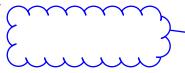
ROLLING HILLS BOOSTER PUMP STATION SMALL SUBDIVISION DRAINAGE REPORT

(THE HILLS AT LORSON RANCH FILING NO. 1)



Add PCD File # PPR-21-075

PREPARED BY

Rich Gallegos, P.E., CFM RESPEC 121 S. Tejon St., Suite 1110 Colorado Springs, CO 80903

PREPARED FOR

Widefield Water and Sanitation District 8495 Fontaine Boulevard Colorado Springs, CO 80925

DECEMBER 2021





RESPEC ENGINEER'S STATEMENT

The attached drainage letter plan and report were prepared under my direction and supervision and
are correct to the best of my knowledge and belief. Said drainage report has been prepared according
to criteria established by the City/County for drainage reports, and said report is in conformity with
the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent
acts, errors, or omissions on my part in preparing this report.

Richard Gallegos, P.E., CFM Date
Registered Professional Engineer State of Colorado No. 36247

DEVELOPER'S STATEMENT

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Date

Lucas Hale
Widefield Water and Sanitation District
8490 Fontaine Boulevard
Colorado Springs, CO 80925

EL PASO COUNTY

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual, and Land Development Code as amended.

Jennifer Irvine P.E.	Date	
County Engineer/EMC Administra	tor	
Conditions:	Correct to ECM Administrator	



On behalf of the Widefield Water and Sanitation District, RESPEC Engineering is submitting this Small Subdivision Drainage Report to support the development of the Rolling Hills Booster Pump station.

This site is also covered by the "Skyline at Lorson Ranch Filing #1" PRELIM and FINAL Drainage Report. PUDSP212. Specify which drainage report takes precedence.

1. GENERAL PROPERTY DESCRIPTION WITH ACREAGE

The 0.707-acre site is located within the larger Lorson Ranch Master Planned Community east of Marksheffel Road and Fontaine Boulevard. More specifically, the pump station is northeast of the intersection of Grayling Drive and Yellowthroat Terrace. The County-approved *Final Drainage Plan (SSF 21-010) Hills at Lorson Ranch Filing 1* (Lorson Ranch FDR), revised March 23, 2021, included the project site within the master plan, which provides an existing and proposed condition analysis. The Lorson Ranch FDR is also in conformance with the Drainage Basin Planning Study for Jimmy Camp Creek.

Pertinent excerpts from the Lorson Ranch FDR report are attached to this Small Subdivision Drainage Report.

2. GENERAL EXISTING DRAINAGE CHARACTERISTICS

The site drains to the south toward Grayling Drive with average slopes of 2%-3%. Currently, underground water lines and appurtenances exist on the site. They shall be extended to a new booter pump housing, which is part of a more extensive regional potable water distribution system.

The Lorson Ranch FDR included the site as part of the existing conditions analysis within Drainage Area C5.2-ex; see attached Lorson Ranch FDR Existing Drainage Area Map. C5.2-ex totals 13.32 acres with peak runoff rates of 3.2 cfs for the 5-year storm event and 21.8 cfs for the 100-year storm event. Flows travel overland and eventually to Jimmy Camp Creek, located to the west.

The site is located in an Unshaded Zone X (areas outside of the 500-year floodplain) flood hazard area per the Federal Emergency Management Agency's Flood Insurance Rate Map panel 08041C0976G, effective date December 7, 2018. See attached floodplain exhibit.

3. GENERAL PROPOSED DRAINAGE CHARACTERISTICS

The site is split between two proposed drainage areas in the Lorson Ranch FDR.

The majority of the improvements will be located in Drainage Area C10.8. Per the FDR, basin C10.8 totals 1.89-acres and generates peak runoff rates of 3.4 cfs for the 5-year storm event and 7.4 cfs for the 100-year storm event. In addition, the drainage area includes portions of a future single-family development located to the north. Runoff rates are based upon a landuse of 1/8 acre single-family lots, with an assumed percent impervious cover of 65%.

The easternmost portion of the site is located in Drainage Area C10.9, which is 3.73-acres in size and will be comprised of future single-family lots, the pump station site, and an



easement for overhead electrical lines. Peak runoff rates for the drainage area are listed as 5.9 cfs for the 5-year storm event and 13.0 cfs for the 100-year storm event in the Lorson Ranch FDR. Land use for this area is assumed to be predominately 1/8 acre single-family lots with 65% impervious cover.

Runoff from both drainage areas listed above are designed to surface flow to Grayling Drive. Based upon the Lorson Ranch FDR, flows from both drainage areas will be collected by a 25' Type R inlet and conveyed to existing Detention Pond C3 via public 30" and 48" RCP storm sewers. Detention Basin C3 has sufficient detention and water quality capacity to develop the Rolling Hills Booster Pump Station.

4. HYRDOLOGOCIAL CALCULATIONS

For this drainage analysis, we have completed two evaluations. The first is to compute the overall percent of impervious cover to verify that the proposed land use complies with the assumptions outlined in the Lorson Ranch FDR. The second is to establish the flow rates within the site to size two sidewalk chases properly.

Compliance with the Lorson Ranch FDR:

As part of the analysis of the proposed conditions completed for Lorson Ranch, the assumptions for the two drainage areas (C10.8 and C10.9) were reviewed. Both areas assume a hydrologic soil type "B" and a maximum percent impervious of 65%. The overall percent impervious calculation for the Rolling Hills Booster Pump Station as designed was completed to verify that the proposed development complies with the FDR analysis.

- Total Acreage: 0.707-acres
 - Total area of pavement/building: 0.07-acres @ 100% Impervious
 - Total area of Gravel Drive/Rock Landscaping: 0.39-acres @ 80% Impervious
 - Total area of vegetated area: 0.25-ac @ 0% Impervious
- Computed Site Percent Impervious: 54%
- Maximum Allowable Percent Impervious: 65%

The site as designed has an overall percent impervious of 54%, which is less than the maximum allowed by the Lorson Ranch FDR of 65%; therefore, the land use design complies with the approved master drainage plan. Sufficient stormwater mitigation capacity and water quality exist within Detention Pond C3.

Sidewalk Chase Calculations:

Two drainage areas were delineated to compute flows directed to two sidewalk chases. Each area considers the proposed grades for the Future Lorson Ranch development, which is imminent. The Rational Method was used to determine the peak 5-year and 100-year flows and utilize the same assumptions given within the Lorson Ranch FDR.

The 5-year, C(5) = 0.45, and 100-year, C(100) = 0.59, runoff coefficients as those given in the FDR are used to compute peak flow rates. The drainage areas delineated specifically for this analysis also are contained within the larger FDR drainage delineations given within the County approved report.

Sidewalk chases are not recommended due to maintenance requirements. Recommend conveying site flows to rear and connect to RCP flowing to Pond C-3



Drainage Area PR1 consists of 0.29-acres of area on the western portion of the project limits. The drainage area extends offsite and will accept minor sheet flow from the adjacent single-family development. Computed peak flow rates of Q(5) = 0.7 cfs and Q(100) = 1.5 cfs will surface flow to a 4' wide sidewalk chase and into Grayling Drive. The 4' wide concrete sidewalk will slope to Grayling Drive at a 2% grade and will have sufficient capacity to accommodate the 100-year peak flow rates.

Drainage Area PR2 is 0.56-acres in size, encompassing the east side of the project site. Flows from this drainage area include adjacent single-family lots and open space in the Future Lorson Ranch development area. Peak flow rates of Q(5) = 1.2 cfs and Q(100) = 2.4 cfs were computed PR2. A proposed 4' wide concrete sidewalk chase at a 2% flowline grade will convey flows to Grayling Drive. The chase will have sufficient capacity to accommodate the 100-year peak flow rates.

5. Drainage fees

The site is located within an existing easement and will not be platted. As such, no drainage fees are required for the project. However, the site will be platted later when the adjacent future single-family Lorson Ranch subdivision is developed.

Drainage fee will be required when Skyline at Lorson Range Filing #1 is platted to include this area and tract. Please correct statement. This statement conflict with the FDR and Letter of Intent which states that Drainage Basin Fees will be paid.

From Lorson Range Filing #1 FDR

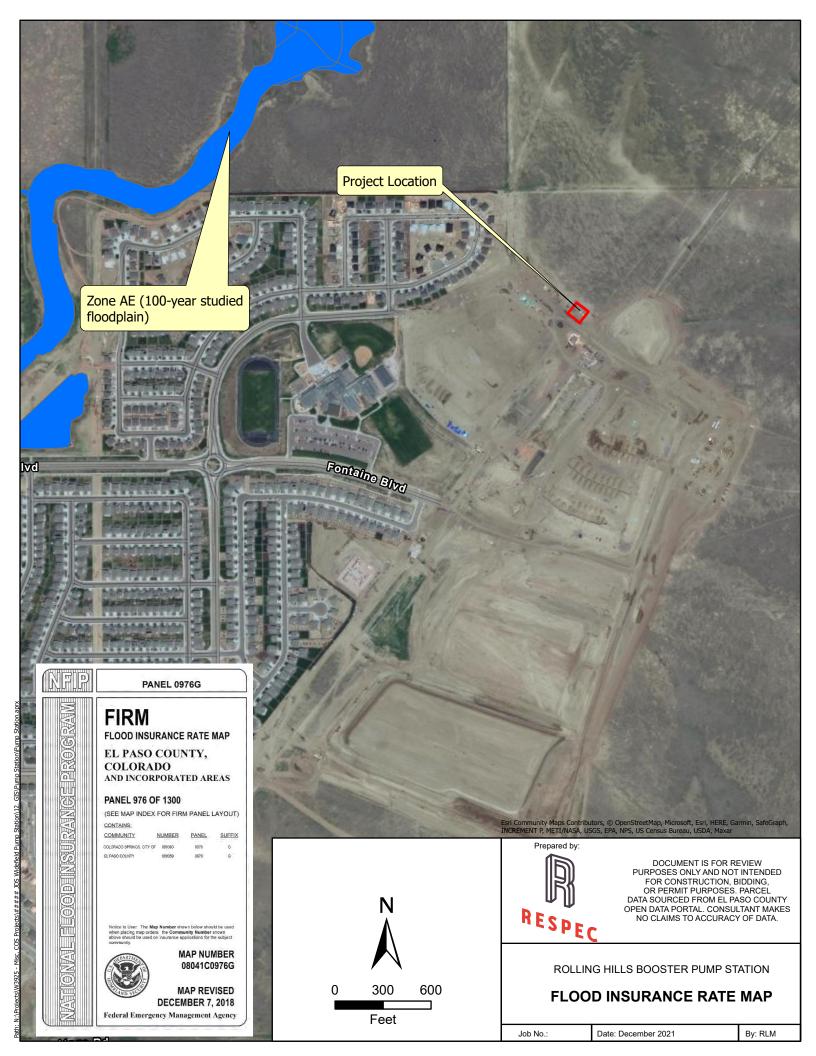
7.0 DRAINAGE AND BRIDGE FEES

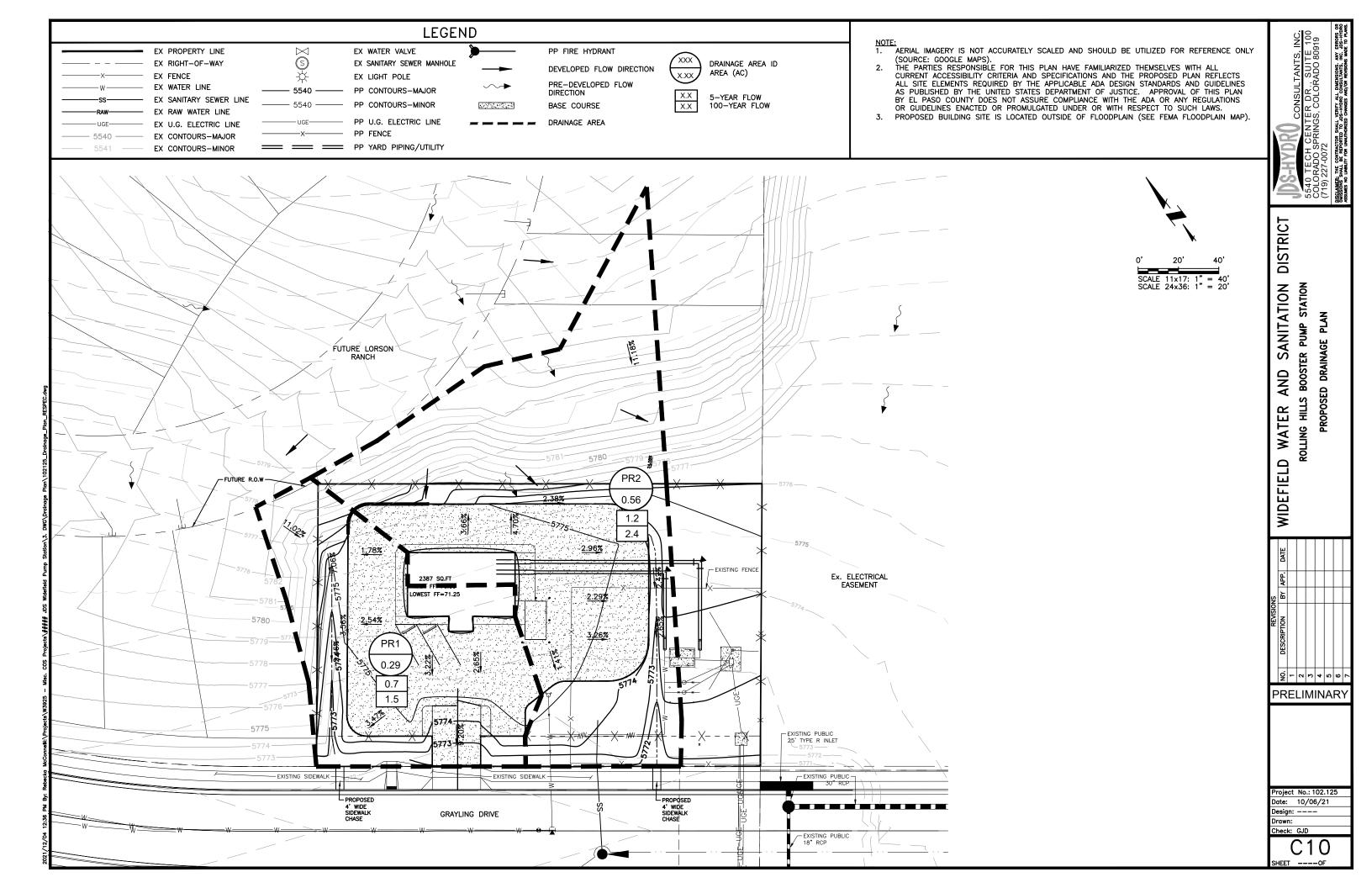
The Hills at Lorson Ranch is located within the Jimmy Camp Creek drainage basin which is currently a

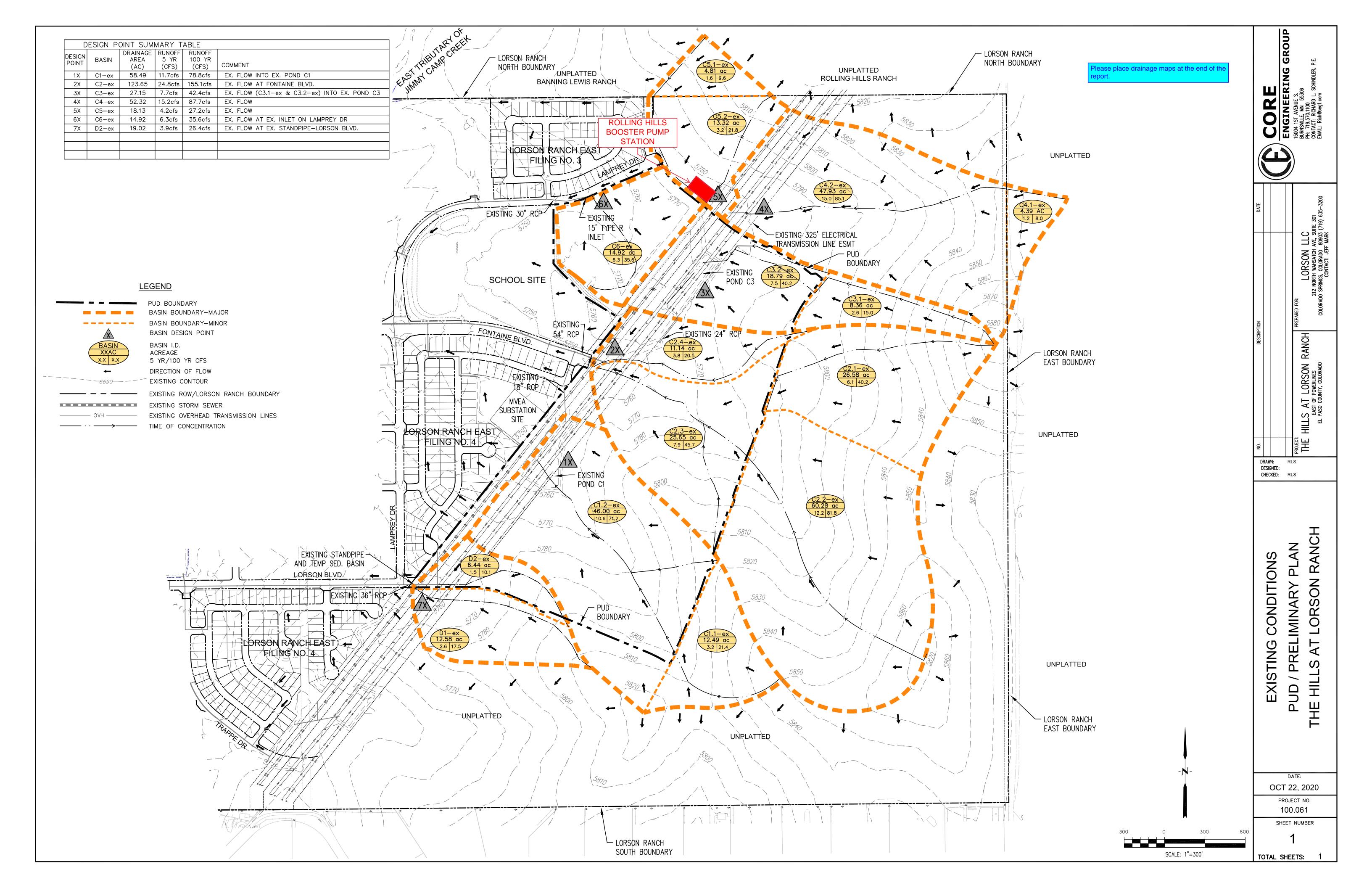
fee basin in El Paso County. Current El Paso County regulations require drainage and bridge fees to be paid for platting of land as part of the plat recordation process.

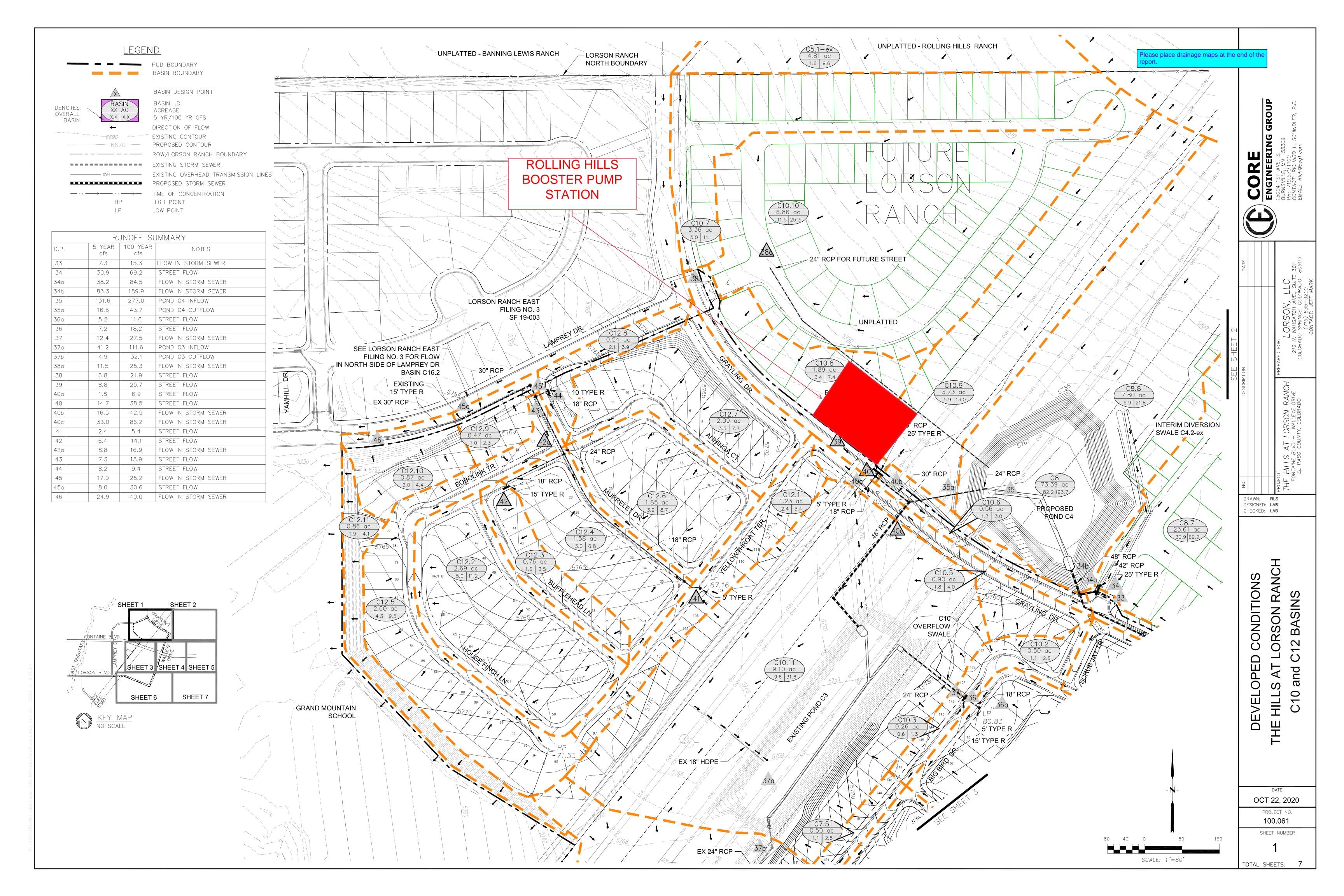
Lorson Ranch Metro District will compile and submit to the county on a yearly basis the Drainage and bridge fees for the approved plats and shall show all credits they have received for the same yearly time frame.











PROJECT	WWSD Rolling Hills Booster Pump Station		
PREPARED BY	AC	DATE	12/01/21

	PROPOSED CONDITIONS - TIME OF CONCENTRATION CALCULATIONS													
Sub-Bas	in Data		Overland F	low Time		Channel Flow Mannings Analysis					t _c Check nized Basins)	Final t _c	Remarks	
DESIGN	AREA (A) acres	C ₁₀ (t _i)	LENGTH (L) feet	SLOPE (S) %	t _{oL} minutes	LENGTH (L) feet	n	SLOPE (S) ft/ft	Velocity (V) ft/sec	t _{CF} minutes	TOTAL LENGTH (L) feet	t _c =(L/180)+10 minutes	t _c = t _i +t _t minutes	
PR1	0.29	0.15	52	11.00	5.8	100	0.025	0.025	5.72	0.3	152	10.8	6.1	
PR2	0.56	0.15	135	15	8.4	130	0.025	0.025	5.80	0.4	265	11.5	8.8	

PROJECT	WWSD Rolling Hills Booster Pump Station
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PREPARED BY AC DATE 12/01/21

		PROPOSED CONDI	TIONS -	- RATIO	NAL METI	HOD (CALCUL	ATIONS	
STREET			D	IRECT RU	NOFF	REMARKS			
	DESIGN POINT	AREA DESIGN	AREA (A)	RUNOFF COEFF (C)	••	85	_	Ø	
	DE	AR	ac	10yr	min		in/hr	cfs	
	1		5-Yea	r Runoff C	alculations	T	1		1
	PR1		0.29	0.45	6.10	0.13	5.7	0.7	
	PR2		0.56	0.45	8.82	0.25	4.9	1.2	
	•		100-Ye	ar Runoff	Calculation	s			
	PR1		0.29	0.59	6.10	0.171	8.6	1.5	
	PR2		0.56	0.59	8.82	0.328	7.4	2.4	