

Traffic Impact Study

Mayberry Communities

Filing 5 Traffic Impact Study

El Paso County, Colorado

February 16, 2023

Add PCD File No. PUDSP233

Traffic Impact Study

Mayberry Communities – Filing 5

El Paso County, Colorado

February 16, 2023

Prepared for

Mayberry Communities

Prepared by

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Introduction

Mayberry Communities have retained HDR Engineering, Inc. to perform a Traffic Impact Study (TIS) for the proposed Filing 5 (Tract K) development located in the southeast quadrant of New Log Road and SH 94, as shown in Figure 1. The development is anticipated to consist of the following proposed land uses:

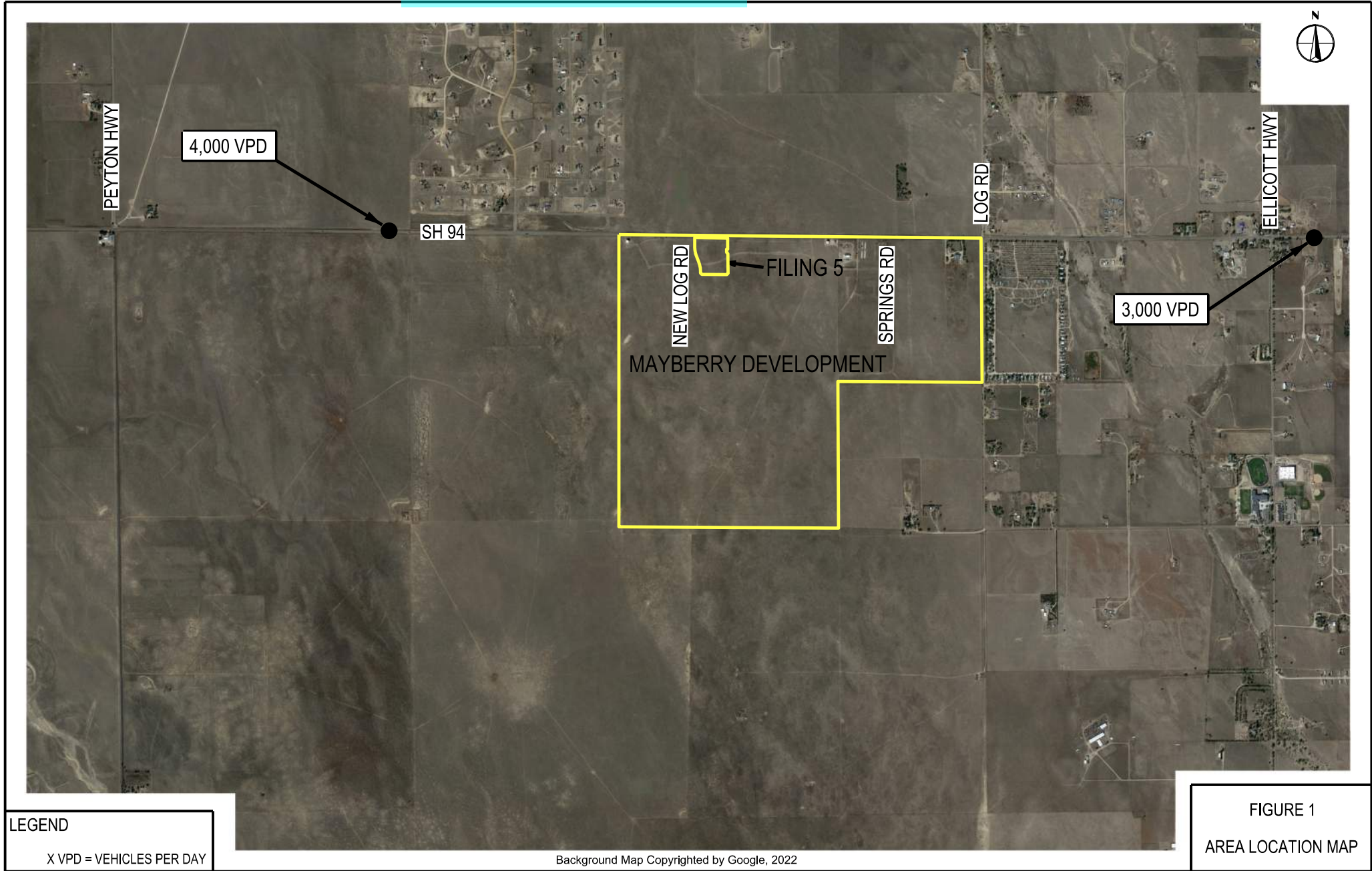
- 38 dwelling units of single-family detached housing
- 108 dwelling units of mid-rise, multi-family housing

The project site is currently vacant, and the development is expected to be complete by 2024. This study serves as part of an update to the approved *2020 - June - Ellicott Town Center Commercial Rezone TIS Report (LSC 194060) (Ref 1)* and uses assumptions and traffic data from the *2022 - September - Mayberry Filing No. 3 (Ref 2)* TIS. Filing 5 is part of the broader proposed Mayberry Communities Development just west of Ellicott between Peyton Highway and Log Road. This community is being developed in phases, and this report details the traffic impacts only due to the Filing 5 phase of development.

please include Filing 4 TIS in the list of the referenced reports.

the filing 3 TIS is dated October 2022. revise. (see PCD file No. SF2219)

please provide information as to what time period is the ADT that is shown. Are these existing values?



LEGEND
X VPD = VEHICLES PER DAY

FIGURE 1
AREA LOCATION MAP

Background Map Copyrighted by Google, 2022

The commercial rezone TIS listed above is the study that has these assumptions as the filing 3 study was a memo that referenced the commercial rezone TIS. Please reference the commercial rezone study.

Analysis Assumptions

This traffic impact study uses the Highway Capacity Manual 6 (HCM) (see Appendix A for a brief description of level of service) as a basis for the capacity analysis as well as primary data and engineering judgment, which is required to estimate background traffic, pass-by capture, and internal capture reductions, further described in the following paragraphs.

Directional Distribution

Existing traffic projections are based on data collected for the development of the *2022 - September - Mayberry Filing No. 3*. Turning movement counts were collected for the Peyton Highway/SH 94 intersection (west of Mayberry Communities) and the Ellicott Highway/SH 94 intersection (east of Mayberry Communities).

This study follows the assumption established in the *2022 - September - Mayberry Filing No. 3* that 90% of vehicle trips go to and come from points west of the development, while 10% go to and come from points east of the development. Following the 90/10 assumption, future traffic is then assumed to be proportionally distributed according to the turning movement counts collected at Peyton Highway and Ellicott Highway intersections. These counts provide the basis for the overall directional distribution of traffic approaching and departing the project site, as summarized in Table 1.

Table 1: Forecasted Overall Directional Distribution Site-Oriented Traffic

Direction/Roadway	AM % Overall Distribution	PM % Overall Distribution
SH 94 W	82.4%	76.6%
SH 94 E	5.3%	6.0%
Peyton Hwy S	2.3%	5.9%
Peyton Hwy N	5.3%	7.5%
Ellicott Hwy S	4.0%	2.3%
Ellicott Hwy N	0.6%	1.7%

HDR has not found other studies in the area. Based on current land use at the site, this study does not use pass-by, internal capture, pedestrian, and bicycle reductions.

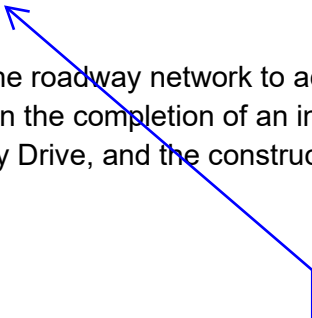
Filing 3 Roadway Improvements

The LOS analysis is based on the proposed improvements from *2022 - September - Mayberry Filing No. 3*. The roadway network proposed in Filing 3 is assumed to be in place at the time of completion for Filing 5.

New Log Road and SH 94 will be an unsignalized intersection with stop control on the northbound approach. The approaches will be constructed according to the following parameters:

- One left-turn lane and one right-turn lane for the northbound approach on New Log Road
- A through lane and a dedicated right-turn lane on the eastbound approach of SH 94
- A dedicated left-turn lane and one through lane on the westbound approach of SH 94

The ability of the roadway network to accommodate the generated traffic of Filing 5 is contingent upon the completion of an internal roadway network comprised of Village Main, Mayberry Drive, and the construction of New Log Road and Springs Road.



a left turn acceleration lane (northbound to westbound) is identified to be installed with filing 3. See Filing 3 TIS table 12 and update the narrative accordingly.

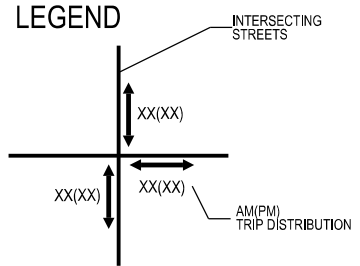
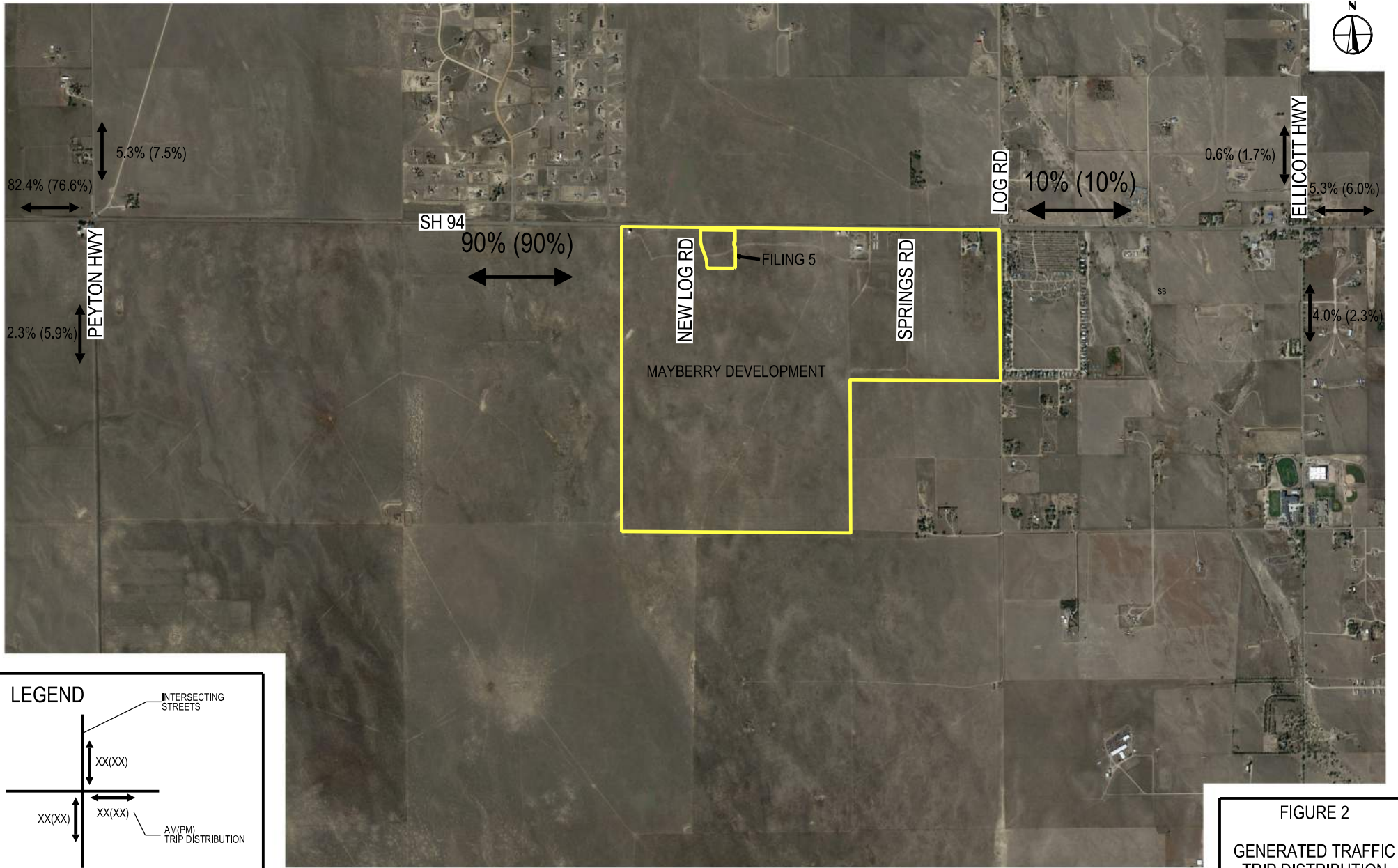


FIGURE 2
GENERATED TRAFFIC TRIP DISTRIBUTION

new counts are required at the study area intersections. Per ECM App. B.3.1.A counts shall be no more than 1 yr old.

Existing Thoroughfare System

As indicated on the area location map (Figure 1) and the conceptual site plan (Figure 2), the project is located in the southeast quadrant of New Log Road and SH 94, near Ellicott, CO.

Average daily traffic estimates SH 94 were obtained from the Colorado Department of Transportation (CDOT) Online Transportation Information System (OTIS) (Ref. 3) and turning movement counts provided in the previous TIS dated **September 2022**. To adequately describe these roadways, further characterization is provided for each adjacent major roadway to the development.

SH 94

CDOT classifies SH 94 as a functional type Minor Arterial and an access control type as a Non-Rural Principal Highway (NR-A) west of County Road 493 and a Regional Highway (R-A) east of County Road 493. The posted speed limit is 65 miles per hour near the development. An OTIS straight-line diagram of SH 94 near the project site is provided in Appendix A. According to CDOT's traffic volume database, the existing daily traffic volume on SH 94 is listed below:

- 4,000 vpd between Peyton Highway and Ellicott Highway
- 3,000 vpd east of Ellicott Highway

Peyton Highway

The El Paso County 2040 Major Transportation Corridor Plan (MTCP)(Ref. 4) classifies Peyton Highway as a Minor Arterial and has a speed limit of 55 mph.

Ellicott Highway

The El Paso County MTCP classifies Ellicott Highway as a Minor Arterial and has a speed limit of 55 mph.

Site and Access Characteristics

As shown in Figure 3, access to Filing 5 will be provided via two full-movement driveways along Marketplace Drive.

please also identify that the El Paso County 2040 Major transportation Corridors Plan (MTCP) indicates that this roadway as a 2 lane principal arterial an

please also include the roadways Village mainstreet, Market Place Dr, and Mayberry Dr (east west roadway) and their classifications.



FIGURE 3
CONCEPTUAL SITE PLAN

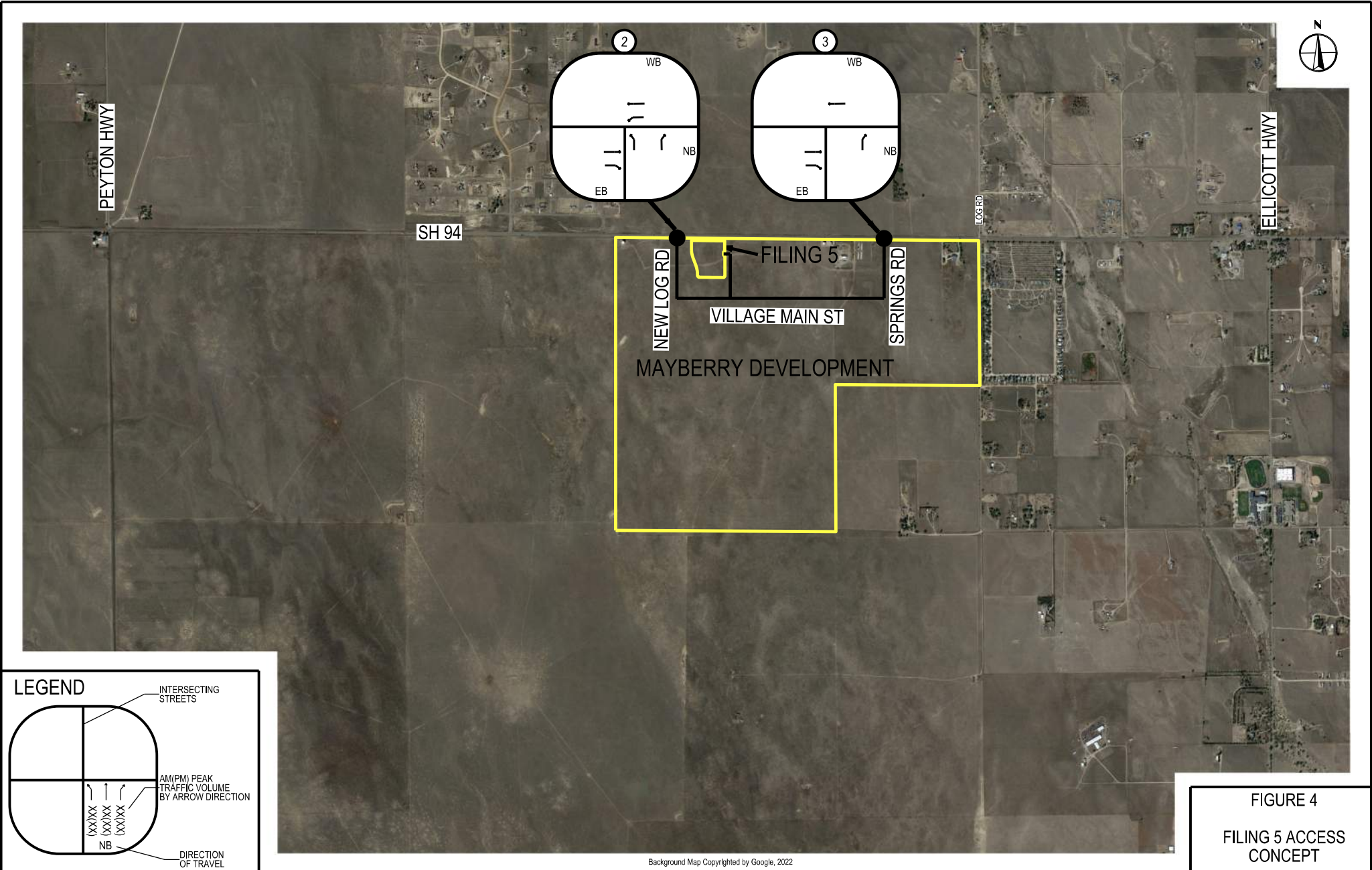


FIGURE 4
 FILING 5 ACCESS
 CONCEPT

Background Map Copyrighted by Google, 2022

please also include long term analysis as required per ECM appendix B.

Traffic Analysis

To assess the traffic impacts of the proposed development, two (2) time periods (AM Peak Hour and PM Peak Hour) and three (3) travel conditions were evaluated:

- 2024 Forecasted Traffic Conditions
- 2024 Forecasted plus Previous Filing 3 Background Traffic Conditions
- 2024 Background plus Site-Generated Traffic Conditions

Intersections in the vicinity of the site are considered the locations of principal concern because they are the locations of the highest traffic conflict and delay. The standard used to evaluate traffic conditions at intersections is level of service (LOS), which is a qualitative measure of the effect of a number of factors such as speed, the volume of traffic, geometric features, traffic interruptions, freedom to maneuver, safety, driving comfort, convenience, and operating cost.

2024 Forecasted Traffic Conditions

The analysis of existing traffic required the collection of data on the major roadways and intersections. Traffic counts for the following study area intersections were collected in March and August 2022 while schools were in session unless otherwise noted:

- Peyton Highway and SH 94
- Ellicott Highway and SH 94

The existing TMC values were grown by the growth rate provided by OTIS to reach a 2024 forecast year. This process used trends established by prior data for the major roadways and intersections near the project site. The adjusted 2024 existing turning movement counts are provided in Figure 4. Descriptions of existing study intersections are discussed in the following sections as well as the forecasted LOS for the Year 2024. Table 2 provides the summary of both LOS and delay.

Peyton Highway and SH 94

Peyton Highway and SH 94 is currently an unsignalized intersection with stop controls on the northbound and southbound approaches. The northbound and southbound approaches of Peyton Highway provide one left-turn/through/right-turn shared lane. The eastbound and westbound approaches of SH 94 provide one left-turn lane and a through/right-turn shared lane. The northbound leg of the intersection currently operates at LOS B under the existing traffic conditions during both the AM and PM peak periods.

Ellicott Highway and SH 94

Ellicott Highway and SH 94 is currently an unsignalized intersection with stop controls on the northbound and southbound approaches. The northbound and southbound approaches of Ellicott Highway provide one left-turn/through/right-turn shared lane. The eastbound and westbound approaches of SH 94 provide one left-turn lane and a

through/right-turn shared lane. The northbound leg of the intersection currently operates at LOS C under the existing traffic conditions during both the AM and PM peak periods.

Table 2: 2024 Existing Forecasted Level of Service Summary

Intersection	2024 Existing	
	AM	PM
Peyton Highway and SH 94	B (14.1)	B (13.5)
Ellicott Highway and SH 94	C (16.0)	C (15.5)

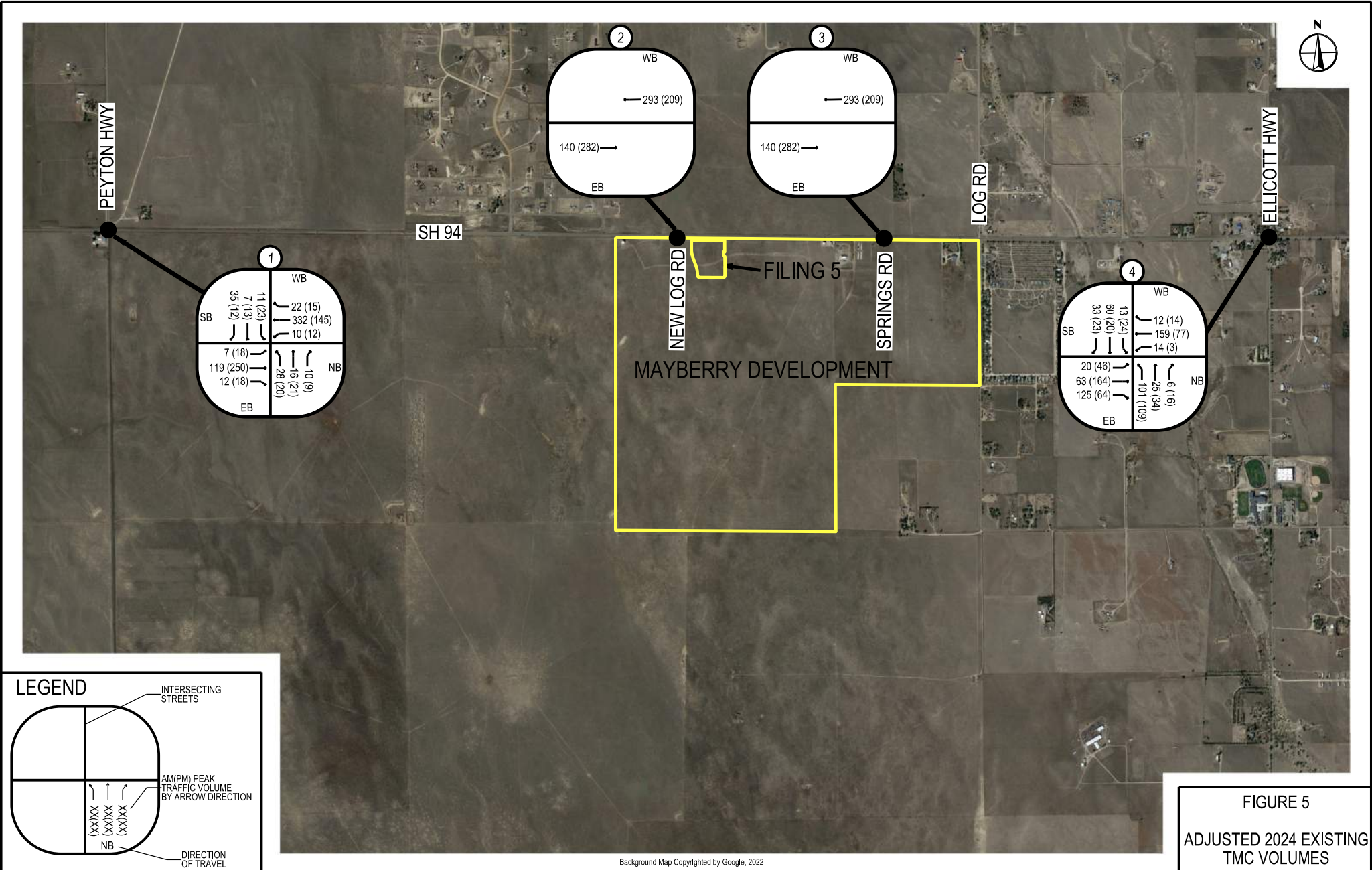
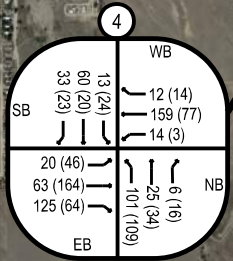
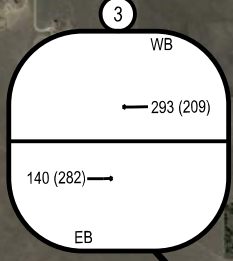
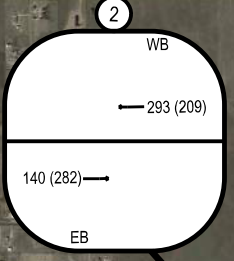
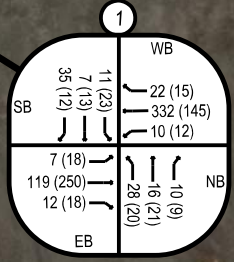
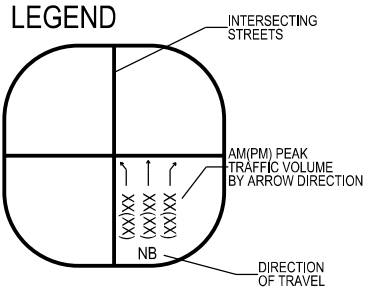


FIGURE 5
ADJUSTED 2024 EXISTING
TMC VOLUMES

please also include ADT in all the figures for study area roadways.



2024 Existing plus Previous Filing Background Traffic Conditions

The generated traffic from the previous Filings 1, 2, and 3 are assumed to be part of the background traffic. The proposed access roads that will accommodate this traffic are studied for the background traffic and the development traffic to follow. The additional intersections that will be built as part of Mayberry Filing 3 are listed below:

- New Log Road and SH 94
- Spring Road and SH 94

how about Filing 4? There is a final plat application for filing 4 currently in review ahead of this project and the assumption is that it would gain approval prior to filing 5. Please include filing 4 traffic in your analysis. Filing 4 is listed in the table below but not mentioned in the narrative.

Previous Filing Site-Generated Traffic

Determining the site-generated traffic, or the traffic generated due to the development of the previous Filings, is a major element of this analysis. Unadjusted daily trips and the peak hour traffic associated with these Filings were estimated using recommendations and data contained in the Institute of Transportation Engineers Trip Generation, 11th Edition (Ref. 6).

These previous Filings generate approximately 2,801 unadjusted daily trips upon build-out. Table 3 provides a detailed traffic generation summary related to the assumed land use plan.

Table 3: Summary of Unadjusted Daily and Peak Hour Trip Generation from Previous Filings

Site	Land Use	Land Use Code	Size	Trip Generation Method ¹	24-Hour Two-Way Volume	AM Peak Hour		PM Peak Hour	
						Enter	Exit	Enter	Exit
Filing 1/1A/3	Single Family Detached Housing	210	240 DU	Fitted Curve	2,257	43	123	143	84
Filing 2	General Light Industrial	110	30 KSF	Fitted Curve	163	21	3	2	15
Filing 4	General Light Industrial	110	88 KSF	Fitted Curve	381	56	8	5	32
Total					2,801	120	134	150	131

¹Trip Generation is based on the higher of the ITE's average rate and fitted curve method for all land uses.

The LOS summary for the trips generated from the previous Filings are discussed below. Table 4 provides the summary of both LOS and delay. Background plus the previous Filings volumes are shown in Figure 5.

Peyton Highway and SH 94

The intersection will operate at LOS C under 2024 Forecasted plus Previous Filing Background Traffic Conditions during the AM and PM peak periods.

New Log Road and SH 94

New Log Road and SH 94 will be an unsignalized intersection with stop controls on the northbound approach. The northbound approach of New Log Road will provide one left-turn lane and one right-turn lane. The eastbound approach of SH 94 will provide a through lane and a dedicated right-turn lane. The westbound approach of SH 94 will provide a dedicated left-turn lane and one through lane. These improvements will be built concurrently with these Filings and will be in place by the time they are occupied. The intersection will operate at LOS C under 2024 Forecasted plus Previous Filing traffic conditions during the AM and PM peak periods.

Springs Road and SH 94

The intersection will operate at LOS A and B under 2024 Forecasted plus Previous Filing conditions during the AM and PM peak periods, respectively. Assuming the connections at both New Log Road and Springs Road are provided

Ellicott Highway and SH 94

The intersection will operate at LOS C under 2024 Forecasted plus Previous Filing traffic conditions during the AM and PM peak periods.

Table 4: Filing 1, 2 and 3 Level of Service Summary

Intersection	2024 Background + Previous Filings	
	AM	PM
Peyton Highway and SH 94	C (16.7)	C (19.8)
New Log Road and SH 94	C (15.2)	C (16.4)
Springs Road and SH 94	A (9.2)	B (10.2)
Ellicott Highway and SH 94	C (16.9)	C (16.5)

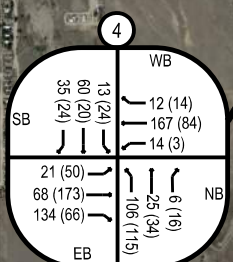
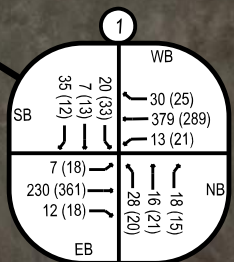
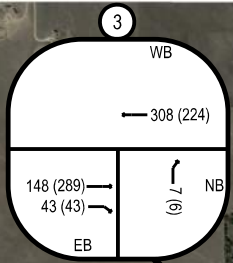
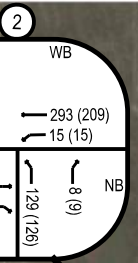
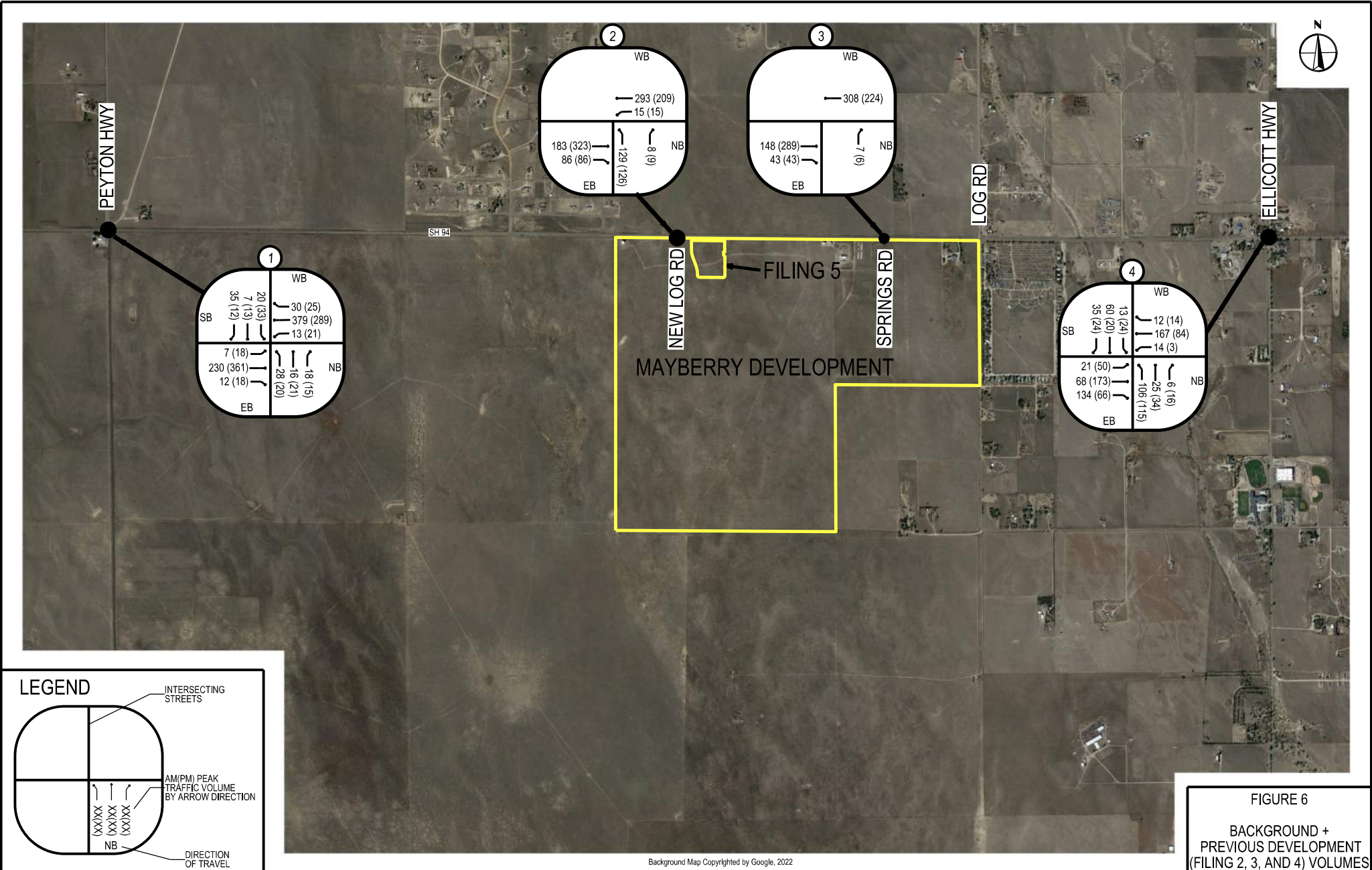


FIGURE 6
BACKGROUND + PREVIOUS DEVELOPMENT (FILING 2, 3, AND 4) VOLUMES

revise to include also filing 1 as stated above

Please clarify which improvements are assumed. The filing 3 TIS identified that beyond filing 3 the full urban minor arterial (full couplet segments) is to be constructed (>3000 ADT). Is the full configuration for New Log road assumed therefore filing 4 is installing the improvements?

2024 Conditions with Filing 5 Site-Generated Traffic

Filing 5 is anticipated to be completed in 2024. The forecasted traffic was projected using available information and was used to assess the major roadway impacts and evaluate potential improvements. All analysis assumes the completion of New Log Road and Springs Road improvements upon which previous filings are contingent.

Filing 5 Site Generated Traffic

Unadjusted total trips per day and the peak hour traffic associated with the project were estimated using recommendations and data contained in the Institute of Transportation Engineers Trip Generation, 11th Edition.

The proposed project will generate approximately 1,489 unadjusted daily trips upon build-out. Table 5 provides a detailed traffic production summary directly related to the assumed land use plan.

Table 5: Summary of Unadjusted Daily and Peak Hour Trip Generation from Filing 5

Site	Land Use	Land Use Code	Size	Trip Generation Method ¹	24-Hour Two-Way Volume	AM Peak Hour		PM Peak Hour	
						Enter	Exit	Enter	Exit
Filing 5	Multifamily Housing (Low-Rise)	220	108 DU	Fitted Curve	1,075	17	54	55	32
	Single Family Attached	215	38 DU	Fitted Curve	239	4	10	11	8
Total					1,314	21	64	66	40

¹Trip Generation is based on the higher of the ITE's average rate and fitted curve method for all land uses.
²Trips to and from the proposed clubhouse development are assumed to be internal and included in ITE trip generation values and assumed to include a small clubhouse office that has negligible impact on trip generation

The LOS summary for the trips generated from Filing 5 are discussed below. Table 6 provides the summary of both LOS and delay. Filing 5 generated volumes are shown in Figure 6, and Background + Filing 3 + Filing 5 volumes are shown in Figure 7.

Peyton Highway and SH 94

see comment above about filing 4 and adjust as necessary.

The intersection will operate at LOS C under 2024 site plus forecasted traffic conditions during the AM and PM peak periods. There are no improvements recommended at this intersection as part of this TIS.

New Log Road and SH 94

The intersection will operate at LOS C under 2024 site plus forecasted traffic conditions during the AM and PM peak periods with the improvements identified in the previous

section. Assuming the connections at both New Log Road and Springs Road are provided, there are no improvements recommended at this intersection as part of this TIS.

Springs Road and SH 94

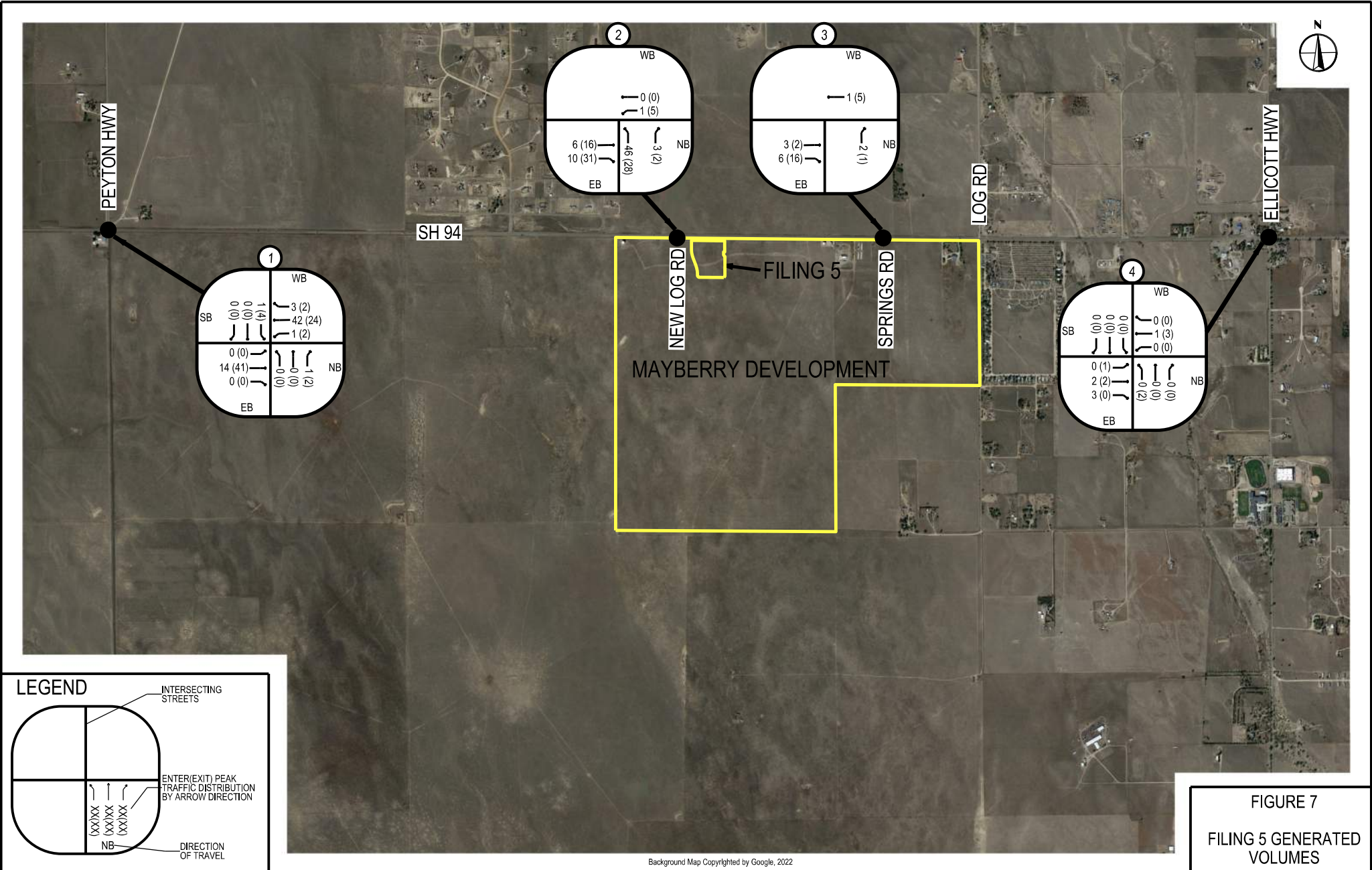
The intersection will operate at LOS A and B under 2024 site plus forecasted traffic conditions during the AM and PM peak periods, respectively. Assuming the connections at both New Log Road and Springs Road are provided, there are no improvements recommended at this intersection as part of this TIS.

Ellicott Highway and SH 94

The intersection will operate at LOS C under 2024 site plus forecasted traffic conditions during the AM and PM peak periods. There are no improvements recommended at this intersection as part of this TIS.

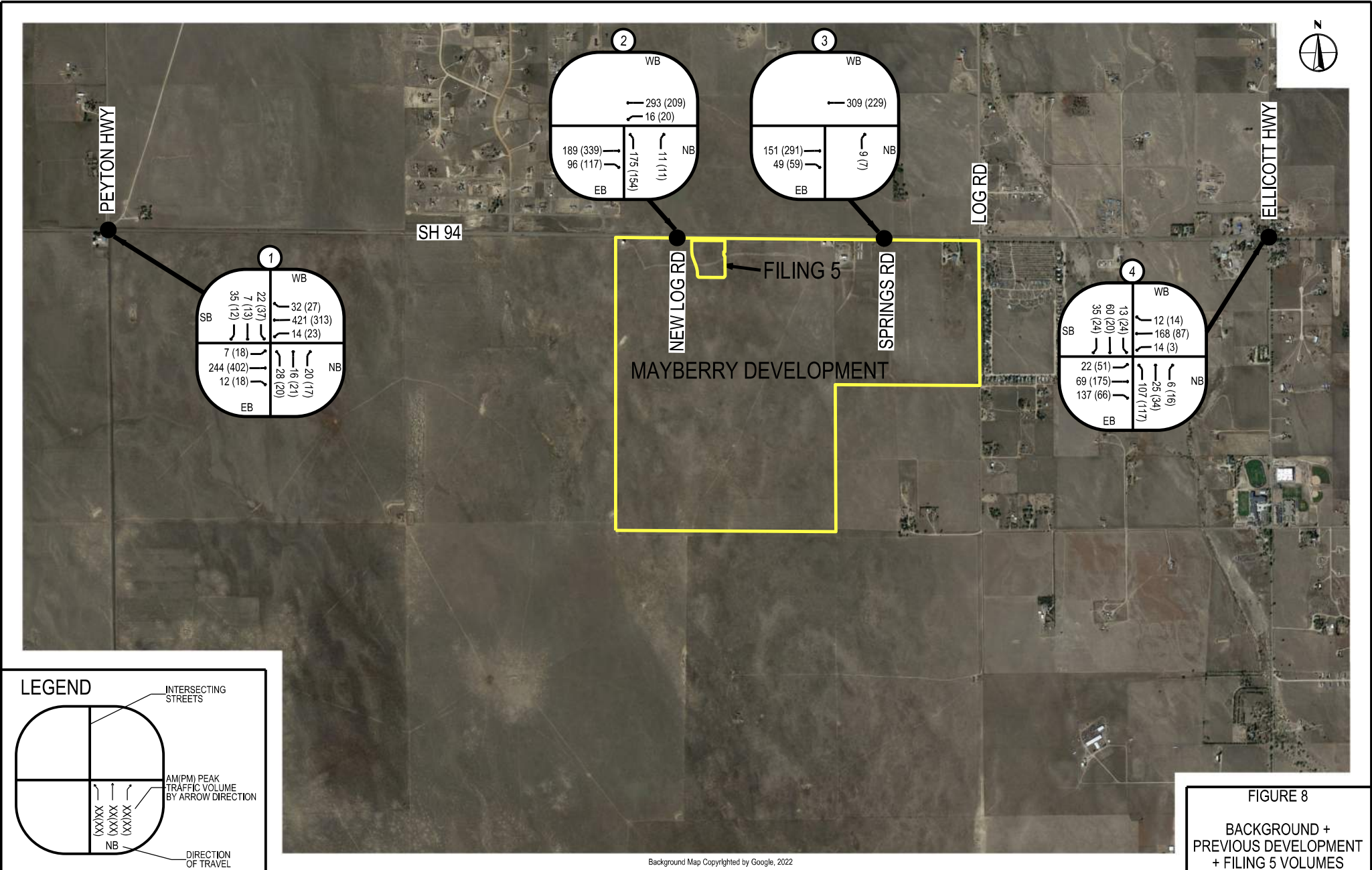
Table 6: Filing 5 Level of Service Summary

Intersection	2024 Background + Previous Filings + Filing 5	
	AM	PM
Peyton Highway and SH 94	C (17.8)	C (23.1)
New Log Road and SH 94	C (17.2)	C (18.6)
Springs Road and SH 94	A (9.3)	B (10.2)
Ellicott Highway and SH 94	C (17.1)	C (16.8)

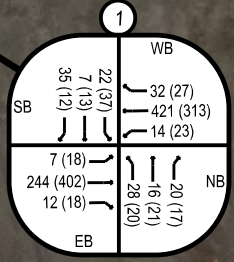


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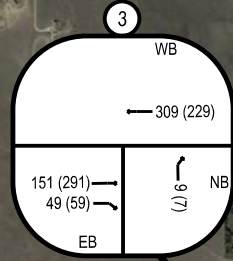
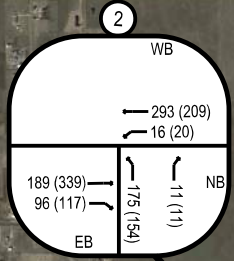
FIGURE 7
FILING 5 GENERATED VOLUMES



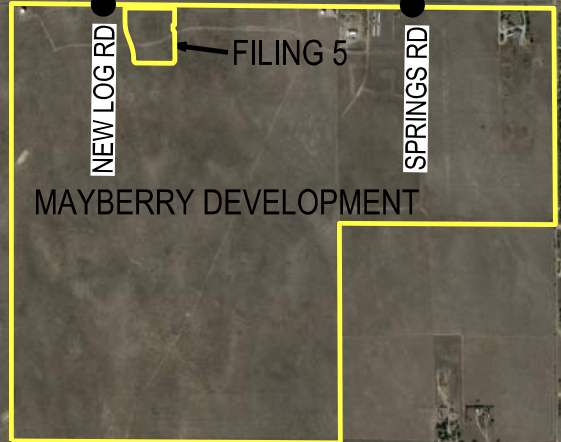
PEYTON HWY



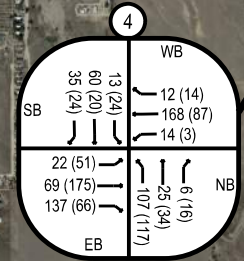
SH 94



LOG RD



ELLICOTT HWY



See comments regarding filing 4 above and include filing 4 traffic in your analysis.

Summary of Findings

Intersections adjacent to the development on SH 94 will operate at LOS C or better for all scenarios analyzed in this TIA. Therefore, the infrastructure that is anticipated to be in place by the time Filing 3 and Filing 5 are developed and occupied will have the capacity to handle the generated traffic. No improvements are needed for the addition of Filing 5 to the Mayberry Communities Development. Intersection LOS and delay results are presented in Table 7.

Table 7: Level of Service Summary

Intersection	2024 Existing		2024 Background + Previous Filings		2024 Background + Previous Filings + Filing 5	
	AM	PM	AM	PM	AM	PM
Highest delay minor street approach is reported for all unsignalized intersections.						
Peyton Highway and SH 94	B (14.1)	B (13.5)	C (16.7)	C (19.8)	C (17.8)	C (23.1)
New Log Road and SH 94	-	-	C (15.2)	C (16.4)	C (17.2)	C (18.6)
Springs Road and SH 94	-	-	A (9.2)	B (10.2)	A (9.3)	B (10.2)

-please discuss escrow requirements for intersection improvements at Hwy 94/Peyton Hwy & Hwy 94/Ellicott hwy. Each previous filing has indicated its fair share amount. Contact and coordinate with CDOT as it is anticipated that updated CDOT access permits at New Log Rd and Springs Rd will be needed.

-please address the full roadway section of New log Rd. per filing 3 TIS it identified that beyond filing 3 (ADT above 3000) required the full construction of New Log Rd. Per previous traffic studies it appears that this development will trigger the construction of the full roadway cross section identified in the previous PUDSP plan. Please provide a table of the required/recommended improvements as done in the previous traffic studies.

-please analyze the intersection of Village Main St and Market Place Dr. Identify whether any improvements (i.e. turn lanes) are needed due to this developments traffic. Identify whether the current classification of the roadways (non residential collector for Village Main & Urban Local for Marketplace Dr) are adequate due to this developments traffic impacts or do they need to be upgraded. Identify any turn lanes that may be needed on Marketplace Dr into the site access points due to this developments traffic.

- Analyze the sites access points per ECM 2.4. State whether the proposed access points meet ECM criteria.

-Please address/analyze the construction of the full section of Mayberry Drive (east/west roadway). Previous TIS identified that this was to be determined with future PUD applications. Analyze and state whether this will be triggered due to this developments traffic.

-Please analyze the westbound left turn deceleration lane at New Log and Hwy 94. Indicate whether it needs lengthening. The previous TIS identified that this may be needed with future development.

-Please analyze and indicate if an eastbound right turn acceleration lane is required at New Log/Hwy 94 due to this developments impacts.

-Please identify the road classification of any proposed internal private roadway(s).

Per ECM Appendix B.8 traffic report standards, please provide the following:

- clearly state in the text and supporting documentation what the ADT is at all accesses currently, at full development, and long term.
- State whether or not any improvements affected by the project are reimbursable under the current Major Transportation Corridor Plan (MTCP).
- State whether the MTCP or other approved corridor study calls for the construction of improvements in the immediate area.
- State what the current applicable Transportation Impact Fees are and what option the developer will be selecting for payment. If the site is in a special district, so state and summarize the applicable fees.
- Include LOS for all affected intersections.
- Include an engineer's certification page with the engineer's stamp, signature, and date. The statement must read as follows:

- "The attached traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports."

- Include a developer's statement on the certification page. The statement must read as follows:

"I, the Developer, have read and will comply with all commitments made on my behalf within this report." Include a printed or typed developer name and address as well as a signature block.

Appendix A: Highway Capacity Manual Description

HCM Unsignalized Intersection Level of Service

Unsignalized intersections were analyzed for this study. Unsignalized intersection LOS is defined in terms of average control delay and, in some cases, volume to capacity (v/c) ratio. Control delay is that portion of total delay attributed to traffic control measures, either traffic signals or stop signs. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay.

For two-way stop-controlled intersections, the analysis method assumes that major street-through traffic is not affected by minor street flows. Major street left-turning traffic and the traffic on the minor approaches will be affected by opposing movements. Stop or yield signs are used to assign the right-of-way to the major street, and this designation forces drivers on the controlled street to judgmentally select gaps in the major street flow through which to execute crossing or turning maneuvers. Thus, the capacity of the controlled legs is based on two factors:

- The distribution of gaps in the major street traffic stream.
- Driver judgment in selecting gaps through which to execute their desired maneuvers.

The LOS procedure computes a capacity for each movement based on the critical time gap required to complete the maneuver and the volume of traffic that is opposing the movement. The average control delay for any particular movement is calculated as a function of the capacity of the approach and the degree of saturation (v/c ratio). The degree of saturation is defined as the volume for a movement, expressed as an hourly flow rate, divided by the movement's capacity, expressed as an hourly flow rate. With the HCM 6 methodology (Ref. 5), overall intersection LOS is best quantified based on minor street movement average control delay. The HCM 6 methodology adjusts individual movement delay to account for a degree of saturation (v/c ratio) that is greater than 1.0. Those movements are assigned a LOS of F, regardless of the average control delay. Engineering judgment must be used to determine which minor street movement controls for overall intersection LOS and whether unacceptable LOS on minor street movements appropriately reflects unacceptable LOS for the overall intersection.

Table 2 shows the relationship between the average control delay and the LOS. The LOS range for unsignalized intersections is different than that for signalized intersections, and this difference is because drivers expect different levels of performance from other kinds of transportation facilities. Unsignalized intersections carry less traffic volume than signalized intersections, and delays at unsignalized intersections are variable. For these reasons, control delay would be less for an unsignalized intersection than for a signalized intersection. The overall approach LOS is computed as a weighted average of the vehicle delay for each movement; therefore, an approach may have an overall LOS of C or D and have individual movements, which are LOS E or F.

Analysis was performed using the microcomputer program "Synchro 11" (Ref. 6), based on the procedures contained in the Highway Capacity Manual.

**Table 1: Unsignalized Intersection:
Level of Service Measurement**

Level of Service	Control Delay Per Vehicle (sec)
A	< 10
B	> 10 and < 15
C	> 15 and < 25
D	> 25 and < 35
E	> 35 and < 50
F	> 50

Appendix B: Synchro Outputs

Intersection						
Int Delay, s/veh	3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	183	86	15	293	129	8
Future Vol, veh/h	183	86	15	293	129	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	570	570	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	199	93	16	318	140	9

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	292	0	549
Stage 1	-	-	-	-	199
Stage 2	-	-	-	-	350
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	1270	-	497
Stage 1	-	-	-	-	835
Stage 2	-	-	-	-	713
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1270	-	491
Mov Cap-2 Maneuver	-	-	-	-	491
Stage 1	-	-	-	-	835
Stage 2	-	-	-	-	704

Approach	EB	WB	NB
HCM Control Delay, s	0	0.4	14.9
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	491	842	-	-	1270	-
HCM Lane V/C Ratio	0.286	0.01	-	-	0.013	-
HCM Control Delay (s)	15.2	9.3	-	-	7.9	-
HCM Lane LOS	C	A	-	-	A	-
HCM 95th %tile Q(veh)	1.2	0	-	-	0	-

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↑		↗
Traffic Vol, veh/h	148	43	0	308	0	7
Future Vol, veh/h	148	43	0	308	0	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	161	47	0	335	0	8

Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	-	-	-	185
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.318
Pot Cap-1 Maneuver	-	-	0	-	0	857
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	857
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	9.2
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT
Capacity (veh/h)	857	-	-	-
HCM Lane V/C Ratio	0.009	-	-	-
HCM Control Delay (s)	9.2	-	-	-
HCM Lane LOS	A	-	-	-
HCM 95th %tile Q(veh)	0	-	-	-

HCM 6th TWSC
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Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	21	68	134	14	167	12	106	25	6	13	60	35
Future Vol, veh/h	21	68	134	14	167	12	106	25	6	13	60	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	200	-	-	400	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	23	74	146	15	182	13	115	27	7	14	65	38

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	195	0	0	220	0	0	463	418	147	429	485	189
Stage 1	-	-	-	-	-	-	193	193	-	219	219	-
Stage 2	-	-	-	-	-	-	270	225	-	210	266	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1378	-	-	1349	-	-	509	526	900	536	482	853
Stage 1	-	-	-	-	-	-	809	741	-	783	722	-
Stage 2	-	-	-	-	-	-	736	718	-	792	689	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1378	-	-	1349	-	-	426	511	900	500	469	853
Mov Cap-2 Maneuver	-	-	-	-	-	-	426	511	-	500	469	-
Stage 1	-	-	-	-	-	-	795	728	-	770	714	-
Stage 2	-	-	-	-	-	-	632	710	-	744	677	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.7			0.6			16.9			13.2		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	450	1378	-	-	1349	-	-	554
HCM Lane V/C Ratio	0.331	0.017	-	-	0.011	-	-	0.212
HCM Control Delay (s)	16.9	7.7	-	-	7.7	-	-	13.2
HCM Lane LOS	C	A	-	-	A	-	-	B
HCM 95th %tile Q(veh)	1.4	0.1	-	-	0	-	-	0.8

HCM 6th TWSC
14: Peyton & SH 94 EB

02/01/2023

Intersection												
Int Delay, s/veh	2.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↶	↷		↶	↷			↕			↕	
Traffic Vol, veh/h	7	230	12	13	379	30	28	16	18	20	7	35
Future Vol, veh/h	7	230	12	13	379	30	28	16	18	20	7	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	532	-	-	532	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	8	250	13	14	412	33	30	17	20	22	8	38

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	445	0	0	263	0	0	753	746	257	748	736	429
Stage 1	-	-	-	-	-	-	273	273	-	457	457	-
Stage 2	-	-	-	-	-	-	480	473	-	291	279	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1115	-	-	1301	-	-	326	342	782	329	346	626
Stage 1	-	-	-	-	-	-	733	684	-	583	568	-
Stage 2	-	-	-	-	-	-	567	558	-	717	680	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1115	-	-	1301	-	-	297	336	782	304	340	626
Mov Cap-2 Maneuver	-	-	-	-	-	-	297	336	-	304	340	-
Stage 1	-	-	-	-	-	-	728	679	-	579	562	-
Stage 2	-	-	-	-	-	-	520	552	-	676	675	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			0.2			16.7			14.8		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	376	1115	-	-	1301	-	-	436
HCM Lane V/C Ratio	0.179	0.007	-	-	0.011	-	-	0.155
HCM Control Delay (s)	16.7	8.3	-	-	7.8	-	-	14.8
HCM Lane LOS	C	A	-	-	A	-	-	B
HCM 95th %tile Q(veh)	0.6	0	-	-	0	-	-	0.5

Intersection						
Int Delay, s/veh	3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	323	86	15	209	126	9
Future Vol, veh/h	323	86	15	209	126	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	570	570	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	351	93	16	227	137	10

Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	444	0	610	351
Stage 1	-	-	-	-	351	-
Stage 2	-	-	-	-	259	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1116	-	458	692
Stage 1	-	-	-	-	713	-
Stage 2	-	-	-	-	784	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1116	-	452	692
Mov Cap-2 Maneuver	-	-	-	-	452	-
Stage 1	-	-	-	-	713	-
Stage 2	-	-	-	-	773	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.6	16
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	452	692	-	-	1116	-
HCM Lane V/C Ratio	0.303	0.014	-	-	0.015	-
HCM Control Delay (s)	16.4	10.3	-	-	8.3	-
HCM Lane LOS	C	B	-	-	A	-
HCM 95th %tile Q(veh)	1.3	0	-	-	0	-

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑		↑
Traffic Vol, veh/h	289	43	0	224	0	6
Future Vol, veh/h	289	43	0	224	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	314	47	0	243	0	7

Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	-	-	-	338
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.318
Pot Cap-1 Maneuver	-	-	0	-	0	704
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	704
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	10.2
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT
Capacity (veh/h)	704	-	-	-
HCM Lane V/C Ratio	0.009	-	-	-
HCM Control Delay (s)	10.2	-	-	-
HCM Lane LOS	B	-	-	-
HCM 95th %tile Q(veh)	0	-	-	-

Intersection												
Int Delay, s/veh	6.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	50	173	66	3	84	14	115	34	16	24	20	24
Future Vol, veh/h	50	173	66	3	84	14	115	34	16	24	20	24
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	200	-	-	400	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	54	188	72	3	91	15	125	37	17	26	22	26

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	106	0	0	260	0	0	461	444	224	464	473	99
Stage 1	-	-	-	-	-	-	332	332	-	105	105	-
Stage 2	-	-	-	-	-	-	129	112	-	359	368	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1485	-	-	1304	-	-	511	508	815	508	490	957
Stage 1	-	-	-	-	-	-	681	644	-	901	808	-
Stage 2	-	-	-	-	-	-	875	803	-	659	621	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1485	-	-	1304	-	-	466	489	815	455	471	957
Mov Cap-2 Maneuver	-	-	-	-	-	-	466	489	-	455	471	-
Stage 1	-	-	-	-	-	-	656	621	-	869	806	-
Stage 2	-	-	-	-	-	-	826	801	-	584	599	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.3			0.2			16.5			12.3		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	491	1485	-	-	1304	-	-	565
HCM Lane V/C Ratio	0.365	0.037	-	-	0.003	-	-	0.131
HCM Control Delay (s)	16.5	7.5	-	-	7.8	-	-	12.3
HCM Lane LOS	C	A	-	-	A	-	-	B
HCM 95th %tile Q(veh)	1.7	0.1	-	-	0	-	-	0.4

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Intersection												
Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	18	361	18	21	289	25	20	21	15	33	13	12
Future Vol, veh/h	18	361	18	21	289	25	20	21	15	33	13	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	532	-	-	532	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	20	392	20	23	314	27	22	23	16	36	14	13

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	341	0	0	412	0	0	829	829	402	836	826	328
Stage 1	-	-	-	-	-	-	442	442	-	374	374	-
Stage 2	-	-	-	-	-	-	387	387	-	462	452	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1218	-	-	1147	-	-	290	306	648	287	307	713
Stage 1	-	-	-	-	-	-	594	576	-	647	618	-
Stage 2	-	-	-	-	-	-	637	610	-	580	570	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1218	-	-	1147	-	-	267	295	648	256	296	713
Mov Cap-2 Maneuver	-	-	-	-	-	-	267	295	-	256	296	-
Stage 1	-	-	-	-	-	-	584	567	-	637	606	-
Stage 2	-	-	-	-	-	-	599	598	-	534	561	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.5			18.3			19.8		
HCM LOS							C			C		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	331	1218	-	-	1147	-	-	306
HCM Lane V/C Ratio	0.184	0.016	-	-	0.02	-	-	0.206
HCM Control Delay (s)	18.3	8	-	-	8.2	-	-	19.8
HCM Lane LOS	C	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.7	0	-	-	0.1	-	-	0.8

Intersection						
Int Delay, s/veh	4.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	189	96	16	293	175	11
Future Vol, veh/h	189	96	16	293	175	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	570	570	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	205	104	17	318	190	12

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	309	0	557
Stage 1	-	-	-	-	205
Stage 2	-	-	-	-	352
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	1252	-	491
Stage 1	-	-	-	-	829
Stage 2	-	-	-	-	712
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1252	-	484
Mov Cap-2 Maneuver	-	-	-	-	484
Stage 1	-	-	-	-	829
Stage 2	-	-	-	-	702

Approach	EB	WB	NB
HCM Control Delay, s	0	0.4	16.7
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	484	836	-	-	1252	-
HCM Lane V/C Ratio	0.393	0.014	-	-	0.014	-
HCM Control Delay (s)	17.2	9.4	-	-	7.9	-
HCM Lane LOS	C	A	-	-	A	-
HCM 95th %tile Q(veh)	1.8	0	-	-	0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↑		↗
Traffic Vol, veh/h	151	49	0	309	0	9
Future Vol, veh/h	151	49	0	309	0	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	164	53	0	336	0	10

Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	-	-	-	191
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.318
Pot Cap-1 Maneuver	-	-	0	-	0	851
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	851
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	9.3
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT
Capacity (veh/h)	851	-	-	-
HCM Lane V/C Ratio	0.011	-	-	-
HCM Control Delay (s)	9.3	-	-	-
HCM Lane LOS	A	-	-	-
HCM 95th %tile Q(veh)	0	-	-	-

HCM 6th TWSC
 13: Ellicott Highway & SH 94 EB

02/01/2023

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	22	69	137	14	168	12	107	25	6	13	60	35
Future Vol, veh/h	22	69	137	14	168	12	107	25	6	13	60	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	200	-	-	400	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	24	75	149	15	183	13	116	27	7	14	65	38

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	196	0	0	224	0	0	469	424	150	435	492	190
Stage 1	-	-	-	-	-	-	198	198	-	220	220	-
Stage 2	-	-	-	-	-	-	271	226	-	215	272	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1377	-	-	1345	-	-	505	522	896	531	478	852
Stage 1	-	-	-	-	-	-	804	737	-	782	721	-
Stage 2	-	-	-	-	-	-	735	717	-	787	685	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1377	-	-	1345	-	-	422	507	896	495	465	852
Mov Cap-2 Maneuver	-	-	-	-	-	-	422	507	-	495	465	-
Stage 1	-	-	-	-	-	-	790	724	-	769	713	-
Stage 2	-	-	-	-	-	-	631	709	-	739	673	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.7			0.6			17.1			13.3		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	446	1377	-	-	1345	-	-	550
HCM Lane V/C Ratio	0.336	0.017	-	-	0.011	-	-	0.213
HCM Control Delay (s)	17.1	7.7	-	-	7.7	-	-	13.3
HCM Lane LOS	C	A	-	-	A	-	-	B
HCM 95th %tile Q(veh)	1.5	0.1	-	-	0	-	-	0.8

HCM 6th TWSC
14: Peyton & SH 94 EB

02/01/2023

Intersection												
Int Delay, s/veh	2.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↶	↷		↶	↷			↕			↕	
Traffic Vol, veh/h	7	244	12	14	421	32	28	16	20	22	7	35
Future Vol, veh/h	7	244	12	14	421	32	28	16	20	22	7	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	532	-	-	532	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	8	265	13	15	458	35	30	17	22	24	8	38

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	493	0	0	278	0	0	817	811	272	813	800	476
Stage 1	-	-	-	-	-	-	288	288	-	506	506	-
Stage 2	-	-	-	-	-	-	529	523	-	307	294	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1071	-	-	1285	-	-	295	313	767	297	318	589
Stage 1	-	-	-	-	-	-	720	674	-	549	540	-
Stage 2	-	-	-	-	-	-	533	530	-	703	670	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1071	-	-	1285	-	-	267	307	767	272	312	589
Mov Cap-2 Maneuver	-	-	-	-	-	-	267	307	-	272	312	-
Stage 1	-	-	-	-	-	-	715	669	-	545	534	-
Stage 2	-	-	-	-	-	-	486	524	-	660	665	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			0.2			17.8			16.1		
HCM LOS							C			C		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	350	1071	-	-	1285	-	-	393
HCM Lane V/C Ratio	0.199	0.007	-	-	0.012	-	-	0.177
HCM Control Delay (s)	17.8	8.4	-	-	7.8	-	-	16.1
HCM Lane LOS	C	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.7	0	-	-	0	-	-	0.6

Intersection						
Int Delay, s/veh	3.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	339	117	20	209	154	11
Future Vol, veh/h	339	117	20	209	154	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	570	570	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	368	127	22	227	167	12

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	495	0	639 368
Stage 1	-	-	-	-	368 -
Stage 2	-	-	-	-	271 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1069	-	440 677
Stage 1	-	-	-	-	700 -
Stage 2	-	-	-	-	775 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1069	-	431 677
Mov Cap-2 Maneuver	-	-	-	-	431 -
Stage 1	-	-	-	-	700 -
Stage 2	-	-	-	-	759 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.7	18.1
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	431	677	-	-	1069	-
HCM Lane V/C Ratio	0.388	0.018	-	-	0.02	-
HCM Control Delay (s)	18.6	10.4	-	-	8.4	-
HCM Lane LOS	C	B	-	-	A	-
HCM 95th %tile Q(veh)	1.8	0.1	-	-	0.1	-

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑		↑
Traffic Vol, veh/h	291	59	0	229	0	7
Future Vol, veh/h	291	59	0	229	0	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	316	64	0	249	0	8

Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	-	-	-	348
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.318
Pot Cap-1 Maneuver	-	-	0	-	0	695
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	695
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	10.2
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT
Capacity (veh/h)	695	-	-	-
HCM Lane V/C Ratio	0.011	-	-	-
HCM Control Delay (s)	10.2	-	-	-
HCM Lane LOS	B	-	-	-
HCM 95th %tile Q(veh)	0	-	-	-

Intersection												
Int Delay, s/veh	6.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	51	175	66	3	87	14	117	34	16	24	20	24
Future Vol, veh/h	51	175	66	3	87	14	117	34	16	24	20	24
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	200	-	-	400	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	55	190	72	3	95	15	127	37	17	26	22	26

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	110	0	0	262	0	0	469	452	226	472	481	103
Stage 1	-	-	-	-	-	-	336	336	-	109	109	-
Stage 2	-	-	-	-	-	-	133	116	-	363	372	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1480	-	-	1302	-	-	505	503	813	502	485	952
Stage 1	-	-	-	-	-	-	678	642	-	896	805	-
Stage 2	-	-	-	-	-	-	870	800	-	656	619	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1480	-	-	1302	-	-	460	483	813	449	466	952
Mov Cap-2 Maneuver	-	-	-	-	-	-	460	483	-	449	466	-
Stage 1	-	-	-	-	-	-	653	618	-	863	803	-
Stage 2	-	-	-	-	-	-	821	798	-	581	596	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.3			0.2			16.8			12.4		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	485	1480	-	-	1302	-	-	559
HCM Lane V/C Ratio	0.374	0.037	-	-	0.003	-	-	0.132
HCM Control Delay (s)	16.8	7.5	-	-	7.8	-	-	12.4
HCM Lane LOS	C	A	-	-	A	-	-	B
HCM 95th %tile Q(veh)	1.7	0.1	-	-	0	-	-	0.5

HCM 6th TWSC
 14: Peyton Highway & SH 94

02/01/2023

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	18	402	18	23	313	32	20	21	17	37	13	12
Future Vol, veh/h	18	402	18	23	313	32	20	21	17	37	13	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	532	-	-	532	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	20	437	20	25	340	35	22	23	18	40	14	13

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	375	0	0	457	0	0	908	912	447	916	905	358
Stage 1	-	-	-	-	-	-	487	487	-	408	408	-
Stage 2	-	-	-	-	-	-	421	425	-	508	497	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1183	-	-	1104	-	-	256	274	612	253	276	686
Stage 1	-	-	-	-	-	-	562	550	-	620	597	-
Stage 2	-	-	-	-	-	-	610	586	-	547	545	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1183	-	-	1104	-	-	234	263	612	222	265	686
Mov Cap-2 Maneuver	-	-	-	-	-	-	234	263	-	222	265	-
Stage 1	-	-	-	-	-	-	552	541	-	609	583	-
Stage 2	-	-	-	-	-	-	571	573	-	500	536	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.5			20.2			23.1		
HCM LOS							C			C		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	300	1183	-	-	1104	-	-	266
HCM Lane V/C Ratio	0.21	0.017	-	-	0.023	-	-	0.253
HCM Control Delay (s)	20.2	8.1	-	-	8.3	-	-	23.1
HCM Lane LOS	C	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.8	0.1	-	-	0.1	-	-	1

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Daniel Torres (19)

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Add PCD File No. PUDSP233

the filing 3 TIS is dated October 2022. revise. (see PCD file No. SF2219)

Author: Daniel Torres
Subject: Callout
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the filing 3 TIS is dated October 2022. revise. (see PCD file No. SF2219)

please include Filing 4 TIS in the list of the referenced reports.

Author: Daniel Torres
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Page Label: 5
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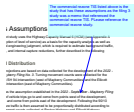
please include Filing 4 TIS in the list of the referenced reports.

please provide information as to what time period is the ADT that is shown. Are these existing values?



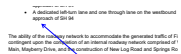
Author: Daniel Torres
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please provide information as to what time period is the ADT that is shown. Are these existing values?



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Subject: Callout
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The commercial rezone TIS listed above is the study that has these assumptions as the filing 3 study was a memo that referenced the commercial rezone TIS. PLease reference the commercial rezone study.



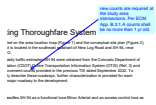
Author: Daniel Torres
Subject: Callout
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a left turn acceleration lane (northbound to westbound) is identified to be installed with filing 3. See Filing 3 TIS table 12 and update the narrative accordingly.

in the Colorado Department of Transportation System (OTIS) (Ref. 3) at dated September 2022. To information is provided for each

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September 2022.



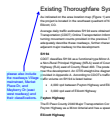
Author: Daniel Torres
Subject: Callout
Page Label: 10
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new counts are required at the study area intersections. Per ECM App. B.3.1.A counts shall be no more than 1 yr old.



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please also identify that the El Paso County 2040 Major transportation Corridors Plan (MTCP) indicates that this roadway as a 2 lane principal arterial an



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please also include the roadways Village mainstreet, Market Place Dr, and Mayberry Dr (east west roadway) and their classifications.



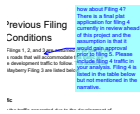
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please also include long term analysis as required per ECM appendix B.



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please also include ADT in all the figures for study area roadways.



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how about Filing 4? There is a final plat application for filing 4 currently in review ahead of this project and the assumption is that it would gain approval prior to filing 5. Please include filing 4 traffic in your analysis. Filing 4 is listed in the table below but not mentioned in the narrative.



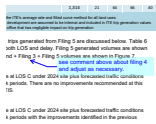
Author: Daniel Torres
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revise to include also filing 1 as stated above



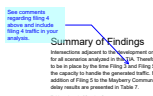
Author: Daniel Torres
Subject: Callout
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Please clarify which improvements are assumed. The filing 3 TIS identified that beyond filing 3 the full urban minor arterial (full couplet segments) is to be constructed (>3000 ADT). Is the full configuration for New Log road assumed therefore filing 4 is installing the improvements?



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see comment above about filing 4 and adjust as necessary.



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See comments regarding filing 4 above and include filing 4 traffic in your analysis.



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-please discuss escrow requirements for intersection improvements at Hwy 94/Peyton Hwy & Hwy 94/Ellicott hwy. Each previous filing has indicated its fair share amount. Contact and coordinate with CDOT as it is anticipated that updated CDOT access permits at New Log Rd and Springs Rd will be needed.

-please address the full roadway section of New log Rd. per filing 3 TIS it identified that beyond filing 3 (ADT above 3000) required the full construction of New Log Rd. Per previous traffic studies it appears that this development will trigger the construction of the full roadway cross section identified in the previous PUDSP plan. Please provide a table of the required/recommended improvements as done in the previous traffic studies.

-please analyze the intersection of Village Main St and Market Place Dr. Identify whether any improvements (i.e. turn lanes) are needed due to this developments traffic. Identify whether the current classification of the roadways (non residential collector for Village Main & Urban Local for Marketplace Dr) are adequate due to this developments traffic impacts or do they need to be upgraded. Identify any turn lanes that may be needed on Marketplace Dr into the site access points due to this developments traffic.

- Analyze the sites access points per ECM 2.4. State whether the proposed access points meet ECM criteria.

-Please address/analyze the construction of the full section of Mayberry Drive (east/west roadway). Previous TIS identified that this was to be determined with future PUD applications. Analyze and state whether this will be triggered due to this developments traffic.

-Please analyze the westbound left turn deceleration lane at New Log and Hwy 94. Indicate whether it needs lengthening. The previous TIS identified that this may be needed with future development.



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-Please analyze and indicate if an eastbound right turn acceleration lane is required at New Log/Hwy 94 due to this developments impacts.

-Please identify the road classification of any proposed internal private roadway(s).

Per ECM Appendix B.8 traffic report standards, please provide the following:

- clearly state in the text and supporting documentation what the ADT is at all accesses currently, at full development, and long term.
- State whether or not any improvements affected by the project are reimbursable under the current Major Transportation Corridors Plan (MTCP).
- State whether the MTCP or other approved corridor study calls for the construction of improvements in the immediate area.
- State what the current applicable Transportation Impact Fees are and what option the developer will be selecting for payment. If the site is in s special district, so state and summarize the applicable fees.

-Include LOS for all affected intersections.

-Include an engineer's certification page with the engineer's stamp, signature, and date. The statement must read as follows:

- "The attached traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports."

- Include a developer's statement on the certification page. The statement must read as follows:

"I, the Developer, have read and will comply with all commitments made on my behalf within this report." Include a printed or typed developer name and address as well as a signature block.