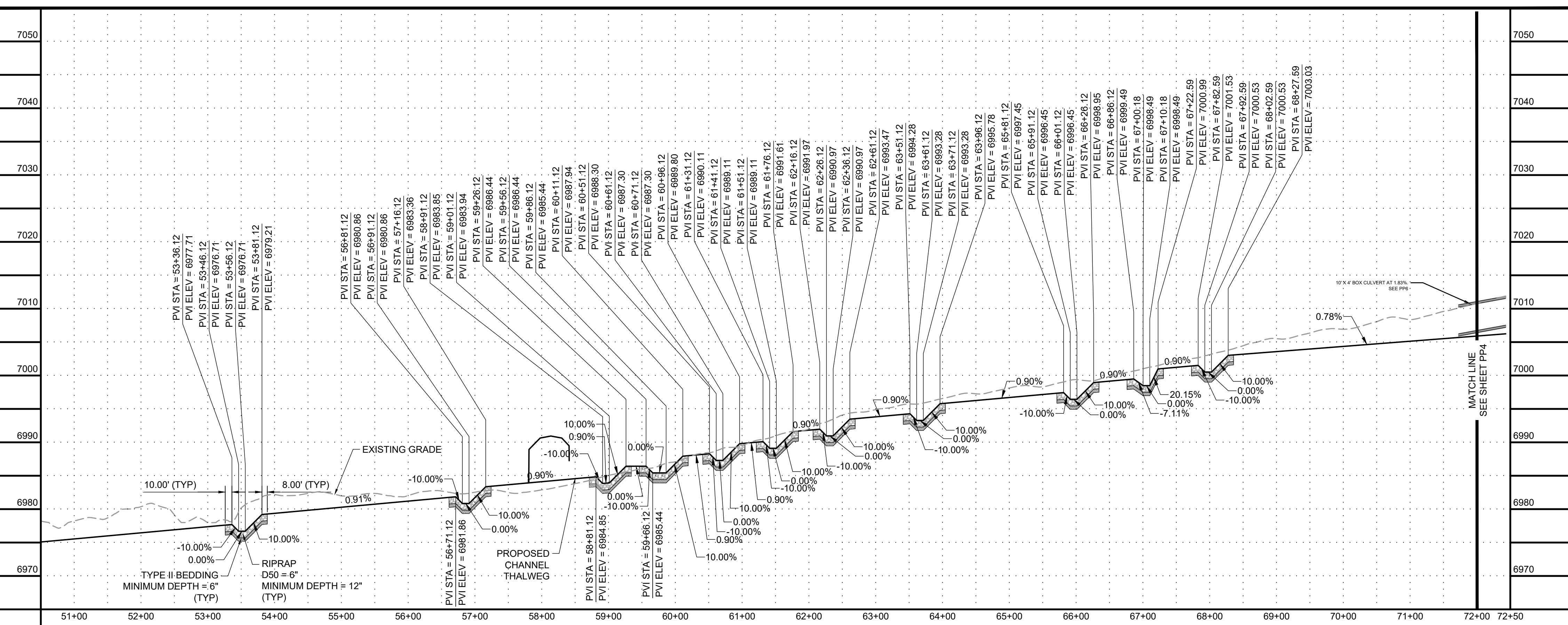
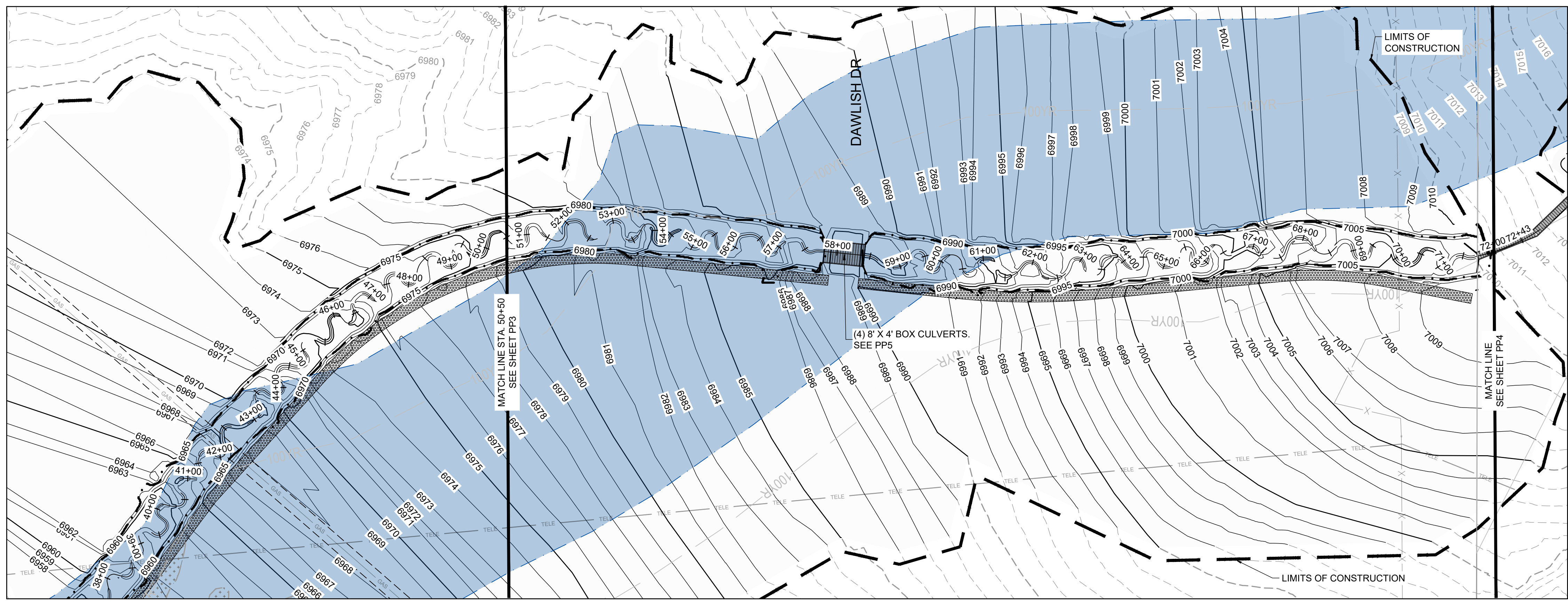
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SHEET  
**GR19**  
23



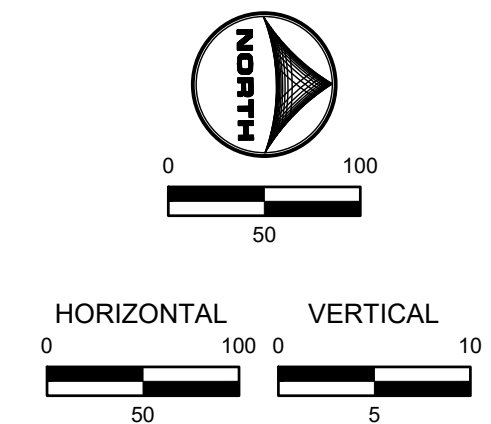


HR GREEN - DENVER 5613 DTC PARKWAY SUITE 950 DENVER CO 80111 PHONE: 720.602.4999 FAX: 713.965.0044  
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- KEYMAP**
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  - 5250 PROPOSED MAJOR CONTOUR
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  2. BENCHMARK: DESIGNATION = F 24  
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CONTROL POINT COORDINATE SYSTEM: NAVD88  
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EASTING: 3273631.55  
ELEVATION: 6866.33
  3. ALIGNMENT NOT FOR USE IN CONSTRUCTION. REFER TO NORTHINGS AND EASTINGS
  4. PLAN SET APPROVAL APPLIES TO THE LIMITS OF CONSTRUCTION. PLEASE SEE GRANDVIEW FILING NUMBER 1 PLAN SET FOR GRADING OUTSIDE OF THE CHANNEL LIMITS OF CONSTRUCTION.
  5. TRIBUTARY 1 TO BE CONSTRUCTED WITH FILING 1 (AREA SOUTH OF TRIBUTARY 1), TRIBUTARY 2 TO BE CONSTRUCTED WITH FILING 2 (AREA NORTH OF TRIBUTARY 1 AND SOUTHWEST OF TRIBUTARY 2), BOX CULVERTS/CROSSINGS ALONG TRIBUTARY 2 TO BE CONSTRUCTED WITH TRIBUTARY IMPROVEMENTS.



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 Utility Notification Center of Colorado

**FLORIDA LICENSED PROFESSIONAL ENGINEER**  
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 Greg L. Parks

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NO.	DATE	BY	REVISION DESCRIPTION

**HRGreen**  
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 5613 DTC PARKWAY SUITE 950  
 DENVER CO 80111  
 PHONE: 720.602.4999  
 FAX: 713.965.0044

**GRANDVIEW RESERVE**  
**DR HORTON**  
 FALCON, COLORADO

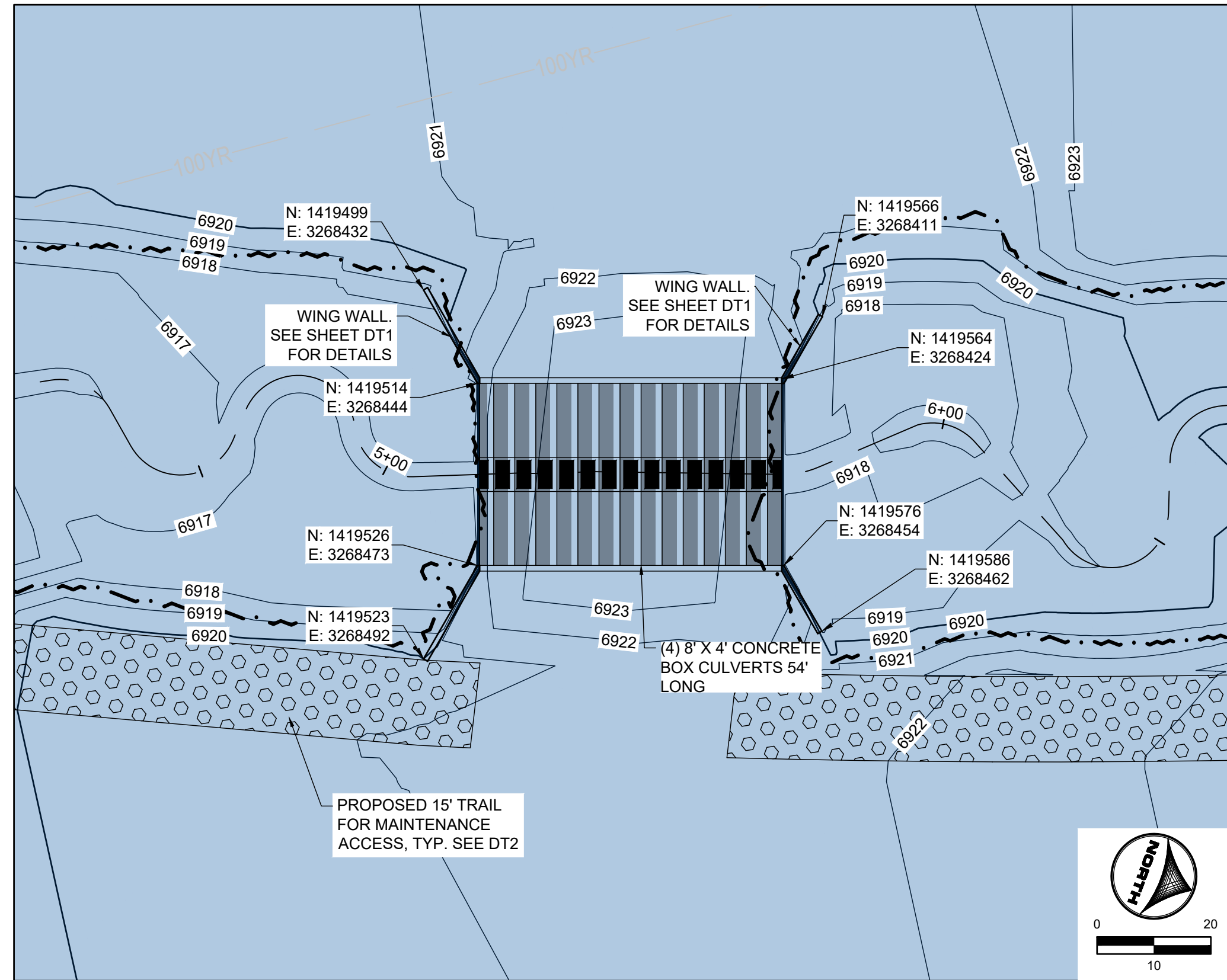
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SHEET PP4 33

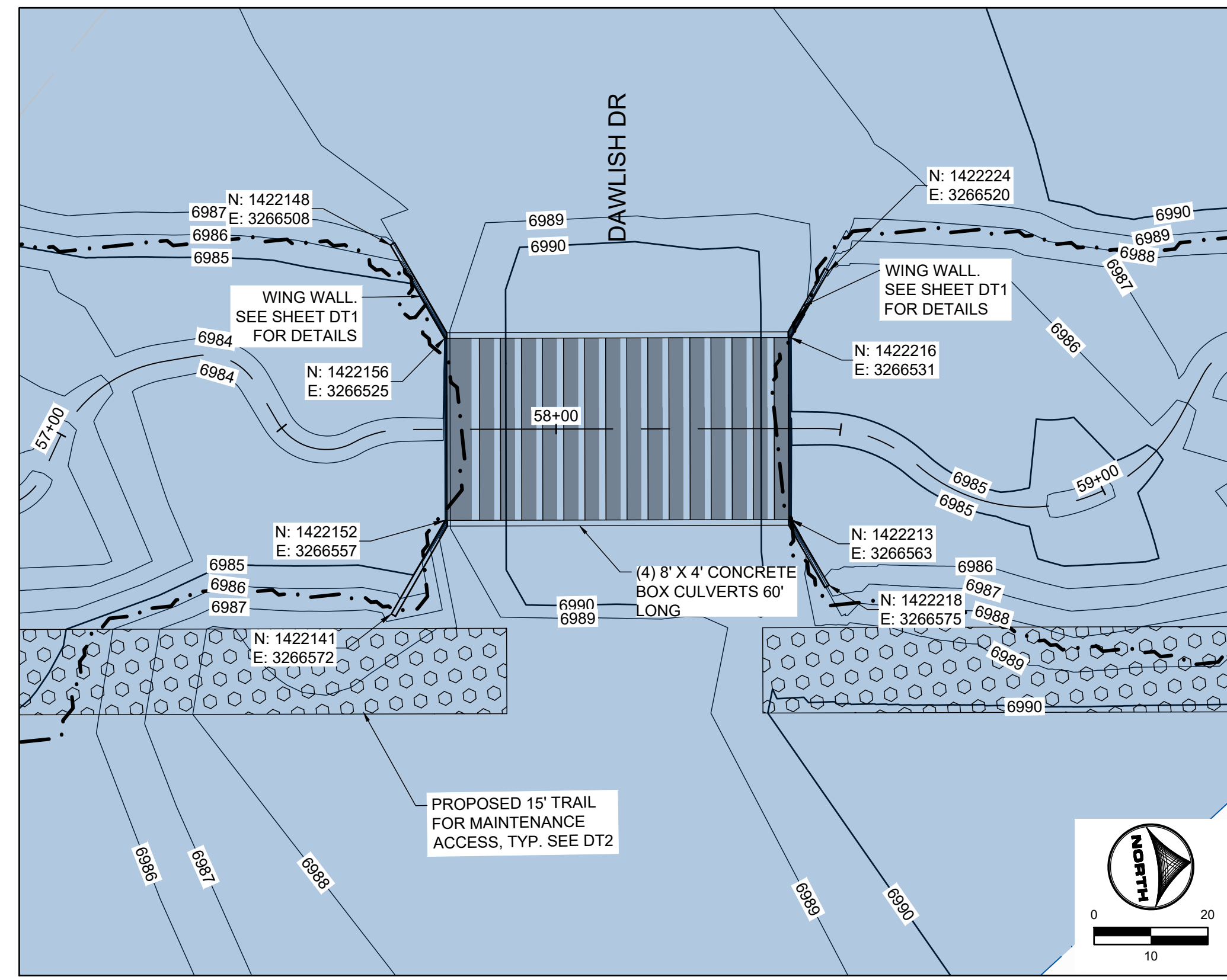




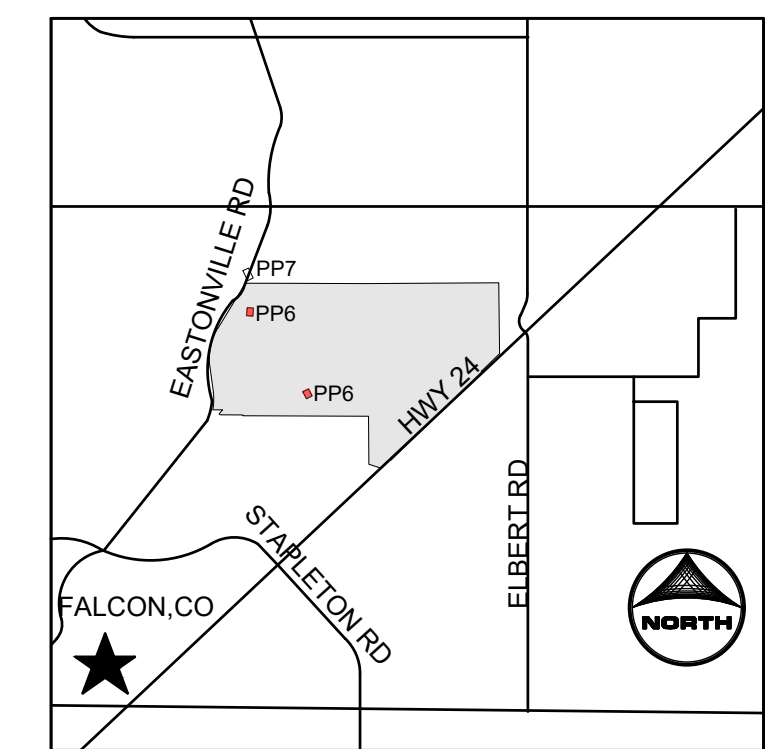
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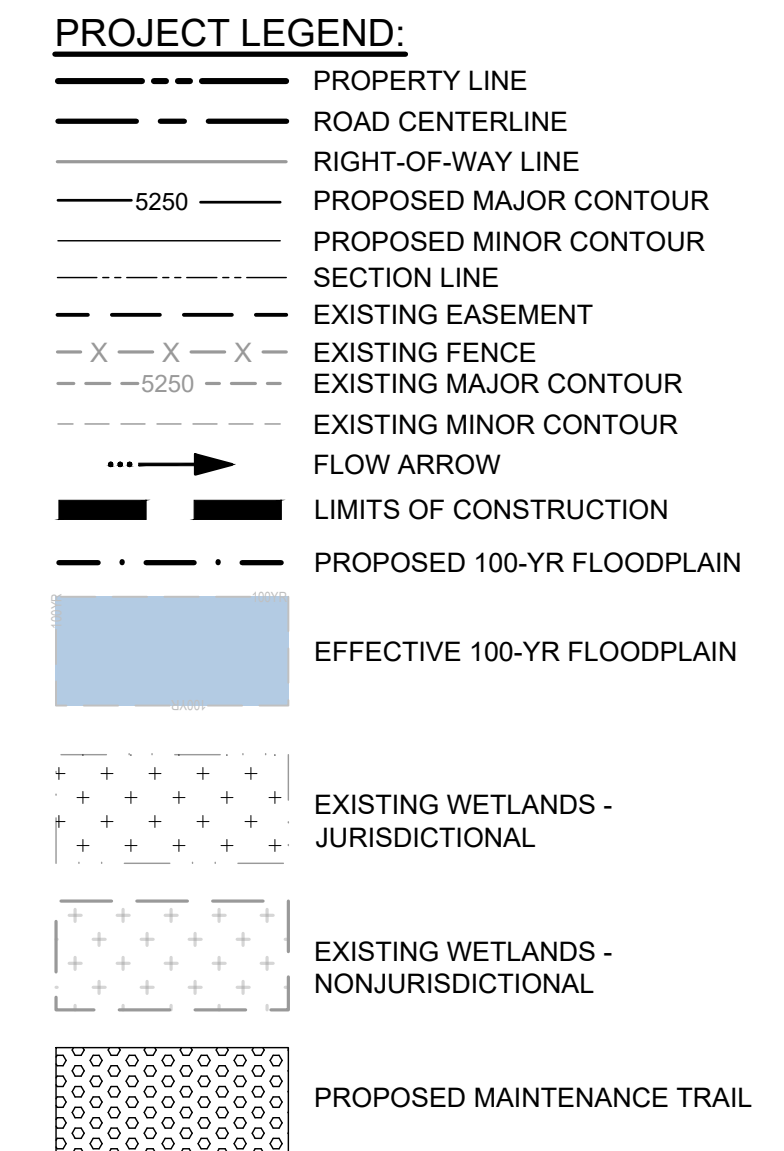
ROAD G CULVERT CROSSING



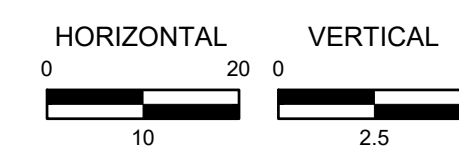
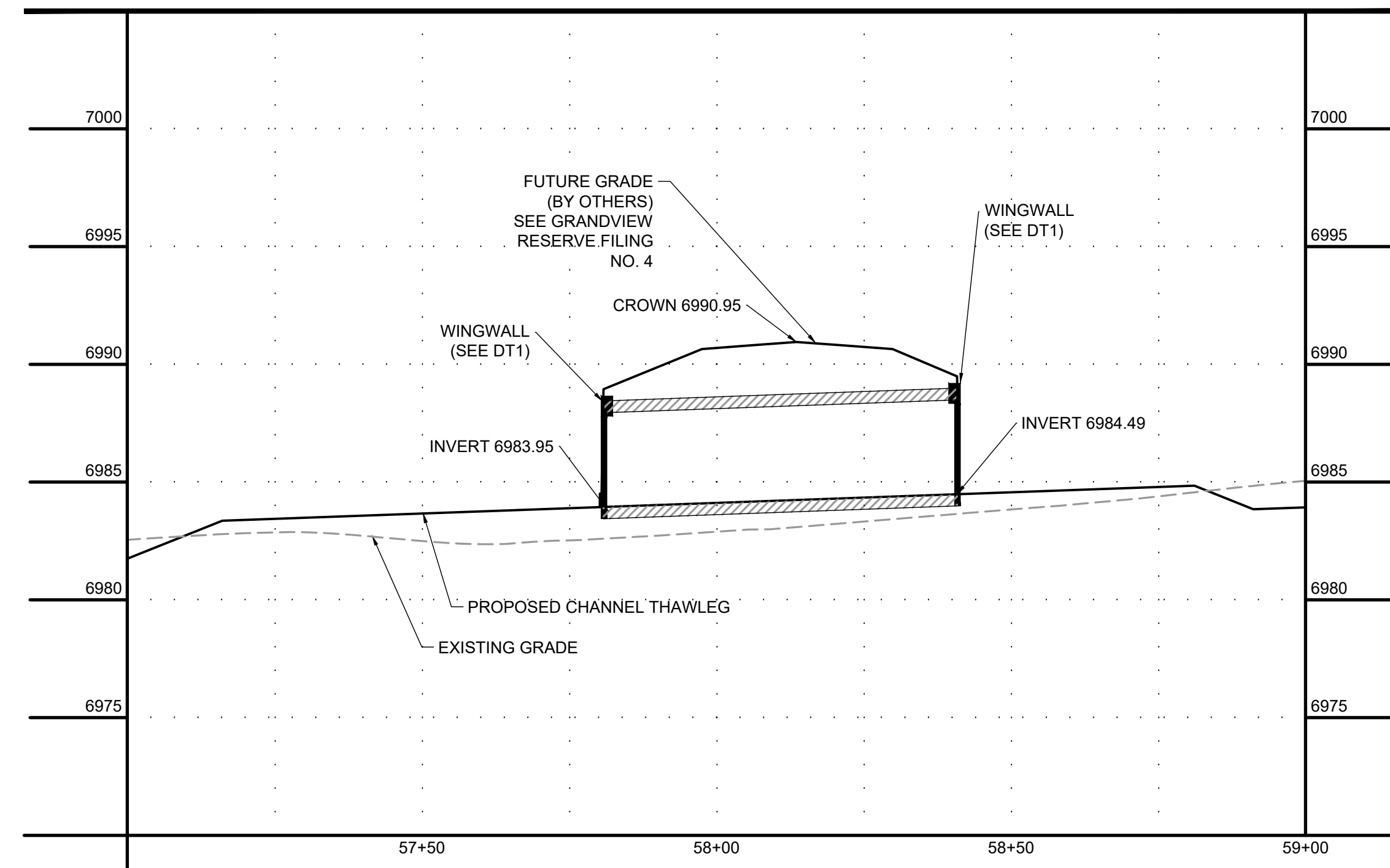
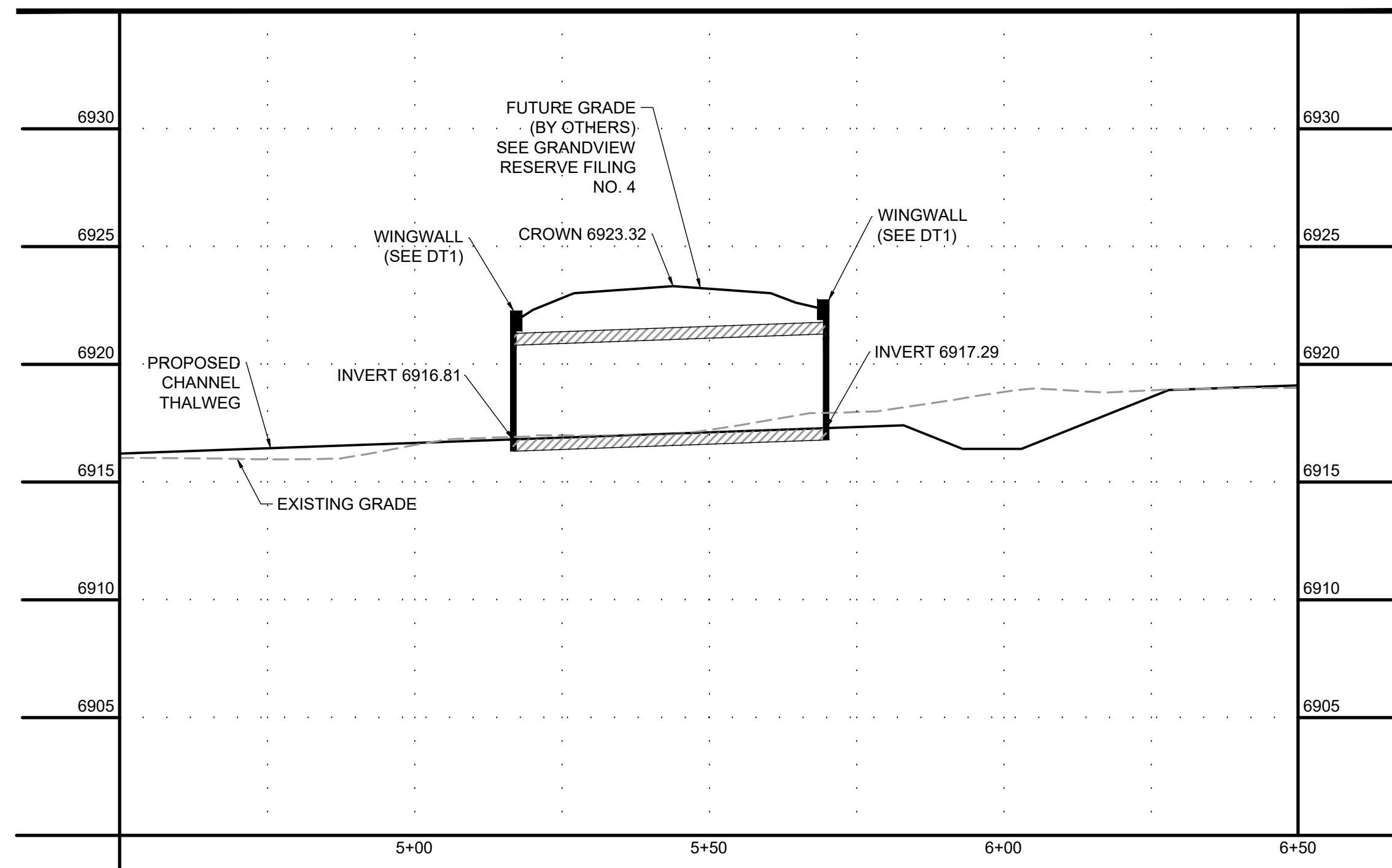
DAWLISH DR CULVERT CROSSING



KEYMAP



- NOTES:**
- BASIS OF BEARINGS: THE EAST LINE OF SECTION 21, BEING MONUMENTED AT THE SOUTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, AND BEING MONUMENTED AT THE NORTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, BEING ASSUMED TO BEAR NORTH 00 DEGREES 52 MINUTES 26 SECONDS WEST, A DISTANCE OF 5290.17 FEET.
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CAD FILE: J:\2020\201662.03\CAD\DWGS\CIP\PLAN AND PROFILE		

NO.	DATE	BY	REVISION DESCRIPTION

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 HR GREEN - DENVER  
 5613 DTC PARKWAY SUITE 950  
 DENVER CO 80111  
 PHONE: 720.602.4999  
 FAX: 713.965.0044

**GRANDVIEW RESERVE**  
**DR HORTON**  
 FALCON, COLORADO

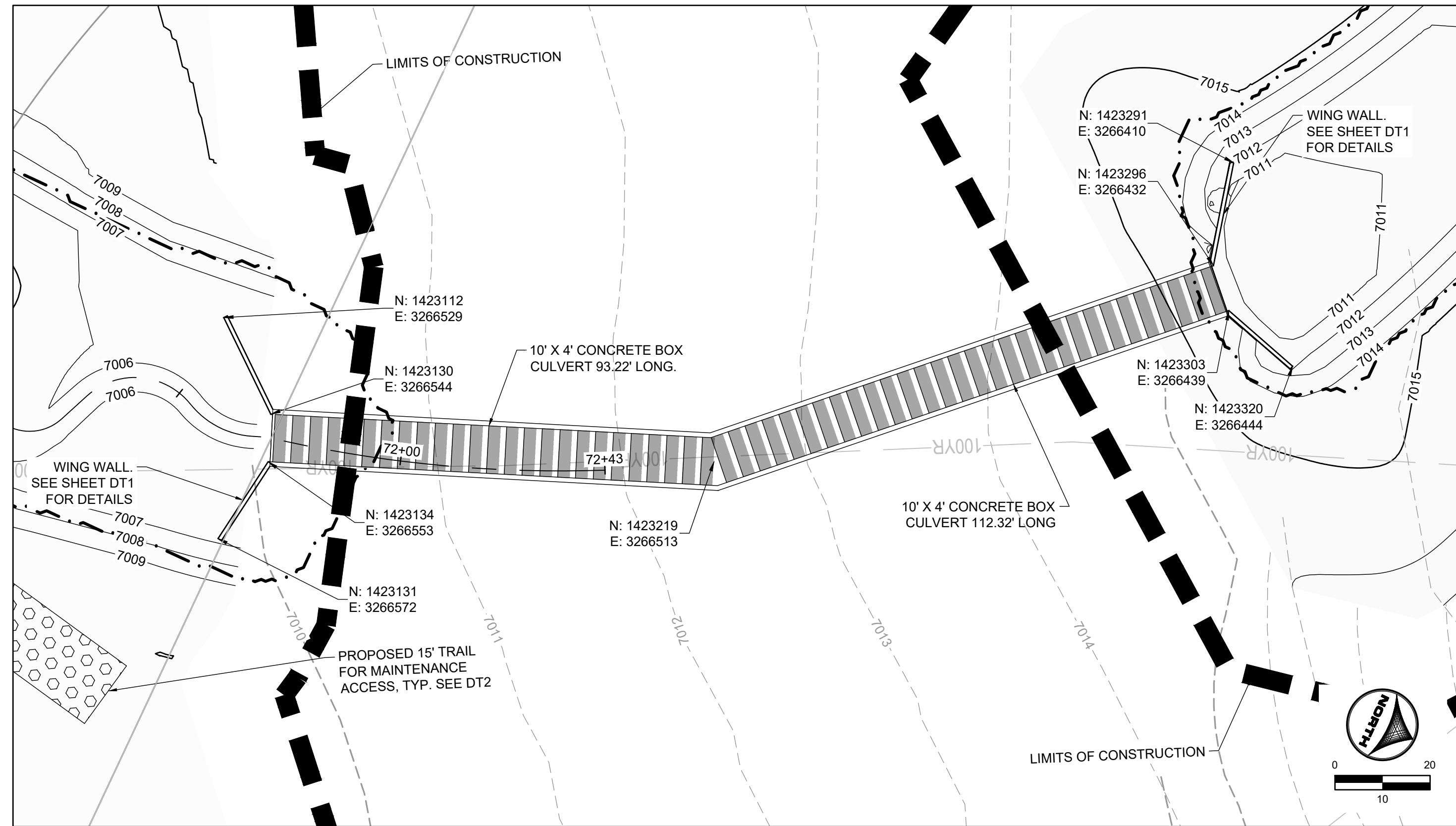
CONSTRUCTION DOCUMENTS  
 DRAINAGE CULVERTS PLAN AND PROFILE

SHEET  
**PP6**  
 35

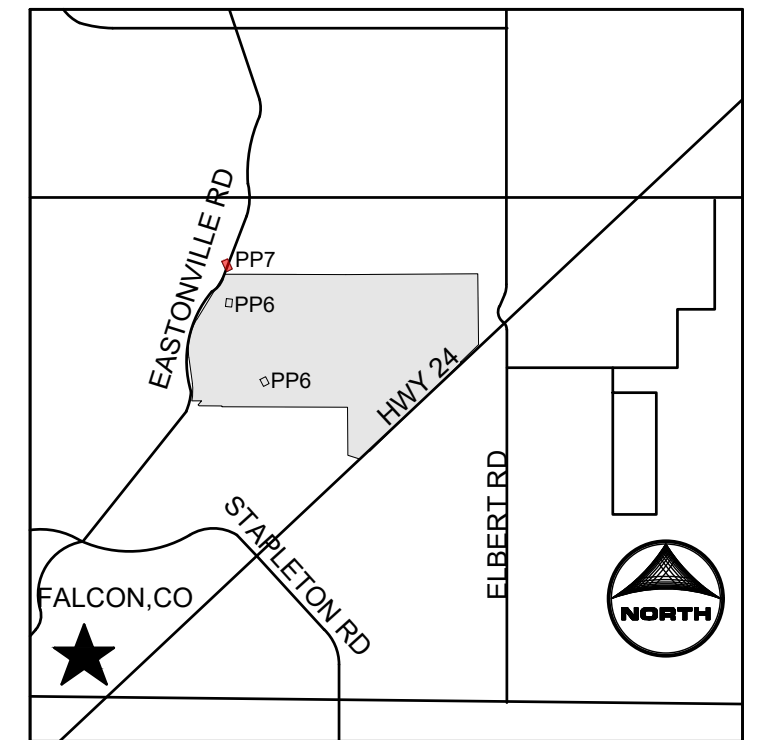
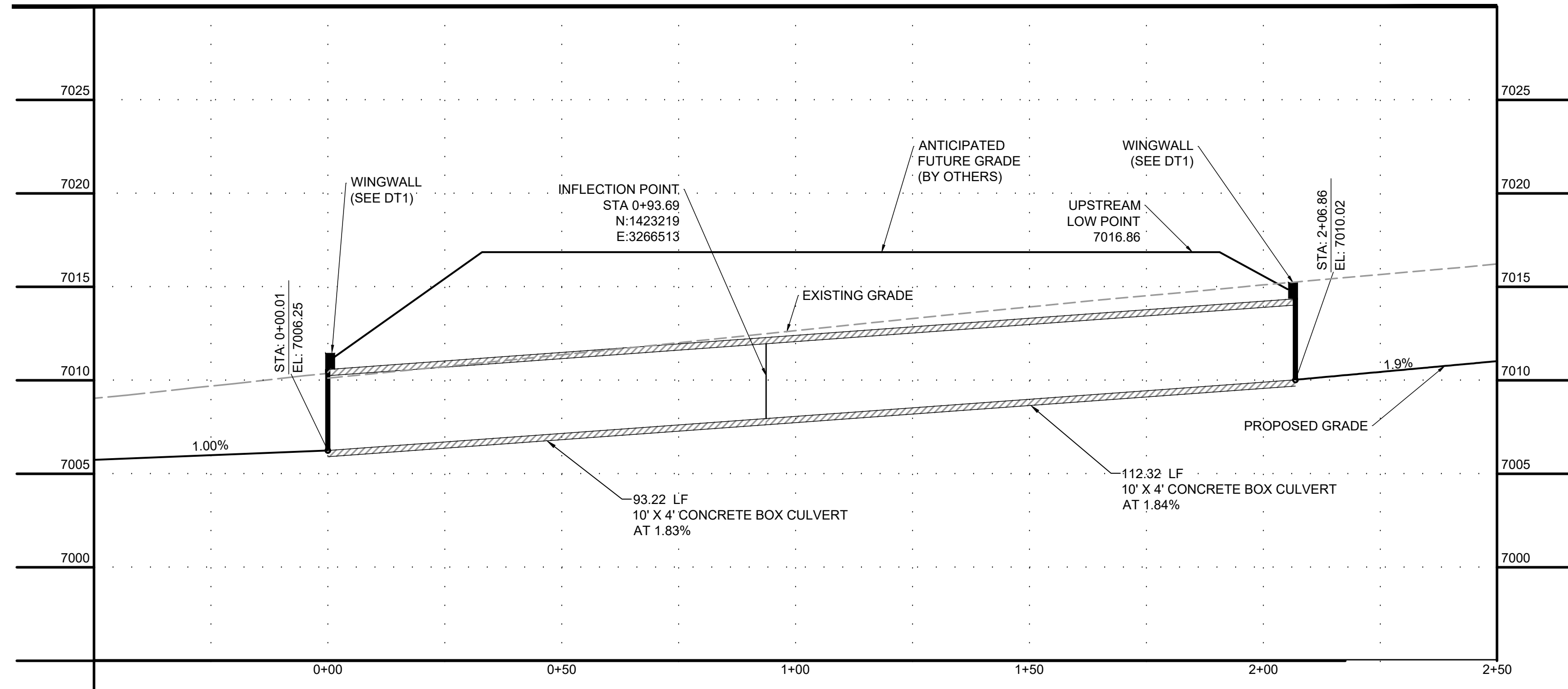
**811** UNCC CALL BEFORE YOU DIG  
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 Utility Notification Center of Colorado

PROFESSIONAL ENGINEER  
 37081  
 7-25-22

HR GREEN Xref: xgs-c-dh01; P&P KEY: HRG X\_EBaser; 01-XC\_Wingwalls; x-dsgr; 662; xc-dsgr-662.10; xc-dsgr-F1-662.10; xc-dsgr-F1-662.10; xc-dsgr-F2; xc-dsgr-F2; xc-dsgr-F2; xc-dsgr-F3; xc-dsgr-F3; xc-dsgr-F4; xc-dsgr-F4; 01-XC-PPR\_100YR\_FP\_DELININATION; xc-channel; xc-filing-bnchs; 01-XC-INITIAL-EC; xc-row-PH2; xc-row-PH2; 01-XC-Channel



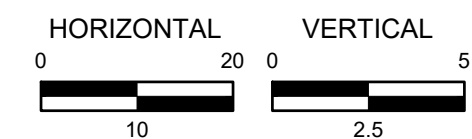
REX ROAD CULVERT CROSSING



**PROJECT LEGEND:**

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- SECTION LINE
- EXISTING EASEMENT
- EXISTING FENCE
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- FLOW ARROW
- LIMITS OF CONSTRUCTION
- PROPOSED 100-YR FLOODPLAIN
- EFFECTIVE 100-YR FLOODPLAIN
- EXISTING WETLANDS - JURISDICTIONAL
- EXISTING WETLANDS - NONJURISDICTIONAL
- PROPOSED MAINTENANCE TRAIL

- NOTES:**
- BASIS OF BEARINGS: THE EAST LINE OF SECTION 21, BEING MONUMENTED AT THE SOUTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, AND BEING MONUMENTED AT THE NORTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, BEING ASSUMED TO BEAR NORTH 00 DEGREES 52 MINUTES 26 SECONDS WEST, A DISTANCE OF 5290.17 FEET.
  - BENCHMARK: DESIGNATION = F 24  
PID = JK0240  
DESCRIPTION = DISK ON TOP OF CONCRETE MONUMENT  
CONTROL POINT COORDINATE SYSTEM: NAVD88  
NORTHING: 1421049.80  
EASTING: 3273631.55  
ELEVATION: 6866.33
  - ALIGNMENT NOT FOR USE IN CONSTRUCTION. REFER TO NORTHINGS AND EASTINGS
  - PLAN SET APPROVAL APPLIES TO THE LIMITS OF CONSTRUCTION. PLEASE SEE GRANDVIEW FILING NUMBER 1 PLAN SET FOR GRADING OUTSIDE OF THE CHANNEL LIMITS OF CONSTRUCTION.
  - TRIBUTARY 1 TO BE CONSTRUCTED WITH FILING 1 (AREA SOUTH OF TRIBUTARY 1), TRIBUTARY 2 TO BE CONSTRUCTED WITH FILING 2 (AREA NORTH OF TRIBUTARY 1 AND SOUTHWEST OF TRIBUTARY 2), BOX CULVERTS/CROSSINGS ALONG TRIBUTARY 2 TO BE CONSTRUCTED WITH TRIBUTARY IMPROVEMENTS.



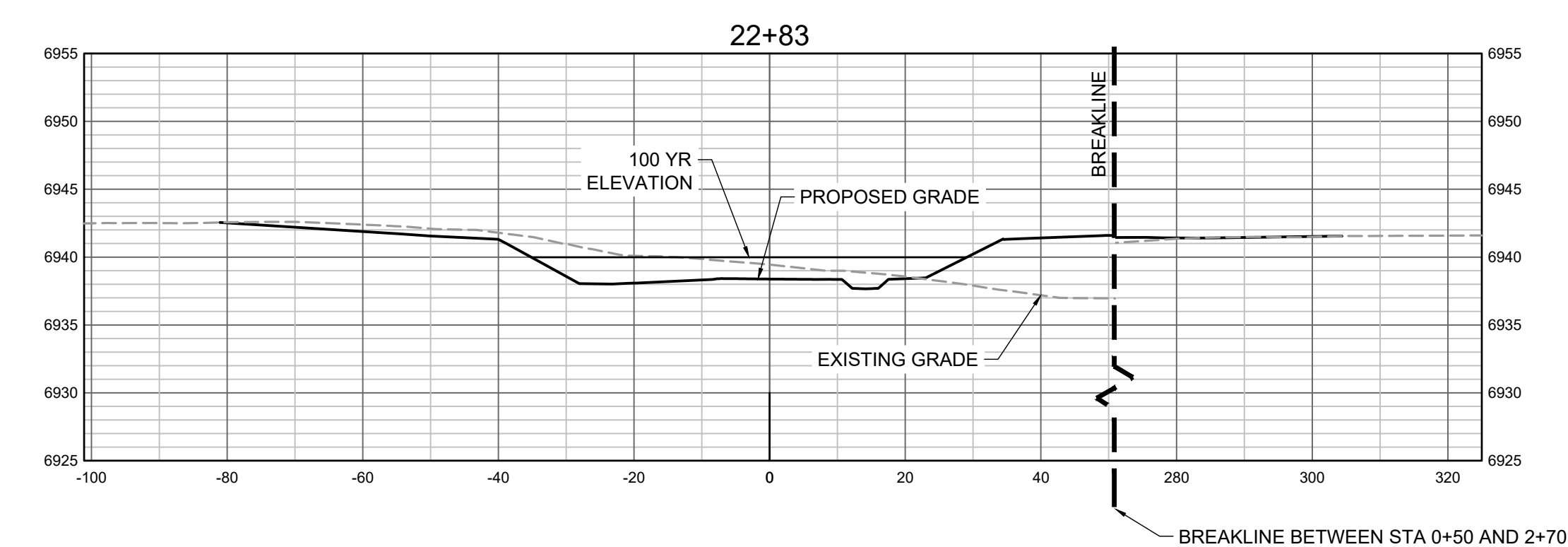
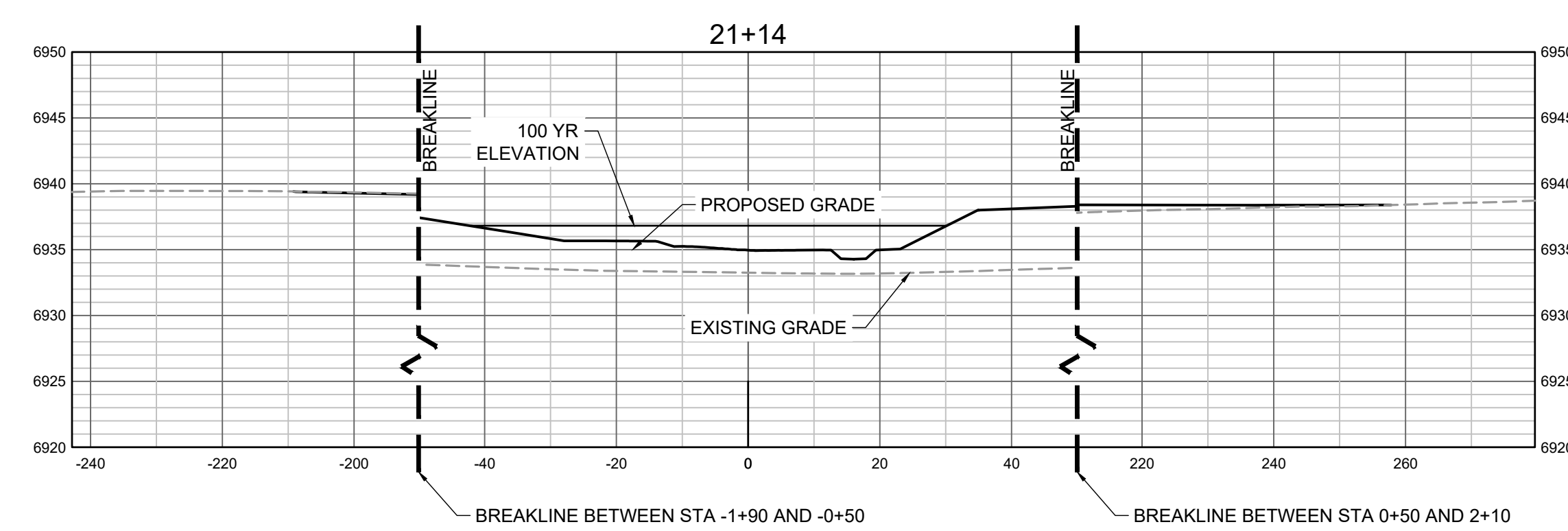
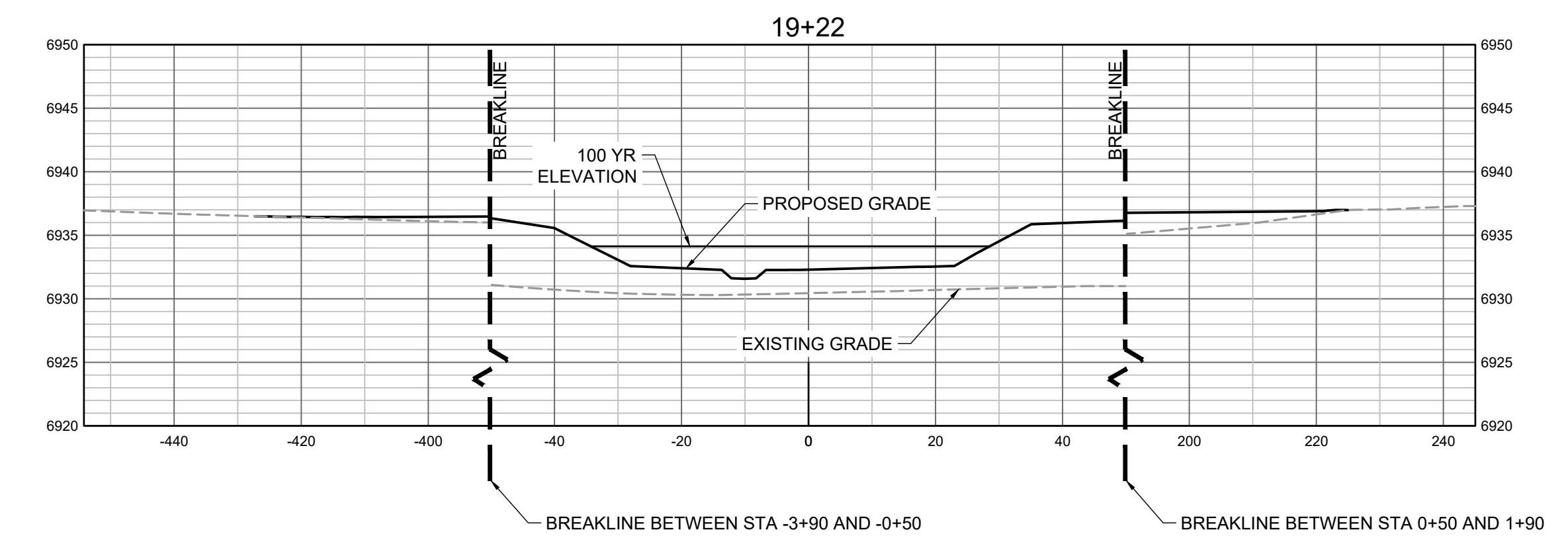
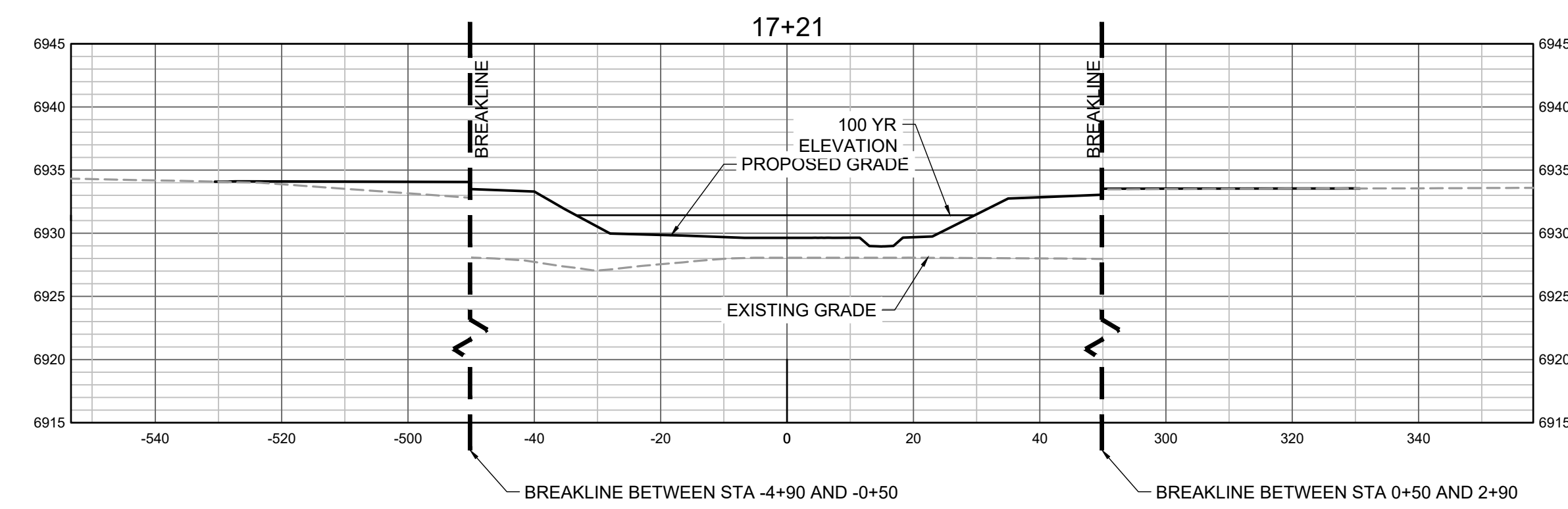
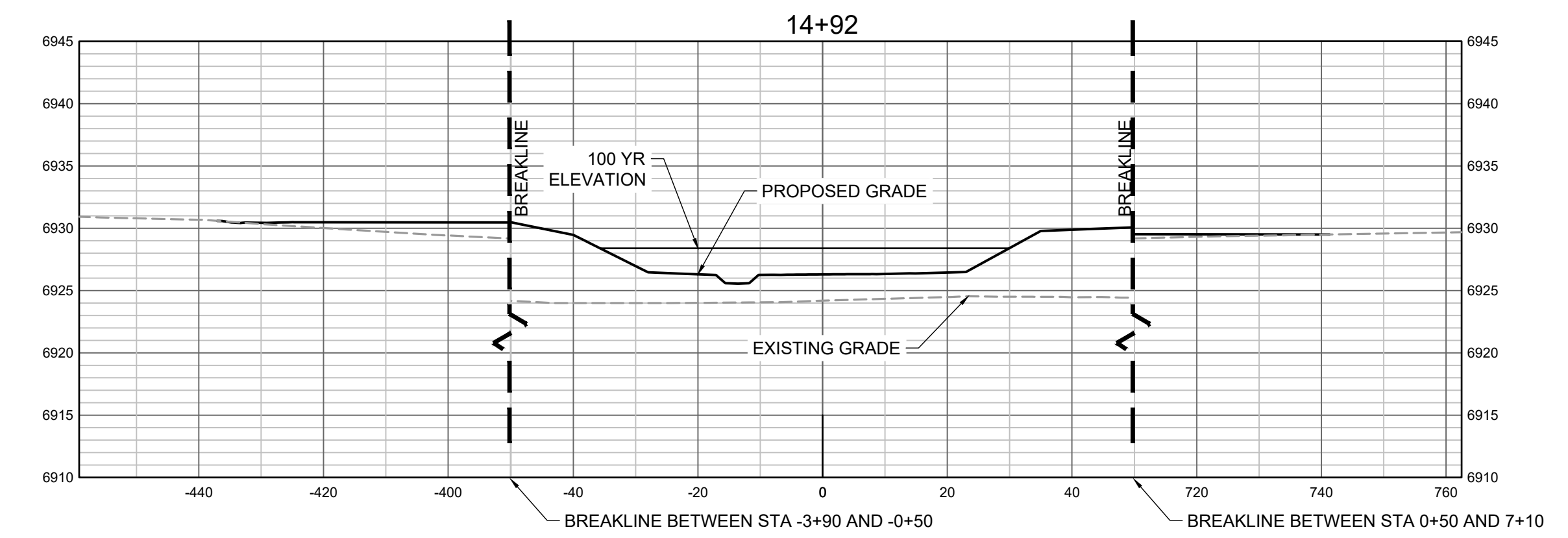
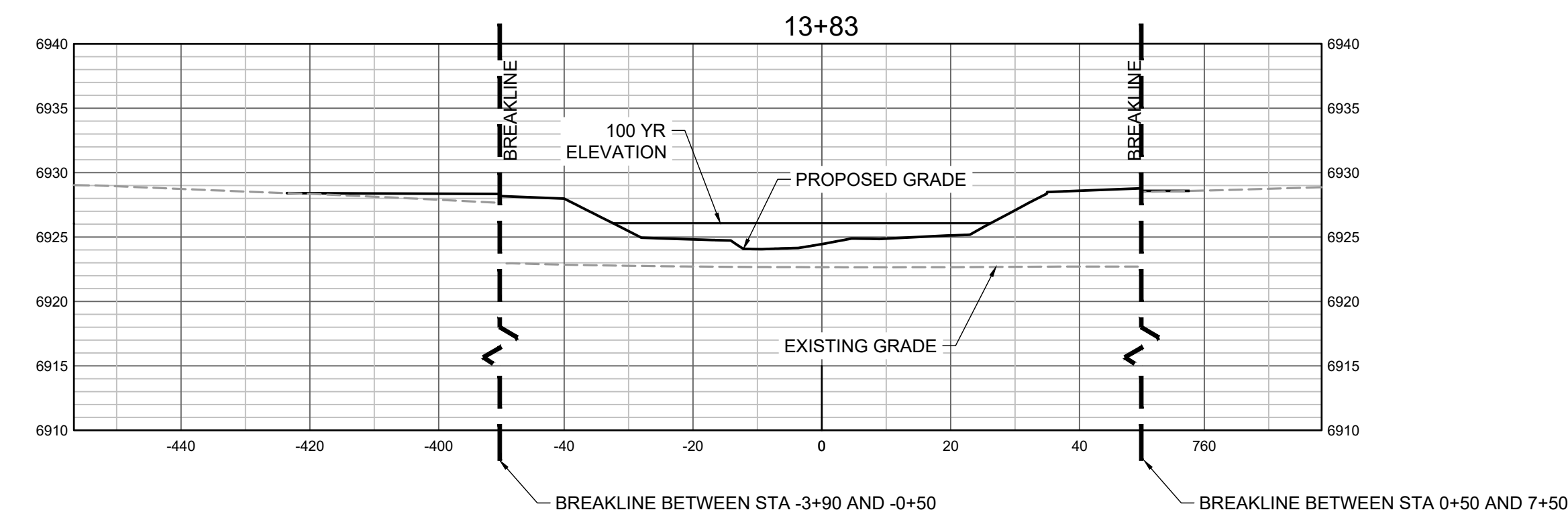
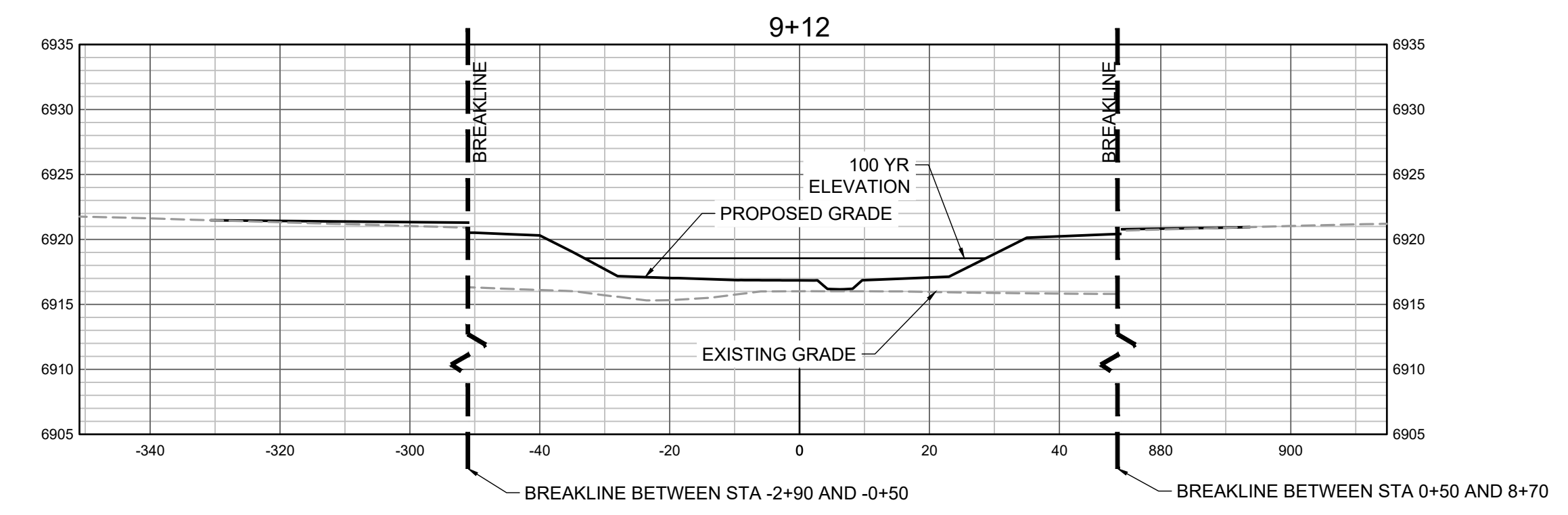
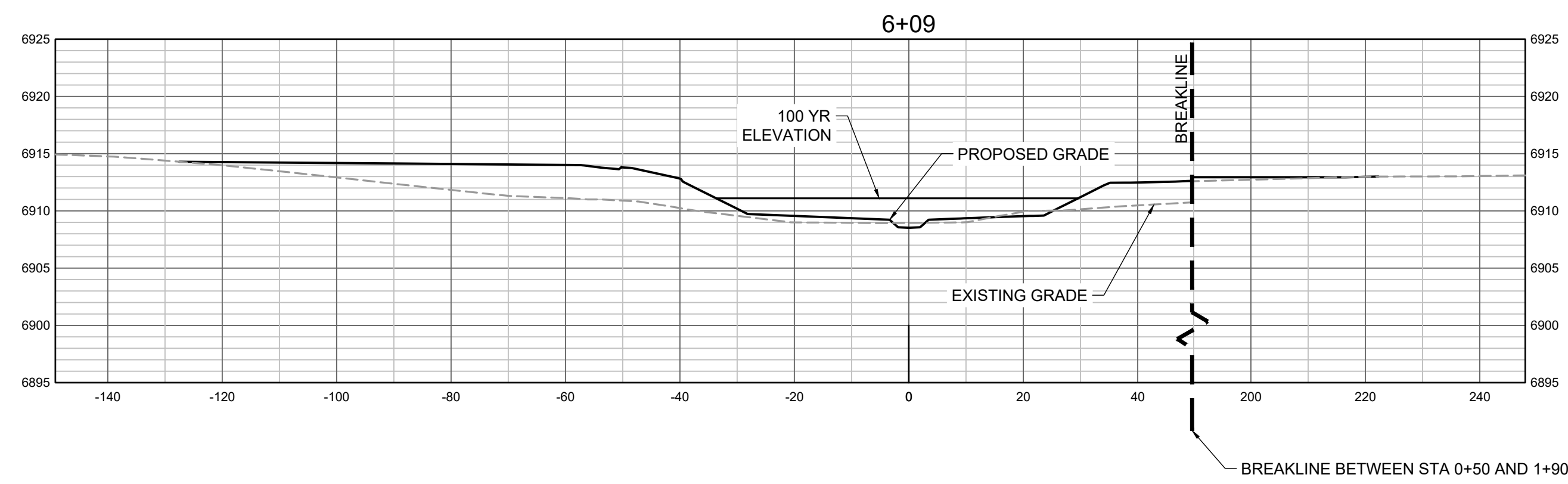
DRAWN BY: TBI	JOB DATE: 11/30/2023	BAR IS ONE INCH ON OFFICIAL DRAWINGS.
APPROVED: GLP	JOB NUMBER: 201662.03	0" = 1"
CAD DATE: 11/30/2023		IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.
CAD FILE: J:\2020\201662.03\CAD\Drawings\C\PLAN AND PROFILE		

NO.	DATE	BY	REVISION DESCRIPTION

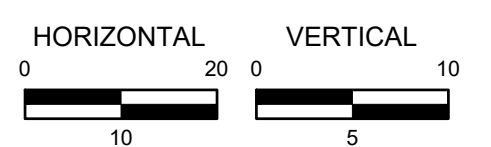
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DENVER CO 80111  
PHONE: 720.602.4999  
FAX: 713.965.0044

GRANDVIEW RESERVE  
DR HORTON  
FALCON, COLORADO

CONSTRUCTION DOCUMENTS  
DRAINAGE CULVERT PLAN AND PROFILE



PROPOSED GRADES TO TIE INTO GRANDVIEW RESERVE FILING 1. REFER TO THE GRANDVIEW RESERVE FILING 1 PLAN SET FOR CONTINUATION OF GRADING THAT IS BEING TIED INTO OUTSIDE OF CHANNEL GRADING LIMITS.



HR GREEN Xref: xgs-1-dh01; 01-XC-PR-100%\_FP\_DELIMITATION

DRAWN BY: TBI	JOB DATE: 11/30/2023	BAR IS ONE INCH ON OFFICIAL DRAWINGS.
APPROVED: GLP	JOB NUMBER: 201662.03	0" = 1"
CAD DATE: 11/30/2023		IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.
CAD FILE: J:\2020\201662.03\CAD\dwgs\C\CROSS SECTIONS		

NO.	DATE	BY	REVISION DESCRIPTION

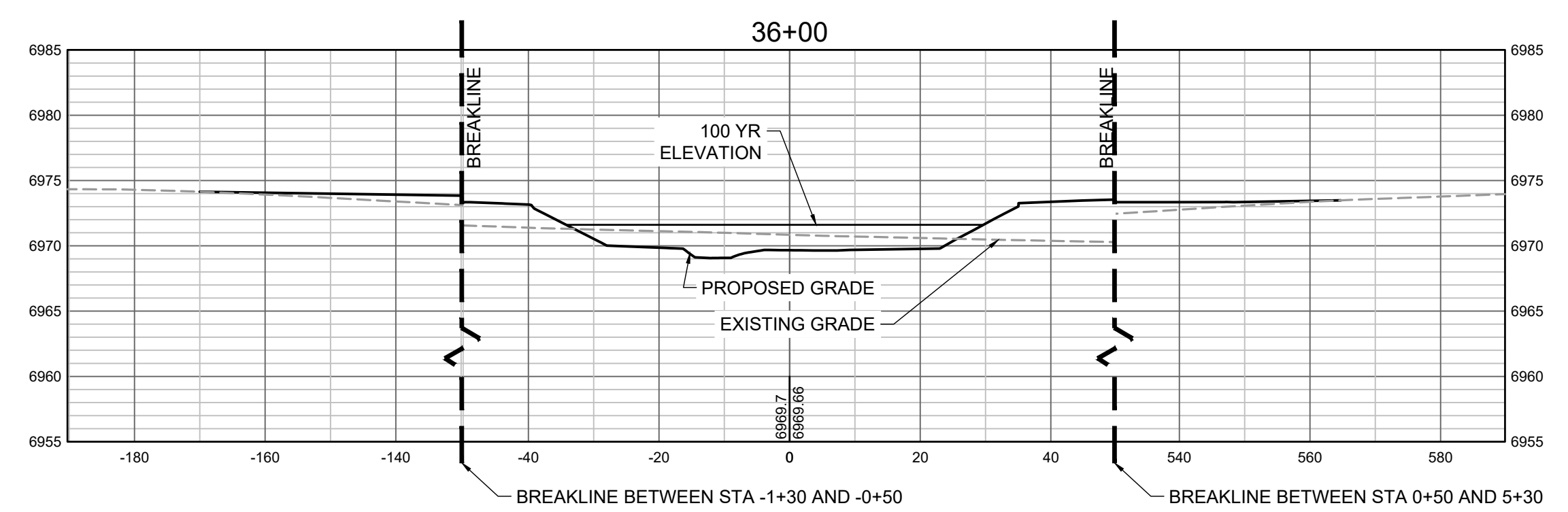
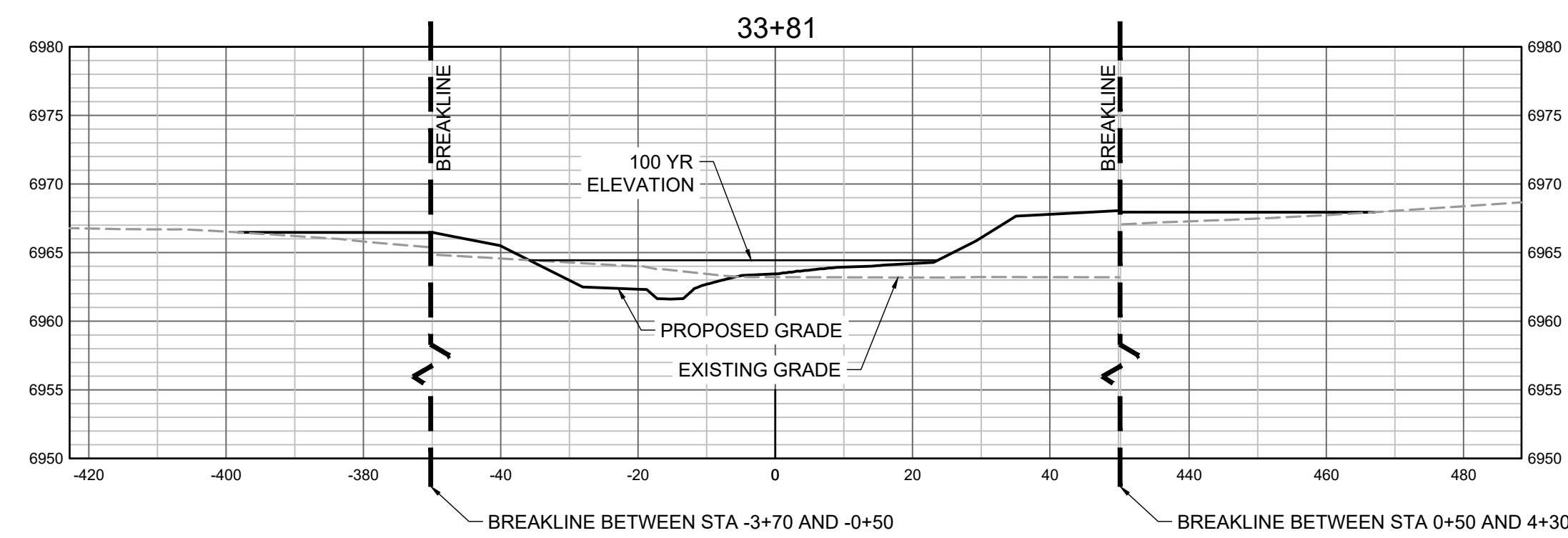
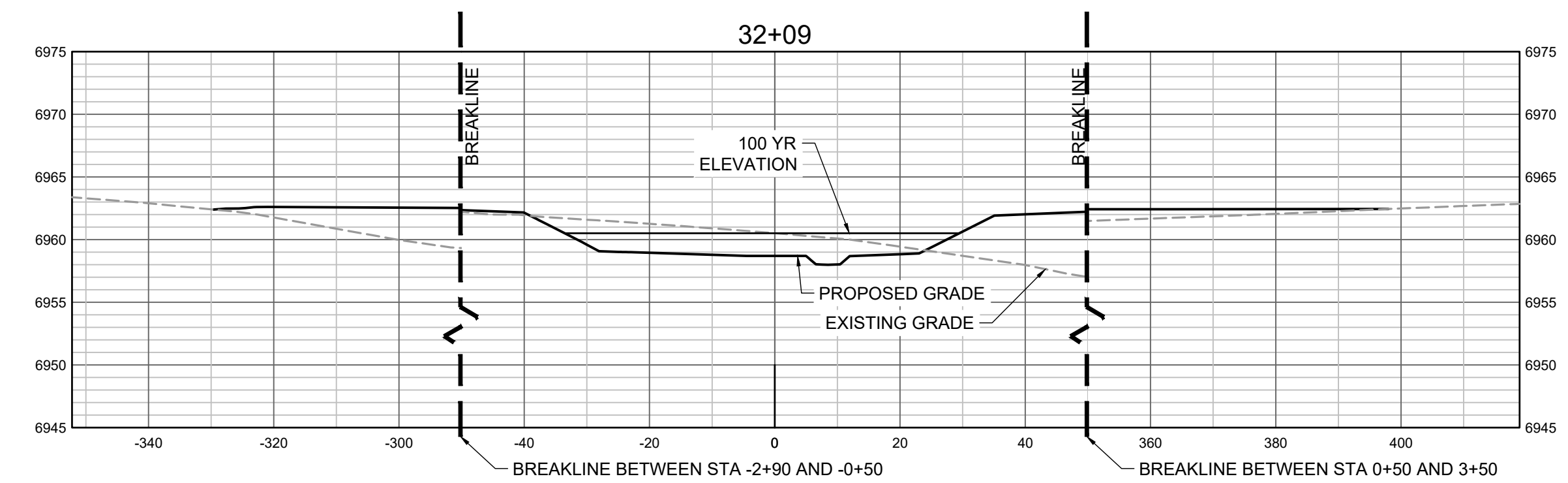
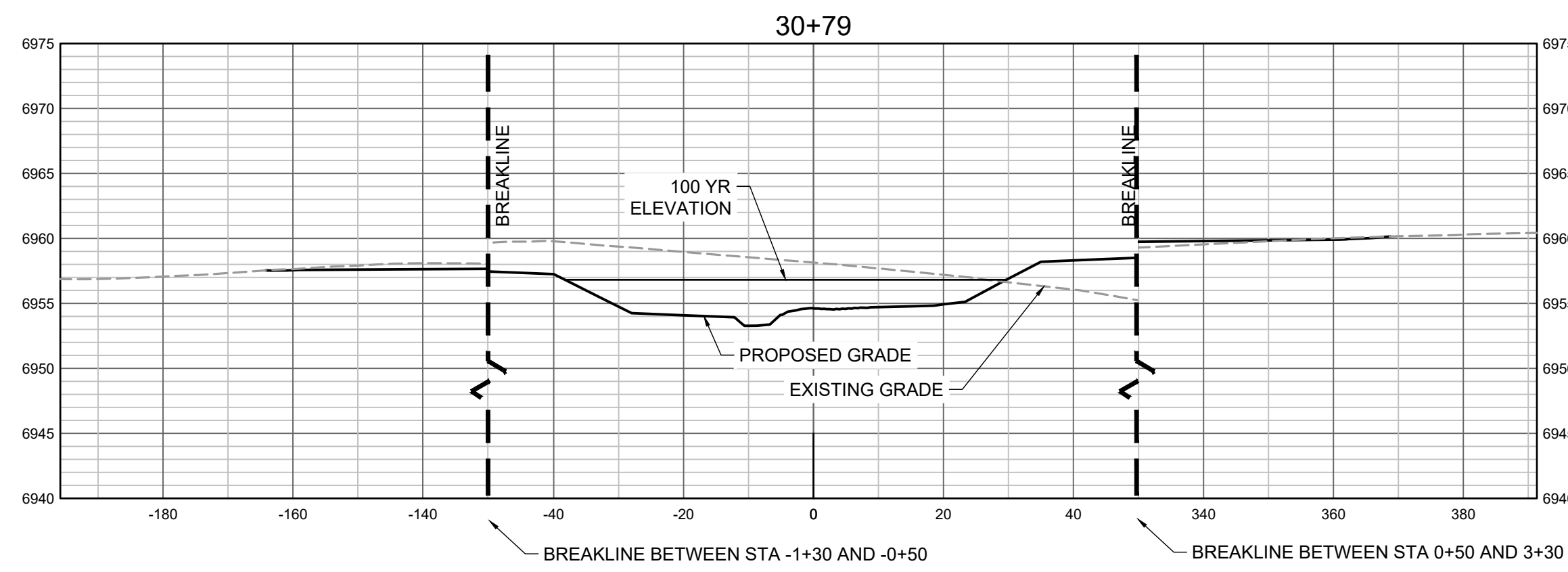
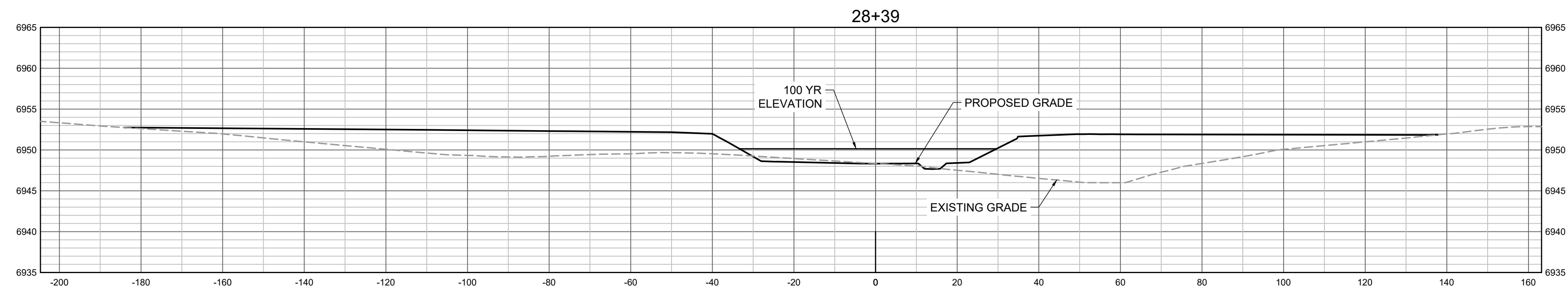
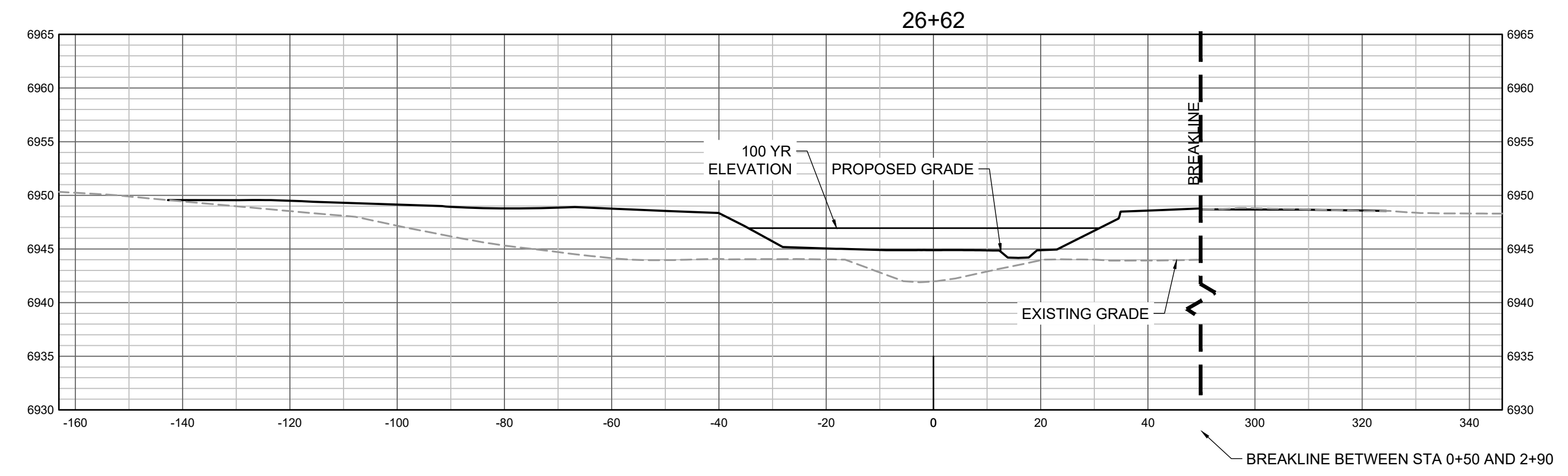
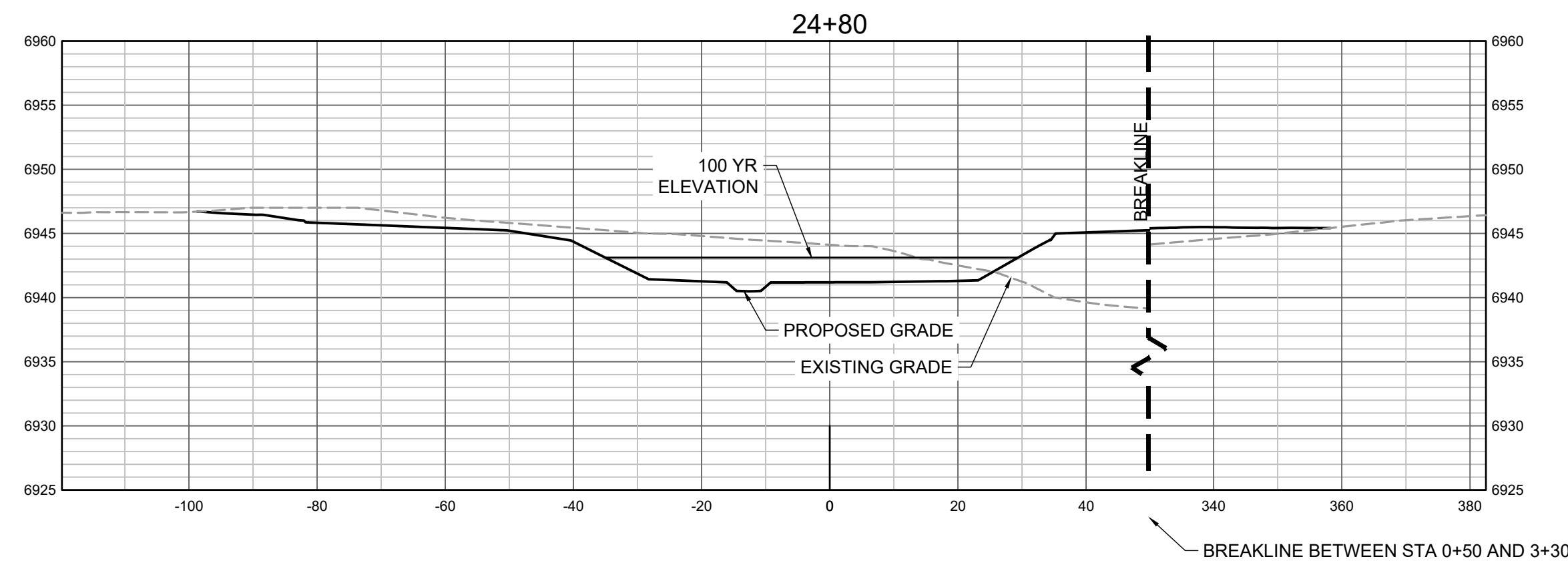
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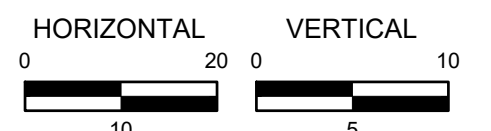
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FALCON, COLORADO

CONSTRUCTION DOCUMENTS  
TRIBUTARY 2 CROSS SECTIONS

SHEET  
**CS1**  
37



PROPOSED GRADES TO TIE INTO GRANDVIEW RESERVE FILING 1. REFER TO THE GRANDVIEW RESERVE FILING 1 PLAN SET FOR CONTINUATION OF GRADING THAT IS BEING TIED INTO OUTSIDE OF CHANNEL GRADING LIMITS.



HR GREEN Xref: xref: 01-XC-PR-100%\_FP\_DELIMITATION

DRAWN BY: TBI JOB DATE: 11/30/2023  
 APPROVED: GLP JOB NUMBER: 201662.03  
 CAD DATE: 11/30/2023  
 CAD FILE: J:\2020\201662.03\CAD\DWGS\C\CROSS SECTIONS

BAR IS ONE INCH ON  
 OFFICIAL DRAWINGS.  
 IF NOT ONE INCH,  
 ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

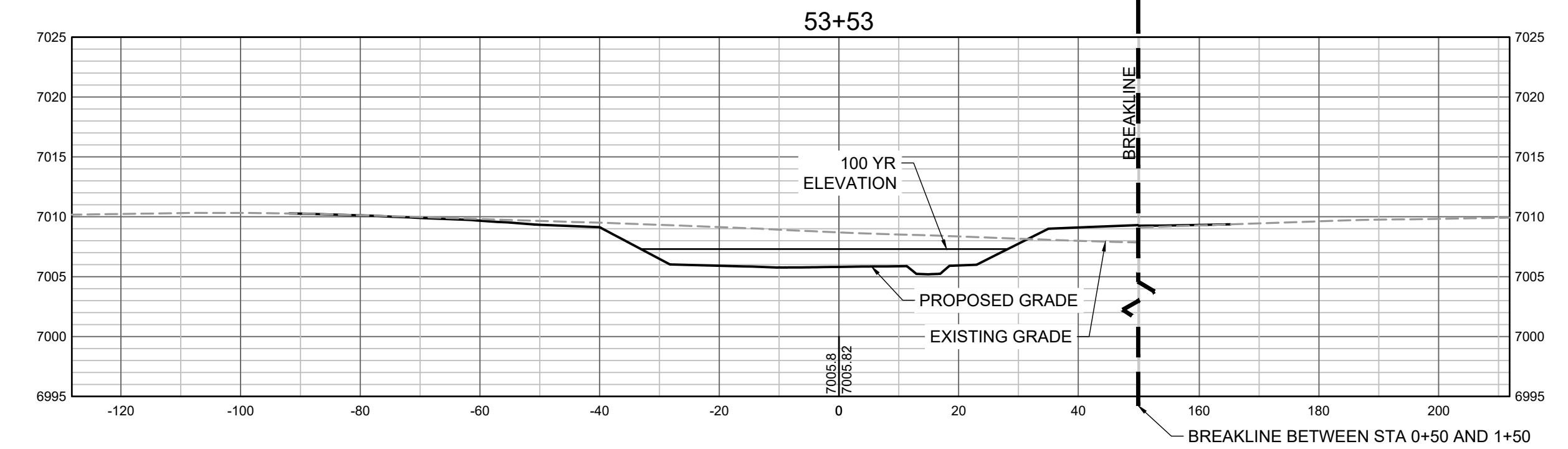
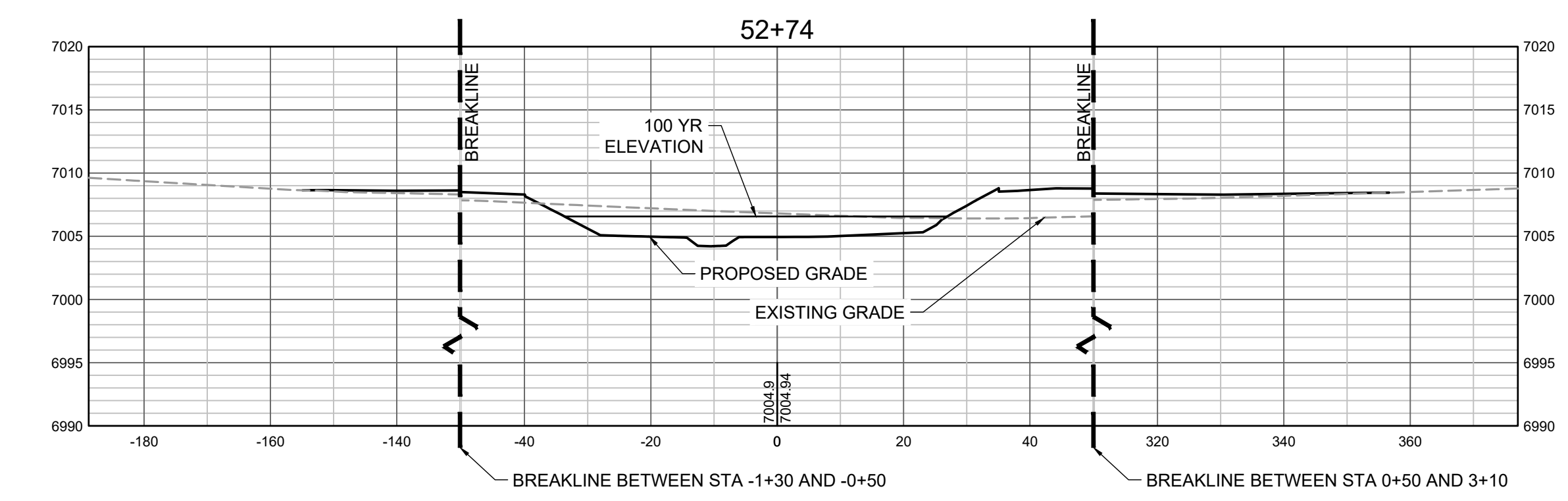
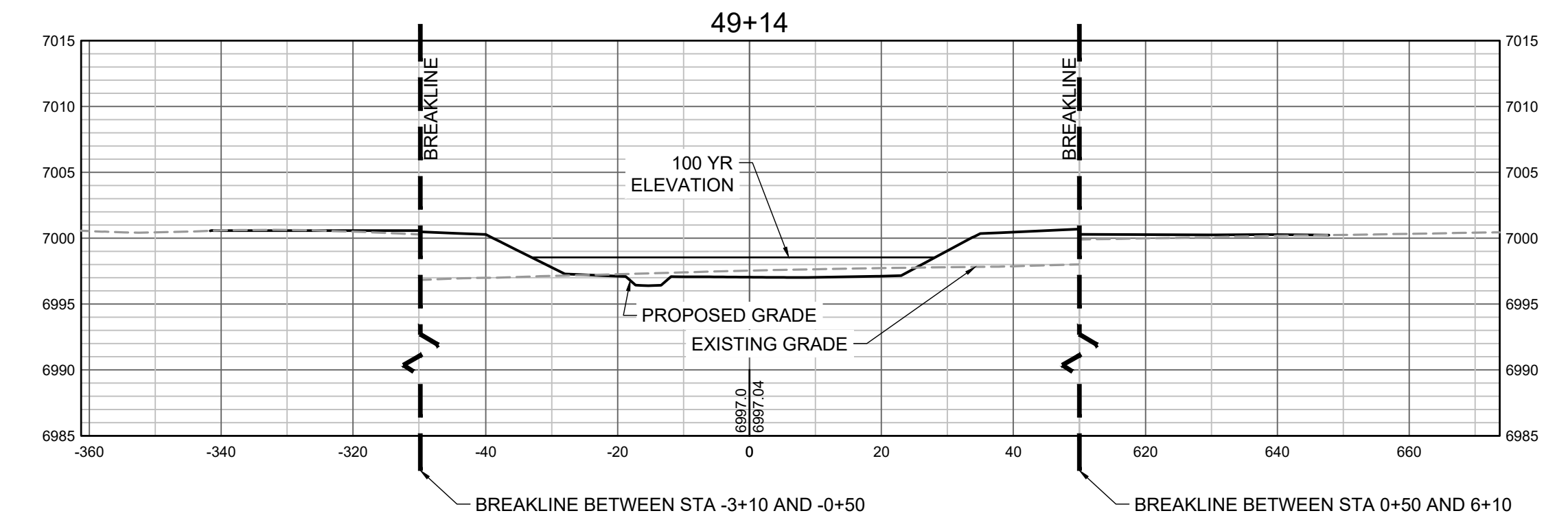
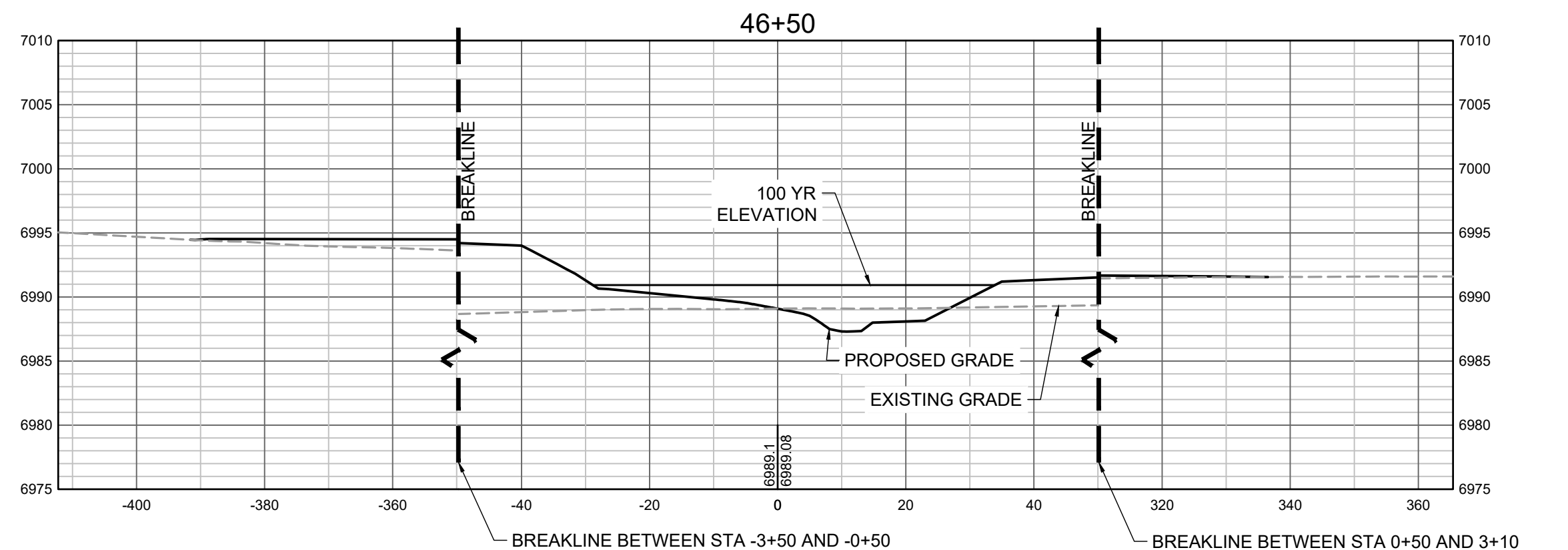
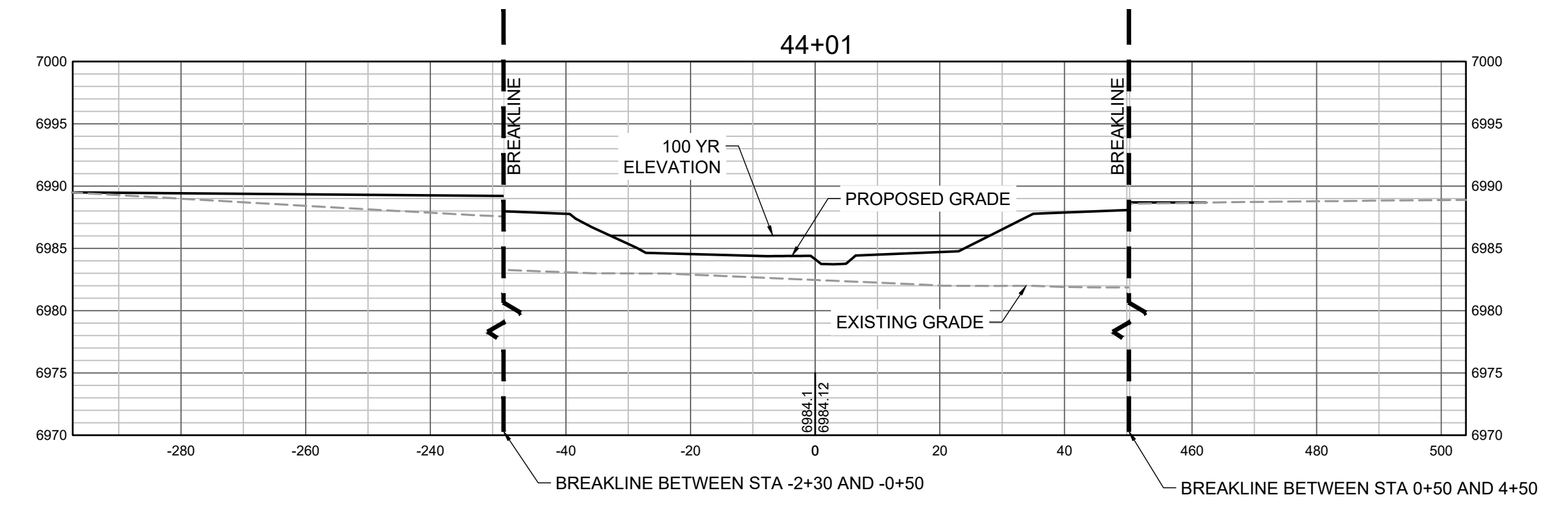
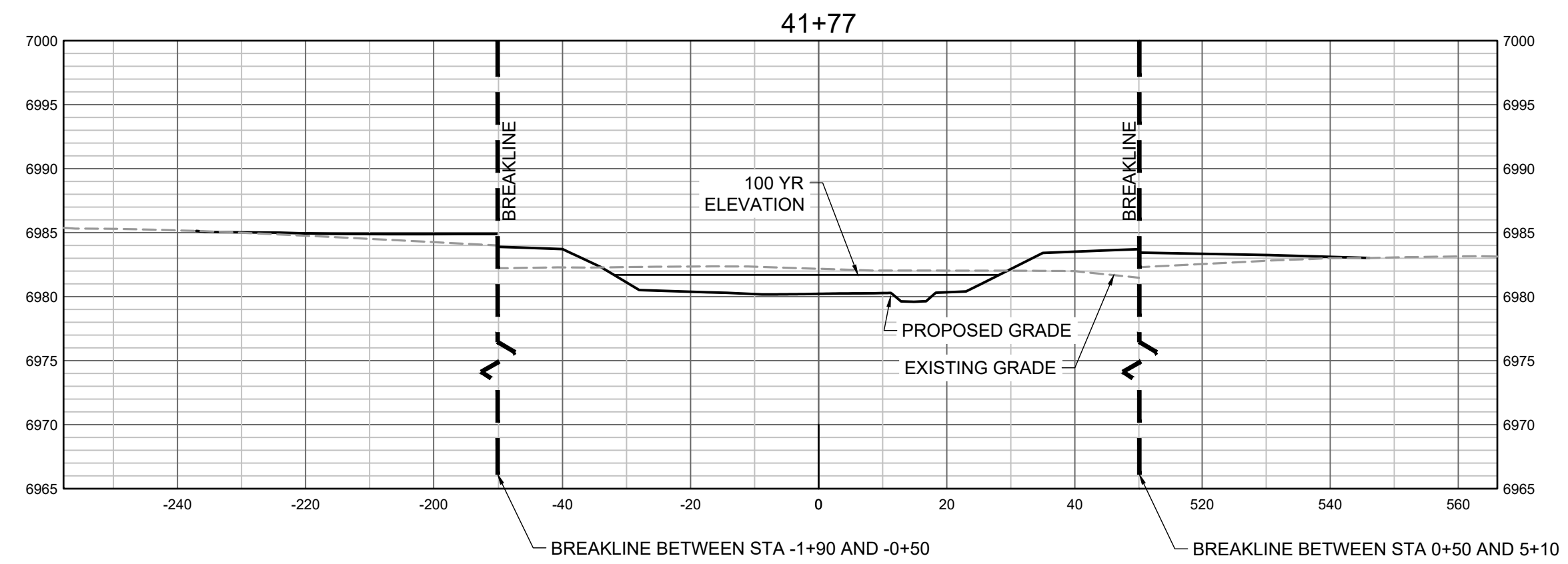
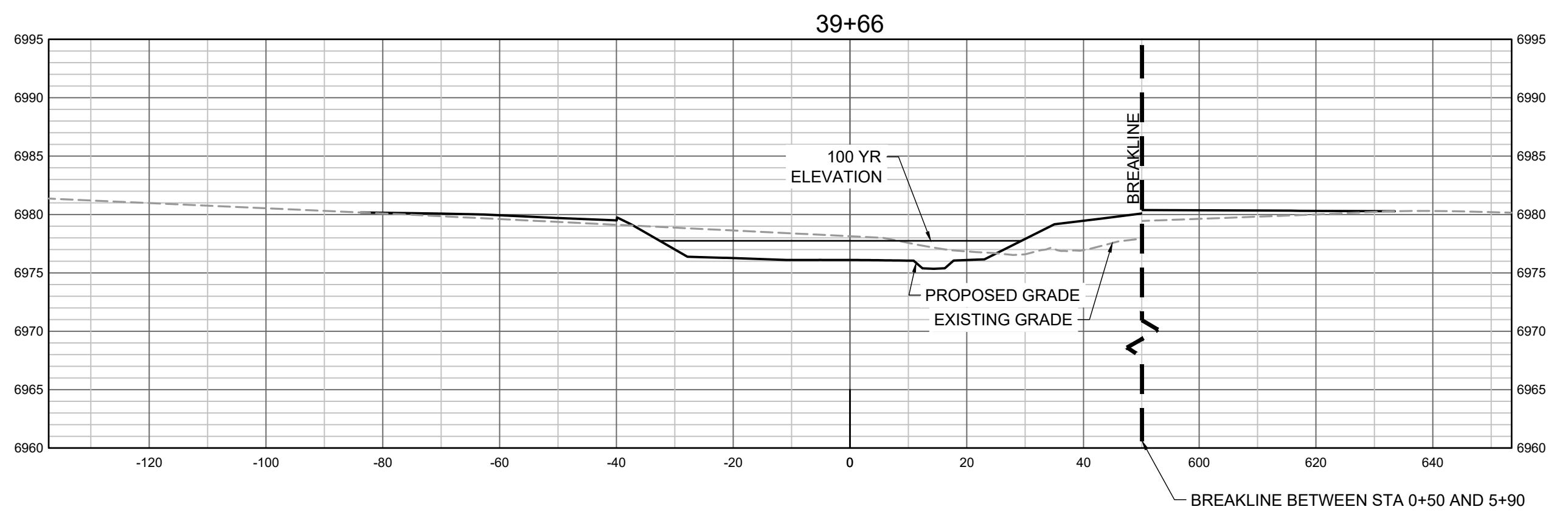
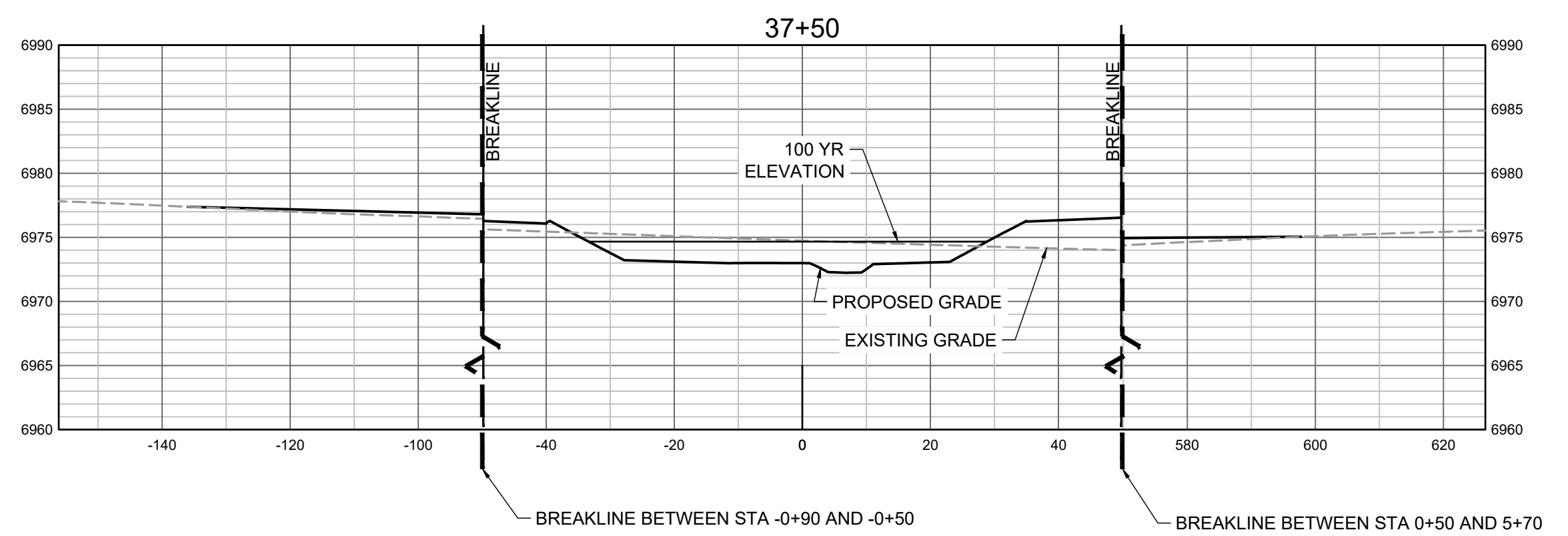


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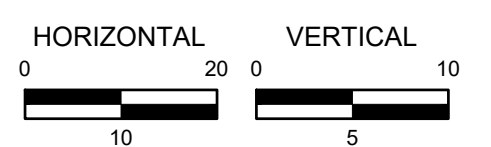
GRANDVIEW RESERVE  
 DR HORTON  
 FALCON, COLORADO

CONSTRUCTION DOCUMENTS  
 TRIBUTARY 2 CROSS SECTIONS

SHEET  
 CS2  
 38



PROPOSED GRADES TO TIE INTO GRANDVIEW RESERVE FILING 1. REFER TO THE GRANDVIEW RESERVE FILING 1 PLAN SET FOR CONTINUATION OF GRADING THAT IS BEING TIED INTO OUTSIDE OF CHANNEL GRADING LIMITS.



HR GREEN Xrefs: xgs-1-dh01; 01-XC-PR\_100YR\_FP\_DELIMITATION

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 APPROVED: GLP JOB NUMBER: 201662.03  
 CAD DATE: 11/30/2023  
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BAR IS ONE INCH ON OFFICIAL DRAWINGS.  
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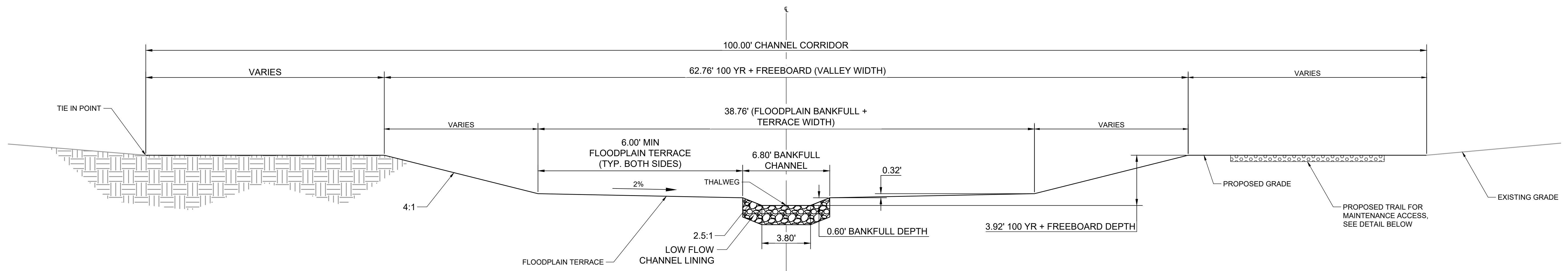
NO.	DATE	BY	REVISION DESCRIPTION

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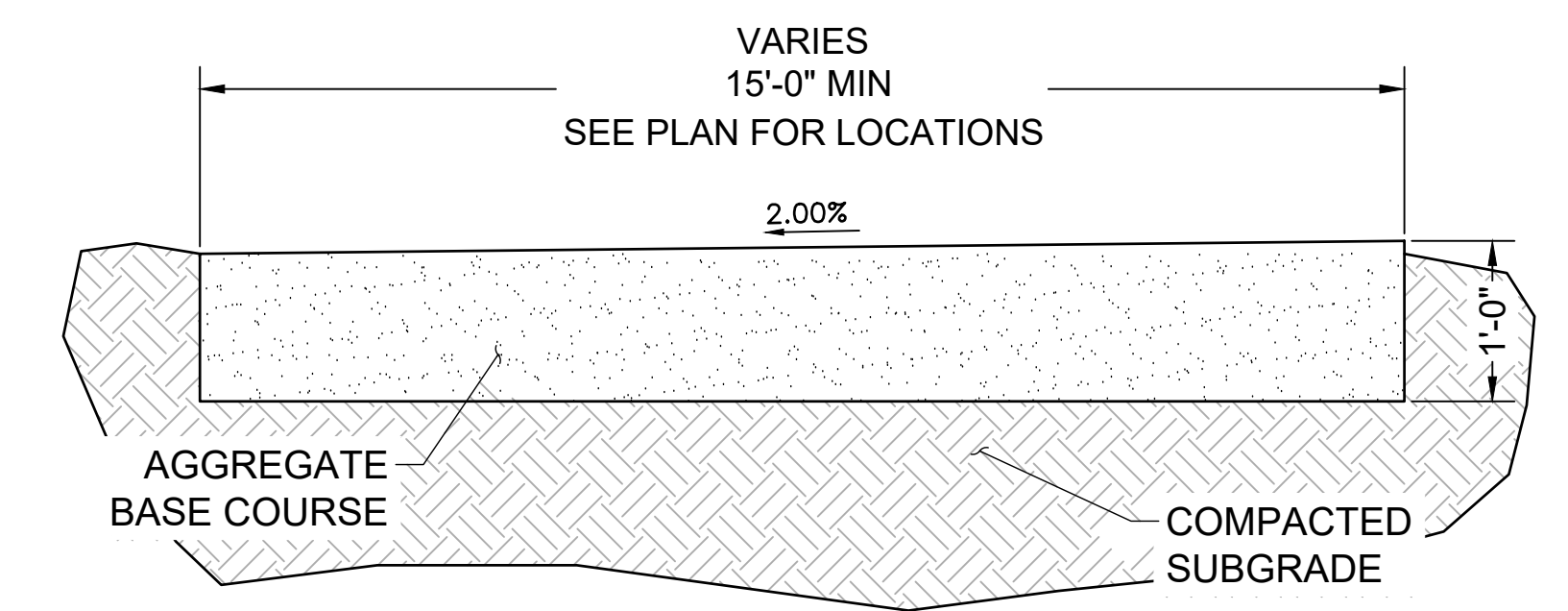
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**DR HORTON**  
 FALCON, COLORADO

CONSTRUCTION DOCUMENTS  
 TRIBUTARY 2 CROSS SECTIONS

SHEET  
**CS3**  
**39**



**TYPICAL CROSS SECTION – TRIBUTARY 2**  
SCALE: N.T.S.



**MAINTENANCE ROAD TYPICAL SECTION**  
SCALE: NTS

- NOTES:
- BANKFULL CHANNEL MAY SHIFT LEFT OR RIGHT WITHIN THE BANKFULL + TERRACE WIDTH SO LONG AS THE MINIMUM FLOOD PLAIN TERRACE WIDTH OF 6' IS MAINTAINED ON BOTH SIDES.
  - VALLEY WIDTH MAY SHIFT WITHIN THE 100' CHANNEL CORRIDOR.
  - SEE PROFILES FOR ELEVATION AT THALWEG.

DRAWN BY: TBI	JOB DATE: 5/10/2023	BAR IS ONE INCH ON OFFICIAL DRAWINGS.
APPROVED: GLP	JOB NUMBER: 201662.03	0" = 1"
CAD DATE: 11/30/2023		IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.
CAD FILE: J:\2020\201662.03\CAD\DWG\SD\CIDetails		

NO.	DATE	BY	REVISION DESCRIPTION

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 DENVER CO 80111  
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 FAX: 713.965.0044

**GRANDVIEW RESERVE**  
**DR HORTON**  
 FALCON, COLORADO

CONSTRUCTION DOCUMENTS  
 DETAILS

SHEET  
 DT2

41

HR GREEN Xref: xref.dwg



**GRANDVIEW - BASIN C**  
**PROPOSED CONDITIONS**  
**LOCATION: COLORADO SPRINGS, COLORADO**

**Calc'd by:** NQJ  
**Checked by:**  
**Date:** 2/29/2024

SUMMARY RUNOFF TABLE				
BASIN	AREA (ac)	% IMPERVIOUS	Q <sub>5</sub> (cfs)	Q <sub>100</sub> (cfs)
C1	4.53	65	8.2	18.0
C2	2.71	65	4.8	10.6
C3	2.47	65	4.7	10.3
C4	3.09	65	4.5	9.8
C5	3.13	65	4.7	10.4
C6	0.81	45	1.0	2.6
C7	6.07	65	7.7	17.0
C8	5.11	65	7.4	16.2
C9	3.50	65	5.5	12.1
C10	3.97	66	6.3	13.6
C11	3.64	67	5.9	12.9
C12	0.46	65	0.9	2.0
C13	1.66	65	2.9	6.4
C14	2.37	65	4.2	9.3
C15	1.69	65	3.9	8.7

DESIGN POINT SUMMARY TABLE			
DESIGN POINT	CONTRIBUTING BASINS	ΣQ <sub>5</sub> (cfs)	ΣQ <sub>100</sub> (cfs)
20	C1	8.2	18.0
21	C2	4.8	10.6
21.1	C1-C2	10.3	17.7
22	C3	4.7	10.3
22.1	DP21.1 & C3	17.9	25.3
23	C4	4.5	11.4
24	C5	5.0	14.7
24.1	DP22.1, DP23 & DP24	23.4	44.1
25	C6	1.0	2.6
26	DP25 & C7	11.5	22.0
27	C8	7.4	16.2
27.1	DP26-27	16.5	25.4
28	C9	5.5	12.1
28.1	DP27.1 & DP28	20.8	34.6
29.1	DP24.1 & DP28.1	40.2	73.0
30	C10	6.3	13.6
30.1	DP29.1 & DP30	44.8	91.1
31	C11	5.9	12.9
31.1	DP30.1 & DP31	46.0	97.6
32	C12	0.9	2.0
32.1	DP31.1 & DP32	46.4	98.4
33	C13	2.9	6.4
33.1	DP32.1 & DP33	48.1	102.3
34	DP33.1 & C14	50.5	107.4
35	C15	3.9	8.7





**GRANDVIEW - BASIN C**

**PROPOSED CONDITIONS**

**LOCATION: COLORADO SPRINGS, COLORADO**

**Calc'd by:**

**NQJ**


**Checked by:**

**Date:**

**2/29/2024**

**COMPOSITE 'C' FACTORS**

BASIN	UNDEVELOPED	PAVED	1/8 AC		TOTAL	SOIL TYPE	UNDEVELOPED			PAVED			1/8 AC RESIDENTIAL			NEIGHBORHOOD AREAS			COMPOSITE IMPERVIOUSNESS & C					
			RESIDENTIAL	AREAS			%I	C <sub>5</sub>	C <sub>100</sub>	%I	C <sub>5</sub>	C <sub>100</sub>	%I	C <sub>5</sub>	C <sub>100</sub>	%I	C <sub>5</sub>	C <sub>100</sub>	%I	C <sub>5</sub>	C <sub>100</sub>			
			ACRES																					
C1	0.00	0.00	4.53	0.00	4.53	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C2	0.00	0.00	2.71	0.00	2.71	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C3	0.00	0.00	2.47	0.00	2.47	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C4	0.00	0.00	3.09	0.00	3.09	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C5	0.00	0.00	3.13	0.00	3.13	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C6	0.26	0.00	0.55	0.00	0.81	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	45	0.33	0.52			
C7	0.00	0.00	6.07	0.00	6.07	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C8	0.00	0.00	5.11	0.00	5.11	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C9	0.00	0.00	3.50	0.00	3.50	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C10	0.00	0.17	3.80	0.00	3.97	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	66	0.47	0.61			
C11	0.00	0.17	3.47	0.00	3.64	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	67	0.47	0.61			
C12	0.00	0.00	0.46	0.00	0.46	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C13	0.00	0.00	1.66	0.00	1.66	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C14	0.00	0.00	2.37	0.00	2.37	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
C15	0.00	0.00	1.69	0.00	1.69	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59			
<b>POND C</b>						<b>43.52</b>																<b>64.9</b>		
<b>TOTAL BASIN C</b>						<b>45.21</b>																<b>64.9</b>		

	<b>GRANDVIEW - BASIN C</b>	<b>Calc'd by:</b>	<b>NQJ</b>
	<b>PROPOSED CONDITIONS</b>	<b>Checked by:</b>	
	<b>LOCATION: COLORADO SPRINGS, COLORADO</b>	<b>Date:</b>	<b>2/29/2024</b>

<b>TIME OF CONCENTRATION</b>											
<b>BASIN DATA</b>			<b>OVERLAND TIME (T<sub>o</sub>)</b>			<b>TRAVEL TIME (T<sub>t</sub>)</b>					<b>TOTAL</b>
DESIGNATION	C <sub>s</sub>	AREA (ac)	LENGTH (ft)	SLOPE %	t <sub>o</sub> (min)	C <sub>v</sub>	LENGTH (ft)	SLOPE %	V (ft/s)	t <sub>t</sub> (min)	t <sub>c</sub> (min)
C1	0.45	4.53	65	3.0	6.7	20	680	1.80	2.7	4.2	10.9
C2	0.45	2.71	55	2.0	7.0	20	680	1.80	2.7	4.2	11.2
C3	0.45	2.47	55	2.0	7.0	20	350	1.50	2.4	2.4	9.4
C4	0.45	3.09	150	2.0	11.6	20	820	1.00	2.0	6.8	18.4
C5	0.45	3.13	110	2.0	9.9	20	820	1.00	2.0	6.8	16.8
C6	0.33	0.81	161	3.0	12.3	15	110	2.00	2.1	0.9	13.2
C7	0.45	6.07	150	2.0	11.6	20	1300	0.80	1.8	12.1	23.7
C8	0.45	5.11	110	2.0	9.9	20	930	0.80	1.8	8.7	18.6
C9	0.45	3.50	110	2.0	9.9	20	570	0.80	1.8	5.3	15.2
C10	0.47	3.97	150	2.0	11.2	20	770	1.50	2.4	5.2	16.5
C11	0.47	3.64	110	3.0	8.4	20	1050	1.50	2.4	7.1	15.5
C12	0.45	0.46	90	2.0	9.0	20	5	2.00	2.8	0.0	9.0
C13	0.45	1.66	110	2.0	9.9	20	250	1.40	2.4	1.8	11.7
C14	0.45	2.37	230	10.0	8.4	15	250	1.00	1.5	2.8	11.2
C15	0.45	1.69	50	25.0	2.9	15	200	2.00	2.1	1.6	5.0

**FORMULAS:**

$$t_i = \frac{0.395(1.1 - C_s)\sqrt{L}}{S^{0.33}} \quad V = C_v S_w^{0.5}$$

**Table 6-7. Conveyance Coefficient, C<sub>v</sub>**

Type of Land Surface	C <sub>v</sub>
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

\*For buried riprap, select C<sub>v</sub> value based on type of vegetative cover.



**GRANDVIEW - BASIN C**  
**PROPOSED CONDITIONS**  
**DESIGN STORM: 5-YEAR**

Calc'd by:  
 Checked by:  
 Date:

**NQJ**  
  
**2/29/2024**

STREET	DESIGN POINT	BASIN ID	DIRECT RUNOFF					TOTAL RUNOFF				STREET			PIPE			TRAVEL TIME			REMARKS					
			AREA (ac)	C <sub>s</sub>	f <sub>c</sub> (min)	C <sub>s</sub> *A (ac)	f (in./hr.)	Q (cfs)	f <sub>c</sub> (min)	C <sub>s</sub> *A (ac)	f (in./hr.)	Q (cfs)	Q <sub>street</sub> (cfs)	C <sub>s</sub> *A (ac)	SLOPE %	Q <sub>PIPE</sub> (cfs)	C <sub>s</sub> *A (ac)	SLOPE %	PIPE SIZE (ft)	LENGTH (FT)		VEL. (FPS)	TRAVEL TIME (min)			
	20	C1	4.53	0.45	10.9	2.04	4.00	8.2						0.5	0.11	1.0		7.7	1.92	2.0	2.0	820	2.0	6.83	DP20 FLOWBY, C&G FLOW TO DP24	
																						28	10.2	0.05	BASIN C1 CAPTURED IN 15' TYPE R ONGRADE @ DP20, PIPE TO DP21.1	
	21	C2	2.71	0.45	11.2	1.22	3.95	4.8										4.8	1.21	2.0	2.0	5	10.2	0.01	BASIN C2 CAPTURED IN 15' TYPE R ONGRADE @ DP21, PIPE TO DP21.1	
	21.1									17.7	3.14	3.27	10.3					10.3	3.14	2.0	2.0					COMBINED DP20 & DP21, PIPE TO DP22.1
	22	C3	2.47	0.45	9.4	1.11	4.22	4.7										4.7	1.11	2.0	2.0				BASIN 3 CAPTURED IN 15' TYPE R ONGRADE @ DP22, PIPE TO DP22.1	
	22.1									9.4	4.25	4.22	17.9					17.9	4.25	1.0	2.0	820	7.2	1.90	COMBINED DP21.1 & DP22, PIPE TO DP24.1	
	23	C4	3.09	0.45	18.4	1.39	3.21	4.5										4.5	1.39	2.0	2.0	5	10.2	0.01	BASIN C4 CAPTURED IN 15' TYPE R ONGRADE @ DP24, PIPE TO DP24.1	
	24	C5	3.13	0.45	16.8	1.41	3.36	4.7	17.7	1.52	3.27	5.0						5.0	1.52	2.0	2.0	28	10.2	0.05	DP20 FLOWBY & BASIN C5 CAPTURED IN 15' TYPE R ONGRADE @ DP24, PIPE TO DP24.1	
	24.1									17.8	7.16	3.27	23.4					23.4	7.16	2.0	2.0	53	10.2	0.09	COMBINED DP22.1, DP23 & DP24, PIPE TO DP29.1	
	25	C6	0.81	0.33	13.2	0.27	3.71	1.0						1.0	0.27	0.8					605	1.8	5.64		BASIN C6 FLOW @ DP25, C&G FLOW TO DP26	
	26	C7	6.07	0.45	23.7	2.73	2.84	7.7	23.7	3.00	2.84	11.5		1.8	0.47	1.5		9.7	2.53	2.0	2.0	1050	2.4	7.14	DP26 FLOWBY, C&G FLOW TO DP31	
																					5	10.2	0.01	DP17, DP18, DP25 & BASIN C7 CAPTURED IN 15' TYPE R ONGRADE @ DP26, PIPE TO DP27.1		
	27	C8	5.11	0.45	18.6	2.30	3.20	7.4						0.2	0.05	1.5		7.2	2.25	2.0	2.0	270	2.4	1.84	DP27 FLOWBY, C&G FLOW TO DP28	
																					18	10.2	0.03	BASIN C8 FLOW CAPTURED IN 15' TYPE R ONGRADE, PIPE TO DP27.1		
	27.1									23.7	4.78	2.83	16.5					16.5	4.78	2.0	2.0	265	10.2	0.43	COMBINED DP26 & DP27, PIPE TO DP28.1	
	28	C9	3.50	0.45	15.2	1.58	3.50	5.5	20.4	1.62	3.06	8.0		0.4	0.07	1.5		7.6	1.55	2.0	2.0	780	2.4	5.31	DP28 FLOWBY, C&G FLOW TO DP30	
																					30	10.2	0.05	DP27 FLOWBY & BASIN C9 CAPTURED IN 15' TYPE R ON GRADE @ DP28, PIPE TO DP28.1		
	28.1									24.1	6.33	2.81	20.8					20.8	6.33	2.0	2.0	490	10.2	0.80	COMBINED DP27.1 & DP29, PIPE TO DP29.1	
	29.1									24.9	13.49	2.76	40.2					40.2	13.49	2.0	2.0	335	10.2	0.55	COMBINED DP28.1 & DP24.1, PIPE TO DP30.1	
	30	C10	3.97	0.47	16.5	1.86	3.38	6.3	25.7	1.94	2.71	8.3						8.3	1.94	2.0	2.0	30	10.2	0.05	DP28 FLOWBY & BASIN C10 FLOW CAPTURED IN 20' TYPE R SUMP @ DP30, PIPE TO DP30.1	
	30.1									25.8	15.43	2.71	44.8					44.8	15.43	2.0	2.0	5	10.2	0.01	COMBINED DP29.1 & DP20, PIPE TO DP31.1	
	31	C11	3.64	0.47	15.5	1.71	3.47	5.9	30.8	2.19	2.44	8.3						8.3	2.19	2.0	2.0				DP26 FLOWBY & BASIN C11 CAPTURED IN 20' TYPE R SUMP @ DP31, PIPE TO DP31.1	
	31.1									30.8	17.62	2.44	46.0					46.0	17.62	2.0	2.0	90	10.2	0.15	COMBINED DP30.1 & DP31, PIPE TO DP32.1	
	32	C12	0.46	0.45	9.0	0.21	4.29	0.9										0.9	0.21	2.0	2.0				BASIN 32 CAPTURED IN 5' TYPE R SUMP, PIPE TO DP32.1	
	32.1									31.0	17.82	2.43	46.4					46.4	17.82	2.0	2.0	32	10.2	0.05	COMBINED DP31.1 & DP32, PIPE TO DP33.1	
	33	C13	1.66	0.45	11.7	0.75	3.90	2.9										2.9	0.75	2.0	2.0				BASIN C13 CAPTURED IN 5' TYPE R SUMP, PIPE TO DP33.1	
	33.1									31.0	18.57	2.43	48.1					48.1	18.57	2.0	2.0	170	10.2	0.28	COMBINED DP32.1 & DP33, PIPE TO POND C	
	34	C14	2.37	0.45	11.2	1.07	3.96	4.2	31.3	19.64	2.42	50.5														<b>TOTAL FLOW ENTERING POND C</b>
	35	C15	1.69	0.45	5.0	0.76	5.17	3.9																		BASIN C15 FLOW, FOLLOWS HISTORIC DRAINAGE PATTERNS TOWARDS DP35



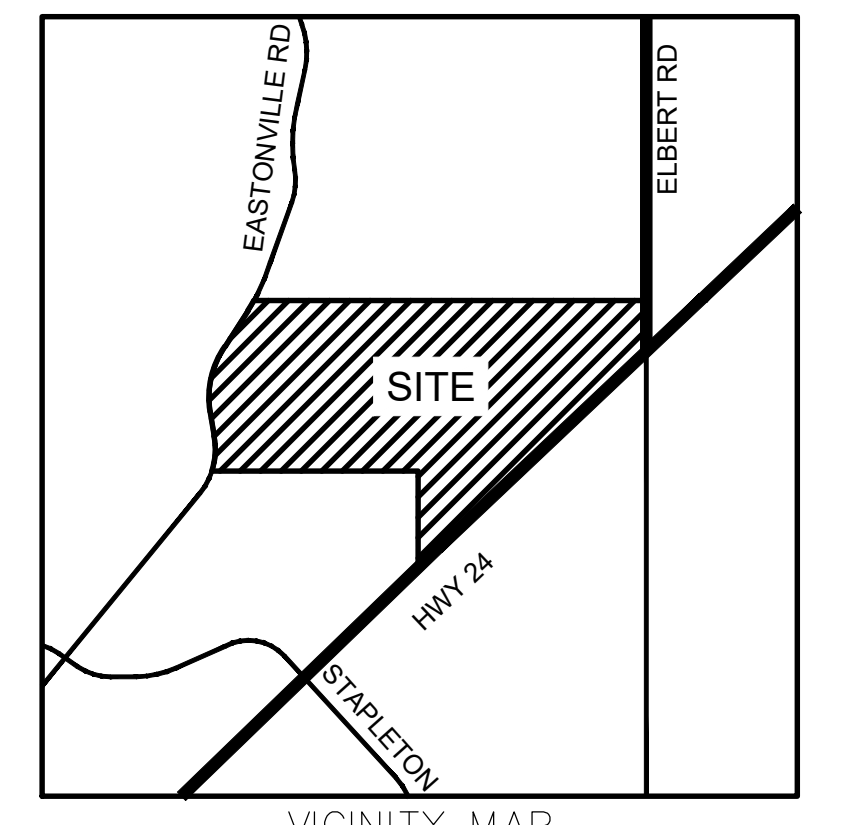
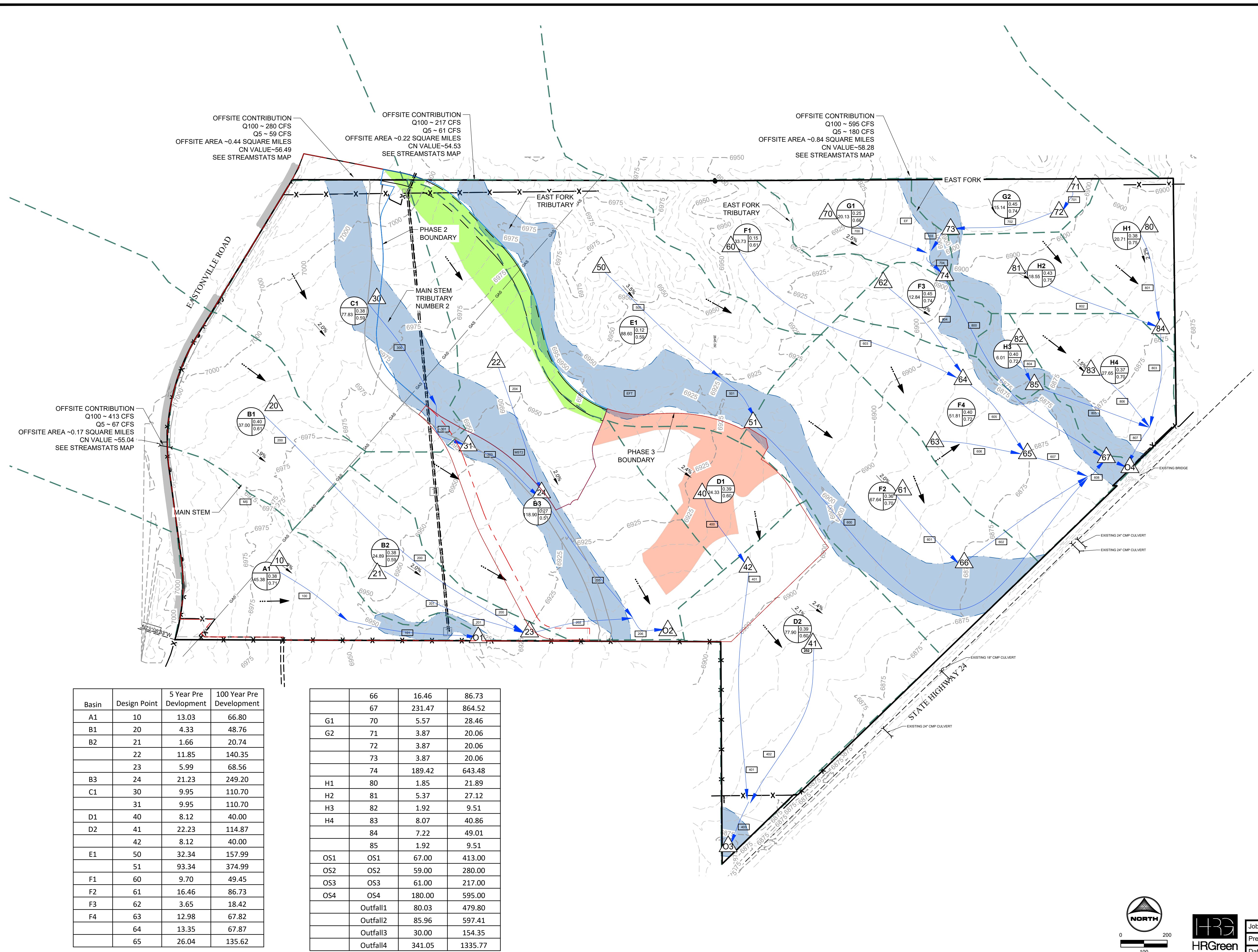
**GRANDVIEW - BASIN C**  
**PROPOSED CONDITIONS**  
**DESIGN STORM: 100-YEAR**

Calc'd by:  
 Checked by:  
 Date:

**NQJ**  
  
**2/29/2024**

STREET	BASIN ID	DIRECT RUNOFF						TOTAL RUNOFF				STREET			PIPE			TRAVEL TIME			REMARKS
		AREA (ac)	C <sub>100</sub>	f <sub>c</sub> (min)	C <sub>100</sub> *A (ac)	/ (in./hr.)	Q (cfs)	f <sub>c</sub> (min)	C <sub>100</sub> *A (ac)	/ (in./hr.)	Q (cfs)	Q <sub>street</sub> (cfs)	C <sub>100</sub> *A (ac)	SLOPE %	Q <sub>PIPE</sub> (cfs)	C <sub>100</sub> *A (ac)	SLOPE %	PIPE SIZE (ft)	LENGTH (ft)	VEL. (ft/s)	
20	C1	4.53	0.59	10.9	2.67	6.72	18.0				5.6	0.83	1.0	12.4	1.85	2.0	2.0	820	2.0	6.83	DP20 FLOWBY, C&G FLOW TO DP24
21	C2	2.71	0.59	11.2	1.60	6.64	10.6				1.4	0.21	1.5	9.2	1.39	2.0	2.0	350	2.4	2.38	BASIN C1 CAPTURED IN 15' TYPE R ONGRADE @ DP20, PIPE TO DP21.1
21.1																		5	10.2	0.01	DP21 FLOWBY, C&G FLOW TO DP22
22	C3	2.47	0.59	9.4	1.46	7.09	10.3				2.0	0.29	1.0	9.8	1.38	2.0	2.0	820	2.0	6.83	BASIN C2 CAPTURED IN 15' TYPE R ONGRADE @ DP21, PIPE TO DP21.1
22.1																					COMBINED DP20 & DP21, PIPE TO DP22.1
23	C4	3.09	0.59	18.4	1.82	5.39	9.8				1.8	0.33	0.5	25.3	4.61	1.0	2.0	820	7.2	1.90	DP22 FLOWBY, C&G FLOW TO DP23
24	C5	3.13	0.59	16.8	1.85	5.63	10.4				3.6	0.65	0.5	9.6	1.78	2.0	2.0	350	1.4	4.12	DP21 FLOWBY & BASIN 3 CAPTURED IN 15' TYPE R ONGRADE @ DP22, PIPE TO DP22.1
24.1														11.1	2.02	2.0	2.0	350	1.4	4.12	COMBINED DP21.1 & DP22, PIPE TO DP24.1
25	C6	0.81	0.52	13.2	0.42	6.23	2.6				2.6	0.42	0.8	44.1	8.42	2.0	2.0	605	1.8	5.64	DP23 FLOWBY, C&G FLOW TO DP30
26	C7	6.07	0.59	23.7	3.58	4.76	17.0				8.2	1.49	1.5	11.8	2.20	2.0	2.0	1050	2.4	7.14	DP24 FLOWBY, C&G FLOW TO DP30
27	C8	5.11	0.59	18.6	3.01	5.37	16.2				4.4	0.82	1.5	11.8	2.20	2.0	2.0	270	2.4	1.84	DP22 FLOWBY & BASIN C4 CAPTURED IN 15' TYPE R ONGRADE @ DP24, PIPE TO DP24.1
27.1														25.4	4.70	2.0	2.0	28	10.2	0.05	DP24 FLOWBY, C&G FLOW TO DP30
28	C9	3.50	0.59	15.2	2.07	5.87	12.1				5.4	0.87	1.5	12.4	2.01	2.0	2.0	780	2.4	5.31	DP20 FLOWBY & BASIN C5 CAPTURED IN 15' TYPE R ONGRADE @ DP24, PIPE TO DP24.1
28.1														34.6	6.71	2.0	2.0	30	10.2	0.05	COMBINED DP22.1, DP23 & DP24, PIPE TO DP29.1
29.1														34.6	6.71	2.0	2.0	490	10.2	0.80	BASIN C6 FLOW @ DP25, C&G FLOW TO DP26
30	C10	3.97	0.61	16.5	2.41	5.67	13.6							73.0	15.13	2.0	2.0	1050	2.4	7.14	DP26 FLOWBY, C&G FLOW TO DP31
30.1														11.8	2.20	2.0	2.0	5	10.2	0.01	DP17, DP18, DP25 & BASIN C7 CAPTURED IN 15' TYPE R ONGRADE @ DP26, PIPE TO DP27.1
31	C11	3.64	0.61	15.5	2.21	5.82	12.9							11.8	2.20	2.0	2.0	270	2.4	1.84	DP27 FLOWBY, C&G FLOW TO DP28
31.1														25.4	4.70	2.0	2.0	18	10.2	0.03	BASIN C8 FLOW CAPTURED IN 15' TYPE R ONGRADE, PIPE TO DP27.1
32	C12	0.46	0.59	9.0	0.27	7.20	2.0							25.4	4.70	2.0	2.0	265	10.2	0.43	COMBINED DP26 & DP27, PIPE TO DP28.1
32.1														780	2.4	5.31	2.0	780	2.4	5.31	DP28 FLOWBY, C&G FLOW TO DP30
33	C13	1.66	0.59	11.7	0.98	6.54	6.4							12.4	2.01	2.0	2.0	30	10.2	0.05	DP27 FLOWBY & BASIN C9 CAPTURED IN 15' TYPE R ON GRADE @ DP28, PIPE TO DP28.1
33.1														34.6	6.71	2.0	2.0	490	10.2	0.80	COMBINED DP27.1 & DP29, PIPE TO DP29.1
34	C14	2.37	0.59	11.2	1.40	6.65	9.3							73.0	15.13	2.0	2.0	335	10.2	0.55	COMBINED DP28.1 & DP24.1, PIPE TO DP30.1
35	C15	1.69	0.59	5.0	1.00	8.68	8.7							22.4	4.26	2.0	2.0	30	10.2	0.05	DP23, DP24 & DP28 FLOWBY & BASIN C10 FLOW CAPTURED IN 20' TYPE R SUMP @ DP30, PIPE TO DP30.1
														91.1	19.39	2.0	2.0	5	10.2	0.01	COMBINED DP29.1 & DP20, PIPE TO DP31.1
														18.2	3.70	2.0	2.0				DP26 FLOWBY & BASIN C11 CAPTURED IN 20' TYPE R SUMP @ DP31, PIPE TO DP31.1
														97.6	23.09	2.0	2.0	90	10.2	0.15	COMBINED DP30.1 & DP31, PIPE TO DP32.1
														2.0	0.27	2.0	2.0				BASIN 32 CAPTURED IN 5' TYPE R SUMP, PIPE TO DP32.1
														98.4	23.37	2.0	2.0	32	10.2	0.05	COMBINED DP31.1 & DP32, PIPE TO DP33.1
														6.4	0.98	2.0	2.0				BASIN C13 CAPTURED IN 5' TYPE R SUMP, PIPE TO DP33.1
														102.3	24.34	2.0	2.0	170	10.2	0.28	COMBINED DP32.1 & DP33, PIPE TO POND C
																					<b>TOTAL FLOW ENTERING POND C</b>
																					BASIN C15 FLOW, FOLLOWS HISTORIC DRAINAGE PATTERNS TOWARDS DP35

## **APPENDIX F – DRAINAGE MAPS**



**LEGEND:**

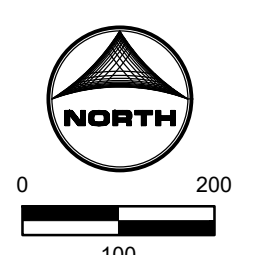
- PROPOSED MAJOR CONTOUR: 5250
- PROPOSED MINOR CONTOUR: 5250
- EXISTING MAJOR CONTOUR: 5250
- EXISTING MINOR CONTOUR: 5250
- PROPOSED STORM DRAIN PIPE
- EXISTING STORM DRAIN PIPE
- PROPOSED DRAINAGE CHANNEL
- PROPOSED ROAD
- PROPERTY LINE
- DIRECTIONAL FLOW ARROW
- EMERGENCY OVERFLOW ARROW
- EXISTING 100-YR FLOODWAY
- EXISTING 100-YR FLOODPLAIN
- PROPOSED 100-YR FLOODPLAIN
- WATERSHED BOUNDARY
- MAJOR BASIN LINE
- 100YR ZONE A FLOODPLAIN
- POTENTIAL WATER QUALITY LOCATION (WQ)
- SWMM CONVEYANCE ELEMENT (SWMM)
- PROPOSED PEAK FLOW RATE (CFS) 850
- DESIGN POINT
- PROPOSED BASIN LABEL: XX BASIN DESIGNATION, XX C5, XX C100
- LAND USE: LOW DENSITY, MEDIUM DENSITY, HIGH/MED DENSITY, HIGH DENSITY, CHURCH, COMMERCIAL, ELEMENTARY SCHOOL, COMMUNITY PARK

**NOTES:**

- DRAINAGE AREA REROUTED AWAY FROM CHANNEL C = 16.1 ac
- DRAINAGE AREA REROUTED TOWARD CHANNEL C = 21.2 ac

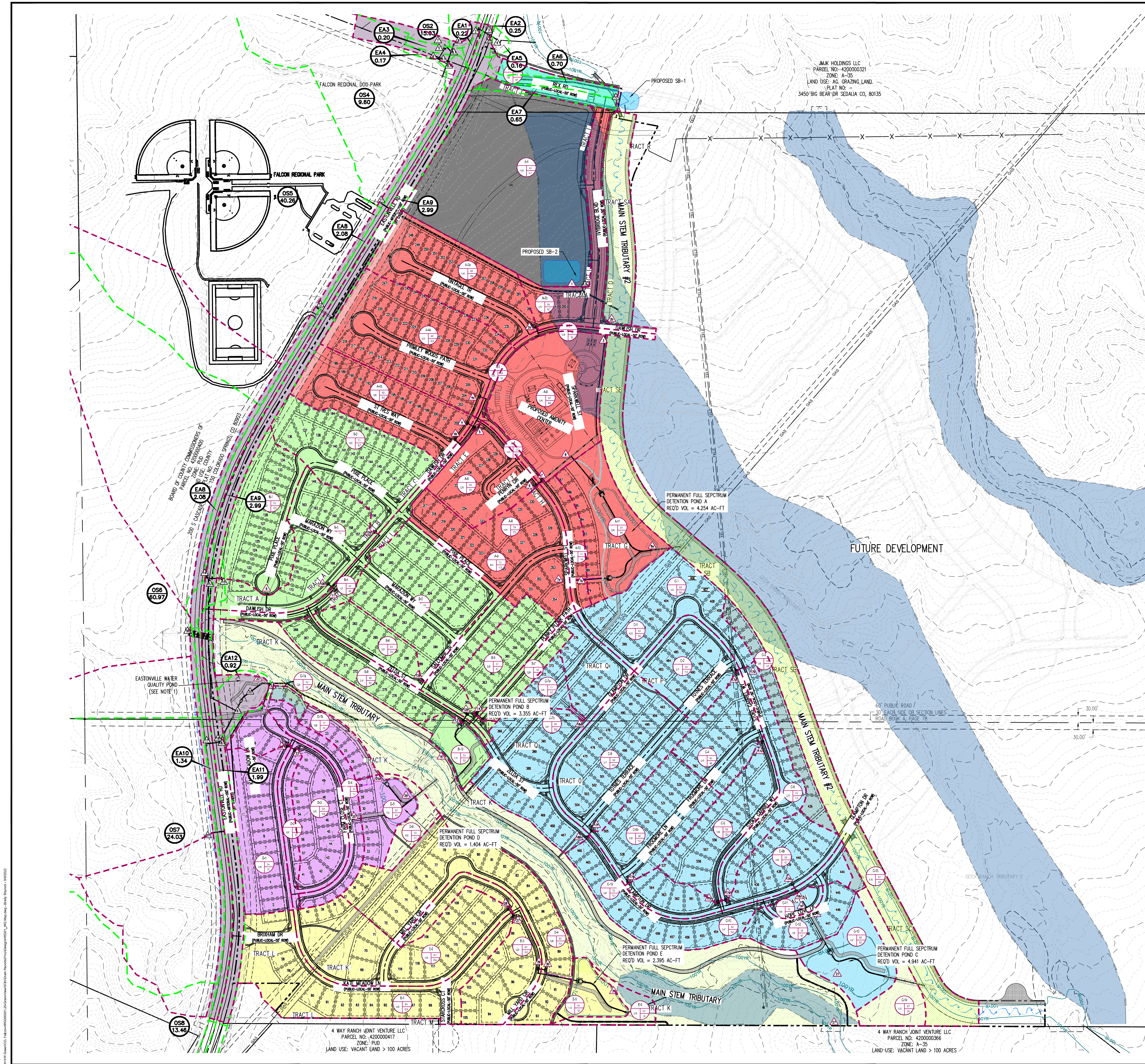
Basin	Design Point	5 Year Pre Development	100 Year Pre Development
A1	10	13.03	66.80
B1	20	4.33	48.76
B2	21	1.66	20.74
	22	11.85	140.35
	23	5.99	68.56
B3	24	21.23	249.20
C1	30	9.95	110.70
	31	9.95	110.70
D1	40	8.12	40.00
D2	41	22.23	114.87
	42	8.12	40.00
E1	50	32.34	157.99
	51	93.34	374.99
F1	60	9.70	49.45
F2	61	16.46	86.73
F3	62	3.65	18.42
F4	63	12.98	67.82
	64	13.35	67.87
	65	26.04	135.62

	66	16.46	86.73
	67	231.47	864.52
G1	70	5.57	28.46
G2	71	3.87	20.06
	72	3.87	20.06
	73	3.87	20.06
	74	189.42	643.48
H1	80	1.85	21.89
H2	81	5.37	27.12
H3	82	1.92	9.51
H4	83	8.07	40.86
	84	7.22	49.01
	85	1.92	9.51
OS1	OS1	67.00	413.00
OS2	OS2	59.00	280.00
OS3	OS3	61.00	217.00
OS4	OS4	180.00	595.00
	Outfall1	80.03	479.80
	Outfall2	85.96	597.41
	Outfall3	30.00	154.35
	Outfall4	341.05	1335.77



Job No.: 191897.01  
 Prepared By: TBI  
 Date: 04/14/2020

EXISTING EX1

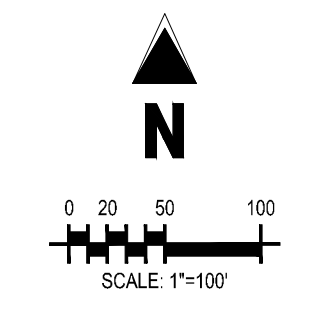


**DRAINAGE LEGEND**

- PROPERTY LINE
  - - - PROPOSED ROAD CENTERLINE
  - BASIN BOUNDARY LINE
  - ▨ EXISTING WETLANDS
  - - - EXISTING LIMITS OF WETLAND
  - - - EXISTING WETLAND SETBACK
  - ▩ EXISTING FEMA FLOOD PLAIN, ZONE A
  - ▬ CENTERLINE OF STREAM
  - ▨ PROPOSED RIPRAP
  - ▨ PROPOSED MAINTENANCE ACCESS
- 
- BASIN DESIGNATION
  - 5-YEAR RUNOFF IN CUBIC FEET PER SECOND
  - 100-YEAR RUNOFF IN CUBIC FEET PER SECOND
  - BASIN AREA IN ACRES
- 
- ▬ FUTURE DEVELOPMENT (NOT PART - TSB PROVIDED)
  - ▬ ROADWAY (DESIGN BY OTHERS - SEE NOTE 1)
  - ▬ AREA TO BE DETAINED IN PBMP (POND A)
  - ▬ AREA TO BE DETAINED IN PBMP (POND B)
  - ▬ AREA TO BE DETAINED IN PBMP (POND C)
  - ▬ AREA TO BE DETAINED IN PBMP (POND D)
  - ▬ AREA TO BE DETAINED IN PBMP (POND E)
  - ▬ AREA NOT TO BE DETAINED IN PBMP PER SECTION 17.1.8.7 (LAND DISTURBANCE TO UNDEVELOPED LAND THAT WILL REMAIN UNDEVELOPED)
  - ▬ AREA TO BE DETAINED IN FUTURE PBMP WITH THE REMAINDER OF THE REX RD DEVELOPMENT - TSB PROVIDED
  - ▬ AREA WHERE WATER QUALITY IS ACHIEVED THROUGH RUNOFF REDUCTION (ROOF DRAINS & IMPERVIOUS SURFACES NOT PERMITTED ON BACK OF THESE LOTS)
  - ▬ AREA TO BE DETAINED IN PUBLIC PBMP (EASTONVILLE ROAD)

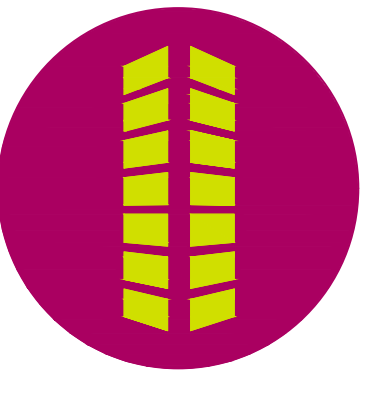
**NOTES:**

1. EASTONVILLE ROAD AND THE WESTERN PORTION OF REX ROAD AT EASTONVILLE IS TO BE CONSTRUCTED CONCURRENTLY WITH THIS DEVELOPMENT. DRAINAGE FOR THESE AREAS WILL BE CAPTURED AND TREATED BY OFFSITE INFRASTRUCTURE AND PONDS AS DESCRIBED IN THE "FINAL DRAINAGE REPORT FOR EASTONVILLE ROAD FROM FUTURE REX ROAD TO LONDONDERRY DRIVE", BY HR GREEN, MARCH 2022. BASIN SHOWN ARE FOR REFERENCE ONLY.



**PRELIMINARY**  
 NOT FOR BIDDING  
 NOT FOR CONSTRUCTION

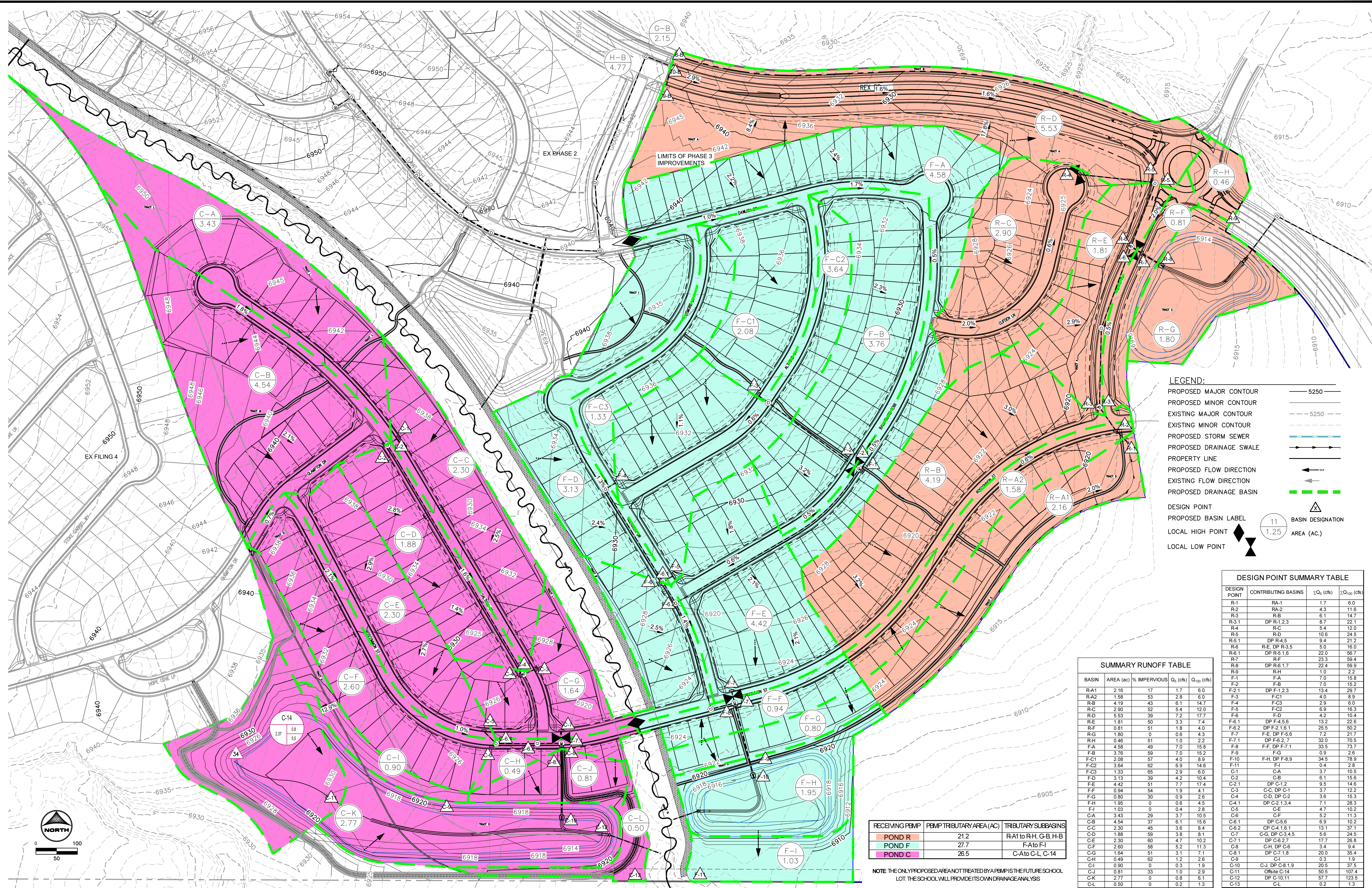
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**PRELIMINARY DRAINAGE PLAN**  
**GRANDVIEW RESERVE FILING NO. 1**  
**FOR**  
**HR GREEN, INC**  
**EASTONVILLE RD**  
**EL PASO COUNTY, PEYTON, CO 80831**

#	Date	Issue / Description	Init.

Project No:	HRG 1.20
Drawn By:	NJA
Checked By:	GRD
Date:	9/9/2022



**LEGEND:**

- PROPOSED MAJOR CONTOUR ——— 5250
- PROPOSED MINOR CONTOUR ——— 5250
- EXISTING MAJOR CONTOUR - - - - - 5250
- EXISTING MINOR CONTOUR - - - - -
- PROPOSED STORM SEWER ———
- PROPOSED DRAINAGE SWALE ———
- PROPERTY LINE ———
- PROPOSED FLOW DIRECTION ———
- EXISTING FLOW DIRECTION ———
- PROPOSED DRAINAGE BASIN ———
- DESIGN POINT  $\Delta$
- PROPOSED BASIN LABEL  $\bigcirc$  11
- LOCAL HIGH POINT  $\blacklozenge$  1.25
- LOCAL LOW POINT  $\blacktriangledown$
- BASIN DESIGNATION  $\Delta$
- AREA (AC.)  $\bigcirc$

RECEIVING PMP	PMP TRIBUTARY AREA (AC)	TRIBUTARY SUBBASINS
POND R	21.2	RA1 to RH, G-B, H-B
POND F	27.7	F-A to F-I
POND C	28.5	C-A to C-L, C-14

NOTE: THE ONLY PROPOSED AREA NOT TREATED BY A PMP IS THE FUTURE SCHOOL LOT. THIS SCHOOL WILL PROVIDE ITS OWN DRAINAGE ANALYSIS.

**SUMMARY RUNOFF TABLE**

BASIN	AREA (ac)	% IMPERVIOUS	Q <sub>2</sub> (cfs)	Q <sub>100</sub> (cfs)
RA1	2.16	17	1.7	6.0
RA2	1.58	53	2.9	6.0
R-B	4.19	43	6.1	14.7
R-C	2.90	52	5.4	12.0
R-D	5.53	39	7.2	17.7
R-E	1.81	50	3.3	7.4
R-F	0.81	51	1.8	4.0
R-G	1.80	0	0.6	4.3
R-H	0.46	51	1.0	2.2
F-A	4.58	49	7.0	15.8
F-B	3.76	59	7.0	15.2
F-C1	2.08	57	4.0	8.9
F-C2	3.64	62	6.9	14.6
F-C3	1.33	65	2.9	6.0
F-D	3.13	39	4.2	10.4
F-E	4.42	51	7.7	17.4
F-F	0.94	54	1.9	4.1
F-G	0.80	30	0.9	2.6
F-H	1.95	0	0.6	4.5
F-I	1.03	0	0.4	2.8
CA	3.43	29	3.7	10.5
CB	4.54	37	6.1	15.6
CC	2.30	45	3.6	8.4
CD	1.88	59	3.8	8.1
CE	2.30	60	4.7	10.2
CF	2.60	58	5.2	11.3
CG	1.64	51	3.1	7.1
CH	0.49	62	1.2	2.6
CI	0.90	0	0.3	1.9
CJ	0.81	33	1.0	2.9
CK	2.77	0	0.8	6.1
CL	0.50	0	0.2	1.3

**DESIGN POINT SUMMARY TABLE**

DESIGN POINT	CONTRIBUTING BASINS	ΣQ <sub>2</sub> (cfs)	ΣQ <sub>100</sub> (cfs)
R-1	RA-1	1.7	6.0
R-2	RA-2	4.3	11.6
R-3	R-B	6.1	14.7
R-3.1	DP R-1,2,3	8.7	22.1
R-4	R-C	5.4	12.0
R-5	R-D	10.6	24.5
R-5.1	DP R-4,5	9.4	21.2
R-6	R-E, DP R-3,5	5.0	16.0
R-6.1	DP R-5,1,6	22.0	56.7
R-7	R-F	23.3	59.4
R-8	DP R-6,7	22.4	59.9
R-9	R-H	1.0	2.2
F-1	F-A	7.0	15.8
F-2	F-B	7.0	15.2
F-2.1	DP F-1,2,3	13.4	29.7
F-3	F-C1	4.0	8.9
F-4	F-C3	2.9	6.0
F-5	F-C2	6.9	16.3
F-6	F-D	4.2	10.4
F-6.1	DP F-4,5,6	13.2	22.6
F-6.2	DP F-1,6.1	25.5	50.2
F-7	F-E, DP F-5,6	7.2	21.7
F-7.1	DP F-6,2,7	32.0	70.5
F-8	F-F, DP F-7,1	33.5	73.7
F-9	F-G	0.9	2.6
F-10	F-H, DP F-8,9	34.5	78.9
F-11	F-I	0.4	2.8
C-1	CA	3.7	10.5
C-2	CB	6.1	15.6
C-2.1	DP C-1,2	9.5	14.6
C-3	CC, DP C-1	3.7	12.2
C-4	CD, DP C-2	3.6	15.3
C-4.1	DP C-2,1,3,4	7.1	28.3
C-5	CE	4.7	10.2
C-6	CF	5.2	11.3
C-6.1	DP C-5,6	6.9	10.2
C-6.2	CP C-4,1,6.1	13.1	37.1
C-7	C-G, DP C-3,4,5	5.6	24.5
C-7.1	DP C-6,2,7	17.7	26.8
C-8	C-H, DP C-6	3.4	9.4
C-8.1	DP C-7,1,8	20.0	35.4
C-9	C-I	0.3	1.9
C-10	C-J, DP C-8,1,9	20.5	37.5
C-11	Offsite C-14	50.5	107.4
C-12	DP C-10,11	57.7	123.5
C-13	CL	0.2	1.3

DRAWN BY: CBM JOB DATE: 3/20/2024  
 APPROVED: KMH JOB NUMBER: 201662  
 CAD DATE: 3/20/2024  
 CAD FILE: J:\2020\201662\CAD\DWG\CIPUD\_Phase\_3\_662.203\Drainage\03\_DR

NO.	DATE	BY	REVISION DESCRIPTION

**HRGreen**  
 HR GREEN - COLORADO SPRINGS  
 1975 RESEARCH PKWY SUITE 230  
 COLORADO SPRINGS CO 80920  
 PHONE: 719.300.4140  
 FAX: 713.965.0044

**GRANDVIEW RESERVE - PHASE 3**  
 D.R. HORTON  
 EL PASO COUNTY, CO

**GRANDVIEW PHASE 3**  
 PROPOSED DRAINAGE MAP

**SHEET DR 1**