EPC STORMWATER REVIEW COMMENTS
IN ORANGE BOXES WITH BLACK TEXT

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HRGreen

Grandview Reserve Phase 3
Preliminary Drainage Report

Unresolved: A final drainage report for the early grading will need to be submitted with the project.

September 2024

HR Green Project No: 201662.203

EPC PCD Filing No.: PUD-SP241

Prepared For:

D.R. Horton

Contact: Riley Hillen, P.E.

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Prepared By:

HR Green Development, LLC

Contact: Ken Huhn, P.E.

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Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Ken Huhn, P.E. Date

State of Colorado No. 54022

For and on behalf of HR Green Development, LLC

Owner/Developer's Statement:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

By: Date

Authorized Signature

Address: D.R. Horton
9555 S. Kingston Court
Englewood, CO

El Paso County Statement

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development code, as amended.

Joshua Palmer, P.E. Date

County Engineer/ECM Administrator

Conditions:

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I. General Purpose, Location and Description

a. Purpose

The purpose of this Preliminary Drainage Report (PDR) for the Grandview Reserve Subdivision Phase 3 is to describe the onsite and offsite drainage patterns, size drainage infrastructure to safely capture and convey developed runoff to water quality and detention facilities, and to safely route detained stormwater to adequate outfalls.

b. Location

The Grandview Reserve Phase 3 site is located in unincorporated El Paso County, Colorado. The Phase 3 location (referred to as the site herein) is located west of Grandview Reserve Filings 1-4 and MST2, south of Grandview Reserve Phase 2, and east of Rex Road.

The site lies within a tract of land within Sections 21, 22, 27 and 28, Township 12 South, Range 64 West of the 6th Principal Meridian, in El Paso County, State of Colorado. A Vicinity Map is included in **Appendix A**.

The site is bound by a segment of Rex Road to be developed with this project to the east and undeveloped land that has historically been used as ranching lands. The northwest of the site is bound by Phase 2 of the subdivision. The west of the site is bound by Grandview Reserve Filings 1-4 and MST2. A vicinity map is presented in Appendix A.

The Gieck Ranch Tributary #2 “MST2” (Channel B) is a part of the Gieck Ranch Drainage Basin tributary to Black Squirrel Creek. The channel draining through the site is an ongoing project with associated CLOMR Report and the PCD File No. is CDR228 with El Paso County. The Grandview Reserve improvements will follow any requirements of that report. There is another floodplain channel to the north of Rex Road that will not be disturbed by this phase of development and studies as a future project.

The existing surrounding platted developments include the Grandview Reserve Phase 1 Filings 1-4 and Grandview Reserve Phase 2.

c. Description of Property

The site is approximately 102.82 acres of proposed residential development with associated right of way, open space tracts, public improvements, and stormwater treatment infrastructure.

The existing groundcover and topography of the site is native grasses/weeds and exposed soil on gently rolling hillside with slopes ranging from 2% to 4%.

Per a NRCS soil survey, the site is made up of Type A Columbine gravelly sandy loam. The NRCS soil survey is presented in **Appendix A**.

There is one major drainageway through the site. The Gieck Ranch Tributary #2 (MST2 in the MDDP) traverses through the site and divides it in two. The drainageway generally flows from the northwest to the southeast towards Highway 24, before crossing through existing drainage infrastructure. The CLOMR report by HR Green for MST2 is ongoing and pending approval for this channel. Refer to the CLOMR report included in **Appendix E** for more specific design information regarding the MST2 channel. Gieck Ranch Tributary #3 (future Channel C) traverses the site along its northeastern boundary and forms the northeast boundary for Phase 3 along Rex Road. The initial analysis of this drainage way has been performed by HR Green in

conjunction with Phase 2. The analysis delineated the 100-yr floodplain and ensures the construction of Rex Road will not impact the floodplain. No fill will be placed in this channel in this phase of development. A CLOMR report is not required by the County at this time.

There are no known irrigation facilities in the area.

There are no known existing utilities or other encumbrances on site.

d. Floodplain Statement

Based on FEMA Firm map 08041C0552G & 08041C0556G (eff. 12/7/2018), the site contains flood Zone A through the site which is part of the Gieck Ranch Tributary #2. See FEMA Firm Maps in **Appendix A**. This floodplain is being studied and revised in the Gieck Ranch Tributary # 2 CLOMR report. A copy of the current revised floodplain map is also provided in **Appendix A**. There is a Zone A floodplain northeast of the site which will not be altered with this project's improvements.

II. Drainage Design Criteria

a. Drainage Criteria

Hydrologic data and calculations were performed using Drainage Criteria Manual Volume 1 of El Paso County (EPCDCM), with County adopted Chapter 6 and Section 3.2.1 of Chapter 13 of the City of Colorado Springs Drainage Criteria Manual (CCSDCM), May 2014 revised January 2021.

Onsite drainage improvements are designed for the 5-year storm (minor event) and 100-year storm (major event) using rainfall values from the NOAA Atlas 14 Point Precipitation Frequency Data Server. Runoff was calculated per CCSDCM Section 6.3.0 - Rational Method. Private, full spectrum pond design was completed using the latest version of Mile High Flood District's (MHFD) UD-Detention per CCSDCM Section 13.3.2.1 – Private, full spectrum Detention. Detention pond allowable release rate will be limited to less than historic rates.

Rainfall Depths per NOAA Atlas 14		
Return Period (yr)	5	100
1-hr Rainfall Depth (in)	1.21	2.49

Storm sewer and inlet sizing shown is preliminary at this stage of the project. Calculations for the storm sewer system on site will be provided with the Final Drainage Report (FDR) for the project. The sizing methodology that will be used is per the methods described in EPCDCM Section III Chapter 7 – Street Drainage and Storm Water Inlets. Storm sewer sizing was performed per the methods described in EPCDCM Section III Chapter 8 – Storm Drains and Appurtenances.

This preliminary drainage report follows any recommendations and is in conformance with the previously approved MDDP for the site prepared by HR Green, "*Grandview Reserve Master Development Drainage Plan",* HR Green, November 2020 (MDDP).

III. Drainage Basins and Subbasins

a. Major Basin Description

The site is located within the Gieck Ranch Drainage Basin. The site's drainage characteristics were previously studied in the following reports:

1. "Gieck Ranch Drainage Basin Planning Study" prepared by Drexel, Barrel & Co, February 2010.
2. "Grandview Reserve Master Development Drainage Plan" prepared by HR Green, August 2021.
3. "Grandview Reserve Filing No. 1 Preliminary Drainage Report" prepared by Galloway & Company, Inc., September 2022.
4. "Grandview Reserve CLOMR REPORT" prepared by HR Green, March 2023

Gieck Ranch Drainage Basin is a 22.05 square mile watershed located in El Paso County, Colorado. Gieck Ranch Drainage Basin is tributary to Black Squirrel Creek which drains to the Arkansas River. The majority of the basin is undeveloped and rolling range land of 2% - 4% slopes.

The Grandview Reserve MDDP divided the site into 8 major drainage basins (A-H), where each basin is tributary to a full spectrum detention pond facility. The Grandview Reserve Phase 3 improvements are located in subbasins B3 and D1 of the MDDP.

There are no known existing irrigation facilities or other obstructions that could influence or will be influenced by local drainage characteristics. Proposed local drainage characteristics will continue to follow historic patterns. The offsite flows entering Phase 3 from Phase 2 have been accounted for in the drainage calculations.

b. Existing Subbasin Description

The Grandview Reserve Phase 3 site drains from the northwest to the southeast slopes ranging from 2% - 4%. The majority of the site has historically drained into the Gieck Ranch Tributary #2 (existing MST2), with portions draining to East Fork Tributary (EFT).

The existing subbasins for the Grandview Reserve Phase 3 site were studied in the approved MDDP for Grandview Reserve. This site is located within subbasins B3 and D1 of this report and are described per the MDDP as follows.

"Subbasin B3 is located between MS and EF and to the northeast of basin B2. The existing MST2 tributary runs through the basin. The site drains towards the southeast and towards (proposed) Detention Pond B. Current planning documents call for high, medium-high, and medium density dwelling units along with a pocket park. The basin is 118.90 acres, with a composite impervious value of 49.42% and runoff rates for the 5 and 100 year of 92.76 cfs and 295.27 cfs respectively."

"Subbasin D1 is located between MS and MST2 to the east of Basin B3 and adjacent to the MST2 channel. The basin drains towards the southeast and towards drainage basin D2. Current planning documents call for medium density dwelling units along with a pocket park. The basin is 24.33 acres, with a composite impervious value of 53.89% and runoff rates for the 5 and 100 year of 24.15 cfs and 70.07 cfs respectively."

A copy of the approved MDDP has been included in **Appendix E** of this report. The proposed drainage conditions for this development will follow historic drainage patterns as described in the MDDP.

Unresolved: If this is proposed conditions, please move to that section of report.

c. Proposed Subbasin Description

Description of Proposed Project

The proposed drainage conditions for the site generally follow historic drainage patterns. The site drains from the northwest to the southeast at slopes between 0.6% - 4%, into proposed public storm sewer systems via sheet flow/curb & gutter/channel flow which drain to proposed private extended detention basins for treatment and flood attenuation.

The northeastern corner of the site will drain to and be treated by "Pond R", which will outfall in the EFT, henceforth being referred to as "Channel C". Although the tributary area of Pond R would historically drain to MST2, it will now outfall in Channel C. The increase in discharge volume to Channel C will be negligible as a significant tributary area of Phase 2 Pond A and Pond B historically drained to Channel C and will now outfall into MST2. A comparison of the drainage areas is overlayed on the existing drainage map. Any increase in volume entering the channel will be mitigated with slope stabilization measures.

The west and south portions of the site will drain to and be treated by two separate detention ponds, "Pond F," and "Pond C." Both Pond F and Pond C will outfall into the rerouted channel MST2, henceforth being referred to as "Channel B." Drainage from both of these ponds has been accounted for in the channel re-alignment design and is detailed in the CLOMR report.

The 25.0 acre future school site will remain undeveloped and will not be accounted for in this report. Water quality treatment and detention will need to be provided on the future school site. A temporary sediment basin has been established at the future school site pond location to capture runoff not detained by the aforementioned permanent ponds.

There are minor offsite flows from Phase 2 which are accounted for.

Subbasins Tributary to and Treated by Pond R

Basin R-A1 is 1.20 acres of landscaped area. Stormwater (Q_5 = 2.6 cfs Q_{100} = 6.0 cfs) is conveyed via sheet flow to a 2' temporary grassed swale at the southern border of the site. Runoff captured combines with flow from DP F-11 and is conveyed east to a temporary sediment basin, which will be converted to an FSD Pond upon development of the future school site. This basin is treated as a WQCV exclusion with the current development. Total WQCV exclusions for the site consist of 3.68 acres and 4.9% of the overall drainage area.

Basin R-A2 is 2.54 acres of El Paso County (EPC) Local right-of-way (ROW), landscaped area and 50' lots area. Stormwater (Q_5 = 5.9 cfs Q_{100} = 13.5 cfs) is conveyed via curb and gutter to a 15' at grade type R inlet at DP R-2 and ultimately drains to Pond R via the proposed public storm sewer network.

Basin R-B is 4.19 acres of EPC Local ROW area, landscaped area, and 35' lots area. Stormwater (Q_5 = 6.1 cfs Q_{100} = 14.7 cfs) is conveyed via curb and gutter to a type R inlet at DP R-3 and ultimately drains to Pond R via the proposed public storm sewer network. Flowby from DP R-3 is captured at DP R-6 sump inlet.

Basin R-C is 2.90 acres of EPC Local ROW area, and 35' lots area. Stormwater (Q_5 = 5.4 cfs Q_{100} = 12.0 cfs) is conveyed via curb and gutter to a type R sump inlet at DP R-4 and ultimately drains to Pond R via the proposed public storm sewer network. In the event of inlet clogging, stormwater will overtop the curb and flow via an overflow swale to Rex Road.

Basin R-D is 5.53 acres of EPC Minor Arterial ROW area, landscaped area, and 35' lots area. Stormwater (Q_5 = 7.2 cfs Q_{100} = 17.7 cfs) is conveyed via curb and gutter to a type R inlet at DP R-5. Flowby stormwater from

Grandview Phase 2 Basin G-B, H-B is also captured at DP R-5 and ultimately drains to Pond R via the proposed public storm sewer network. Flowby from DP R-5 is captured at DP R-6 sump inlet.

Basin R-E is 1.81 acres of EPC Local ROW area, 35' lots area. Stormwater (Q_5 = 3.3 cfs Q_{100} = 7.4 cfs) is conveyed via curb and gutter to a type R sump inlet at DP R-6 and ultimately drains to Pond R via the proposed public storm sewer network.

Basin R-F is 0.81 acres of EPC Local ROW area and landscaped area. Stormwater (Q_5 = 1.8 cfs Q_{100} = 4.0 cfs) is conveyed via curb and gutter to a type R sump inlet at DP R-7 and ultimately drains to Pond R via the proposed public storm sewer network.

Basin R-G is 1.80 acres of landscaped detention facility area. Stormwater (Q_5 = 0.6 cfs Q_{100} = 4.3 cfs) is conveyed via sheet flow in Tract C and flows directly into Pond R at DP R-8.

Basin R-H comprises 0.46 acres of the northeastern section of proposed roundabout geometry & undeveloped area. The northern and eastern approaches are temporary dead ends until development of Grandview future phases, therefore, runoff (Q_5 = 1.0 cfs Q_{100} = 2.2 cfs) discharges directly to the East Fork Tributary. This basin is treated as a WQCV treatment exclusion with this development. Upon development of future phases a majority of this basin will be able to be treated with future ponds. Total WQCV exclusions for the site consist of 3.68 acres and 4.9% of the overall drainage area.

what exclusion?

Subbasins Tributary to and Treated by Pond F

Basin F-A is 4.58 acres of EPC Local ROW area, landscaped area, and 35' lots area. Stormwater (Q_5 = 7.0 cfs Q_{100} = 15.8 cfs) is conveyed via curb and gutter to a type R sump inlet at DP F-1 and ultimately drains to Pond F via the proposed public storm sewer network.

Basin F-B is 3.76 acres of EPC Local ROW area and 40' lots area. Stormwater (Q_5 = 7.0 cfs Q_{100} = 15.2 cfs) is conveyed via curb and gutter to a type R sump inlet at DP F-2 and ultimately drains to Pond F via the proposed public storm sewer network.

Basin F-C1 is 2.08 acres of EPC Local ROW area, landscaped area, and 40' lots area. Stormwater (Q_5 = 4.0 cfs Q_{100} = 8.9 cfs) is conveyed via curb and gutter to a type R inlet at DP F-3 and ultimately drains to Pond F via the proposed public storm sewer network. Flowby from DP F-3 will flow to DP F-5 via curb and gutter.

Basin F-C2 is 3.64 acres of EPC Local ROW area and 40' lots area. Stormwater (Q_5 = 6.9 cfs Q_{100} = 14.6 cfs) is conveyed via curb and gutter to a type R inlet at DP F-5 and ultimately drains to Pond F via the proposed public storm sewer network. Flowby from DP F-5 is captured in DP F-7 sump inlet.

Basin F-C3 is 1.33 acres of EPC Local ROW, landscaped area, and 40' lots area. Stormwater (Q_5 = 2.9 cfs Q_{100} = 6.0 cfs) is conveyed via curb and gutter to a type R inlet at DP F-4 and ultimately drains to Pond F via the proposed public storm sewer network. Flowby from DP F-4 is routed to DP F-5.

Basin F-D is 3.13 acres of EPC Local ROW, landscaped area, and 50' lots area. Stormwater (Q_5 = 4.2 cfs Q_{100} = 10.4 cfs) is conveyed via curb and gutter to a type R inlet at DP F-6 and ultimately drains to Pond F via the proposed public storm sewer network. Flowby from DP F-6 is captured in DP F-7 sump inlet.

Basin F-E is 4.42 acres of EPC Local ROW, landscaped area, 35' lots area, 40' lots area, and 50' lots area. Stormwater (Q_5 = 7.7 cfs Q_{100} = 17.4 cfs) is conveyed via curb and gutter to a type R sump inlet at DP F-7 and ultimately drains to Pond F via the proposed public storm sewer network. In the event of inlet clogging, stormwater from DP F-7 is permitted to overtop the crown of the road and be captured at DP R-8 sump inlet.

Unresolved from Submittal 1: F-I is not entirely undeveloped. Discuss potential applicable exclusions.Based on the maps a trail or maintenance roadway appear to also be in this sub-basin

Basin F-F is 0.94 acres of EPC Local ROW, landscaped area, and 50' lots area. Stormwater (Q_5 = 1.9 cfs Q_{100} = 4.1 cfs) is conveyed via curb and gutter to a type R sump inlet at DP F-8 and ultimately drains to Pond F via the proposed public storm sewer network. In the event of inlet clogging, stormwater at DP F-8 will overtop the curb and follow an overflow swale to Pond F.

Basin F-G is 0.63 acres of landscaped area and 50' lots area. Stormwater (Q_5 = 0.9 cfs Q_{100} = 2.2 cfs) is conveyed via sheet flow and drains directly to Pond F at DP F-9.

Basin F-H is 1.64 acres of landscaped detention facility area. Stormwater (Q_5 = 0.5 cfs Q_{100} = 3.8 cfs) is conveyed via sheet flow and drains directly to Pond F at DP F-10.

Basin F-I is 1.52 acres of undeveloped area. Stormwater (Q_5 = 3.6 cfs Q_{100} = 8.3 cfs) is collected in a 2' temporary grassed swale at DP F-11, where runoff is then conveyed east to a temporary sediment basin. Future development of the school site will introduce a permanent pond at this location. Total WQCV exclusions for the site consist of 3.68 acres and 4.9% of the overall drainage area.

For this basin describe the applicable exclusions for that basin alone.

Subbasins Tributary to and Treated by Pond C

Basin C-A is 3.43 acres of EPC Local ROW area, landscaped area, and 50' lots area. Stormwater (Q_5 = 3.7 cfs Q_{100} = 10.5 cfs) is conveyed via curb and gutter to a type R inlet at DP C-1 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-1 will flow to DP C-3 via curb and gutter.

Basin C-B is 4.54 acres of EPC Local ROW area, landscaped area,40' lots area, and 50' lots area. Stormwater (Q_5 = 6.1 cfs Q_{100} = 15.6 cfs) is conveyed via curb and gutter to a type R inlet at DP C-2 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-2 will flow to DP C-4 via curb and gutter.

Basin C-C is 2.30 acres of EPC Local ROW area, landscaped area, and 50' lots area. Stormwater (Q_5 = 3.6 cfs Q_{100} = 8.4 cfs) is conveyed via curb and gutter to a type R inlet at DP C-3 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-3 is captured by CP C-7 sump inlet.

Basin C-D is 1.88 acres of EPC Local ROW and 40' lots area. Stormwater (Q_5 = 3.8 cfs Q_{100} = 8.1 cfs) is conveyed via curb and gutter to a type R inlet at DP C-4 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-4 is captured by CP C-7 sump inlet.

Basin C-E is 2.30 acres of EPC Local ROW and 40' lots area. Stormwater (Q_5 = 4.7 cfs Q_{100} = 10.2 cfs) is conveyed via curb and gutter to a type R inlet at DP C-5 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-5 is captured by CP C-7 sump inlet.

Basin C-F is 2.60 acres of EPC Local ROW, landscaped area, and 40' lots area. Stormwater (Q_5 = 5.2 cfs Q_{100} = 11.3 cfs) is conveyed via curb and gutter to a type R inlet at DP C-6 and ultimately drains to Pond C via the proposed public storm sewer network. Flowby from DP C-6 is captured by CP C-8 sump inlet.

Basin C-G is 1.64 acres of EPC Local ROW, landscaped area, 40' lots area, and 50' lots area. Stormwater (Q_5 = 3.1 cfs Q_{100} = 7.1 cfs) is conveyed via curb and gutter to a type R sump inlet at DP C-7 and ultimately drains to Pond C via the proposed public storm sewer network. In the event of inlet clogging, stormwater from DP C-7 is permitted to overtop the crown of the road and be captured at DP C-8 sump inlet.

Basin C-H is 0.49 acres of EPC Local ROW, landscaped area, and 40' lots area. Stormwater (Q_5 = 1.2 cfs Q_{100} = 2.6 cfs) is conveyed via curb and gutter to a type R sump inlet at DP C-8 and ultimately drains to Pond C via the proposed public storm sewer network. In the event of inlet clogging, stormwater at DP C-8 will overtop the curb and follow an overflow swale to Pond F.

Basin C-I is 0.90 acres of landscaped area. Stormwater (Q_5 = 0.3 cfs Q_{100} = 1.9 cfs) is conveyed via swale flow and sheet flow at DP C-9 and drains directly into Pond C.

Basin C-J is 0.81 acres of landscaped area and 40' lots area. Stormwater (Q_5 = 1.0 cfs Q_{100} = 2.9 cfs) is conveyed via sheet flow at DP C-10 and drains directly into Pond C.

Offsite Basins C1 - C14 is 43.50 acres of Grandview Filing 4 subdivision. Stormwater (Q_5 = 50.5 cfs Q_{100} = 107.4 cfs) is conveyed via offsite storm network and trickle channel flow at DP C-11 and flows directly into Pond C.

Basin C-K is 3.01 acres of landscaped detention facility area. Stormwater (Q_5 = 0.9 cfs Q_{100} = 6.6 cfs) is conveyed via trickle channel flow at DP C-12 and drains directly into Pond C.

Basin C-L is 0.50 acres of undeveloped area & proposed gravel trail. Stormwater (Q_5 = 1.2 cfs Q_{100} = 8.8 cfs) follows historic drainage patterns and sheet flows offsite at DP C-13. Basin C-L is accounted as a WQCV exclusion area of the site. Total WQCV exclusions for the site consist of 3.68 acres and 4.9% of the overall drainage area.

what exclusion applies?

Make this sentence a new paragraph it is not tied to only Basin C-L. Clarify which basins

IV. Drainage Facility Design

a. General Concept

The proposed improvements will generally follow historic drainage patterns. Inlets will be placed at low points and in the public ROW where the street capacity would be exceeded. Stormwater from the development will be routed via a proposed public storm sewer system to a full spectrum detention pond which release runoff into Channel B and Channel C. All ponds and water quality features will discharge the design storms at less than rates specified in the Grandview reserve MDDP.

b. Water Quality & Detention

Pond R (Full Spectrum Detention Basin)

Water quality and detention for Basins RA-1 through R-G is provided in Pond R; a private, full spectrum extended detention basin within Phase 3 of Grandview Reserve. A total of 19.29 acres at 42% composite imperviousness will be detained. The pond has been sized to provide water quality treatment, and detention for up to the 100-yr storm volume to be released at or below historic rates. The WQCV is 0.298 ac-ft, the EURV is 0.897 ac-ft, and the 100-year detention volume is 1.373 ac-ft. The WQCV, EURV and 100-year storms are released in 40, 72 and 77 hours, respectively. A forebay is located at the outfall into the pond and a 6.0' trickle channel conveys flow towards the outlet structure. A 15' access and maintenance road is provided to the bottom of the pond to facilitate maintenance of the pond facilities. A 9' emergency overflow spillway is provided that conveys the developed, peak 100-yr flow rate with 1.0' of freeboard towards Channel C.

Pond F (Full Spectrum Detention Basin)

Water quality and detention for Basins F-A through F-H is provided in Pond F; a private, full spectrum extended detention basin within Phase 3 of Grandview Reserve. A total of 26.63 acres at 49% composite imperviousness will be detained. The pond has been sized to provide water quality treatment, and detention for up to the 100-yr storm volume to be released at or below historic rates. The WQCV is 0.461 ac-ft, the EURV is 1.540 ac-ft, and the 100-year detention volume is 2.353 ac-ft. The WQCV, EURV and 100-year storms are released in 40, 71 and 74 hours, respectively. A forebay is located at the outfall into the pond and

a 6.0' trickle channel conveys flow towards the outlet structure. A 15' access and maintenance road is provided to the bottom of the pond to facilitate maintenance of the pond facilities. A 30.0' emergency overflow spillway is provided that conveys the developed, peak 100-yr flow rate with 1.0' of freeboard towards Channel B.

Pond C (Full Spectrum Detention Basin)

Water quality and detention for offsite Basins C1- C14, and onsite Basins C-A through C-K is provided in Pond C; a private, full spectrum extended detention basin majority within Phase 3 of Grandview Reserve, and partially within Filing 4 of Grandview Reserve. A total of 67.16 acres at 53% composite imperviousness will be detained. The pond has been sized to provide water quality treatment, and detention for up to the 100-yr storm volume to be released at or below historic rates. The WQCV is 1.209 ac-ft, the EURV is 4.205 ac-ft, and the 100-year detention volume is 6.247 ac-ft. The WQCV, EURV and 100-year storms are released in 40, 71 and 72 hours, respectively. A forebay is located at the outfall into the pond and a 6.0' trickle channel conveys flow towards the outlet structure. A 15' access and maintenance road is provided to the bottom of the pond to facilitate maintenance of the pond facilities. A 75.0' emergency overflow spillway is provided that conveys the developed, peak 100-yr flow rate with 1.0' of freeboard towards Channel B.

c. Channel Improvements

The Gieck Ranch Tributary #2 is proposed to be rerouted. As part of this rerouting of the channel, offsite upstream tributary flows will be captured upstream from the proposed Rex Road extension and be conveyed via culvert to the rerouted channel along the Grandview Reserve Phase 2 western boundary. An analysis has been done for the channel with both existing and future condition flows as described within the Grandview Reserve CLOMR Report, HR Green; September 2021; revised January 2022 (CLOMR). Both scenarios, throughout the channel fall within the channel stability criteria. Channel improvement construction plans have been submitted to El Paso County for review as a separate project (#CDR228).

d. Inspection and Maintenance

After completion of construction and upon the Board of County Commissioners acceptance, it is anticipated that all drainage facilities within the public Right-of-Way are to be owned and maintained by El Paso County.

All private detention ponds are to be owned and maintained by the Grandview Reserve Metropolitan District NO. 2 (DISTRICT), once established, unless an agreement is reached stating otherwise. Maintenance access for all full spectrum detention facilities will be provided from public Right-of-Way. Maintenance access for the drainageways will be provided through the proposed tracts.

V. Wetlands Mitigation

There is one existing wetlands on site associated with the Gieck Ranch Tributary #2. The wetlands are contained within the existing channel and classified as non-jurisdictional. The wetlands USACE determination will be provided with the Grandview Reserve CLOMR Report, HR Green; April 2022, which can be found in **Appendix E**. Wetlands maintenance will be the responsibility of the Grandview Reserve Metropolitan District No. 2.

For C-L, what are exclusions proposed for that area that will not receive treatment. For any other areas not addressed in the PDR such as F-I,R-A1, and R-H that are stated to be treated later, those will need to be addressed with their respective FDRs.

Unresolved. Please expand further. In the MDDP Summary submitted with the Sketch Plan (SKP-20-001) it is noted that due to the increased volume of flow, low impact design should be taken into account for design of each filing. Please include additional information on where and how this is being accomplished within such a high density area.

VI. Four Step Method to Minimize Urbanization

Step 1 – Reducing Runoff Volumes: Low impact development (LID) practices are utilized to reduce runoff at the source. In general, stormwater discharges are routed across pervious areas prior to capture in storm sewer. This practice promotes infiltration and reduces peak runoff rates. Rear and side yard swales are also being utilized in the design to promote infiltration from individual lot impervious areas. Roof drainas will be routed to these areas minimizing directly connected impervious areas. Larger lots were utilized where possible to increase open area to impervious roof ratios and reduce overall density. The Impervious Reduction Factor (IRF) method will be used in the final design and calculations provided with the FDR.

Step 2 – Treat and slowly release the WQCV: This step utilizes full spectrum water quality and detention to capture the WQCV and slowly release runoff from the site. Onsite full spectrum detention pond provides water quality treatment for the site. The WQCV is released over a period of 40 hours while the EURV is released over a period of 68-72 hours. A small portion of the site is unable to be treated with Phase 3 development due to topographical/grading constraints. This area is discussed below:

C-L?

Basins C-L, F-I, R-A1, and R-H (cumulative 3.68 acres and 4.9% of overall drainage area) are not treated with this development. Basin C-I will discharge directly to the Main Stem Tributary 2. Basins F-I and R-A1 will be treated with development of the future school site. A temporary grassed swale collects flows along the southern border of the site, conveying them to a temporary sediment basin located at the future pond location of the school site. Basin R-H will be directly discharged to the East Fork Tributary. A majority of Basin R-H will be able to be treated upon development of Grandview Future Phases.

Step 3 – Stabilize stream channels: This step establishes practices to stabilize drainageways and provide scour protection at stormwater outfalls. Erosion protection is provided at all concentrated stormwater discharge points in the form of riprap pads.

Step 4 – Consider the need for source controls: No industrial or commercial uses are proposed within this development and therefore no source controls are proposed.

VII. Drainage and Bridge Fees

Gieck Ranch drainage basin has not been established as a fee basin within El Paso County. Therefore, no drainage basin fees are due at time of platting.

VIII. Opinion of Probable Cost

An engineer's opinion of probable cost will be provided with the Final Drainage Report (FDR) for the site.

IX. Hydraulic Grade Line Analysis

Hydraulic grade line analysis and final pipe sizes will be provided with the FDR for the site.

X. Summary

The Grandview Reserve Phase 3 site lies within the Gieck Ranch Drainage Basin. Water quality and detention for the site is provided in full spectrum water quality and detention ponds. There is one major drainageway that traverses the site: Gieck Ranch Tributary #2. The water quality and detention features

ponds will be maintained by the Grandview Reserve Metropolitan District No. 2 (DISTRICT). All drainage facilities were sized per the El Paso County Drainage Criteria Manuals.

The development of this project will not adversely affect adjacent or downstream properties.

XI. Drawings

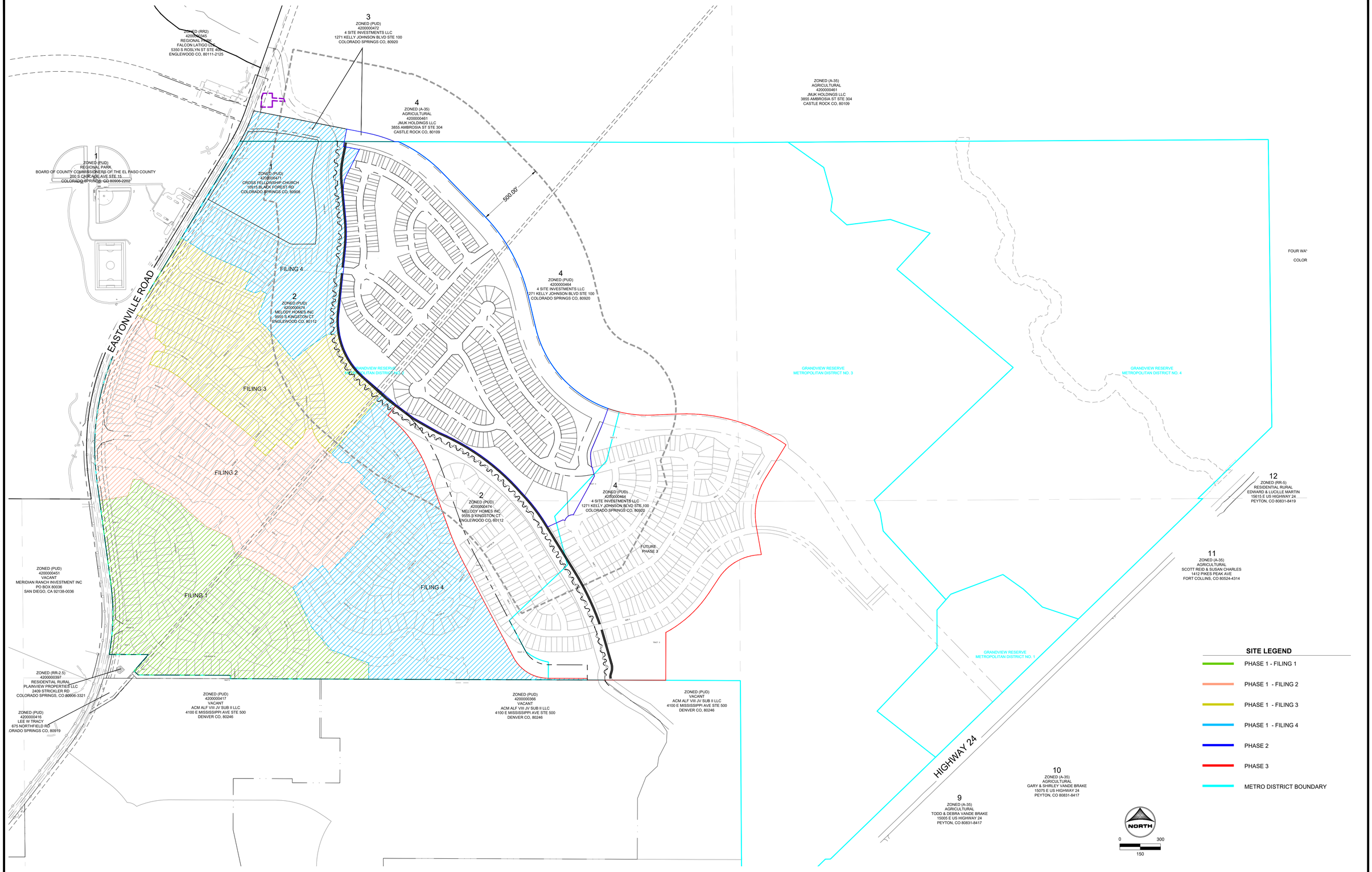
Refer to the appendices for vicinity and drainage basin maps.

XII. References

1. City of Colorado Springs – Drainage Criteria Manual, May 2014, Revised January 2021.
2. Drainage Criteria Manual of El Paso, Colorado, October 2018.
3. Urban Storm Drainage Criteria Manual, Urban Drainage Flood Control District, January 2018.
4. “Gieck Ranch Drainage Basin Planning Study” prepared by Drexel, Barrel & Co, February 2010.
5. “Grandview Reserve Master Development Drainage Plan” prepared by HR Green, August 2021.
6. “Grandview Reserve Filing No. 1 Preliminary Drainage Report” prepared by Galloway & Company, Inc., September 2022.
7. “Grandview Reserve CLOMR REPORT” prepared by HR Green, March 2023

APPENDIX A – VICINITY MAP, PHOTOS, SOIL MAP, FEMA MAP

HERGER, DANIEL, 12/15/2023 9:06 AM



DRAWN BY:	DLH	JOB DATE:	----
APPROVED:	KMH	JOB NUMBER:	201662.2
CAD DATE:	12/15/2023		
CAD FILE:	J:\2020\201662\CAD\Dwgs\C\PUD_Phase_2_662.202\Exhibit\Vicinity_Map		

BAR IS ONE INCH ON OFFICIAL DRAWINGS.
0 1"
IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

HRGreen
HR GREEN - COLORADO SPRINGS
1975 RESEARCH PKWY SUITE 230
COLORADO SPRINGS CO 80920
PHONE: 719.300.4140
FAX: 713.965.0044

GRANDVIEW RESERVE - PHASE 2
D.R. HORTON
EL PASO COUNTY, CO

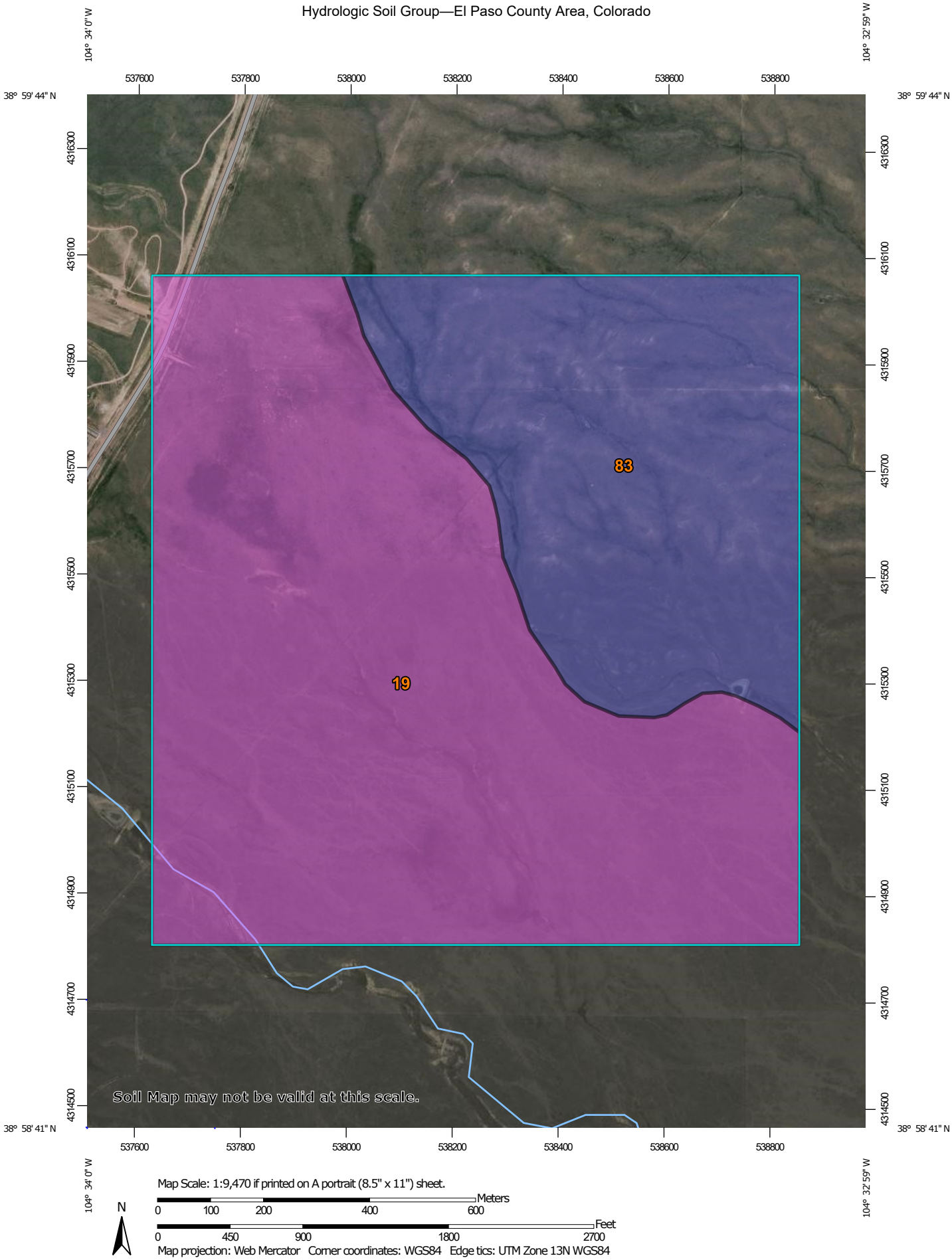
D·R·HORTON America's Builder

GRANDVIEW PHASE 2
VICINITY MAP

SHEET
MAP

1

HR GREEN Xrefs: xgl-1-arch dh01_PH2_PUD; xv-row-662; xv-row-F4; xc-row-F3; xc-row-F2; xc-row-F1-662.10; xc-row_PH2; 01-XC-channel; xc-row-PH3; xc-row_662.09; Eastonville; xv-dsgn_662; xc-filing-bndys


104° 34' 0" W
104° 32' 59" W
38° 59' 44" N
38° 58' 41" N
537600
537800
538000
538200
538400
538600
538800
4316300
4316100
4315900
4315700
4315500
4315300
4315100
4314900
4314700
4314500
83
19
Soil Map may not be valid at this scale.
Map Scale: 1:9,470 if printed on A portrait (8.5" x 11") sheet.
N
Meters
0 100 200 400 600
Feet
0 450 900 1800 2700
Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 20, Sep 2, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 11, 2018—Jun 12, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	254.0	66.5%
83	Stapleton sandy loam, 3 to 8 percent slopes	B	127.8	33.5%
Totals for Area of Interest			**381.8**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

104°35'37.96"W 39°0'6.61"N

104°33'44.61"W 38°58'N

FLOOD HAZARD INFORMATION

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR DRAFT FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee See Notes *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 Cross Sections with 1% Annual Chance
- 17.5 Water Surface Elevation
- 8 Coastal Transect
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature
- 513 Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary

NOTES TO USERS

For information and questions about this Flood Insurance Rate Map (FIRM), available products associated with this FIRM, including historic versions, the current map date for each FIRM panel, how to order products, or the National Flood Insurance Program (NFIP) in general, please call the FEMA Map Information eXchange at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA Flood Map Service Center website at https://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website.

Communities annexing land on adjacent FIRM panels must obtain a current copy of the adjacent panel as well as the current FIRM Index. These may be ordered directly from the Flood Map Service Center at the number listed above.

For community and countywide map dates, refer to the Flood Insurance Study Report for this jurisdiction.

To determine if flood insurance is available in this community, contact your Insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

Basemap information shown on this FIRM was provided in digital format by the United States Geological Survey (USGS). The basemap shown is the USGS National Map: Orthoimagery. Last refreshed October, 2020.

This map was exported from FEMA's National Flood Hazard Layer (NFHL) on **12/14/2023 3:20 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. For additional information, please see the Flood Hazard Mapping Updates Overview Fact Sheet at https://www.fema.gov/media-library/assets/documents/118418

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date.

SCALE

Map Projection:
GCS, Geodetic Reference System 1980;
Vertical Datum: NAVD88

For information about the specific vertical datum for elevation features, datum conversions, or vertical monuments used to create this map, please see the Flood Insurance Study (FIS) Report for your community at https://msc.fema.gov

N

1 inch = 500 feet 1:6,000

0 250 500 1,000 1,500 2,000 Feet

0 50 100 200 300 400 Meters

NATIONAL FLOOD INSURANCE PROGRAM

FLOOD INSURANCE RATE MAP

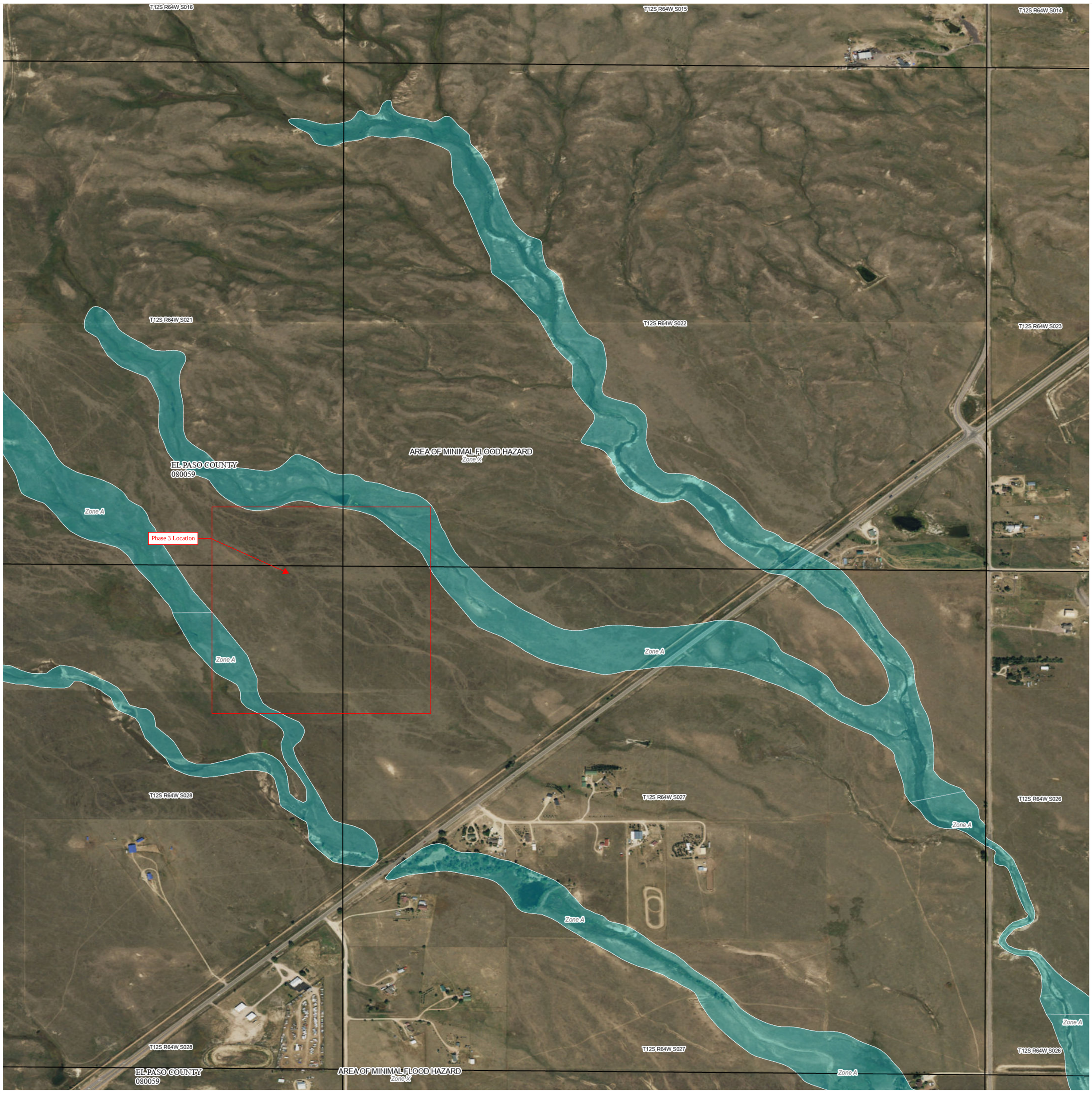
PANEL **552** OF **1275**

Panel Contains:

COMMUNITY	NUMBER	PANEL
EL PASO COUNTY	080059	0552

MAP NUMBER
08041C0552G
EFFECTIVE DATE
December 07, 2018

104°33'45.46"W 39°0'6.61"N



104°31'52.11"W 38°58'N

FLOOD HAZARD INFORMATION

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR DRAFT FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
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- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
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- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
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- Levee, Dike, or Floodwall

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- 8 Coastal Transect
- Coastal Transect Baseline
- Profile Baseline
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- 513 Base Flood Elevation Line (BFE)
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- Jurisdiction Boundary

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This map was exported from FEMA's National Flood Hazard Layer (NFHL) on **12/14/2023 3:22 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. For additional information, please see the Flood Hazard Mapping Updates Overview Fact Sheet at https://www.fema.gov/media-library/assets/documents/118418

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date.

SCALE

Map Projection:
GCS, Geodetic Reference System 1980;
Vertical Datum: No elevation features on this FIRM
For information about the specific vertical datum for elevation features, datum conversions, or vertical monuments used to create this map, please see the Flood Insurance Study (FIS) Report for your community at https://msc.fema.gov

N

1 inch = 500 feet 1:6,000

0 250 500 1,000 1,500 2,000 Feet

0 50 100 200 300 400 Meters

NATIONAL FLOOD INSURANCE PROGRAM

FLOOD INSURANCE RATE MAP

PANEL **556** OF **1275**

Panel Contains:

COMMUNITY	NUMBER	PANEL
EL PASO COUNTY	080059	0556

MAP NUMBER
08041C0556G
EFFECTIVE DATE
December 07, 2018

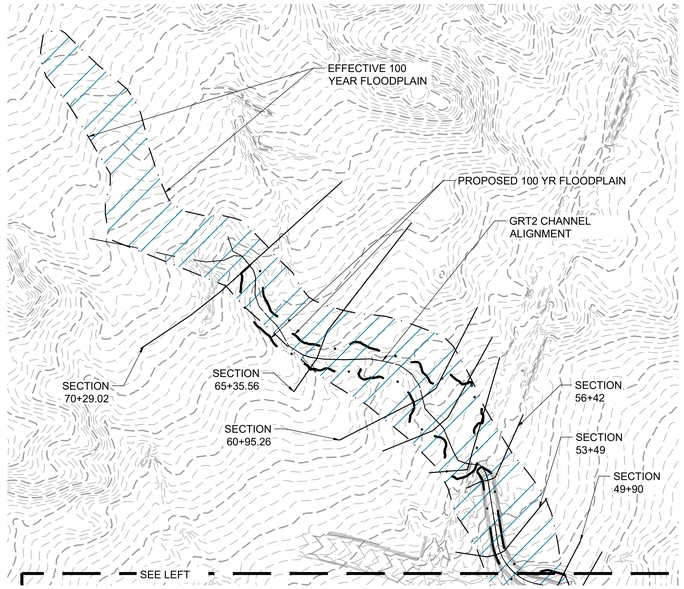
HOOD, ANDREW, 3/21/2023 4:25 PM

HRGreen DRAWING PATH:J:\2020\201662.03\CAD\Dwgs_Exhibits\MT-2_Form_2_Part_C_TOPO.dwg

SEE RIGHT

EASTONVILLE RD

SECTION 49+90
PR 10'x4' CULVERT
SECTION 45+80
SECTION 45+20
SECTION 43+65
SECTION 43+20
SECTION 42+10
SECTION 40+70
SECTION 40+25
SECTION 39+25
SECTION 38+80
SECTION 38+30
SECTION 36+50
SECTION 34+05
SECTION 33+60
SECTION 30+40
SECTION 27+15
SECTION 26+75
SECTION 24+60
SECTION 24+20
SECTION 22+60
SECTION 47+30
SECTION 47+00
SECTION 46+62.7
SECTION 46+36.94
SECTION 44+85
SECTION 44+40
PR(3) 8'x4' CULVERTS
SECTION 37+60
SECTION 36+85
SECTION 25+70
SECTION 25+45
SECTION 20+45
SECTION -20+00
SECTION 37+12.84
SECTION 36+18.51
SECTION 34+24.50
SECTION 32+14.73
SECTION 28+82.47
SECTION 27+72.34
SECTION 27+08.73
SECTION 25+92.31
SECTION 25+27.18
SECTION 23+03.17
SECTION 21+21.94
SECTION 18+14.04
SECTION 17+40
SECTION 14+90
SECTION 13+64
SECTION 15+56.67
SECTION 12+97.03
SECTION 10+84.03
SECTION 9+66.01
SECTION 15+35
SECTION 14+40
SECTION 13+85
SECTION 12+30
SECTION 10+50
SECTION 7+45
SECTION 7+00
SECTION 7+54.34
SECTION 6+42.96
SECTION 5+23.77
SECTION 2+90.78
SECTION 2+14.43
SECTION 1+48.72
SECTION 0+33.13
SECTION 5+90
SECTION 4+45
SECTION 4+00
SECTION 2+00
SECTION 0+70.18
SECTION -0+90.21
SECTION -2+96.57
SECTION -5+30.97
SECTION -7+34.97
PROPOSED 100 YR FLOODPLAIN
GEICK RANCH TRIBUTARY
PROPOSED 100 YR FLOODPLAIN
PROPOSED GRT2 CHANNEL ALIGNMENT
EAST FORK TRIBUTARY
EFFECTIVE 100 YEAR FLOODPLAIN
EXISTING GEICK RANCH TRIBUTARY 2 ALIGNMENT
EFFECTIVE 100 YEAR FLOODPLAIN



LEGEND:

- EFFECTIVE 100 YEAR FLOODPLAIN
- PROPOSED 100 YEAR FLOODPLAIN
- MAJOR TOPO
- MINOR TOPO
- GRANDVIEW RESERVE DEVELOPMENT BOUNDARY

COLORADO LICENSED
GREGORY L. PAZ
37081
3-23-23
PROFESSIONAL ENGINEER

NOTES:

1. BASIS OF BEARINGS: THE EAST LINE OF SECTION 21, BEING MONUMENTED AT THE SOUTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, AND BEING MONUMENTED AT THE NORTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, BEING ASSUMED TO BEAR NORTH 00 DEGREES 52 MINUTES 26 SECONDS WEST, A DISTANCE OF 5290.17 FEET.

NAVD88

NORTH
0 150 300
HRGreen

Job No.:	201662
Prepared By:	SJF
Date:	3/21/2023

FLOODPLAIN EXHIBIT

FIG.2

APPENDIX B – HYDROLOGIC CALCULATIONS

NOAA Atlas 14, Volume 8, Version 2
Location name: Peyton, Colorado, USA*
Latitude: 38.9877°, Longitude: -104.5596°
Elevation: 6971 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	**0.239** (0.189-0.304)	**0.291** (0.230-0.371)	**0.382** (0.300-0.487)	**0.461** (0.360-0.591)	**0.576** (0.438-0.771)	**0.670** (0.497-0.906)	**0.769** (0.552-1.06)	**0.874** (0.602-1.24)	**1.02** (0.675-1.48)	**1.14** (0.731-1.67)
10-min	**0.350** (0.276-0.446)	**0.427** (0.337-0.544)	**0.559** (0.439-0.714)	**0.675** (0.528-0.866)	**0.844** (0.642-1.13)	**0.982** (0.728-1.33)	**1.13** (0.808-1.56)	**1.28** (0.881-1.82)	**1.49** (0.989-2.17)	**1.66** (1.07-2.44)
15-min	**0.427** (0.337-0.543)	**0.520** (0.410-0.663)	**0.681** (0.536-0.870)	**0.823** (0.643-1.06)	**1.03** (0.783-1.38)	**1.20** (0.888-1.62)	**1.37** (0.985-1.90)	**1.56** (1.07-2.22)	**1.82** (1.21-2.65)	**2.03** (1.30-2.98)
30-min	**0.607** (0.480-0.773)	**0.740** (0.583-0.942)	**0.967** (0.761-1.24)	**1.17** (0.912-1.50)	**1.46** (1.11-1.95)	**1.69** (1.26-2.29)	**1.94** (1.39-2.68)	**2.20** (1.51-3.12)	**2.56** (1.70-3.73)	**2.85** (1.83-4.19)
60-min	**0.774** (0.611-0.985)	**0.932** (0.735-1.19)	**1.21** (0.952-1.55)	**1.46** (1.14-1.88)	**1.84** (1.40-2.47)	**2.15** (1.60-2.92)	**2.49** (1.79-3.45)	**2.85** (1.96-4.05)	**3.35** (2.22-4.90)	**3.76** (2.42-5.54)
2-hr	**0.941** (0.749-1.19)	**1.12** (0.894-1.42)	**1.46** (1.15-1.84)	**1.76** (1.39-2.24)	**2.22** (1.71-2.97)	**2.61** (1.96-3.52)	**3.03** (2.20-4.19)	**3.49** (2.43-4.94)	**4.14** (2.78-6.02)	**4.68** (3.04-6.84)
3-hr	**1.03** (0.824-1.29)	**1.22** (0.973-1.53)	**1.57** (1.25-1.98)	**1.90** (1.50-2.40)	**2.41** (1.88-3.23)	**2.86** (2.16-3.85)	**3.34** (2.44-4.60)	**3.87** (2.72-5.47)	**4.64** (3.13-6.72)	**5.27** (3.44-7.67)
6-hr	**1.19** (0.961-1.48)	**1.40** (1.12-1.74)	**1.78** (1.43-2.23)	**2.16** (1.72-2.71)	**2.76** (2.17-3.67)	**3.28** (2.50-4.40)	**3.86** (2.85-5.29)	**4.50** (3.19-6.33)	**5.44** (3.70-7.84)	**6.21** (4.10-8.98)
12-hr	**1.38** (1.12-1.70)	**1.61** (1.30-1.98)	**2.05** (1.66-2.53)	**2.47** (1.99-3.07)	**3.14** (2.49-4.15)	**3.73** (2.87-4.96)	**4.38** (3.26-5.96)	**5.10** (3.64-7.12)	**6.14** (4.23-8.80)	**7.01** (4.67-10.1)
24-hr	**1.59** (1.30-1.95)	**1.86** (1.52-2.28)	**2.37** (1.93-2.90)	**2.84** (2.30-3.50)	**3.58** (2.86-4.66)	**4.22** (3.27-5.55)	**4.92** (3.69-6.62)	**5.68** (4.09-7.86)	**6.79** (4.71-9.65)	**7.70** (5.17-11.0)
2-day	**1.85** (1.53-2.24)	**2.17** (1.79-2.63)	**2.75** (2.26-3.34)	**3.28** (2.68-4.00)	**4.09** (3.28-5.26)	**4.78** (3.73-6.21)	**5.52** (4.17-7.36)	**6.33** (4.59-8.67)	**7.48** (5.23-10.5)	**8.42** (5.71-12.0)
3-day	**2.02** (1.68-2.44)	**2.38** (1.97-2.86)	**3.01** (2.48-3.64)	**3.58** (2.94-4.35)	**4.45** (3.58-5.68)	**5.18** (4.06-6.69)	**5.97** (4.52-7.90)	**6.81** (4.97-9.28)	**8.02** (5.63-11.2)	**8.99** (6.13-12.7)
4-day	**2.17** (1.81-2.61)	**2.55** (2.12-3.06)	**3.21** (2.66-3.86)	**3.81** (3.14-4.61)	**4.72** (3.80-6.00)	**5.48** (4.31-7.04)	**6.29** (4.79-8.30)	**7.17** (5.24-9.73)	**8.42** (5.93-11.8)	**9.42** (6.45-13.3)
7-day	**2.57** (2.16-3.06)	**2.97** (2.48-3.54)	**3.67** (3.06-4.39)	**4.31** (3.58-5.17)	**5.27** (4.28-6.64)	**6.08** (4.81-7.76)	**6.94** (5.32-9.09)	**7.87** (5.80-10.6)	**9.20** (6.53-12.8)	**10.3** (7.08-14.4)
10-day	**2.92** (2.46-3.46)	**3.35** (2.82-3.98)	**4.11** (3.44-4.89)	**4.79** (3.99-5.73)	**5.81** (4.73-7.28)	**6.66** (5.29-8.45)	**7.56** (5.82-9.85)	**8.53** (6.32-11.4)	**9.90** (7.06-13.7)	**11.0** (7.63-15.4)
20-day	**3.90** (3.31-4.57)	**4.50** (3.81-5.28)	**5.51** (4.65-6.49)	**6.37** (5.36-7.55)	**7.61** (6.22-9.37)	**8.60** (6.87-10.8)	**9.62** (7.44-12.4)	**10.7** (7.95-14.1)	**12.1** (8.71-16.6)	**13.3** (9.28-18.4)
30-day	**4.68** (3.99-5.46)	**5.42** (4.61-6.33)	**6.63** (5.63-7.76)	**7.64** (6.45-8.99)	**9.03** (7.39-11.0)	**10.1** (8.11-12.5)	**11.2** (8.70-14.3)	**12.3** (9.20-16.2)	**13.8** (9.95-18.7)	**14.9** (10.5-20.6)
45-day	**5.64** (4.84-6.55)	**6.52** (5.58-7.58)	**7.94** (6.77-9.25)	**9.09** (7.71-10.6)	**10.6** (8.73-12.8)	**11.8** (9.49-14.5)	**13.0** (10.1-16.4)	**14.1** (10.6-18.4)	**15.6** (11.3-21.0)	**16.7** (11.8-22.9)
60-day	**6.45** (5.55-7.46)	**7.42** (6.37-8.59)	**8.96** (7.68-10.4)	**10.2** (8.69-11.9)	**11.8** (9.74-14.2)	**13.1** (10.5-16.0)	**14.2** (11.1-17.9)	**15.4** (11.6-20.0)	**16.8** (12.2-22.6)	**17.9** (12.7-24.6)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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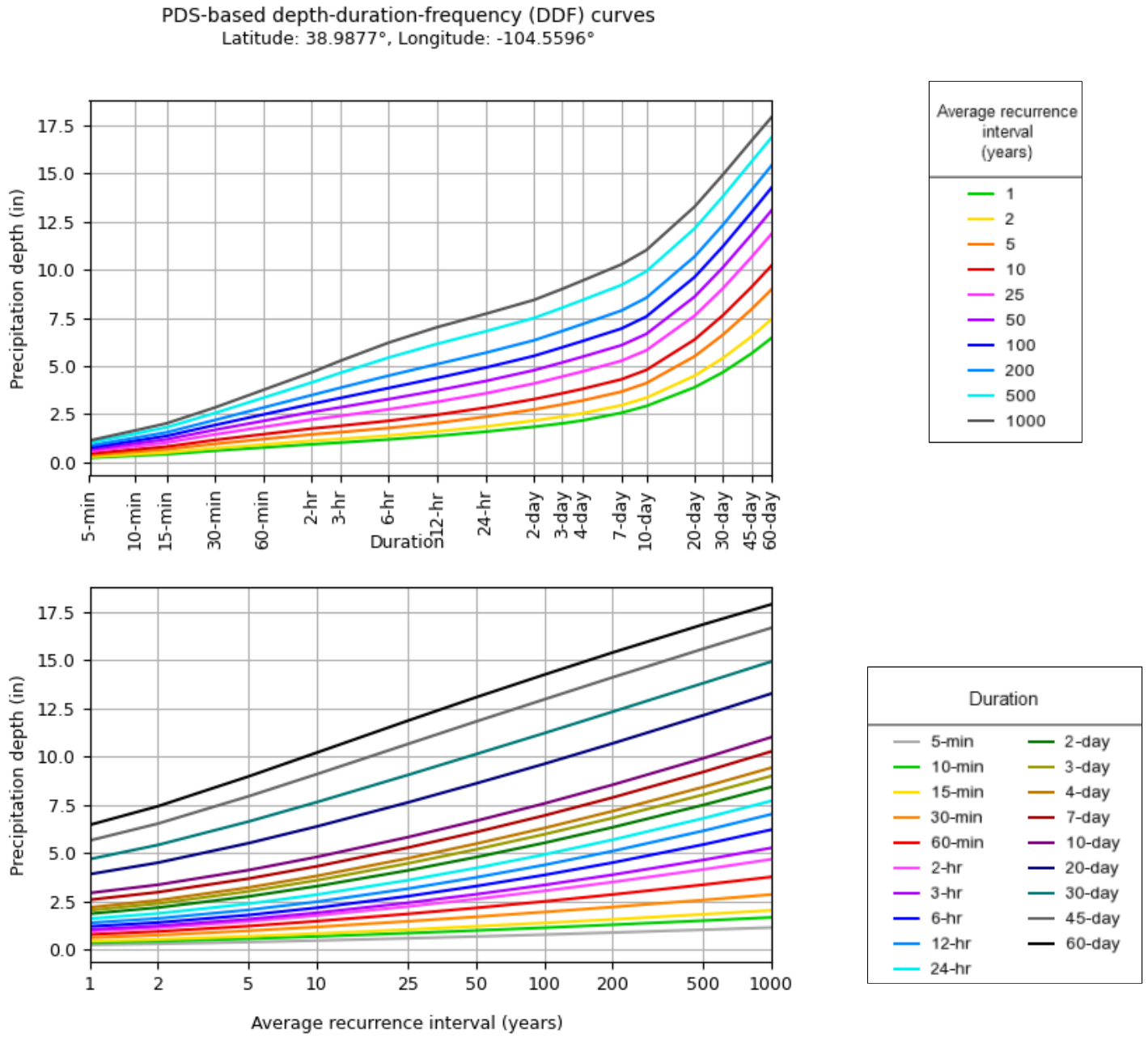
PF graphical



NOAA Atlas 14, Volume 8, Version 2 Created (GMT): Tue Sep 5 16:10:04 2023

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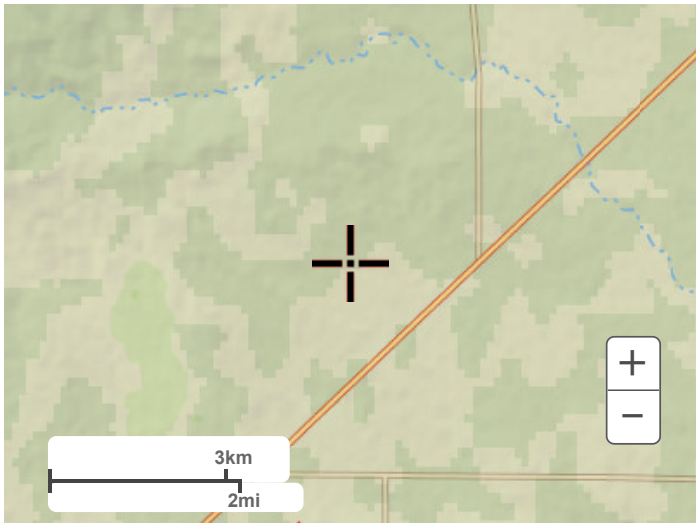
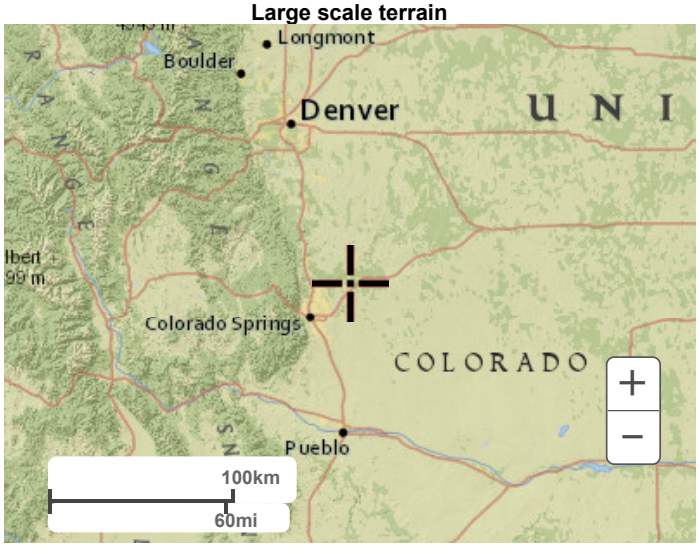
Maps & aerials

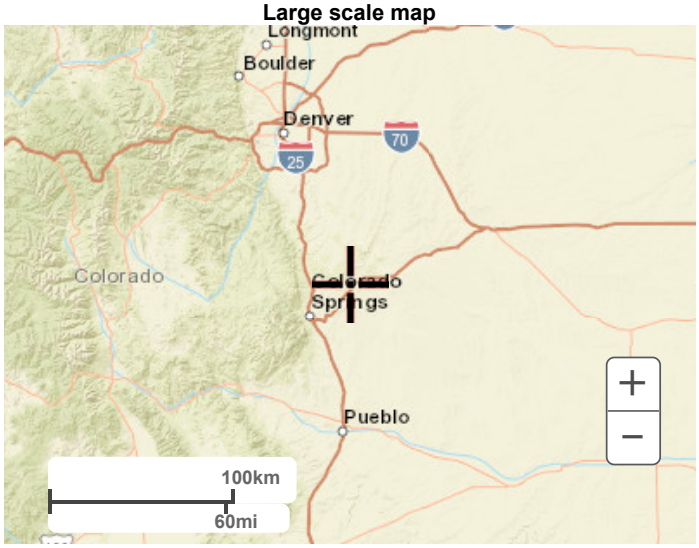
Small scale terrain



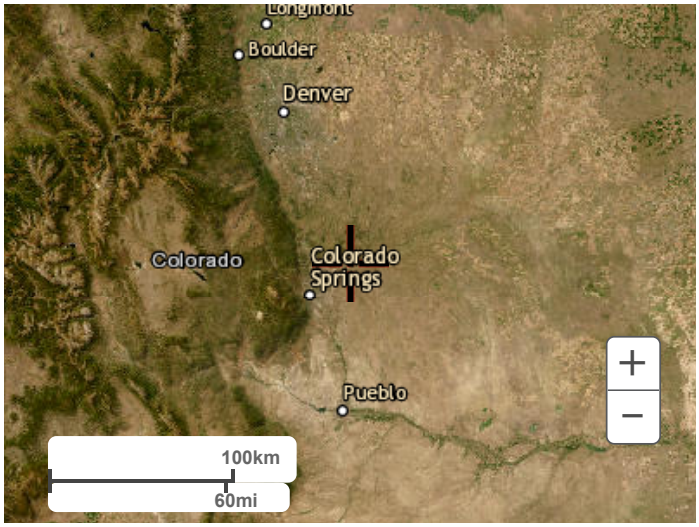
Large scale terrain



Large scale map



Large scale aerial



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National Oceanic and Atmospheric Administration
National Weather Service
National Water Center
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

Disclaimer

HRGreen	**GRANDVIEW RESERVE (PHASE 3)**	**Calc'd by:**	**CBM**
	PROPOSED CONDITIONS	**Checked by:**	
	EL PASO COUNTY, CO	**Date:**	**9/18/2024**

SUMMARY RUNOFF TABLE

BASIN	AREA (ac)	% IMPERVIOUS	Q_5 (cfs)	Q_{100} (cfs)
R-A1	1.20	52	2.6	6.0
R-A2	2.54	47	3.9	9.1
R-B	4.19	43	6.1	14.7
R-C	2.90	52	5.4	12.0
R-D	5.53	39	7.2	17.7
R-E	1.81	50	3.3	7.4
R-F	0.81	51	1.8	4.0
R-G	1.80	0	0.6	4.3
R-H	0.46	51	1.0	2.2
F-A	4.58	49	7.0	15.8
F-B	3.76	59	7.0	15.2
F-C1	2.08	57	4.0	8.9
F-C2	3.64	62	6.9	14.6
F-C3	1.33	65	2.9	6.0
F-D	3.13	39	4.2	10.4
F-E	4.42	51	7.7	17.4
F-F	0.94	54	1.9	4.1
F-G	0.63	38	0.9	2.2
F-H	1.64	0	0.5	3.8
F-I	1.52	52	3.6	8.3
C-A	3.43	29	3.7	10.5
C-B	4.54	37	6.1	15.6
C-C	2.30	45	3.6	8.4
C-D	1.88	59	3.8	8.1
C-E	2.30	60	4.7	10.2
C-F	2.60	58	5.2	11.3
C-G	1.64	51	3.1	7.1
C-H	0.49	62	1.2	2.6
C-I	0.90	0	0.3	1.9
C-J	0.81	33	1.0	2.9
C-K	3.01	0	0.9	6.6
C-L	0.50	52	1.2	8.8
OS-1	10.29	0.0	3.0	22.0

DESIGN POINT SUMMARY TABLE

DESIGN POINT	CONTRIBUTING BASINS	ΣQ_5 (cfs)	ΣQ_{100} (cfs)
R-1	RA-1	2.6	6.0
R-2	RA-2	5.9	13.5
R-3	R-B	6.1	14.7
R-3.1	DP R-1,2,3	9.9	24.0
R-4	R-C	5.4	12.0
R-5	R-D	10.6	24.5
R-5.1	DP R-4,5	9.4	21.2
R-6	R-E, DP R-3,5	5.0	16.0
R-6.1	DP R-5.1,6	23.1	58.4
R-7	R-F	24.3	61.1
R-8	DP R-6.1,7	23.5	61.6
R-9	R-H	1.0	2.2
F-1	F-A	7.0	15.8
F-2	F-B	7.0	15.2
F-2.1	DP F-1,2,3	13.4	29.7
F-3	F-C1	4.0	8.9
F-4	F-C3	2.9	6.0
F-5	F-C2	6.9	16.3
F-6	F-D	4.2	10.4
F-6.1	DP F-4,5,6	13.2	22.6
F-6.2	DP F-2.1,6.1	25.5	50.2
F-7	F-E, DP F-5,6	7.2	21.7
F-7.1	DP F-6.2, 7	32.0	70.5
F-8	F-F, DP F-7.1	33.5	73.7
F-9	F-G	0.9	2.2
F-10	F-H, DP F-8,9	34.4	78.0
F-11	F-I	3.6	8.3
C-1	C-A	3.7	10.5
C-2	C-B	6.1	15.6
C-2.1	DP C-1,2	9.5	14.6
C-3	C-C, DP C-1	3.7	12.2
C-4	C-D, DP C-2	3.6	15.3
C-4.1	DP C-2.1,3,4	7.1	28.3
C-5	C-E	4.7	10.2
C-6	C-F	5.2	11.3
C-6.1	DP C-5,6	6.9	10.2
C-6.2	CP C-4.1,6.1	13.1	37.1
C-7	C-G, DP C-3,4,5	5.6	24.5
C-7.1	DP C-6.2,7	17.7	26.8
C-8	C-H, DP C-6	3.4	9.4
C-8.1	DP C-7.1,8	20.0	35.4
C-9	C-I	0.3	1.9
C-10	C-J, DP C-8.1,9	20.5	37.5
C-11	Offsite C-14	50.5	107.4
C-12	DP C-10,11	57.7	123.8
C-13	C-L	1.2	8.8
O-1	OS-1	3.0	22.0

GRANDVIEW RESERVE (PHASE 3)
PROPOSED CONDITIONS
EL PASO COUNTY, CO

Calc'd by: CBM

Checked by:

Date: 9/18/2024

SOIL TYPE: HSG A&B

COMPOSITE 'C' FACTORS

BASIN	LAND USE TYPE																		TOTAL	COMPOSITE IMPERVIOUSNESS & C FACTOR		
	EPC LOCAL			EPC MINOR ARTERIAL			Lawns			35' LOTS			40' LOTS			50' LOTS						
	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}				
	90	0.82	0.90	68	0.63	0.76	0	0.08	0.35	38	0.37	0.56	52	0.46	0.63	40	0.38	0.56				
	ACRES			ACRES			ACRES			ACRES			ACRES			ACRES			ACRES	%I	C_5	C_{100}
G-B				1.77			0.38												2.15	56	0.53	0.69
H-B				1.79			1.56						1.47						4.77	42	0.40	0.59
**R-A1													1.20						1.20	52	0.46	0.63
R-A2	0.53						0.20									1.81			2.54	47	0.45	0.61
R-B	0.55						0.17			3.47									4.19	43	0.42	0.60
R-C	0.79									2.11									2.90	52	0.49	0.65
R-D				3.16			2.29			0.08									5.53	39	0.40	0.59
R-E	0.42									1.39									1.81	50	0.47	0.64
R-F	0.46						0.35												0.81	51	0.50	0.66
R-G							1.80												1.80	0	0.08	0.35
R-H	0.26						0.20												0.46	51	0.50	0.66
F-A	1.11						0.13			3.35									4.58	49	0.47	0.64
F-B	0.70												3.06						3.76	59	0.53	0.68
F-C1	0.44						0.14						1.50						2.08	57	0.51	0.67
F-C2	0.95												2.69						3.64	62	0.55	0.70
F-C3	0.50						0.04						0.79						1.33	65	0.58	0.72
F-D	0.82						1.11									1.20			3.13	39	0.39	0.57
F-E	0.98						0.13			1.60			0.80			0.91			4.42	51	0.48	0.64
F-F	0.31						0.06									0.58			0.94	54	0.51	0.66
F-G							0.04									0.59			0.63	38	0.36	0.55
F-H							1.64												1.64	0	0.08	0.35
**F-I													1.52						1.52	52	0.46	0.63
C-A	0.40						1.46									1.58			3.43	29	0.30	0.51
C-B	0.81						1.62						0.91			1.20			4.54	37	0.37	0.56
C-C	0.38						0.19									1.72			2.30	45	0.43	0.60
C-D	0.35												1.51						1.88	59	0.52	0.68

COMPOSITE 'C' FACTORS

	LAND USE TYPE																		TOTAL	COMPOSITE IMPERVIOUSNESS & C FACTOR		
	EPC LOCAL			EPC MINOR ARTERIAL			Lawns			35' LOTS			40' LOTS			50' LOTS						
	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}				
	90	0.82	0.90	68	0.63	0.76	0	0.08	0.35	38	0.37	0.56	52	0.46	0.63	40	0.38	0.56				
BASIN	ACRES			ACRES			ACRES			ACRES			ACRES			ACRES			ACRES	%I	C_5	C_{100}
C-E	0.46												1.84						2.30	60	0.53	0.68
C-F	0.56						0.09						1.95						2.60	58	0.52	0.68
C-G	0.37						0.16						0.43			0.69			1.64	51	0.47	0.64
C-H	0.17						0.04						0.28						0.49	62	0.56	0.70
C-I							0.90												0.90	0	0.08	0.35
C-J							0.30						0.51						0.81	33	0.32	0.53
C-K							3.01												3.01	0	0.08	0.35
*C-F4																			43.50	0	-	-
**C-L													0.50						0.50	52	0.46	0.63
OS-1							10.29												10.29	0	0.08	0.35
Pond R	3.01			3.16			5.00			7.05			1.20			1.81			**20.78**	**42**		
Pond F	5.80			0.00			3.28			4.95			8.84			3.28			**26.15**	**50**		
Pond C																			**67.40**	**13**		

(*): REFER TO "PRELIMINARY DRAINAGE REPORT FOR GRANDVIEW RESERVE FILING NO. 1" BY GALLOWAY & COMPANY INC., APPROVED 03/06/2023

(**): 40' LOT CHARACTERISTICS UTILIZED TO CONSERVATIVELY ACCOUNT FOR PROPOSED GRAVEL TRAIL SUB AREA

HRGreen	**GRANDVIEW RESERVE (PHASE 3)**	**Calc'd by:**	**CBM**
	PROPOSED CONDITIONS	**Checked by:**	
	EL PASO COUNTY, CO	**Date:**	**9/18/2024**

TIME OF CONCENTRATION

BASIN DATA			OVERLAND TIME (T_i)			TRAVEL TIME (T_t)					TOTAL	*tc=(L/180)+10*	Design tc
DESIGNATION	C_5	AREA (ac)	LENGTH (ft)	SLOPE %	t_i (min)	C_V	LENGTH (ft)	SLOPE %	ΣQ_5 (cfs)	t_t (min)	t_c (min)	*tc max*	*tc design (min)*
G-B	0.53	2.15	30	2.0	4.5	20	1600	1.0	2.0	13.3	17.9	19.1	17.9
H-B	0.40	4.77	100	2.0	10.1	20	1450	1.0	2.0	12.1	22.2	18.6	18.6
R-A1	0.46	1.20	100	6.0	6.5	15	50	8.0	4.2	0.2	6.7	10.8	6.7
R-A2	0.45	2.54	100	2.0	9.5	20	900	0.6	1.5	9.7	19.2	15.6	15.6
R-B	0.42	4.19	100	2.0	9.9	20	850	0.6	1.5	9.1	19.1	15.3	15.3
R-C	0.49	2.90	100	2.0	8.8	20	450	1.0	2.0	3.8	12.6	13.1	12.6
R-D	0.40	5.53	100	2.0	10.2	20	1360	2.0	2.9	7.8	18.0	18.1	18.0
R-E	0.47	1.81	100	2.0	9.1	20	360	0.8	1.8	3.4	12.5	12.6	12.5
R-F	0.50	0.81	30	2.0	4.8	20	360	0.8	1.8	3.4	8.1	12.2	8.1
R-G	0.08	1.80	100	2.0	14.8	15	0	1.0	1.5	0.0	14.8	10.6	10.6
R-H	0.50	0.46	100	2.0	8.8	15	50	1.0	1.5	0.6	9.3	10.8	9.3
F-A	0.47	4.58	100	2.0	9.2	20	1375	1.6	2.5	9.1	18.2	18.2	18.2
F-B	0.53	3.76	100	2.0	8.3	20	750	0.6	1.5	8.1	16.4	14.7	14.7
F-C1	0.51	2.08	100	2.0	8.6	20	350	0.5	1.4	4.1	12.7	12.5	12.5
F-C2	0.55	3.64	100	2.0	7.9	20	1000	1.0	2.0	8.3	16.3	16.1	16.1
F-C3	0.58	1.33	100	2.0	7.5	20	650	1.0	2.0	5.4	12.9	14.2	12.9
F-D	0.39	3.13	100	2.0	10.3	20	950	1.4	2.4	6.7	17.0	15.8	15.8
F-E	0.48	4.42	100	2.0	9.0	20	575	0.7	1.7	5.7	14.8	13.8	13.8
F-F	0.51	0.94	100	2.0	8.6	20	220	0.6	1.5	2.4	11.0	11.8	11.0
F-G	0.36	0.63	100	2.0	10.7	15	300	0.7	1.3	4.0	14.7	12.2	12.2
F-H	0.08	1.64	100	2.0	14.8	15	150	3.0	2.6	1.0	15.8	11.4	11.4
F-I	0.46	1.52	100	20.0	4.3	15	20	5.0	3.4	0.1	5.0	10.7	5.0
C-A	0.30	3.43	100	3.5	9.6	20	800	1.8	2.7	5.0	14.6	15.0	14.6
C-B	0.37	4.54	100	4.0	8.5	20	625	1.0	2.0	5.2	13.7	14.0	13.7
C-C	0.43	2.30	100	2.0	9.8	20	650	1.6	2.5	4.3	14.1	14.2	14.1
C-D	0.52	1.88	100	2.0	8.4	20	600	1.6	2.5	4.0	12.3	13.9	12.3
C-E	0.53	2.30	100	2.0	8.3	20	650	2.1	2.9	3.7	12.0	14.2	12.0
C-F	0.52	2.60	100	2.0	8.4	20	700	2.1	2.9	4.0	12.4	14.4	12.4
C-G	0.47	1.64	100	2.0	9.1	20	175	1.0	2.0	1.5	10.6	11.5	10.6
C-H	0.56	0.49	100	2.0	7.9	20	50	1.0	2.0	0.4	8.3	10.8	8.3
C-I	0.08	0.90	100	4.0	11.8	15	575	1.0	1.5	6.4	18.2	13.8	13.8
C-J	0.32	0.81	100	2.0	11.3	15	50	2.0	2.1	0.4	11.7	10.8	10.8
C-K	0.08	3.01	100	10.0	8.7	20	500	1.0	2.0	4.2	12.8	13.3	12.8
C-L	0.46	0.50	100	10.0	5.4	20	50	5.0	4.5	0.2	5.6	10.8	5.6
OS-1	0.08	10.29	100	4.0	11.8	7	600	4.0	1.4	7.1	18.9	13.9	13.9

HRGreen	**GRANDVIEW RESERVE (PHASE 3)**	**Calc'd by:**	**CBM**
	PROPOSED CONDITIONS	**Checked by:**	
	EL PASO COUNTY, CO	**Date:**	**9/18/2024**

TIME OF CONCENTRATION

BASIN DATA			OVERLAND TIME (T_i)			TRAVEL TIME (T_t)					TOTAL	*tc=(L/180)+10*	**Design tc**
DESIGNATION	C_5	AREA (ac)	LENGTH (ft)	SLOPE %	t_i (min)	C_V	LENGTH (ft)	SLOPE %	ΣQ_5 (cfs)	t_t (min)	t_c (min)	*tc max*	*tc design (min)*

FORMULAS:

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \qquad V = C_v S_w^{0.5}$$

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

* For buried riprap, select C_v value based on type of vegetative cover.

HRGreen

GRANDVIEW RESERVE (PHASE 3)

PROPOSED CONDITIONS

DESIGN STORM: 5-YEAR

Calc'd by: CBM

Checked by:

Date: 9/18/2024

			DIRECT RUNOFF						ΣQ5 (cfs)				STREET			PIPE				TRAVEL TIME			REMARKS
STREET	DESIGN POINT	BASIN ID	AREA (ac)	C_5	t_c (min)	C_5*A (ac)	I (in./ hr.)	Q (cfs)	t_c (min)	C_5*A (ac)	I (in./ hr.)	Q (cfs)	Q_{street} (cfs)	C_5*A (ac)	SLOPE %	Q_{PIPE} (cfs)	C_5*A (ac)	SLOPE %	PIPE SIZE (ft)	LENGTH (FT)	VEL. (FPS)	TRAVEL TIME (min)	
		G-B	2.15	0.53	17.9	1.15	3.26	3.7	17.9	1.15	3.26	3.7	0.0	0.00	0.8					1000	1.8	9.32	
																3.7	1.15	2.0	1.5				*OFFSITE STORMWATER CURB AND GUTTER FLOW TO R-5*
		H-B	4.77	0.40	18.6	1.93	3.20	6.2	18.6	1.93	3.20	6.2	0.0	0.00	0.8					1000	1.8	9.32	
																6.2	1.93	2.0	1.5				*OFFSITE STORMWATER CURB AND GUTTER FLOW TO R-5*
	R-1	R-A1	1.20	0.46	6.7	0.55	4.74	2.6	6.7	0.55	4.74	2.6				2.6	0.55	1.0	1.0	95	4.5	0.35	BASIN R-A1 STORMWATER SWALE FLOW TO R-1 AND CAPTURED IN TYPE-C INLET
													0.0	0.00	0.6								***R-2 FLOWBY TO BE CAPTURED BY FUTURE PHASE STORM NETWORK***
	R-2	R-A2	2.54	0.45	15.6	1.14	3.47	3.9	15.6	1.69	3.47	5.9				3.9	1.14	0.5	1.5	65	4.2	0.26	BASIN R-A2 STORMWATER CAPTURED IN TYPE R INLET, COMBINED WITH R-1 AND PIPED TO R-3.1
													3.9	0.00	0.6					400	1.5	4.30	R-3 FLOWBY FLOW TO R-6, CAPTURED BY SUMP INLET
	R-3	R-B	4.19	0.42	15.3	1.75	3.49	6.1	15.3	1.75	3.49	6.1				6.1	1.75	0.6	1.5	8	4.6	0.03	R-B STORMWATER CAPTURED IN TYPE R INLET AND PIPED TO R-3.1
	R-3.1								15.8	2.88	3.44	9.9				9.9	2.88	0.5	2.0	375	5.1	1.23	DP R-3.1 PIPE FLOW TO R-6.1
	R-4	R-C	2.90	0.49	12.6	1.43	3.78	5.4	12.6	1.43	3.78	5.4				5.4	1.43	2.0	1.5	215	8.4	0.43	R-C STORMWATER CAPTURED IN SUMP INLET AND PIPED TO R-5.1
													3.5	0.72	0.9					215	1.9	1.89	
	R-5	R-D	5.53	0.40	18.0	2.21	3.25	7.2	18.0	2.21	3.25	10.6				7.1	1.48	0.5	1.5	50	4.2	0.20	G-B,H-B FLOWBY, BASIN R-D STORMWATER CAPTURED IN TYPE R INLET, FLOWBY TO R-6
	R-5.1								18.2	2.91	3.23	9.4				9.4	2.91	0.5	2.0	375	5.1	1.23	R-5.1 STORMWATER PIPE FLOW TO R-6.1
	R-6	R-E	1.81	0.47	12.5	0.86	3.80	3.3	19.4	1.59	3.13	5.0				5.0	1.59	1.0	2.0	6	7.2	0.01	BASIN R-E STORMWATER, R-3 AND R-5 FLOWBY, CAPTURED IN SUMP INLET AT R-6
	R-6.1								19.5	7.38	3.13	23.1				23.1	7.38	1.0	2.5	30	8.4	0.06	R-6.1 STORMWATER PIPE FLOW TO R-7
	R-7	R-F	0.81	0.50	8.1	0.40	4.44	1.8	19.5	7.79	3.13	24.3				24.3	0.40	1.0	2.5	65	8.4	0.13	BASIN R-F CAPTURED IN SUMP INLET AND PIPED TO R-8
	R-8	R-G	1.80	0.08	10.6	0.14	4.05	0.6	19.5	7.52	3.13	23.5											**TOTAL FLOW ENTERING POND R**
	R-9	R-H	0.46	0.50	9.3	0.23	4.24	1.0	9.3	0.23	4.24	1.0											**R-H STORMWATER FLOWS OFFSITE AND WILL BE DETAINED BY FUTURE DEVELOPMENT**
	F-1	F-A	4.58	0.47	18.2	2.16	3.23	7.0	18.2	2.16	3.23	7.0				7.0	2.16	1.0	2.0	5	7.2	0.01	BASIN F-A STORMWATER CAPTURED IN A SUMP INLET, PIPED TO F-2.1
	F-2	F-B	3.76	0.53	14.7	1.98	3.55	7.0	14.7	1.98	3.55	7.0				7.0	1.98	1.0	2.0	28	7.2	0.06	BASIN F-B STORMWATER CAPTURED IN A SUMP INLET, PIPED TO DP F-2.1
	F-2.1								18.2	4.14	3.23	13.4				13.4	4.14	1.0	2.5	375	8.4	0.75	F-1, F-2, F-3 COMBINE AT F-2.1, PIPED TO F-6.2
													0.0	0.00	0.5					650	1.4	7.66	F-3 FLOWBY TO F-5
	F-3	F-C1	2.08	0.51	12.5	1.06	3.79	4.0	12.5	1.06	3.79	4.0				4.0	1.06	0.5	1.5	300	4.2	1.19	BASIN F-C1 STORMWATER CAPTURED IN TYPE R INLET, PIPED TO F-2.1
													0.0	0.00	1.4					250	2.4	1.76	F-4 FLOWBY TO F-5
	F-4	F-C3	1.33	0.58	12.9	0.78	3.74	2.9	12.9	0.78	3.74	2.9				2.9	0.78	1.0	1.5	300	5.9	0.84	BASIN F-C3 STORMWATER CAPTURED IN TYPE R AND PIPED TO F-6.1
													0.2	0.05	1.4					325	2.4	2.29	F-5 FLOWBY TO F-7
	F-5	F-C2	3.64	0.55	16.1	2.02	3.41	6.9	16.1	2.02	3.41	6.9				6.7	1.96	1.0	1.5	85	5.9	0.24	
													0.2	0.06	1.4					355	2.4	2.50	F-6 FLOWBY TO F-7
	F-6	F-D	3.13	0.39	15.8	1.22	3.44	4.2	15.8	1.22	3.44	4.2				4.0	1.16	1.0	1.5	55	5.9	0.15	BASIN F-D STORMWATER CAPTURED IN TYPE R INLET AND PIPED TO F-6.1
	F-6.1								16.3	3.90	3.39	13.2				13.2	3.90	1.0	2.5	60	8.4	0.12	F-5, F-6 COMBINE AT F-6.1 AND IS PIPED TO F-6.2
	F-6.2								19.0	8.04	3.17	25.5				25.5	8.04	1.0	2.5	325	8.4	0.65	F-6.1, F-2.1 COMBINE AT F-6.2 AND IS PIPED TO F-7.1
	F-7	F-E	4.42	0.48	13.8	2.12	3.65	7.7	18.4	2.23	3.21	7.2				7.2	2.23	1.0	2.0	30	7.2	0.07	BASIN F-E STORMWATER AND FLOWBY FROM F-5,6 CAPTURED IN SUMP AND PIPED TO F-7.1
	F-7.1								19.6	10.27	3.12	32.0				32.0	10.27	1.0	2.5	10	8.4	0.02	F-7.1 PIPED TO F-8
	F-8	F-F	0.94	0.51	11.0	0.48	3.98	1.9	19.6	10.74	3.12	33.5				33.5	10.74	2.0	3.0	180	13.3	0.22	BASIN F-F STORMWATER CAPTURED IN SUMP INLET AT F-8 AND PIPED TO F-9
													0.9	0.23	5.0					50	4.5	0.19	
	F-9	F-G	0.63	0.36	12.2	0.23	3.83	0.9	12.2	0.23	3.83	0.9											F-G STORMWATER COMBINED WITH F-8 PIPE FLOW AT PONDF
	F-10	F-H	1.64	0.08	11.4	0.13	3.93	0.5	19.8	11.10	3.10	34.4											**TOTAL FLOW ENTERING POND F**

HRGreen

GRANDVIEW RESERVE (PHASE 3)

PROPOSED CONDITIONS

DESIGN STORM: 5-YEAR

Calc'd by: CBM

Checked by:

Date: 9/18/2024

			DIRECT RUNOFF						ΣQ5 (cfs)				STREET			PIPE				TRAVEL TIME			REMARKS
STREET	DESIGN POINT	BASIN ID	AREA (ac)	C_5	t_c (min)	C_5*A (ac)	I (in./ hr.)	Q (cfs)	t_c (min)	C_5*A (ac)	I (in./ hr.)	Q (cfs)	Q_{street} (cfs)	C_5*A (ac)	SLOPE %	Q_{PIPE} (cfs)	C_5*A (ac)	SLOPE %	PIPE SIZE (ft)	LENGTH (FT)	VEL. (FPS)	TRAVEL TIME (min)	
	F-11	F-I	1.52	0.46	5.0	0.70	5.17	3.6	5.0	0.70	5.17	3.6											F-I STORMWATER FLOWS OFFSITE FOLLOWING EXISTING DRAINAGE PATTERNS
	C-1	C-A	3.43	0.30	14.6	1.04	3.56	3.7	14.6	1.04	3.56	3.7	0.1	0.03	1.0	3.6	1.01	1.0	1.5	650	2.0	5.42	C-1 FLOWBY TO C-3
																				6	5.9	0.02	BASIN C-A STORMWATER CAPTURED IN TYPE R INLET, PIPED TO C-2.1
	C-2	C-B	4.54	0.37	13.7	1.67	3.66	6.1	13.7	1.67	3.66	6.1	1.1	0.00	1.0	5.0	1.67	1.0	1.5	650	2.0	5.42	C-2 FLOWBY TO C-4
																				28	5.9	0.08	BASIN C-B STORMWATER CAPTURED IN TYPE R INLET, PIPED TO C-2.1
	C-2.1								14.6	2.68	3.56	9.5				9.5	2.68	1.0	1.5	650	5.9	1.82	C-1 AND C-2 COMBINE AT C-2.1, PIPED TO C-4.1
	C-3	C-C	2.30	0.43	14.1	0.99	3.62	3.6	14.1	1.02	3.62	3.7	0.1	0.02	1.0	3.6	0.96	1.0	1.5	170	2.0	1.42	C3- FLOWBY TO C-7
																				6	5.9	0.02	BASIN C-C, C-1 FLOWBY CAPTURED IN TYPE R INLET, PIPED TO C-4.1
	C-4	C-D	1.88	0.52	12.3	0.99	3.81	3.8	13.8	0.99	3.65	3.6	0.1	0.00	1.0	3.5	0.99	1.0	1.5	200	2.0	1.67	C-4 FLOWBY TO C-7
																				28	5.9	0.08	BASIN C-D, C-2 FLOWBY CAPTURED IN TYPE R INLET, PIPED TO C-4.1
	C-4.1								14.1	1.95	3.62	7.1				7.1	1.95	1.0	2.0	650	7.2	1.50	C-3, C-4, C-2.1 COMBINE AT C-4.1 AND PIPE FLOW TO C-6.2
	C-5	C-E	2.30	0.53	12.0	1.22	3.85	4.7	12.0	1.22	3.85	4.7	0.4	0.11	1.0	4.3	1.12	1.0	1.5	180	2.0	1.50	C-5 FLOWBY TO C-7
																				40	5.9	0.11	BASIN C-E CAPTUREDIN TYPE R INLET AND PIPED TO C-6.1
	C-6	C-F	2.60	0.52	12.4	1.36	3.81	5.2	12.4	1.36	3.81	5.2	2.5	0.66	1.0	2.7	0.71	1.0	1.5	180	2.0	1.50	C-6 FLOWBY TO C-7
																				10	5.9	0.03	BASIN C-F STORMWATER CAPTURED IN TYPE R INLET AND PIPED TO C-6.1
	C-6.1								12.4	1.82	3.80	6.9				6.9	1.82	1.0	2.0	650	7.2	1.50	C-E, C-F STORMWATER COMBINED AT C-6.1, PIPED TO C-6.2
	C-6.2								15.6	3.77	3.46	13.1				13.1	3.77	1.0	2.0	650	7.2	1.50	C-6.1, C-4.1 COMBINE AT C-6.2 AND PIPED TO C-7.1
	C-7	C-G	1.64	0.47	10.6	0.77	4.04	3.1	13.9	1.54	3.64	5.6				5.6	1.54	1.0	2.0	30	7.2	0.07	C-G, AND C-3,4,5 FLOWBY CAPTURED BY SUMP INLET AND PIPED TO C-7.1
	C-7.1								17.1	5.31	3.33	17.7				17.7	5.31	1.0	2.0	650	7.2	1.50	C-7 AND C-6.1 COMBINE AT C-7.1, PIPED TO C-8
	C-8	C-H	0.49	0.56	8.3	0.27	4.41	1.2	13.9	0.93	3.64	3.4				3.4	0.93	1.0	2.5	30	8.4	0.06	BASIN C-H, C-6 FLOWBY CAPTURED IN SUMP INLET, PIPED TO C-8.1
	C-8.1								18.6	6.24	3.20	20.0				20.0	6.24	1.5	2.0	250	8.8	0.47	C-8, C-7.1 STORMWATER COMBINE AT C-8.1, PIPED TO C-10
	C-9	C-I	0.90	0.08	13.8	0.07	3.65	0.3	13.8	0.07	3.65	0.3											C-I STORMWATER SHEET FLOW TO C-9, TRICKLE CHANNEL FLOW TO C-12
	C-10	C-J	0.81	0.32	10.8	0.26	4.01	1.0	19.1	6.50	3.16	20.5											BASIN C-J STORMWATER SHEET FLOW TO C-10, COMBINE WITH C-8.1. TRICKLE FLOW TO C-11
	C-11	*C-14*	2.37	0.45	11.2	1.07	3.96	4.2	31.3	19.64	2.42	50.5	50.5	19.64	2.0					900	2.8	5.30	FILING 4 BASIN C-14 STORMWATER CONVEYED VIA TRICKLE CHANNEL TO C-12
	C-12	C-K	3.01	0.08	12.8	0.24	3.75	0.9	36.6	26.45	2.18	57.7											**TOTAL FLOW ENTERING POND C**
	C-12	C-L	3.01	0.08	5.6	0.24	4.99	1.2	5.6	0.24	4.99	1.2											**C-L DISCHARGES TO MAIN STEM TRIBUTARY NUMBER 2**
	O-1	OS-1	10.29	0.08	13.9	0.82	3.64	3.0	13.9	0.82	3.64	3.0											OS-1 DISCHARGES TO TSB THAT OUTFALLS TO EXISTING AREA THAT NATURALLY CHANNELIZES FLOW

HRGreen

GRANDVIEW RESERVE (PHASE 3)

PROPOSED CONDITIONS

DESIGN STORM: 100-YEAR

Calc'd by: CBM

Checked by:

Date: 9/18/2024

			DIRECT RUNOFF						ΣQ100 (cfs)				STREET			PIPE				TRAVEL TIME			REMARKS
STREET	DESIGN POINT	BASIN ID	AREA (ac)	C_{100}	t_c (min)	C_{100}*A (ac)	I (in./ hr.)	Q (cfs)	t_c (min)	C_{100}*A (ac)	I (in./ hr.)	Q (cfs)	Q_{street} (cfs)	C_{100}*A (ac)	SLOPE %	Q_{PIPE} (cfs)	C_{100}*A (ac)	SLOPE %	PIPE SIZE (ft)	LENGTH (ft)	VEL. (ft/s)	TRAVEL TIME (min)	
		G-B	2.15	0.69	17.9	1.48	5.47	8.1	17.9	1.48	5.47	8.1	0.3	0.05	0.8	7.8	1.43	2.0	1.5	1000	1.8	9.32	
																							OFFSITE STORMWATER CURB AND GUTTER FLOW TO R-5
		H-B	4.77	0.59	18.6	2.83	5.37	15.2	18.6	2.83	5.37	15.2	3.1	0.58	0.8	12.1	2.25	2.0	1.5	1000	1.8	9.32	
																							OFFSITE STORMWATER CURB AND GUTTER FLOW TO R-5
	R-1	R-A1	1.20	0.63	6.7	0.76	7.96	6.0	6.7	0.76	7.96	6.0				6.0	0.76	1.0	1.0	95	4.5	0.35	BASIN R-A1 STORMWATER SWALE FLOW TO R-1 AND CAPTURED IN TYPE-C INLET
													4.1	0.70	0.6								***R-2 FLOWBY TO BE CAPTURED BY FUTURE PHASE STORM NETWORK***
	R-2	R-A2	2.54	0.61	15.6	1.56	5.82	9.1	15.6	2.32	5.82	13.5				5.0	0.86	0.5	1.5	65	4.2	0.26	BASIN R-A2 STORMWATER CAPTURED IN TYPE R INLET, COMBINED WITH R-1 AND PIPED TO R-3.1
													3.9	0.66	0.6					400	1.5	4.30	R-3 FLOWBY FLOW TO R-6, CAPTURED BY SUMP INLET
	R-3	R-B	4.19	0.60	15.3	2.50	5.86	14.7	15.3	2.50	5.86	14.7				10.8	1.84	0.6	1.5	8	4.6	0.03	R-B STORMWATER CAPTURED IN TYPE R INLET AND PIPED TO R-3.1
	R-3.1								15.8	4.16	5.78	24.0				24.0	4.16	0.5	2.0	375	5.1	1.23	DP R-3.1 PIPE FLOW TO R-6.1
	R-4	R-C	2.90	0.65	12.6	1.89	6.35	12.0	12.6	1.89	6.35	12.0				12.0	1.89	2.0	1.5	215	8.4	0.43	R-C STORMWATER CAPTURED IN SUMP INLET AND PIPED TO R-5.1
													9.2	1.22	0.9					215	1.9	1.89	
	R-5	R-D	5.53	0.59	18.0	3.25	5.45	17.7	18.0	3.88	5.45	24.5				15.3	2.03	0.5	1.5	50	4.2	0.20	G-B,H-B FLOWBY, BASIN R-D STORMWATER CAPTURED IN TYPE R INLET, FLOWBY TO R-6
	R-5.1								18.2	3.92	5.42	21.2				21.2	3.92	0.5	2.0	375	5.1	1.23	R-5.1 STORMWATER PIPE FLOW TO R-6.1
	R-6	R-E	1.81	0.64	12.5	1.16	6.38	7.4	19.4	3.04	5.26	16.0				16.0	3.04	1.0	2.0	6	7.2	0.01	BASIN R-E STORMWATER, R-3 AND R-5 FLOWBY, CAPTURED IN SUMP INLET AT R-6
	R-6.1								19.5	11.11	5.25	58.4				58.4	11.11	1.0	2.5	30	8.4	0.06	R-6.1 STORMWATER PIPE FLOW TO R-7
	R-7	R-F	0.81	0.66	8.1	0.54	7.45	4.0	19.5	11.65	5.25	61.1				61.1	0.54	1.0	2.5	65	8.4	0.13	BASIN R-F CAPTURED IN SUMP INLET AND PIPED TO R-8
	R-8	R-G	1.80	0.35	10.6	0.63	6.80	4.3	19.5	11.74	5.25	61.6											**TOTAL FLOW ENTERING POND R**
	R-9	R-H	0.46	0.66	9.3	0.30	7.11	2.2	9.3	0.30	7.11	2.2											**R-H STORMWATER FLOWS OFFSITE AND WILL BE DETAINED BY FUTURE DEVELOPMENT**
	F-1	F-A	4.58	0.64	18.2	2.91	5.42	15.8	18.2	2.91	5.42	15.8				15.8	2.91	1.0	2.0	5	7.2	0.01	BASIN F-A STORMWATER CAPTURED IN A SUMP INLET, PIPED TO F-2.1
	F-2	F-B	3.76	0.68	14.7	2.56	5.96	15.2	14.7	2.56	5.96	15.2				15.2	2.56	1.0	2.0	28	7.2	0.06	BASIN F-B STORMWATER CAPTURED IN A SUMP INLET, PIPED TO DP F-2.1
	F-2.1								18.2	5.47	5.42	29.7				29.7	5.47	1.0	2.5	375	8.4	0.75	F-1, F-2, F-3 COMBINE AT F-2.1, PIPED TO F-6.2
													0.9	0.13	0.5					650	1.4	7.66	F-3 FLOWBY TO F-5
	F-3	F-C1	2.08	0.67	12.5	1.39	6.37	8.9	12.5	1.39	6.37	8.9				8.0	1.26	0.5	1.5	300	4.2	1.19	BASIN F-C1 STORMWATER CAPTURED IN TYPE R INLET, PIPED TO F-2.1
													1.0	0.17	1.4					250	2.4	1.76	F-4 FLOWBY TO F-5
	F-4	F-C3	1.33	0.72	12.9	0.96	6.28	6.0	12.9	0.96	6.28	6.0				5.0	0.80	1.0	1.5	300	5.9	0.84	BASIN F-C3 STORMWATER CAPTURED IN TYPE R AND PIPED TO F-6.1
													4.8	0.54	1.4					325	2.4	2.29	F-5 FLOWBY TO F-7
	F-5	F-C2	3.64	0.70	16.1	2.55	5.73	14.6	16.1	2.85	5.73	16.3				11.5	2.01	1.0	1.5	85	5.9	0.24	
													3.7	0.64	1.4					355	2.4	2.50	F-6 FLOWBY TO F-7
	F-6	F-D	3.13	0.57	15.8	1.80	5.77	10.4	15.8	1.80	5.77	10.4				6.7	1.16	1.0	1.5	55	5.9	0.15	BASIN F-D STORMWATER CAPTURED IN TYPE R INLET AND PIPED TO F-6.1
	F-6.1								16.3	3.96	5.69	22.6				22.6	3.96	1.0	2.5	60	8.4	0.12	F-5, F-6 COMBINE AT F-6.1 AND IS PIPED TO F-6.2
	F-6.2								19.0	9.44	5.32	50.2				50.2	9.44	1.0	2.5	325	8.4	0.65	F-6.1, F-2.1 COMBINE AT F-6.2 AND IS PIPED TO F-7.1
	F-7	F-E	4.42	0.64	13.8	2.84	6.13	17.4	18.4	4.02	5.40	21.7				21.7	4.02	1.0	2.0	30	7.2	0.07	BASIN F-E STORMWATER AND FLOWBY FROM F-5,6 CAPTURED IN SUMP AND PIPED TO F-7.1
	F-7.1								19.6	13.46	5.24	70.5				70.5	13.46	1.0	2.5	10	8.4	0.02	F-7.1 PIPED TO F-8
	F-8	F-F	0.94	0.66	11.0	0.62	6.69	4.1	19.6	14.07	5.23	73.7				73.7	14.07	2.0	3.0	180	13.3	0.22	BASIN F-F STORMWATER CAPTURED IN SUMP INLET AT F-8 AND PIPED TO F-9
													2.2	0.34	5.0					50	4.5	0.19	
	F-9	F-G	0.63	0.55	12.2	0.34	6.43	2.2	12.2	0.34	6.43	2.2											F-G STORMWATER COMBINED WITH F-8 PIPE FLOW AT PONDF
	F-10	F-H	1.64	0.35	11.4	0.57	6.60	3.8	19.8	14.99	5.21	78.0											**TOTAL FLOW ENTERING POND F**

HRGreen

GRANDVIEW RESERVE (PHASE 3)

PROPOSED CONDITIONS

DESIGN STORM: 100-YEAR

Calc'd by: CBM

Checked by:

Date: 9/18/2024

			DIRECT RUNOFF						ΣQ100 (cfs)				STREET			PIPE				TRAVEL TIME			REMARKS
STREET	DESIGN POINT	BASIN ID	AREA (ac)	C_{100}	t_c (min)	C_{100}*A (ac)	I (in./ hr.)	Q (cfs)	t_c (min)	C_{100}*A (ac)	I (in./ hr.)	Q (cfs)	Q_{street} (cfs)	C_{100}*A (ac)	SLOPE %	Q_{PIPE} (cfs)	C_{100}*A (ac)	SLOPE %	PIPE SIZE (ft)	LENGTH (ft)	VEL. (ft/s)	TRAVEL TIME (min)	
	F-11	F-I	1.52	0.63	5.0	0.96	8.68	8.3	5.0	0.96	8.68	8.3											F-I STORMWATER FLOWS OFFSITE FOLLOWING EXISTING DRAINAGE PATTERNS
													3.8	0.63	1.6					650	2.5	4.28	C-1 FLOWBY TO C-3
	C-1	C-A	3.43	0.51	14.6	1.75	5.98	10.5	14.6	1.75	5.98	10.5				6.7	1.12	1.5	1.5	6	7.3	0.01	BASIN C-A STORMWATER CAPTURED IN TYPE R INLET, PIPED TO C-2.1
													7.5	1.22	1.6					650	2.5	4.28	C-2 FLOWBY TO C-4
	C-2	C-B	4.54	0.56	13.7	2.54	6.14	15.6	13.7	2.54	6.14	15.6				8.1	1.32	1.5	1.5	28	7.3	0.06	BASIN C-B STORMWATER CAPTURED IN TYPE R INLET, PIPED TO C-2.1
	C-2.1								14.6	2.44	5.98	14.6				14.6	2.44	1.5	1.5	650	7.3	1.49	C-1 AND C-2 COMBINE AT C-2.1, PIPED TO C-4.1
													5.0	0.19	1.6					170	2.5	1.12	C3- FLOWBY TO C-7
	C-3	C-C	2.30	0.60	14.1	1.38	6.07	8.4	14.1	2.01	6.07	12.2				7.2	1.19	1.5	1.5	6	7.3	0.01	BASIN C-C, C-1 FLOWBY CAPTURED IN TYPE R INLET, PIPED TO C-4.1
													7.3	1.19	1.6					200	2.5	1.32	C-4 FLOWBY TO C-7
	C-4	C-D	1.88	0.68	12.3	1.27	6.40	8.1	13.7	2.49	6.13	15.3				8.0	1.30	1.5	1.5	28	7.3	0.06	BASIN C-D, C-2 FLOWBY CAPTURED IN TYPE R INLET, PIPED TO C-4.1
	C-4.1								16.1	4.93	5.73	28.3				28.3	4.93	1.0	2.5	150	8.4	0.30	C-3, C-4, C-2.1 COMBINE AT C-4.1 AND PIPE FLOW TO C-6.2
													3.6	0.55	1.6					180	2.5	1.19	C-5 FLOWBY TO C-7
	C-5	C-E	2.30	0.68	12.0	1.57	6.47	10.2	12.0	1.57	6.47	10.2				6.6	1.02	1.0	1.5	40	5.9	0.11	BASIN C-E CAPTUREDIN TYPE R INLET AND PIPED TO C-6.1
													7.6	1.19	1.6					180	2.5	1.19	C-6 FLOWBY TO C-7
	C-6	C-F	2.60	0.68	12.4	1.76	6.39	11.3	12.4	1.76	6.39	11.3				3.7	0.58	1.0	1.5	10	5.9	0.03	BASIN C-F STORMWATER CAPTURED IN TYPE R INLET AND PIPED TO C-6.1
	C-6.1								12.4	1.60	6.39	10.2				10.2	1.60	1.5	2.0	100	8.8	0.19	C-E, C-F STORMWATER COMBINED AT C-6.1, PIPED TO C-6.2
	C-6.2								16.4	6.53	5.69	37.1				37.1	6.53	0.5	2.5	60	5.9	0.17	C-6.1, C-4.1 COMBINE AT C-6.2 AND PIPED TO C-7.1
	C-7	C-G	1.64	0.64	10.6	1.04	6.79	7.1	15.2	4.16	5.88	24.5				24.5	4.16	0.5	2.0	60	5.1	0.20	C-G, AND C-3,4,5 FLOWBY CAPTURED BY SUMP INLET AND PIPED TO C-7.1
	C-7.1								16.6	4.74	5.66	26.8				26.8	4.74	1.5	2.5	60	10.2	0.10	C-7 AND C-6.1 COMBINE AT C-7.1, PIPED TO C-8
	C-8	C-H	0.49	0.70	8.3	0.35	7.40	2.6	13.6	1.53	6.16	9.4				9.4	1.53	0.5	2.0	60	5.1	0.20	BASIN C-H, C-6 FLOWBY CAPTURED IN SUMP INLET, PIPED TO C-8.1
	C-8.1								16.7	6.27	5.65	35.4				35.4	6.27	1.5	2.0	180	8.8	0.34	C-8, C-7.1 STORMWATER COMBINE AT C-8.1, PIPED TO C-10
													1.9	0.32	10.0					250	6.3	0.66	
	C-9	C-I	0.90	0.35	13.8	0.32	6.13	1.9	13.8	0.32	6.13	1.9											C-I STORMWATER SHEET FLOW TO C-9, TRICKLE CHANNEL FLOW TO C-12
	C-10	C-J	0.81	0.53	10.8	0.43	6.73	2.9	17.0	6.70	5.60	37.5											BASIN C-J STORMWATER SHEET FLOW TO C-10, COMBINE WITH C-8.1. TRICKLE FLOW TO C-11
													107.4	25.74	2.0					900	2.8	5.30	
	C-11	*C-14*	2.37	0.59	11.2	1.40	6.65	9.3	31.3	25.74	4.06	107.4											FILING 4 BASIN C-14 STORMWATER CONVEYED VIA TRICKLE CHANNEL TO C-12
	C-12	C-K	3.01	0.35	12.8	1.05	6.30	6.6	36.6	33.81	3.66	123.8											**TOTAL FLOW ENTERING POND C**
	C-12	C-L	3.01	0.35	5.6	1.05	8.38	8.8	5.6	1.05	8.38	8.8											**C-L FLOWS FOLLOW HISTORIC PATTERNS OFFSITE**
	O-1	OS-1	10.29	0.35	13.9	3.60	6.10	22.0	13.9	3.60	6.10	22.0											OS-1 DISCHARGES TO TSB THAT OUTFALLS TO EXISTING AREA THAT NATURALLY CHANNELIZES FLOW

APPENDIX C – HYDRAULIC CALCULATIONS
(TO BE PROVIDED WITH FDR)

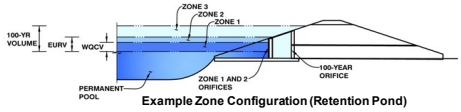
APPENDIX D – WATER QUALITY & DETENTION

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: Grandview-PHASE 3

Basin ID: POND C



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	EDB	
Watershed Area =	67.53	acres
Watershed Length =	1,890	ft
Watershed Length to Centroid =	1,050	ft
Watershed Slope =	0.020	ft/ft
Watershed Imperviousness =	53.00%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

		Optional User Overrides	
Water Quality Capture Volume (WQCV) =	1.208 acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	4.195 acre-feet		acre-feet
2-yr Runoff Volume (P1 = 0.93 in.) =	2.379 acre-feet	0.93	inches
5-yr Runoff Volume (P1 = 1.21 in.) =	3.190 acre-feet	1.21	inches
10-yr Runoff Volume (P1 = 1.46 in.) =	4.011 acre-feet	1.46	inches
25-yr Runoff Volume (P1 = 1.84 in.) =	5.462 acre-feet	1.84	inches
50-yr Runoff Volume (P1 = 2.15 in.) =	6.932 acre-feet	2.15	inches
100-yr Runoff Volume (P1 = 2.49 in.) =	8.780 acre-feet	2.49	inches
500-yr Runoff Volume (P1 = 3.35 in.) =	13.395 acre-feet	3.35	inches
Approximate 2-yr Detention Volume =	2.114 acre-feet		
Approximate 5-yr Detention Volume =	2.867 acre-feet		
Approximate 10-yr Detention Volume =	3.609 acre-feet		
Approximate 25-yr Detention Volume =	4.851 acre-feet		
Approximate 50-yr Detention Volume =	5.606 acre-feet		
Approximate 100-yr Detention Volume =	6.488 acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	1.208	acre-feet
Zone 2 Volume (EURV - Zone 1) =	2.987	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	2.293	acre-feet
Total Detention Basin Volume =	6.488	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

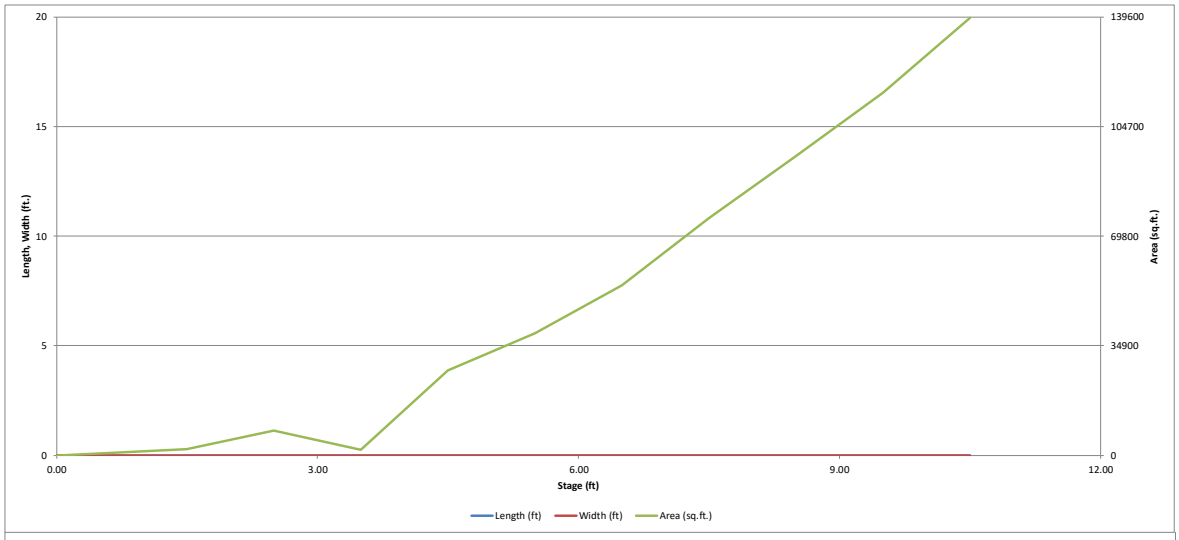
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

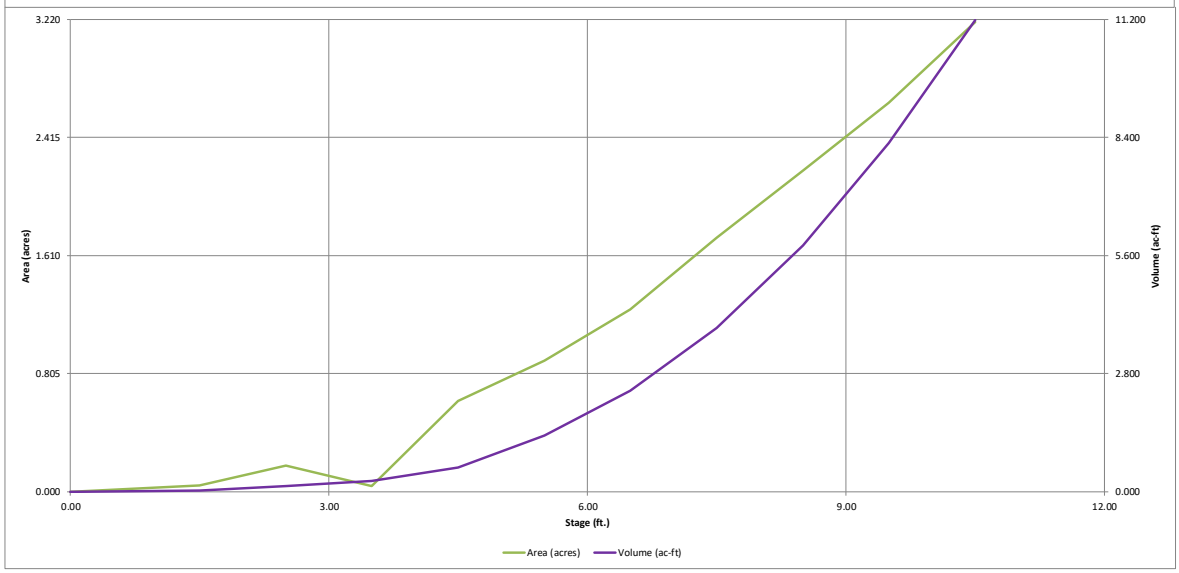
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
6910.5 **Top of Micropool**	--	0.00	--	--	--	0	0.000		
6912	--	1.50	--	--	--	1,805	0.041	1,354	0.031
6913	--	2.50	--	--	--	7,751	0.178	6,132	0.141
6914	--	3.50	--	--	--	1,650	0.038	10,832	0.249
6915	--	4.50	--	--	--	26,906	0.618	25,110	0.576
6916	--	5.50	--	--	--	38,893	0.893	58,009	1.332
6917	--	6.50	--	--	--	54,061	1.241	104,486	2.399
6918	--	7.50	--	--	--	75,492	1.733	169,263	3.886
6919	--	8.50	--	--	--	95,279	2.187	254,648	5.846
6920	--	9.50	--	--	--	115,443	2.650	360,009	8.265
6921	--	10.50	--	--	--	139,514	3.203	487,488	11.191

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)



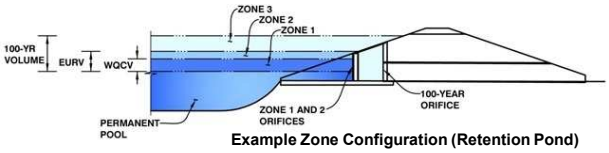


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **Grandview-PHASE 3**

Basin ID: **POND C**



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	5.36	1.208	Orifice Plate
Zone 2 (EURV)	7.68	2.987	Circular Orifice
Zone 3 (100-year)	8.79	2.293	Weir&Pipe (Restrict)
Total (all zones)		6.488	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =		ft (distance below the filtration media surface)	Underdrain Orifice Area =		ft²
Underdrain Orifice Diameter =		inches	Underdrain Orifice Centroid =		feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	1.979E-02	ft²
Depth at top of Zone using Orifice Plate =	7.68	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	N/A	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	2.85	sq. inches (diameter = 1-7/8 inches)	Elliptical Slot Area =	N/A	ft²

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.79	3.57					
Orifice Area (sq. inches)	2.85	2.85	2.85					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected			Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	5.36	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	0.11	N/A	ft²
Depth at top of Zone using Vertical Orifice =	7.68	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	0.19	N/A	feet
Vertical Orifice Diameter =	4.50	N/A	inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	7.70	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	7.70	N/A	feet
Overflow Weir Front Edge Length =	8.00	N/A	feet	Overflow Weir Slope Length =	8.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	16.29	N/A	
Horiz. Length of Weir Sides =	8.00	N/A	feet	Overflow Grate Open Area w/o Debris =	44.54	N/A	ft²
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	22.27	N/A	ft²
Debris Clogging % =	50%	N/A	%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	1.00	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	2.73	N/A	ft²
Outlet Pipe Diameter =	24.00	N/A	inches	Outlet Orifice Centroid =	0.88	N/A	feet
Restrictor Plate Height Above Pipe Invert =	19.50		inches	Half-Central Angle of Restrictor Plate on Pipe =	2.25	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	8.70	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.79	feet
Spillway Crest Length =	75.00	feet	Stage at Top of Freeboard =	10.49	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	3.19	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	11.13	acre-ft

Calculated Parameters for Spillway

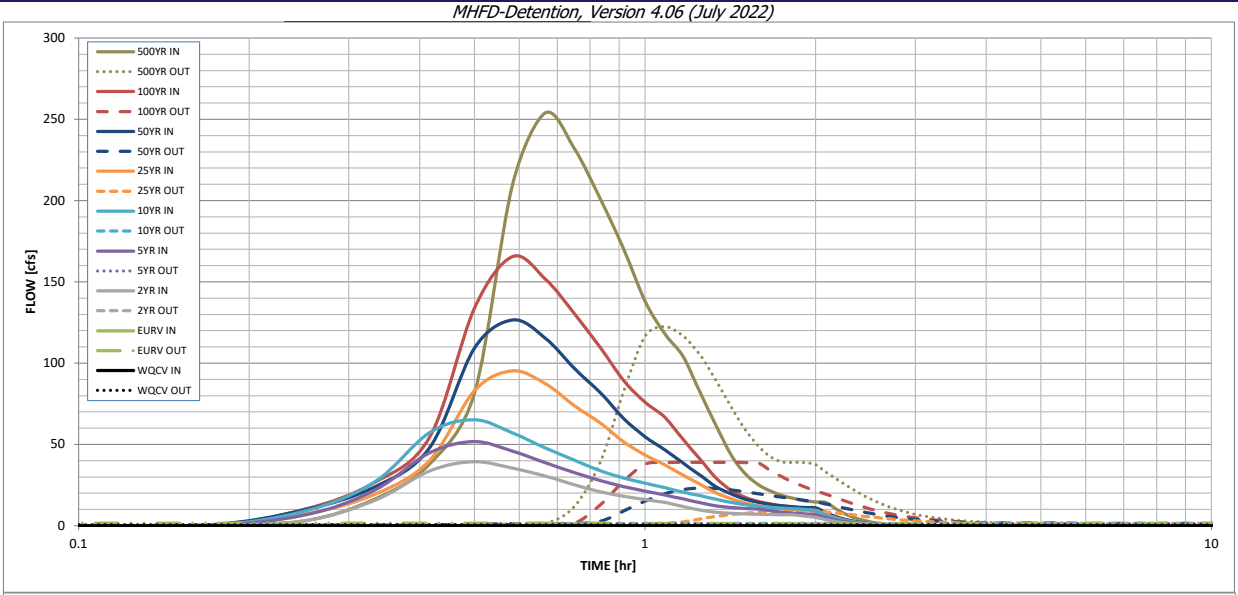
Routed Hydrograph Results

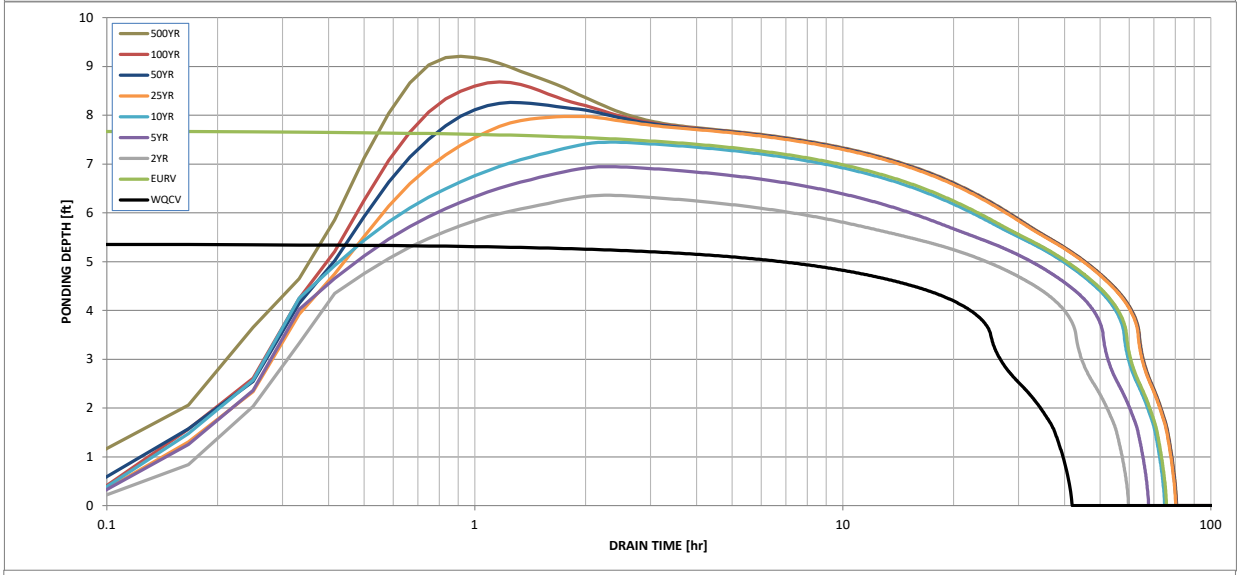
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

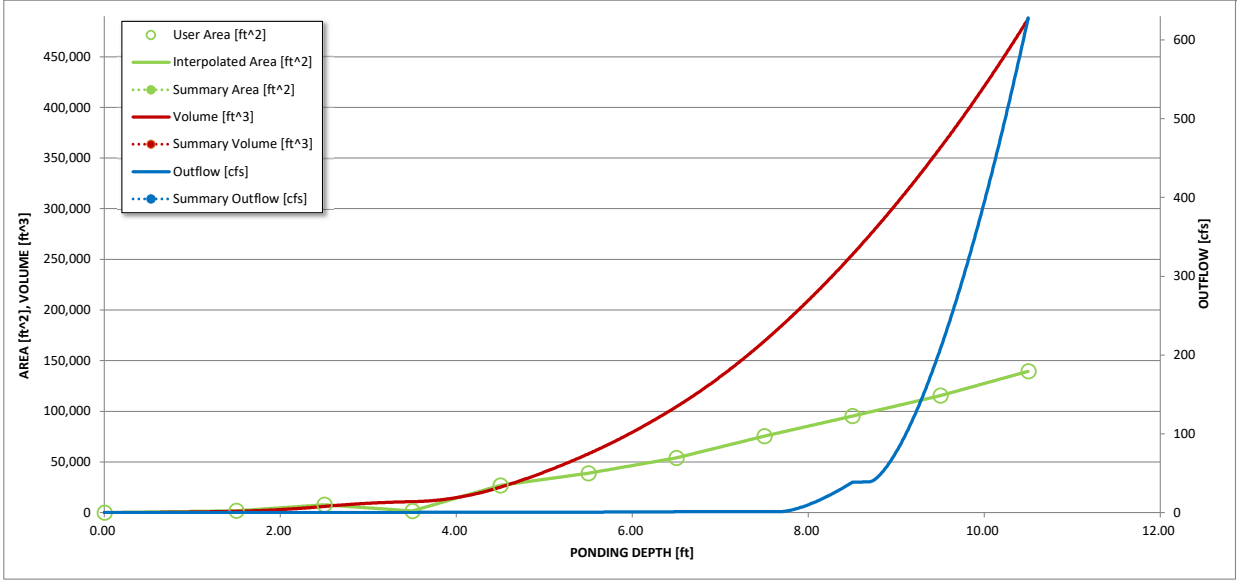
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	0.93	1.21	1.46	1.84	2.15	2.49	3.35
CUHP Runoff Volume (acre-ft) =	1.208	4.195	2.379	3.190	4.011	5.462	6.932	8.780	13.395
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	2.379	3.190	4.011	5.462	6.932	8.780	13.395
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.0	0.5	1.0	5.6	22.9	44.9	96.8
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.01	0.01	0.08	0.34	0.66	1.43
Peak Inflow Q (cfs) =	N/A	N/A	39.4	51.7	65.1	95.2	126.5	165.5	253.9
Peak Outflow Q (cfs) =	0.5	1.5	1.1	1.3	1.4	8.9	23.1	39.0	122.4
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	2.4	1.5	1.6	1.0	0.9	1.3
Structure Controlling Flow =	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.2	0.5	0.8	0.9
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	37	65	52	59	64	67	66	63	59
Time to Drain 99% of Inflow Volume (hours) =	**40**	71	56	63	70	74	73	72	69
Maximum Ponding Depth (ft) =	5.36	7.68	6.36	6.94	7.44	7.97	8.26	8.68	9.20
Area at Maximum Ponding Depth (acres) =	0.85	1.81	1.19	1.46	1.70	1.95	2.07	2.27	2.51
Maximum Volume Stored (acre-ft) =	1.209	4.205	2.216	2.992	3.783	4.750	5.313	6.247	7.490

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







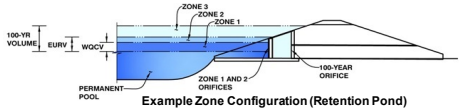
S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: GRANDVIEW - PHASE 3

Basin ID: POND F (SOUTHEAST PHASE 3)



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	**EDB**	
Watershed Area =	26.63	acres
Watershed Length =	2,000	ft
Watershed Length to Centroid =	1,000	ft
Watershed Slope =	0.018	ft/ft
Watershed Imperviousness =	50.00%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

		Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.458 acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	1.535 acre-feet		acre-feet
2-yr Runoff Volume (P1 = 0.93 in.) =	0.871 acre-feet	0.93	inches
5-yr Runoff Volume (P1 = 1.21 in.) =	1.172 acre-feet	1.21	inches
10-yr Runoff Volume (P1 = 1.46 in.) =	1.478 acre-feet	1.46	inches
25-yr Runoff Volume (P1 = 1.84 in.) =	2.029 acre-feet	1.84	inches
50-yr Runoff Volume (P1 = 2.15 in.) =	2.601 acre-feet	2.15	inches
100-yr Runoff Volume (P1 = 2.49 in.) =	3.327 acre-feet	2.49	inches
500-yr Runoff Volume (P1 = 3.35 in.) =	5.141 acre-feet	3.35	inches
Approximate 2-yr Detention Volume =	0.772 acre-feet		
Approximate 5-yr Detention Volume =	1.048 acre-feet		
Approximate 10-yr Detention Volume =	1.323 acre-feet		
Approximate 25-yr Detention Volume =	1.785 acre-feet		
Approximate 50-yr Detention Volume =	2.069 acre-feet		
Approximate 100-yr Detention Volume =	2.411 acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.458	acre-feet
Zone 2 Volume (EURV - Zone 1) =	1.078	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.876	acre-feet
Total Detention Basin Volume =	2.411	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

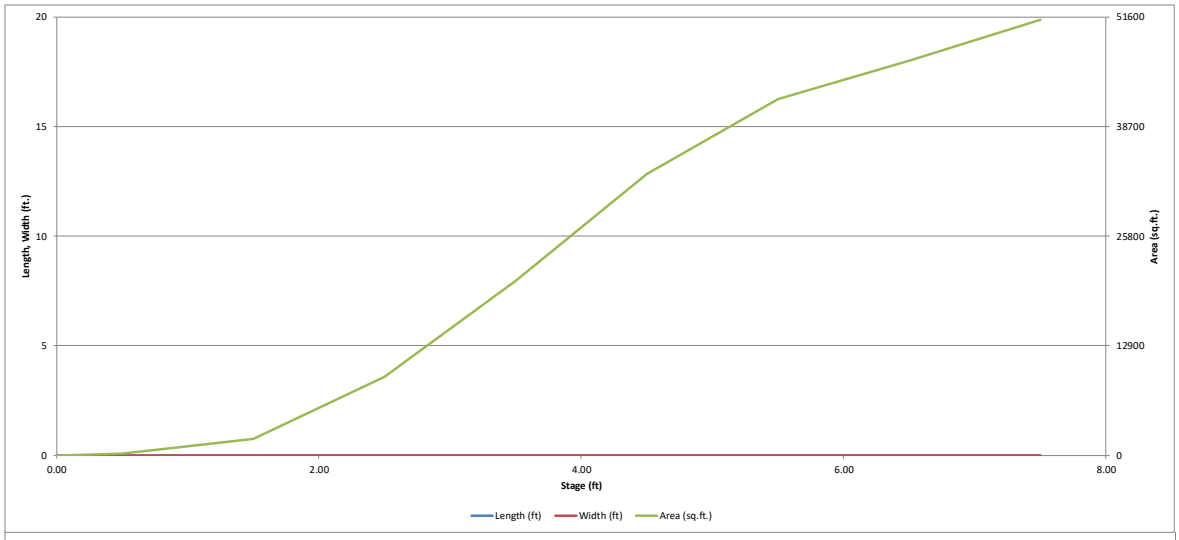
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

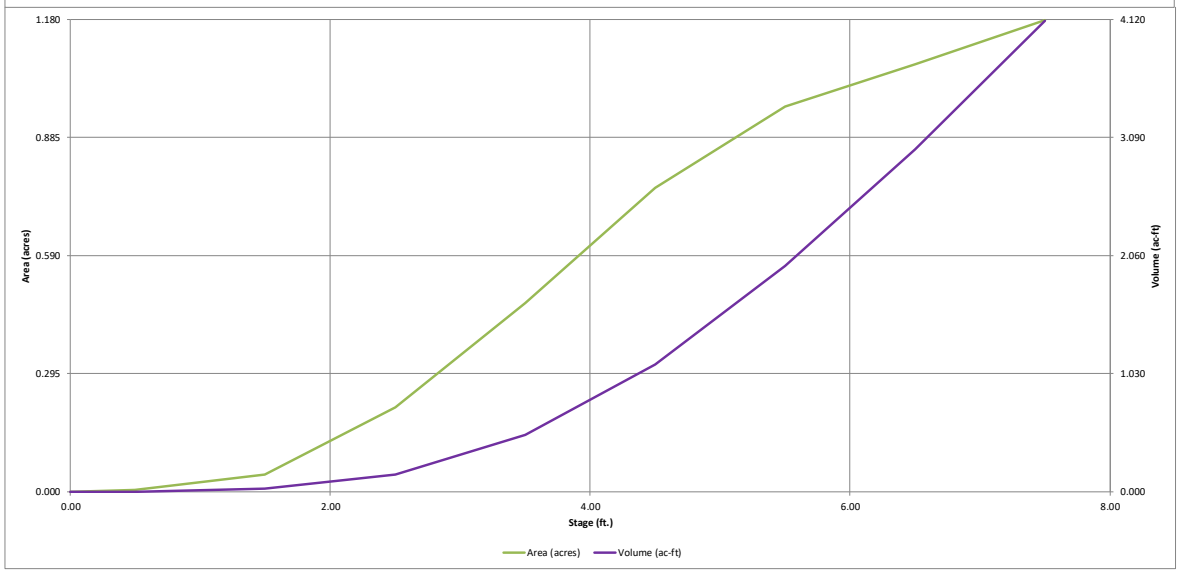
Depth Increment = 1.00 ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
6911.5 **Top of Micropool**	--	0.00	--	--	--	0	0.000		
6912	--	0.50	--	--	--	200	0.005	50	0.001
6913	--	1.50	--	--	--	1,891	0.043	1,095	0.025
6914	--	2.50	--	--	--	9,182	0.211	6,632	0.152
6915	--	3.50	--	--	--	20,549	0.472	21,497	0.494
6916	--	4.50	--	--	--	33,087	0.760	48,315	1.109
6917	--	5.50	--	--	--	41,903	0.962	85,810	1.970
6918	--	6.50	--	--	--	46,500	1.067	130,012	2.985
6919	--	7.50	--	--	--	51,345	1.179	178,934	4.108

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)



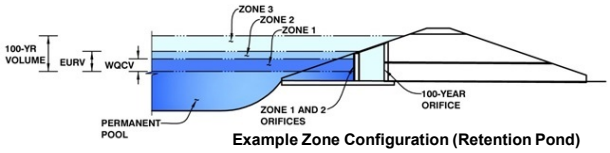


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **GRANDVIEW - PHASE 3**

Basin ID: **POND F (SOUTHEAST PHASE 3)**



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.43	0.458	Orifice Plate
Zone 2 (EURV)	5.03	1.078	Circular Orifice
Zone 3 (100-year)	5.95	0.876	Weir&Pipe (Restrict)
Total (all zones)		2.411	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

			Calculated Parameters for Underdrain		
Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft²
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

			Calculated Parameters for Plate		
Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	9.097E-03	ft²
Depth at top of Zone using Orifice Plate =	3.43	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	13.40	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	1.31	sq. inches (diameter = 1-1/4 inches)	Elliptical Slot Area =	N/A	ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.14	2.29					
Orifice Area (sq. inches)	1.31	1.31	1.31					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected		Calculated Parameters for Vertical Orifice	Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	3.43	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	0.05	N/A	ft²
Depth at top of Zone using Vertical Orifice =	5.03	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	0.13	N/A	feet
Vertical Orifice Diameter =	3.00	N/A	inches				

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected		Calculated Parameters for Overflow Weir	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	5.10	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	5.10	N/A	feet
Overflow Weir Front Edge Length =	5.67	N/A	feet	Overflow Weir Slope Length =	2.92	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	12.62	N/A	
Horiz. Length of Weir Sides =	2.92	N/A	feet	Overflow Grate Open Area w/o Debris =	11.52	N/A	ft²
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	5.76	N/A	ft²
Debris Clogging % =	50%	N/A	%				

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected		Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	1.00	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	0.91	N/A	ft²
Outlet Pipe Diameter =	21.00	N/A	inches	Outlet Orifice Centroid =	0.41	N/A	feet
Restrictor Plate Height Above Pipe Invert =	8.50		inches	Half-Central Angle of Restrictor Plate on Pipe =	1.38	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

			Calculated Parameters for Spillway		
Spillway Invert Stage=	5.90	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.60	feet
Spillway Crest Length =	30.00	feet	Stage at Top of Freeboard =	7.50	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	1.18	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	4.11	acre-ft

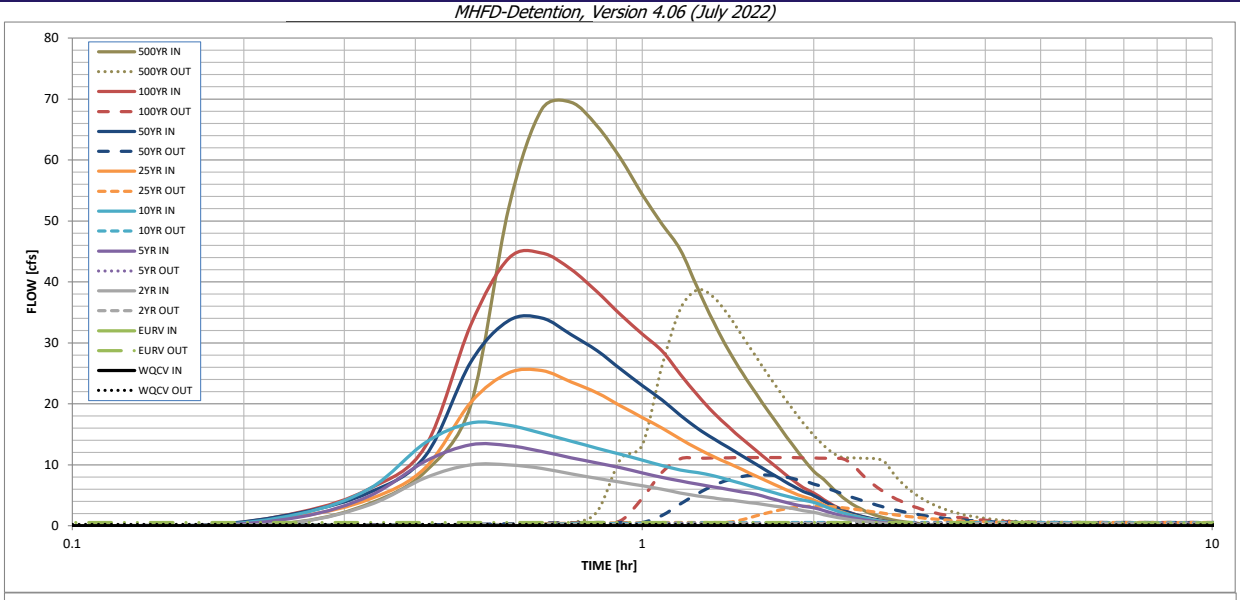
Routed Hydrograph Results

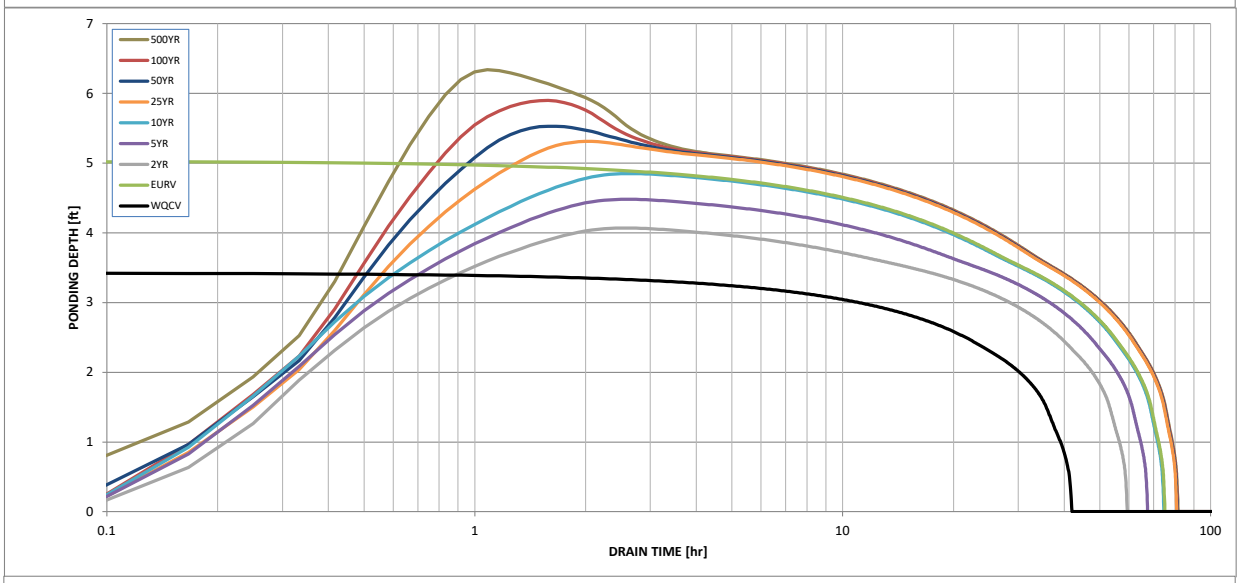
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

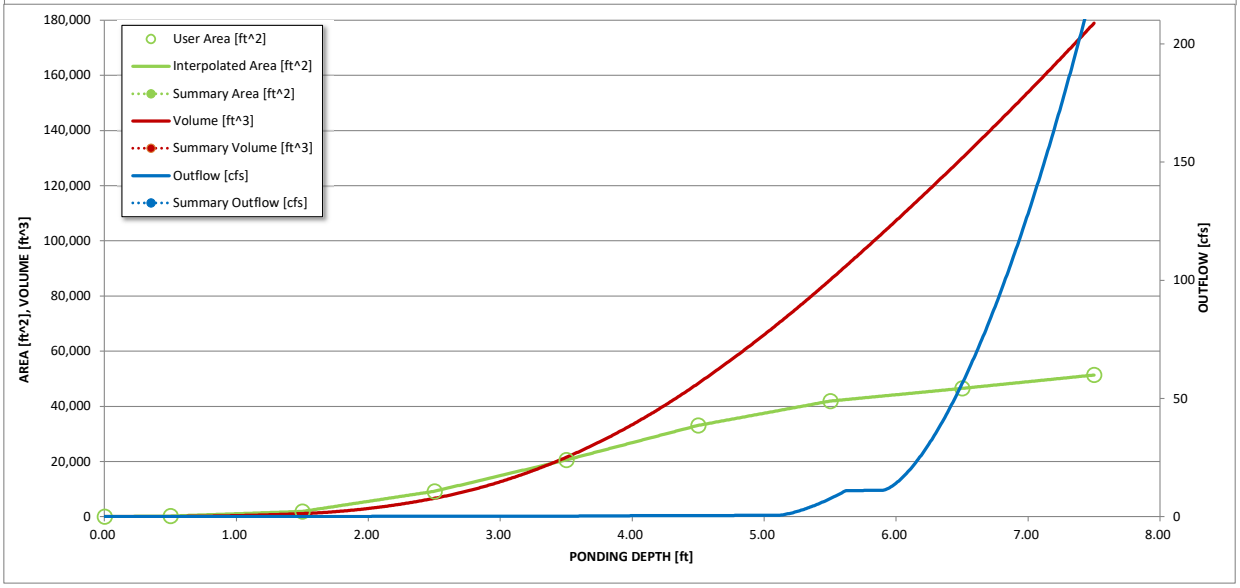
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	0.93	1.21	1.46	1.84	2.15	2.49	3.35
CUHP Runoff Volume (acre-ft) =	0.458	1.535	0.871	1.172	1.478	2.029	2.601	3.327	5.141
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.871	1.172	1.478	2.029	2.601	3.327	5.141
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.0	0.1	0.3	1.4	5.9	11.9	26.3
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.01	0.01	0.05	0.22	0.45	0.99
Peak Inflow Q (cfs) =	N/A	N/A	10.0	13.3	16.9	25.4	34.1	44.7	69.5
Peak Outflow Q (cfs) =	0.2	0.5	0.4	0.5	0.5	3.2	8.3	11.2	38.7
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	3.4	2.1	2.3	1.4	0.9	1.5
Structure Controlling Flow =	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Spillway	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.2	0.7	0.9	0.9
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	66	53	60	66	70	68	66	61
Time to Drain 99% of Inflow Volume (hours) =	**40**	71	57	64	70	76	75	74	72
Maximum Ponding Depth (ft) =	3.43	5.03	4.07	4.48	4.85	5.31	5.53	5.90	6.34
Area at Maximum Ponding Depth (acres) =	0.45	0.87	0.63	0.75	0.83	0.92	0.96	1.00	1.05
Maximum Volume Stored (acre-ft) =	0.461	1.540	0.803	1.087	1.379	1.782	1.989	2.353	2.805

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







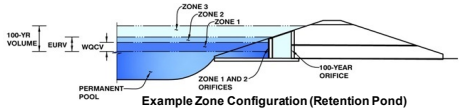
S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: GRANDVIEW - PHASE 3

Basin ID: POND R (REX RD ENTRANCE)



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	**EDB**	
Watershed Area =	19.29	acres
Watershed Length =	1,500	ft
Watershed Length to Centroid =	1,000	ft
Watershed Slope =	0.020	ft/ft
Watershed Imperviousness =	42.00%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

			Optional User Overrides
Water Quality Capture Volume (WQCV) =	0.298	acre-feet	acre-feet
Excess Urban Runoff Volume (EURV) =	0.890	acre-feet	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.675	acre-feet	1.19 inches
5-yr Runoff Volume (P1 = 1.5 in.) =	0.905	acre-feet	1.50 inches
10-yr Runoff Volume (P1 = 1.75 in.) =	1.089	acre-feet	1.75 inches
25-yr Runoff Volume (P1 = 2 in.) =	1.420	acre-feet	2.00 inches
50-yr Runoff Volume (P1 = 2.25 in.) =	1.743	acre-feet	2.25 inches
100-yr Runoff Volume (P1 = 2.52 in.) =	2.164	acre-feet	2.52 inches
500-yr Runoff Volume (P1 = 3.14 in.) =	3.074	acre-feet	inches
Approximate 2-yr Detention Volume =	0.568	acre-feet	
Approximate 5-yr Detention Volume =	0.751	acre-feet	
Approximate 10-yr Detention Volume =	0.924	acre-feet	
Approximate 25-yr Detention Volume =	1.143	acre-feet	
Approximate 50-yr Detention Volume =	1.288	acre-feet	
Approximate 100-yr Detention Volume =	1.485	acre-feet	

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.298	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.592	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.596	acre-feet
Total Detention Basin Volume =	1.485	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

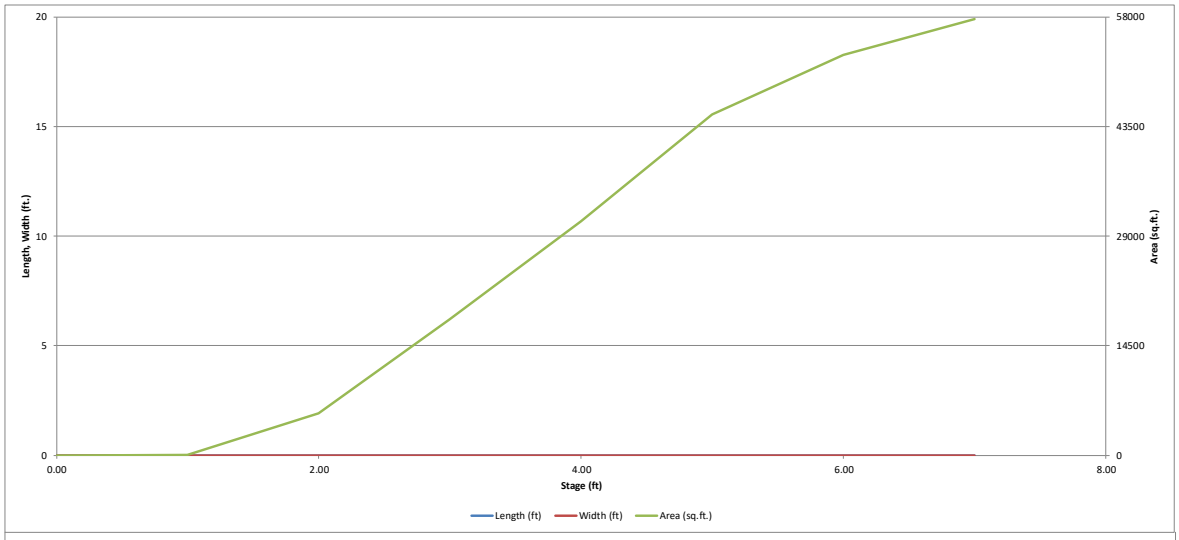
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

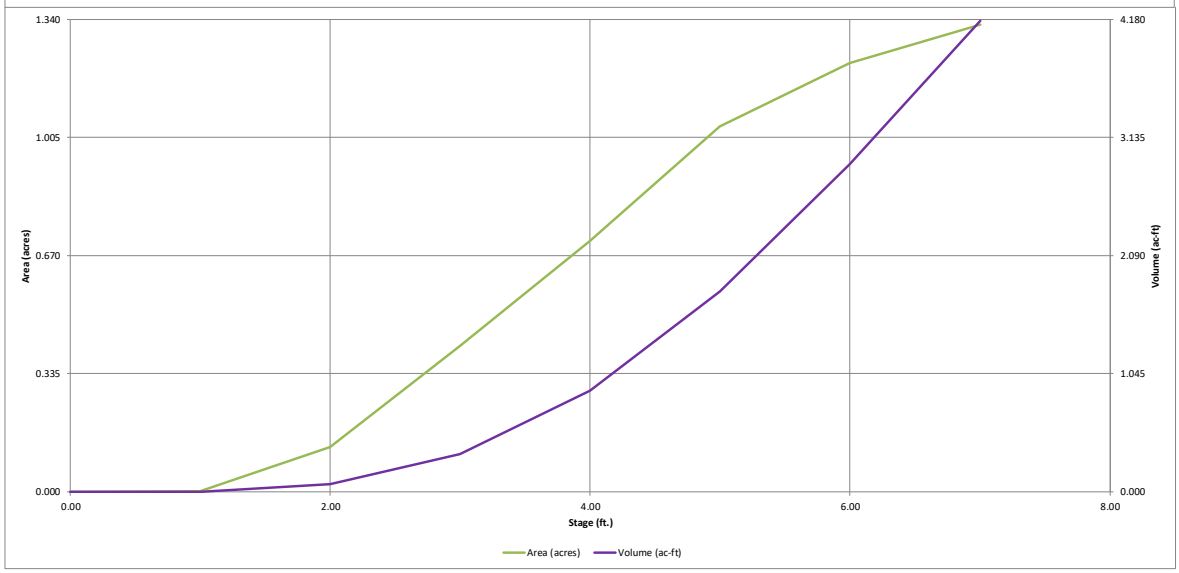
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
6909 **Top of Micropool**	--	0.00	--	--	--	0	0.000		
6910	--	1.00	--	--	--	50	0.001	25	0.001
6911	--	2.00	--	--	--	5,520	0.127	2,810	0.065
6912	--	3.00	--	--	--	18,000	0.413	14,570	0.334
6913	--	4.00	--	--	--	30,967	0.711	39,053	0.897
6914	--	5.00	--	--	--	45,118	1.036	77,096	1.770
6915	--	6.00	--	--	--	53,000	1.217	126,155	2.896
6916	--	7.00	--	--	--	57,768	1.326	181,539	4.168

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)



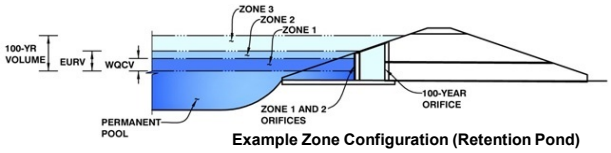


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: GRANDVIEW - PHASE 3

Basin ID: POND R (REX RD ENTRANCE)



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.91	0.298	Orifice Plate
Zone 2 (EURV)	4.00	0.592	Orifice Plate
Zone 3 (100-year)	4.72	0.596	Weir&Pipe (Restrict)
Total (all zones)		1.485	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)

Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = ft^2

Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)

Depth at top of Zone using Orifice Plate = 4.00 ft (relative to basin bottom at Stage = 0 ft)

Orifice Plate: Orifice Vertical Spacing = N/A inches

Orifice Plate: Orifice Area per Row = N/A sq. inches

Calculated Parameters for Plate

WQ Orifice Area per Row = N/A ft^2

Elliptical Half-Width = N/A feet

Elliptical Slot Centroid = N/A feet

Elliptical Slot Area = N/A ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.33	2.67	3.00				
Orifice Area (sq. inches)	1.08	1.08	1.08	3.15				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft^2
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.10	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	4.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	4.00	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	0%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	4.10	N/A	feet
Overflow Weir Slope Length =	4.00	N/A	feet
Grate Open Area / 100-yr Orifice Area =	13.56	N/A	
Overflow Grate Open Area w/o Debris =	11.14	N/A	ft^2
Overflow Grate Open Area w/ Debris =	11.14	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	1.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	8.50		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	0.82	N/A	ft^2
Outlet Orifice Centroid =	0.41	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	1.52	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage= 4.60 ft (relative to basin bottom at Stage = 0 ft)

Spillway Crest Length = 9.00 feet

Spillway End Slopes = 4.00 H:V

Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway

Spillway Design Flow Depth= 0.85 feet

Stage at Top of Freeboard = 6.45 feet

Basin Area at Top of Freeboard = 1.26 acres

Basin Volume at Top of Freeboard = 3.44 acre-ft

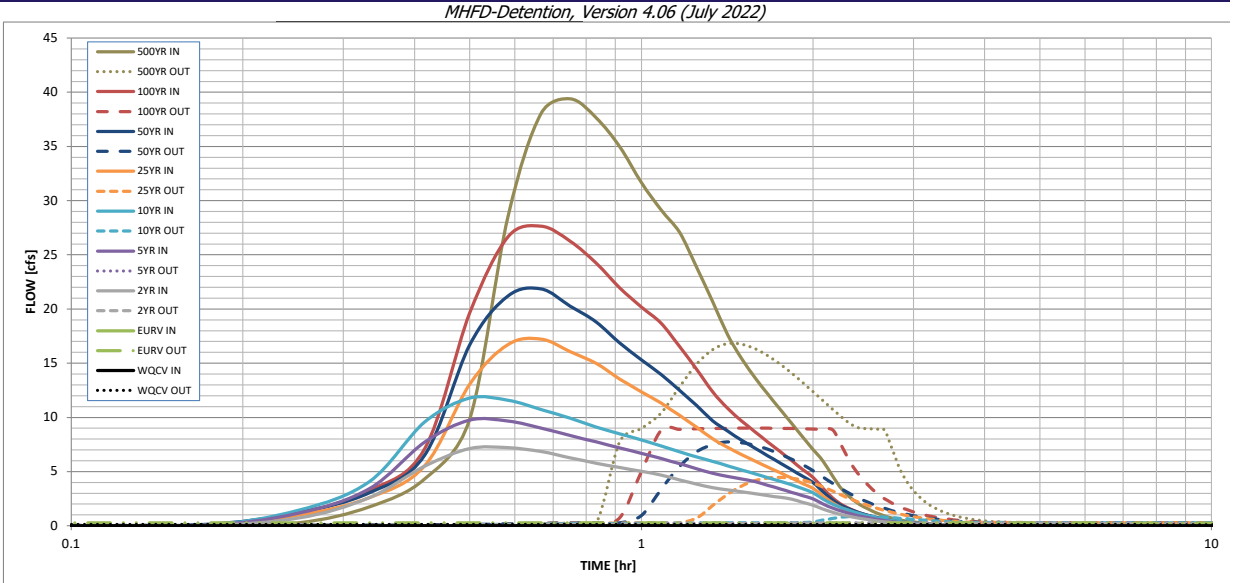
Routed Hydrograph Results

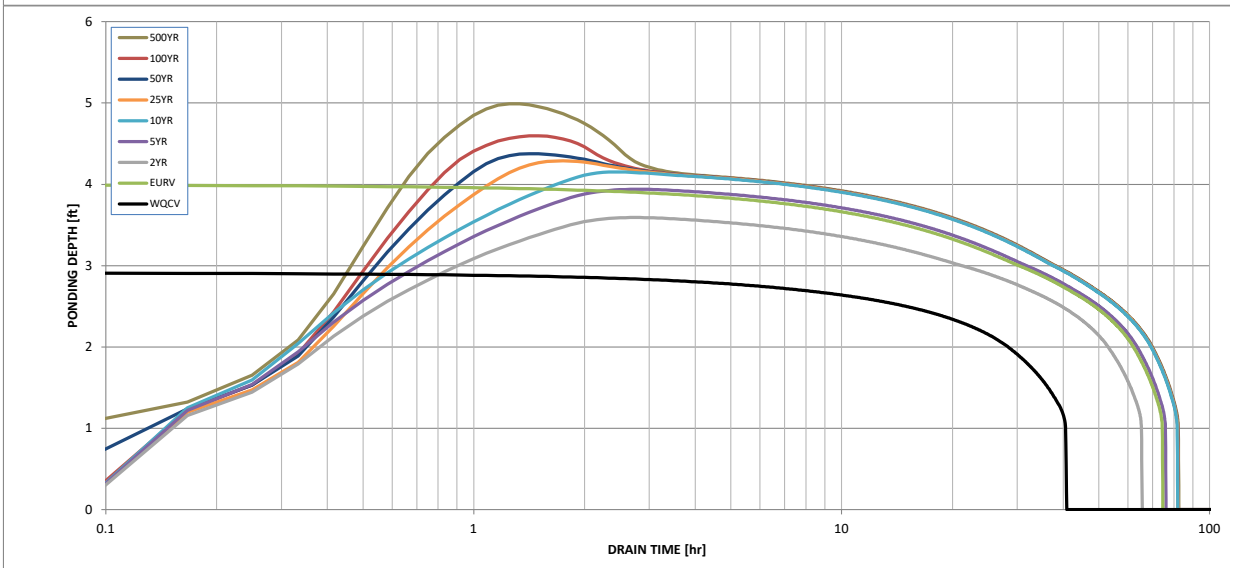
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

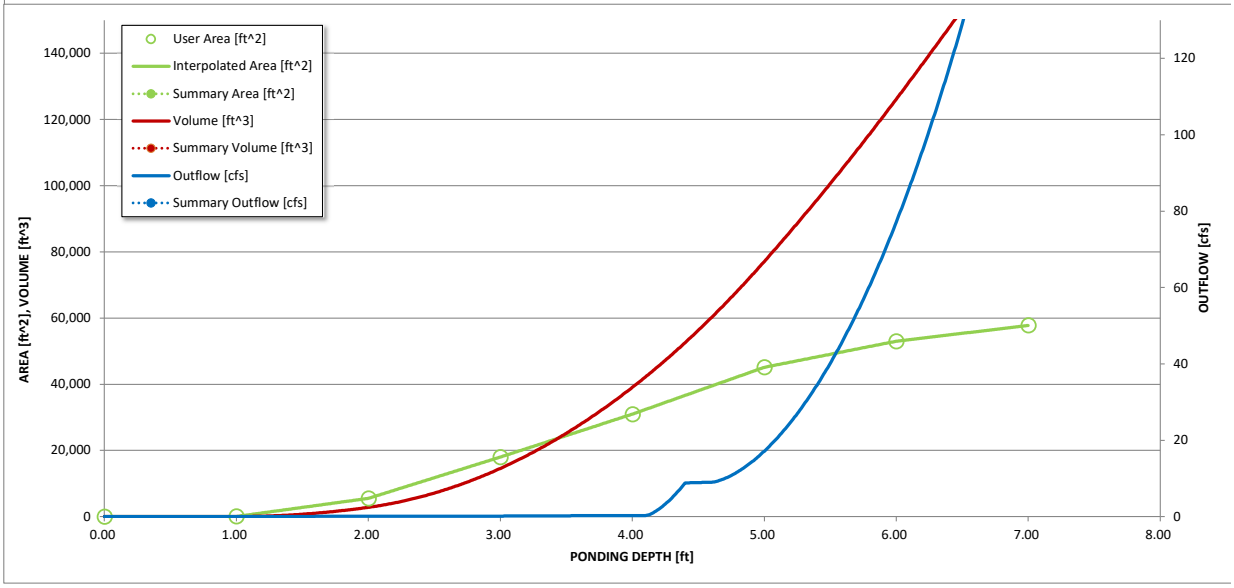
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.14
CUHP Runoff Volume (acre-ft) =	0.298	0.890	0.675	0.905	1.089	1.420	1.743	2.164	3.074
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.675	0.905	1.089	1.420	1.743	2.164	3.074
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.1	0.2	0.3	2.7	5.4	9.1	16.6
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.01	0.01	0.02	0.14	0.28	0.47	0.86
Peak Inflow Q (cfs) =	N/A	N/A	7.2	9.7	11.8	17.2	21.9	27.6	39.4
Peak Outflow Q (cfs) =	0.1	0.3	0.2	0.3	0.9	4.5	7.7	9.0	16.8
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.3	2.9	1.6	1.4	1.0	1.0
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.1	0.4	0.6	0.8	0.8
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	68	61	70	75	73	72	70	66
Time to Drain 99% of Inflow Volume (hours) =	**40**	72	64	73	79	78	78	77	75
Maximum Ponding Depth (ft) =	2.91	4.00	3.59	3.94	4.15	4.29	4.37	4.59	4.99
Area at Maximum Ponding Depth (acres) =	0.39	0.71	0.59	0.69	0.76	0.80	0.83	0.90	1.03
Maximum Volume Stored (acre-ft) =	0.298	0.897	0.624	0.848	1.007	1.108	1.182	1.373	1.749

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

APPENDIX E – REFERENCES

HRGreen

HRGREEN.COM

Grandview Reserve
Master Development Drainage Plan

August 2021

HR Green Project No: 191850

Prepared For:

4 SITE INVESTMENTS, LLC

Mr. Peter Martz or Paul Howard

1271 Kelly Johnson Blvd., Ste. 100

Colorado Springs, CO 80920

719-499-8416

Prepared By:

HR Green Development, LLC

Contact: Chris McFarland, PE

cmcfarland@hrgreen.com

720-602-4956

Subbasin Description

The entire site drains in a south easterly direction and is divided into 8 major drainage basins and a total of 18 subbasins together as described below.

- Subbasin A1 is located in the southwestern corner of the site, to the south and west of MS. The basin drains towards the southeast to proposed detention pond A. Current planning documents call for medium density dwelling units and a small pocket park. The basin is 37.00 acres, with a composite impervious value of 35.22% and runoff rates for the 5 and 100 year of 30.72 cfs and 100.64 cfs respectively. The pond will discharge at predevelopment rates and into MS via the ponds outlet structure.
- Subbasin B1 is located between MS and MST2 to the east of subbasin A1. The basin drains towards the southeast and towards subbasin B2. Current planning documents call for medium density dwelling units and some parkland area. The basin is 37.00 acres, with a composite impervious value of 45.00% and runoff rates for the 5 and 100 year of 29.46 cfs and 97.08 cfs respectively.
- Subbasin B2 is located between MS and MST2 to the northeast of subbasin A1. The basin drains towards the southeast and towards Detention Pond B. Current planning documents call for medium density dwelling units. The basin is 24.89 acres, with a composite impervious value of 43.26% and runoff rates for the 5 and 100 year of 12.02 cfs and 42.26 cfs respectively.
- Subbasin B3 is located between MS and EF and to the northeast of east of basin B2. The existing MST2 tributary runs through the basin. The site drains towards the southeast and towards Detention Pond B. Current planning documents call for high, medium-high, and medium density dwelling units along with a pocket park. The basin is 118.90 acres, with a composite impervious value of 49.42% and runoff rates for the 5 and 100 year of 92.76 cfs and 295.27 cfs respectively.
- Subbasin C1 is located to the northeast of east of basin B1 and the existing MST2 tributary runs through the middle of the basin. The basin drains towards the southeast and towards Detention Pond C. Current planning documents call for an institutional parcel, medium and high density dwelling units and a pocket park. The basin is 77.83 acres, with a composite impervious value of 51.20% and runoff rates for the 5 and 100 year of 77.99 cfs and 238.03 cfs respectively.
- Subbasin D1 is located between MS and MST2 to the east of Basin B3 and adjacent to the MST2 channel. The basin drains towards the southeast and towards drainage basin D2. Current planning documents call for medium density dwelling units along with a pocket park. The basin is 24.33 acres, with a composite impervious value of 53.89% and runoff rates for the 5 and 100 year of 24.15 cfs and 70.07 cfs respectively.
- Subbasin D2 is located between MS and MST2 to the south of basins D1 and B3. The basin drains towards the southwest and towards detention pond D. Current planning documents call for high density dwelling units along with a pocket park and a commercial parcel. The basin is 77.90 acres, with a composite impervious value of 62.10% and runoff rates for the 5 and 100 year of 98.47 cfs and 252.18 cfs respectively.
- Subbasin E1 is located just east of EFT along the northern portion of the site. The basin drains towards the southeast and towards basins F3 and F4. Current planning documents call for low

These basins along with the offsite basins which lie east of Eastonville Road contribute flows onto the site through the major tributaries. Estimate oncoming flows for each tributary are as follows:

Offsite Flow Summary		
Tributary	**5 Year Peak Runoff (cfs)**	**100 Year Peak Runoff (cfs)**
Main Stem	36	628
Main Stem Tributary 2	8	122
East Fork Tributary*	56	116
East Fork*	175	357

*Flows from Gieck Ranch DBPS, Oct 2010

As hydraulic analysis continues for the channels, these offsite flows will be used to size the channels for proper conveyance of the flow however it should be noted that the flows mentioned for the Main Stem and Main Stem Tributary 2 assume proper conveyance of the flow through (below or above) Eastonville Road. Due to the unknown nature of these conditions at the time of buildout, a probable scenario of the split flows will require analysis and agreed upon flow rates to each channel will be required. Currently some of the flow shown going to the Main Stem Tributary 2 may be diverted into the Main Stem. Previous analysis done by JR Engineering assumed approximately 160 additional cfs going to the Main Stem Tributary #2 during the 100-year event and as such it is recommended the following flows be used for analysis of the oncoming offsite flows:

Revised Offsite Flow Summary		
Tributary	**5 Year Peak Runoff (cfs)**	**100 Year Peak Runoff (cfs)**
Main Stem**	67	413
Main Stem Tributary 2**	59	280
East Fork Tributary*	61	217
East Fork*	180	595

*Flows from Gieck Ranch DBPS, Oct 2010

**Flows from 4 Way Ranch LOMR, Mar 2004

Please note that the preliminary drainage reports will be required to reconcile any differences between the various reports done for these channels.

b. Methodology

Design rainfall was determined utilizing figures from the NOAA Atlas 14, Volume 8, Version 2 to determine the 5-year and 100-year rainfall values for 1, 6 and 24-hour events. The 1-hour rainfall depths are 1.22 and 2.50 in/hr respectively, 6 hour 1.79 and 3.87 in/hr respectively and 2.36 and 4.90 in/hr for the 24 hour event. The rainfall values were then used as inputs into the Colorado Urban Hydrograph Procedure (CUHP) spreadsheets to determine runoff values for both pre-development and post-development site.

CUHP is an evolution of the Snyder unit hydrograph and is calibrated for use along the Colorado Front Range. 1 Hour rainfall amounts are input into the program to produce a storm hyetograph that is then uses to calculate a storm hydrograph for each basin depending on the subbasins properties including

slope, length, shape, impervious area, pervious depression storage area, and various infiltration rates. Tabular hydrographs are then computed and can be used in EPA SWMM. The CUHP results are included within Appendix B.

EPA SWMM was used to determine flow routing via the kinematic wave method. Subbasins were routed to their respective design points and detention ponds for both the developed and predeveloped condition to determine peak runoff amounts for the 5-year and 100-year storm events. Information from these models along with information and calculations performed in the Colorado Springs BMP spreadsheets was used to determine pond sizing calculations and release rates.

c. Basin Hydrology

A summary of the flows for both the predeveloped and developed cases for each basin, subbasin and Pond are found on next page along with the full computation found in Appendix B.

SWMM Basin and Pond Summary						
Basin Description	**Basin Area (ac)**	**% Impervious**	**5 Year Peak Runoff (cfs)**	**100 Year Peak Runoff (cfs)**	**5 Year Pond Volume (ac-ft)**	**100 Year Pond Volume (ac-ft)**
A1	45.38	35.22%	30.72	100.64		
			Pond A		1.83	3.50
B1	37.00	45.00%	29.46	97.08		
B2	24.89	43.26%	12.02	42.26		
B3	118.90	49.42%	92.76	295.27		
			Pond B		5.90	19.00
C1	77.83	51.20%	77.99	238.03		
			Pond C		3.91	6.87
D1	24.33	44.14%	24.15	70.07		
D2	77.90	62.10%	98.47	252.18		
			Pond D		6.61	10.19
E1	88.60	19.54%	46.88	178.04		
			Pond E		1.96	2.44
F1	33.73	25.00%	16.28	58.95		
F2	67.64	51.39%	60.11	170.90		
F3	12.84	45.00%	11.36	32.93		
F4	51.81	46.54%	42.32	124.89		
			Pond F		7.38	12.62
G1	20.13	36.52%	13.78	43.95		
G2	15.14	25.00%	6.55	23.95		
			Pond G		0.72	2.03
H1	20.71	24.49%	5.68	27.62		
H2	18.55	43.68%	16.24	47.62		
H3	6.01	40.57%	5.21	15.60		
H4	27.65	38.24%	20.93	64.71		
			Pond H		2.93	6.17

IV. Hydraulic Analysis

a. Major Drainageways

In general, the site runoff flows towards the 4 major drainageways and in a southeasterly direction. These basins are described in more detail below:

Main Stem

The Main Stem (MS) is in the southwestern portion of the site. Offsite flows collect and are conveyed under Eastonville Road via a culvert. MS travels in a southeasterly direction and combines with the Main Stem Tributary #2 (MST2) just off site where it is then conveyed past Highway 24 via a culvert. An existing breached stock pond exists in the approximate center point of the channel within the site. Jurisdictional wetlands exist within this channel and the area is within a Zone A floodplain towards the southern portion of the site. This channel sees only intermittent flows at this time however once development occurs there may be a more constant baseflow.

Proposed improvements for MS include the removal of the breached stock pond berm and regrading of the affected stretch of channel to restore its historic state. Proposed flow rates through MS are not to exceed historic flowrates and as such, the remainder of the channel is to remain in its current state sans any preemptive check structures; modeling indicates the channel shall remain stable despite the removal of the existing berm.

Main Stem Tributary #2

MST2 crosses Eastonville road via an existing culvert and flows through the site in a southeasterly direction. Portions of this channel are within a mapped floodplain as shown in the existing FIRM Panel. Per a July email from the USACE this drainage channel was preliminarily determined to be a non-jurisdictional waters/wetland.

Proposed improvements for MST2 include the realignment of the channel, generally shifting the channel towards the west to accommodate the proposed land plan. There is to be a dedicated 100’ corridor in which the valley will meander. Preliminary analysis indicates the valley will have an average width of approximately 63’ at the elevation necessary to meet freeboard requirements; initial sizing approximates the bankfull width to be 6.8’. The valley and channel thalweg will generally follow the same profile, with some deviation as the bankfull channel meanders through the valley in turn decreasing the low flow channels average slope. The average valley profile is to be approximately 1% with a series of grade control structures to both decrease elevation and dissipate energy to meet natural channel criteria as outline in El Paso County criteria and agreed upon channel parameters.

East Fork Tributary

The East Fork tributary (EFT) crosses the north property line and flows are conveyed through the site via a natural channel. The channel has been mapped as a Zone A floodplain per the existing FIRM panel; it appears any hydraulic effects of the crossing at Eastonville Road was not accounted for in the floodplain delineation. While the current floodplain delineation shows the channel continuing through Highway 24, there is no existing crossing for this section of the drainage channel below Highway 24 and instead the flows are conveyed to the northeast towards the East Fork Upper (EF). Per a July email from the USACE this drainage channel was preliminarily determined to be a non-jurisdictional waters/wetland.

Per SWMM modeling the current velocities will require channel stabilization. The EFT channel is to be engineered later in the design which will likely include a combination of channel widening, lowering of slope facilitated by the implementation of drop structures to meet non erosive velocity requirements. Bank stabilization, should it be necessary, may include coir rolls, erosion control blankets, live willow staking, soil lifts and/or other measures to ensure successful bank stabilization. The drainageway will require further analysis and design which will be completed as the project progresses.

East Fork Upper

The EF crosses the north property line approximately 1500' east of the EFT crossing. The flow through the site is via a natural channel and travels in a southeasterly direction. The channel is mapped as a Zone A floodplain, and the channel crosses Highway 24 via an existing shallow bridge. The current floodplain delineation shows EF and EFT eventually merging approximately 1750' southeast of the site, however, as mentioned above Highway 24 blocks the flow of the EFT and flows are conveyed northeast to the EF bridge crossing.

Per SWMM modeling the current velocities will require channel stabilization. The EF channel is to be engineered later in the design which will likely include a combination of channel widening, lowering of slope facilitated by the implementation of drop structures to meet non erosive velocity requirements. Bank stabilization, should it be necessary, may include coir rolls, erosion control blankets, live willow staking, soil lifts and/or other measures to ensure successful bank stabilization. The drainageway will require further analysis and design which will be completed as the project progresses.

V. Environmental Evaluations

a. Significant existing or potential wetland and riparian areas impacts

As part of this work, the developer has engaged Ecosystem Services, LLC (ECOS) to perform environmental studies of the site that will be submitted with the planning documents. Major information from this report related to the wetlands shows that two of the tributaries through the site, the Main Stem and the East Fork contain jurisdictional wetlands and the other two tributaries, the East Fork Tributary and the Main Stem Tributary #2 are non-jurisdictional wetlands.

At this time, only minor improvements to the jurisdictional channels are proposed. These stream improvements will be made with keeping the natural habitat intact and the natural function of these channels as it is to maintain the wetland habitat. The non-jurisdictional channels will be modified, and the design of those channels is forthcoming.

b. Stormwater quality considerations and proposed practices

As part of the development, full spectrum detention facilities will be installed to provide water quality for the development. The facilities will be designed using El Paso County criteria and provide stormwater quality by slowing the release of stormwater captured by the ponds and allowing solids to settle out. Additionally, when possible, the revised drainage channels, which were not jurisdictional wetlands, will be used to convey stormwater via a natural channel. Stormwater must be treated before entering the natural channels. The natural channel will provide a pervious means to transport stormwater and provide some water quality benefits as well.

On site practices for the homes, schools, churches, and other buildings should use means such that impervious areas drain across pervious area to allow for infiltration during the minor events. This would

Grandview Reserve CLOMR Report

Introduction

This report was prepared by HR Green to support the submission of MT-2 forms and documents in a request for a Conditional Letter of Map Revision (CLOMR) for channel improvements along Geick Ranch Tributary 2. This request impacts the current delineation of the 100-year boundary on Flood Insurance Rate Maps (FIRMs) 08041C0552G and 08041C0556G.

Grandview Reserve is located in Falcon, Colorado within El Paso County and contains approximately 776 acres within the south half of section 21 and 22 and the north half of section 27 and 28, Township 12 South, and Range 66 West of the Sixth Principal Meridian in Ela Paso County, Colorado.

Grandview Reserve (GVR) falls within the Gieck Ranch Drainage Basin which covers approximately 22 square miles. This drainage basin is tributary to Black Squirrel Creek and joins said creek just to the south of Ellicott, CO about 18 miles to the south. Black Squirrel Creek eventually drains to the Arkansas River in Pueblo Colorado. Much of the Gieck Ranch Drainage basin is undeveloped and consists of rural farmland. The Gieck Ranch Drainage basin lies north of the Haegler Ranch drainage basin. The channels through the Grandview property can all be described as gently sloping drainages that roll through the site towards the creeks to which they are tributary.

Per the NRCS web soil survey, the site is made up entirely of Type A and B soils. The majority of which are Type B soils. The vegetation found within Grandview Reserve consists of wetland communities in the floodplain with a transitional area to shortgrass prairie communities that dominate the site. The primary species found in the shortgrass prairie regions include little bluestem, blue grama, and buffalograss. The transitional area between the wetlands and shortgrass prairie includes patches of snowberry, and wood's rose. There are a few plains cottonwoods along the main channels. The area has historically been heavily grazed and there are weeds throughout the site. Weeds found onsite include Canada thistle, Russian thistle, common mullein and yellow toadflax spp.

Observations of the existing channels suggest that they are at equilibrium with their watershed flows; evidence including relatively stable bank full channels, adequate floodplain (above bank full channel elevations) and in-tact plant communities that would be expected in this type of reach support the notion that the reach is in equilibrium.

At present, the preliminary analysis and design of Geick Ranch Tributary 2 (GRT2) has been completed. Proposed improvements for Geick Ranch Tributary 2 include refinement of the existing channel alignment and a stabilizing natural stream design that will allow a more predictable floodplain. There is to be a dedicated 100' wide corridor in which the channel valley will meander. The valley is the area needed to fully contain the 100-year event. Preliminary analysis indicates the valley will have an average width of approximately 63'; initial sizing approximates the bank full width to be 8.8' – 13.8'. The valley and channel thalweg will generally follow the same profile, with some deviation as the bank full channel meanders through the valley in turn decreasing the low flow channels average slope. The average valley profile is to be approximately 0.9% with a series of grade control structures to both decrease elevation and dissipate energy to meet natural channel criteria as outlined in El Paso County criteria.

Hydrology

El Paso County criteria states that all developments are required to detain storm flows down to their historic peaks. For this reason GRT2 has been designed using the flows that drain to it in the existing conditions.

PROPOSED TRIBUTARY 2 UPPER GRADING. SEE SHEETS GR13

EXISTING TRIBUTARY 2 ALIGNMENT

PROPOSED TRIBUTARY 2 ALIGNMENT. SEE SHEETS GR2 - GR13

EXISTING HIGH VOLTAGE OVERHEAD POWER LINES

EXISTING TELEPHONE

EXISTING GAS LINE

EASTONVILLE RD

FUTURE GRANDVIEW RESERVE FILING NO. 4

FILING 4

EXISTING TRIBUTARY 2 ALIGNMENT

PROPOSED TRIBUTARY 2 GRADING. SEE SHEETS GR2 - GR11

FUTURE GRANDVIEW RESERVE FILING NO. 3

FILING 3

FUTURE GRANDVIEW RESERVE FILING NO. 2

FILING 2

EXISTING JURISDICTIONAL WETLANDS

PROPOSED TRIBUTARY 1 GRADING. SEE SHEET GR1

FUTURE GRANDVIEW RESERVE FILING NO. 4

FILING 4

FUTURE GRANDVIEW RESERVE FILING NO. 1

EFFECTIVE 100-YR FLOODPLAIN

EXISTING NON-JURISDICTIONAL WETLANDS

EXISTING JURISDICTIONAL WETLANDS

TRIBUTARY 1 ALIGNMENT

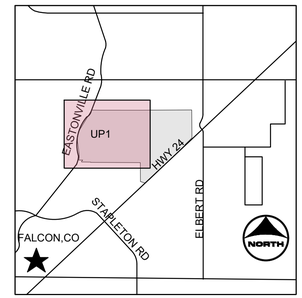
PS INC T12S R64W

S21 S22 S28 S27

1996 PLS30087

NORTH

0 250 125



KEYMAP

PROJECT LEGEND:

- PROPERTY LINE
- EXISTING EASEMENT
- EXISTING GAS LINE
- EXISTING TELEPHONE LINE
- EXISTING FENCE LINE
- EXISTING SECTION LINE
- EXISTING SWALE
- EXISTING OVERHEAD ELECTRIC
- EXISTING TELEPHONE POLE
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- PROPOSED 100-YR FLOODPLAIN
- EXISTING 100-YR FLOODPLAIN
- EXISTING WETLANDS - JURISDICTIONAL
- EXISTING WETLANDS - NONJURISDICTIONAL

NOTES:

1. BASIS OF BEARINGS: THE EAST LINE OF SECTION 21, BEING MONUMENTED AT THE SOUTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, AND BEING MONUMENTED AT THE NORTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, BEING ASSUMED TO BEAR NORTH 00 DEGREES 52 MINUTES 26 SECONDS WEST, A DISTANCE OF 5290.17 FEET.
2. BENCHMARK:
   DESIGNATION = F 24
   PID = JK0240
   DESCRIPTION = DISK ON TOP OF CONCRETE MONUMENT
   CONTROL POINT COORDINATE SYSTEM: NAVD88
   NORTHING: 1421049.80
   EASTING: 3273631.55
   ELEVATION: 6866.33

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. FAY 37081 PROFESSIONAL ENGINEER 5/22/24

DRAWN BY:	TBI	JOB DATE:	5/22/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
CAD DATE:	3/8/2024		
CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\OUP		

BAR IS ONE INCH ON OFFICIAL DRAWINGS. 0 1" IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

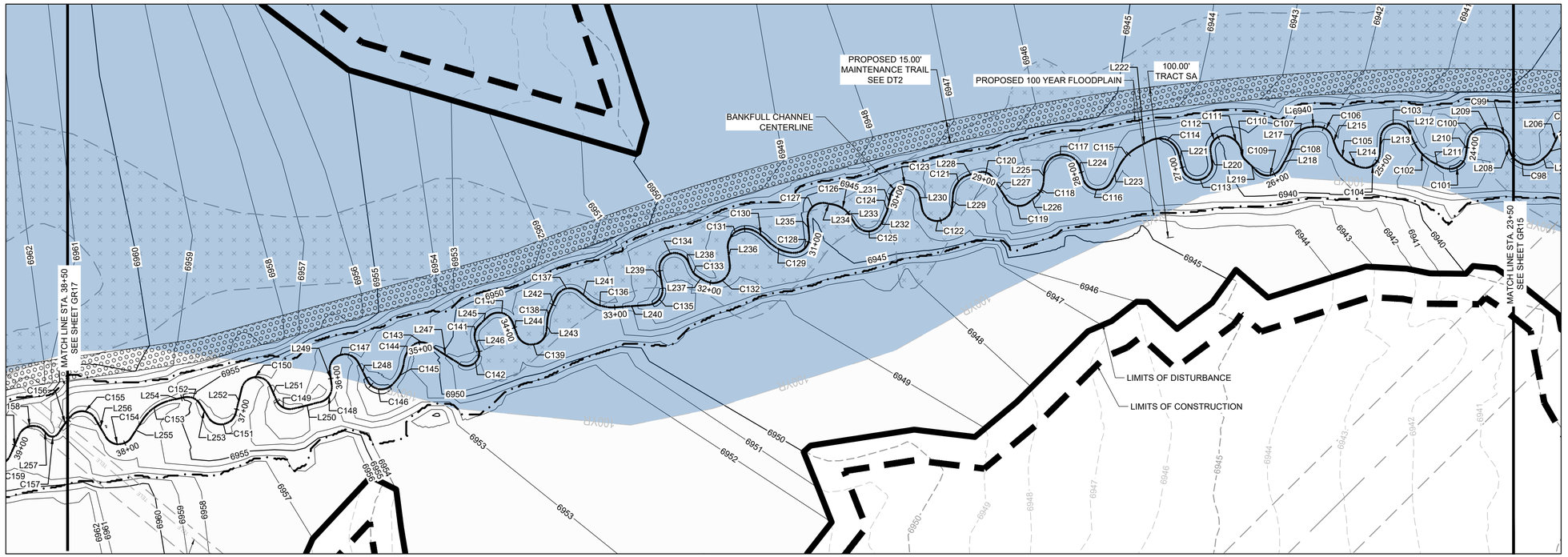
HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

CONSTRUCTION DOCUMENTS
OVERALL UTILITY PLAN

SHEET UP1 04

HR GREEN Xrefs: xgf-1-dh01; OUP KEY: xv-dsgn_662; xv-util_662; xv-dsgn-F1-662.10; xv-cnw-F1-662.10; xv-dsgn-F2; xv-cnw-F2; xv-dsgn-F3; xv-cnw-F3; xv-dsgn-F4; xv-cnw-F4; xv-channel; xv-filing-bnd

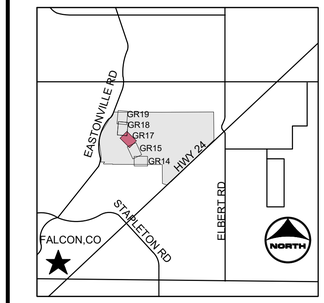


TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
L208	1.73		N41° 53' 24.03"E	
C99	17.18	11.42	N1° 13' 02.63"E	15.61
L209	6.08		N41° 50' 45.40"W	
C100	14.93	10.00	N84° 36' 57.43"W	13.58
L210	8.40		S52° 36' 50.54"W	
C101	21.26	11.00	N72° 00' 20.89"W	18.10
L211	6.02		N16° 37' 32.32"W	
C102	19.50	30.00	N1° 59' 34.48"E	19.16
L212	6.39		N20° 36' 40.80"E	
C103	26.57	10.00	N55° 30' 50.85"W	19.42
L213	10.38		S48° 21' 37.51"W	
C104	19.05	10.00	N77° 03' 24.45"W	16.30
L214	4.49		N22° 28' 26.42"W	
C105	16.30	20.00	N0° 52' 04.01"E	15.85
L215	7.35		N24° 12' 34.43"E	
C106	12.68	10.00	N12° 07' 40.58"W	11.85
L216	4.63		N48° 27' 55.60"W	
C107	14.10	12.00	N82° 08' 01.06"W	13.31
L217	7.48		S64° 11' 53.48"W	
C108	5.01	30.06	S68° 58' 34.98"W	5.00
L218	8.03		S73° 31' 44.52"W	
C109	18.41	10.00	N53° 43' 58.80"W	15.92
L219	10.16		N0° 59' 42.13"W	
C110	6.71	8.87	N15° 30' 59.41"E	6.55
C111	15.56	12.49	N1° 33' 53.25"E	14.58
C112	17.28	9.02	S88° 33' 46.47"W	14.76
L220	6.95		S36° 36' 49.17"W	
C113	30.29	10.00	N56° 37' 28.74"W	19.97
L221	10.63		N30° 08' 14.43"E	

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
C114	23.32	13.00	N21° 15' 09.13"W	20.32
L222	8.89		N72° 38' 32.69"W	
C115	18.57	30.00	S89° 37' 21.45"W	18.28
L223	10.97		S71° 53' 15.59"W	
C116	23.85	10.00	N39° 46' 52.01"W	18.59
L224	7.78		N28° 32' 58.51"E	
C117	33.50	12.00	N51° 25' 50.20"W	23.63
L225	6.48		S48° 35' 21.08"W	
C118	11.32	14.00	S71° 45' 22.12"W	11.02
L226	6.19		N85° 04' 36.85"W	
C119	16.12	10.01	N38° 54' 34.31"W	14.43
L227	18.94		N11° 39' 57.18"E	
C120	8.87	7.66	N28° 37' 10.59"W	8.39
L228	5.88		N57° 56' 20.06"W	
C121	19.07	15.00	S85° 38' 25.55"W	17.81
L229	7.91		S49° 13' 09.23"W	
C122	28.76	10.00	N48° 23' 34.94"W	19.82
L230	7.92		N33° 59' 40.88"E	
C123	27.69	10.00	N45° 19' 19.11"W	19.65
L231	6.44		S55° 21' 40.89"W	
C124	5.15	20.00	S62° 44' 22.73"W	5.14
L232	6.17		S70° 07' 03.81"W	
C125	21.42	10.00	N48° 30' 32.07"W	17.56
L233	6.32		N12° 51' 50.87"E	
C126	14.36	17.00	N11° 20' 32.97"W	13.94
L234	4.96		N35° 32' 56.81"W	
C127	18.28	10.00	N87° 55' 40.41"W	15.84
L235	11.09		S39° 41' 37.22"W	
C128	18.49	10.13	N87° 52' 25.26"W	16.03

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
C129	17.56	25.25	N18° 38' 53.36"W	17.21
C130	12.52	34.40	N9° 02' 14.74"W	12.45
C131	24.08	11.98	N80° 39' 48.26"W	20.23
L236	10.86		S38° 07' 07.02"W	
C132	24.70	11.00	N77° 33' 02.36"W	19.83
L237	5.56		N13° 13' 11.74"W	
C133	6.28	10.00	N4° 46' 29.57"E	6.18
L238	4.58		N22° 46' 10.88"E	
C134	29.21	10.00	N60° 55' 11.65"W	19.88
L239	11.08		S35° 23' 25.82"W	
C135	16.69	10.00	S83° 12' 10.72"W	14.82
L240	22.46		N48° 59' 05.72"W	
C136	19.46	25.00	N26° 41' 15.43"W	18.97
L241	6.51		N4° 23' 25.15"W	
C137	18.49	10.00	N57° 22' 01.09"W	15.97
L242	9.62		S69° 39' 22.97"W	
C138	6.95	20.00	S59° 42' 29.37"W	6.91
L243	6.80		S49° 45' 35.79"W	
C139	26.96	10.00	N52° 59' 53.73"W	19.51
L244	8.77		N24° 14' 35.42"E	
C140	19.62	10.00	N31° 58' 39.54"W	16.62
L245	7.77		N88° 11' 54.49"W	
C141	11.68	10.00	S58° 20' 24.97"W	11.03
L246	4.56		S24° 52' 44.43"W	
C142	20.88	8.44	S89° 22' 23.65"W	15.95
L247	24.22		N12° 38' 45.62"W	
C143	11.75	8.95	N44° 25' 39.16"W	10.92
C144	14.21	17.19	S72° 27' 21.39"W	13.81
C145	17.07	24.50	S74° 40' 14.78"W	16.72

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
C146	18.13	10.00	N34° 01' 03.83"W	15.75
L248	13.71		N17° 55' 53.54"E	
C147	28.09	10.00	N62° 32' 31.87"W	19.72
L249	9.08		S36° 59' 02.72"W	
C148	14.66	10.00	S78° 58' 38.11"W	13.38
L250	17.98		N59° 01' 48.57"W	
C149	14.28	10.00	N18° 08' 03.26"W	13.10
L251	9.68		N21° 32' 16.30"E	
C150	24.93	10.00	N49° 52' 37.35"W	18.96
L252	16.41		S58° 42' 29.00"W	
C151	23.51	10.00	N53° 56' 30.21"W	18.46
L253	10.05		N13° 24' 30.57"E	
C152	12.41	10.00	N22° 09' 24.69"W	11.63
L254	9.00		N57° 43' 19.96"W	
C153	24.24	30.00	N80° 52' 06.53"W	23.58
L255	13.93		S77° 44' 49.86"W	
C154	19.03	10.00	N39° 33' 47.95"W	16.29
L256	13.71		N14° 57' 35.33"E	
C155	16.88	10.00	N33° 23' 36.03"W	14.95



KEYMAP

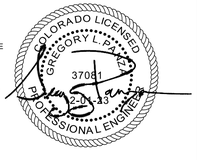
PROJECT LEGEND:

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
- SECTION LINE
- EXISTING EASEMENT
- EXISTING FENCE
- 5250 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- 5250 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- CONSTRUCTION EASEMENT
- FLOW ARROW
- LIMITS OF CONSTRUCTION
- LIMITS OF DISTURBANCE
- PROPOSED 100-YR FLOODPLAIN
- EFFECTIVE 100-YR FLOODPLAIN
- EXISTING WETLANDS - JURISDICTIONAL
- EXISTING WETLANDS - NONJURISDICTIONAL
- PROPOSED MAINTENANCE TRAIL

NOTES:

1. BASIS OF BEARINGS: THE EAST LINE OF SECTION 21, BEING MONUMENTED AT THE SOUTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, AND BEING MONUMENTED AT THE NORTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, BEING ASSUMED TO BEAR NORTH 00 DEGREES 52 MINUTES 26 SECONDS WEST, A DISTANCE OF 5290.17 FEET.
2. BENCHMARK:
   DESIGNATION = F 24
   PID = JK0240
   DESCRIPTION = DISK ON TOP OF CONCRETE MONUMENT
   CONTROL POINT COORDINATE SYSTEM: NAVD88
   NORTHING: 1421049.80
   EASTING: 3273631.55
   ELEVATION: 6866.33

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado



DRAWN BY:	ACH	JOB DATE:	11/29/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
CAD DATE:	11/30/2023		
CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\GRADING		

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NO.	DATE	BY	REVISION DESCRIPTION

HRGreen

HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

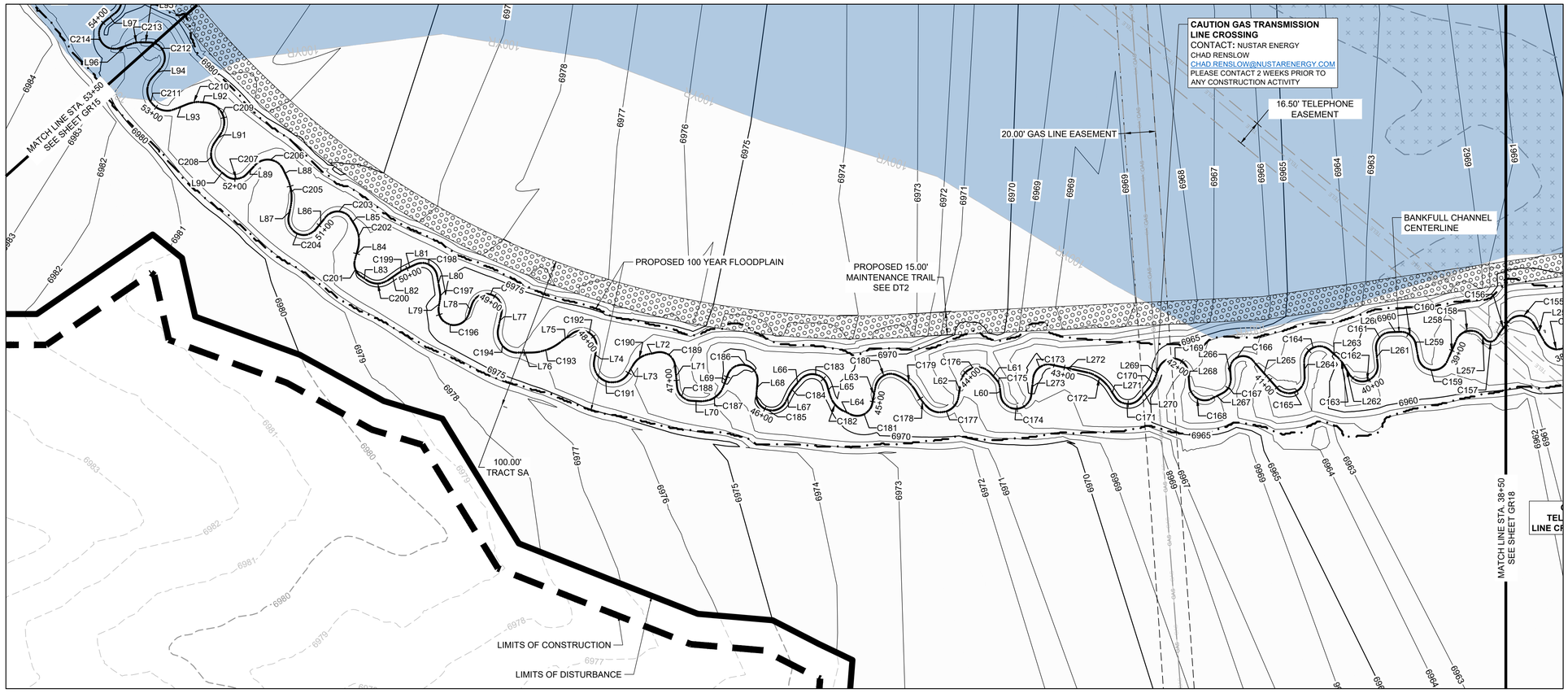
CONSTRUCTION DOCUMENTS

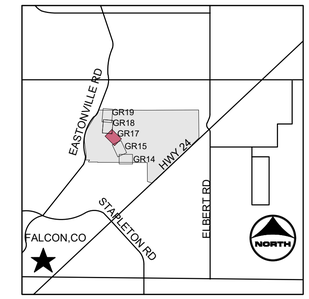
TRIBUTARY 2 BANKFULL GRADING

SHEET GR16

20

HR GREEN Xrefs: xg1-dtl01; GRADING KEY: HRG1 20X_EBase; xv-dsgn_662; xc-dsgn-662.10; xc-dsgn-F1-662.10; xc-row-F1-662.10; xc-row-662.10; xc-dsgn-F2; xc-row-F2; xv-util_662; xv-row-662; xc-dsgn-F3; xc-row-F3; xc-dsgn-F4; 01-XC-INITIAL-EC; xc-row-F4; xc-dsgn-PH2; xc-dsgn-PH3; 01-XC-channel; (01_XV-Row; 01-XC-PR_100YR_FP_DELINEATION





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   CONTROL POINT COORDINATE SYSTEM: NAVD88
   NORTHING: 1421049.80
   EASTING: 3273631.55
   ELEVATION: 6866.33

TRIBUTARY 2 UPSTREAM

LINE # / CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
C156	15.76	60.71	S82° 15' 15.94"W	15.72
C157	9.19	4.73	N48° 26' 22.95"W	7.81
L257	3.58		N3° 04' 06.31"E	
C158	19.37	10.00	N58° 30' 53.78"W	16.48
L258	14.86		S68° 36' 16.40"W	
C159	24.92	10.00	N36° 46' 43.51"W	18.95
L259	8.54		N34° 36' 24.38"E	
C160	14.25	10.00	N6° 12' 38.24"W	13.07
L260	6.03		N47° 01' 39.20"W	
C161	16.43	10.00	S85° 53' 35.89"W	14.65
L261	9.89		S38° 48' 50.97"W	
C162	27.21	10.00	N63° 13' 46.48"W	19.56
L262	6.59		N14° 43' 35.02"E	
C163	6.02	20.00	N6° 06' 03.29"E	6.00
L263	7.20		N2° 31' 29.50"W	
C164	23.46	10.00	N69° 43' 47.95"W	18.44
L264	9.75		S43° 03' 53.60"W	
C165	25.51	10.26	N64° 27' 12.24"W	19.43
L265	24.22		N3° 28' 47.39"E	
C166	20.88	8.44	N74° 30' 03.34"W	15.95
L266	4.56		S41° 00' 17.45"W	
C167	11.68	10.00	S74° 27' 57.98"W	11.03

TRIBUTARY 2 UPSTREAM

LINE # / CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
L267	7.77		N72° 04' 21.48"W	
C168	19.62	10.00	N15° 51' 06.52"W	16.62
L268	8.77		N40° 22' 08.43"E	
C169	26.96	10.00	N36° 52' 20.72"W	19.51
L269	6.80		S65° 53' 08.80"W	
C170	6.95	20.00	S75° 50' 02.38"W	6.91
L270	9.62		S85° 46' 55.98"W	
C171	18.49	10.00	N41° 14' 28.08"W	15.97
L271	6.51		N11° 44' 07.86"E	
C172	19.46	25.00	N10° 33' 42.42"W	18.97
L272	22.46		N32° 51' 32.70"W	
C173	16.69	10.00	N80° 40' 16.27"W	14.82
L273	11.08		S51° 30' 58.83"W	
C174	29.21	10.00	N44° 47' 38.64"W	19.88
L60	4.58		N38° 53' 43.89"E	
C175	6.28	10.00	N20° 54' 02.58"E	6.18
L61	5.56		N2° 54' 21.27"E	
C176	24.70	11.00	N61° 25' 29.35"W	19.83
L62	10.86		S54° 14' 40.03"W	
C177	24.08	11.98	N64° 32' 15.25"W	20.23
C178	12.52	34.40	N7° 05' 18.27"E	12.45
C179	17.56	25.25	N2° 31' 20.35"W	17.21

TRIBUTARY 2 UPSTREAM

LINE # / CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
C180	18.49	10.13	N71° 44' 52.25"W	16.03
L63	11.09		S55° 49' 10.23"W	
C181	18.28	10.00	N71° 48' 07.40"W	15.84
L64	4.96		N19° 25' 23.80"W	
C182	14.36	17.00	N4° 47' 00.04"E	13.94
L65	6.32		N28° 59' 23.88"E	
C183	21.42	10.00	N32° 22' 59.06"W	17.56
L66	6.17		S86° 14' 36.82"W	
C184	5.15	20.00	S78° 51' 55.74"W	5.14
L67	6.44		S71° 29' 13.91"W	
C185	27.69	10.00	N29° 11' 46.10"W	19.65
L68	7.92		N50° 07' 13.89"E	
C186	28.76	10.00	N32° 16' 01.93"W	19.82
L69	7.91		S65° 20' 42.25"W	
C187	19.07	15.00	N78° 14' 01.44"W	17.81
L70	5.88		N41° 48' 47.05"W	
C188	14.23	10.00	N1° 03' 38.55"W	13.06
L71	13.81		N39° 41' 29.96"E	
C189	16.83	10.01	N10° 00' 57.74"W	14.91
L72	6.19		N58° 12' 37.69"W	
C190	11.32	14.00	N81° 22' 38.72"W	11.02
L73	6.48		S75° 27' 20.25"W	

TRIBUTARY 2 UPSTREAM

LINE # / CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
C191	33.50	12.00	N24° 33' 51.04"W	23.63
L74	7.78		N55° 24' 57.68"E	
C192	23.85	10.00	N12° 54' 52.85"W	18.59
L75	10.97		N81° 14' 45.25"W	
C193	18.57	30.00	N63° 30' 39.38"W	18.28
L76	8.89		N45° 46' 33.52"W	
C194	23.32	13.00	N5° 36' 50.04"E	20.32
L77	10.63		N57° 00' 13.60"E	
C195	30.29	10.00	N29° 45' 29.57"W	19.97
L78	6.95		S63° 28' 48.34"W	
C196	29.92	10.00	N30° 47' 54.08"W	19.94
L79	4.15		N54° 55' 22.26"E	
C197	6.25	18.00	N44° 58' 12.97"E	6.22
L80	10.16		N35° 01' 04.53"E	
C198	18.41	10.00	N17° 43' 12.15"W	15.92
L81	8.03		N70° 27' 28.82"W	
C199	5.01	30.06	N75° 00' 38.36"W	5.00
L82	7.48		N79° 47' 19.86"W	
C200	14.10	12.00	N46° 07' 14.40"W	13.31
L83	4.63		N12° 27' 08.94"W	
C201	12.68	10.00	N23° 53' 06.07"E	11.85
L84	7.35		N60° 13' 21.09"E	

TRIBUTARY 2 UPSTREAM

LINE # / CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
C202	16.30	20.00	N36° 52' 50.66"E	15.85
L85	4.49		N13° 32' 20.24"E	
C203	19.05	10.00	N41° 02' 37.80"W	16.30
L86	10.38		S84° 22' 24.17"W	
C204	26.57	10.00	N19° 30' 04.19"W	19.42
L87	6.39		N56° 37' 27.46"E	
C205	19.50	30.00	N38° 00' 21.14"E	19.16
L88	6.02		N19° 23' 14.34"E	
C206	21.26	11.00	N35° 59' 34.23"W	18.10
L89	8.40		S88° 37' 37.20"W	
C207	14.93	10.00	N48° 36' 10.78"W	13.58
L90	6.08		N5° 49' 58.75"W	
C208	19.45	13.00	N37° 02' 16.06"E	17.69
L91	6.84		N79° 54' 30.86"E	
C209	27.37	13.00	N19° 35' 19.50"E	22.59
L92	3.27		N40° 43' 51.79"W	
C210	9.28	20.00	N54° 01' 48.85"W	9.20
L93	8.27		N67° 19' 45.91"W	
C211	35.19	13.00	N10° 12' 28.14"E	25.39
L94	7.78		N87° 44' 40.12"E	
C212	23.02	10.00	N21° 47' 21.49"E	18.26

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. PAYNE 37061 PROFESSIONAL ENGINEER 2-24-23

NORTH 0 40 20

DRAWN BY:	ACH	JOB DATE:	11/29/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
CAD DATE:	11/30/2023		
CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\GRADING		

BAR IS ONE INCH ON OFFICIAL DRAWINGS. 0 1" IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

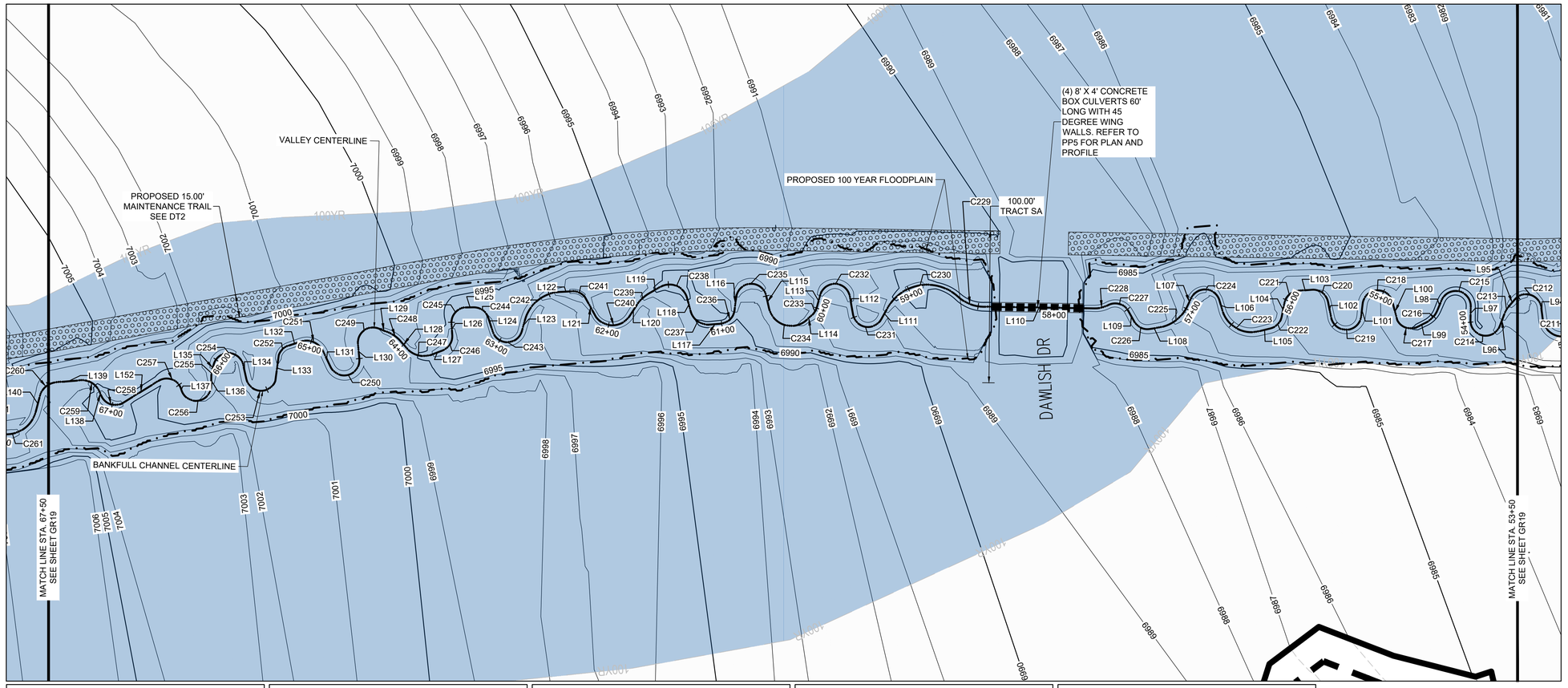
HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

CONSTRUCTION DOCUMENTS
TRIBUTARY 2 BANKFULL GRADING

SHEET GR17 | 21

HR GREEN Xrefs: xgt-t_dt01, GRADING KEY, HRG1.20X_EBase; xv-dsgn_662; xc-dsgn-F3; xc-crow-F3; xc-crow-F4; xc-dsgn-F4; 01-XC-INITIAL-EC; xc-crow-PH2; xc-dsgn-PH2; xc-dsgn-PH3; 01-XC-channel; (1)_XV-Row; 01-XC-PR_100YR_FP_DELINEATION



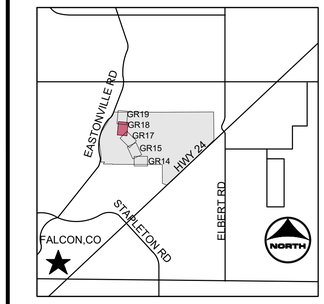
TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
L95	5.17		N44° 09' 57.14"W	
C213	6.98	20.00	N54° 09' 33.54"W	6.94
L96	8.04		N64° 09' 09.93"W	
C214	28.01	10.00	N16° 04' 45.78"E	19.71
L97	9.02		S83° 41' 18.51"E	
C215	27.69	10.00	N16° 58' 58.63"E	19.65
L98	7.09		N62° 20' 44.23"W	
C216	11.33	16.00	N42° 04' 06.40"W	11.09
L99	6.19		N21° 47' 27.14"W	
C217	15.51	10.00	N22° 38' 51.73"E	14.00
L100	11.78		N80° 12' 45.19"E	
C218	28.61	10.00	N3° 46' 45.22"E	19.80
L101	8.42		N78° 10' 24.49"W	
C219	28.24	10.00	N2° 43' 43.88"E	19.75
L102	10.54		N83° 37' 52.24"E	
C220	14.48	10.00	N42° 08' 43.05"E	13.25
L103	9.11		N0° 39' 32.79"E	
C221	12.97	10.00	N36° 30' 06.87"W	12.08
L104	9.22		N73° 39' 45.15"W	
C222	11.31	10.00	N41° 16' 14.49"W	10.71
L105	7.94		N8° 52' 43.84"W	
C223	30.67	20.00	N35° 03' 26.18"E	27.75

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
L106	6.69		N78° 59' 36.19"E	
C224	24.18	10.00	N9° 42' 54.27"E	18.71
L107	5.63		N59° 33' 47.65"W	
C225	24.74	25.02	N32° 09' 09.97"W	23.74
L108	6.03		N3° 48' 42.59"W	
C226	12.18	10.00	N31° 04' 04.22"E	11.44
L109	7.66		N65° 56' 51.04"E	
C227	15.54	10.00	N21° 25' 50.86"E	14.02
C228	9.09	18.00	N8° 36' 52.18"W	9.00
L110	73.49		N5° 51' 24.97"E	
C229	18.19	25.00	N26° 42' 11.88"E	17.79
C230	54.03	30.00	N4° 02' 38.02"W	47.02
L111	8.57		N55° 38' 14.84"W	
C231	24.53	10.00	N14° 38' 23.93"E	18.83
L112	12.12		N84° 55' 01.43"E	
C232	26.99	10.00	N7° 35' 33.85"E	19.51
L113	8.04		N69° 43' 52.36"W	
C233	6.99	12.00	N53° 02' 49.83"W	6.89
L114	4.55		N36° 21' 48.50"W	
C234	31.01	15.00	N22° 51' 23.94"E	25.77
L115	8.57		N82° 04' 36.38"E	
C235	27.86	10.00	N2° 16' 03.64"E	19.68

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
L116	8.98		N77° 32' 29.10"W	
C236	13.59	10.00	N38° 36' 17.81"W	12.57
L117	7.25		N0° 19' 53.48"E	
C237	16.68	12.00	N40° 09' 12.75"E	15.37
L118	8.96		N79° 58' 33.23"E	
C238	21.88	12.00	N27° 44' 22.92"E	18.97
L119	8.16		N24° 29' 46.28"W	
C239	11.91	25.00	N38° 08' 52.11"W	11.80
L120	7.82		N51° 47' 57.21"W	
C240	26.65	13.48	N1° 49' 28.09"E	22.51
L121	15.08		N66° 02' 17.32"E	
C241	9.50	10.00	N29° 12' 46.81"E	9.14
L122	8.34		N2° 00' 28.98"E	
C242	31.18	25.00	N33° 43' 31.88"W	29.20
L123	7.80		N69° 27' 32.74"W	
C243	25.58	10.00	N3° 49' 59.88"E	19.16
L124	9.62		N77° 07' 32.51"E	
C244	12.73	10.00	N40° 39' 52.64"E	11.89
L125	4.77		N4° 12' 10.94"E	
C245	16.25	10.00	N42° 20' 24.85"W	14.52
L126	7.68		N88° 53' 00.63"W	
C246	12.69	10.00	N52° 31' 59.36"W	11.85

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
L127	7.41		N16° 10' 58.09"W	
C247	18.38	15.00	N18° 55' 33.27"E	17.25
L128	6.36		N54° 02' 04.63"E	
C248	9.39	20.00	N40° 35' 03.09"E	9.30
L129	7.15		N27° 08' 01.54"E	
C249	20.51	10.00	N31° 37' 14.85"W	17.10
L130	9.76		S89° 37' 28.76"W	
C250	27.90	10.00	N10° 27' 09.67"W	19.69
L131	11.50		N69° 28' 12.96"E	
C251	17.33	12.00	N28° 06' 25.18"E	15.86
L132	8.67		N13° 15' 22.59"W	
C252	9.13	10.00	N39° 24' 31.73"W	8.82
L133	17.18		N77° 27' 43.71"W	
C253	25.62	10.00	N9° 01' 06.61"E	19.17
L134	9.39		N82° 24' 48.02"E	
C254	26.62	10.00	N6° 09' 26.70"E	19.43
L135	4.72		N70° 05' 54.62"W	
C255	2.40	10.00	N76° 59' 14.73"W	2.40
L136	7.28		N83° 52' 34.84"W	
C256	28.51	10.00	N2° 12' 50.31"W	19.79
L137	0.10		N79° 26' 54.22"E	
C257	18.54	10.00	N26° 20' 13.82"E	16.00

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
L152	27.44		N26° 46' 26.58"W	
C258	16.95	10.00	N21° 46' 36.50"E	14.99
L138	3.84		N70° 19' 39.57"E	
C259	14.65	12.00	N35° 21' 09.72"E	13.76
L139	16.46		N0° 22' 41.14"E	



KEYMAP

PROJECT LEGEND:

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
- SECTION LINE
- EXISTING EASEMENT
- EXISTING FENCE
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- CONSTRUCTION EASEMENT
- FLOW ARROW
- LIMITS OF CONSTRUCTION
- LIMITS OF DISTURBANCE
- PROPOSED 100-YR FLOODPLAIN
- EFFECTIVE 100-YR FLOODPLAIN
- EXISTING WETLANDS - JURISDICTIONAL
- EXISTING WETLANDS - NONJURISDICTIONAL
- PROPOSED MAINTENANCE TRAIL

NOTES:

1. BASIS OF BEARINGS: THE EAST LINE OF SECTION 21, BEING MONUMENTED AT THE SOUTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, AND BEING MONUMENTED AT THE NORTHEAST CORNER BY A 3-1/4" ALUMINUM SURVEYOR'S CAP STAMPED "PS INC PLS 30087 1996", BEING APPROPRIATELY MARKED, BEING ASSUMED TO BEAR NORTH 00 DEGREES 52 MINUTES 26 SECONDS WEST, A DISTANCE OF 5290.17 FEET.
2. BENCHMARK:
   DESIGNATION = F 24
   PID = JK0240
   DESCRIPTION = DISK ON TOP OF CONCRETE MONUMENT
   CONTROL POINT COORDINATE SYSTEM: NAVD88
   NORTHING: 1421049.80
   EASTING: 3273631.55
   ELEVATION: 6866.33

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. PAYNE 37081 PROFESSIONAL ENGINEER 12-01-23

DRAWN BY:	ACH	JOB DATE:	11/29/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
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NO.	DATE	BY	REVISION DESCRIPTION

HRGreen

HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

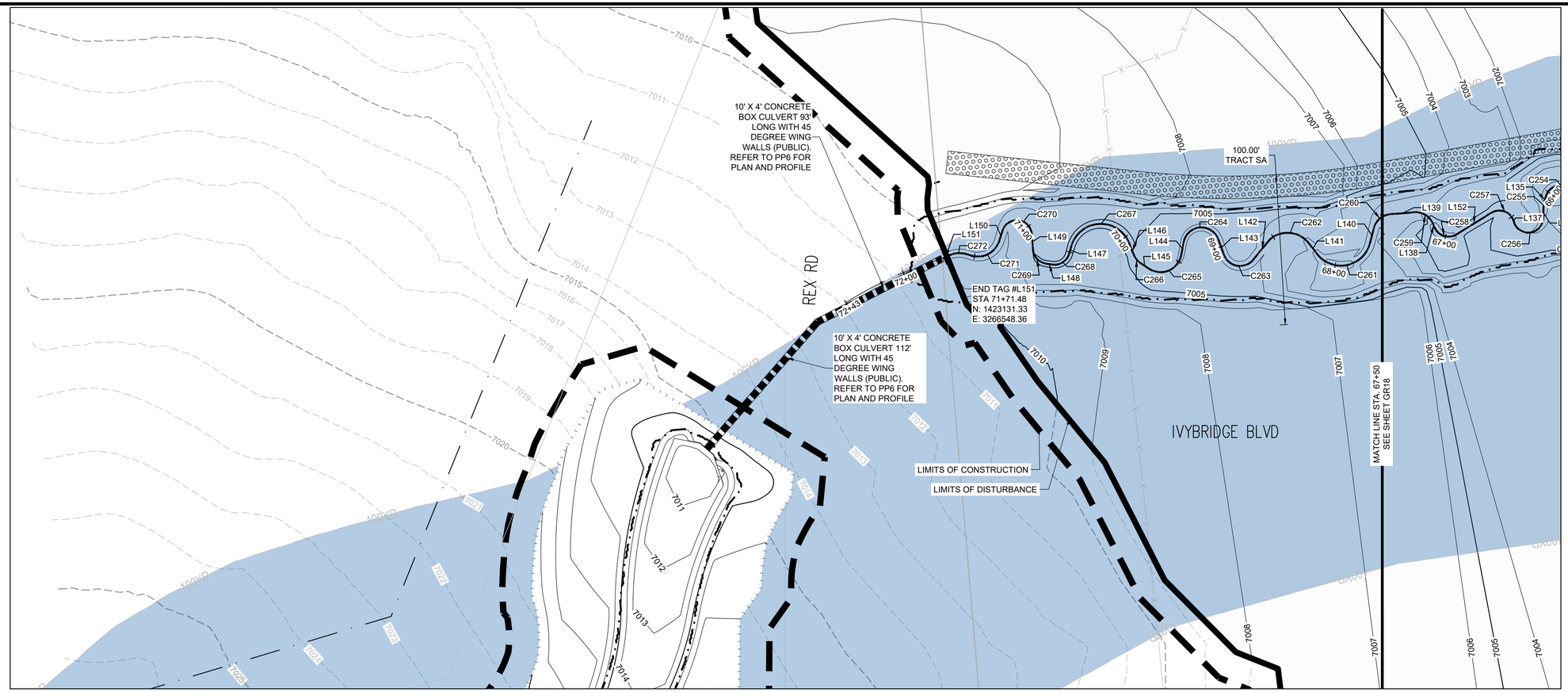
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

CONSTRUCTION DOCUMENTS
TRIBUTARY 2 BANKFULL GRADING

SHEET GR18

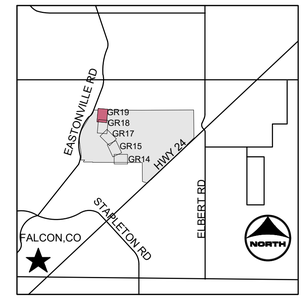
22

HR GREEN Xrefs: xgl-1-dfh01; GRADING KEY: HRG1 20X_EBase: xv-dsgn_662; xc-dsgn-662.10; xc-dsgn-F1-662.10; xc-row-F1-662.10; xc-row-662.10; xc-dsgn-F2; xc-row-F2; xv-util_662; xv-row-662; xc-dsgn-F3; xc-row-F3; xc-row-F4; 01-XC-INITIAL-EC; xc-row-PH2; xc-dsgn-PH2; xc-dsgn-PH3; 01-XC-channel; (1)_XV-Row; 01-XC-PR_100YR_FP_DELINEATION



TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
C260	14.80	12.00	N34° 56' 39.85"W	13.88
L140	10.77		N70° 15' 59.85"W	
C261	44.55	20.00	N6° 27' 33.85"W	35.89
L141	8.98		N57° 20' 52.15"E	
C262	29.24	18.00	N10° 48' 15.97"E	26.13
L142	16.23		N45° 31' 56.12"W	
C263	24.34	11.44	N9° 01' 06.38"E	20.00
L143	11.94		N74° 50' 42.15"E	
C264	31.22	12.00	N0° 18' 44.36"E	23.13
L144	8.82		N74° 13' 13.43"W	
C265	27.28	14.00	N18° 24' 07.71"W	23.16
L145	7.63		N38° 57' 54.40"E	
C266	5.69	10.00	N55° 15' 07.16"E	5.61
L146	6.73		N71° 32' 18.85"E	
C267	53.21	20.40	N5° 38' 58.70"E	39.35
L147	5.41		N68° 39' 59.99"W	
C268	14.28	10.00	N27° 44' 59.82"W	13.10
L148	4.50		N13° 10' 00.36"E	
C269	13.50	10.00	N51° 50' 51.43"E	12.50
L149	10.57		S89° 28' 17.51"E	
C270	27.53	10.00	N11° 39' 24.20"E	19.62

TRIBUTARY 2 UPSTREAM				
LINE #/ CURVE#	LENGTH	RADIUS	LNE/CHORD DIRECTION	CHORD LENGTH
L150	7.19		N67° 12' 54.10"W	
C271	24.50	15.00	N20° 25' 45.98"W	21.86
C272	14.98	18.00	N2° 31' 05.79"E	14.55
L151	3.20		N21° 19' 11.62"W	



KEYMAP

PROJECT LEGEND:

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
- SECTION LINE
- EXISTING EASEMENT
- EXISTING FENCE
- 5250 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- 5250 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
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- FLOW ARROW
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- LIMITS OF DISTURBANCE
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NOTES:

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2. BENCHMARK:
   DESIGNATION = F 24
   PID = JK0240
   DESCRIPTION = DISK ON TOP OF CONCRETE MONUMENT
   CONTROL POINT COORDINATE SYSTEM: NAVD88
   NORTHING: 1421049.80
   EASTING: 3273631.55
   ELEVATION: 6866.33

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. PAZ 37081 12-01-23 PROFESSIONAL ENGINEER

DRAWN BY:	ACH	JOB DATE:	11/29/2023
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HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
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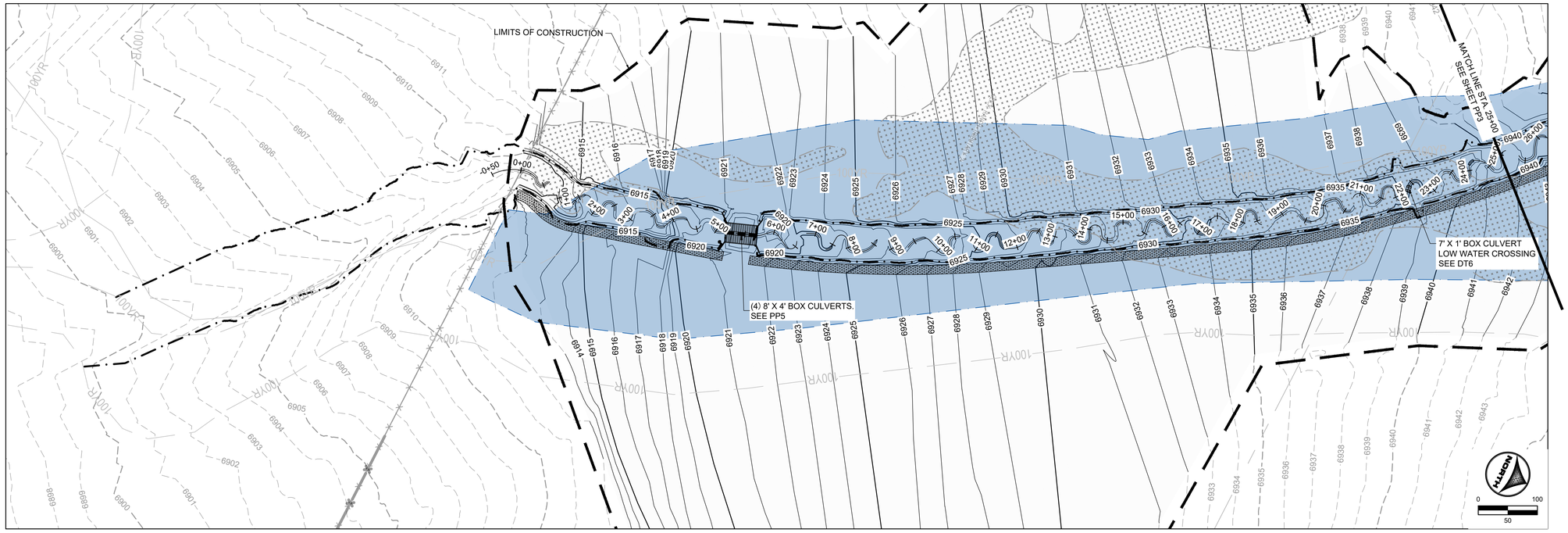
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

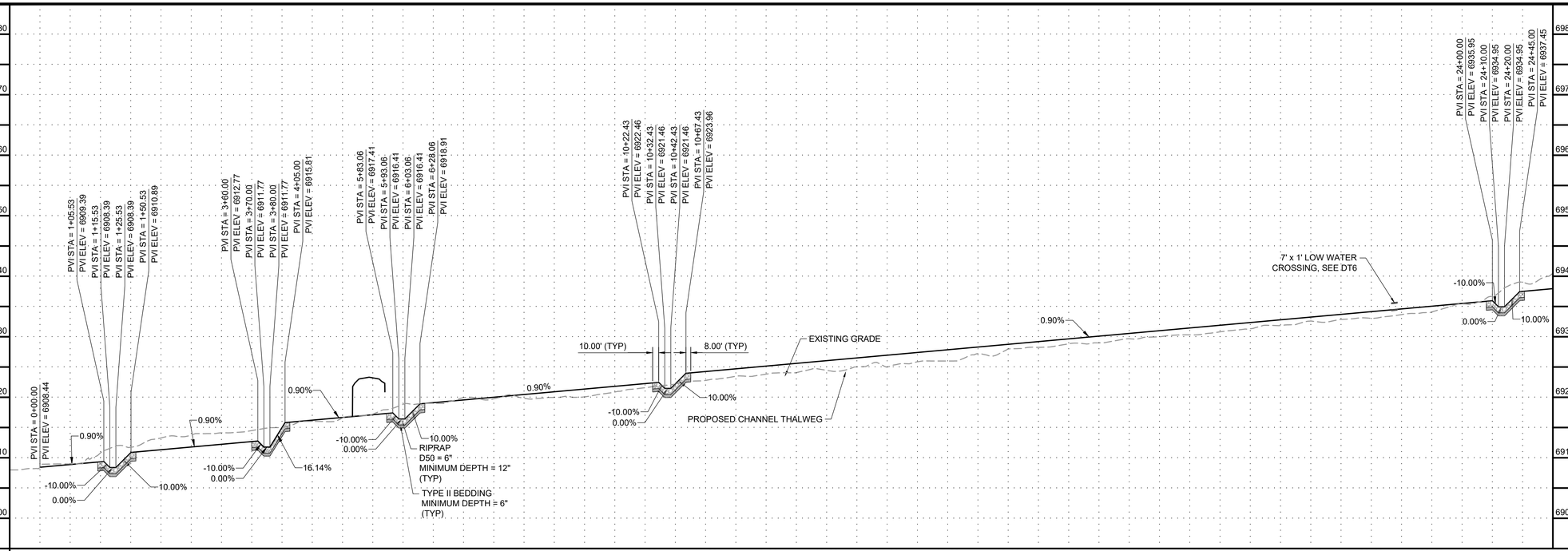
CONSTRUCTION DOCUMENTS
TRIBUTARY 2 BANKFULL GRADING

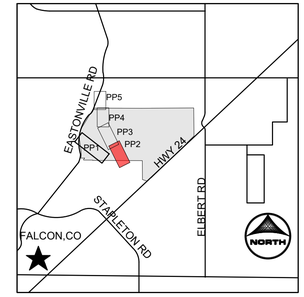
SHEET GR19

23

HR GREEN Xrefs: xg:1-dft01,GRADING KEY; HRG1 20X_EBase; xc-dsgn_662; xc-dsgn-662.10; xc-dsgn-F1-662.10; xc-row-F1-662.10; xc-row-662.10; xc-dsgn-F2; xc-row-F2; xx-util_662; xx-row-662; xc-dsgn-F3; xc-row-F3; xc-row-F4; 01-XC-INITIAL-EC; xc-row-PH2; xc-dsgn-PH2; xc-dsgn-PH3; 01-XC-channel; (01_XV-Row; 01-XC-PR_100YR_FP_DELINEATION







KEYMAP

PROJECT LEGEND:

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
- 5250 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- SECTION LINE
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NOTES:

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   CONTROL POINT COORDINATE SYSTEM: NAVD88
   NORTHING: 1421049.80
   EASTING: 3273631.55
   ELEVATION: 6866.33
3. ALIGNMENT NOT FOR USE IN CONSTRUCTION. REFER TO NORTHINGS AND EASTINGS
4. PLAN SET APPROVAL APPLIES TO THE LIMITS OF CONSTRUCTION. PLEASE SEE GRANDVIEW FILING NUMBER 1 PLAN SET FOR GRADING OUTSIDE OF THE CHANNEL LIMITS OF CONSTRUCTION.
5. TRIBUTARY 1 TO BE CONSTRUCTED WITH FILING 1 (AREA SOUTH OF TRIBUTARY 1), TRIBUTARY 2 TO BE CONSTRUCTED WITH FILING 2 (AREA NORTH OF TRIBUTARY 1 AND SOUTHWEST OF TRIBUTARY 2), BOX CULVERTS/CROSSINGS ALONG TRIBUTARY 2 TO BE CONSTRUCTED WITH TRIBUTARY IMPROVEMENTS.

HORIZONTAL 0 100 VERTICAL 0 10
50 5

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. LAZ 37081 PROFESSIONAL ENGINEER 11-17-23

DRAWN BY:	TBI	JOB DATE:	11/30/2023
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NO.	DATE	BY	REVISION DESCRIPTION

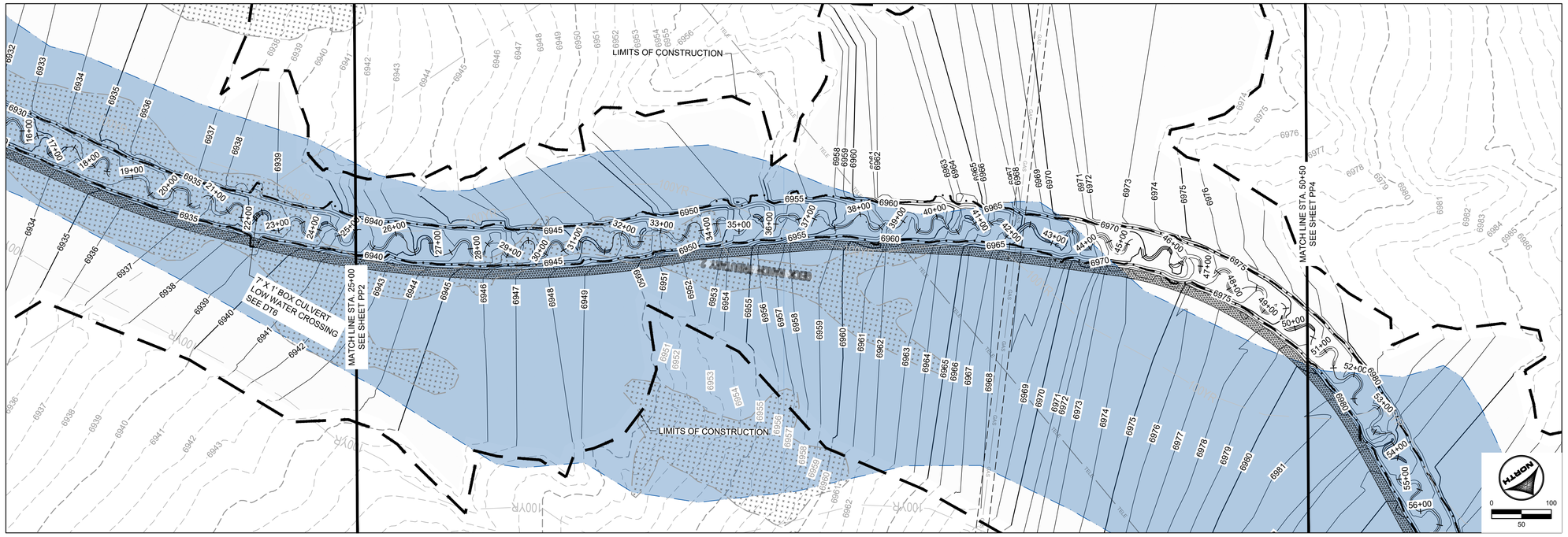
HRGreen
HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

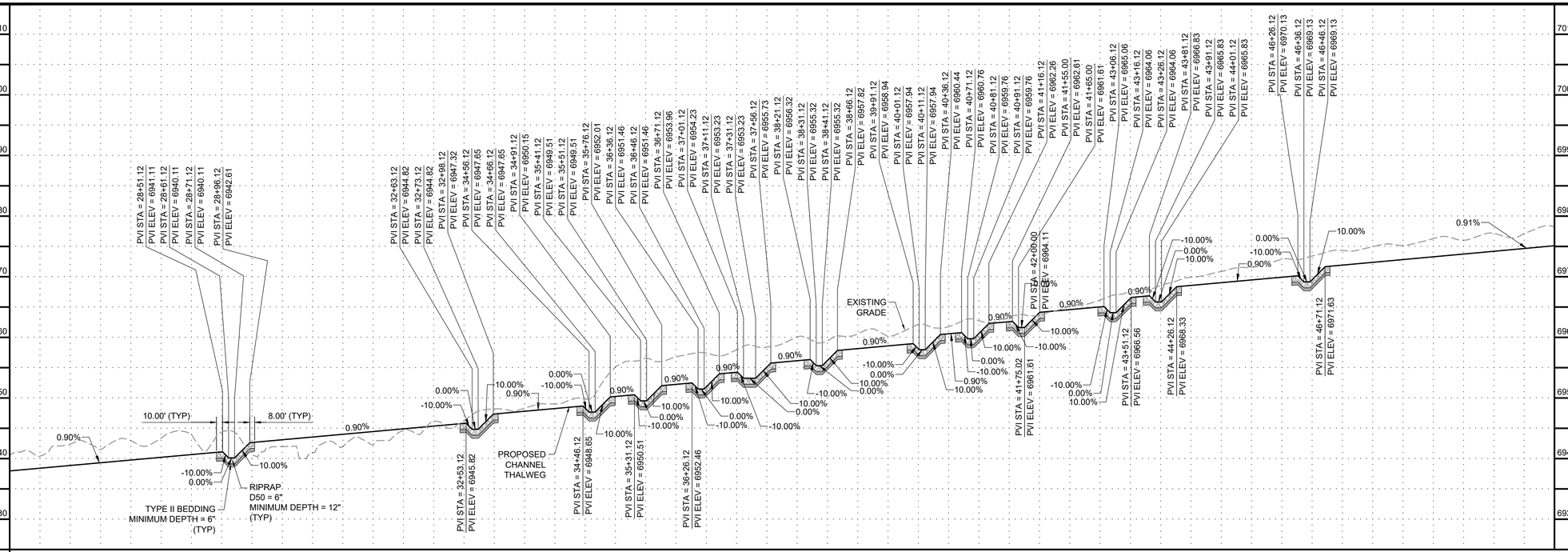
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

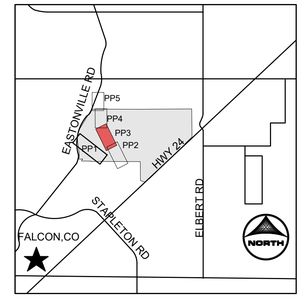
CONSTRUCTION DOCUMENTS
DRAINAGE TRIBUTARY 2 PLAN AND PROFILE

SHEET PP2 | 31

HR GREEN Xrefs: xg1-dh01; P&P KEY: HRG1X_EBase; 01-XC_Wingwalls; xv-dsgn_662; xc-dsgn-662.10; xc-dsgn-F1-662.10; xv-cnw-F1-662.10; xv-cnw-662; xc-row-F2; xc-dsgn-F2; xc-dsgn-F3; xv-cnw-F3; xv-dsgn-F4; xc-row-F4; 01-XC-PR_100YR_FP_DELINEATION; xc-channel; xc-filing-bndys; 01-XC-INITIAL-EC; xv-row-PH2; xv-row-PH3; 01_XC_RIPRAP; 01_XV-Row; 01-XC-channel







KEYMAP

PROJECT LEGEND:

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
- 5250 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- SECTION LINE
- EXISTING EASEMENT
- X X X EXISTING FENCE
- 5250 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- FLOW ARROW
- LIMITS OF CONSTRUCTION
- PROPOSED 100-YR FLOODPLAIN
- EFFECTIVE 100-YR FLOODPLAIN
- EXISTING WETLANDS - JURISDICTIONAL
- EXISTING WETLANDS - NONJURISDICTIONAL
- PROPOSED MAINTENANCE TRAIL

NOTES:

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2. BENCHMARK:
   DESIGNATION = F 24
   PID = JK0240
   DESCRIPTION = DISK ON TOP OF CONCRETE MONUMENT
   CONTROL POINT COORDINATE SYSTEM: NAVD88
   NORTHING: 1421049.80
   EASTING: 3273631.55
   ELEVATION: 6866.33
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HORIZONTAL 0 50 100 VERTICAL 0 5 10

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. PAYNE 37081 PROFESSIONAL ENGINEER

DRAWN BY:	TBI	JOB DATE:	11/30/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
CAD DATE:	11/30/2023		
CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\PLAN AND PROFILE		

BAR IS ONE INCH ON OFFICIAL DRAWINGS. 0 1" IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

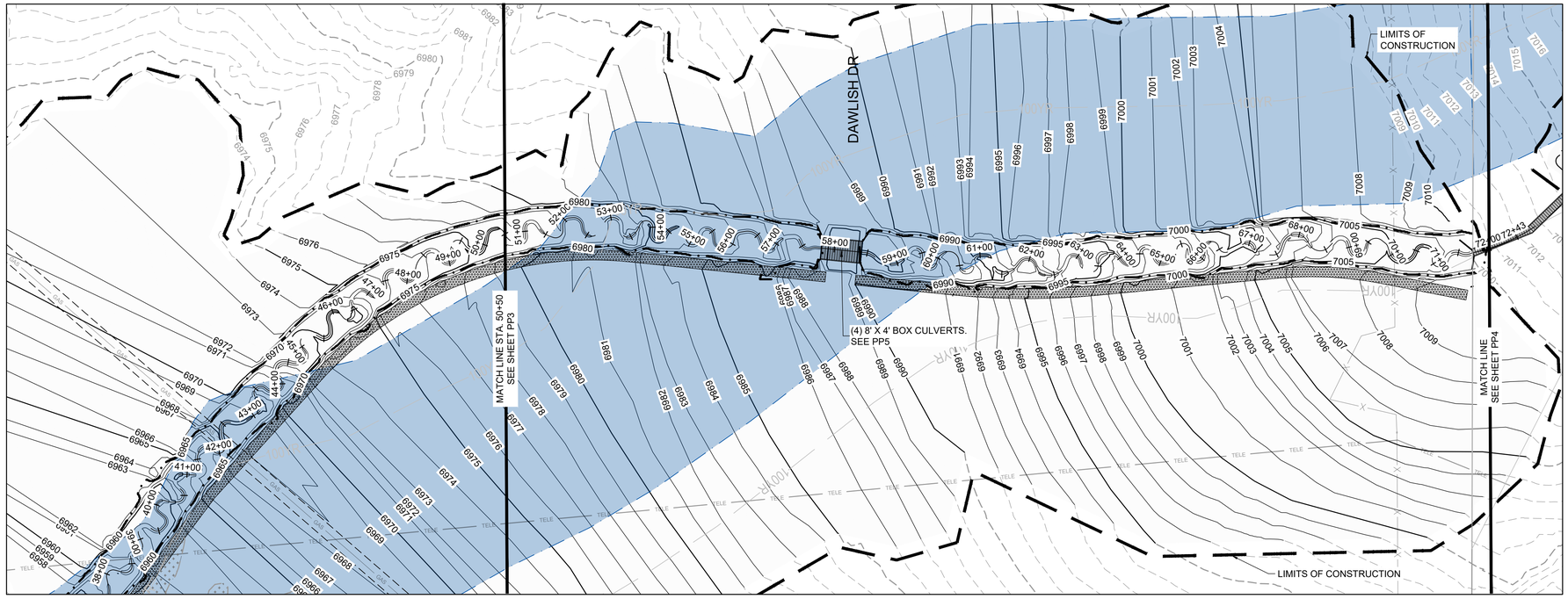
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

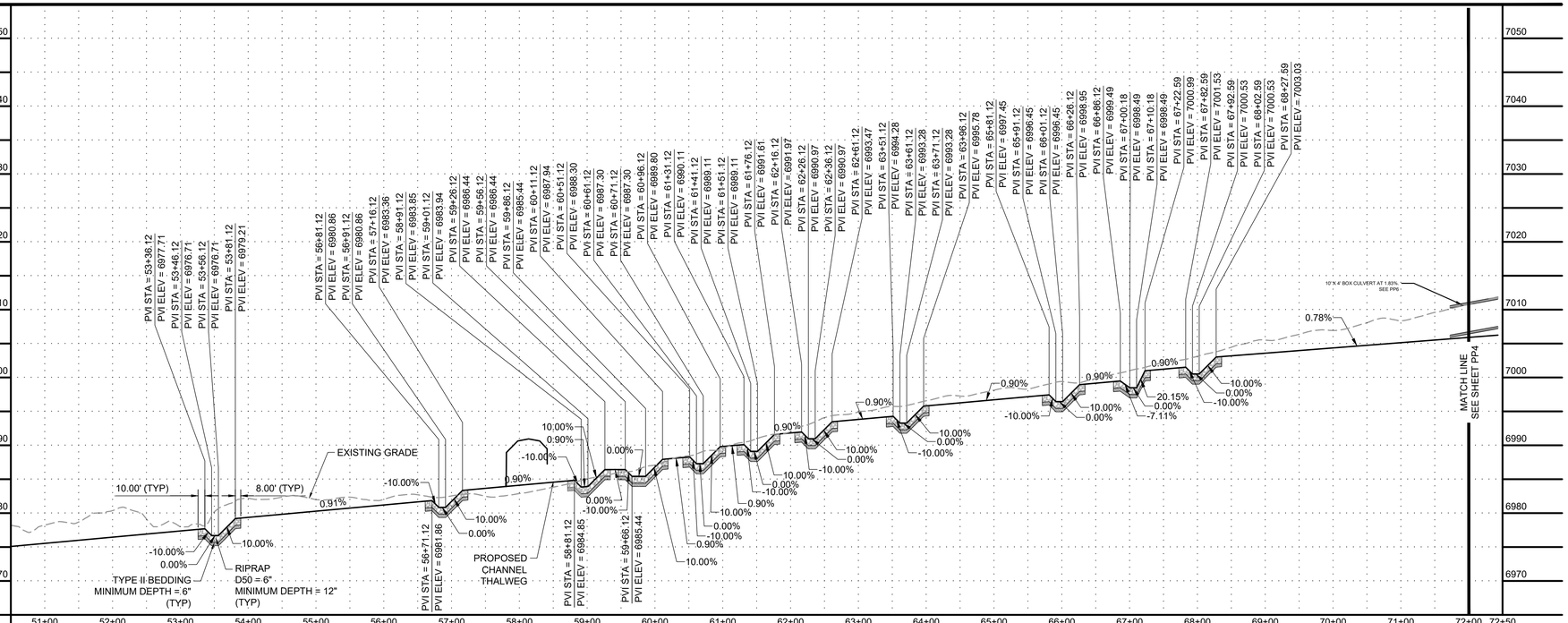
CONSTRUCTION DOCUMENTS
DRAINAGE TRIBUTARY 2 PLAN AND PROFILE

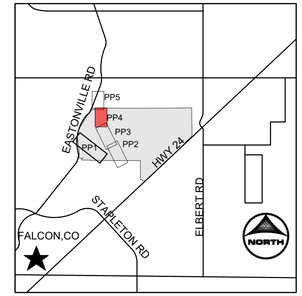
SHEET PP3

32

HR GREEN Xrefs: xgl-1-dh01; P&P KEY: HRG1X_EBase; 01-XC_Wingwalls; xv-dsgn_662; xc-dsgn-662.10; xc-dsgn-F1-662.10; xv-cnw-F1-662.10; xv-cnw-662; xc-cnw-F2; xc-dsgn-F2; xc-dsgn-F3; xv-cnw-F3; xv-dsgn-F4; xc-cnw-F4; 01-XC-PR_100YR_FP_DELINEATION; xc-channel; xc-filing-bndys; 01-XC-INITIAL-EC; xc-cnw-PH2; xc-dsgn-PH2; 01_XC_RIPRAP; 01_XV-Row; 01-XC-channel







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- PROPOSED MINOR CONTOUR
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- 5250 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- FLOW ARROW
- LIMITS OF CONSTRUCTION
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NORTH
0 100
50

HORIZONTAL 0 100 50
VERTICAL 0 10 5

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. PLATZ 37081 PROFESSIONAL ENGINEER

DRAWN BY: TBI JOB DATE: 11/30/2023
APPROVED: GLP JOB NUMBER: 201662.03
CAD DATE: 11/30/2023
CAD FILE: J:\2020\201662.03\CAD\Dwgs\C\PLAN AND PROFILE

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NO.	DATE	BY	REVISION DESCRIPTION

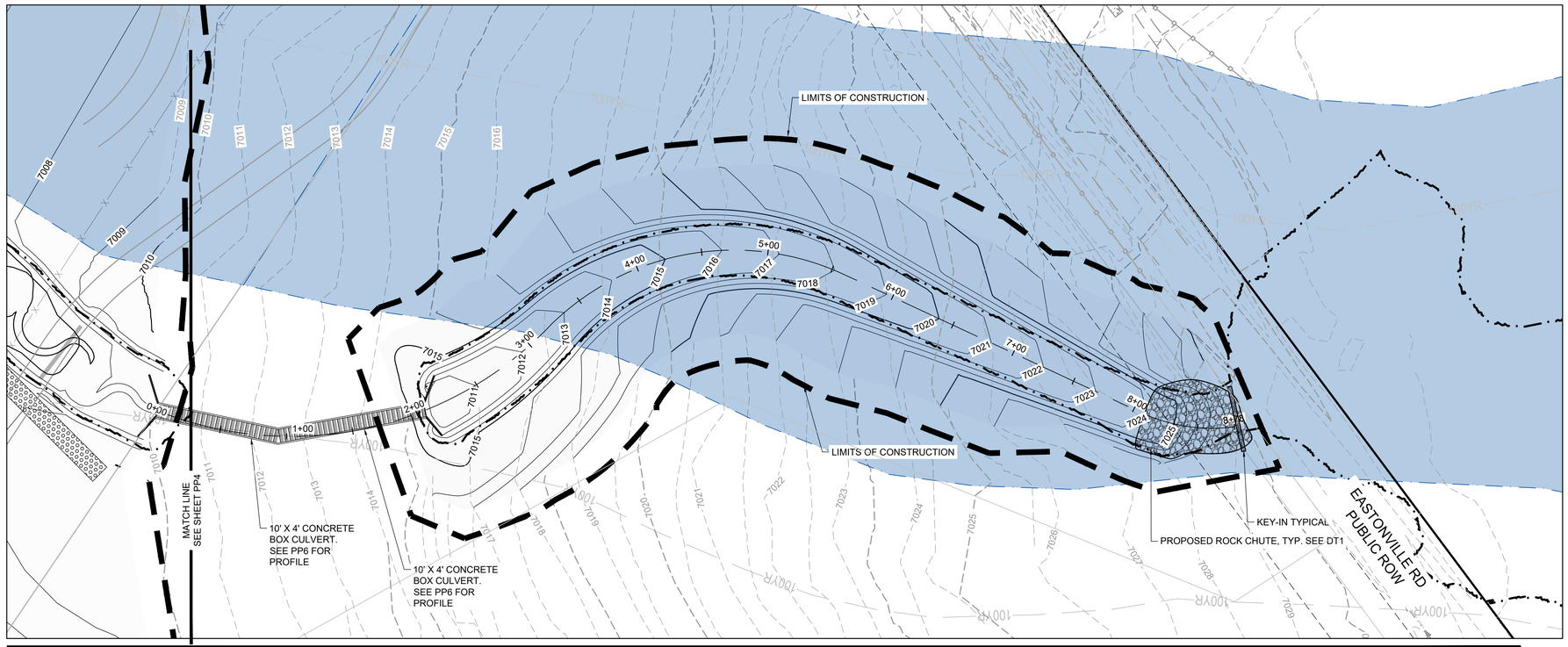
HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

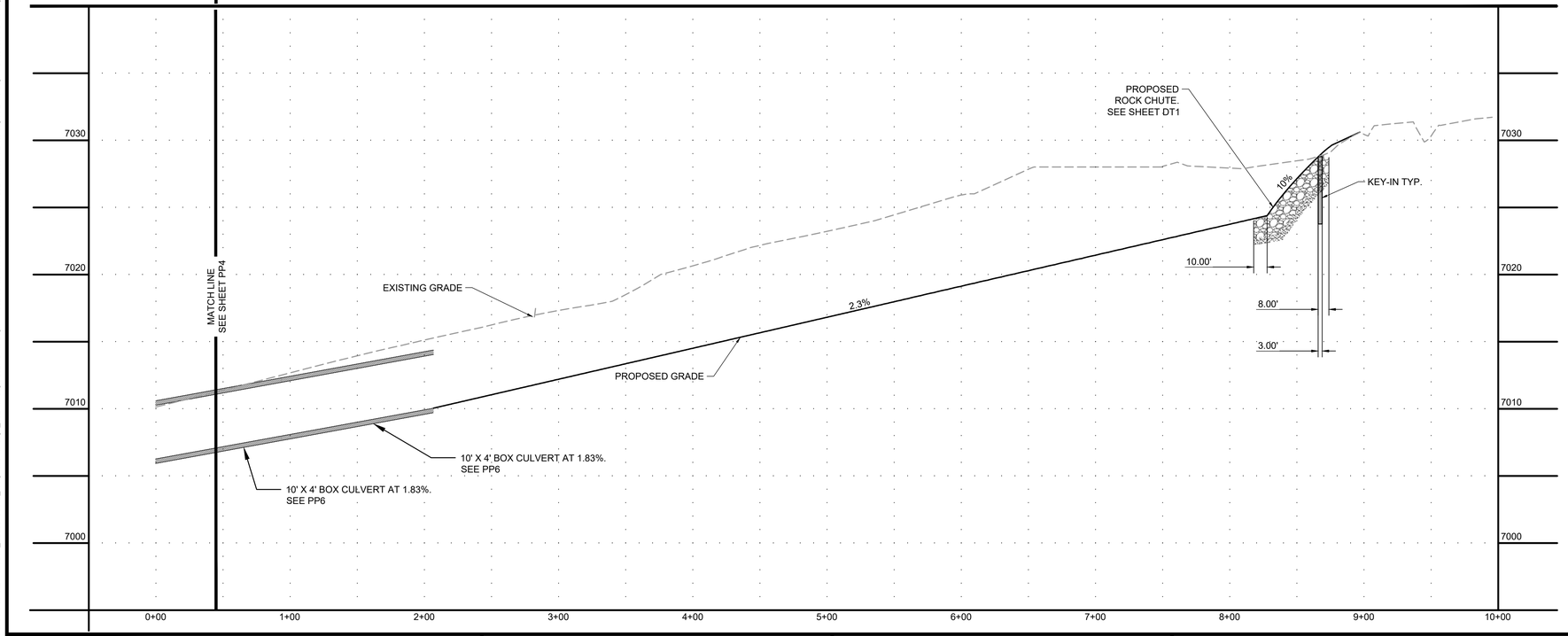
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

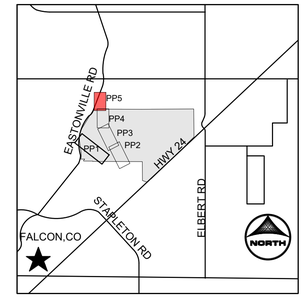
CONSTRUCTION DOCUMENTS
DRAINAGE TRIBUTARY 2 PLAN AND PROFILE

SHEET PP4 33

HR GREEN Xrefs: xg-t-dh01; P&P KEY: HRG1X_EBase; 01-XC_Wingwalls; xv-dsgn_662; xc-dsgn-662.10; xc-dsgn-F1-662.10; xc-xsw-F1-662.10; xv-xsw-662; xc-row-PH2; xc-dsgn-F2; xc-dsgn-F3; xc-row-F3; xc-dsgn-F4; xc-row-F4; 01-XC-PR_100YR_FP_DELINEATION; xc-channel; xc-filing-bndys; 01-XC-INITIAL-EC; xc-row-PH2; xc-row-PH3; 01_XC_RIPRAP; 01_XV-Row; 01-XC-channel







KEYMAP

PROJECT LEGEND:

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
- 5250 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- SECTION LINE
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- EXISTING FENCE
- 5250 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- FLOW ARROW
- LIMITS OF CONSTRUCTION
- PROPOSED 100-YR FLOODPLAIN
- EFFECTIVE 100-YR FLOODPLAIN
- EXISTING WETLANDS - JURISDICTIONAL
- EXISTING WETLANDS - NONJURISDICTIONAL
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NORTH
0 50
25

HORIZONTAL 0 50 25
VERTICAL 0 5 2.5

811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. PAYNE 37081 PROFESSIONAL ENGINEER 11-25-23

DRAWN BY:	TBI
APPROVED:	GLP
CAD DATE:	11/30/2023
CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\PLAN AND PROFILE
JOB DATE:	11/30/2023
JOB NUMBER:	201662.03

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NO.	DATE	BY	REVISION DESCRIPTION

HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

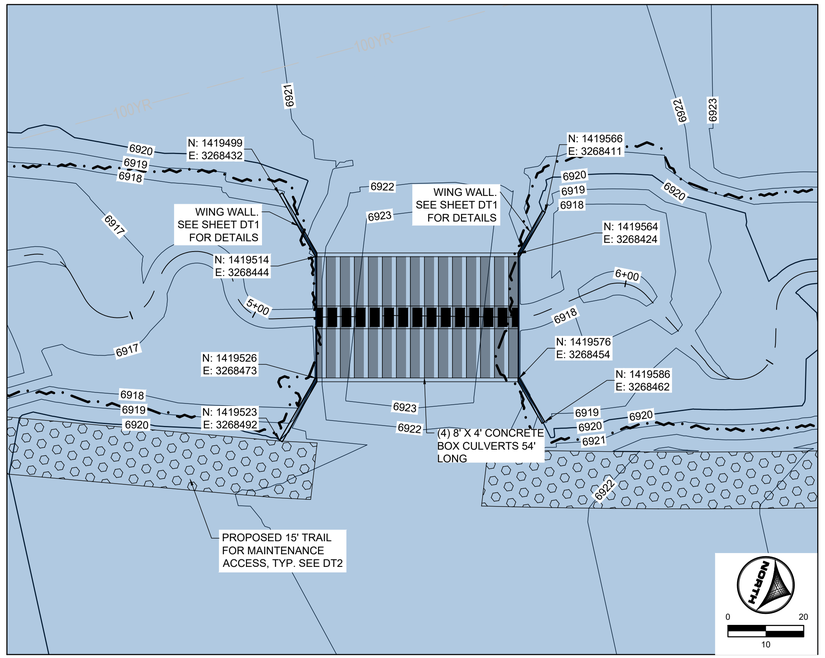
CONSTRUCTION DOCUMENTS

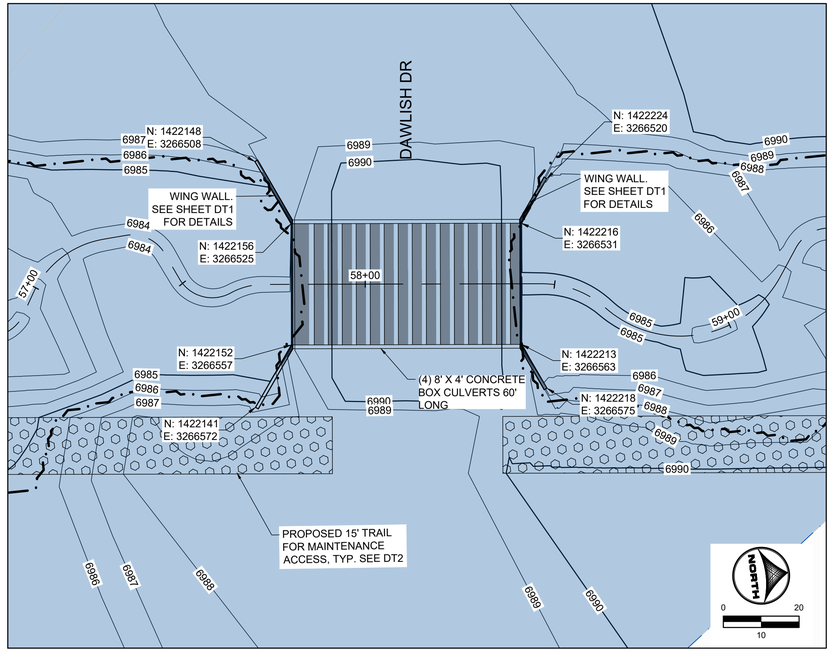
DRAINAGE TRIBUTARY 2 PLAN AND PROFILE

SHEET PP5

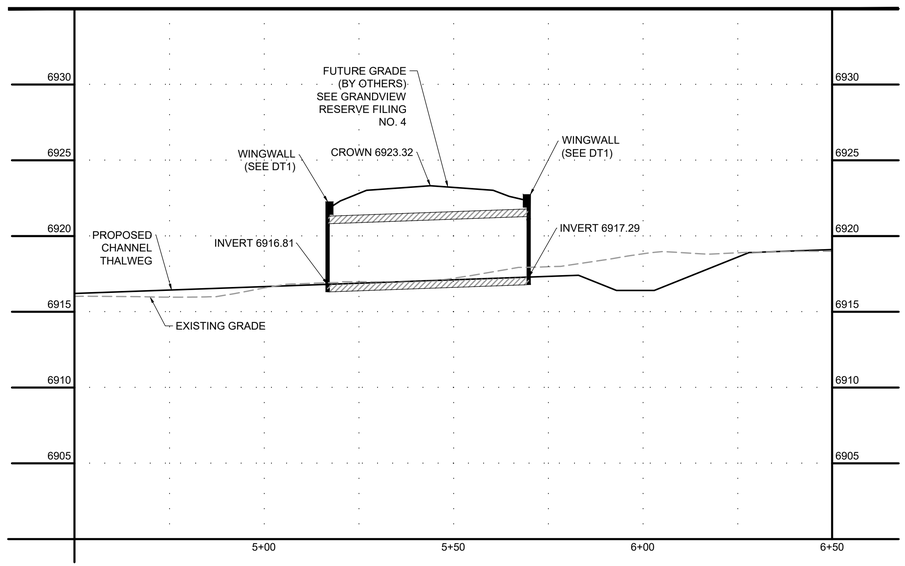
34

HR GREEN Xrefs: xgf-1-dh01; P&P KEY: HRG1X_EBase, 01-XC_Wtgwalls; xv-dsgn_662; xc-dsgn-662; 10; xc-dsgn-F1-662; 10; xv-row-F1-662; 10; xv-row-662; xc-row-F2; xv-dsgn-F2; xc-dsgn-F3; xv-row-F3; xc-dsgn-F4; xv-row-F4; 01-XC-PR_100YR_FP_DELINEATION; xc-channel; xc-filing-bndys; 01-XC-INITIAL-EC; xv-row-PH2; xc-dsgn-PH3; 01_XC_RIPRAP; 01_XV-Row; 01-XC-channel

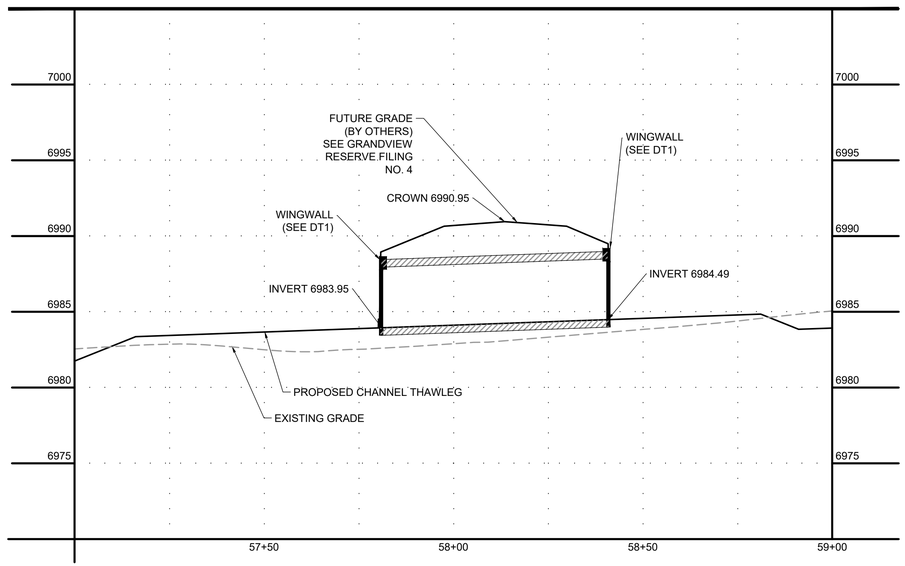




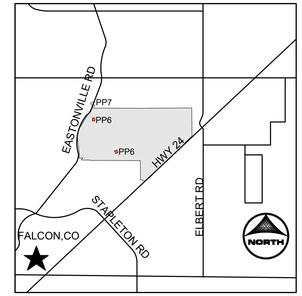
ROAD G CULVERT CROSSING



DAWLISH DR CULVERT CROSSING



HORIZONTAL 0 20 10 VERTICAL 0 5 2.5



KEYMAP

PROJECT LEGEND:

- PROPERTY LINE
- ROAD CENTERLINE
- RIGHT-OF-WAY LINE
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811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. ARP 37081 PROFESSIONAL ENGINEER 11-24-23

DRAWN BY:	TBI	JOB DATE:	11/30/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
CAD DATE:	11/30/2023		
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HR GREEN - DENVER
5613 DTC PARKWAY SUITE 950
DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

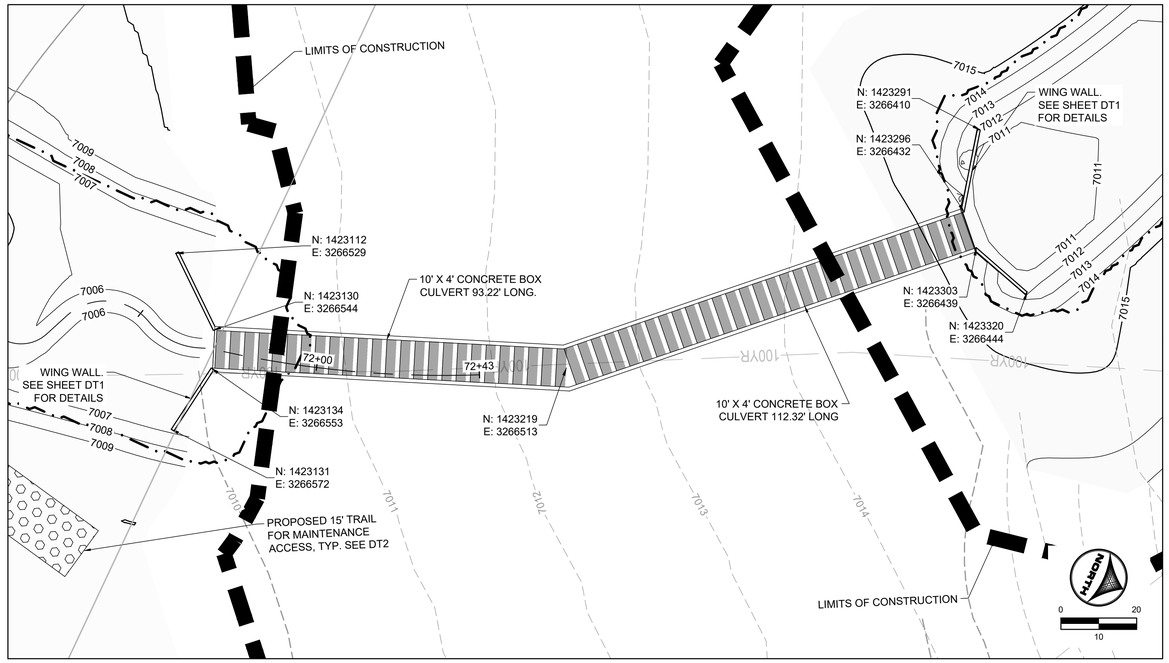
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

CONSTRUCTION DOCUMENTS

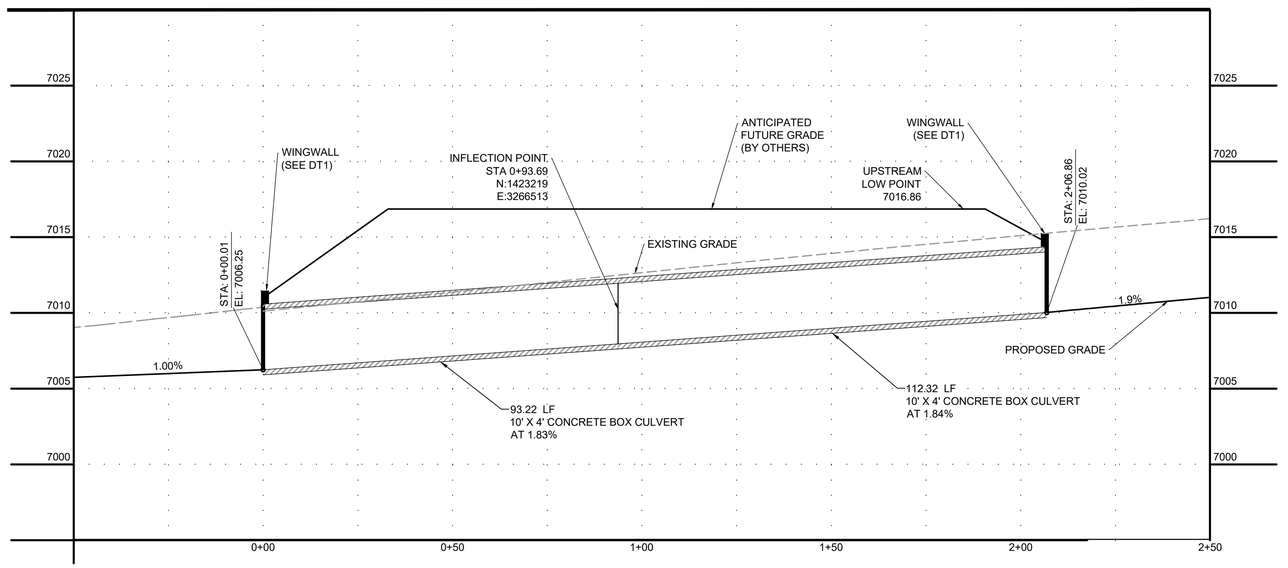
DRAINAGE CULVERTS PLAN AND PROFILE

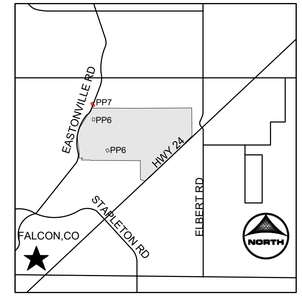
SHEET PP6 35

HR GREEN Xrefs: xgt-1-dh01; P&P KEY: HRG1X_EBase; 01-XC_Wingwalls; xv-dsgn_682; xc-dsgn-682 10; xc-dsgn-F1-662 10; xv-cnw-F1-662 10; xv-cnw-662; xc-cnw-F2; xc-dsgn-F2; xc-dsgn-F3; xv-cnw-F3; xv-dsgn-F4; xc-row-F4; 01-XC-PR_100YR_FP_DELINEATION; xc-channel; xc-filing-bndys; 01-XC-INITIAL-EC; xc-row-PH2; xv-cnw-662; xv-row-F1-662 10; xv-cnw-F1-662 10; xc-dsgn-F1-662 10; xc-dsgn-682 10; xc-dsgn-682; xv-dsgn_682; xv-dsgn; 01-XC_RIPRAP; 01_XV-Row; 01-XC-channel



REX ROAD CULVERT CROSSING





KEYMAP

PROJECT LEGEND:

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811 UNCC CALL BEFORE YOU DIG 811 OR 1-800-922-1987 Utility Notification Center of Colorado

COLORADO LICENSED GREGORY L. PAYNE 37081 PROFESSIONAL ENGINEER 12-12-23

HORIZONTAL 0 20 10 VERTICAL 0 5 2.5

DRAWN BY: TBI JOB DATE: 11/30/2023
APPROVED: GLP JOB NUMBER: 201662.03
CAD DATE: 11/30/2023
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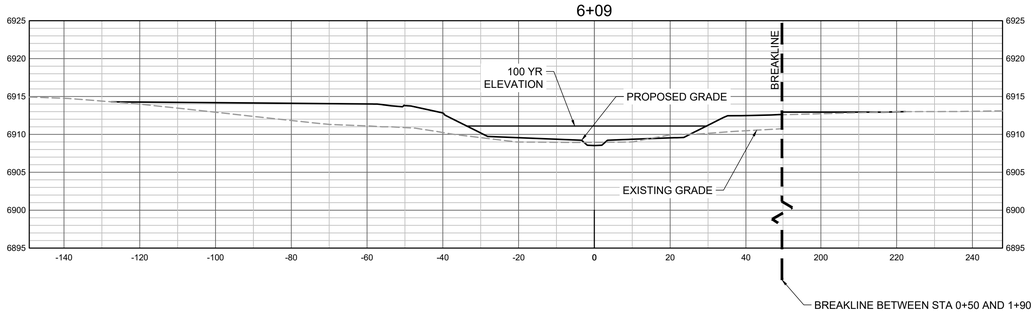
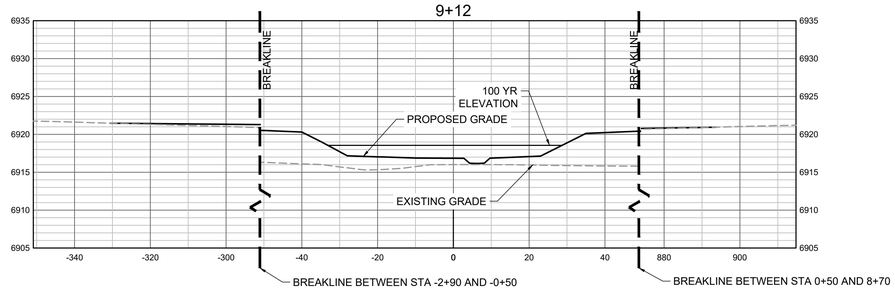
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

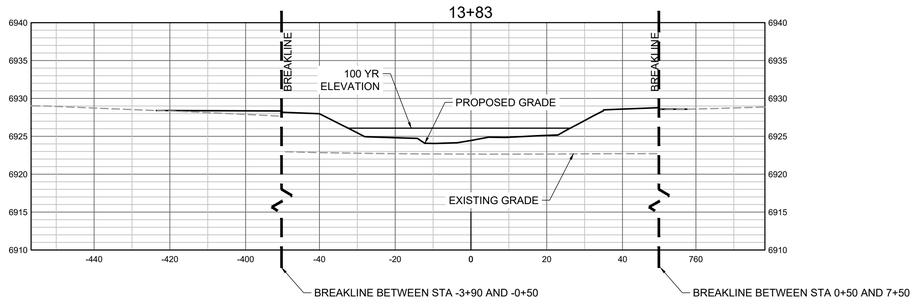
CONSTRUCTION DOCUMENTS
DRAINAGE CULVERT PLAN AND PROFILE

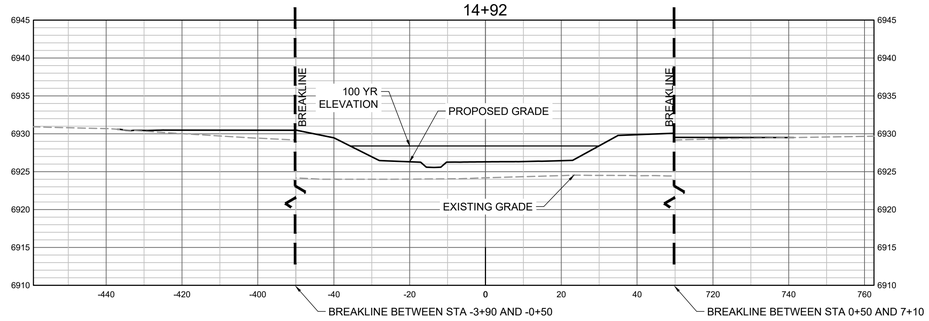
SHEET PP7 36

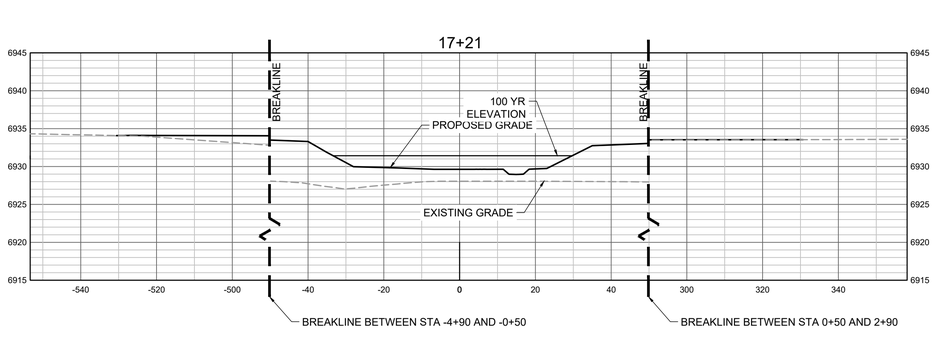
HR GREEN Xrefs: xgl-1-dh01; P&P KEY: HRG1X_EBase; 01-XC_Wingwalls; xv-dsgn_682; xc-dsgn-682.10; xc-dsgn-F1-662.10; xc-vow-F1-662.10; xv-vow-662; xc-row-PH2; xc-dsgn-F2; xc-dsgn-F3; xc-vow-F3; xc-dsgn-F4; xc-row-F4; 01-XC-PR_100YR_FP_DELINEATION; xc-channel; xc-filing-bndys; 01-XC-INITIAL-EC; xc-row-PH3; 01_XC_RIPRAP_01_XV-Row; 01-XC-channel

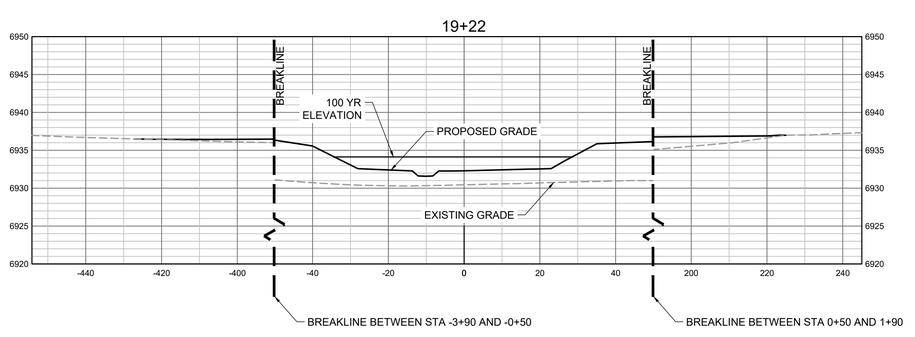


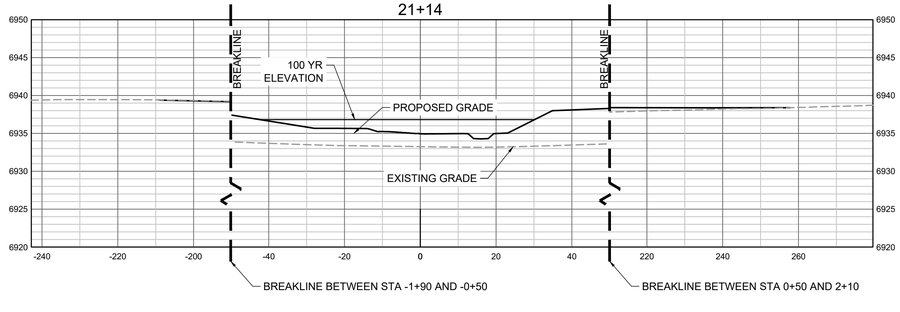


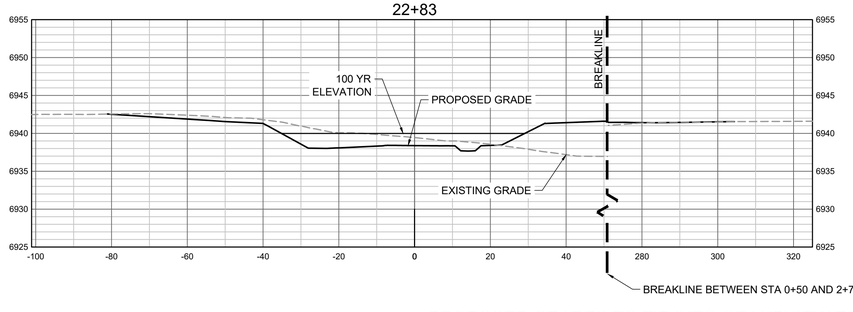












PROPOSED GRADES TO TIE INTO GRANDVIEW RESERVE FILING 1. REFER TO THE GRANDVIEW RESERVE FILING 1 PLAN SET FOR CONTINUATION OF GRADING THAT IS BEING TIED INTO OUTSIDE OF CHANNEL GRADING LIMITS.

HORIZONTAL 0 20 10 VERTICAL 0 10 5

HR GREEN X:\xref\ xgh1-db01_01-XC-PR_100YR_FP_DELINEATION

DRAWN BY:	TBI	JOB DATE:	11/30/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
CAD DATE:	11/30/2023		
CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\CROSS SECTIONS		

BAR IS ONE INCH ON OFFICIAL DRAWINGS. 0 1" IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

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DENVER CO 80111
PHONE: 720.602.4999
FAX: 713.965.0044

GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

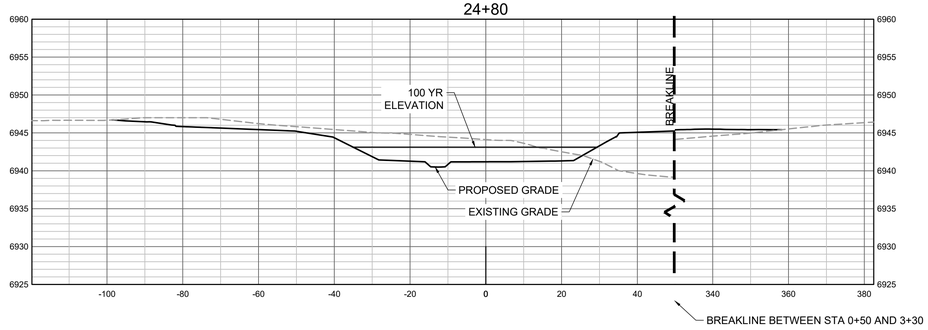
CONSTRUCTION DOCUMENTS

TRIBUTARY 2 CROSS SECTIONS

SHEET CS1

37

24+80



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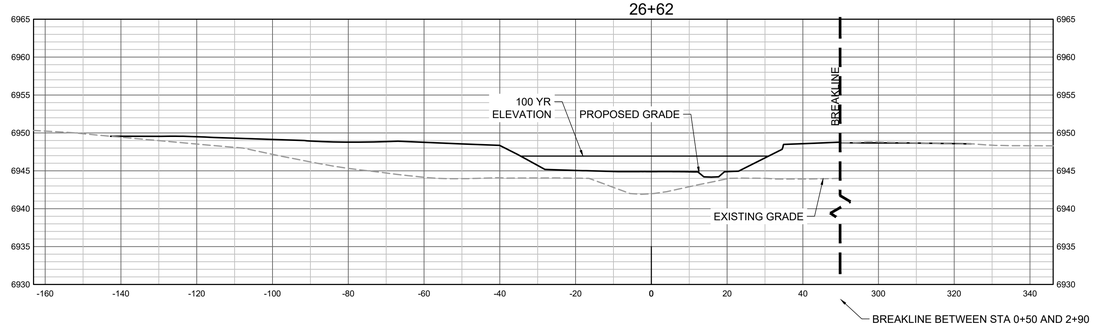

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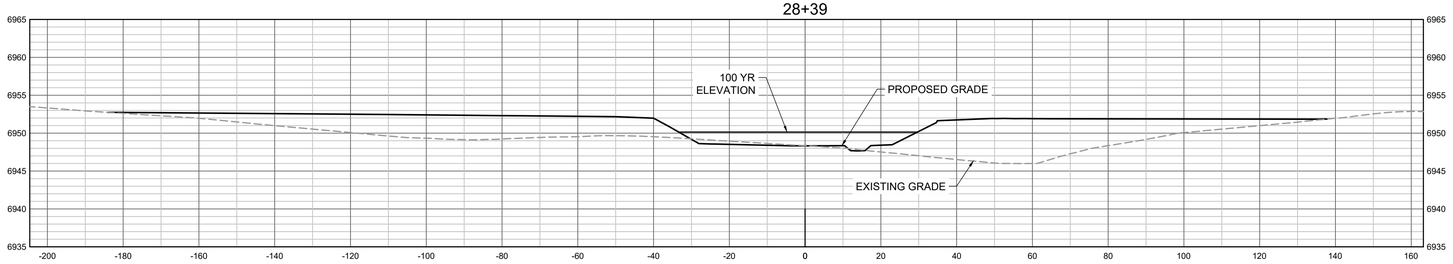

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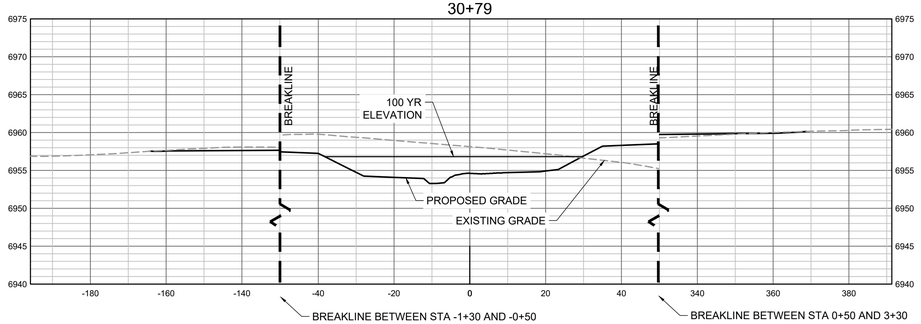

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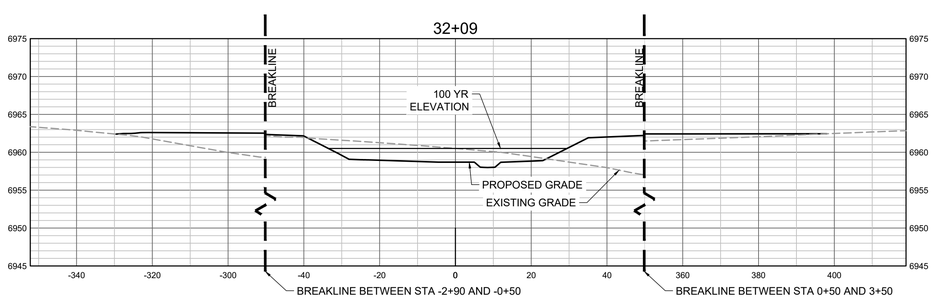

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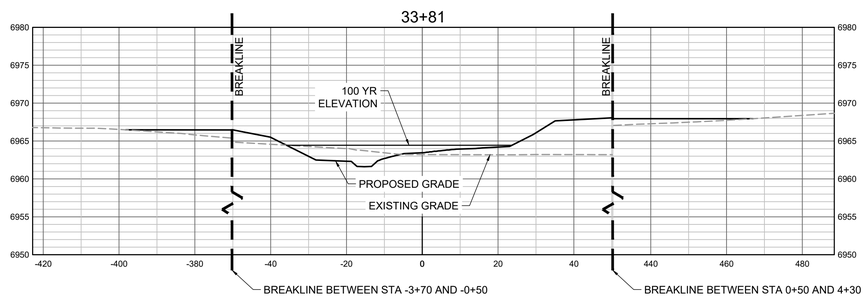

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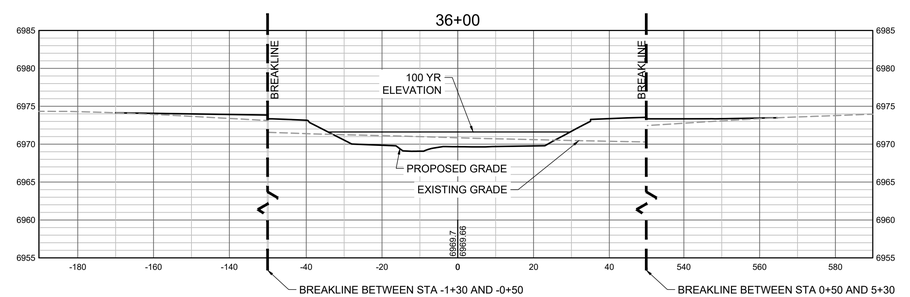

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HORIZONTAL 0 20 10 VERTICAL 0 10 5

HR GREEN Xrefs: xgt-1-dh01; 01-XC-PR_100YR_FP_DELINEATION

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CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\CROSS SECTIONS		

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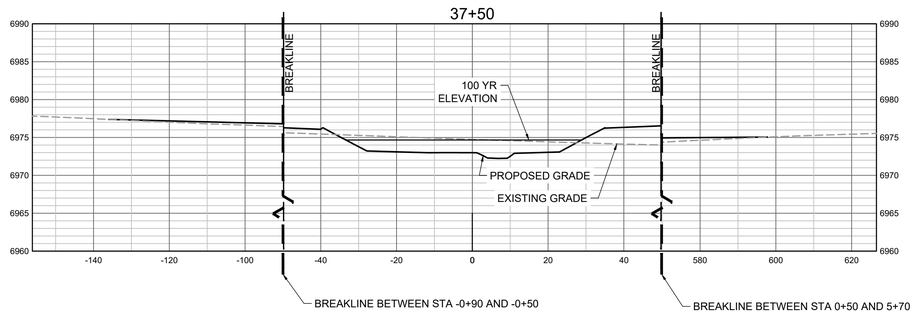
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

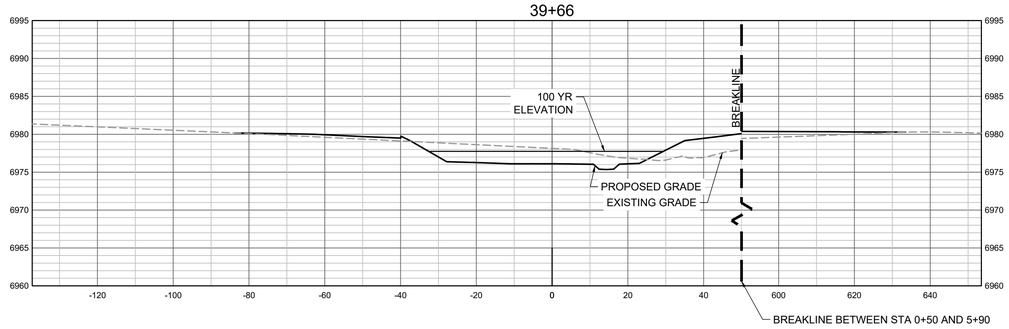
CONSTRUCTION DOCUMENTS

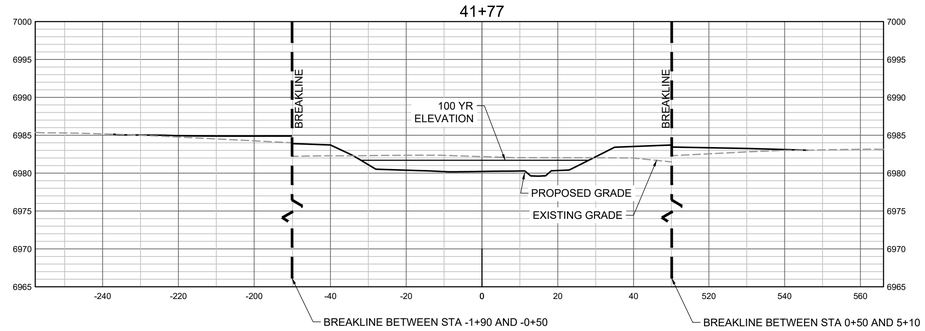
TRIBUTARY 2 CROSS SECTIONS

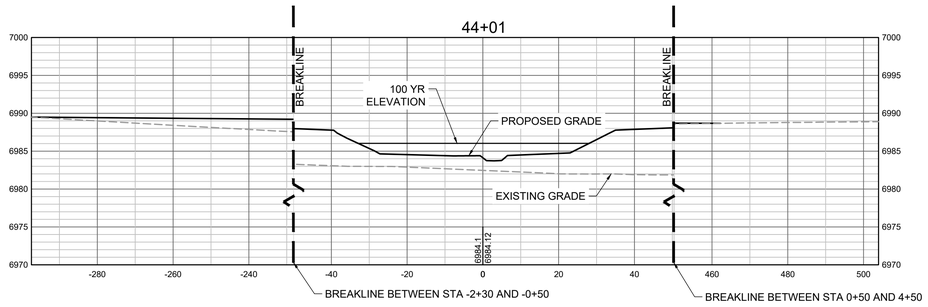
SHEET CS2

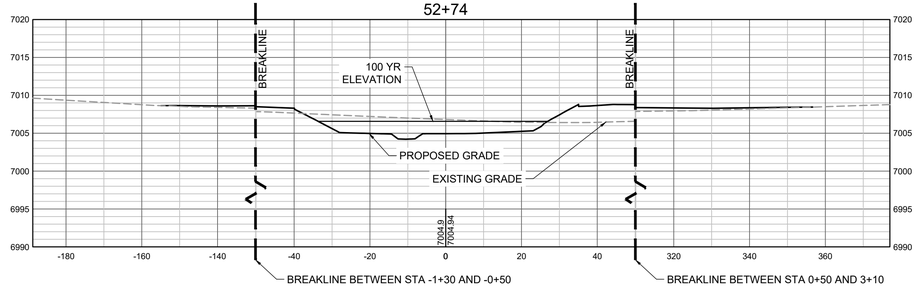
38

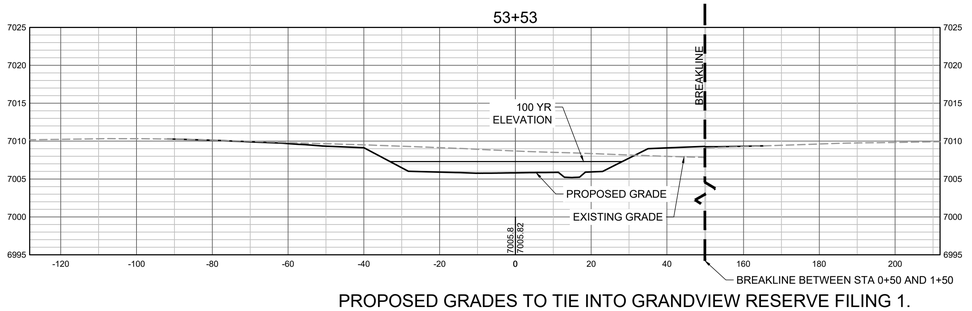












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HORIZONTAL 0 20 40 — VERTICAL 0 10 20

DRAWN BY:	TBI	JOB DATE:	11/30/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
CAD DATE:	11/30/2023		
CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\CROSS SECTIONS		

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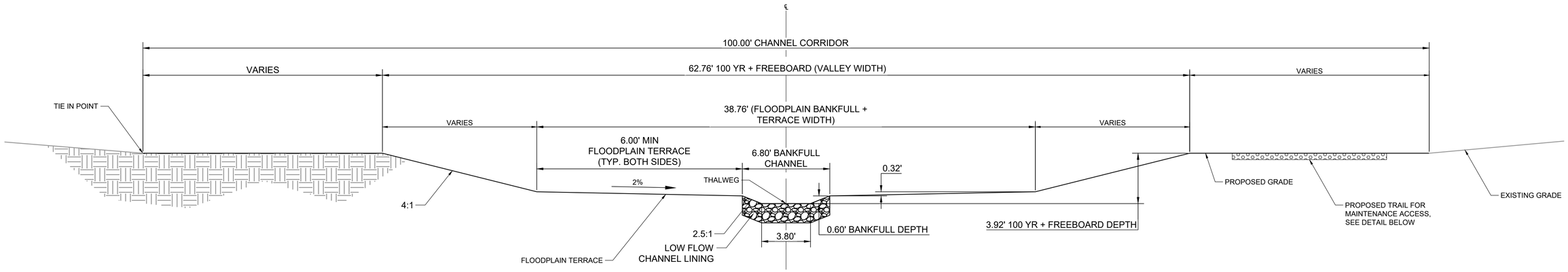
GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

CONSTRUCTION DOCUMENTS
TRIBUTARY 2 CROSS SECTIONS

SHEET CS3

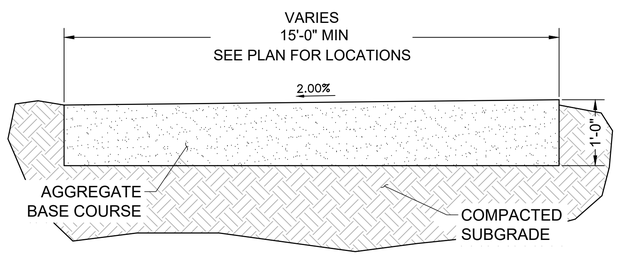
HR GREEN Xrefs: xgt-1-dh01_01-XC-PR_100YR_FP_DELINEATION



TYPICAL CROSS SECTION – TRIBUTARY 2

SCALE: N.T.S.

NOTES:
1. BANKFULL CHANNEL MAY SHIFT LEFT OR RIGHT WITHIN THE BANKFULL +TERRACE WIDTH SO LONG AS THE MINIMUM FLOOD PLAIN TERRACE WIDTH OF 6' IS MAINTAINED ON BOTH SIDES.
2. VALLEY WIDTH MAY SHIFT WITHIN THE 100' CHANNEL CORRIDOR.
3. SEE PROFILES FOR ELEVATION AT THALWEG.



MAINTENANCE ROAD TYPICAL SECTION

SCALE: NTS

DRAWN BY:	TBI	JOB DATE:	5/10/2023
APPROVED:	GLP	JOB NUMBER:	201662.03
CAD DATE:	11/30/2023		
CAD FILE:	J:\2020\201662.03\CAD\Dwgs\C\DETAILS		

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GRANDVIEW RESERVE
DR HORTON
FALCON, COLORADO

CONSTRUCTION DOCUMENTS
DETAILS

SHEET DT2

41

HR GREEN Xrefs: xgt-1-dh01

HRGreen	**GRANDVIEW - BASIN C**	**Calc'd by:**	**NQJ**
	PROPOSED CONDITIONS	**Checked by:**	
	LOCATION: COLORADO SPRINGS, COLORADO	**Date:**	**2/29/2024**

SUMMARY RUNOFF TABLE

BASIN	AREA (ac)	% IMPERVIOUS	Q_5 (cfs)	Q_{100} (cfs)
C1	4.53	65	8.2	18.0
C2	2.71	65	4.8	10.6
C3	2.47	65	4.7	10.3
C4	3.09	65	4.5	9.8
C5	3.13	65	4.7	10.4
C6	0.81	45	1.0	2.6
C7	6.07	65	7.7	17.0
C8	5.11	65	7.4	16.2
C9	3.50	65	5.5	12.1
C10	3.97	66	6.3	13.6
C11	3.64	67	5.9	12.9
C12	0.46	65	0.9	2.0
C13	1.66	65	2.9	6.4
C14	2.37	65	4.2	9.3
C15	1.69	65	3.9	8.7

DESIGN POINT SUMMARY TABLE

DESIGN POINT	CONTRIBUTING BASINS	ΣQ_5 (cfs)	ΣQ_{100} (cfs)
20	C1	8.2	18.0
21	C2	4.8	10.6
21.1	C1-C2	10.3	17.7
22	C3	4.7	10.3
22.1	DP21.1 & C3	17.9	25.3
23	C4	4.5	11.4
24	C5	5.0	14.7
24.1	DP22.1, DP23 & DP24	23.4	44.1
25	C6	1.0	2.6
26	DP25 & C7	11.5	22.0
27	C8	7.4	16.2
27.1	DP26-27	16.5	25.4
28	C9	5.5	12.1
28.1	DP27.1 & DP28	20.8	34.6
29.1	DP24.1 & DP28.1	40.2	73.0
30	C10	6.3	13.6
30.1	DP29.1 & DP30	44.8	91.1
31	C11	5.9	12.9
31.1	DP30.1 & DP31	46.0	97.6
32	C12	0.9	2.0
32.1	DP31.1 & DP32	46.4	98.4
33	C13	2.9	6.4
33.1	DP32.1 & DP33	48.1	102.3
34	DP33.1 & C14	50.5	107.4
35	C15	3.9	8.7

HRGreen

GRANDVIEW - BASIN C

PROPOSED CONDITIONS

LOCATION: COLORADO SPRINGS, COLORADO

Calc'd by: NQJ

Checked by:

Date: 2/29/2024

COMPOSITE 'C' FACTORS

BASIN	UNDEVELOPED	PAVED	1/8 AC RESIDENTIAL	NEIGHBORHOOD AREAS	TOTAL	SOIL TYPE	UNDEVELOPED			PAVED			1/8 AC RESIDENTIAL			NEIGHBORHOOD AREAS			COMPOSITE IMPERVIOUSNESS & C		
	ACRES						%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}	%I	C_5	C_{100}
C1	0.00	0.00	4.53	0.00	4.53	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C2	0.00	0.00	2.71	0.00	2.71	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C3	0.00	0.00	2.47	0.00	2.47	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C4	0.00	0.00	3.09	0.00	3.09	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C5	0.00	0.00	3.13	0.00	3.13	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C6	0.26	0.00	0.55	0.00	0.81	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	45	0.33	0.52
C7	0.00	0.00	6.07	0.00	6.07	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C8	0.00	0.00	5.11	0.00	5.11	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C9	0.00	0.00	3.50	0.00	3.50	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C10	0.00	0.17	3.80	0.00	3.97	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	66	0.47	0.61
C11	0.00	0.17	3.47	0.00	3.64	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	67	0.47	0.61
C12	0.00	0.00	0.46	0.00	0.46	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C13	0.00	0.00	1.66	0.00	1.66	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C14	0.00	0.00	2.37	0.00	2.37	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
C15	0.00	0.00	1.69	0.00	1.69	A/B	2	0.09	0.36	100	0.90	0.96	65	0.45	0.59	70	0.53	0.68	65	0.45	0.59
POND C					**43.52**														**64.9**		
TOTAL BASIN C					**45.21**														**64.9**		

HRGreen	**GRANDVIEW - BASIN C**	**Calc'd by:**	**NQJ**
	PROPOSED CONDITIONS	**Checked by:**	
	LOCATION: COLORADO SPRINGS, COLORADO	**Date:**	**2/29/2024**

TIME OF CONCENTRATION

BASIN DATA			OVERLAND TIME (T_i)			TRAVEL TIME (T_t)					TOTAL
DESIGNATION	C_5	AREA (ac)	LENGTH (ft)	SLOPE %	t_i (min)	C_V	LENGTH (ft)	SLOPE %	V (ft/s)	t_t (min)	t_c (min)
C1	0.45	4.53	65	3.0	6.7	20	680	1.80	2.7	4.2	10.9
C2	0.45	2.71	55	2.0	7.0	20	680	1.80	2.7	4.2	11.2
C3	0.45	2.47	55	2.0	7.0	20	350	1.50	2.4	2.4	9.4
C4	0.45	3.09	150	2.0	11.6	20	820	1.00	2.0	6.8	18.4
C5	0.45	3.13	110	2.0	9.9	20	820	1.00	2.0	6.8	16.8
C6	0.33	0.81	161	3.0	12.3	15	110	2.00	2.1	0.9	13.2
C7	0.45	6.07	150	2.0	11.6	20	1300	0.80	1.8	12.1	23.7
C8	0.45	5.11	110	2.0	9.9	20	930	0.80	1.8	8.7	18.6
C9	0.45	3.50	110	2.0	9.9	20	570	0.80	1.8	5.3	15.2
C10	0.47	3.97	150	2.0	11.2	20	770	1.50	2.4	5.2	16.5
C11	0.47	3.64	110	3.0	8.4	20	1050	1.50	2.4	7.1	15.5
C12	0.45	0.46	90	2.0	9.0	20	5	2.00	2.8	0.0	9.0
C13	0.45	1.66	110	2.0	9.9	20	250	1.40	2.4	1.8	11.7
C14	0.45	2.37	230	10.0	8.4	15	250	1.00	1.5	2.8	11.2
C15	0.45	1.69	50	25.0	2.9	15	200	2.00	2.1	1.6	5.0

FORMULAS:

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \qquad V = C_v S_w^{0.5}$$

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

*For buried riprap, select C_v value based on type of vegetative cover.

HRGreen

GRANDVIEW - BASIN C
PROPOSED CONDITIONS
DESIGN STORM: 5-YEAR

Calc'd by: NQJ
Checked by:
Date: 2/29/2024

			DIRECT RUNOFF						TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
STREET	DESIGN POINT	BASIN ID	AREA (ac)	C_5	t_c (min)	C_5*A (ac)	I (in./ hr.)	Q (cfs)	t_c (min)	C_5*A (ac)	I (in./ hr.)	Q (cfs)	Q_{street} (cfs)	C_5*A (ac)	SLOPE %	Q_{PIPE} (cfs)	C_5*A (ac)	SLOPE %	PIPE SIZE (ft)	LENGTH (FT)	VEL. (FPS)	TRAVEL TIME (min	
	20	C1	4.53	0.45	10.9	2.04	4.00	8.2					0.5	0.11	1.0	7.7	1.92	2.0	2.0	820 28	2.0 10.2	6.83 0.05	DP20 FLOWBY, C&G FLOW TO DP24 BASIN C1 CAPTURED IN 15' TYPE R ONGRADE @ DP20, PIPE TO DP21.1
	21	C2	2.71	0.45	11.2	1.22	3.95	4.8								4.8	1.21	2.0	2.0	5	10.2	0.01	BASIN C2 CAPTURED IN 15' TYPE R ONGRADE @ DP21, PIPE TO DP21.1
	21.1								17.7	3.14	3.27	10.3				10.3	3.14	2.0	2.0				COMBINED DP20 & DP21, PIPE TO DP22.1
	22	C3	2.47	0.45	9.4	1.11	4.22	4.7								4.7	1.11	2.0	2.0				BASIN 3 CAPTURED IN 15' TYPE R ONGRADE @ DP22, PIPE TO DP22.1
	22.1								9.4	4.25	4.22	17.9				17.9	4.25	1.0	2.0	820	7.2	1.90	COMBINED DP21.1 & DP22, PIPE TO DP24.1
	23	C4	3.09	0.45	18.4	1.39	3.21	4.5								4.5	1.39	2.0	2.0	5	10.2	0.01	BASIN C4 CAPTURED IN 15' TYPE R ONGRADE @ DP24, PIPE TO DP24.1
	24	C5	3.13	0.45	16.8	1.41	3.36	4.7	17.7	1.52	3.27	5.0				5.0	1.52	2.0	2.0	28	10.2	0.05	DP20 FLOWBY & BASIN C5 CAPTURED IN 15' TYPE R ONGRADE @ DP24, PIPE TO DP24.1
	24.1								17.8	7.16	3.27	23.4				23.4	7.16	2.0	2.0	53	10.2	0.09	COMBINED DP22.1, DP23 & DP24, PIPE TO DP29.1
	25	C6	0.81	0.33	13.2	0.27	3.71	1.0					1.0	0.27	0.8					605	1.8	5.64	BASIN C6 FLOW @ DP25, C&G FLOW TO DP26
	26	C7	6.07	0.45	23.7	2.73	2.84	7.7	23.7	3.00	2.84	11.5	1.8	0.47	1.5	9.7	2.53	2.0	2.0	1050 5	2.4 10.2	7.14 0.01	DP26 FLOWBY, C&G FLOW TO DP31 DP17, DP18, DP25 & BASIN C7 CAPTURED IN 15' TYPE R ONGRADE @ DP26, PIPE TO DP27.1
	27	C8	5.11	0.45	18.6	2.30	3.20	7.4					0.2	0.05	1.5	7.2	2.25	2.0	2.0	270 18	2.4 10.2	1.84 0.03	DP27 FLOWBY, C&G FLOW TO DP28 BASIN C8 FLOW CAPTURED IN 15' TYPE R ONGRADE, PIPE TO DP27.1
	27.1								23.7	4.78	2.83	16.5				16.5	4.78	2.0	2.0	265	10.2	0.43	COMBINED DP26 & DP27, PIPE TO DP28.1
	28	C9	3.50	0.45	15.2	1.58	3.50	5.5	20.4	1.62	3.06	8.0	0.4	0.07	1.5	7.6	1.55	2.0	2.0	780 30	2.4 10.2	5.31 0.05	DP28 FLOWBY, C&G FLOW TO DP30 DP27 FLOWBY & BASIN C9 CAPTURED IN 15' TYPE R ON GRADE @ DP28, PIPE TO DP28.1
	28.1								24.1	6.33	2.81	20.8				20.8	6.33	2.0	2.0	490	10.2	0.80	COMBINED DP27.1 & DP29, PIPE TO DP29.1
	29.1								24.9	13.49	2.76	40.2				40.2	13.49	2.0	2.0	335	10.2	0.55	COMBINED DP28.1 & DP24.1, PIPE TO DP30.1
	30	C10	3.97	0.47	16.5	1.86	3.38	6.3	25.7	1.94	2.71	8.3				8.3	1.94	2.0	2.0	30	10.2	0.05	DP28 FLOWBY & BASIN C10 FLOW CAPTURED IN 20' TYPE R SUMP @ DP30, PIPE TO DP30.1
	30.1								25.8	15.43	2.71	44.8				44.8	15.43	2.0	2.0	5	10.2	0.01	COMBINED DP29.1 & DP20, PIPE TO DP31.1
	31	C11	3.64	0.47	15.5	1.71	3.47	5.9	30.8	2.19	2.44	8.3				8.3	2.19	2.0	2.0				DP26 FLOWBY & BASIN C11 CAPTURED IN 20' TYPE R SUMP @ DP31, PIPE TO DP31.1
	31.1								30.8	17.62	2.44	46.0				46.0	17.62	2.0	2.0	90	10.2	0.15	COMBINED DP30.1 & DP31, PIPE TO DP32.1
	32	C12	0.46	0.45	9.0	0.21	4.29	0.9								0.9	0.21	2.0	2.0				BASIN 32 CAPTURED IN 5' TYPE R SUMP, PIPE TO DP32.1
	32.1								31.0	17.82	2.43	46.4				46.4	17.82	2.0	2.0	32	10.2	0.05	COMBINED DP31.1 & DP32, PIPE TO DP33.1
	33	C13	1.66	0.45	11.7	0.75	3.90	2.9								2.9	0.75	2.0	2.0				BASIN C13 CAPTURED IN 5' TYPE R SUMP, PIPE TO DP33.1
	33.1								31.0	18.57	2.43	48.1				48.1	18.57	2.0	2.0	170	10.2	0.28	COMBINED DP32.1 & DP33, PIPE TO POND C
	34	C14	2.37	0.45	11.2	1.07	3.96	4.2	31.3	19.64	2.42	50.5											**TOTAL FLOW ENTERING POND C**
	35	C15	1.69	0.45	5.0	0.76	5.17	3.9															BASIN C15 FLOW, FOLLOWS HISTORIC DRAINAGE PATTERNS TOWARDS DP35

GRANDVIEW - BASIN C

PROPOSED CONDITIONS

DESIGN STORM: 100-YEAR

HRGreen

Calc'd by: NQJ

Checked by:

Date: 2/29/2024

			DIRECT RUNOFF						TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
STREET		BASIN ID	AREA (ac)	C_{100}	t_c (min)	C_{100}*A (ac)	I (in./ hr.)	Q (cfs)	t_c (min)	C_{100}*A (ac)	I (in./ hr.)	Q (cfs)	Q_{street} (cfs)	C_{100}*A (ac)	SLOPE %	Q_{PIPE} (cfs)	C_{100}*A (ac)	SLOPE %	PIPE SIZE (ft)	LENGTH (ft)	VEL. (ft/s)	TRAVEL TIME (min)	
	20	C1	4.53	0.59	10.9	2.67	6.72	18.0					5.6	0.83	1.0	12.4	1.85	2.0	2.0	820	2.0	6.83	DP20 FLOWBY, C&G FLOW TO DP24
																				28	10.2	0.05	BASIN C1 CAPTURED IN 15' TYPE R ONGRADE @ DP20, PIPE TO DP21.1
	21	C2	2.71	0.59	11.2	1.60	6.64	10.6					1.4	0.21	1.5	9.2	1.39	2.0	2.0	350	2.4	2.38	DP21 FLOWBY, C&G FLOW TO DP22
																				5	10.2	0.01	BASIN C2 CAPTURED IN 15' TYPE R ONGRADE @ DP21, PIPE TO DP21.1
	21.1								17.7	3.23	5.49	17.7				17.7	3.23	2.0	2.0				COMBINED DP20 & DP21, PIPE TO DP22.1
	22	C3	2.47	0.59	9.4	1.46	7.09	10.3	9.4	1.67	7.09	11.8	2.0	0.29	1.0	9.8	1.38	2.0	2.0	820	2.0	6.83	DP22 FLOWBY, C&G FLOW TO DP23
																							DP21 FLOWBY & BASIN 3 CAPTURED IN 15' TYPE R ONGRADE @ DP22, PIPE TO DP22.1
	22.1								17.7	4.61	5.49	25.3				25.3	4.61	1.0	2.0	820	7.2	1.90	COMBINED DP21.1 & DP22, PIPE TO DP24.1
	23	C4	3.09	0.59	18.4	1.82	5.39	9.8	18.4	2.11	5.39	11.4	1.8	0.33	0.5	9.6	1.78	2.0	2.0	350	1.4	4.12	DP23 FLOWBY, C&G FLOW TO DP30
																				5	10.2	0.01	DP22 FLOWBY & BASIN C4 CAPTURED IN 15' TYPE R ONGRADE @ DP24, PIPE TO DP24.1
	24	C5	3.13	0.59	16.8	1.85	5.63	10.4	17.7	2.67	5.49	14.7	3.6	0.65	0.5	11.1	2.02	2.0	2.0	350	1.4	4.12	DP24 FLOWBY, C&G FLOW TO DP30
																				28	10.2	0.05	DP20 FLOWBY & BASIN C5 CAPTURED IN 15' TYPE R ONGRADE @ DP24, PIPE TO DP24.1
	24.1								19.6	8.42	5.23	44.1				44.1	8.42	2.0	2.0	53	10.2	0.09	COMBINED DP22.1, DP23 & DP24, PIPE TO DP29.1
	25	C6	0.81	0.52	13.2	0.42	6.23	2.6					2.6	0.42	0.8					605	1.8	5.64	BASIN C6 FLOW @ DP25, C&G FLOW TO DP26
	26	C7	6.07	0.59	23.7	3.58	4.76	17.0	23.7	4.00	4.76	22.0	8.2	1.49	1.5	13.8	2.51	2.0	2.0	1050	2.4	7.14	DP26 FLOWBY, C&G FLOW TO DP31
																				5	10.2	0.01	DP17, DP18, DP25 & BASIN C7 CAPTURED IN 15' TYPE R ONGRADE @ DP26, PIPE TO DP27.1
	27	C8	5.11	0.59	18.6	3.01	5.37	16.2					4.4	0.82	1.5	11.8	2.20	2.0	2.0	270	2.4	1.84	DP27 FLOWBY, C&G FLOW TO DP28
																				18	10.2	0.03	BASIN C8 FLOW CAPTURED IN 15' TYPE R ONGRADE, PIPE TO DP27.1
	27.1								23.7	4.70	4.76	25.4				25.4	4.70	2.0	2.0	265	10.2	0.43	COMBINED DP26 & DP27, PIPE TO DP28.1
	28	C9	3.50	0.59	15.2	2.07	5.87	12.1	20.4	2.88	5.13	17.8	5.4	0.87	1.5	12.4	2.01	2.0	2.0	780	2.4	5.31	DP28 FLOWBY, C&G FLOW TO DP30
																				30	10.2	0.05	DP27 FLOWBY & BASIN C9 CAPTURED IN 15' TYPE R ON GRADE @ DP28, PIPE TO DP28.1
	28.1								24.1	6.71	4.71	34.6				34.6	6.71	2.0	2.0	490	10.2	0.80	COMBINED DP27.1 & DP29, PIPE TO DP29.1
	29.1								24.9	15.13	4.63	73.0				73.0	15.13	2.0	2.0	335	10.2	0.55	COMBINED DP28.1 & DP24.1, PIPE TO DP30.1
	30	C10	3.97	0.61	16.5	2.41	5.67	13.6	25.7	4.26	4.55	22.4				22.4	4.26	2.0	2.0	30	10.2	0.05	DP23, DP24 & DP28 FLOWBY & BASIN C10 FLOW CAPTURED IN 20' TYPE R SUMP @ DP30, PIPE TO DP30.1
	30.1								25.8	19.39	4.55	91.1				91.1	19.39	2.0	2.0	5	10.2	0.01	COMBINED DP29.1 & DP20, PIPE TO DP31.1
	31	C11	3.64	0.61	15.5	2.21	5.82	12.9	30.8	3.70	4.09	18.2				18.2	3.70	2.0	2.0				DP26 FLOWBY & BASIN C11 CAPTURED IN 20' TYPE R SUMP @ DP31, PIPE TO DP31.1
	31.1								30.8	23.09	4.09	97.6				97.6	23.09	2.0	2.0	90	10.2	0.15	COMBINED DP30.1 & DP31, PIPE TO DP32.1
	32	C12	0.46	0.59	9.0	0.27	7.20	2.0								2.0	0.27	2.0	2.0				BASIN 32 CAPTURED IN 5' TYPE R SUMP, PIPE TO DP32.1
	32.1								31.0	23.37	4.08	98.4				98.4	23.37	2.0	2.0	32	10.2	0.05	COMBINED DP31.1 & DP32, PIPE TO DP33.1
	33	C13	1.66	0.59	11.7	0.98	6.54	6.4								6.4	0.98	2.0	2.0				BASIN C13 CAPTURED IN 5' TYPE R SUMP, PIPE TO DP33.1
	33.1								31.0	24.34	4.08	102.3				102.3	24.34	2.0	2.0	170	10.2	0.28	COMBINED DP32.1 & DP33, PIPE TO POND C
	34	C14	2.37	0.59	11.2	1.40	6.65	9.3	31.3	25.74	4.06	107.4											**TOTAL FLOW ENTERING POND C**
	35	C15	1.69	0.59	5.0	1.00	8.68	8.7															BASIN C15 FLOW, FOLLOWS HISTORIC DRAINAGE PATTERNS TOWARDS DP35

PRELIMINARY DRAINAGE REPORT

GRANDVIEW RESERVE FILING NO. 1

El Paso County, Colorado

PREPARED FOR:
D.R. Horton
9555 S. Kingston Court
Englewood, CO

PREPARED BY:
Galloway & Company, Inc.
1155 Kelly Johnson Blvd., Suite 305
Colorado Springs, CO 80920

DATE:
May 27, 2022

PCD Filing No.: PUDSP2110

COMPOSITE % IMPERVIOUS CALCULATIONS: EXISTING & PROPOSED

Subdivision: Grandview Reserve
Location: CO, El Paso County

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

1	2	3	4	5	6	7	8	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27
Basin ID	**Total Area (ac)**	**Paved/Gravel Roads**			**Lawns/Undeveloped**			**Residential - 1/8 Acre**			**Residential - 1/4 Acre**			**Residential - 1/3 Acre**			**Residential - 1/2 Acre**			**Residential - 1 Acre**			**Basins Total Weighted % Imp.**
		% Imp.	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	
EXISTING																							
See HR Green Rational Calcs Included, titled "Eastonville Road - Existing Conditions," for Western Off-site Sub-Basins																							
EX-1	16.18	100	0	0	2	16.18	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
EX-2	46.06	100	0	0	2	46.06	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
EX-3	64.34	100	0	0	2	64.34	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
EX-4	2.68	100	0	0	2	2.68	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
EX-5	26.15	100	0	0	2	26.15	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
EX-6	31.53	100	0	0	2	31.53	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
PROPOSED																							
Basin-1	1.22	100	0.98	80.3	2	0.24	0.4	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	80.7
See HR Green Rational Calcs Included, titled "Eastonville Road - Proposed Conditions," for Eastonville Road Sub-Basins EA-1, EA-2 & Western Off-site Sub-Basins																							
EA-3	0.94	100	0.00	0.0	2	0.94	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
A-1	11.67	100	0.00	0.0	2	11.67	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
A-2a	4.42	100	0.00	0.0	2	0.00	0.0	65.0	4.42	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
A-2b	2.75	100	1.80	65.5	2	0.00	0.0	65.0	0.95	22.5	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	88.0
A-3	0.36	100	0.36	100.0	2	0.00	0.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	100.0
A-4a	6.31	100	0.00	0.0	2	0.00	0.0	65.0	6.31	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
A-4b	3.99	100	0.00	0.0	2	0.00	0.0	65.0	3.99	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
A-5	0.35	100	0.35	100.0	2	0.00	0.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	100.0
A-6	2.76	100	0.00	0.0	2	0.00	0.0	65.0	2.76	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
A-7	0.23	100	0.23	100.0	2	0.00	0.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	100.0
A-8	5.44	100	4.06	74.5	2	1.39	0.5	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	75.0
A-9	4.91	100	0.00	0.0	2	0.00	0.0	65.0	4.91	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
A-10	1.02	100	0.00	0.0	2	0.00	0.0	65.0	1.02	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
A-11	3.56	100	0.00	0.0	2	2.77	1.6	65.0	0.79	14.4	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	16.0
B-1	3.81	100	0.00	0.0	2	0.00	0.0	65.0	3.33	56.8	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	56.8
B-2	4.62	100	0.00	0.0	2	0.00	0.0	65.0	4.51	63.5	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	63.5
B-3	4.15	100	0.00	0.0	2	0.00	0.0	65.0	4.15	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
B-4	1.37	100	1.07	78.1	2	0.30	0.4	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	78.5
B-5	5.12	100	0.00	0.0	2	0.00	0.0	65.0	5.12	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
B-6	2.28	100	0.00	0.0	2	0.00	0.0	65.0	2.28	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
B-7	0.89	100	0.00	0.0	2	0.00	0.0	65.0	0.89	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
B-8	3.23	100	0.00	0.0	2	0.00	0.0	65.0	3.23	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
B-9	2.42	100	0.00	0.0	2	0.00	0.0	65.0	2.42	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
B-10	1.10	100	0.00	0.0	2	1.10	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
C-1	4.12	100	0.00	0.0	2	0.00	0.0	65.0	4.12	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-2	2.71	100	0.00	0.0	2	0.00	0.0	65.0	2.71	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-3	1.56	100	0.08	5.1	2	1.48	1.9	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	7.0
C-4	2.47	100	0.00	0.0	2	0.00	0.0	65.0	2.47	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-5	3.09	100	0.00	0.0	2	0.00	0.0	65.0	3.09	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-6	2.1	100	0.00	0.0	2	0.00	0.0	65.0	2.10	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-7a	0.81	100	0.00	0.0	2	0.26	0.6	65.0	0.55	44.1	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	44.7
C-7b	5.91	100	0.00	0.0	2	0.00	0.0	65.0	5.91	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-8	5.11	100	0.00	0.0	2	0.00	0.0	65.0	5.11	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-9a	3.5	100	0.00	0.0	2	0.00	0.0	65.0	3.50	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-9b	3.69	100	0.00	0.0	2	0.00	0.0	65.0	3.69	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-10	3.47	100	0.00	0.0	2	0.00	0.0	65.0	3.47	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-11	0.46	100	0.00	0.0	2	0.00	0.0	65.0	0.46	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-12	1.66	100	0.00	0.0	2	0.00	0.0	65.0	1.66	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
C-13	2.37	100	0.00	0.0	2	2.37	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
C-14	1.53	100	0.00	0.0	2	1.53	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
C-15	0.16	100	0.01	6.3	2	0.15	1.9	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	8.2
D-1	3.48	100	0.00	0.0	2	0.00	0.0	65.0	3.48	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
D-2	0.87	100	0.00	0.0	2	0.00	0.0	65.0	0.87	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
D-3	3.62	100	0.00	0.0	2	0.00	0.0	65.0	3.62	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
D-4	1.77	100	0.00	0.0	2	0.00	0.0	65.0	1.77	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
D-5	1.53	100	0.00	0.0	2	0.71	0.9	65.0	0.82	34.8	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	35.7
D-6	0.83	100	0.00	0.0	2	0.83	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
D-7a	0.25	100	0.02	8.0	2	0.23	1.8	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	9.8
D-7b	0.88	100	0.00	0.0	2	0.00	0.0	65.0	0.88	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
E-1	5.33	100	0.00	0.0	2	0.00	0.0	65.0	5.33	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
E-2	5.42	100	0.00	0.0	2	0.00	0.0	65.0	5.42	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
E-3	3.20	100	0.00	0.0	2	0.00	0.0	65.0	3.20	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
E-4	6.28	100	0.00	0.0	2	0.00	0.0	65.0	6.28	65.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	65.0
E-5	1.13	100	0.00	0.0	2	1.13	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
E-6	0.74	100	0.00	0.0	2	0.74	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0

Lot Type Identification:	
Lot Size (SF)	*Lot Size (Acre)*
0 - 8,167	1/8 Acre
8,168 - 12,704	1/4 Acre
12,705 - 18,149	1/3 Acre
18,150 - 32,670	1/2 Acre
32,671 - 43,560	1 Acre

NOTES:
% Impervious values are taken directly from Table 6-6 in the Colorado Springs DCM Vol. 1. CH. 6 (Referencing UDFCD 2001)

COMPOSITE RUNOFF COEFFICIENT CALCULATIONS: EXISTING & PROPOSED

Subdivision: Grandview Reserve
Location: CO, El Paso County

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28
Basin ID	**Total Area (ac)**	**Paved/Gravel Roads**			**Lawns/Undeveloped**			**Roofs**			**Residential - 1/8 Acre**			**Residential - 1/4 Acre**			**Residential - 1/3 Acre**			**Residential - 1/2 Acre**			**Residential - 1 Acre**			**Composite C_5**	**Composite C_{100}**
		C_5	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**		
EXISTING																											
See HR Green Rational Calcs Included, titled "Eastonville Road - Existing Conditions," for Western Off-site Sub-Basins																											
EX-1	16.18	0.90	0.96	0.00	0.09	0.36	16.18	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
EX-2	46.06	0.90	0.96	0.00	0.09	0.36	46.06	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
EX-3	64.34	0.90	0.96	0.00	0.09	0.36	64.34	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
EX-4	2.68	0.90	0.96	0.00	0.09	0.36	2.68	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
EX-5	26.15	0.90	0.96	0.00	0.09	0.36	26.15	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
EX-6	31.53	0.90	0.96	0.00	0.09	0.36	31.53	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
PROPOSED																											
Basin-1	1.22	0.90	0.96	0.98	0.09	0.36	0.24	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.74	0.84
See HR Green Rational Calcs Included, titled "Eastonville Road - Proposed Conditions," for Eastonville Road Sub-Basins EA-1, EA-2 & Western Off-site Sub-Basins																											
EA-3	0.94	0.90	0.96	0.00	0.09	0.36	0.94	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
A-1	11.67	0.90	0.96	0.00	0.09	0.36	11.67	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
A-2a	4.42	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	4.42	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
A-2b	2.75	0.90	0.96	1.80	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	0.95	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.74	0.83
A-3	0.36	0.90	0.96	0.36	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.90	0.96
A-4a	6.31	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	6.31	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
A-4b	3.99	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.99	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
A-5	0.35	0.90	0.96	0.35	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.90	0.96
A-6	2.76	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	2.76	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
A-7	0.23	0.90	0.96	0.23	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.90	0.96
A-8	5.44	0.90	0.96	4.06	0.09	0.36	1.39	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.69	0.81
A-9	4.91	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	4.91	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
A-10	1.02	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	1.02	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
A-11	3.56	0.90	0.96	0.00	0.09	0.36	2.77	0.73	0.81	0.00	0.45	0.59	0.79	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.17	0.41
B-1	3.81	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.33	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.39	0.52
B-2	4.62	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	4.51	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.44	0.58
B-3	4.15	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	4.15	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
B-4	1.37	0.90	0.96	1.07	0.09	0.36	0.30	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.72	0.83
B-5	5.12	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	5.12	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
B-6	2.28	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	2.28	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
B-7	0.89	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	0.89	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
B-8	3.23	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.23	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
B-9	2.42	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	2.42	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
B-10	1.10	0.90	0.96	0.00	0.09	0.36	1.10	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
C-1	4.12	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	4.12	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-2	2.71	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	2.71	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-3	1.56	0.90	0.96	0.08	0.09	0.36	1.48	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.13	0.39
C-4	2.47	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	2.47	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-5	3.09	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.09	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-6	2.10	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	2.10	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-7a	0.81	0.90	0.96	0.00	0.09	0.36	0.26	0.73	0.81	0.00	0.45	0.59	0.55	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.33	0.52
C-7b	5.91	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	5.91	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-8	5.11	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	5.11	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-9a	3.50	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.50	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-9b	3.69	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.69	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-10	3.47	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.47	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-11	0.46	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	0.46	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-12	1.66	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	1.66	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
C-13	2.37	0.90	0.96	0.00	0.09	0.36	2.37	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
C-14	1.53	0.90	0.96	0.00	0.09	0.36	1.53	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
C-15	0.16	0.90	0.96	0.01	0.09	0.36	0.15	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.14	0.40
D-1	3.48	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.48	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
D-2	0.87	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	0.87	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
D-3	3.62	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.62	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
D-4	1.77	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	1.77	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
D-5	1.53	0.90	0.96	0.00	0.09	0.36	0.71	0.73	0.81	0.00	0.45	0.59	0.82	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.28	0.48
D-6	0.83	0.90	0.96	0.00	0.09	0.36	0.83	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
D-7a	0.25	0.90	0.96	0.02	0.09	0.36	0.23	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.15	0.41
D-7b	0.88	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	0.88	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
E-1	5.33	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	5.33	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
E-2	5.42	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	5.42	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
E-3	3.20	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	3.20	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
E-4	6.28	0.90	0.96	0.00	0.09	0.36	0.00	0.73	0.81	0.00	0.45	0.59	6.28	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.45	0.59
E-5	1.13	0.90	0.96	0.00	0.09	0.36	1.13	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
E-6	0.74	0.90	0.96	0.00	0.09	0.36	0.74	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36

Lot Type Identification:	
Lot Size (SF)	*Lot Size (Acre)*
0 - 8,167	<= 1/8 Acre
8,168 - 12,704	1/4 Acre
12,705 - 18,149	1/3 Acre
18,150 - 32,670	1/2 Acre
32,671 - 43,560	1 Acre

NOTES:
C values are taken directly from Table 6-6 in the Colorado Springs DCM Vol. 1. CH. 6 (Referencing UDFCD 2001)
Coeffficients use HSG A&B soils - Refer to "Appendix A: Exhibits and Figures" for soil map

STANDARD FORM SF-2: EXISTING & PROPOSED
TIME OF CONCENTRATION

Subdivision: Grandview Reserve
Location: CO, El Paso County

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					Tc CHECK (T_c)			FINAL
BASIN ID	D.A. (AC)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (FT)	S (%)	T_i (MIN)	L (FT)	S (%)	Cv	VEL. (FPS)	T_t (MIN)	COMP. T_c (MIN)	TOTAL LENGTH(FT)	Calculated T_c (MIN)	T_c (MIN)
EXISTING																	
See HR Green Rational Calcs Included, titled "Eastonville Road - Existing Conditions," for Western Off-site Sub-Basins																	
EX-1	16.18	A	2.0	0.09	0.36	300	3.3	21.6	1433	2.5	15	2.4	10.0	31.6	1732.7	19.6	31.6
EX-2	46.06	A	2.0	0.09	0.36	300	2.5	23.6	3127	2.0	15	2.1	24.7	48.3	3427.0	29.0	48.3
EX-3	64.34	A	2.0	0.09	0.36	300	3.2	21.7	3964	2.1	15	2.2	30.4	52.1	4263.6	33.7	52.1
EX-4	2.68	A	2.0	0.09	0.36	300	2.5	23.8	462	2.4	15	2.3	3.3	27.1	762.3	14.2	27.1
EX-5	26.15	A	2.0	0.09	0.36	300	3.1	22.1	2121	2.3	15	2.3	15.6	37.7	2420.8	23.4	37.7
EX-6	31.53	A	2.0	0.09	0.36	300	3.6	20.9	1488	2.1	15	2.2	11.4	32.3	1788.5	19.9	32.3
PROPOSED																	
Basin-1	1.22	A	80.7	0.74	0.84	46	2.0	3.5	556	1.8	20	2.7	3.5	7.0	602.0	13.3	7.0
See HR Green Rational Calcs Included, titled "Eastonville Road - Proposed Conditions," for Eastonville Road Sub-Basins EA-1, EA-2 & Western Off-site Sub-Basins																	
EA-3	0.94	A	2.0	0.09	0.36	22	25.0	3.0	285	3.0	20	3.5	1.4	4.3	307.0	11.7	5.0
A-1	11.67	A	2.0	0.09	0.36	50	10.0	6.1	957	5.0	20	4.5	3.6	9.6	1007.0	15.6	9.6
A-2a	4.42	A	65.0	0.45	0.59	50	5.0	4.9	742	2.5	20	3.2	3.9	8.8	792.0	14.4	8.8
A-2b	2.75	A	88.0	0.74	0.83	250	2.0	8.3	300	2.5	20	3.2	1.6	9.9	550.0	13.1	9.9
A-3	0.36	A	100.0	0.90	0.96	18	2.0	1.2	560	1.9	20	2.8	3.4	4.6	578.0	13.2	5.0
A-4a	6.31	A	65.0	0.45	0.59	230	2.0	14.3	700	2.5	20	3.2	3.7	18.0	930.0	15.2	15.2
A-4b	3.99	A	65.0	0.45	0.59	100	2.0	9.4	770	2.5	20	3.2	4.1	13.5	870.0	14.8	13.5
A-5	0.35	A	100.0	0.90	0.96	18	2.0	1.2	332	1.4	20	2.4	2.3	3.6	350.0	11.9	5.0
A-6	2.76	A	65.0	0.45	0.59	217	4.5	10.6	310	1.0	20	2.0	2.6	13.2	527.0	12.9	12.9
A-7	0.23	A	100.0	0.90	0.96	36	3.0	1.5	340	2.3	20	3.0	1.9	3.4	376.0	12.1	5.0
A-8	5.44	A	75.0	0.69	0.81	250	2.0	9.4	300	2.0	20	2.8	1.8	11.2	550.0	13.1	11.2
A-9	4.91	A	65.0	0.45	0.59	160	2.0	11.9	950	1.5	20	2.4	6.5	18.4	1110.0	16.2	16.2
A-10	1.02	A	65.0	0.45	0.59	18	3.0	3.5	450	1.0	20	2.0	3.8	7.3	468.0	12.6	7.3
A-11	3.56	A	16.0	0.17	0.41	450	5.0	21.1	718	1.0	20	2.0	6.0	27.1	1168.0	16.5	16.5
B-1	3.81	A	56.8	0.39	0.52	210	3.5	12.4	560	1.7	20	2.6	3.6	16.0	770.0	14.3	14.3
B-2	4.62	A	63.5	0.44	0.58	230	3.0	12.7	611	2.5	20	3.2	3.2	15.9	841.0	14.7	14.7
B-3	4.15	A	65.0	0.45	0.59	34	2.0	5.5	680	2.7	20	3.3	3.4	9.0	714.0	14.0	9.0
B-4	1.37	A	78.5	0.72	0.83	10	6.0	1.2	700	1.0	20	2.0	5.8	7.0	710.0	13.9	7.0
B-5	5.12	A	65.0	0.45	0.59	60	1.0	9.2	946	1.7	20	2.6	6.0	15.3	1006.0	15.6	15.3
B-6	2.28	A	65.0	0.45	0.59	186	3.0	11.3	480	1.0	20	2.0	4.0	15.3	666.0	13.7	13.7
B-7	0.89	A	65.0	0.45	0.59	62	3.0	6.5	509	1.0	20	2.0	4.2	10.7	571.0	13.2	10.7
B-8	3.23	A	65.0	0.45	0.59	177	5.0	9.3	700	2.0	20	2.8	4.1	13.4	877.0	14.9	13.4
B-9	2.42	A	65.0	0.45	0.59	152	3.0	10.2	800	2.4	20	3.1	4.3	14.5	952.0	15.3	14.5
B-10	1.10	A	2.0	0.09	0.36	66	25.0	5.1	187	1.0	20	2.0	1.6	6.7	253.0	11.4	6.7
C-1	4.12	A	65.0	0.45	0.59	65	3.0	6.7	1077	2.0	20	2.8	6.3	13.0	1142.0	16.3	13.0
C-2	2.71	A	65.0	0.45	0.59	55	2.0	7.0	620	1.9	20	2.8	3.7	10.8	675.0	13.8	10.8
C-3	1.56	A	7.0	0.13	0.39	77	4.0	9.8	0	0.0	20	0.0	0.0	9.8	77.0	10.4	9.8
C-4	2.47	A	65.0	0.45	0.59	194	2.0	13.2	345	1.3	20	2.3	2.5	15.7	539.0	13.0	13.0
C-5	3.09	A	65.0	0.45	0.59	38	4.0	4.6	761	1.0	20	2.0	6.3	11.0	799.0	14.4	11.0
C-6	2.10	A	65.0	0.45	0.59	61	3.0	6.4	1176	1.0	20	2.0	9.8	16.2	1236.5	16.9	16.2
C-7a	0.81	A	44.7	0.33	0.52	142	8.3	8.3	136	2.5	15	2.4	1.0	9.3	278.0	11.5	9.3
C-7b	5.91	A	65.0	0.45	0.59	35	4.0	4.4	1278	1.7	20	2.6	8.2	12.6	1313.0	17.3	12.6
C-8	5.11	A	65.0	0.45	0.59	58	2.0	7.2	834	1.6	20	2.5	5.5	12.7	892.0	15.0	12.7
C-9a	3.50	A	65.0	0.45	0.59	193	2.0	13.1	570	0.7	20	1.7	5.7	18.8	763.0	14.2	14.2
C-9b	3.69	A	65.0	0.45	0.59	160	3.0	10.4	665	2.0	20	2.8	3.9	14.4	825.0	14.6	14.4
C-10	3.47	A	65.0	0.45	0.59	122	3.0	9.1	1084	1.5	20	2.4	7.4	16.5	1206.0	16.7	16.5
C-11	0.46	A	65.0	0.45	0.59	26	2.0	4.8	152	0.5	20	1.4	1.8	6.6	178.0	11.0	6.6
C-12	1.66	A	65.0	0.45	0.59	160	4.0	9.5	200	0.5	20	1.4	2.4	11.8	360.0	12.0	11.8
C-13	2.37	A	2.0	0.09	0.36	225	15.0	11.3	352	1.0	20	2.0	2.9	14.2	577.0	13.2	13.2
C-14	1.53	A	2.0	0.09	0.36	300	5.0	18.7	0	0.0	10	0.0	0.0	18.7	300.0	11.7	11.7
C-15	0.16	A	8.2	0.14	0.40	72	5.0	8.7	0	0.0	20	0.0	0.0	8.7	72.0	10.4	8.7
D-1	3.48	A	65.0	0.45	0.59	170	3.0	10.8	715	1.0	20	2.0	6.0	16.7	885.0	14.9	14.9
D-2	0.87	A	65.0	0.45	0.59	10	2.0	3.0	700	1.3	20	2.3	5.1	8.1	710.0	13.9	8.1
D-3	3.62	A	65.0	0.45	0.59	140	3.0	9.8	660	2.2	20	3.0	3.7	13.5	800.0	14.4	13.5
D-4	1.77	A	65.0	0.45	0.59	50	3.0	5.8	663	2.0	20	2.8	3.9	9.7	713.0	14.0	9.7
D-5	1.53	A	35.7	0.28	0.48	110	25.0	5.4	201	1.0	20	2.0	1.7	7.1	311.0	11.7	7.1
D-6	0.83	A	2.0	0.09	0.36	300	5.0	18.7	0	0.0	10	0.0	0.0	18.7	300.0	11.7	11.7
D-7a	0.25	A	9.8	0.15	0.41	75	5.0	8.8	0	0.0	20	0.0	0.0	8.8	75.0	10.4	8.8
D-7b	0.88	A	65.0	0.45	0.59	75	8.0	5.2	478	2.0	15	2.1	3.8	8.9	553.0	13.1	8.9
E-1	5.33	A	65.0	0.45	0.59	25	4.0	3.7	1360	3.3	20	3.6	6.2	10.0	1385.0	17.7	10.0
E-2	5.42	A	65.0	0.45	0.59	20	2.0	4.2	1250	3.5	20	3.7	5.6	9.8	1270.0	17.1	9.8
E-3	3.20	A	65.0	0.45	0.59	10	2.0	3.0	965	1.5	20	2.4	6.6	9.6	975.0	15.4	9.6
E-4	6.28	A	65.0	0.45	0.59	305	7.0	10.9	1125	1.6	20	2.5	7.4	18.3	1430.0	17.9	17.9
E-5	1.13	A	2.0	0.09	0.36	127	25.0	7.1	315	1.0	20	2.0	2.6	9.8	442.0	12.5	9.8
E-6	0.74	A	2.0	0.09	0.36	350	2.0	27.5	113	2.0	10	1.4	1.3	28.8	463.0	12.6	12.6

NOTES:

$T_i = (0.395*(1.1 - C_5)*(L)^0.5)/((S)^0.33)$, S in ft/ft

T_t=L/60V (Velocity From Fig. 501)

Velocity V=Cv*S^0.5, S in ft/ft

Tc Check = 10+L/180

For Urbanized basins a minimum T_c of 5.0 minutes is required.

For non-urbanized basins a minimum T_c of 10.0 minutes is required

STANDARD FORM SF-3: EXISTING & PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve
Location: CO, El Paso County
Design Storm: 5-Year

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

STREET	Design Point	DIRECT RUNOFF: Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	TOTAL RUNOFF: Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	STREET: Slope (%)	Street Flow (cfs)	PIPE: Design Flow (cfs)	Slope (%)	Pipe Size (inches)	TRAVEL TIME: Length (ft)	Velocity (fps)	Tt (min)	REMARKS
EXISTING																					
	1	EX-1	16.18	0.09	31.6	1.46	2.35	3.4				4.7									Sheet flow to Main Stem Channel Total Flow from DP 10, DP 11 & Basin EX-1
	2	EX-2	46.06	0.09	48.3	4.15	1.82	7.6				79.1									Sheet flow to Main Stem Channel Total Flow from DP 8, DP 9 & Basin EX-2
	3	EX-3	64.34	0.09	52.1	5.79	1.73	10.0				10.0									Sheet flow offiste - outfalls to Main Stem Tributary #2 Channel
	4	EX-4	2.68	0.09	27.1	0.24	2.57	0.6				0.6									Sheet flow offiste - outfalls to Main Stem Tributary #2 Channel
	5	EX-5	26.15	0.09	37.7	2.35	2.12	5.0				5.0									Sheet flow offiste - outfalls to Main Stem Tributary #2 Channel
	6	EX-6	31.53	0.09	32.3	2.84	2.32	6.6				14.6									Sheet flow offiste - outfalls to Main Stem Tributary #2 Channel Total Flow from DP 7 & EX-6
See HR Green Rational Calcs Included, titled "Eastonville Road - Existing Conditions," for Western Off-site Sub-Basins																					
	12											30.2									Total Existing Flow offsite - outfalls to Main Stem Tributary #2 Channel
PROPOSED																					
		Basin-1	1.22	0.74	7.0	0.90	4.64	4.2				4.2									East Leg of Rex Road Intersection
See HR Green Rational Calcs Included, titled "Eastonville Road - Proposed Conditions," for Eastonville Road Sub-Basins EA-1, EA-2 & Western Off-site Sub-Basins																					
		EA-3	0.94	0.09	5.0	0.08	5.10	0.4													Eastonville Road Pond
	1	A-1	11.67	0.09	9.6	1.05	4.16	4.4				4.4									Institutional Tract Basin will have own water quality & detention pond
	2a	A-2a	4.42	0.45	8.8	1.99	4.29	8.5				8.5									On-Grade 15' CDOT Type R Inlet (0.6 cfs bypass to DP 2b)
	2b	A-2b	2.75	0.74	9.9	2.04	4.13	8.4				9.0									Sump 20' CDOT Type R Inlet (Receives 0.6 cfs upstream bypass)
	3	A-3	0.36	0.90	5.0	0.32	5.10	1.6				1.6									Sump 5' CDOT Type R Inlet
	4a	A-4a	6.31	0.45	15.2	2.84	3.44	9.8				9.8									On-Grade 15' CDOT Type R Inlet (1.2 cfs bypass to DP 4)
	4b 4	A-4b	3.99	0.45	13.5	1.80	3.63	6.5				6.5 2.5									On-Grade 15' CDOT Type R Inlet (1.3 cfs bypass to DP 4) Sump 15' CDOT Type R Inlet (Receives 2.5 cfs upstream bypass)
	5	A-5	0.35	0.90	5.0	0.32	5.10	1.6				1.6									Sump 5' CDOT Type R Inlet
	6	A-6	2.76	0.45	12.9	1.24	3.70	4.6				4.6									On-Grade 10' CDOT Type R Inlet (0.4 cfs bypass to DP 7a)
	7	A-7	0.23	0.90	5.0	0.21	5.10	1.1				1.1									On-Grade 5' CDOT Type R Inlet (0.1 cfs bypass to DP 7b)
	8	A-8	5.44	0.69	11.2	3.75	3.93	14.7				14.7									Proposed Amenitity Center - Assumed 75% Imperviousness
	7a	A-9	4.91	0.45	16.2	2.21	3.34	7.4				7.8									Sump 20' CDOT Type R Inlet (Receives 0.4 cfs upstream bypass)
	7b	A-10	1.02	0.45	7.3	0.46	4.59	2.1				2.2									Sump 5' CDOT Type R Inlet (Receives 0.1 cfs upstream bypass)
	8a	A-11	3.56	0.17	16.5	0.61	3.31	2.0	16.5	17.79	3.31	58.9									Total of Flows to Pond A
	9	B-1	3.81	0.39	14.3	1.49	3.54	5.3				5.3									Sump 15' CDOT Type R Inlet
	10a	B-2	4.62	0.44	14.7	2.03	3.50	7.1				7.1									On-Grade 10' CDOT Type R Inlet (1.6 cfs bypass to DP 10b)
	10b	B-3	4.15	0.45	9.0	1.87	4.27	8.0				9.6									Sump 20' CDOT Type R Inlet (Receives 1.6 cfs of upstream bypass)
	11	B-4	1.37	0.72	7.0	0.99	4.63	4.6				4.6									Sump 15' CDOT Type R Inlet
	12a	B-5	5.12	0.45	15.3	2.30	3.43	7.9				7.9									On-Grade 10' CDOT Type R Inlet (2.0 cfs bypass to DP 12b)

STANDARD FORM SF-3: EXISTING & PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve
Location: CO, El Paso County
Design Storm: 5-Year

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

		DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
	14	B-6	2.28	0.45	13.7	1.03	3.61	3.7				3.7									On-Grade 10' CDOT Type R Inlet (0.1 cfs bypass to DP 12b)
	15	B-7	0.89	0.45	10.7	0.40	3.99	1.6				1.6									On-Grade 10' CDOT Type R Inlet (0.0 cfs bypass to DP 12b)
	12b	B-8	3.23	0.45	13.4	1.45	3.64	5.3				7.4									Sump 20' CDOT Type R Inlet (Receives 2.1 cfs of upstream bypass)
	13	B-9	2.42	0.45	14.5	1.09	3.52	3.8				3.8									Sump 10' CDOT Type R Inlet
	16	B-10	1.10	0.09	6.7	0.10	4.70	0.5	15.3	12.75	3.43	43.7									Total of flows to Pond B
	17b	C-1	4.12	0.45	13.0	1.85	3.69	6.8				6.8									On-Grade 15' CDOT Type R (0.1 cfs bypass to DP 17e)
	17a	C-2	2.71	0.45	10.8	1.22	3.99	4.9				4.9									On-Grade 15' CDOT Type R (1.7 cfs bypass to DP 17c)
	17c	C-4	2.47	0.45	13.0	1.11	3.69	4.1				5.8									Receives 1.7 cfs of Bypass from DP 17a On-Grade 15' CDOT Type R (0.0 cfs bypass to DP 17d)
	17d	C-5	3.09	0.45	11.0	1.39	3.96	5.5				5.5									Receives 0.0 cfs of Bypass from DP 17c On-Grade 15' CDOT Type R (0.0 cfs bypass to DP 17h)
	17e	C-6	2.10	0.45	16.2	0.95	3.34	3.2				3.3									Receives 0.1 cfs of Bypass from DP 17b On-Grade 15' CDOT Type R (0.0 cfs bypass to DP 17h)
	17f	C-8	5.11	0.45	12.7	2.30	3.73	8.6				8.6									On-Grade 15' CDOT Type R (0.6 cfs bypass to DP 17g)
	17g	C-9a	3.50	0.45	14.2	1.58	3.54	5.6				6.2									Receives 0.6 cfs of Bypass from DP 17f On-Grade 15' CDOT Type R (0.0 cfs bypass to DP 17h)
	17h	C-9b	3.69	0.45	14.4	1.66	3.53	5.9				5.9									Sump 20' CDOT Type R (Receives 0.0 cfs of upstream bypass)
	18a	C-7a	0.81	0.33	9.3	0.27	4.22	1.1				1.1									Drainage Swale/SW Chase - Flows to DP 18b
	18b	C-7b	5.91	0.45	12.6	2.66	3.74	9.9	12.6	2.93	3.74	11.0									On-Grade 15' CDOT Type R (1.6 cfs bypass to DP 18c)
	18c	C-10	3.47	0.45	16.5	1.56	3.31	5.2				6.9									Sump 15' CDOT Type R (Receives 1.6 cfs of upstream bypass)
	19	C-11	0.46	0.45	6.6	0.21	4.72	1.0				1.0									Sump 5' CDOT Type R (Receives 0.0 cfs of upstream bypass)
	20	C-12	1.66	0.45	11.8	0.75	3.84	2.9				2.9									Sump 5' CDOT Type R (Receives 0.0 cfs of upstream bypass)
	21	C-13	2.37	0.09	13.2	0.21	3.66	0.8	16.5	17.72	3.31	58.7									Total combined flows to Pond C
		C-3	1.56	0.13	9.8	0.20	4.13	0.8													Back of Lots 409-426 - Sheet Flows to MS 2
		C-14	1.53	0.09	11.7	0.14	3.86	0.5													Un-developed area - Sheet flows to MS 2
		C-15	0.16	0.14	8.7	0.02	4.31	0.1													Portion of Lot 444 - Sheet flows to MS 2
	22	D-1	3.48	0.45	14.9	1.57	3.47	5.4				5.4									On-Grade 10' CDOT Type R Inlet (0.7 cfs bypass to DP 24)
	23	D-2	0.87	0.45	8.1	0.39	4.42	1.7				1.7									On-Grade 10' CDOT Type R Inlet (0.0 cfs bypass to DP 24)
	24	D-3	3.62	0.45	13.5	1.63	3.63	5.9				6.6									Receives 0.4 cfs of upstream bypass Sump 15' CDOT Type R Inlet
	25	D-4	1.77	0.45	9.7	0.80	4.14	3.3				3.3									Sump 10' CDOT Type R Inlet
	25a	D-7b	0.88	0.45	8.9	0.40	4.28	1.7				1.7									Sheet flows to Channel and Conveyed to Pond D
	26	D-5	1.53	0.28	7.1	0.43	4.63	2.0	14.9	5.22	3.47	18.1									Total of flows to Pond D
		D-6	0.83	0.09	11.7	0.07	3.86	0.3													Un-developed area - Sheet flows to MS
		D-7a	0.25	0.15	8.8	0.04	4.30	0.2													Back of Lots 18-20 - Sheet Flows to MST

STANDARD FORM SF-3: EXISTING & PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve
Location: CO, El Paso County
Design Storm: 5-Year

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

		DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
	27	E-1	5.33	0.45	10.0	2.40	4.10	9.8				9.8									On-Grade 15' CDOT Type R Inlet (0.9 cfs bypass to DP 29)
	28	E-2	5.42	0.45	9.8	2.44	4.13	10.1				10.1									On-Grade 15' CDOT Type R Inlet (1.2 cfs bypass to DP 29)
	29	E-3	3.20	0.45	9.6	1.44	4.17	6.0				8.1									Receives 2.1 cfs of upstream bypass Sump 15' CDOT Type R Inlet
	30	E-4	6.28	0.45	17.9	2.83	3.18	9.0				9.0									Sump 20' CDOT Type R Inlet
	31	E-5	1.13	0.09	9.8	0.10	4.14	0.4	17.9	9.21	3.18	29.3									Total of flows to Pond E
		E-6	0.74	0.09	12.6	0.07	3.74	0.3													Un-developed area - Sheet flows to MS

STANDARD FORM SF-3: EXISTING & PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve
Location: CO, El Paso County
Design Storm: 100-Year

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
EXISTING																					
	1	EX-1	16.18	0.36	31.6	5.82	4.19	24.4				33.3									Sheet flow to Main Stem Channel Total Flow from DP 10, DP 11 & Basin EX-1
	2	EX-2	46.06	0.36	48.3	16.58	3.24	53.7				497.2									Sheet flow to Main Stem Channel Total Flow from DP 8, DP 9 & Basin EX-2
	3	EX-3	64.34	0.36	52.1	23.16	3.09	71.6				71.6									Sheet flow offiste - outfalls to Main Stem Tributary #2 Channel
	4	EX-4	2.68	0.36	27.1	0.96	4.57	4.4				4.4									Sheet flow offiste - outfalls to Main Stem Tributary #2 Channel
	5	EX-5	26.15	0.36	37.7	9.41	3.77	35.5				35.5									Sheet flow offiste - outfalls to Main Stem Tributary #2 Channel
	6	EX-6	31.53	0.36	32.3	11.35	4.13	46.9				584.9									Sheet flow offiste - outfalls to Main Stem Tributary #2 Channel Total Flow from DP 7 & EX-6
See HR Green Rational Calcs Included, titled "Eastonville Road - Existing Conditions," for Western Off-site Sub-Basins																					
	12											696.3									Total Existing Flow offsite - outfalls to Main Stem Tributary #2 Channel
PROPOSED																					
		Basin-1	1.22	0.84	7.0	1.02	8.26	8.4				8.4									East Leg of Rex Road Intersection
See HR Green Rational Calcs Included, titled "Eastonville Road - Proposed Conditions," for Eastonville Road Sub-Basins EA-1, EA-2 & Western Off-site Sub-Basins																					
		EA-3	0.94	0.36	5.0	0.34	9.09	3.1													Eastonville Road Pond
	1	A-1	11.67	0.36	9.6	4.20	7.40	31.1				31.1									Institutional Tract Basin will have own water quality & detention pond
	2a	A-2a	4.42	0.59	8.8	2.61	7.64	19.9				19.9									On-Grade 15' CDOT Type R Inlet (7.0 cfs bypass to DP 2b)
	2b	A-2b	2.75	0.83	9.9	2.28	7.34	16.7				23.7									Sump 20' CDOT Type R Inlet (Receives 7.0 cfs upstream bypass)
	3	A-3	0.36	0.96	5.0	0.35	9.09	3.2				3.2									Sump 5' CDOT Type R Inlet
	4a	A-4a	6.31	0.59	15.2	3.72	6.13	22.8				22.8									On-Grade 15' CDOT Type R Inlet (9.0 cfs bypass to DP 4)
	4b 4	A-4b	3.99	0.59	13.5	2.35	6.46	15.2				15.2 16.1									On-Grade 15' CDOT Type R Inlet (7.1 cfs bypass to DP 4) Sump 15' CDOT Type R Inlet (Receives 16.1 cfs upstream bypass)
	5	A-5	0.35	0.96	5.0	0.34	9.09	3.1				3.1									Sump 5' CDOT Type R Inlet
	6	A-6	2.76	0.59	12.9	1.63	6.58	10.7				10.7									On-Grade 10' CDOT Type R Inlet (3.8 cfs bypass to DP 7a)
	7	A-7	0.23	0.96	5.0	0.22	9.09	2.0				2.0									On-Grade 5' CDOT Type R Inlet (0.4 cfs bypass to DP 7b)
	8	A-8	5.44	0.81	11.2	4.41	6.99	30.8				30.8									Proposed Amenitity Center - Assumed 75% Imperviousness
	7a	A-9	4.91	0.59	16.2	2.90	5.95	17.3				21.1									Sump 20' CDOT Type R Inlet (Receives 3.8 cfs upstream bypass)
	7b	A-10	1.02	0.59	7.3	0.60	8.17	4.9				5.3									Sump 5' CDOT Type R Inlet (Receives 0.4 cfs upstream bypass)
	8a	A-11	3.56	0.41	16.5	1.46	5.90	8.6	16.5	22.87	5.90	134.9									Total of Flows to Pond A
	9	B-1	3.81	0.52	14.3	1.98	6.30	12.5				12.5									Sump 15' CDOT Type R Inlet
	10a	B-2	4.62	0.58	14.7	2.68	6.22	16.7				16.7									On-Grade 10' CDOT Type R Inlet (8.3 cfs bypass to DP 10b)
	10b	B-3	4.15	0.59	9.0	2.45	7.61	18.6				26.9									Sump 20' CDOT Type R Inlet (Receives 8.3 cfs of upstream bypass)
	11	B-4	1.37	0.83	7.0	1.14	8.25	9.4				9.4									Sump 15' CDOT Type R Inlet
	12a	B-5	5.12	0.59	15.3	3.02	6.11	18.5				18.5									On-Grade 10' CDOT Type R Inlet (9.5 cfs bypass to DP 12b)

STANDARD FORM SF-3: EXISTING & PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve
Location: CO, El Paso County
Design Storm: 100-Year

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

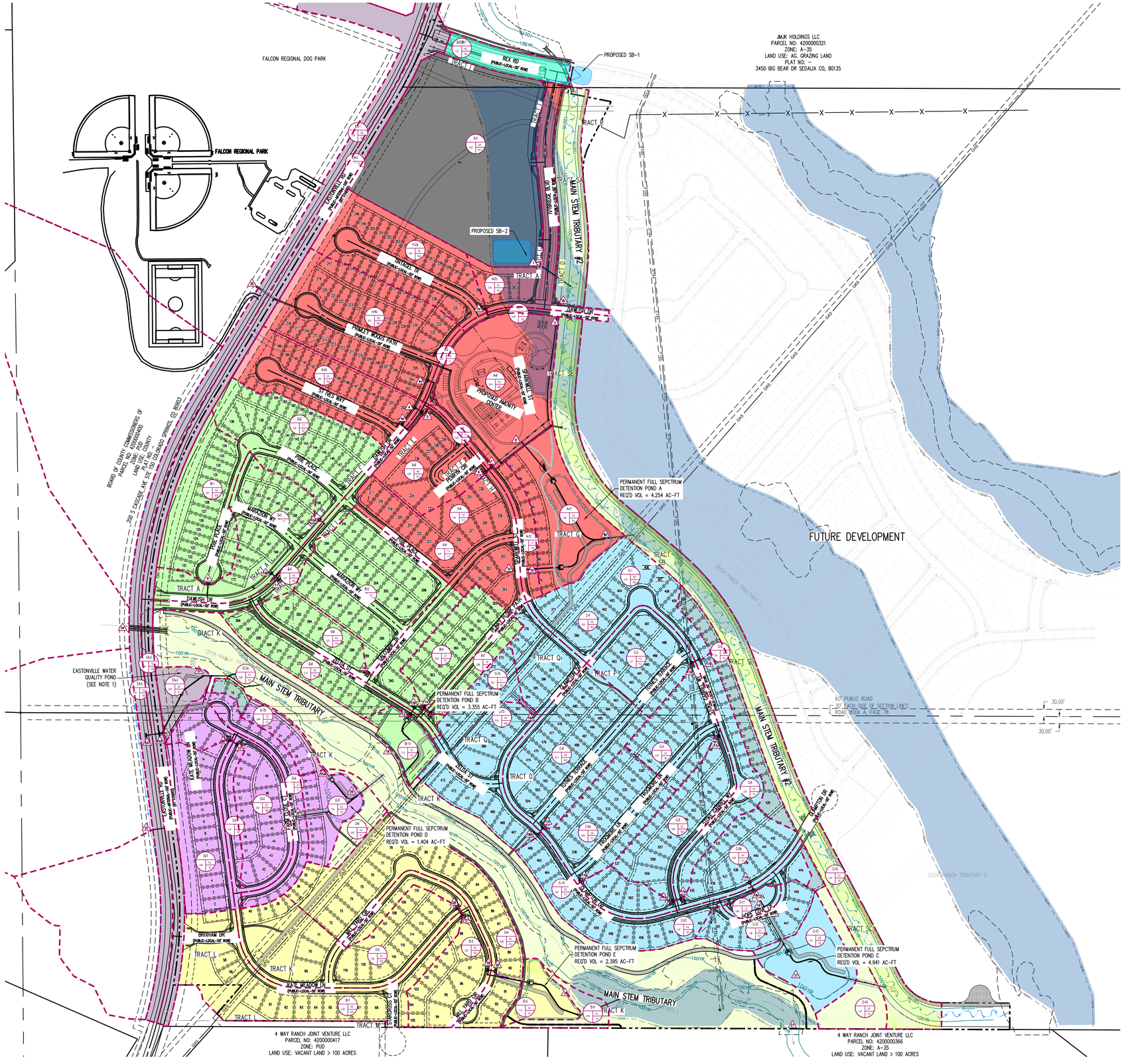
		DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
	14	B-6	2.28	0.59	13.7	1.35	6.42	8.7				8.7									On-Grade 10' CDOT Type R Inlet (2.5 cfs bypass to DP 12b)
	15	B-7	0.89	0.59	10.7	0.53	7.10	3.8				3.8									On-Grade 10' CDOT Type R Inlet (0.1 cfs bypass to DP 12b)
	12b	B-8	3.23	0.59	13.4	1.91	6.48	12.4				24.5									Sump 20' CDOT Type R Inlet (Receives 12.1 cfs of upstream bypass)
	13	B-9	2.42	0.59	14.5	1.43	6.26	9.0				9.0									Sump 10' CDOT Type R Inlet
	16	B-10	1.10	0.36	6.7	0.40	8.37	3.3	15.3	16.89	6.11	103.2									Total of flows to Pond B
	17b	C-1	4.12	0.59	13.0	2.43	6.57	16.0				16.0									On-Grade 15' CDOT Type R (4.3 cfs bypass to DP 17e)
	17a	C-2	2.71	0.59	10.8	1.60	7.10	11.4				11.4									On-Grade 15' CDOT Type R (11.2 cfs bypass to DP 17c)
	17c	C-4	2.47	0.59	13.0	1.46	6.57	9.6				20.8									Receives 11.2 cfs of Bypass from DP 17a On-Grade 15' CDOT Type R (7.4 cfs bypass to DP 17d)
	17d	C-5	3.09	0.59	11.0	1.82	7.04	12.8				20.2									Receives 7.4 cfs of Bypass from DP 17c On-Grade 15' CDOT Type R (7.0 cfs bypass to DP 17h)
	17e	C-6	2.10	0.59	16.2	1.24	5.94	7.4				11.7									Receives 4.3 cfs of Bypass from DP 17b On-Grade 15' CDOT Type R (2.0 cfs bypass to DP 17h)
	17f	C-8	5.11	0.59	12.7	3.01	6.63	20.0				20.0									On-Grade 15' CDOT Type R (6.9 cfs bypass to DP 17g)
	17g	C-9a	3.50	0.59	14.2	2.07	6.31	13.1				20.0									Receives 6.9 cfs of Bypass from DP 17f On-Grade 15' CDOT Type R (6.8 cfs bypass to DP 17h)
	17h	C-9b	3.69	0.59	14.4	2.18	6.29	13.7				29.5									Sump 20' CDOT Type R (Receives 15.8 cfs of upstream bypass)
	18a	C-7a	0.81	0.52	9.3	0.42	7.51	3.2				3.2									Drainage Swale/SW Chase - Flows to DP 18b
	18b	C-7b	5.91	0.59	12.6	3.49	6.65	23.2	12.6	3.91	6.65	26.0									On-Grade 15' CDOT Type R (11.3 cfs bypass to DP 18c)
	18c	C-10	3.47	0.59	16.5	2.05	5.90	12.1				23.3									Sump 15' CDOT Type R (Receives 11.3 cfs of upstream bypass)
	19	C-11	0.46	0.59	6.6	0.27	8.41	2.3				2.3									Sump 5' CDOT Type R (Receives 0.0 cfs of upstream bypass)
	20	C-12	1.66	0.59	11.8	0.98	6.83	6.7				6.7									Sump 5' CDOT Type R (Receives 0.0 cfs of upstream bypass)
	21	C-13	2.37	0.36	13.2	0.85	6.52	5.5	16.5	23.87	5.90	140.8									Total combined flows to Pond C
		C-3	1.56	0.39	9.8	0.61	7.35	4.5													Back of Lots 409-426 - Sheet Flows to MS 2
		C-14	1.53	0.36	11.7	0.55	6.87	3.8													Un-developed area - Sheet flows to MS 2
		C-15	0.16	0.40	8.7	0.06	7.68	0.5													Portion of Lot 444 - Sheet flows to MS 2
	22	D-1	3.48	0.59	14.9	2.05	6.18	12.7				12.7									On-Grade 10' CDOT Type R Inlet (5.2 cfs bypass to DP 24)
	23	D-2	0.87	0.59	8.1	0.51	7.88	4.0				4.0									On-Grade 10' CDOT Type R Inlet (0.2 cfs bypass to DP 24)
	24	D-3	3.62	0.59	13.5	2.14	6.46	13.8				19.2									Receives 5.4 cfs of upstream bypass Sump 15' CDOT Type R Inlet
	25	D-4	1.77	0.59	9.7	1.04	7.37	7.7				7.7									Sump 10' CDOT Type R Inlet
	25a	D-7b	0.88	0.59	8.9	0.52	7.62	4.0				4.0									Sheet flows to Channel and Conveyed to Pond D
	26	D-5	1.53	0.48	7.1	0.73	8.24	6.0	14.9	6.99	6.18	43.2									Total of flows to Pond D
		D-6	0.83	0.36	11.7	0.30	6.87	2.1													Un-developed area - Sheet flows to MS
		D-7a	0.25	0.41	8.8	0.10	7.65	0.8													Back of Lots 18-20 - Sheet Flows to MST

STANDARD FORM SF-3: EXISTING & PROPOSED
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve
Location: CO, El Paso County
Design Storm: 100-Year

Project Name: Grandview Subdivision PDR
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 5/26/22

		DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
[redacted]	27	E-1	5.33	0.59	10.0	3.14	7.30	22.9				22.9									On-Grade 15' CDOT Type R Inlet (8.8 cfs bypass to DP 29)
	28	E-2	5.42	0.59	9.8	3.20	7.36	23.6				23.6									On-Grade 15' CDOT Type R Inlet (9.3 cfs bypass to DP 29)
	29	E-3	3.20	0.59	9.6	1.89	7.43	14.0				32.1									Receives 18.1 cfs of upstream bypass Sump 15' CDOT Type R Inlet
	30	E-4	6.28	0.59	17.9	3.71	5.66	21.0				21.0									Sump 20' CDOT Type R Inlet
	31	E-5	1.13	0.36	9.8	0.41	7.37	3.0	17.9	12.35	5.66	69.9									Total of flows to Pond E
		E-6	0.74	0.36	12.6	0.27	6.66	1.8													Un-developed area - Sheet flows to MS



DRAINAGE LEGEND

- PROPERTY LINE
- PROPOSED ROAD CENTERLINE
- BASIN BOUNDARY LINE
- EXISTING WETLANDS
- EXISTING LIMITS OF WETLAND
- EXISTING WETLAND SETBACK
- EXISTING FEMA FLOOD PLAIN, ZONE A
- CENTERLINE OF STREAM
- PROPOSED RIPRAP
- PROPOSED MAINTENANCE ACCESS
- BASIN DESIGNATION
- 5-YEAR RUNOFF IN CUBIC FEET PER SECOND
- 100-YEAR RUNOFF IN CUBIC FEET PER SECOND
- BASIN AREA IN ACRES
- FUTURE DEVELOPMENT (NOT PART – TSB PROVIDED)
- ROADWAY (DESIGN BY OTHERS – SEE NOTE 1)
- AREA TO BE DETAINED IN PBMP (POND A)
- AREA TO BE DETAINED IN PBMP (POND B)
- AREA TO BE DETAINED IN PBMP (POND C)
- AREA TO BE DETAINED IN PBMP (POND D)
- AREA TO BE DETAINED IN PBMP (POND E)
- AREA NOT TO BE DETAINED IN PBMP PER SECTION I.7.1.B.7 (LAND DISTURBANCE TO UNDEVELOPED LAND THAT WILL REMAIN UNDEVELOPED)
- AREA TO BE DETAINED IN FUTURE PBMP WITH THE REMAINDER OF THE REX RD DEVELOPMENT – TSB PROVIDED
- AREA WHERE WATER QUALITY IS ACHIEVED THROUGH RUNOFF REDUCTION (ROOF DRAINS & IMPERVIOUS SURFACES NOT PERMITTED ON BACK OF THESE LOTS)
- AREA TO BE DETAINED IN PUBLIC PBMP (EASTONVILLE ROAD)

NOTES:

1. EASTONVILLE ROAD AND THE WESTERN PORTION OF REX ROAD AT EASTONVILLE IS TO BE CONSTRUCTED CONCURRENTLY WITH THIS DEVELOPMENT. DRAINAGE FOR THESE AREAS WILL BE CAPTURED AND TREATED BY OFFSITE INFRASTRUCTURE AND PONDS AS DESCRIBED IN THE "FINAL DRAINAGE REPORT FOR EASTONVILLE ROAD FROM FUTURE REX ROAD TO LONDONDERRY DRIVE", BY HR GREEN, MARCH 2022. BASIN SHOWN ARE FOR REFERENCE ONLY.

Galloway
1155 Kelly Johnson Blvd., Suite 305
Colorado Springs, CO 80920
719.900.7220
GallowayUS.com

PRELIMINARY
NOT FOR BIDDING
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PRELIMINARY DRAINAGE PLAN
GRANDVIEW RESERVE FILING NO. 1
FOR
HR GREEN, INC

EASTONVILLE RD
EL PASO COUNTY, PEYTON, CO 80831

#	Date	Issue / Description	Init.

Project No:	HRG 1.20
Drawn By:	NJA
Checked By:	GRD
Date:	5/27/2022

WQ MAP

DR-4

M:\HR Green\CO\...\Grandview\800000\...\DrawingSheets\DR-4 Grandview Reserve Filing No 1 Drainage Plan.dwg, DR-4 Water Quality Exhibit, 5/27/2022

APPENDIX F – DRAINAGE MAPS

MARSHALL, CLARK, 3/20/2024 3:55 PM

OFFSITE CONTRIBUTION
Q100 ~ 280 CFS
Q5 ~ 59 CFS
OFFSITE AREA ~0.44 SQUARE MILES
CN VALUE~56.49
SEE STREAMSTATS MAP

OFFSITE CONTRIBUTION
Q100 ~ 217 CFS
Q5 ~ 61 CFS
OFFSITE AREA ~0.22 SQUARE MILES
CN VALUE~54.53
SEE STREAMSTATS MAP

OFFSITE CONTRIBUTION
Q100 ~ 595 CFS
Q5 ~ 180 CFS
OFFSITE AREA ~0.84 SQUARE MILES
CN VALUE~58.28
SEE STREAMSTATS MAP

OFFSITE CONTRIBUTION
Q100 ~ 413 CFS
Q5 ~ 67 CFS
OFFSITE AREA ~0.17 SQUARE MILES
CN VALUE ~55.04
SEE STREAMSTATS MAP

EASTONVILLE ROAD

EAST FORK

EAST FORK TRIBUTARY

PHASE 2 BOUNDARY

PHASE 3 BOUNDARY

MAIN STEM TRIBUTARY NUMBER 2

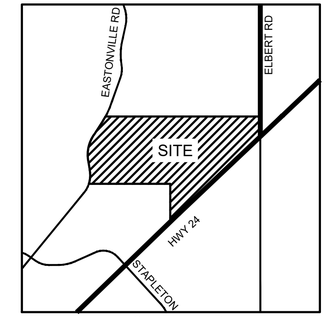
MAIN STEM

STATE HIGHWAY 24

EXISTING 24" CMP CULVERT

EXISTING 18" CMP CULVERT

EXISTING BRIDGE



VICINITY MAP

LEGEND:

- PROPOSED MAJOR CONTOUR — 5250
- PROPOSED MINOR CONTOUR
- EXISTING MAJOR CONTOUR — 5250
- EXISTING MINOR CONTOUR
- PROPOSED STORM DRAIN PIPE
- EXISTING STORM DRAIN PIPE
- PROPOSED DRAINAGE CHANNEL
- PROPOSED ROAD
- PROPERTY LINE
- DIRECTIONAL FLOW ARROW
- EMERGENCY OVERFLOW ARROW
- EXISTING 100-YR FLOODWAY
- EXISTING 100-YR FLOODPLAIN
- PROPOSED 100-YR FLOODPLAIN
- WATERSHED BOUNDARY
- MAJOR BASIN LINE
- 100YR ZONE A FLOODPLAIN
- PROPOSED DETENTION LOCATION (A)
- POTENTIAL WATER QUALITY LOCATION (WQ)
- SWMM CONVEYANCE ELEMENT (SWMM)
- PROPOSED PEAK FLOW RATE (CFS) (850)
- DESIGN POINT (X)
- PROPOSED BASIN LABEL (XX / AREA (AC.) XX XX / BASIN DESIGNATION C5 C100)

LAND USE

- LOW DENSITY
- MEDIUM DENSITY
- HIGH/MED DENSITY
- HIGH DENSITY
- CHURCH
- COMMERCIAL
- ELEMENTARY SCHOOL
- COMMUNITY PARK

NOTES:

DRAINAGE AREA REROUTED AWAY FROM CHANNEL C= 16.1 ac

DRAINAGE AREA REROUTED TOWARD CHANNEL C = 21.2 ac

Refer to this a the pre-development map and then include an existing analysis of the phase 3 area undeveloped, but with Filings 1-4 & Phase 2 developed

Basin	Design Point	5 Year Pre Devlopment	100 Year Pre Development
A1	10	13.03	66.80
B1	20	4.33	48.76
B2	21	1.66	20.74
	22	11.85	140.35
	23	5.99	68.56
B3	24	21.23	249.20
C1	30	9.95	110.70
	31	9.95	110.70
D1	40	8.12	40.00
D2	41	22.23	114.87
	42	8.12	40.00
E1	50	32.34	157.99
	51	93.34	374.99
F1	60	9.70	49.45
F2	61	16.46	86.73
F3	62	3.65	18.42
F4	63	12.98	67.82
	64	13.35	67.87
	65	26.04	135.62
	66	16.46	86.73
	67	231.47	864.52
G1	70	5.57	28.46
G2	71	3.87	20.06
	72	3.87	20.06
	73	3.87	20.06
	74	189.42	643.48
H1	80	1.85	21.89
H2	81	5.37	27.12
H3	82	1.92	9.51
H4	83	8.07	40.86
	84	7.22	49.01
	85	1.92	9.51
OS1	OS1	67.00	413.00
OS2	OS2	59.00	280.00
OS3	OS3	61.00	217.00
OS4	OS4	180.00	595.00
	Outfall1	80.03	479.80
	Outfall2	85.96	597.41
	Outfall3	30.00	154.35
	Outfall4	341.05	1335.77

NORTH

0 200
100

HRGreen

Job No.:	191897.01
Prepared By:	TBI
Date:	04/14/2020

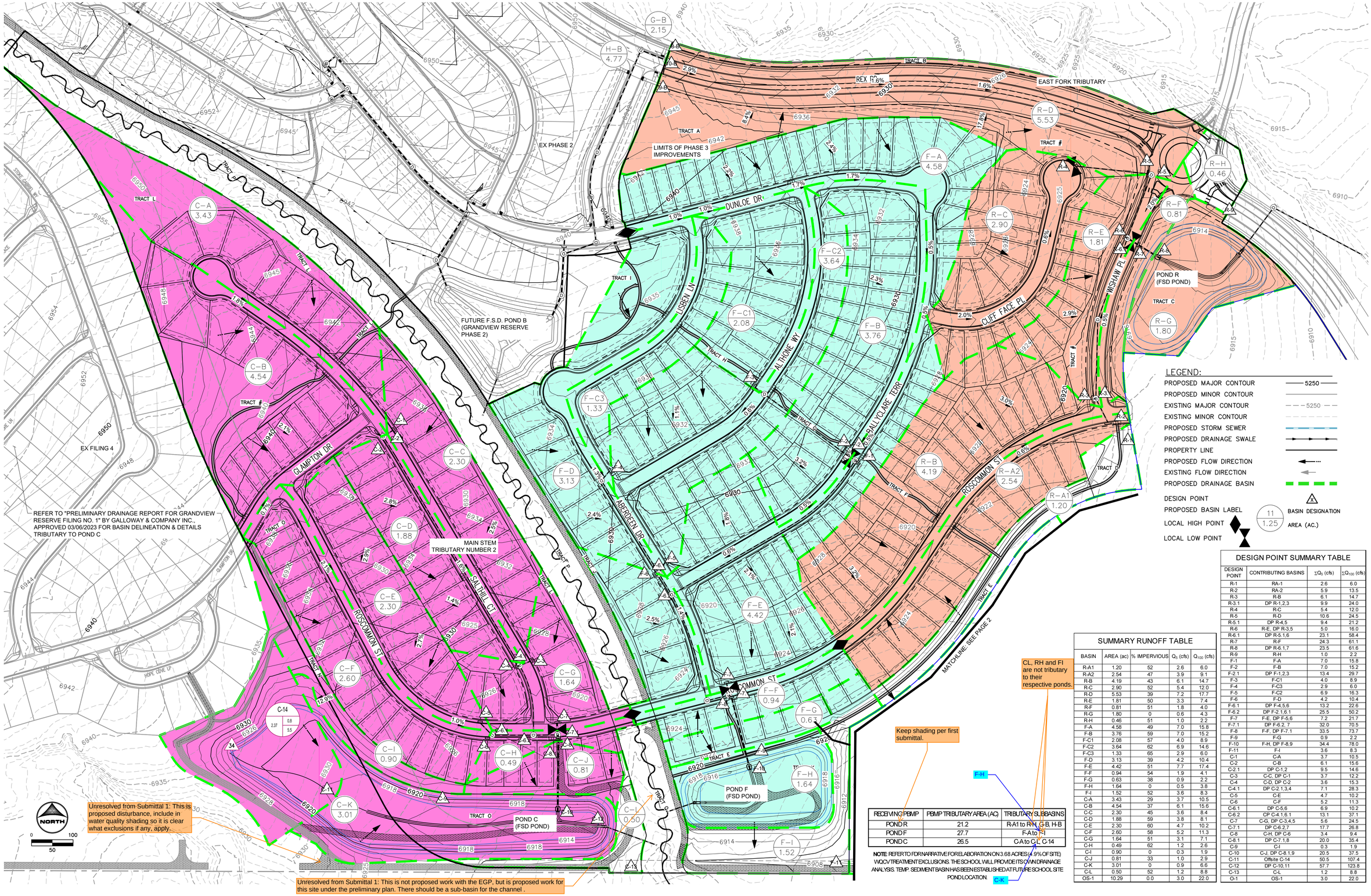
EXISTING EX1

HRGreen DRAWING PATH: J:\2020\201662\CAD\Dwgs\CIPUD_Phase_3_662.203\Drainage\03-DR-EX.dwg

FIG.EX1

WALTON, CHRIS, 10/17/2024 9:27 AM

HR GREEN Xrefs: xc-row-PH3; xc-dsgn-PH3; xc-row-PH2; 01-XC-channel; xgt-1-arch dt01_PH3_PUD; xc-dsgn-PH4; xc-row-PH4; xc-dsgn-PH2; drain_map_legend; PLAN AND PROFILE



Unresolved from Submittal 1: This is proposed disturbance, include in water quality shading so it is clear what exclusions if any, apply.

Unresolved from Submittal 1: This is not proposed work with the EGP, but is proposed work for this site under the preliminary plan. There should be a sub-basin for the channel .

Keep shading per first submittal.

CL, RH and FI are not tributary to their respective ponds.

LEGEND:

- PROPOSED MAJOR CONTOUR — 5250
- PROPOSED MINOR CONTOUR
- EXISTING MAJOR CONTOUR — 5250
- EXISTING MINOR CONTOUR
- PROPOSED STORM SEWER
- PROPOSED DRAINAGE SWALE
- PROPERTY LINE
- PROPOSED FLOW DIRECTION
- EXISTING FLOW DIRECTION
- PROPOSED DRAINAGE BASIN
- DESIGN POINT
- PROPOSED BASIN LABEL — 11 / 1.25 — BASIN DESIGNATION / AREA (AC.)
- LOCAL HIGH POINT
- LOCAL LOW POINT

RECEIVING PBMP	PBMP TRIBUTARY AREA (AC)	TRIBUTARY SUBBASINS
POND R	21.2	R-A1 to R-H, G-B, H-B
POND F	27.7	F-A to F-I
POND C	26.5	C-A to C-L, C-14

NOTE: REFER TO FDR NARRATIVE FOR ELABORATION ON 3.68 ACRES (4.9% OF SITE) WQCV TREATMENT EXCLUSIONS. THE SCHOOL WILL PROVIDE ITS OWN DRAINAGE ANALYSIS. TEMP. SEDIMENT BASIN HAS BEEN ESTABLISHED AT FUTURE SCHOOL SITE POND LOCATION.

DESIGN POINT SUMMARY TABLE

DESIGN POINT	CONTRIBUTING BASINS	ΣQ_5 (cfs)	ΣQ_{100} (cfs)
R-1	RA-1	2.6	6.0
R-2	RA-2	5.9	13.5
R-3	R-B	6.1	14.7
R-3.1	DP R-1,2,3	9.9	24.0
R-4	R-C	5.4	12.0
R-5	R-D	10.6	24.5
R-5.1	DP R-4,5	9.4	21.2
R-6	R-E, DP R-3.5	5.0	16.0
R-6.1	DP R-5.1,6	23.1	58.4
R-7	R-F	24.3	61.1
R-8	DP R-6.1,7	23.5	61.6
R-9	R-H	1.0	2.2
F-1	F-A	7.0	15.8
F-2	F-B	7.0	15.2
F-2.1	DP F-1,2,3	13.4	29.7
F-3	F-C1	4.0	8.9
F-4	F-C3	2.9	6.0
F-5	F-C2	6.9	16.3
F-6	F-D	4.2	10.4
F-6.1	DP F-4,5,6	13.2	22.6
F-6.2	DP F-2.1,6.1	25.5	50.2
F-7	F-E, DP F-5,6	7.2	21.7
F-7.1	DP F-8.2, 7	32.0	70.5
F-8	F-F, DP F-7.1	33.5	73.7
F-9	F-G	0.9	2.2
F-10	F-H, DP F-8,9	34.4	78.0
F-11	F-I	3.6	8.3
C-1	C-A	3.7	10.5
C-2	C-B	6.1	15.6
C-2.1	DP C-1,2	9.5	14.6
C-3	C-C, DP C-1	3.7	12.2
C-4	C-D, DP C-2	3.6	15.3
C-4.1	DP C-2.1,3,4	7.1	28.3
C-5	C-E	4.7	10.2
C-6	C-F	5.2	11.3
C-6.1	DP C-5,6	6.9	10.2
C-6.2	CP C-4.1,6.1	13.1	37.1
C-7	C-G, DP C-3,4,5	5.6	24.5
C-7.1	DP C-6.2,7	17.7	26.8
C-8	C-H, DP C-6	3.4	9.4
C-8.1	DP C-7.1,8	20.0	35.4
C-9	C-I	0.3	1.9
C-10	C-J, DP C-8.1,9	20.5	37.5
C-11	Offsite C-14	50.5	107.4
C-12	DP C-10,11	57.7	123.8
C-13	C-L	1.2	8.8
O-1	OS-1	3.0	22.0

SUMMARY RUNOFF TABLE

BASIN	AREA (ac)	% IMPERVIOUS	Q_5 (cfs)	Q_{100} (cfs)
R-A1	1.20	52	2.6	6.0
R-A2	2.54	47	3.9	9.1
R-B	4.19	43	6.1	14.7
R-C	2.90	52	5.4	12.0
R-D	5.53	39	7.2	17.7
R-E	1.81	50	3.3	7.4
R-F	0.81	51	1.8	4.0
R-G	1.80	0	0.6	4.3
R-H	0.46	51	1.0	2.2
F-A	4.58	49	7.0	15.8
F-B	3.76	59	7.0	15.2
F-C1	2.08	57	4.0	8.9
F-C2	3.64	62	6.9	14.6
F-C3	1.33	65	2.9	6.0
F-D	3.13	39	4.2	10.4
F-E	4.42	51	7.7	17.4
F-F	0.94	54	1.9	4.1
F-G	0.63	38	0.9	2.2
F-H	1.64	0	0.5	3.8
F-I	1.52	52	3.6	8.3
C-A	3.43	29	3.7	10.5
C-B	4.54	37	6.1	15.6
C-C	2.30	45	3.6	8.4
C-D	1.88	59	3.8	8.1
C-E	2.30	60	4.7	10.2
C-F	2.60	58	5.2	11.3
C-G	1.64	51	3.1	7.1
C-H	0.49	62	1.2	2.6
C-I	0.90	0	0.3	1.9
C-J	0.81	33	1.0	2.9
C-K	3.01	0	0.9	6.6
C-L	0.50	52	1.2	8.8
OS-1	10.29	0.0	3.0	22.0

DRAWN BY:	CBM	JOB DATE:	9/18/2024
APPROVED:	KMH	JOB NUMBER:	201662
CAD DATE:	10/17/2024		
CAD FILE:	J:\2020\201662\CAD\Dwgs\C\PUD_Phase_3_662.203\Drainage\03_DR		

BAR IS ONE INCH ON OFFICIAL DRAWINGS. 0 1" IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

HR GREEN - COLORADO SPRINGS
1975 RESEARCH PKWY SUITE 230
COLORADO SPRINGS CO 80920
PHONE: 719.300.4140
FAX: 713.965.0044

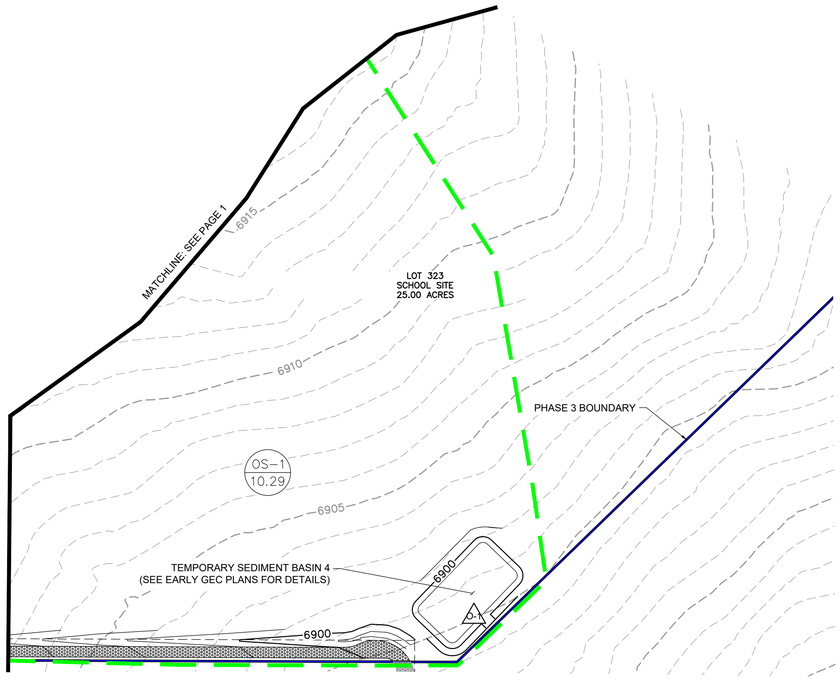
GRANDVIEW RESERVE - PHASE 3
D.R. HORTON
EL PASO COUNTY, CO

GRANDVIEW PHASE 3
PROPOSED DRAINAGE MAP

SHEET DR

1

WALTON, CHRIS, 9/19/2024 1:05 PM



LEGEND:

PROPOSED MAJOR CONTOUR	5250
PROPOSED MINOR CONTOUR	
EXISTING MAJOR CONTOUR	5250
EXISTING MINOR CONTOUR	
PROPOSED STORM SEWER	
PROPOSED DRAINAGE SWALE	
PROPERTY LINE	
PROPOSED FLOW DIRECTION	
EXISTING FLOW DIRECTION	
PROPOSED DRAINAGE BASIN	
DESIGN POINT	A
PROPOSED BASIN LABEL	11 / 1.25 (BASIN DESIGNATION / AREA (AC.))
LOCAL HIGH POINT	
LOCAL LOW POINT	

NORTH
0 100
50

SUMMARY RUNOFF TABLE

BASIN	AREA (ac)	% IMPERVIOUS	Q_5 (cfs)	Q_{100} (cfs)
R-A1	1.20	52	2.6	6.0
R-A2	2.54	47	3.9	9.1
R-B	4.19	43	6.1	14.7
R-C	2.90	52	5.4	12.0
R-D	5.53	39	7.2	17.7
R-E	1.81	50	3.3	7.4
R-F	0.81	51	1.8	4.0
R-G	1.80	0	0.6	4.3
R-H	0.46	51	1.0	2.2
F-A	4.58	49	7.0	15.8
F-B	3.76	59	7.0	15.2
F-C1	2.08	57	4.0	8.9
F-C2	3.64	62	6.9	14.6
F-C3	1.33	65	2.9	6.0
F-D	3.13	39	4.2	10.4
F-E	4.42	51	7.7	17.4
F-F	0.94	54	1.9	4.1
F-G	0.63	38	0.9	2.2
F-H	1.64	0	0.5	3.8
F-I	1.52	52	3.6	8.3
C-A	3.43	29	3.7	10.5
C-B	4.54	37	6.1	15.6
C-C	2.30	45	3.6	8.4
C-D	1.88	59	3.8	8.1
C-E	2.30	60	4.7	10.2
C-F	2.60	58	5.2	11.3
C-G	1.64	51	3.1	7.1
C-H	0.49	62	1.2	2.6
C-I	0.90	0	0.3	1.9
C-J	0.81	33	1.0	2.9
C-K	3.01	0	0.9	6.6
C-L	0.50	52	1.2	8.8
OS-1	10.29	0.0	3.0	22.0

DESIGN POINT SUMMARY TABLE

DESIGN POINT	CONTRIBUTING BASINS	ΣQ_5 (cfs)	ΣQ_{100} (cfs)
R-1	RA-1	2.6	6.0
R-2	RA-2	5.9	13.5
R-3	R-B	6.1	14.7
R-3.1	DP R-1,2,3	9.9	24.0
R-4	R-C	5.4	12.0
R-5	R-D	10.6	24.5
R-5.1	DP R-4,5	9.4	21.2
R-6	R-E, DP R-3,5	5.0	16.0
R-6.1	DP R-5.1,6	23.1	58.4
R-7	R-F	24.3	61.1
R-8	DP R-6.1,7	23.5	61.6
R-9	R-H	1.0	2.2
F-1	F-A	7.0	15.8
F-2	F-B	7.0	15.2
F-2.1	DP F-1,2,3	13.4	29.7
F-3	F-C1	4.0	8.9
F-4	F-C3	2.9	6.0
F-5	F-C2	6.9	16.3
F-6	F-D	4.2	10.4
F-6.1	DP F-4,5,6	13.2	22.6
F-6.2	DP F-2.1,6.1	25.5	50.2
F-7	F-E, DP F-5,6	7.2	21.7
F-7.1	DP F-6.2, 7	32.0	70.5
F-8	F-F, DP F-7.1	33.5	73.7
F-9	F-G	0.9	2.2
F-10	F-H, DP F-8,9	34.4	78.0
F-11	F-I	3.6	8.3
C-1	C-A	3.7	10.5
C-2	C-B	6.1	15.6
C-2.1	DP C-1,2	9.5	14.6
C-3	C-C, DP C-1	3.7	12.2
C-4	C-D, DP C-2	3.6	15.3
C-4.1	DP C-2.1,3,4	7.1	28.3
C-5	C-E	4.7	10.2
C-6	C-F	5.2	11.3
C-6.1	DP C-5,6	6.9	10.2
C-6.2	CP C-4.1,6.1	13.1	37.1
C-7	C-G, DP C-3,4,5	5.6	24.5
C-7.1	DP C-6.2,7	17.7	26.8
C-8	C-H, DP C-8	3.4	9.4
C-8.1	DP C-7.1,8	20.0	35.4
C-9	C-I	0.3	1.9
C-10	C-J, DP C-8.1,9	20.5	37.5
C-11	Offsite C-14	50.5	107.4
C-K	DP C-10,11	57.7	123.8
C-L	C-L	1.2	8.8
O-1	OS-1	3.0	22.0

RECEIVING PBMP	PBMP TRIBUTARY AREA (AC)	TRIBUTARY SUBBASINS
POND R	21.2	R-A1 to R-H, G-B, H-B
POND F	27.7	F-A to F-I
POND C	26.5	C-A to C-L, C-14

NOTE: REFER TO FDR NARRATIVE FOR ELABORATION ON 3.68 ACRES (4.9% OF SITE) WQCV TREATMENT EXCLUSIONS. THE SCHOOL WILL PROVIDE ITS OWN DRAINAGE ANALYSIS. TEMP. SEDIMENT BASIN H HAS BEEN ESTABLISHED AT FUTURE SCHOOL SITE POND LOCATION.

HR GREEN Xrefs: xc--row--PH3; xc--dsgn--PH3; xc--row--PH2; 01--XC--channel; xgl--1--arch dh01_PH3_PUD; xc--dsgn--F4; xc--row--F4; xc--dsgn--PH2; drain_map_legend

DRAWN BY: CBM JOB DATE: 9/18/2024
APPROVED: KMH JOB NUMBER: 201662
CAD DATE: 9/19/2024
CAD FILE: J:\2020\201662\CAD\Dwgs\C\PUD_Phase_3_662.203\Drainage\03_DR

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IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	REVISION DESCRIPTION

HRGreen
HR GREEN - COLORADO SPRINGS
1975 RESEARCH PKWY SUITE 230
COLORADO SPRINGS CO 80920
PHONE: 719.300.4140
FAX: 713.965.0044

GRANDVIEW RESERVE - PHASE 3
D.R. HORTON
EL PASO COUNTY, CO

D·R·HORTON America's Builder

GRANDVIEW PHASE 3
PH3-2

SHEET ----

2