FINAL DRAINAGE PLAN

CREEKSIDE AT LORSON RANCH FILING NO. 1

APRIL 15, 2019

REV. SEPTEMBER 19, 2019

SF-19-013

Prepared for:

Lorson, LLC
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Project No. 100.045

CORE
ENGINEERING GROUP

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ENGINEER'S STATEMENT

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

33997 9-19-2019

Richard L. Schindler, P.E. #33997 Date
For and on Behalf of Core Engineering Group, LLC

OWNER'S STATEMENT

I, the Owner, have read and will comply with all the requirements specified in the drainage report and plan.

9/19/19

Lorson, LLC Date

By
Jeff Mark

Title
Manager

Address
212 N. Wahsatch Avenue, Suite 301, Colorado Springs, CO 80903

FLOODPLAIN STATEMENT

To the best of my knowledge and belief, this development is located within a designated floodplain as shown on Flood Insurance Rate Map Panel No. 08041C0957 G, dated December 7, 2018 and modified by modified per LOMR Case No. 14-08-0534P. (See Appendix A, FEMA FIRM Exhibit)

Richard L. Schindler, #33997 9-19-2019 Date

EL PASO COUNTY

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volume 1 and 2, and Engineering Criteria Manual, As Amended.

Jennifer Irvine Date
County Engineer/ECM Administrator

Conditions: ____________________

1.0 LOCATION and DESCRIPTION

Creekside at Lorson Ranch Filing No. 1 is located north of the East Tributary of Jimmy Camp Creek (Etrib). The site is located on approximately 83.088 acres of vacant land. Future plans are to develop this site into single-family residential developments. Also included in this report and plan is the proposed layout for Creekside at Lorson Ranch Filing No. 1 which is located west and north of the East Tributary of Jimmy Camp Creek. The land is currently owned by Lorson LLC or its nominees for Lorson Ranch.

The site is located in the North 1/2 of Section 23, Township 15 South and Range 65 West of the 6th Principal Meridian. The property is bounded on the north by Lorson Boulevard, on the east by the Etrib, the west by Jimmy Camp Creek, and the south by unplatted land in Lorson Ranch. For reference, a vicinity map is included in Appendix A of this report.

Conformance with applicable Drainage Basin Planning Studies
There is an existing (unapproved) DBPS for Jimmy Camp Creek prepared by Wilson & Company in 1987, and is referenced in this report. The only major drainage improvements for this study area according to the 1987 Wilson study was the reconstruction of the East Tributary of Jimmy Camp Creek (East Tributary). In 2014 a portion of the East Tributary was reconstructed from Fontaine Boulevard south 2,800 feet in accordance with the 1987 study which is located within this project. This section of the East Tributary included a trapezoidal channel section with 6:1 side slopes and a sand bottom. On March 9, 2015 a new DBPS for Jimmy Camp Creek and the East Tributary was completed by Kiowa Engineering. The Kiowa Engineering DBPS for Jimmy Camp Creek has not been adopted by El Paso County but is allowed for concept design. The concept design for the remaining portions of the Etrib include an armoring concept and full spectrum detention pond requirements. The Kiowa DBPS did not calculate drainage fees so current El Paso County drainage/bridge fees apply to this development. Per the Kiowa DBPS concept the preferred channel improvements include selective channel armoring on outer bends and a low flow channel for the East Tributary. Channel improvements in the East Tributary are potentially reimbursable against drainage fees for future development but need to be processed through the county process for reimbursement.

Conformance with Lorson Ranch MDDP1 by Pentacor Engineering
Lorson Ranch MDDP1 (October 26, 2006) includes this preliminary plan area and the East Tributary. This PDR conforms to the MDDP1 for Lorson Ranch and is referenced in this report. The major infrastructure to be constructed in this PDR site includes the Etrib armoring from the south property line of Lorson Ranch east and north to the previously reconstructed Etrib completed in 2014 and construction of several on-site detention ponds. Kiowa Engineering is currently designing this section of the East Tributary and is included in the appendix of this report. Detention/WQ Pond C1-R (existing) and several proposed detention ponds are shown within this preliminary plan area and will be designed/constructed as part of Creekside at Lorson Ranch Filing No. 1.

Reconstruction of the East Tributary of Jimmy Camp Creek
The Kiowa DBPS shows the East Tributary to be protected using selective armoring (soil rip rap) at the outside stream bends (500' minimum radius) and a stabilized low flow channel. The East Tributary has been divided into three different sections, south, middle, and north. The first section (south) is from the south property line east and north to design point ET-3 (see drainage map) and is roughly 2,900 feet in length. The south section is within this preliminary plan area and will be armored in accordance with the Kiowa DBPS and is currently being designed by Kiowa Engineering. The Etrib construction plans will be submitted for approval in conjunction with this final plat submittal and the design report is attached in appendix F of this report. The 100-year flow rate for design is 5,500cfs for the south section. The middle section is from Design Point ET-3 north 2,800 feet to the future extension of Fontaine Boulevard. The channel for this section was reconstructed and stabilized in 2014 in accordance with the 1987 Wilson DBPS. LOMR Case No. 14-08-0534P was approved by FEMA for this middle section. The northern section is from Fontaine Boulevard and extends north to the north

property line. The north section completed construction in 2018 in conformance with the Kiowa DBPS as part of Lorson Ranch East Filing No. 1 improvements. The channel consists of a stabilized low flow channel and soil rip rap armored outer bends. A CLOMR for the creek construction is approved by FEMA under Case No. 17-08-1043R. The 100-year flow rate for design is from FEMA FIS data and is from 4,400cfs to 4,750cfs for this section. The low flow channel is sized using 10% of the 100-yr FEMA flow rates and is from 440cfs to 475cfs.

Creekside at Lorson Ranch Filing No. 1 is located within the ***"Jimmy Camp Creek Drainage Basin"***, which is a fee basin in El Paso County.

See previous paragraph and Appendix F for the East Tributary of Jimmy Camp Creek Reconstruction (south section) report by Kiowa Engineering.

2.0 DRAINAGE CRITERIA

The supporting drainage design and calculations were performed in accordance with the City of Colorado Springs and El Paso County "Drainage Criteria Manual (DCM)", dated November, 1991, the El Paso County "Engineering Criteria Manual", Chapter 6 and Section 3.2.1 Chapter 13 of the City of Colorado Springs Drainage Criteria Manual dated May 2014, and the UDFCD "Urban Storm Drainage Criteria Manual" Volumes 1, 2 and 3 for inlet sizing and full spectrum ponds. No deviations from these published criteria are requested for this site. The proposed improvements to the Lorson Ranch Development will be in substantial compliance with the "Jimmy Camp Creek Drainage Basin Planning Study", prepared by Kiowa Engineering Corp., Colorado Springs, CO.

The Rational Method as outlined in Section 6.3.0 of the May 2014 "Drainage Criteria Manual" and in Section 3.2.8.F of the El Paso County "Engineering Criteria Manual" was used for basins less than 130 acres to determine the rainfall and runoff conditions for the proposed development of the site. The runoff rates for the 5-year initial storm and 100-year major design storm were calculated.

Current updates to the Drainage Criteria manual for El Paso County states the if detention is necessary, Full Spectrum Detention will be included in the design, based on this criteria, Full Spectrum Detention will be required for this development

3.0 EXISTING HYDROLOGICAL CONDITIONS

The site is currently undeveloped with native vegetation (grass with no shrubs) and slopes in a southerly direction to the East Tributary of Jimmy Camp Creek.

The Soil Conservation Service (SCS) classifies the soils within the Lorson Ranch East property as Blendon Sandy Loam (40%); Ellicott Loamy Coarse Sand (1%) Manzanst clay loam (59%) [3]. The sandy loams are considered hydrologic soil group A/B soils with moderate to moderately rapid permeability. The clay loams are considered hydrologic soil group C soils with slow permeability. For the purposes of this report the Ellicot Loamy Coarse Sand will not be used since it is only 1% of the site and is in an area that will not be disturbed. All of these soils are susceptible to erosion by wind and water, have low bearing strength, moderate shrink-swell potential, and high frost heave potential (see table 3.1 below). The clay loams are difficult to vegetate. These soils can be mitigated easily by limiting their use as topsoil.

Table 3.1: SCS Soils Survey.

Soil	Hydro. Group	Shrink/Swell Potential	Permeability	Surface Runoff Potential	Erosion Hazard
10-Blendon Sandy	B	Low	Moderately	Slow	Moderate

Loam (40%)			Rapid		
28-Ellicott Loamy Coarse Sand (1%)	A	Low	Rapid	Slow	High
52Manzanst Clay Loam (59%)	C	Moderate to High	Slow	Medium	Moderate

Excerpts from the SCS “Soil Survey of El Paso County Area, Colorado” are provided in ***Appendix A*** for further reference.

For the purpose of preparing hydrologic calculations for this report, the soil of each basin are assumed to be wholly comprised of the majority soil hydrologic group.

Portions of the site are located within the delineated 100-year floodplain of the East Tributary of Jimmy Camp Creek per the Federal Emergency Management Agency (FEMA) Flood Rate Insurance Map (FIRM) number 08041C0957 F, effective March 17, 1997 [2]. Floodplain along Jimmy Camp Creek was modified per LOMR Case No. 06-08-B643P, effective August 29, 2007 (see appendix). Floodplain along the East Tributary was modified per LOMR Case No. 14-08-0534P, effective January 29, 2015 (see appendix). Floodplain designations include Zone AE and Zone X within the property boundary. A portion of this map is provided in ***Appendix A*** for reference. A CLOMR for the creek construction by Kiowa Engineering will not be necessary since BFE’s are not changing.

Basin EX-B
This 35.5 acre basin includes the east portions of the site. Under existing conditions, this area flows overland south to the East Tributary contributes 17.6cfs and 94.0cfs for 5-year and 100-year events respectively.

Basin EX-C1
This 10.32 acre basin includes the middle portions of the site. Under existing conditions, this area flows overland south to the East Tributary contributes 5.3cfs and 29.7cfs for 5-year and 100-year events respectively.

Basin EX-D
This 29.29 acre basin includes the west portions of the site. Under existing conditions, this area flows overland south to the East Tributary contributes 8.6cfs and 57.5cfs for 5-year and 100-year events respectively. A very small portion of the runoff at the south property line of Lorson Ranch flows south onto the golf course property but was not calculated because the proposed Pond CR2 located next to the south property line will capture all the flow from the developed areas of the site.

4.0 DEVELOPED HYDROLOGICAL CONDITIONS

Hydrology for the **Creekside at Lorson Ranch Filing No. 1** drainage report was based on the City of Colorado Springs/El Paso County Drainage Criteria. Sub-basins that lie within this project were determined and the 5-year and 100-year peak discharges for the developed conditions have been presented in this report. Based on these flows, storm inlets will be added when the street capacity is exceeded.

This site can be broken into two soil types. The west portions are Soil Type B and the east portions are Soil Type C. See Appendix A for SCS Soils Map.

The time of concentration for each basin and sub-basin was developed using an overland, ditch, street and pipe flow components. The maximum overland flow length for developed conditions was limited to 100 feet. Travel time velocities ranged from 2 to 6 feet per second. The travel time calculations are included in the back of this report. Runoff coefficients for the various land uses were obtained from the City of Colorado Springs/El Paso County Drainage Criteria Manual.

Drainage concepts for each of the basins are briefly discussed as follow:

Basin C1.1

This basin consists of runoff from residential development. Runoff will be directed west in Kalama Drive to Design Point 1 in curb/gutter where it will be collected by a Type R inlet on Alsea Drive. The developed flow from this basin is 3.8cfs and 8.4cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.2

This basin consists of runoff from residential development. Runoff will be directed west in Castor Drive to Design Point 1 in curb/gutter where it will be collected by a Type R inlet on Alsea Drive. The developed flow from this basin is 5.4cfs and 12.1cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.3-C1.4

These basins consist of runoff from residential development. Runoff will be directed west in Kalama Drive to Design Point 2 in curb/gutter where it will be collected by a Type R inlet on Alsea Drive. The developed flow from these basins is 2.2cfs/ 4.9cfs for the 5/100-year storm event for Basin C1.3 and 4.5cfs/ 10.0cfs for the 5/100-year storm event for Basin C1.3. See the appendix for detailed calculations.

Basin C1.5

This basin consists of runoff from residential development. Runoff will be directed to Design Point 3 in curb/gutter where it will be collected by a Type R inlet on Alsea Drive. The developed flow from this basin is 0.4cfs and 1.0cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.6

This basin consists of runoff from residential development. Runoff will be directed west in Castor Drive to Design Point 6 in curb/gutter where it will be collected by a Type R inlet on Castor Drive. The developed flow from this basin is 1.5cfs and 3.3cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.7

This basin consists of runoff from residential development. Runoff will be directed east in Castor Drive to Design Point 6 in curb/gutter where it will be collected by a Type R inlet on Castor Drive. The developed flow from this basin is 3.1cfs and 6.8cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.8

This basin consists of runoff from residential development. Runoff will be directed east in Castor Drive to Design Point 6 in curb/gutter where it will be collected by a Type R inlet on Castor Drive. The developed flow from this basin is 1.6cfs and 3.5cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.9

This basin consists of runoff from residential development. Runoff will be directed west in Castor Drive to Design Point 10 in curb/gutter where it will be collected by a Type R inlet on Castor Drive. The developed flow from this basin is 4.9cfs and 10.8cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.10-C1.11

These basins consist of runoff from residential development. Runoff will be directed north in Maidford Drive to Design Point 2 in curb/gutter on Castor Drive. The developed flow from these basins is 0.4cfs/ 0.8cfs for the 5/100-year storm event for Basin C1.10 and 0.4cfs/ 0.9cfs for the 5/100-year storm event for Basin C1.11. See the appendix for detailed calculations.

Basin C1.12

This basin consists of runoff from residential development. Runoff will be directed west in Castor Drive to Design Point 10 in curb/gutter where it will be collected by a Type R inlet on Castor Drive. The developed flow from this basin is 2.5cfs and 5.5cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.13

This basin consists of runoff from residential development. Runoff will be directed east in Castor Drive to Design Point 10 in curb/gutter where it will be collected by a Type R inlet on Castor Drive. The developed flow from this basin is 1.4cfs and 3.0cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.14

This basin consists of runoff from residential development. Runoff will be directed east in Castor Drive to Design Point 10 in curb/gutter where it will be collected by a Type R inlet on Castor Drive. The developed flow from this basin is 2.3cfs and 5.1cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.15

This basin consists of runoff from residential development. Runoff will be directed south in Maidford Drive Design Point 11 in curb/gutter where it will be collected by a Type R inlet. The developed flow from this basin is 2.7cfs and 6.1cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.16

This basin consists of runoff from residential development. Runoff will be directed south in Maidford Drive Design Point 11 in curb/gutter where it will be collected by a Type R inlet. The developed flow from this basin is 1.1cfs and 2.5cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.17

This basin consists of runoff from residential development. Runoff will be directed south overland to Design Point 12 where it will be collected by a CDOT Type D inlet. The developed flow from this basin is 2.9cfs and 6.3cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C1.18

This basin consists of runoff from residential development and open space areas draining directly to Pond C1-R. Runoff will be directed overland to Pond C1-R. The developed flow from this basin is 5.7cfs and 19.5cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C2

This basin consists of runoff from open space areas draining directly to the East Tributary. The developed flow from this basin is 7.4cfs and 16.4cfs for the 5/100-year storm event.

Basin C4

This basin consists of runoff from backyards of residential development and open space areas draining directly to the East Tributary. The developed flow from this basin is 4.1cfs and 9.2cfs for the 5/100-year storm event. The backyard runoff will cross a grass buffer BMP prior to entering the East Tributary. See the appendix for detailed calculations. A deviation has been submitted with this preliminary plan.

Basin C5.1

This basin consists of runoff from residential development. Runoff will be directed south in Yazoo Drive Design Point 15 in curb/gutter where it will be collected by a Type R inlet. The developed flow from this basin is 2.2cfs and 3.7cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin C5.2

This basin consists of runoff from backyards of residential development and open space areas draining to Pond CR3. The developed flow from this basin is 1.3cfs and 2.3cfs for the 5/100-year storm event. The runoff will be detained/treated in Pond CR3 prior to entering the East Tributary. See the appendix for detailed calculations.

Overall Basin C5

This overall basin consists of runoff from backyards of residential development and open space areas draining directly to Pond CR3. The developed flow from this overall basin is 3.5cfs and 6.0cfs for the 5/100-year storm event. The runoff will be detained/treated in Pond CR3 prior to entering the East Tributary. See the appendix for detailed calculations.

Basin C6

This basin consists of runoff from backyards of residential development and open space areas draining directly to Pond CR3. The developed flow from this basin is 1.5cfs and 3.3cfs for the 5/100-year storm event. The runoff will be detained/treated in Pond CR3 prior to entering the East Tributary. See the appendix for detailed calculations.

Basin D1.1

This basin consists of runoff from backyards of residential development and open space areas draining south to an 18" end section at Design Point 16. The developed flow from this basin is 2.1cfs and 4.6cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D1.2

This basin consists of runoff from Lorson Boulevard west of Tensas Drive. The runoff flows east to Tensas Drive then flows south in Tensas Drive. The developed flow from this basin is 2.2cfs and 3.9cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D1.3

This basin consists of runoff from residential development. Runoff will be directed west in Castor Drive to Design Point 17 at Tensas Drive. The developed flow from this basin is 0.8cfs and 1.7cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D1.4

This basin consists of runoff from residential development. Runoff will be directed south in Castor Drive to Design Point 18. The developed flow from this basin is 2.1cfs and 4.7cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D1.5

This basin consists of runoff from residential development. Runoff will be directed south in Castor Drive to Design Point 23 in curb/gutter where it will be collected by a Type R inlet. The developed flow from this basin is 1.9cfs and 4.1cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D1.6
This basin consists of runoff from residential development. Runoff will be directed south in Castor Drive to Design Point 20 in curb/gutter. The developed flow from this basin is 2.2cfs and 4.8cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D1.7
This basin consists of runoff from residential development. Runoff will be directed southwest in Winnicut Drive to Design Point 20 in curb/gutter. The developed flow from this basin is 2.2cfs and 4.9cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D1.8
This basin consists of runoff from residential development. Runoff will be directed southwest in Winnicut Drive to Design Point 21 in curb/gutter. The developed flow from this basin is 1.7cfs and 3.7cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D1.9
This basin consists of runoff from residential development. Runoff will be directed south in Castor Drive to Design Point 23 in curb/gutter where it will be collected by a Type R inlet. The developed flow from this basin is 0.5cfs and 1.1cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Overall Basin D1
This overall basin consists of runoff from backyards of residential development and open space areas draining directly to Pond CR2 and is the total flow in the storm sewer at Design Point 23. The developed flow from this overall basin is 12.4cfs and 26.7cfs for the 5/100-year storm event. The runoff will be detained/treated in Pond CR2 prior to entering the East Tributary. See the appendix for detailed calculations.

Basin D2
This basin consists of runoff from backyards of residential development and open space areas draining directly to Jimmy Camp Creek. The developed flow from this basin is 2.4cfs and 5.2cfs for the 5/100-year storm event. The runoff will cross a grass buffer BMP prior to entering Jimmy Camp Creek. See the appendix for detailed calculations. A deviation must be provided at the final plat stage for the offsite runoff to be treated with a grass buffer.

Basin D3
This basin consists of runoff from open space areas draining directly to Jimmy Camp Creek. The developed flow from this basin is 0.5cfs and 2.2cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D4
This basin consists of runoff from backyards of residential development and open space areas draining directly to the East Tributary. The developed flow from this basin is 2.8cfs and 6.1cfs for the 5/100-year storm event. The runoff will cross a grass buffer BMP prior to entering the East Tributary. See the appendix for detailed calculations. A deviation must be provided at the final plat stage for the offsite runoff to be treated with a grass buffer.

Basin D5
This basin consists of runoff from backyards of residential development and open space areas draining directly to Pond CR2 which is a WQ pond. The developed flow from this basin is 1.4cfs and 4.5cfs for the 5/100-year storm event. See the appendix for detailed calculations.

Basin D6

This basin consists of runoff from open space areas draining south offsite onto the golf course as in existing conditions. No grading will be done in this basin and it will have the same drainage characteristics as in pre-developed conditions. The developed flow from this basin is 0.1cfs and 0.6cfs for the 5/100-year storm event. This flow is the same as pre-developed conditions. See the appendix for detailed calculations.

See the Developed Conditions Hydrology Calculations in the back of this report and the Developed Conditions Drainage Map (Map Pocket) for the 5-year and 100-year storm event amounts.

5.0 HYDRAULIC SUMMARY

The sizing of the hydraulic structures and detentions ponds were prepared by using the *StormSewers* and *Hydrographs* computer software programs developed by Intellisolve, which conforms to the methods outlined in the "City of Colorado Springs/El Paso County Drainage Criteria Manual". Street capacities and Inlets were sized by Denver Urban Drainage's xcel spreadsheet UD-Inlet.

It is the intent of this drainage report to use the proposed curb/gutter and storm sewer in the streets to convey runoff to detention and water quality ponds then to the East Tributary of Jimmy Camp Creek. Inlet size and location are preliminary only as shown on the storm sewer layout in the appendix. See Appendix C for detailed hydraulic calculations and the storm sewer model.

Table 1: Street Capacities (100-year capacity is only ½ of street)

	Residential Local		Residential Collector		Principal Arterial	
Street Slope	5-year	100-year	5-year	100-year	5-year	100-year
0.5%	6.3	26.4	9.7	29.3	9.5	28.5
0.6%	6.9	28.9	10.6	32.1	10.4	31.2
0.7%	7.5	31.2	11.5	34.6	11.2	33.7
0.8%	8.0	33.4	12.3	37.0	12.0	36.0
0.9%	8.5	35.4	13.0	39.3	12.7	38.2
1.0%	9.0	37.3	13.7	41.4	13.4	40.2
1.4%	10.5	44.1	16.2	49.0	15.9	47.6
1.8%	12.0	45.4	18.4	50.4	18.0	50.4
2.2%	13.3	42.8	19.4	47.5	19.5	47.5
2.6%	14.4	40.7	18.5	45.1	18.5	45.1
3.0%	15.5	39.0	17.7	43.2	17.8	43.2
3.5%	16.7	37.2	16.9	41.3	17.0	41.3
4.0%	17.9	35.7	16.2	39.7	16.3	29.7
4.5%	19.0	34.5	15.7	38.3	15.7	38.3
5.0%	19.9	33.4	15.2	37.1	15.2	37.1

Note: all flows are in cfs (cubic feet per second)

Drainage calculations for Lorson Boulevard can be found in Project CDR 18-006 and are not included in this report.

Design Point 1

Design Point 1 is located at a low point in Alsea Drive (east side)

(5-year storm)

Tributary Basins: C1.1-C1.2 **Inlet/MH Number:** Inlet DP-1

Upstream flowby: 0cfs **Total Street Flow:** 9.1cfs

Flow Intercepted: 9.1cfs **Flow Bypassed:** 0

Inlet Size: 15' type R, sump

Street Capacity: Street slope = 1.5%, capacity = 10.9cfs, capacity okay

(100-year storm)

Tributary Basins: C1.1-C1.2 **Inlet/MH Number:** Inlet DP-1

Upstream flowby: 0cfs **Total Street Flow:** 20.2cfs

Flow Intercepted: 20.2cfs **Flow Bypassed:** 0

Inlet Size: 15' type R, sump

Street Capacity: Street slope = 1.5%, capacity = 44.4cfs (half street) is okay

Design Point 2

Design Point 2 is located on Alsea Drive and is located north of Design Point 3. This design point was added to verify the street capacity of Alsea Drive on the north side of Inlet DP-3. The total street flow is 5.7cfs and 12.6cfs in the 5/100-year storm events from Basins C1.3 & C1.4. The street capacity of Alsea Drive at 1.7% slope is 11.3cfs (5-yr) and 44.8cfs (100-yr). The street capacity is not exceeded north of Inlet DP-3.

Design Point 3

Design Point 3 is located at a low point in Alsea Drive (west side)

(5-year storm)

Tributary Basins: C1.3-C1.5 **Inlet/MH Number:** Inlet DP-3

Upstream flowby: 0cfs **Total Street Flow:** 6.0cfs

Flow Intercepted: 6.0cfs **Flow Bypassed:** 0

Inlet Size: 15' type R, sump

Street Capacity: Street slope = 1.5%, capacity = 10.9cfs, capacity okay

(100-year storm)

Tributary Basins: C1.3-C1.5 **Inlet/MH Number:** Inlet DP-3

Upstream flowby: 0cfs **Total Street Flow:** 13.3cfs

Flow Intercepted: 13.3cfs **Flow Bypassed:** 0

Inlet Size: 15' type R, sump

Street Capacity: Street slope = 1.5%, capacity = 44.4cfs (half street) is okay

Design Point 4

Design Point 4 is the total pipe flow in storm sewer from Alsea Drive to Pond C1-R and is located west of Design Point 3. The total pipe flow is 15.0cfs and 33.4cfs in the 5/100-year storm events. Since there is a low point in Alsea Drive an emergency overflow swale must be constructed from Alsea Drive to Pond C1-R for 33.4cfs. The overflow swale has an 8' bottom, 4:1 side slopes, 1.3% slope, and flows at a 0.69' flow depth.

Design Point 5

Design Point 5 is located on the north side of Castor Drive and is located west of Design Point 6. This design point was added to verify the street capacity of Castor Drive on the north side of the street. The total street flow is 4.1cfs and 9.1cfs in the 5/100-year storm events from Basins C1.7 & C1.8. The street capacity of Castor Drive at 0.65% slope is 7.2cfs (5-yr) and 30.0cfs (100-yr). The street capacity is not exceeded west of Inlet DP-6.

Design Point 6

Design Point 6 is located at a low point in Castor Drive adjacent to Pond C1-R (north side of street)

(5-year storm)
Tributary Basins: C1.6-C1.8
Inlet/MH Number: Inlet DP-6
Upstream flowby: 0cfs
Total Street Flow: 5.3cfs

Flow Intercepted: 5.3cfs
Flow Bypassed: 0
Inlet Size: 10' type R, sump

Street Capacity: Street slope = 0.65%, capacity = 7.2cfs, capacity okay

(100-year storm)
Tributary Basins: C1.6-C1.8
Inlet/MH Number: Inlet DP-6
Upstream flowby: 0cfs
Total Street Flow: 11.8cfs

Flow Intercepted: 11.8cfs
Flow Bypassed: 0
Inlet Size: 10' type R, sump

Street Capacity: Street slope = 0.65%, capacity = 30.0cfs (half street) is okay

Design Point 7

Design Point 7 is located on the south side of Castor Drive and is located west of Maidford Drive. This design point was added to verify the street capacity of Castor Drive on the south side of the street. The total street flow is 5.4cfs and 12.1cfs in the 5/100-year storm events from Basins C1.9 - C1.11. The street capacity of Castor Drive at 0.7% slope is 7.5cfs (5-yr) and 31.2cfs (100-yr). The street capacity is not exceeded at this design point.

Design Point 8

Design Point 8 is located on the south side of Castor Drive and is located east of Design Point 10. This design point was added to verify the street capacity of Castor Drive on the south side of the street on the east side of Inlet DP-10. The total street flow is 5.7cfs and 12.8cfs in the 5/100-year storm events from Basins C1.9 - C1.12. The street capacity of Castor Drive at 0.7% slope is 7.5cfs (5-yr) and 31.2cfs (100-yr). The street capacity is not exceeded at this design point.

Design Point 9

Design Point 9 is located on the south side of Castor Drive and is located west of Design Point 10. This design point was added to verify the street capacity of Castor Drive on the south side of the street on the west side of Inlet DP-10. The total street flow is 3.2cfs and 7.0cfs in the 5/100-year storm events from Basins C1.13 - C1.14. The street capacity of Castor Drive at 0.65% slope is 7.2cfs (5-yr) and 30.0cfs (100-yr). The street capacity is not exceeded at this design point.

Design Point 10

Design Point 10 is located at a low point in Castor Drive adjacent to Pond C1-R (south side of street)

(5-year storm)
Tributary Basins: C1.9-C1.14 **Inlet/MH Number:** Inlet DP-10
Upstream flowby: 0cfs **Total Street Flow:** 9.7cfs

Flow Intercepted: 9.7cfs **Flow Bypassed:** 0
Inlet Size: 15’ type R, sump

Street Capacity: Street slope = 0.65%, capacity = 7.2cfs, capacity okay since half flow from east

(100-year storm)
Tributary Basins: C1.9-C1.14 **Inlet/MH Number:** Inlet DP-10
Upstream flowby: 0cfs **Total Street Flow:** 21.5cfs

Flow Intercepted: 21.5cfs **Flow Bypassed:** 0
Inlet Size: 15’ type R, sump

Street Capacity: Street slope = 0.65%, capacity = 30.0cfs (half street) is okay

Design Point 11

Design Point 11 is located at a low point in Maidford Drive.

(5-year storm)
Tributary Basins: C1.15-C1.16 **Inlet/MH Number:** Inlet DP-11
Upstream flowby: 0cfs **Total Street Flow:** 3.7cfs

Flow Intercepted: 3.7cfs **Flow Bypassed:** 0
Inlet Size: 15’ type R, sump

Street Capacity: Street slope = 0.7%, capacity = 7.5cfs, capacity okay

(100-year storm)
Tributary Basins: C1.15-C1.16 **Inlet/MH Number:** Inlet DP-11

Upstream flowby: 0cfs **Total Street Flow:** 8.3cfs

Flow Intercepted: 8.3cfs **Flow Bypassed:** 0
Inlet Size: 15’ type R, sump

Street Capacity: Street slope = 0.7%, capacity = 31.2cfs (half street) is okay

Design Point 12

Design Point 12 is located south of Castor Drive and west of Maidford Drive and Design Point 11. This design point was added to verify flow to Inlet DP-12 from Basin C1.17. The total flow in the backyard swale is 2.9cfs and 6.3cfs in the 5/100-year storm events from Basins C1.17 . A CDOT type D inlet will capture the flow at this design point and convey it via storm sewer to Pond C1-R .

Design Point 13

Design Point 13 is located on the north of Castor Drive and is the total flow in storm sewer entering Pond C1-R from Design Point 11 & 12. The total flow in the storm sewer is 6.3cfs and 14.1cfs in the 5/100-year storm events from Basins C1.15 – C1.17.

Design Point 14

Design Point 14 is located on the north of Castor Drive and is the total flow in storm sewer entering Pond C1-R from Design Point 6 & 10. The total flow in the storm sewer is 14.5cfs and 32.1cfs in the 5/100-year storm events from Basins C1.6 – C1.14.

Design Point 14a

Design Point 14a is located on the south side of Castor Drive and is the total flow from the outlet structure for Pond C1-R. The total outflow is 10.0cfs and 138.0cfs in the 5/100-year storm events from Pond C1-R per the full spectrum EDB worksheets.

Design Point 15

Design Point 15 is located at a low point in Yazoo Drive.

(5-year storm)
Tributary Basins: C5.1
Inlet/MH Number: Inlet DP-15
Upstream flowby: 0cfs
Total Street Flow: 2.2cfs

Flow Intercepted: 2.2cfs
Flow Bypassed: 0
Inlet Size: 5' type R, sump

Street Capacity: Street slope = 0.7%, capacity = 7.5cfs, capacity okay

(100-year storm)
Tributary Basins: C5.1
Inlet/MH Number: Inlet DP-15
Upstream flowby: 0cfs
Total Street Flow: 3.7cfs

Flow Intercepted: 3.7cfs
Flow Bypassed: 0
Inlet Size: 5' type R, sump

Street Capacity: Street slope = 0.7%, capacity = 31.2cfs (half street) is okay

Design Point 15a

Design Point 15a is located south side of Yazoo Drive and is the total flow from the outlet structure for Pond CR3. The total outflow is 0.07cfs and 2.5cfs in the 5/100-year storm events from Pond CR3 per the full spectrum EDB/SFB worksheets.

Design Point 16

Design Point 16 is located south of Castor Drive and west of Winnicut Drive. This design point was added to verify flow to Design Point 16 from Basin D1.1 in a swale. The total flow in the backyard swale is 2.1cfs and 4.6cfs in the 5/100-year storm events from Basins D1.1. An 18" storm sewer and end section will capture the flow at this design point and convey it via south in storm sewer to Design Point 24 .

Design Point 17

Design Point 17 is located on the north side of Castor Drive and is west of Tensas Drive. This design point was added to verify the street capacity of Castor Drive. The total street flow is 2.8cfs and 5.3cfs in the 5/100-year storm events from Basins D1.2 & D1.3. The street capacity of Castor Drive at 0.85% slope is 8cfs (5-yr) and 33.4cfs (100-yr). The street capacity is not exceeded.

Design Point 18

Design Point 18 is located on the west side of Castor Drive and is southwest of Design Point 17. This design point was added to verify the street capacity of Castor Drive. The total street flow is 4.2cfs and 8.6cfs in the 5/100-year storm events from Basins D1.2 - D1.4. The street capacity of Castor Drive at 0.8% slope is 8.2cfs (5-yr) and 34.4cfs (100-yr). The street capacity is not exceeded.

Design Point 19

Design Point 19 is located on the south end of Castor Drive in the cul-de-sac. This design point was added to verify the street capacity of Castor Drive in the cul-de-sac from the west. The total street flow is 5.2cfs and 10.9cfs in the 5/100-year storm events from Basins D1.2 - D1.5. The street capacity of Castor Drive at 0.8% slope is 8cfs (5-yr) and 33.4cfs (100-yr). The street capacity is not exceeded.

Design Point 20

Design Point 20 is located on the north side of Winnicut Drive at Castor Drive south of Design Point 16. This design point was added to verify the street capacity of Castor/Winnicut Drive. The total street flow is 4.3cfs and 9.4cfs in the 5/100-year storm events from Basins D1.6 - D1.7. The street capacity at 0.8% slope is 8cfs (5-yr) and 33.4cfs (100-yr). The street capacity is not exceeded.

Design Point 21

Design Point 21 is located on the south side of Winnicut Drive at Castor Drive south of Design Point 20. This design point was added to verify the street capacity of Castor Drive. The total street flow is 5.9cfs and 12.9cfs in the 5/100-year storm events from Basins D1.6 - D1.8. The street capacity at 0.8% slope is 8cfs (5-yr) and 33.4cfs (100-yr). The street capacity is not exceeded.

Design Point 22

Design Point 22 is located on the south end of Castor Drive in the cul-de-sac. This design point was added to verify the street capacity of Castor Drive in the cul-de-sac from the east. The total street flow is 6.0cfs and 13.3cfs in the 5/100-year storm events from Basins D1.6 - D1.9. The street capacity of Castor Drive at 0.8% slope is 8cfs (5-yr) and 33.4cfs (100-yr). The street capacity is not exceeded.

Design Point 23

Design Point 23 is located at a low point in Castor Drive in the cul-de-sac at the very south end from Design Points 19 and 22.

(5-year storm)

Tributary Basins: D1.2-D1.9 **Inlet/MH Number:** Inlet DP-23

Upstream flowby: 0cfs **Total Street Flow:** 10.8cfs

Flow Intercepted: 10.8cfs **Flow Bypassed:** 0

Inlet Size: 20' type R, sump

Street Capacity: Street slope = 0.8%, capacity = 8.0cfs, capacity okay since half is from each side

(100-year storm)

Tributary Basins: D1.2-D1.9 **Inlet/MH Number:** Inlet DP-23

Upstream flowby: 0cfs **Total Street Flow:** 23.1cfs

Flow Intercepted: 23.1cfs **Flow Bypassed:** 0

Inlet Size: 20' type R, sump

Street Capacity: Street slope = 0.8%, capacity = 33.4cfs (half street) is okay

Design Point 24

Design Point 24 is located south of Castor Drive and Design Point 23. This design point was added to calculate the total flow from the "D1" basins in the storm sewer entering Pond CR2. The total flow in the storm sewer is 12.4cfs and 26.7cfs in the 5/100-year storm events from the Basins D1 basins. A 24" storm sewer at this design point will convey flow south in this storm sewer to Pond CR2.

Design Point 24a

Design Point 24a is located south of the Castor Drive cul-de-sac and is the total flow from the outlet structure for Pond CR2. The total outflow is 0.2cfs and 10.4cfs in the 5/100-year storm events from Pond CR2 per the full spectrum EDB worksheets.

6.0 DETENTION AND WATER QUALITY PONDS

Detention and Storm Water Quality for Creekside at Lorson Ranch Filing No. 1 is required per El Paso County criteria. We have implemented the Full Spectrum approach for detention for Creekside at Lorson Ranch Filing No. 1 per the Denver Urban Drainage Districts specifications. There is one existing detention pond, one proposed detention pond, and one sand filter basin with full spectrum detention for this project site. Nearly all runoff from this site will flow to ponds and will incorporate storm water quality features prior to discharge into the East Tributary. There are some area comprising of backyard runoff that will flow directly to Jimmy Camp Creek or the Etrib which will require a deviation for Water Quality Grass Buffer submitted with this preliminary plan.

Full Spectrum Pond Construction Requirements

Design calculations for full spectrum ponds will include a 10' wide gravel access road on a 15' wide bench at a maximum 10% slope to the pond outlet structures. The final design of the full spectrum ponds consists of an outlet structure, storm sewer outfall to the East Tributary, concrete low flow channels (in new ponds), sediment forebays, and overflow weirs to the East Tributary. Soil borings, embankment, slope, and compaction requirements for detention ponds can be found in the geotechnical report for the Creekside prepared by RMG.

Detention Pond C1-R (Full Spectrum Design)

Pond C1-R formerly known as Pond C1 (Lorson Ranch MDDP1, Allegiant at Lorson Ranch), is an existing pond constructed in 2010 to serve residential subdivisions north of Lorson Boulevard. Pond C1-R included a traditional outlet structure, forebays, low flow channels, and was sized to accommodate residential areas north of Lorson Boulevard and most of the runoff from Creekside at Lorson Ranch Filing No. 1. Since full spectrum detention is now required on new developments we are proposing to remove the old outlet structure and construct a new full spectrum outlet structure to meet current detention requirements. The existing forebays, low flow channels will remain and new forebays/low flow channels will be constructed to accommodate additional storm sewer outfalls to the pond. Based on the overall tributary area to Pond C1-R and the existing as-built pond volumes it appears that the pond was built large enough in 2010 and does not need additional volume to serve the new drainage areas in Creekside. Pond C1-R is designed using the UDCF Full Spectrum spreadsheets. The outlet structure is a standard 17' long x 7' wide full spectrum sloped outlet structure to match pre-developed rates. The full spectrum print outs are in the appendix of this report. See map in appendix for watershed areas.

- Watershed Ares: 119.5acres
- Watershed Imperviousness: 55%
- Hydrologic Soils Group C (80%) and B (20%)
- Zone 1 WQCV: 2.025ac-ft, WSEL: 5686.89, 1.0cfs
- Zone 2 EURV: 5.775ac-ft, WSEL: 5688.71, Top EURV wall set at 5689.23, 17'x7' outlet with 7:1 slope, 5.0cfs
- (5-yr): 7.468ac-ft, WSEL: 5689.46, 9.6 cfs
- Zone 3 (100-yr): 11.939ac-ft, WSEL: 5691.24, 140.50cfs
- Pipe Outlet: 54" RCP at 0.3% with restrictor plate 44" up.
- Overflow Spillway: overtops roadway, elevation=5693.60
- Pre-development release rate into creek compliance from full spectrum pond spreadsheets
- Pond Bottom Elevation: 5683.80

The emergency overflow for Pond C1-R flows across Castor Drive. Per DCM Volume 1, Chapter 13, Figure 13-12a, the overflow depth across the road must be less than 1' deep under undetained fully developed flow conditions. The downstream embankment must be protected with rip rap designed in accordance with Equation 13-9. The minimum rip rap size is 6" but we are proposing to use rip rap salvaged from the old spillway which has a size of 12" D50 rip rap. The flow depth across Castor Drive is located in a vertical curve and was approximated using circular weir calculations and a full developed flow rate of 294cfs resulting in a 0.88' flow depth.

Detention Pond CR2 (Full Spectrum Design)

This is an on-site permanent full spectrum extended detention pond that includes water quality and discharges directly into the East Tributary. Pond CR2 is designed using the UDCF Full Spectrum spreadsheets. The outlet structure is a standard 3'x68" full spectrum sloped outlet structure and the overflow spillway is a weir set above the outlet structure designed by the full spectrum spreadsheets to match pre-developed rates. The full spectrum print outs are in the appendix of this report. See map in appendix for watershed areas.

- Watershed Ares: 10.0 acres
- Watershed Imperviousness: 52%
- Hydrologic Soils Group B
- Forebay: 0.004ac-ft, 18" depth
- Zone 1 WQCV: 0.162ac-ft, WSEL: 5683.29, 0.1cfs
- Zone 2 EURV: 0.525ac-ft, WSEL: 5684.75, Top EURV wall set at 5685.00, 3'x68" outlet with 3:1 slope, 0.2cfs
- (5-yr): 0.582ac-ft, WSEL: 5684.93, 0.2cfs
- Zone 3 (100-yr): 0.957ac-ft, WSEL: 5686.0416-17
- , 10.4cfs
- Pipe Outlet: 18" RCP at 1.0% with restrictor plate up 10"
- Overflow Spillway: 10' wide bottom, elevation=5687.00, 4:1 side slopes, flow depth=0.71'
- Pre-development release rate into creek compliance from full spectrum pond spreadsheets
- Pond Bottom Elevation: 5681.00

Detention Pond CR3 (Full Spectrum Design, Sand Filter Basin)

This is an on-site permanent full spectrum sand filter basin pond that includes water quality, full spectrum detention, and discharges directly into the East Tributary. Pond CR3 is designed using the UDCF Full Spectrum spreadsheets. Water quality is provided by a Sand Filter Basin and full spectrum detention is provided by a CDOT Type C drainage structure modified to meet full spectrum requirements. The primary overflow structure is a CDOT Type D drainage structure connected to the full spectrum structure. The primary overflow structure will collect the incoming undetained developed flows of 7.7cfs at a depth of 0.45' deep and a top elevation of 5688.00 and convey it to the East Tributary via an 18" storm sewer pipe. The secondary overflow structure is a trapezoidal swale set at elevation 5688.50 and a top elevation of 5689.00. The full spectrum outlet structure and spreadsheets are designed to match pre-developed rates. The full spectrum print outs are in the appendix of this report. See map in appendix for watershed areas.

- Watershed Ares: 2.66 acres
- Watershed Imperviousness: 40%
- Hydrologic Soils Group B
- Forebay: 0.00165ac-ft
- Sand Filter Area: 756sf, 11/16" orifice for underdrain restrictor plate
- Zone 1 WQCV: 0.028ac-ft, WSEL: 5685.13, 0.02cfs

- Zone 2 EURV: 0.07ac-ft, WSEL: 5686.45, Top EURV wall set at 5687.00, 3'x3' CDOT Type C outlet, flat top, 0.07cfs
- EURV Orifice = 6.2" orifice, 2.3' below sand filter (5684.00)
- (5-yr): 0.113ac-ft, WSEL: 5686.60, 0.07cfs
- Zone 3 (100-yr): 0.239ac-ft, WSEL: 5687.95, 2.5cfs
- Pipe Outlet: 18" RCP at 1.56%
- Overflow Spillway: 6' wide bottom, elevation=5688.50, 4:1 side slopes, flow depth=0.38'
- Pre-development release rate into creek compliance from full spectrum pond spreadsheets
- Pond Bottom Elevation: 5684.00

Water Quality Design

Water quality will be provided by two permanent extended detention basins (Pond C1-R, CR2) and one Sand Filter Basin (Pond CR3) for 98.9% of the 83.085acre site. Approximately 0.91 acres (1.1% of the total 83.085-acre preliminary plan area) consists of backyards that drain directly to the East Tributary or Jimmy Camp Creek over grass buffers. A deviation from county criteria to use a grass buffer bmp to treat runoff from these backyard drainage areas is submitted. The backyards draining to the grass buffer is broken into three separate areas and the largest of the three areas is 0.4 acres which generates a 2yr runoff of 0.43cfs. Using the grass buffer worksheets the resultant grass buffer width is 9' wide at maximum of 10% slope. All three grass buffers will be a minimum of 9' wide.

7.0 FOUR STEP PROCESS

The site has been developed to minimize wherever possible the rate of developed runoff that will leave the site and to provide water quality management for the runoff produced by the site as proposed on the development plan. The following four step process should be considered and incorporated into the storm water collection system and storage facilities where applicable.

Step 1: Employ Runoff Reduction Practices

Creekside at Lorson Ranch Filing No. 1 has employed several methods of reducing runoff.

- The street configuration was laid out to minimize the length of streets. Many streets are straight and perpendicular resulting in lots with less wasted space.
- Large open space tracts of land act as a buffer between lots and the East Tributary of Jimmy Camp Creek
- East Tributary of Jimmy Camp Creek with a natural sand bottom and vegetated slopes has been preserved through this site
- Only a small portion of lots on the south side of the site discharge runoff south over an open space buffer prior to discharge into the creek. The remainder of lots drain to WQ ponds.
- Lorson Ranch Metro District requires homeowners to maintain landscaping on lots
- Full Spectrum Detention Pond C1-R, CR2, and CR3 (sand filter basin) will be constructed. The full spectrum detention ponds mimics existing storm discharges

Step 2: Implement BMP's that Slowly Release the Water Quality Capture Volume

Treatment and slow release of the water quality capture volume (WQCV) is required. Creekside at Lorson Ranch Filing No. 1 will utilize Pond C1-R, CR2, and CR3 which are full spectrum stormwater detention ponds which includes Water Quality Volumes and WQ outlet structures. Pond CR3 has a sand filter basin for WQ treatment.

Step 3: Stabilize Drainageways

East Tributary of Jimmy Camp Creek is a major drainageway located within this site. The East Tributary of JCC will be stabilized per county criteria for this subdivision. The design includes a low flow channel bottom and selectively armored sides. Kiowa Engineering is providing the East Tributary design.

Step 4: Implement Site Specific & Source Control BMP's

There are no potential sources of contaminants that could be introduced to the County's MS4. During construction the source control will be provided with the proper installation of erosion control BMPs to limit erosion and transport of sediment. Area disturbed by construction will be seeded and mulched. Cut and fill slopes will be reseeded, and the slopes equal to or greater than three-to-one will be protected with erosion control fabric. Silt fences will be placed at the bottom of re-vegetated and rough graded slopes. Inlet protection will be used around proposed inlets. In addition, temporary sediment basins will be constructed so runoff will be treated prior to discharge. Construction BMPs in the form of vehicle tracking control, sediment basins, concrete washout area, rock socks, buffers, and silt fences will be utilized to protect receiving waters.

8.0 DRAINAGE AND BRIDGE FEES

Creekside at Lorson Ranch Filing No. 1 is located within the Jimmy Camp Creek drainage basin which is currently a fee basin in El Paso County. Current El Paso County regulations require drainage and bridge fees to be paid for platting of land as part of the plat recordation process. Lorson Ranch Metro District will be constructing the major drainage infrastructure as part of the district improvements.

Creekside at Lorson Ranch Filing No. 1 contains 83.088 acres. This project consists of 43.514 acres of open space (2% impervious), and the remaining 39.574 acres is residential (57% impervious based on 4600sf lots). The 2019 drainage fees are $18,350, bridge fees are $858 and Drainage Surety fees are $7,285 per impervious acre per Resolution 18-470. The drainage and bridge fees are calculated when the final plat is submitted. The fees are due at plat recordation. The following table details the drainage fees for the platted area.

Table 1: Drainage/Bridge Fees

Type of Land Use	Total Area (ac)	Imperviousness	Drainage Fee	Bridge Fee	Surety Fee
Residential Area	39.574	57%	$413,924	$19,354	$164,329
Open Space, Landscape Tracts,	43.514	2%	$15,970	$746	$6,340
		Total	$429,894	$20,100	$170,669

Table 7.1: Public Drainage Facility Costs (non-reimbursable)

Item	Quantity	Unit	Unit Cost	Item Total
Rip Rap	200	CY	$50/CY	$10,000
Manholes	1	EA	$3000/EA	$3,000
18" Storm	1226	LF	$35	$42,910
24" Storm	286	LF	$40	$11,440

18” FES	1	EA	$200	$200
Inlets	8	EA	$3,000	$24,000
			Subtotal	$91,550
			Eng/Cont 15%)	$13,750
			Total Est. Cost	$105,300

Table 7.2: Lorson Ranch Metro District Drainage Facility Costs (non-reimbursable)

Item	Quantity	Unit	Unit Cost	Item Total
Full Spectrum Ponds and Outlet	2.5	EA	$70,000	$175,000
			Subtotal	$175,000
			Eng/Cont (15%)	$26,250
			Total Est. Cost	$201,250

Table 7.3: Lorson Ranch Metro District Drainage Facility Costs (Potential Reimbursable)

Item	Quantity	Unit	Unit Cost	Item Total
E. Tributary Channel Improvements-Kiowa	1	LS	$1,900,000	$1,900,000
			Subtotal	$1,900,000
			Total Est. Cost	$1,900,000

9.0 CONCLUSIONS

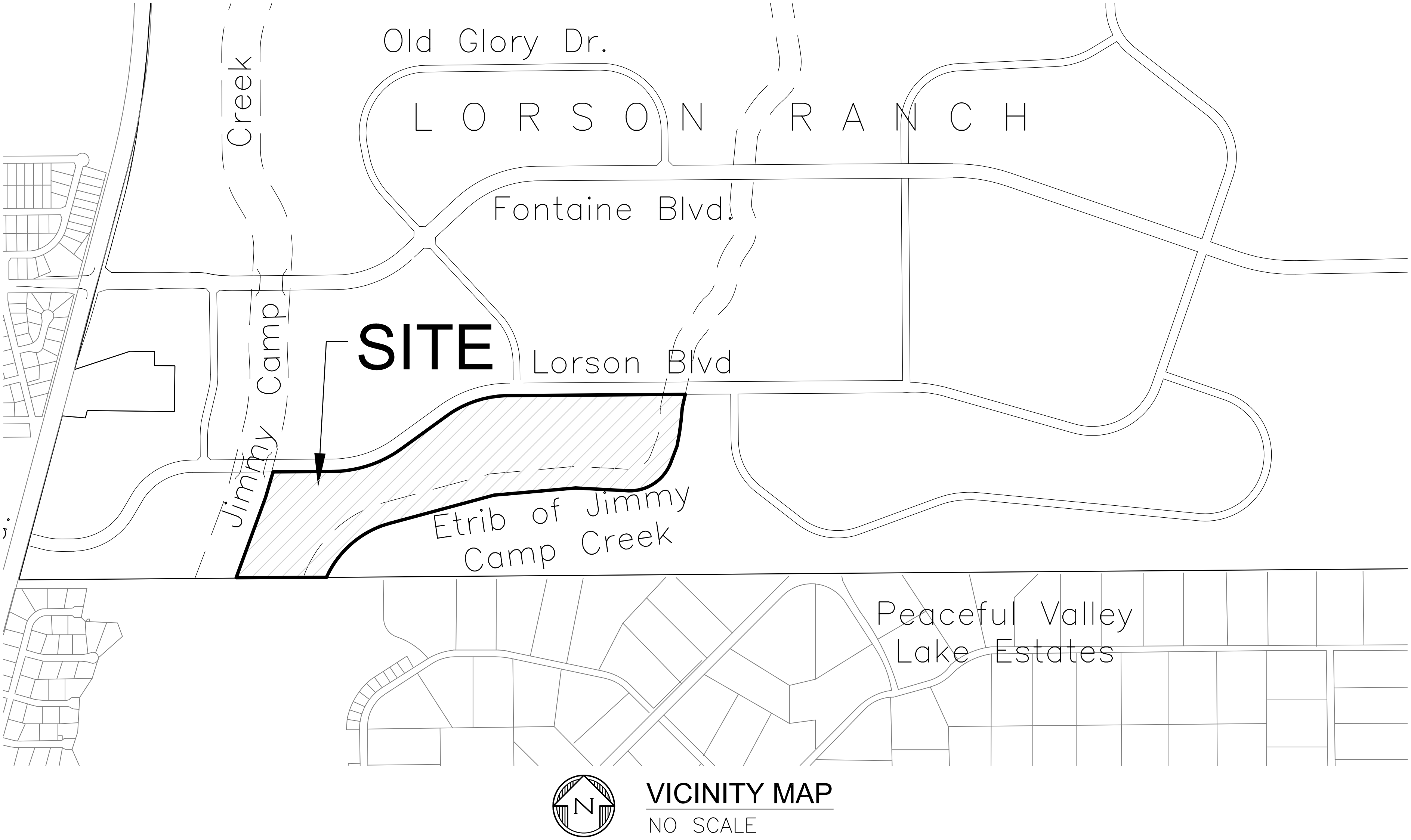
This drainage report has been prepared in accordance with the City of Colorado Springs/El Paso County Drainage Criteria Manual. The proposed development and drainage infrastructure will not cause adverse impacts to adjacent properties or properties located downstream. Several key aspects of the development discussed above are summarized as follows:

- Developed runoff will be conveyed via curb/gutter and storm sewer facilities
- The East Tributary of Jimmy Camp Creek will be reconstructed within Creekside at Lorson Ranch per the construction plans prepared by Kiowa Engineering (see Appendix F for design report). Construction includes a low flow channel and selective bank armoring on the outside bends.
- Detention and water quality for this preliminary plan area will be provided in two permanent ponds and one sand filter basin.

10.0 REFERENCES

1. City of Colorado Springs/El Paso County Drainage Criteria Manual DCM, dated November, 1991
2. Soil Survey of El Paso County Area, Colorado by USDA, SCS
3. Jimmy Camp Creek Drainage Basin Planning Study, Dated March 9, 2015, by Kiowa Engineering Corporation
4. City of Colorado Springs “Drainage Criteria Manual, Volume 2
5. El Paso County “Engineering Criteria Manual”
6. Lorson Ranch MDDP1, October 26, 2006 by Pentacor Engineering.
7. El Paso County Resolution #15-042, El Paso County adoption of Chapter 6 and Section 3.2.1 of the City of Colorado Springs Drainage Criteria Manual dated May, 2014.

APPENDIX A – VICINTIY MAP, SOILS MAP, FEMA MAP



VICINITY MAP
NO SCALE

CORE
ENGINEERING GROUP

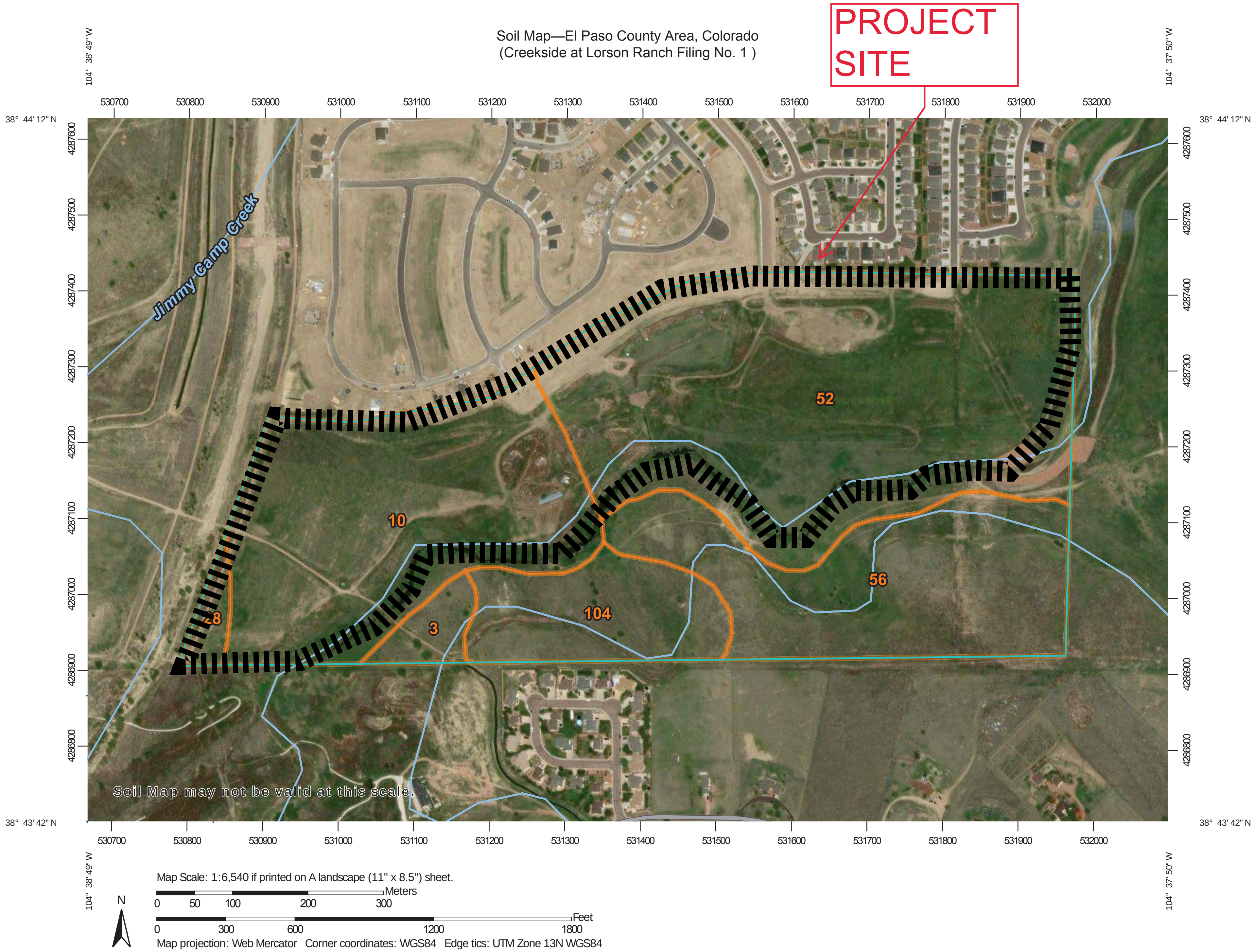
15004 1ST AVE. S.
BURNSVILLE, MN 55306
PH: 719.570.1100

CONTACT: RICHARD L. SCHINDLER, P.E.
EMAIL: Rich@ceg1.com

CREEKSIDE AT LORSON RANCH FILING NO. 1
VICINITY MAP

SCALE:	DATE:	FIGURE NO.
NTS	AUGUST, 2018	--

Soil Map—El Paso County Area, Colorado
(Creekside at Lorson Ranch Filing No. 1)



Map Scale: 1:6,540 if printed on A landscape (11" x 8.5") sheet.

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

- ☐ Area of Interest (AOI)

Soils

- ☐ Soil Map Unit Polygons
- 〰 Soil Map Unit Lines
- ■ Soil Map Unit Points

Special Point Features

- Blowout
- ⊠ Borrow Pit
- Clay Spot
- ◇ Closed Depression
- ✕ Gravel Pit
- ∴ Gravelly Spot
- Landfill
- Λ Lava Flow
- Marsh or swamp
- Mine or Quarry
- ◎ Miscellaneous Water
- ○ Perennial Water
- ∨ Rock Outcrop
- + Saline Spot
- ∷ Sandy Spot
- ⊖ Severely Eroded Spot
- ◈ Sinkhole
- Slide or Slip
- ∅ Sodic Spot
- ≡ Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- △ Other
- Special Line Features

Water Features

- 〰 Streams and Canals

Transportation

- ┿┿┿ Rails
- 〰 Interstate Highways
- 〰 US Routes
- 〰 Major Roads
- 〰 Local Roads

Background

- ▨ Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 15, Oct 10, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Nov 7, 2015—Mar 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
3	Ascalon sandy loam, 3 to 9 percent slopes	2.4	2.0%
10	Blendon sandy loam, 0 to 3 percent slopes	31.3	26.0%
28	Ellicott loamy coarse sand, 0 to 5 percent slopes	1.5	1.2%
52	Manzanst clay loam, 0 to 3 percent slopes	51.4	42.7%
56	Nelson-Tassel fine sandy loams, 3 to 18 percent slopes	23.4	19.4%
104	Vona sandy loam, warm, 0 to 3 percent slopes	10.4	8.7%
Totals for Area of Interest		**120.5**	**100.0%**

Native vegetation is dominantly western wheatgrass, side-oats grama, and needleandthread. This soil is best suited to deep-rooted grasses.

Proper range management is necessary to prevent excessive removal of plant cover from the soil. Interseeding improves the existing vegetation. Deferment of grazing in spring increases plant vigor and soil stability. Proper location of livestock watering facilities helps to control grazing.

Windbreaks and environmental plantings are fairly well suited to this soil. Blowing sand and low available water capacity are the main limitations for the establishment of trees and shrubs. The soil is so loose that trees need to be planted in shallow furrows and plant cover needs to be maintained between the rows. Supplemental irrigation may be needed to insure survival. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, and Siberian elm. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

This soil is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil has good potential for urban development. Soil blowing is a hazard if protective vegetation is removed. Special erosion control practices must be provided to minimize soil losses. Capability subclass VIe.

9—Blakeland complex, 1 to 9 percent slopes. This complex is on uplands, mostly in the Falcon area. The average annual precipitation is about 15 inches, the average annual air temperature is about 47 degrees F, and the frost-free period is about 135 days.

This complex is about 60 percent Blakeland loamy sand, about 30 percent Fluvaquentic Haplaquolls, and 10 percent other soils.

Included with these soils in mapping are areas of Columbine gravelly sandy loam, 0 to 3 percent slopes, Ellicott loamy coarse sand, 0 to 5 percent slopes, and Ustic Torrifluvents, loamy.

The Blakeland soil is in the more sloping areas. It is deep and somewhat excessively drained. It formed in sandy alluvium and eolian material derived from arkosic sedimentary rock. Typically, the surface layer is dark grayish brown loamy sand about 11 inches thick. The substratum, to a depth of 27 inches, is brown loamy sand; it grades to pale brown sand that extends to a depth of 60 inches or more.

Permeability of the Blakeland soil is rapid. The effective rooting depth is more than 60 inches. The available water capacity is moderate to low. Surface runoff is slow, and the hazard of erosion is moderate.

The Fluvaquentic Haplaquolls are in swale areas. They are deep, poorly drained soils. They formed in alluvium derived from arkosic sedimentary rock. Typically, the surface layer is brown. The texture is variable throughout. The water table is at a depth of 0 to 3 feet.

The Blakeland soil is well suited to deep-rooted grasses. Native vegetation is dominantly western wheatgrass, side-oats grama, and needleandthread. Rangeland vegetation on the Fluvaquentic Haplaquolls is dominantly tall grasses, including sand bluestem, switchgrass, prairie cordgrass, little bluestem, and sand reedgrass. Cattails and bulrushes are common in the swampy areas.

Proper range management is needed to prevent excess removal of plant cover from these soils. It is also needed to maintain the productive grasses. Interseeding improves the existing vegetation. Deferment of grazing during the growing season increases plant vigor and soil stability, and it helps to maintain and improve range condition. Proper location of livestock watering facilities helps to control grazing of animals.

Windbreaks and environmental plantings are fairly well suited to these soils. Blowing sand and low available water capacity are the main limitations to the establishment of trees and shrubs. The soils are so loose that trees need to be planted in shallow furrows and plant cover needs to be maintained between the rows. Supplemental irrigation may be needed to insure survival. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, and Siberian elm. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

The Blakeland soil is well suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed. Wetland wildlife can be attracted to the Fluvaquentic Haplaquolls and the wetland habitat can be enhanced by several means. Shallow water developments can be created by digging or by blasting potholes to create open-water areas. Fencing to control livestock grazing is beneficial, and it allows wetland plants such as cattails, reed canarygrass, and rushes to grow. Control of unplanned burning and prevention of drainage that would remove water from the wetlands are good practices. Openland wildlife use the vegetation on these soils for nesting and escape cover. These shallow marsh areas are especially important for winter cover if natural vegetation is allowed to grow.

The Blakeland soil has good potential for homesites, roads, and streets. It needs to be protected from erosion when vegetation has been removed from building sites. The Fluvaquentic Haplaquolls have poor potential for homesites. Their main limitations for this use are the high water table and the hazard of flooding. Capability subclass VIe.

10—Blendon sandy loam, 0 to 3 percent slopes. This deep, well drained soil formed in sandy arkosic alluvium on alluvial fans and terraces. The average annual precipitation is about 15 inches, the mean annual air temperature is about 47 degrees F, and the average frost-free period is about 135 days.

Typically, the surface layer is dark grayish brown sandy loam about 10 inches thick. The subsoil is dark grayish brown and brown sandy loam about 26 inches thick. The substratum is light brownish gray gravelly sandy loam.

Included with this soil in mapping are small areas of Blakeland loamy sand, 1 to 9 percent slopes; Bresser sandy loam, 0 to 3 percent slopes; Truckton sandy loam, 0 to 3 percent slopes; Ellicott loamy coarse sand, 0 to 5 percent slopes; and Ustic Torrifluvents, loamy.

Permeability of this Blendon soil is moderately rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is slow, and the hazards of erosion and soil blowing are moderate.

Most areas of this soil are used as rangeland, but some small areas are cultivated. Some homesite development has taken place on this soil.

Native vegetation is mainly cool- and warm-season grasses such as western wheatgrass, side-oats grama, and needleandthread.

Proper range management is needed to prevent excessive removal of plant cover from the soil. Interseeding improves the existing vegetation. Deferment of grazing in spring increases plant vigor and soil stability. Proper location of livestock watering facilities helps to control grazing.

Windbreaks and environmental plantings are generally suited to this soil. Soil blowing is the principal limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be needed when planting and during dry periods. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

This soil is well suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. In cropland areas, habitat favorable for ring-necked pheasant, mourning dove, and many nongame species can be developed by establishing areas for nesting and escape cover. For pheasant, the provision of undisturbed nesting cover is vital and should be included in plans for habitat development. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil has good potential for homesites. The main limitation for the construction of local roads and streets is a moderate frost action potential. Roads can be designed to overcome this limitation. Capability subclass IIIe.

11—Bresser sandy loam, 0 to 3 percent slopes. This deep, well drained soil formed in arkosic alluvium and residuum on terraces and uplands. Elevation ranges from 6,000 to 6,800 feet. The average annual precipitation is about 15 inches, the average annual air temperature is about 47 degrees F, and the average frost-free period is about 135 days.

Typically, the surface layer is grayish brown sandy loam about 5 inches thick. The subsoil is brown sandy clay loam about 31 inches thick. The substratum is light yellowish brown loamy coarse sand to a depth of 60 inches.

Included with this soil in mapping are small areas of Truckton sandy loam, 0 to 3 percent slopes; Ascalon sandy loam, 1 to 3 percent slopes; Fort Collins loam, 0 to 3 percent slopes; and Yoder gravelly sandy loam, 1 to 8 percent slopes. Some areas of Ustic Torrifluvents, loamy, occur along narrow drainageways.

Permeability of this Bresser soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is slow, the hazard of erosion is slight to moderate, and the hazard of soil blowing is moderate.

Most areas of this soil are cultivated. The remaining acreage is used as rangeland.

A rotation of winter wheat and fallow is used because precipitation is insufficient for annual cropping. A feedgrain crop such as millet or sorghum can be substituted for wheat in some years. Crop residue management and minimum tillage are needed to control erosion.

Native vegetation is mainly cool- and warm-season grasses such as western wheatgrass, side-oats grama, and needleandthread.

Proper range management is needed to prevent excessive removal of plant cover from the soil. Interseeding improves the existing vegetation. Deferment of grazing in spring increases plant vigor and soil stability. Proper location of livestock watering facilities helps to control grazing.

Windbreaks and environmental plantings are generally suited to this soil. Soil blowing is the principal limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be needed when planting and during dry periods. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

This soil is well suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. In cropland areas, habitat favorable for ring-necked pheasant, mourning dove, and many nongame species can be developed by establishing areas for nesting and escape cover. For pheasant, the provision of undisturbed nesting cover is vital and should be included in plans for habitat development. This is especially true in areas of intensive farming. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil has good potential for homesites. Limiting the disturbance of the soil and the removal of existing plant cover during construction helps to control erosion. Capability subclass IIIc.

B2t—8 to 21 inches; grayish brown (10YR 5/2) sandy loam, dark grayish brown (10YR 4/2) moist; moderate coarse prismatic structure parting to moderate medium subangular blocky; very hard, firm, slightly sticky; thin patchy clay films on faces of peds; neutral; clear smooth boundary.

B3—21 to 28 inches; brown (10YR 5/3) sandy loam, dark brown (10YR 4/3) moist; weak coarse prismatic structure parting to weak fine and medium subangular blocky; slightly hard, very friable; neutral; clear smooth boundary.

C1—28 to 60 inches; pale brown (10YR 6/3) loamy coarse sand, dark brown (10YR 4/3) moist; massive; hard, very friable; neutral.

The solum ranges from 21 to 40 inches in thickness. It is 0 to 15 percent coarse fragments. It ranges from slightly acid to mildly alkaline. The A1 horizon is brown or grayish brown sandy loam or loamy sand. The B2t horizon is brown or grayish brown sandy loam to coarse sandy loam. The C horizon is pale brown or brown.

Blakeland series

The Blakeland series consists of deep, somewhat excessively drained soils. These soils formed in arkosic sandy alluvium and eolian sediment on uplands. They have slopes of 1 to 20 percent. Average annual precipitation is about 15 inches, and average annual air temperature is about 47 degrees F.

Blakeland soils are similar to Chaseville, Columbine, and Connerton soils. They are near Bresser and Truckton soils. Chaseville soils have hue of 7.5YR to 10R. Columbine soils have hue of 5Y to 7.5YR and have a control section that is 18 to 35 percent clay. Bresser soils have a B2t horizon that is 18 to 35 percent clay. Truckton soils have a B2t horizon that is 5 to 18 percent clay.

Typical pedon of Blakeland loamy sand, 1 to 9 percent slopes, 1,990 feet north and 1,730 feet west of the southeast corner of sec. 4, T. 14 S., R. 65 W.:

A1—0 to 11 inches; dark grayish brown (10YR 4/2) loamy sand, very dark grayish brown (10YR 3/2) moist; moderate fine granular structure; slightly hard, very friable; slightly acid; clear smooth boundary.

AC—11 to 27 inches; brown (10YR 5/3) loamy sand, dark brown (10YR 4/3) moist; weak coarse prismatic structure parting to weak fine granular; very hard, very friable; neutral; gradual smooth boundary.

C—27 to 60 inches; pale brown (10YR 6/3) sand, brown (10YR 5/3) moist; massive; very hard, very friable; neutral.

The solum ranges from 8 to 20 inches in thickness. It is 0 to 15 percent coarse fragments. It ranges from slightly acid to mildly alkaline. The A1 horizon is dark grayish brown or brown. The AC horizon is brown loamy sand or loamy coarse sand. The C horizon is pale brown to light yellowish brown.

Blendon series

The Blendon series consists of deep, well drained soils that formed in sandy arkosic alluvium. These soils are on terraces, on flood plains, and in drainageways. They have slopes of 0 to 3 percent. Average annual precipitation is about 15 inches, and average annual air temperature is about 47 degrees F.

Blendon soils are similar to Bresser and Truckton soils. They are near Bijou and Blakeland soils. Bresser, Truckton, and Blakeland soils have a mollic epipedon less than 20 inches thick. Bresser soils have a B2t horizon that is 18 to 35 percent clay. Blakeland soils have an AC horizon. Bijou soils lack a mollic epipedon.

Typical pedon of Blendon sandy loam, 0 to 3 percent slopes, about 780 feet east and 30 feet south of fence and east of road that intersects the section line near the northwest quarter of sec. 21, T. 13 S., R. 65 W.:

A11—0 to 6 inches; dark grayish brown (10YR 4/2) sandy loam, very dark brown (10YR 2/2) moist; weak fine granular structure; soft, very friable; 5 percent fine gravel; slightly acid; clear smooth boundary.

A12—6 to 10 inches; dark grayish brown (10YR 3/2) sandy loam, very dark brown (10YR 2/2) moist; weak medium and fine subangular blocky structure parting to moderate medium and fine granular; hard, very friable; 5 percent gravel; neutral; gradual smooth boundary.

B2—10 to 23 inches; dark grayish brown (10YR 4/2) sandy loam, very dark brown (10YR 2/2) moist; weak coarse prismatic structure parting to weak medium subangular blocky; extremely hard, friable; 10 percent gravel; neutral; gradual smooth boundary.

B3—23 to 36 inches; brown (10YR 4/3) sandy loam, dark brown (10YR 3/3) moist; weak coarse subangular blocky structure; very hard, very friable; 10 percent gravel; neutral; clear wavy boundary.

C—36 to 60 inches; light brownish gray (10YR 6/2) gravelly sandy loam, grayish brown (10YR 5/2) moist; massive; hard, friable; 30 percent gravel; neutral.

The solum ranges from 26 to 40 inches in thickness. It is 0 to 20 percent coarse fragments. It is slightly acid or neutral. The A1 horizon is dark grayish brown or brown sandy loam or fine sandy loam. The B2 horizon is dark grayish brown or brown sandy loam to fine sandy loam. The C horizon is light brownish gray or pale brown.

Bresser series

The Bresser series consists of deep, well drained soils that formed in alluvium and residuum derived from arkosic sedimentary rock. These soils are on uplands. They have slopes of 0 to 20 percent. Average annual precipitation is about 15 inches, and average annual air temperature is about 47 degrees F.

Bresser soils are similar to Ascalon and Satanta soils and are near Blakeland and Truckton soils. Ascalon and Satanta soils are calcareous in part of the solum and in the C horizon. Blakeland soils do not have a B2t horizon and are coarse textured throughout. Truckton soils have a B2t horizon that is less than 18 percent clay.

Typical pedon of Bresser sandy loam, 3 to 5 percent slopes, about 0.1 mile south and 200 feet east of the northwest corner of sec. 9, T. 11 S., R. 62 W.:

A1—0 to 5 inches; grayish brown (10YR 5/2) sandy loam, very dark grayish brown (10YR 3/2) moist; moderate fine granular structure; soft, very friable; neutral; clear smooth boundary.

B1—5 to 8 inches thick; grayish brown (10YR 5/2) sandy loam, very dark grayish brown (10YR 3/2) moist; weak coarse prismatic structure parting to moderate medium subangular blocky; hard, very friable; few thin patchy clay films on faces of peds; neutral; clear smooth boundary.

B21t—8 to 12 inches; brown (10YR 4/3) sandy clay loam, dark brown (10YR 3/3) moist; moderate medium prismatic structure parting to moderate medium subangular blocky; thin continuous clay films on faces of peds; neutral; gradual smooth boundary.

B22t—12 to 27 inches; brown (10YR 5/3) sandy clay loam, dark brown (10YR 4/3) moist; moderate medium prismatic structure parting to moderate to strong subangular blocky; very hard, friable, slightly

Woodland wildlife, such as mule deer and wild turkey, is attracted to this soil because of its potential to produce ponderosa pine, Gambel oak, and various grasses and shrubs. Water developments, such as guzzlers, would enhance populations of wild turkey as well as other kinds of wildlife. Where wildlife and livestock share the same range, proper grazing management is needed to prevent overuse and to reduce competition. Livestock watering facilities would also benefit wildlife on this soil.

This soil has good potential for use as homesites. The main limitation is the moderate shrink-swell potential in the subsoil and frost action potential. Special road design is necessary on this soil to overcome these limitations. Slope is also a limitation. Special planning is needed on this soil to minimize site disturbance and tree and seedling damage. During seasons of low precipitation, fire may become a hazard to homesites on this soil. The hazard can be minimized by installing firebreaks and reducing the amount of potential fuel on the forest floor. Capability subclass VIe.

27—**Elbeth-Pring complex, 5 to 30 percent slopes.** These moderately sloping to steep soils are on upland side slopes and ridges. Elevation ranges from 7,200 to 7,400 feet. The average annual precipitation is about 18 inches, the average annual air temperature is about 43 degrees F, and the average frost-free period is about 120 days.

The Elbeth soil makes up about 60 percent of the complex, the Pring about 20 percent, and other soils about 20 percent. The Elbeth soil has slopes of 5 to 15 percent, and the Pring soil has slopes of 5 to 30 percent.

Included with these soils in mapping are areas of Peyton-Pring complex, 8 to 15 percent slopes, Kettle-Rock outcrop complex, and ridges that are covered with gravel and cobbles.

The Elbeth soil is deep and well drained. It formed in material transported from arkose deposits. Typically, the surface layer is very dark grayish brown sandy loam about 3 inches thick. The subsurface layer is light gray loamy sand about 20 inches thick. The subsoil is brown sandy clay loam about 45 inches thick. The substratum is light brown sandy clay loam.

Permeability of the Elbeth soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is medium to rapid, and the hazard of erosion is moderate to high. Deep gullies occur throughout areas of this soil. Some soil slippage occurs on some of the steeper slopes.

The Pring soil is deep and well drained. It formed in arkosic sediment. Typically, the surface layer is dark grayish brown coarse sandy loam about 4 inches thick. The next layer is dark grayish brown coarse sandy loam about 10 inches thick. The underlying material is pale brown gravelly sandy loam to a depth of 60 inches.

Permeability of the Pring soil is rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is medium, and the hazard of erosion is moderate.

The soils in this complex are used for woodland, recreation, livestock grazing, and homesites.

The Elbeth soil is suited to the production of ponderosa pine. It is capable of producing about 2,240 cubic feet, or 4,900 board feet (International rule), of merchantable timber per acre from a fully stocked, even-aged stand of 80-year-old trees. Conventional methods can be used for harvesting, but operations may be restricted during wet periods. Reforestation, after harvesting, must be carefully managed to reduce competition of undesirable understory plants.

The Pring soil is suited to the production of native vegetation suitable for grazing by cattle and sheep. Rangeland vegetation is mainly mountain muhly, little bluestem, needleandthread, Parry oatgrass, and junegrass.

Deferment of grazing in spring promotes plant vigor and reproduction of the cool-season bunchgrasses. Fencing and proper location of livestock watering facilities may be needed to obtain proper distribution of grazing. Locating salt blocks in areas not generally grazed increases the use of the available forage.

Woodland wildlife such as mule deer and wild turkey is attracted to the Elbeth soil because of its potential to produce ponderosa pine, Gambel oak, and various grasses and shrubs. Water developments, such as guzzlers, would enhance populations of wild turkey as well as other kinds of wildlife. Where wildlife and livestock share the same range, proper grazing management is needed to prevent overuse and to reduce competition. Livestock watering facilities would also benefit wildlife on this soil.

The Pring soil is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

The main limitations of this complex for construction are the moderate shrink-swell potential in the subsoil of the Elbeth soil and the steep slopes of both soils. Special site or building designs for dwellings and roads are required to offset these limitations. Special practices must be used to minimize surface runoff and keep soil erosion to a minimum. Capability subclass VIe.

28—**Ellicott loamy coarse sand, 0 to 5 percent slopes.** This deep, somewhat excessively drained soil is on terraces and flood plains (fig. 1). The average annual precipitation is about 14 inches, the average annual air temperature is about 48 degrees F, and the average frost-free period is about 135 days.

Typically, the surface layer is grayish brown loamy coarse sand about 4 inches thick. The underlying material to a depth of 60 inches is light brownish gray coarse sand stratified with layers of loamy sand, loamy coarse sand, and coarse sandy loam.

Included with this soil in mapping are small areas of Ustic Torrifluvents, loamy; Fluvaquentic Haploquolls, nearly level; Blakeland loamy sand, 1 to 9 percent slopes; Blendon sandy loam; and Truckton sandy loam, 0 to 3 percent slopes.

Permeability of this Ellicott soil is rapid. Effective rooting depth is 60 inches or more. Available water capacity is low. Surface runoff is slow, the hazard of erosion is high, and the hazard of soil blowing is moderate.

Almost all areas of this soil are used as rangeland.

The rangeland vegetation on this soil is mainly switchgrass, needleandthread, sand bluestem, and prairie sand reedgrass.

Seeding is a good practice if the range is in poor condition. Seeding of the native grasses is desirable. Yellow or white sweetclover may be added to the seeding mixture to provide a source of nitrogen for the grasses. Too much clover can create a danger of bloat by grazing animals. This soil is subject to flooding and should be managed to keep a heavy cover of grass to protect the soil. Fencing is a necessary practice in range management. Brush control and grazing management may help to improve deteriorated range.

Windbreaks and environmental plantings are fairly well suited to this soil. Blowing sand and low available water capacity are the principal limitations for the establishment of trees and shrubs. The soil is so loose that trees need to be planted in shallow furrows and plant cover needs to be maintained between the rows. Supplemental irrigation may be needed to insure survival of trees. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, and Siberian elm. Shrubs that are best suited to skunkbush sumac, lilac, and Siberian peashrub.

Rangeland wildlife, such as antelope, cottontail, coyote, and scaled quail, is best adapted to life on this droughty soil. Forage production is typically low, and proper livestock grazing management is needed if wildlife and livestock share the range. Livestock watering developments are also important and are used by various wildlife species.

The main limitation of this soil for construction is the hazard of flooding. All construction on this soil should be kept off the flood plain as much as possible. Capability subclass VIw.

29—Fluvaquentic Haplaquolls, nearly level. These deep, poorly drained soils are in marshes, in swales, and on creek bottoms. The average annual precipitation is about 14 inches, and the average annual air temperature is about 47 degrees F.

Included with these soils in mapping are small areas of Ustic Torrifluvents, loamy; Blakeland loamy sand, 1 to 9 percent slopes; Columbine gravelly sandy loam, 0 to 3 percent slopes; and Ellicott loamy coarse sand, 0 to 5 percent slopes.

These soils are stratified. Typically, the surface layer is light gray to very dark gray loamy fine sand to gravelly loam 2 to 6 inches thick. The underlying material, 48 to 58 inches thick, is very pale brown to gray, stratified heavy sandy clay loam to sand and gravel. The lower part of some of the soils, at depths ranging from 18 to 48 inches, ranges from light blueish gray to greenish gray. The water table is usually at a depth of less than 48 inches, and it is on the surface during part of the year.

Permeability of these soils is moderate. Effective rooting depth is limited by the water table. Available water capacity is moderate. Surface runoff is slow, and the hazard of erosion is slight. At times overflow deposits a damaging amount of silt and sand in the lower lying areas.

These soils are in meadow. They are used for native hay or for grazing.

These soils are well suited to the production of native vegetation suitable for grazing. The vegetation is mainly switchgrass, indiangrass, sedges, rushes, prairie cordgrass, western wheatgrass, and bluegrass. Cattails and bulrushes commonly grow in the swampy areas.

Management of distribution of livestock and stocking rates is necessary on these soils to avoid abuse of the range. In large areas, fences should be used to control grazing.

Wetland wildlife can be attracted to these soils and the wetland habitat enhanced by several means. Shallow water developments can be created by digging or by blasting potholes to create open-water areas. Fencing to control livestock use is beneficial, and it allows wetland plants such as cattails, reed canarygrass, and rushes to grow. Control of unplanned burning and prevention of drainage that would remove water from the wetlands are also good practices. These shallow marsh areas are often especially important for winter cover if natural vegetation is allowed to grow.

These soils are severely limited for use as homesites. The main limitations are a high water table and a hazard of periodic flooding. Community sewerage systems are needed because the high water table prevents septic tank absorption fields from functioning properly. Roads must also be designed to prevent frost-heave damage. Capability subclass Vw.

30—Fort Collins loam, 0 to 3 percent slopes. This deep, well drained soil formed in medium textured alluvium on uplands. Elevation ranges from 5,200 to 6,500 feet. The average annual precipitation ranges from about 13 inches at the lower elevations to about 15 inches at the higher elevations; the average annual temperature is about 49 degrees F; and the average frost-free period is about 145 days.

Typically, the surface layer is brown loam about 6 inches thick. The subsoil is brown clay loam about 15 inches thick. The substratum is pale brown loam.

Included with this soil in mapping are small areas of Stoneham sandy loam, 3 to 8 percent slopes; Keith silt loam, 0 to 3 percent slopes; Olney sandy loam, 0 to 3 percent slopes; Bresser sandy loam, 0 to 3 percent slopes; and Wiley silt loam, 1 to 3 percent slopes.

Permeability of this Fort Collins soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is medium, and the hazard of erosion is moderate.

This soil is used as rangeland and for dryland farming. Wheat and feed grains such as millet are the crops commonly grown. Crop residue management, minimum tillage, percent. Average annual precipitation is about 18 inches, and average annual air temperature is about 43 degrees F.

Elbeth soils are similar to Coldcreek, Fortwingate, and Tecolote soils and are near Kettle, Crowfoot, Pring, and Tomah soils. Coldcreek and Tecolote soils have a B2t horizon that is more than 35 percent coarse fragments. Coldcreek soils have bedrock at a depth of 20 to 40 inches. Fortwingate soils have a B2t horizon that is more than 35 percent clay and has hue of 5YR to 10R. Crowfoot, Pring, and Tomah soils have a mollic epipedon. Tomah and Kettle soils have a B2t horizon in which clay has accumulated as lamellae.

Typical pedon of Elbeth sandy loam, 8 to 15 percent slopes (fig. 9), at the southeast corner of the intersection of Frank Road and Swan Road in the NE1/4NE1/4 of sec. 9, T. 12 S., R. 65 W.:

A1—0 to 3 inches; very dark grayish brown (10YR 3/2) sandy loam, black (10YR 2/1) moist; moderate fine granular structure; soft, very friable; slightly acid; clear smooth boundary.

A2—3 to 23 inches; light gray (10YR 7/2) loamy sand, grayish brown (10YR 5/2) moist; weak medium subangular blocky structure; soft, very friable; slightly acid; clear wavy boundary.

B21t—23 to 32 inches; brown (7.5YR 5/4) sandy clay loam, dark brown (7.5YR 4/4) moist; moderate medium prismatic structure parting to moderate medium subangular blocky; extremely hard, firm, sticky and plastic; thin coatings of A2 material on faces of peds; continuous clay films on faces of peds; slightly acid; clear smooth boundary.

B22t—32 to 52 inches; brown (7.5YR 5/4) sandy clay loam, dark brown (7.5YR 4/4) moist; moderate medium prismatic structure parting to moderate medium subangular blocky; very hard, firm, sticky and plastic; continuous clay films on faces of peds; neutral; gradual smooth boundary.

B3—52 to 68 inches; reddish yellow (7.5YR 6/6) sandy clay loam, strong brown (7.5YR 5/6) moist; weak coarse prismatic structure parting to moderate medium subangular blocky; very hard, firm, slightly sticky and slightly plastic; thin patchy clay films on faces of peds; neutral; gradual smooth boundary.

C—68 to 74 inches; light yellowish brown (10YR 6/4) sandy clay loam, yellowish brown (10YR 5/4) moist; massive; hard, friable, slightly sticky; neutral.

The solum ranges from 24 to 60 inches in thickness. It is 0 to 15 percent coarse fragments. It ranges from strongly acid to neutral. The A1 horizon is very dark grayish brown or dark grayish brown. The A2 horizon is loamy sand or sand. The B2t horizon is brown or yellowish brown. The C horizon is light yellowish brown or pale brown.

Ellicott series

The Ellicott series consists of deep, somewhat excessively drained soils that formed in noncalcareous stratified sandy alluvium derived from arkose beds of granite. These soils are on terraces and flood plains. They have slopes of 0 to 5 percent. Average annual precipitation is about 14 inches, and average annual air temperature is about 48 degrees F.

Ellicott soils are similar to Ustic Torrifluvents, loamy, and are near Blakeland and Wigton soils. Ustic Torrifluvents, loamy, have stratified layers containing a higher percentage of clay and have a darker surface layer than Ellicott soils. Blakeland soils have a dark colored surface layer and are not stratified. Wigton soils are not stratified.

Typical pedon of Ellicott loamy coarse sand, 0 to 5 percent slopes, about 300 feet west and 1,650 feet south of the northeast corner of the NW1/4 of sec. 16, T. 14 S., R. 62 W.:

A1—0 to 4 inches; grayish brown (10YR 5/2) loamy coarse sand, dark grayish brown (10YR 4/2) moist; single grained; loose; 10 percent fine gravel; neutral; clear smooth boundary.

C—4 to 60 inches; light brownish gray (10YR 6/2) coarse sand stratified with layers of loamy sand, loamy coarse sand, and coarse sandy loam, dark grayish brown (10YR 4/2) moist; single grained; loose; 15 percent fine gravel; neutral.

The solum ranges from 2 to 8 inches in thickness. It is 0 to 35 percent coarse fragments. It ranges from slightly acid to mildly alkaline. The A1 horizon is grayish brown or brown loamy coarse sand or coarse sand. The C horizon is light brownish gray or pale brown.

Fort Collins series

The Fort Collins series consists of deep, well drained soils that formed in medium textured alluvium. These soils are on terraces and uplands. They have slopes of 0 to 8 percent. Average annual precipitation is about 13 inches, and average annual air temperature is about 49 degrees F.

Fort Collins soils are similar to Cushman, Olney, and Stoneham soils and are near the competing Olney and Stoneham soils. The Cushman soils have a paralithic contact at a depth of 20 to 40 inches. Olney soils have more than 35 percent fine or coarser sand in the B2t and C horizons. Stoneham soils are less than 15 inches deep to the base of any B3ca horizon.

Typical pedon of Fort Collins loam, 0 to 3 percent slopes, about 0.45 mile south and 400 feet east of the northwest corner of sec. 19, T. 17 S., R. 63 W.:

A1—0 to 6 inches; brown (10YR 5/3) loam, dark grayish brown (10YR 4/2) moist; moderate fine granular structure; soft, very friable; neutral; clear smooth boundary.

B1—6 to 9 inches; brown (10YR 5/3) loam, dark grayish brown (10YR 4/2) moist; weak medium subangular blocky structure; slightly hard, very friable; few thin patchy clay films on faces of peds; mildly alkaline; clear smooth boundary.

B2t—9 to 16 inches; brown (10YR 5/3) clay loam, dark brown (10YR 4/3) moist; moderate medium prismatic structure parting to moderate medium subangular blocky; hard, friable, sticky; thin continuous clay films on faces of peds; few fine pebbles; mildly alkaline; clear smooth boundary.

B3ca—16 to 21 inches; brown (10YR 5/3) light clay loam, grayish brown (10YR 5/2) moist; weak coarse prismatic structure parting to weak medium subangular blocky; hard, friable, slightly sticky; some visible calcium carbonate occurring as soft masses; calcareous; mildly alkaline; gradual smooth boundary.

C1ca—21 to 29 inches; pale brown (10YR 6/3) loam, brown (10YR 5/3) moist; weak coarse prismatic structure parting to weak medium and coarse subangular blocky; slightly hard, very friable; visible calcium carbonate occurring as soft masses and in thin seams and streaks; calcareous; moderately alkaline; diffuse smooth boundary.

C2ca—29 to 60 inches; pale brown (10YR 6/3) loam, brown (10YR 5/3) moist; massive; soft, very friable; contains less visible calcium carbonate than the above horizon; calcareous; moderately alkaline.

The solum ranges from 15 to 30 inches in thickness. Its content of coarse fragments ranges from 0 to 15 percent but commonly is less than 5 percent. It is neutral or mildly alkaline. The A1 horizon is grayish brown or brown loam or fine sandy loam. The B2t horizon is brown or

Included with this soil in mapping are small areas of Nunn clay loam, 0 to 3 percent slopes; Sampson loam, 0 to 3 percent slopes; and Ustic Torrifluvents, loamy.

Permeability of this Manzanola soil is slow. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is slow, and the hazard of erosion is moderate.

Most areas of this soil are used for irrigated crops. The main crops are alfalfa, corn, small grain, and pasture. Use of deep-rooted crops, timely tillage, and crop residue to keep the soil in good tilth are necessary on this soil. A small acreage of this soil is used for the production of forage sorghum or sudangrass for feed crops. The remaining acreage is used as nonirrigated cropland and rangeland.

This soil is well suited to plants for suitable grazing, and both grasses and legumes grow well if the soil is irrigated.

The native vegetation is mainly alkali sacaton, vine-mesquite, western wheatgrass, blue grama, and lesser amounts of switchgrass. Big bluestem, switchgrass, and junegrass are also present where this soil occurs in the northern part of the survey area.

Stocking rates and distribution of grazing should be controlled to facilitate uniform grazing. Fencing and properly locating livestock watering facilities help to control grazing. With good range management, this soil produces good quantities of forage.

Windbreaks and environmental plantings are generally well suited to this soil. Summer fallow a year prior to planting and continued cultivation for weed control are needed to insure the establishment and survival of plantings. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, Siberian peashrub, and American plum.

This soil is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. In cropland areas, habitat favorable for ring-necked pheasant, mourning dove, and many nongame species can be developed by establishing wildlife areas for nesting and escape cover. For pheasant, undisturbed nesting cover is vital and should be provided for in plans for habitat development. This is especially true in areas of intensive farming. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

The main limitations for urban use of this soil are slow permeability and shrink-swell potential. Septic tank absorption fields do not function well because of the slow permeability. Special designs for buildings and roads are required to overcome the limitation of the shrink-swell potential. Capability subclasses IIs, irrigated, and IVe, nonirrigated.

52—**Manzanola clay loam, 1 to 3 percent slopes.** This deep, well drained soil formed in calcareous loamy alluvium on fans and terraces. Elevation ranges from about 5,200 to 6,000 feet. The average annual precipitation is about 13 inches, the average annual air temperature is about 49 degrees F, and the average frost-free period is about 145 days.

Typically, the surface layer is grayish brown clay loam about 6 inches thick. The subsoil is grayish brown heavy clay loam about 26 inches thick. The substratum is grayish brown clay loam to a depth of 60 inches or more. The lower part of the subsoil and the substratum contain visible soft masses of lime.

Included with this soil in mapping are small areas of Manzanola clay loam, 0 to 1 percent slopes; Nunn clay loam, 0 to 3 percent slopes; and Sampson loam, 0 to 3 percent slopes.

Permeability of this Manzanola soil is slow. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is medium, and the hazard of erosion is moderate.

About 50 percent of the acreage of this soil is used for irrigated crops. The main crops are alfalfa, corn, small grain, and pasture. Use of deep-rooted crops, timely tillage, and crop residue to keep the soil in good tilth is necessary. A small percentage of this soil is used for the production of forage sorghum or sudangrass for feed crops. The remaining acreage is used as rangeland.

This soil is well suited to plants suitable for grazing, and grass and legumes grow well if it is irrigated.

The native vegetation is mainly alkali sacaton, vine-mesquite, western wheatgrass, blue grama, and lesser amounts of switchgrass. Big bluestem, switchgrass, and junegrass are also present where this soil occurs in the northern part of the survey area.

Stocking rates and distribution of grazing should be controlled to facilitate uniform grazing. Fences and proper location of livestock watering facilities help to control grazing. With good range management, this soil produces good quantities of forage.

Windbreaks and environmental plantings generally are well suited to this soil. Summer fallow a year prior to planting and continued cultivation for weed control are needed to insure the establishment and survival of plantings. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, Siberian peashrub, and American plum.

This soil is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. In cropland areas, habitat favorable for ring-necked pheasant, mourning dove, and many nongame species can be developed by establishing areas for nesting and escape cover. For pheasant, undisturbed nesting cover is vital and should be provided for in plans for habitat development. This is especially true in areas of intensive farming. Rangeland wildlife, such as pronghorn antelope, can be assisted by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

The main limitations for urban use of this soil are slow permeability and high shrink-swell potential. Septic tank absorption fields do not function well as a result of the slow permeability. Special designs for buildings and roads are required to overcome the limitation of the high shrink-swell potential. Capability subclasses IVe, nonirrigated, and IIe, irrigated.

53—Manzanola clay loam, 3 to 9 percent slopes. This deep, well drained soil formed in calcareous loamy alluvium on fans, terraces, and valley side slopes. Elevation ranges from about 5,200 to 6,000 feet. The average annual precipitation is about 13 inches, the average annual air temperature is about 49 degrees F, and the average frost-free period is about 145 days.

Typically, the surface layer is grayish brown clay loam about 6 inches thick. The subsoil is grayish brown heavy clay loam about 26 inches thick. The substratum is grayish brown clay loam to a depth of 60 inches or more. The lower part of the subsoil and the substratum contain visible soft masses of lime.

Included with this soil in mapping are small areas of Manvel loam, 3 to 9 percent slopes; Neville-Rednun complex, 3 to 9 percent slopes; and Satanta-Neville complex, 3 to 8 percent slopes.

Permeability of this Manzanola soil is slow. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is rapid, and the hazard of erosion is high.

Most areas of this soil are used as rangeland and for military maneuvers.

This soil is well suited to the production of native vegetation suitable for grazing. The native vegetation is mainly blue grama, western wheatgrass, side-oats grama, dropseed, and galleta. Production varies from year to year, depending on amount of precipitation.

Fencing and properly locating livestock watering facilities help to control grazing. Deferment of grazing may be necessary to maintain a needed balance between livestock use and forage production. In areas where the plant cover has been depleted, pitting can be used to help the native vegetation recover. Chemical control practices may be needed in disturbed areas where dense stands of pricklypear occur. Ample amounts of litter and forage need to be left on the soil because of the high hazard of soil blowing.

Windbreaks and environmental plantings generally are well suited to this soil. Summer fallow a year prior to planting and continued cultivation for weed control are needed to insure the establishment and survival of plantings. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, Siberian peashrub, and American plum.

This soil is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range wh needed.

The main limitations of this soil for urban uses are s permeability and high shrink-swell potential. Septic t absorption fields do not function well because of the s permeability. Special designs for buildings and roads required to overcome the limitation of high shrink-sw potential. Capability subclass VIe.

54—Midway clay loam, 3 to 25 percent slopes. T shallow, well drained soil formed in residuum deri from calcareous shale on uplands. Elevation ranges fr 5,200 to 6,200 feet. The average annual precipitation about 13 inches, the average annual air temperature about 49 degrees F, and the frost-free period is about 1 days.

Typically, the surface layer is light yellowish bro clay loam about 4 inches thick. The underlying material light yellowish brown clay about 4 inches thick and gr ish brown clay that contains 50 percent soft shale fr ments and is about 5 inches thick. Shale is at a depth 13 inches.

Included with this soil in mapping are small areas Louviers silty clay loam, 3 to 18 percent slopes; Nelso Tassel fine sandy loams, 3 to 18 percent slopes; and Raz clay loam, 3 to 9 percent slopes.

Permeability of this Midway soil is slow. Effective ro ing depth is less than 20 inches. Available water capaci is low. Surface runoff is medium to rapid, and the haza of erosion is moderate to high.

Most areas of this soil are used as rangeland.

The native vegetation is mainly blue grama, galleta, kali sacaton, western wheatgrass, and fourwing saltbus Little bluestem, side-oats grama, and needleandthread a also present where this soil occurs in the northern part the survey area. The presence of princesplume, tw groove milkvetch, and Fremont goldenweed indicates th selenium-bearing plants are in the stand.

This soil is difficult to revegetate, and it is therefo especially important that livestock grazing be carefull managed. Excessive removal of vegetation can result i severe erosion. Properly locating livestock watering facil ties helps to control grazing.

Windbreak and environmental plantings generally ar not suited to this soil. Onsite investigation is needed t determine if plantings are feasible.

This treeless soil produces little vegetation, especiall in times of drought, when annual production may be a low as 300 pounds per acre. Rangeland wildlife, such a antelope and scaled quail, can be encouraged by properl managing livestock grazing, installing livestock waterin facilities, and reseeding range where necessary.

The main limitations for the use of this soil as sites fo buildings and homes are shallow depth to shale and hig shrink-swell potential. Septic tank absorption fields do no function properly because of the slow permeability of thi soil. Practices are needed to reduce surface runoff an thus keep erosion to a minimum. Special designs fo buildings and roads are needed because of the shallow

Manzanola series

The Manzanola series consists of deep, well drained soils that formed in calcareous loamy alluvium. These soils are on fans, terraces, and sides of valleys. The have slopes of 0 to 9 percent. Average annual precipitation is about 13 inches, and average annual air temperature is about 49 degrees F.

The Manzanola soils are similar to Stoneham and Cushman soils and are near Nunn and Razor soils. Stoneham soils have a solum less than 15 inches thick and have a B2t horizon that is 18 to 35 percent clay. Cushman soils have interbedded sandstone and shale at a depth of 20 to 40 inches. Nunn soils have a mollic epipedon. Razor soils have a B2 horizon and have shale at a depth of 20 to 40 inches.

Typical pedon of Manzanola clay loam, 1 to 3 percent slopes, about 1,450 feet east and 20 feet north of the southwest corner of sec. 9, T. 16 S., R. 65 W.:

Ap—0 to 6 inches; grayish brown (10YR 5/2) clay loam, very dark grayish brown (10YR 3/2) moist; moderate medium granular structure; hard, firm, slightly sticky and slightly plastic; mildly alkaline; clear smooth boundary.

B21t—6 to 10 inches; brown (10YR 5/3) heavy clay loam, dark grayish brown (2.5Y 4/2) moist; weak medium prismatic structure parting to moderate medium subangular blocky; extremely hard, very firm, very sticky and very plastic; thin patchy clay films on faces of peds; calcareous; moderately alkaline; clear smooth boundary.

B22t—10 to 17 inches; grayish brown (2.5Y 5/2) heavy clay loam, dark grayish brown (2.5Y 4/2) moist; weak medium subangular blocky structure; very hard, very firm, very sticky and very plastic; thin continuous clay films on faces of peds; few indistinct lime threads; calcareous; moderately alkaline; clear smooth boundary.

B3ca—17 to 32 inches; grayish brown (2.5Y 5/2) clay loam, dark grayish brown (2.5Y 4/2) moist; weak medium subangular blocky structure; very hard, very firm, slightly sticky and slightly plastic; thin patchy clay films on faces of peds; visible lime threads; calcareous; moderately alkaline; clear smooth boundary.

C—32 to 60 inches; grayish brown (2.5Y 5/2) clay loam, dark grayish brown (2.5Y 4/2) moist; massive; extremely hard, very firm, sticky and plastic; 5 percent gravel; threads and soft masses of lime; calcareous; moderately alkaline.

The solum ranges from 20 to 36 inches in thickness. It is 0 to 15 percent coarse fragments. It ranges from mildly alkaline to strongly alkaline. The A1 or Ap horizon is grayish brown or light brownish gray. The B2t horizon is brown or grayish brown heavy clay loam or light clay. The C horizon is light brownish gray or grayish brown.

Midway series

The Midway series consists of shallow, well drained soils that formed in residuum derived from calcareous shale. These soils are on uplands. They have slopes of 3 to 50 percent. Average annual precipitation is about 13 inches, and average annual air temperature is about 49 degrees F.

Midway soils are similar to Louviers soils and are near Razor soils. Louviers soils are noncalcareous throughout. Razor soils have a B2 horizon and have shale bedrock at a depth of 20 to 40 inches.

Typical pedon of Midway clay loam, 3 to 25 percent slopes, near the southwest corner of sec. 13, T. 16 S., R. 65 W.:

A1—0 to 4 inches; light yellowish brown (2.5Y 6/4) clay loam, light olive brown (2.5Y 5/4) moist; weak thin platy structure parting to weak fine granular; soft, very friable, sticky and plastic; calcareous; moderately alkaline; clear smooth boundary.

AC—4 to 8 inches; light yellowish brown (2.5Y 6/4) clay, light olive brown (2.5Y 5/4) moist; weak thick platy structure parting to weak fine subangular blocky; soft, very friable, sticky and plastic; calcareous; strongly alkaline; clear smooth boundary.

C1—8 to 13 inches; grayish brown (2.5Y 5/2) clay, light olive brown (2.5Y 5/4) moist; weak thick platy structure; hard, friable, sticky and plastic; 50 percent shale fragments; calcareous; strongly alkaline.

C2r—13 inches; light olive brown (2.5Y 5/4) shale.

Depth to shale is 10 to 20 inches. The solum ranges from 8 to 20 inches in thickness. It is moderately alkaline or strongly alkaline. The A1 horizon is silty clay loam or clay loam. The C horizon is light brownish gray or grayish brown.

Nederland series

The Nederland series consists of deep, well drained soils that formed in cobbly and gravelly alluvium or outwash. These soils are on upland fans and terraces. They have slopes of 9 to 25 percent. Average annual precipitation is about 15 inches, and average annual air temperature is about 47 degrees F.

Nederland soils are similar to Stroupe soils and are near Neville and Chaseville soils. Stroupe soils have a B2t horizon that is more than 35 percent clay and have hard bedrock at a depth of 20 to 40 inches. Neville soils have a control section that is less than 15 percent coarse fragments. Chaseville soils do not have a B2t horizon and have less than 18 percent clay in the control section.

Typical pedon of Nederland cobbly sandy loam, 9 to 25 percent slopes, about 900 feet southwest of Highway 115 on the southwest bank of Rock Creek in sec. 31, T. 15 S., R. 66 W.:

A1—0 to 5 inches; brown (7.5YR 4/2) cobbly sandy loam, dark brown (7.5YR 3/2) moist; moderate fine granular structure; soft, very friable; 5 percent gravel and 15 percent cobbles; slightly acid; clear smooth boundary.

B1—5 to 11 inches; brown (7.5YR 5/2) very cobbly loam, dark brown (7.5YR 3/2) moist; weak fine subangular blocky structure; slightly hard, friable, slightly sticky and slightly plastic; few thin patchy clay films on faces of peds; 15 percent gravel and 25 percent cobbles; neutral; clear smooth boundary.

B2t—11 to 28 inches; reddish brown (5YR 5/4) very cobbly clay loam, reddish brown (5YR 4/4) moist; weak medium prismatic structure parting to moderate fine subangular blocky; hard, firm, sticky and plastic; thin clay films on faces of peds; 55 percent gravel and cobbles; neutral; gradual wavy boundary.

C—28 to 60 inches; reddish brown (5YR 5/4) very cobbly sandy loam, reddish brown (5YR 4/4) moist; massive; hard, friable; 45 percent cobbles and gravel; neutral.

The solum ranges from 17 to 30 inches in thickness. It is 35 to 60 percent coarse fragments. It ranges from slightly acid to mildly alkaline. The A1 horizon is brown or dark brown. The B2t horizon is reddish brown or light reddish brown very cobbly sandy clay loam to very cobbly clay loam. The C horizon is reddish brown or light reddish brown.

Legend

▩ 1% annual chance (100-Year) Floodplain

▨ 1% annual chance (100-Year) Floodway

░ 0.2% annual chance (500-Year) Floodplain

APPROXIMATE SCALE IN FEET

500 0 500

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP

EL PASO COUNTY,
COLORADO AND
INCORPORATED AREAS

REVISED TO
REFLECT LOMR
EFFECTIVE: January 29, 2015

PANEL 957 OF 1300
(SEE MAP INDEX FOR PANELS NOT PRINTED)

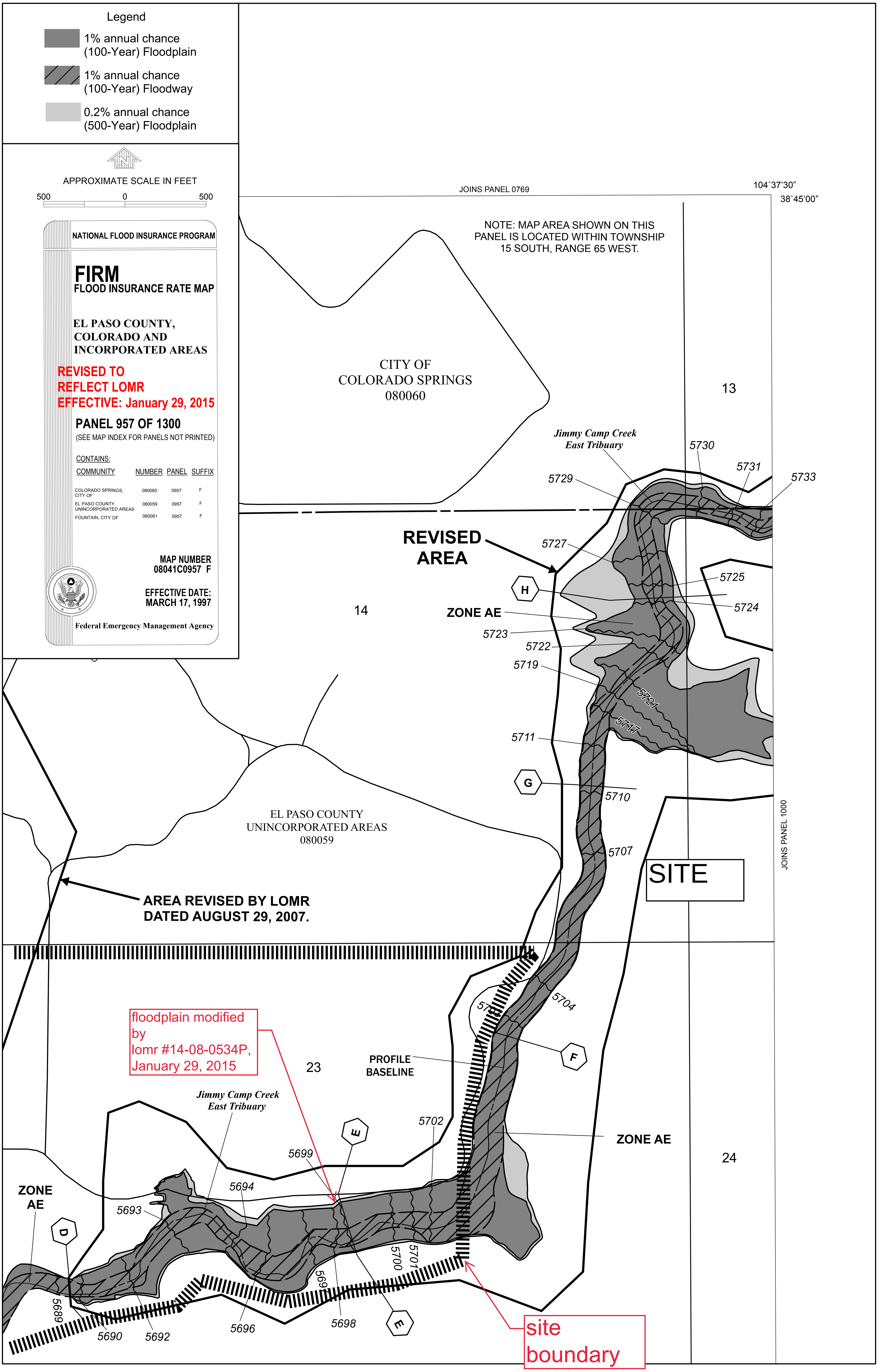
CONTAINS:

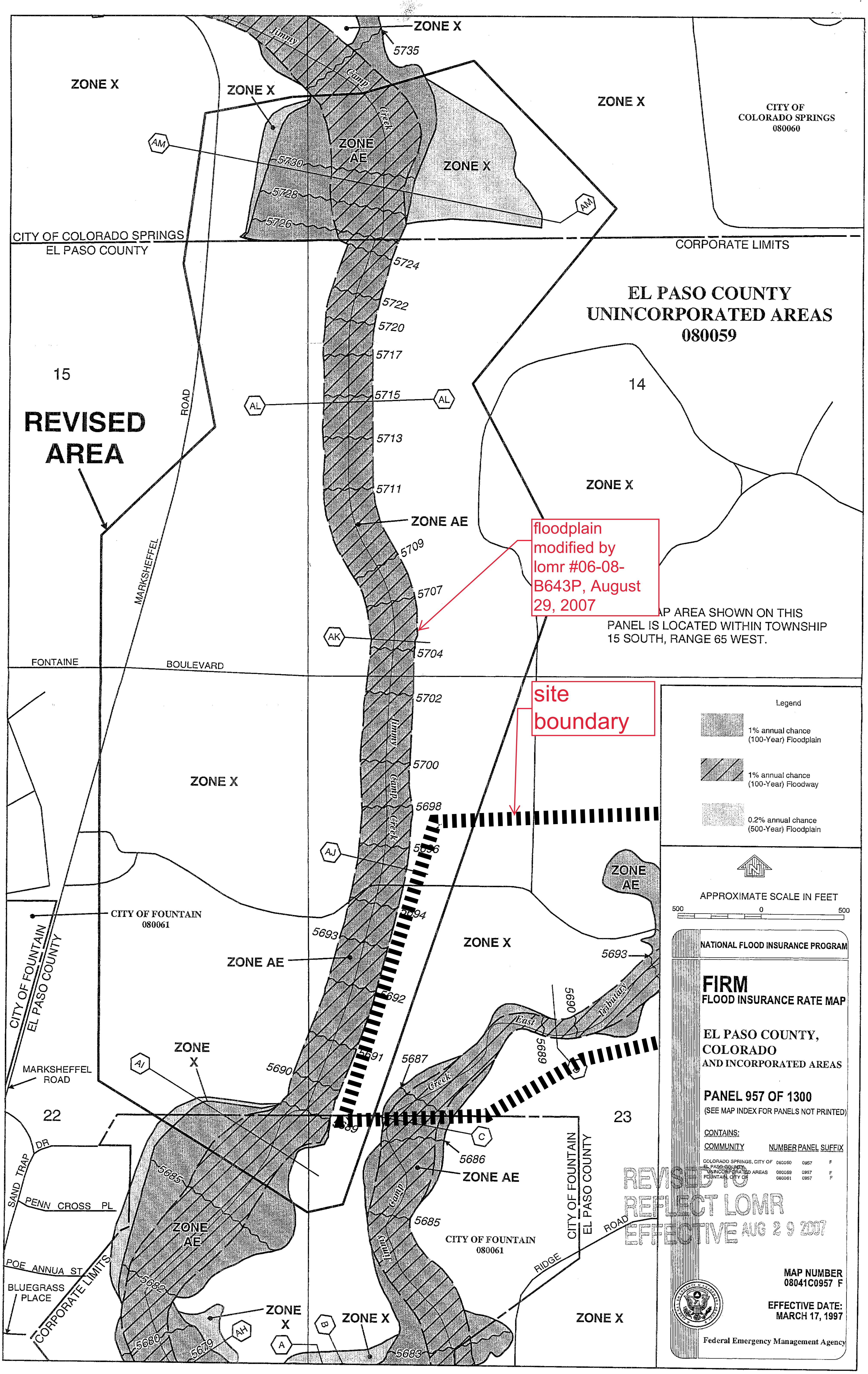
COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	080060	0957	F
EL PASO COUNTY UNINCORPORATED AREAS	080059	0957	F
FOUNTAIN, CITY OF	080061	0957	F

MAP NUMBER
08041C0957 F

EFFECTIVE DATE:
MARCH 17, 1997

Federal Emergency Management Agency





EL PASO COUNTY UNINCORPORATED AREAS 080059

MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 15 SOUTH, RANGE 65 WEST.

floodplain modified by lomr #06-08-B643P, August 29, 2007

site boundary

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO
AND INCORPORATED AREAS

PANEL 957 OF 1300
(SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	080060	0957	F
EL PASO COUNTY, UNINCORPORATED AREAS	080059	0957	F
FOUNTAIN, CITY OF	080061	0957	F

REVISED TO REFLECT LOMR EFFECTIVE AUG 2 9 2007

MAP NUMBER
08041C0957 F

EFFECTIVE DATE:
MARCH 17, 1997

Federal Emergency Management Agency

APPENDIX B – HYDROLOGY CALCULATIONS

CORE
ENGINEERING GROUP

Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: July 17, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **5 - Year Event, Existing Conditions**

Street or Basin	Design Point	Direct Runoff							Total Runoff				Street		Pipe			Travel Time			Remarks
		Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	Σ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
EX-B			35.50	0.16	19.9	5.68	3.09	17.6													
EX-C1			10.32	0.15	16.0	1.55	3.42	5.3													
EX-D			29.29	0.09	18.0	2.64	3.25	8.6													

CORE
ENGINEERING GROUP

Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: July 17, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **100 - Year Event, Existing Conditions**

Street or Basin	Design Point	Direct Runoff							Total Runoff				Street		Pipe			Travel Time			Remarks
		Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	Σ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
EX-B			35.50	0.51	19.9	18.11	5.19	94.0													
EX-C1			10.32	0.50	16.0	5.16	5.75	29.7													
EX-D			29.29	0.36	18.0	10.54	5.45	57.5													

CORE ENGINEERING GROUP

Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: Oct 20, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **5 - Year Event, Proposed Conditions**

Street or Basin	Design Point	Area Design	Direct Runoff						Total Runoff				Street		Pipe			Travel Time			Remarks
			Area (A) ac.	Runoff Coeff. (C)	tc min.	CA	i in/hr	Q cfs	tc min	Σ (CA)	i in/hr	Q cfs	Slope %	Street Flow cfs	Design Flow cfs	Slope %	Pipe Size in	Length ft	Velocity ft/sec	tt min	
C1.1			2.27	0.49	16.46	1.11	3.38	3.8					1.0%	3.8							
C1.2			3.35	0.49	17.36	1.64	3.30	5.4					0.9%	5.4							
(C1.1&C1.2)	**1**		5.62						17.4	2.75	3.30	9.1	L.P.	9.1	9.1	1.0%	24"	35'	5.3	0.1	
C1.3			1.10	0.49	10.47	0.54	4.06	2.2					1.0%	1.8							
C1.4			2.41	0.49	12.59	1.18	3.78	4.5					1.1%	4.5							
(C1.3&C1.4)	**2**		3.51						17.5	1.72	3.29	5.7	L.P.	5.3							
C1.5			0.19	0.49	6.56	0.09	4.76	0.4					1.3%	0.4							
(C1.3-C1.5)	**3**		3.70						17.5	1.81	3.29	6.0	L.P.	5.6							
(C1.1-C1.5)	**4**		9.32						17.5	4.57	3.29	15.0	L.P.	14.7	14.7	2.3%	24"	132'	6.5	0.3	
C1.6			0.73	0.49	9.81	0.36	4.16	1.5					0.8%	1.5							
C1.7			1.92	0.45	14.53	0.86	3.57	3.1					0.6%	3.1							
C1.8			0.77	0.47	8.47	0.36	4.38	1.6					1.0%	1.6							
(C1.7&C1.8)	**5**		2.69						16.6	1.23	3.37	4.1	L.P.	4.1							
(C1.6-C1.8)	**6**		3.42						16.6	1.58	3.37	5.3	L.P.	5.3							
C1.9			2.90	0.49	16.04	1.42	3.42	4.9					0.8%	3.5							
C1.10			0.18	0.49	9.30	0.09	4.24	0.4					0.8%	0.4							
C1.11			0.17	0.49	6.72	0.08	4.73	0.4													

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Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: Oct 20, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **5 - Year Event, Proposed Conditions**

Street or Basin	Design Point	Direct Runoff							Total Runoff				Street		Pipe			Travel Time			Remarks
		Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	Σ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
(C1.9-C1.11)	**7**		3.25						16.0	1.59	3.42	5.4	0.8%	4.1							
C1.12			1.42	0.49	14.53	0.70	3.57	2.5					0.9%	1.8							
(C1.9-C1.12)	**8**		4.67						20.8	2.29	3.03	6.9	L.P.	5.2							
C1.13			0.71	0.45	9.25	0.32	4.25	1.4					0.7%	1.4							
C1.14			1.27	0.46	11.74	0.58	3.89	2.3					0.7%	2.3							
(C1.13&C1.14)	**9**		1.98						15.3	0.90	3.49	3.2	L.P.	3.2							
(C1.9-C1.14)	**10**		6.65						20.8	3.19	3.03	9.7	L.P.	7.9							
C1.15			1.40	0.49	10.96	0.69	3.99	2.7					1.0%	1.6							
C1.16			0.50	0.49	7.61	0.25	4.54	1.1					1.3%	1.1							
(C1.15&C1.16)	**11**		1.30						11.0	0.93	3.99	3.7	L.P.	2.5							
C1.17	**12**		1.38	0.49	9.44	0.68	4.22	2.9													
(C1.15-C1.17)	**13**		2.68						11.4	1.61	3.94	6.3			5.2	1.6%	18"	185'	2.9	1.1	
C1.18			5.81	0.27	13.91	1.57	3.63	5.7													
	14								20.8	4.78	3.03	14.5									
C2			3.44	0.49	8.54	1.69	4.37	7.4													
C4			1.84	0.47	6.48	0.86	4.78	4.1													

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Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: Oct 20, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **5 - Year Event, Proposed Conditions**

Street or Basin	Design Point	Direct Runoff							Total Runoff				Street		Pipe			Travel Time			Remarks
		Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	Σ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
C5.1	**15**		1.14	0.45	9.02	0.51	4.28	2.2					L.P.	1.3	1.3	1.0%	18"	34'	1.2	0.0	
C5.2			0.72	0.45	9.85	0.32	4.15	1.3													
C5			1.86						9.9	0.84	4.15	3.5									
C6			0.80	0.45	9.85	0.36	4.15	1.5													
D1.1	**16**		1.21	0.45	12.00	0.54	3.86	2.1							2.1	1.0%	18"	385'	1.2	5.3	
D1.2			0.55	0.90	8.36	0.50	4.40	2.2													
D1.3			0.42	0.45	10.41	0.19	4.07	0.8													
(D1.2&D1.3)	**17**		0.97						10.4	0.68	4.07	2.8	1.1%	2.8							
D1.4			1.13	0.45	9.53	0.51	4.20	2.1					1.3%	2.1							
(D1.2-D1.4)	**18**		2.10						14.9	1.19	3.53	4.2	1.0%	4.2							
D1.5			1.07	0.45	11.63	0.48	3.90	1.9					0.9%	1.5							
(D1.2-D1.5)	**19**		3.17						19.6	1.67	3.12	5.2	L.P.	4.9	12.1	3.0%	24"	50'	3.9	0.2	
D1.6			1.26	0.45	12.39	0.57	3.81	2.2					1.1%	2.2							
D1.7			1.39	0.45	14.42	0.63	3.58	2.2					0.7%	2.2							
(D1.6&D1.7)	**20**		2.65						14.4	1.19	3.58	4.3	0.7%	4.3							
D1.8			1.05	0.45	14.94	0.47	3.53	1.7					0.8%	1.7							
(D1.6-D1.8)	**21**		3.70						14.9	1.67	3.53	5.9									

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Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: Oct 20, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **5 - Year Event, Proposed Conditions**

		Direct Runoff							Total Runoff				Street		Pipe			Travel Time			
Street or Basin	Design Point	Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	Σ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	Remarks
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
D1.9			0.24	0.45	6.68	0.11	4.73	0.5					1.1%	0.5							
(D1.6-D1.9)	**22**		3.94						16.3	1.77	3.40	6.0	L.P.	6.0	12.1	3.0%	24"	50'	3.9	0.2	
(D1.2-D1.9)	**23**		7.11						19.6	3.45	3.12	10.8	L.P.	12.1	12.1	3.0%	24"	50'	3.9	0.2	
D1	**24**		8.32						19.7	3.99	3.11	12.4									
D2			1.16	0.45	7.68	0.52	4.53	2.4													
D3			0.79	0.16	10.79	0.13	4.02	0.5													
D4			1.28	0.45	6.38	0.58	4.80	2.8													
D5			1.60	0.22	11.76	0.35	3.89	1.4													
D6			0.23	0.16	10.56	0.04	4.05	0.1													

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Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: Oct 20, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **100 - Year Event, Proposed Conditions**

		Direct Runoff							Total Runoff				Street		Pipe			Travel Time			
Street or Basin	Design Point	Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	Σ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	Remarks
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
C1.1			2.27	0.65	16.46	1.48	5.68	8.4													
													1.0%	8.4							
C1.2			3.35	0.65	17.36	2.18	5.54	12.1													
													0.9%	12.1							
(C1.1&C1.2)	**1**		5.62						17.4	3.65	5.54	20.2									
													L.P.	20.2	20.2	1.0%	24"	35'	7.5	0.1	
C1.3			1.10	0.65	10.47	0.72	6.82	4.9													
													1.0%	4.0							
C1.4			2.41	0.65	12.59	1.57	6.35	10.0													
													1.1%	10.0							
(C1.3&C1.4)	**2**		3.51						17.5	2.28	5.52	12.6									
													L.P.	11.9							
C1.5			0.19	0.65	6.56	0.12	7.99	1.0													
													1.3%	1.0							
(C1.3-C1.5)	**3**		3.70						17.5	2.41	5.52	13.3									
													L.P.	12.6							
(C1.1-C1.5)	**4**		9.32						17.5	6.06	5.52	33.4									
													L.P.	32.7	32.7	2.3%	24"	132'	10.4	0.2	
C1.6			0.73	0.65	9.81	0.47	6.98	3.3													
													0.8%	3.3							
C1.7			1.92	0.59	14.53	1.13	5.99	6.8													
													0.6%	6.8							
C1.8			0.77	0.62	8.47	0.48	7.35	3.5													
													1.0%	3.5							
(C1.7&C1.8)	**5**		2.69						16.6	1.61	5.65	9.1									
													L.P.	9.1							
(C1.6-C1.8)	**6**		3.42						16.6	2.08	5.65	11.8									
													L.P.	11.8							
C1.9			2.90	0.65	16.04	1.89	5.74	10.8													
													0.8%	7.8							
C1.10			0.18	0.65	9.30	0.12	7.12	0.8													
													0.8%	0.9							
C1.11			0.17	0.65	6.72	0.11	7.93	0.9													

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Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: Oct 20, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **100 - Year Event, Proposed Conditions**

		Direct Runoff							Total Runoff				Street		Pipe			Travel Time			
Street or Basin	Design Point	Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	∑ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	Remarks
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
(C1.9-C1.11)	**7**		3.25						16.0	2.11	5.74	12.1	0.8%	9.1							
C1.12			1.42	0.65	14.53	0.92	5.99	5.5					0.9%	4.1							
(C1.9-C1.12)	**8**		4.67						20.8	3.04	5.08	15.4	L.P.	11.6							
C1.13			0.71	0.59	9.25	0.42	7.13	3.0					0.7%	3.0							
C1.14			1.27	0.61	11.74	0.77	6.53	5.1					0.7%	5.1							
(C1.13&C1.14)	**9**		1.98						15.3	1.19	5.86	7.0	L.P.	7.0							
(C1.9-C1.14)	**10**		6.65						20.8	4.23	5.08	21.5	L.P.	16.7							
C1.15			1.40	0.65	10.96	0.91	6.70	6.1					1.0%	3.5							
C1.16			0.50	0.65	7.61	0.33	7.62	2.5					1.3%	2.5							
(C1.15&C1.16)	**11**		1.90						11.0	1.24	6.70	8.3	L.P.	5.7							
C1.17	**12**		1.38	0.65	9.44	0.90	7.08	6.3													
(C1.15-C1.17)	**13**		3.28						11.4	2.13	6.61	14.1			11.5	1.6%	18"	185'	6.5	0.5	
C1.18			5.81	0.55	13.91	3.20	6.10	19.5													
	14								20.8	6.31	5.09	32.1									
C2			3.44	0.65	8.54	2.24	7.33	16.4													
C4			1.84	0.62	6.48	1.14	8.03	9.2													

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Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: Oct 20, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **100 - Year Event, Proposed Conditions**

		Direct Runoff							Total Runoff				Street		Pipe			Travel Time			
Street or Basin	Design Point	Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	Σ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	Remarks
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
C5.1	**15**		1.14	0.45	9.02	0.51	7.19	3.7					L.P.	2.2	2.2	1.0%	18"	34'	2.7	0.0	
C5.2			0.72	0.45	9.85	0.32	6.97	2.3													
C5			1.86						9.0	0.84	7.19	6.0									
C6			0.80	0.59	9.85	0.47	6.97	3.3													
D1.1	**16**		1.21	0.59	12.00	0.71	6.47	4.6							4.6	1.0%	18"	385'	2.6	2.5	
D1.2			0.55	0.96	8.36	0.53	7.38	3.9													
D1.3			0.42	0.59	10.41	0.25	6.83	1.7													
(D1.2&D1.3)	**17**		0.97						10.4	0.78	6.83	5.3	1.1%	5.3							
D1.4			1.13	0.59	9.53	0.67	7.05	4.7					1.3%	4.7							
(D1.2-D1.4)	**18**		2.10						14.9	1.44	5.93	8.6	1.0%	8.6							
D1.5			1.07	0.59	11.63	0.63	6.55	4.1					0.9%	3.4							
(D1.2-D1.5)	**19**		3.17						19.6	2.07	5.24	10.9	L.P.	10.3	26.1	3.0%	24"	50'	8.3	0.1	
D1.6			1.26	0.59	12.39	0.74	6.39	4.8					1.1%	4.8							
D1.7			1.39	0.59	14.42	0.82	6.01	4.9					0.7%	4.9							
(D1.6&D1.7)	**20**		2.65						14.4	1.56	6.01	9.4	0.7%	9.4							
D1.8			1.05	0.59	14.94	0.62	5.92	3.7					0.8%	3.7							
(D1.6-D1.8)	**21**		3.70						14.9	2.18	5.92	12.9									

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Standard Form SF-2. Storm Drainage System Design (Rational Method Procedure)

Calculated By: Leonard Beasley
Date: Oct 20, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1
Design Storm: **100 - Year Event, Proposed Conditions**

Street or Basin	Design Point	Direct Runoff							Total Runoff				Street		Pipe			Travel Time			Remarks
		Area Design	Area (A)	Runoff Coeff. (C)	tc	CA	i	Q	tc	∑ (CA)	i	Q	Slope	Street Flow	Design Flow	Slope	Pipe Size	Length	Velocity	tt	
			ac.		min.		in/hr	cfs	min		in/hr	cfs	%	cfs	cfs	%	in	ft	ft/sec	min	
D1.9			0.24	0.59	6.68	0.14	7.95	1.1					1.1%	1.1							
(D1.6-D1.9)	**22**		3.94						16.3	2.32	5.71	13.3	L.P.	13.3	26.1	3.0%	24"	50'	8.3	0.1	
(D1.2-D1.9)	**23**		7.11						19.6	4.40	5.24	23.1	L.P.	26.1	26.1	3.0%	24"	50'	8.3	0.1	
D1	**24**		8.32						19.7	5.11	5.22	26.7									
D2			1.16	0.59	7.68	0.68	7.60	5.2													
D3			0.79	0.41	10.79	0.32	6.74	2.2													
D4			1.28	0.59	6.38	0.76	8.07	6.1													
D5			1.60	0.45	11.76	0.72	6.52	4.7													
D6			0.23	0.41	10.56	0.09	6.80	0.6													

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Standard Form SF-1. Time of Concentration-Proposed

Calculated By: Leonard Beasley
Date: June 29, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1

Sub-Basin Data				Initial Overland Time (t_i)				Travel Time (t_t)					t_c Check (urbanized Basins)		Final t_c
BASIN or DESIGN	C_5	AREA (A) acres	NRCS Convey.	LENGTH (L) feet	SLOPE (S) %	VELOCITY (V) ft/sec	t_i minutes	LENGTH (L) feet	SLOPE (S) %	VELOCITY (V) ft/sec	t_t minutes	Computed tc Minutes	TOTAL LENGTH (L) feet	Regional tc tc=(L/180)+10 minutes	USDCM Recommended $t_c=t_i+t_t$ (min)
C1.1	0.49	2.27	20	86.00	2.10%	0.18	8.00	1076.0	1.05%	2.05	8.75	16.75	1162.00	16.46	16.46
C1.2	0.49	3.35	20	59.00	1.90%	0.14	6.84	1265.0	0.94%	1.94	10.87	17.72	1324.00	17.36	17.36
DP-1	0.49	5.62	20	59.00	1.90%	0.14	6.84	1265.0	0.94%	1.94	10.87	17.72	1324.00	17.36	17.36
C1.3	0.49	0.90	20	76.00	2.00%	0.17	7.64	340.0	1.00%	2.00	2.83	10.47	416.00	12.31	10.47
C1.4	0.49	2.41	20	36.00	2.80%	0.13	4.70	1010.0	1.14%	2.14	7.88	12.59	1046.00	15.81	12.59
DP-2	0.49	3.31	20	76.00	2.00%	0.17	7.64	1280.0	1.00%	2.00	10.67	18.30	1356.00	17.53	17.53
C1.5	0.49	0.19	20	45.00	2.00%	0.13	5.88	93.0	1.29%	2.27	0.68	6.56	138.00	10.77	6.56
DP-3	0.49	3.50	20	76.00	2.00%	0.17	7.64	1280.0	1.00%	2.00	10.67	18.30	1356.00	17.53	17.53
C1.6	0.49	0.73	20	28.00	2.00%	0.10	4.64	559.0	0.81%	1.80	5.18	9.81	587.00	13.26	9.81
C1.7	0.45	1.92	20	100.00	2.00%	0.18	9.34	716.0	0.63%	1.59	7.52	16.85	816.00	14.53	14.53
C1.8	0.47	0.77	20	20.00	2.00%	0.08	4.05	520.0	0.96%	1.96	4.42	8.47	540.00	13.00	8.47
DP-5	0.46	2.69	20	100.00	2.00%	0.18	9.19	1093.0	0.73%	1.71	10.66	19.85	1193.00	16.63	16.63
C1.9	0.49	2.10	20	50.00	2.00%	0.13	6.20	1057.0	0.80%	1.79	9.85	16.04	1107.00	16.15	16.04
C1.10	0.49	0.18	20	100.00	2.30%	0.20	8.37	100.0	0.80%	1.79	0.93	9.30	200.00	11.11	9.30
C1.11	0.49	0.17	20	42.00	2.00%	0.12	5.68	116.0	0.86%	1.85	1.04	6.72	158.00	10.88	6.72
C1.12	0.49	1.42	20	98.00	2.45%	0.20	8.11	717.0	0.71%	1.69	7.09	15.20	815.00	14.53	14.53
DP-8	0.49	3.50	20	50.00	2.00%	0.13	6.20	1902.0	0.76%	1.74	18.18	24.38	1952.00	20.84	20.84

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Standard Form SF-1. Time of Concentration-Proposed

Calculated By: Leonard Beasley
Date: June 29, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1

Sub-Basin Data				Initial Overland Time (t_i)				Travel Time (t_t)					t_c Check (urbanized Basins)		Final t_c
BASIN or DESIGN	C_5	AREA (A) acres	NRCS Convey.	LENGTH (L) feet	SLOPE (S) %	VELOCITY (V) ft/sec	t_i minutes	LENGTH (L) feet	SLOPE (S) %	VELOCITY (V) ft/sec	t_t minutes	Computed t_c Minutes	TOTAL LENGTH (L) feet	Regional tc tc=(L/180)+10 minutes	USDCM Recommended $t_c=t_i+t_t$ (min)
C1.13	0.45	0.71	20	42.00	3.33%	0.14	5.11	400.0	0.65%	1.61	4.13	9.25	442.00	12.46	9.25
C1.14	0.46	1.27	20	34.00	2.00%	0.11	5.36	641.0	0.70%	1.67	6.38	11.74	675.00	13.75	11.74
DP-9	0.46	1.98	20	42.00	3.33%	0.14	5.03	1002.0	0.66%	1.62	10.28	15.31	1044.00	15.80	15.31
C1.15	0.49	0.80	20	85.00	2.47%	0.19	7.53	401.0	0.95%	1.95	3.43	10.96	486.00	12.70	10.96
C1.16	0.49	0.50	20	37.00	2.00%	0.12	5.33	315.0	1.33%	2.31	2.28	7.61	352.00	11.96	7.61
C1.17	0.49	1.38	15	77.00	3.25%	0.20	6.55	300.0	1.33%	1.73	2.89	9.44	377.00	12.09	9.44
DP-12	0.49	2.68	20	85.00	2.47%	0.19	7.53	401.0	0.95%	1.95	3.43				
			18" RCP					185.0	1.62%	7.57	0.41	11.37	671.00	13.73	11.37
C1.18	0.27	5.81	15	100.00	3.00%	0.16	10.43	38.0	23.68%	7.30	0.09				
			20					565.0	0.50%	1.41	6.66	17.17	703.00	13.91	13.91
C1	0.49	26.51	20	50.00	2.00%	0.13	6.20	1902.0	0.76%	1.74	18.18	24.38	1952.00	20.84	20.84
C2	0.49	5.44	15	100.00	4.00%	0.24	6.97	150.0	1.13%	1.59	1.57	8.54	250.00	11.39	8.54
C4	0.47	1.84	15	30.00	2.00%	0.10	4.96	236.0	2.97%	2.59	1.52	6.48	266.00	11.48	6.48
C5.1	0.45	1.14	20	80.00	2.50%	0.17	7.76	197.0	1.68%	2.59	1.27	9.02	277.00	11.54	9.02
C5.2	0.45	0.72	15	100.00	2.00%	0.18	9.34	79.0	6.33%	3.77	0.35				
			15					58.0	15.52%	5.91	0.16	9.85	237.00	11.32	9.85

CORE
ENGINEERING GROUP

Standard Form SF-1. Time of Concentration-Proposed

Calculated By: Leonard Beasley
Date: June 29, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1

Sub-Basin Data				Initial Overland Time (t_i)				Travel Time (t_t)					t_c Check (urbanized Basins)		Final t_c
BASIN or DESIGN	C_5	AREA (A) acres	NRCS Convey.	LENGTH (L) feet	SLOPE (S) %	VELOCITY (V) ft/sec	t_i minutes	LENGTH (L) feet	SLOPE (S) %	VELOCITY (V) ft/sec	t_t minutes	Computed t_c Minutes	TOTAL LENGTH (L) feet	Regional t_c $t_c=(L/180)+10$ minutes	USDCM Recommended $t_c=t_i+t_t$ (min)
C5	0.45	1.86	15	100.00	2.00%	0.18	9.34	79.0	6.33%	3.77	0.35				
			15					58.0	15.52%	5.91	0.16	9.85	237.00	11.32	9.85
C6	0.45	0.80	15	100.00	2.00%	0.18	9.34	120.0	6.67%	3.87	0.52	9.85	220.00	11.22	9.85
D1.1	0.45	1.21	15	90.00	2.67%	0.19	8.05	445.0	1.57%	1.88	3.95	12.00	535.00	12.97	12.00
D1.2	0.90	0.55	20	30.00	2.00%	0.32	1.57	681.0	0.70%	1.67	6.78	8.36	711.00	13.95	8.36
D1.3	0.45	0.42	20	100.00	2.00%	0.18	9.34	135.0	1.10%	2.10	1.07	10.41	235.00	11.31	10.41
D1.4	0.45	1.13	20	46.00	3.26%	0.14	5.39	556.0	1.25%	2.24	4.14	9.53	602.00	13.34	9.53
DP-16	0.57	2.10	20	30.00	2.00%	0.12	4.17	1289.0	1.01%	2.01	10.69	14.86	1319.00	17.33	14.86
D1.5	0.45	0.87	20	61.00	1.64%	0.13	7.79	433.0	0.88%	1.88	3.85	11.63	494.00	12.74	11.63
DP-17	0.53	2.97	20	30.00	2.00%	0.11	4.48	1771.0	0.96%	1.96	15.06	19.55	1801.00	20.01	19.55
D1.6	0.45	1.26	20	47.00	2.00%	0.12	6.40	736.0	1.05%	2.05	5.99	12.39	783.00	14.35	12.39
D1.7	0.45	1.39	20	100.00	3.50%	0.21	7.76	696.0	0.72%	1.70	6.84	14.60	796.00	14.42	14.42
DP-18	0.45	2.65	20	100.00	3.50%	0.21	7.76	696.0	0.72%	1.70	6.84	14.60	796.00	14.42	14.42
D1.8	0.45	1.05	20	100.00	2.00%	0.18	9.34	789.0	0.79%	1.78	7.40	16.73	889.00	14.94	14.94
DP-19	0.45	3.70	20	100.00	2.00%	0.18	9.34	789.0	0.79%	1.78	7.40	16.73	889.00	14.94	14.94
D1.9	0.45	0.24	20	39.00	3.08%	0.13	5.06	206.0	1.12%	2.12	1.62	6.68	245.00	11.36	6.68
DP-20	0.45	3.94	20	100.00	2.00%	0.18	9.34	1029.0	0.86%	1.85	9.25	18.58	1129.00	16.27	16.27

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Standard Form SF-1. Time of Concentration-Proposed

Calculated By: Leonard Beasley
Date: June 29, 2018
Checked By: Leonard Beasley

Job No: 100.045
Project: Creekside Filing No. 1

Sub-Basin Data				Initial Overland Time (t_i)				Travel Time (t_t)					t_c Check (urbanized Basins)		Final t_c
BASIN or DESIGN	C_5	AREA (A) acres	NRCS Convey.	LENGTH (L) feet	SLOPE (S) %	VELOCITY (V) ft/sec	t_i minutes	LENGTH (L) feet	SLOPE (S) %	VELOCITY (V) ft/sec	t_t minutes	Computed t_c Minutes	TOTAL LENGTH (L) feet	Regional tc tc=(L/180)+10 minutes	USDCM Recommended $t_c=t_i+t_t$ (min)
D1	0.48	8.12	20	30.00	2.00%	0.10	4.88	1771.0	0.99%	1.99	14.83	19.71	1801.00	20.01	19.71
D2	0.45	1.16	15	50.00	16.00%	0.25	3.32	314.0	0.64%	1.20	4.36	7.68	364.00	12.02	7.68
D3	0.16	0.79	7	100.00	1.00%	0.10	16.97	43.0	1.00%	0.70	1.02	18.00	143.00	10.79	10.79
D4	0.45	1.28	20	60.00	3.33%	0.16	6.11	67.0	4.48%	4.23	0.26	6.38	127.00	10.71	6.38
D5	0.22	1.60	20	95.00	3.37%	0.15	10.37	81.0	11.11%	6.67	0.20				
			15					140.0	1.00%	1.50	1.56	12.13	316.00	11.76	11.76
D6	0.16	0.23	15	100.00	2.00%	0.12	13.50					13.50	100.00	10.56	10.56

APPENDIX C – HYDRAULIC CALCULATIONS

Channel Report

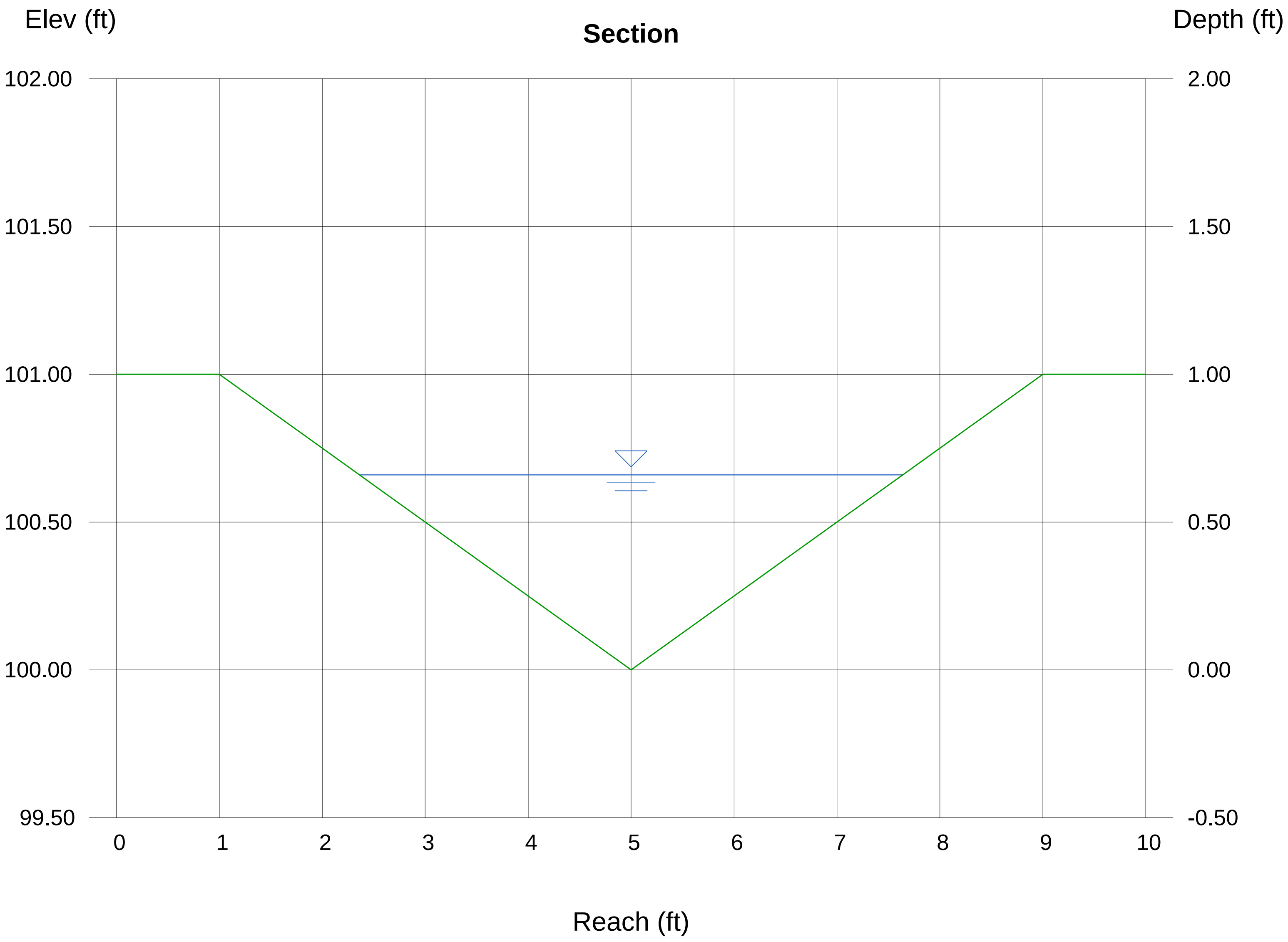
Pond CR3 collection swale

Triangular
Side Slope (z:1) = 4.00
Total Depth (ft) = 1.00

Invert Elev (ft) = 100.00
Slope (%) = 0.50
N-Value = 0.025

Calculations
Compute by: Known Q
Known Q (cfs) = 3.30

Highlighted
Depth (ft) = 0.66
Q (cfs) = 3.300
Area (sqft) = 1.74
Velocity (ft/s) = 1.89
Wetted Perim (ft) = 5.44
Crit Depth, Yc (ft) = 0.54
Top Width (ft) = 5.28
EGL (ft) = 0.72



Channel Report

trickle channel pond cr2

Rectangular

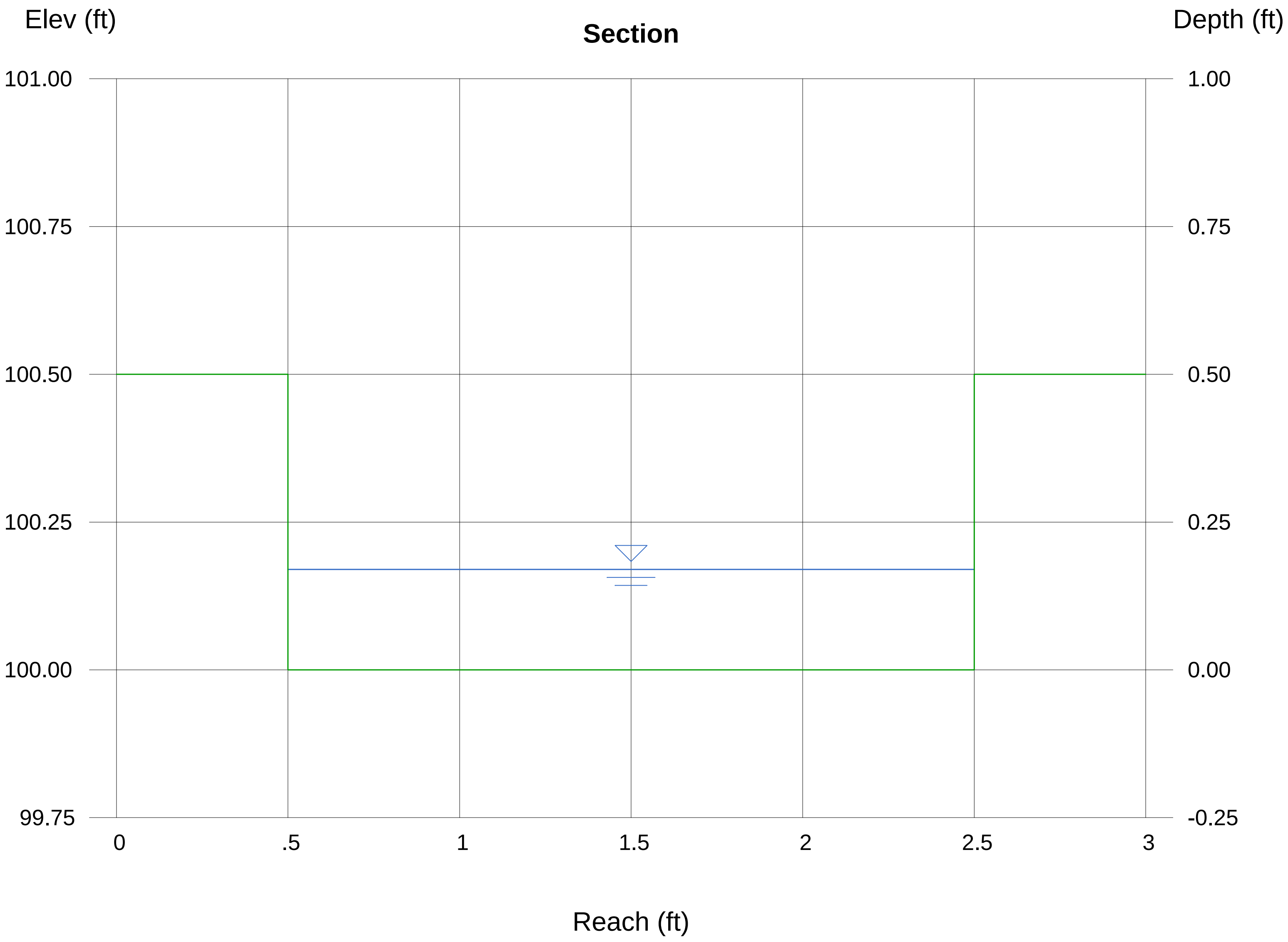
Botom Width (ft)	= 2.00
Total Depth (ft)	= 0.50
Invert Elev (ft)	= 100.00
Slope (%)	= 1.00
N-Value	= 0.013

Calculations

Compute by:	Known Q
Known Q (cfs)	= 1.00

Highlighted

Depth (ft)	= 0.17
Q (cfs)	= 1.000
Area (sqft)	= 0.34
Velocity (ft/s)	= 2.94
Wetted Perim (ft)	= 2.34
Crit Depth, Yc (ft)	= 0.20
Top Width (ft)	= 2.00
EGL (ft)	= 0.30



Channel Report

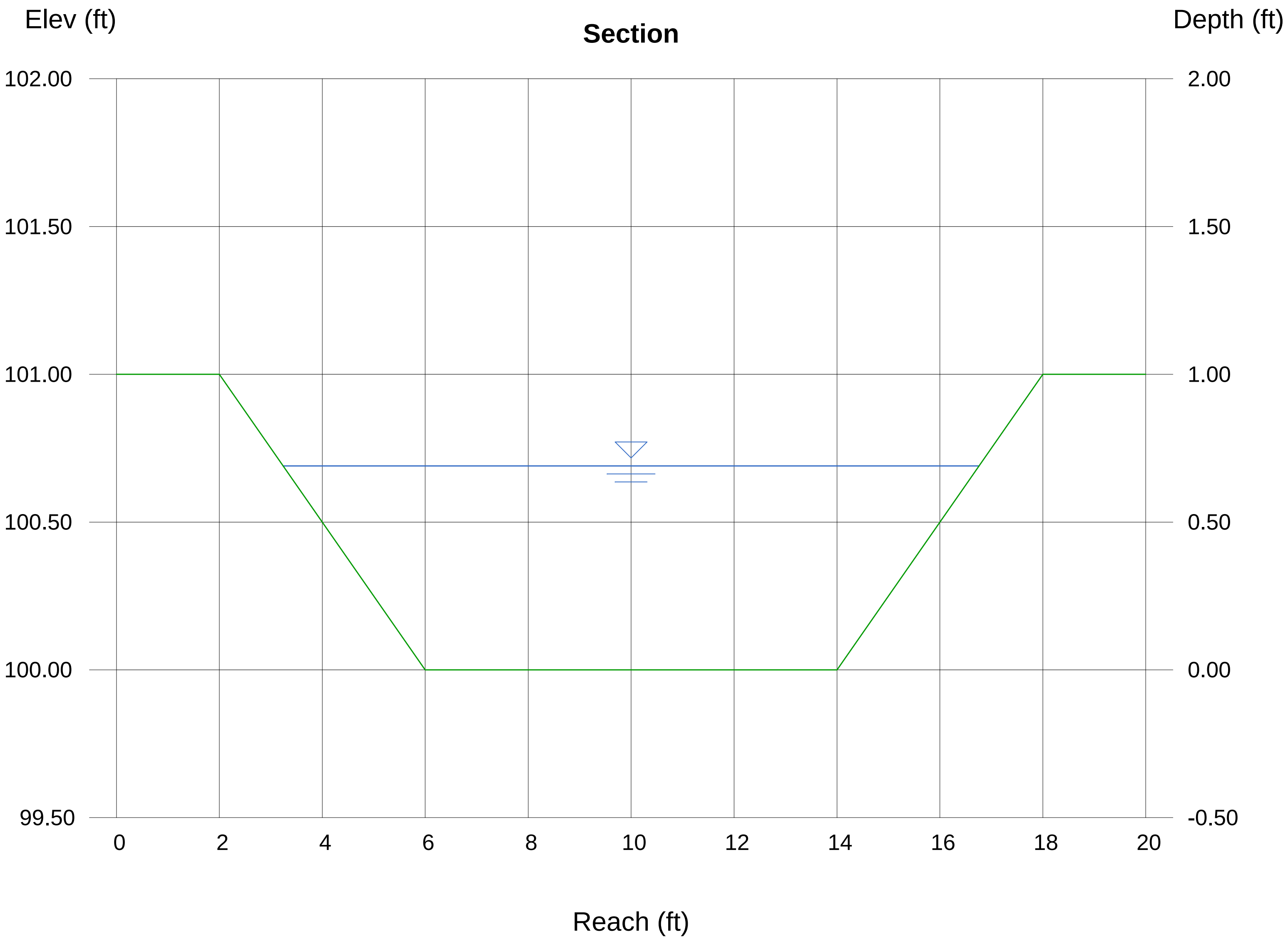
Hydraflow Express by Intelisolve

Tuesday, Dec 11 2018, 1:59 PM

Overflow from Des. Pt 4 (Alsea Dr) to Pond C1-R

Trapezoidal	
Botom Width (ft)	= 8.00
Side Slope (z:1)	= 4.00
Total Depth (ft)	= 1.00
Invert Elev (ft)	= 100.00
Slope (%)	= 1.30
N-Value	= 0.025
Calculations	
Compute by:	Known Q
Known Q (cfs)	= 33.40

Highlighted	
Depth (ft)	= 0.69
Q (cfs)	= 33.40
Area (sqft)	= 7.42
Velocity (ft/s)	= 4.50
Wetted Perim (ft)	= 13.69
Crit Depth, Yc (ft)	= 0.72
Top Width (ft)	= 13.52
EGL (ft)	= 1.00

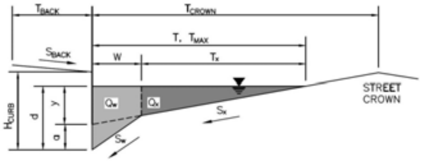


Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Creekside Filing No. 1, Lorson Ranch, El Paso County, CO** **#100.045**
Inlet ID: **Inlet #DP-1**



Gutter Geometry (Enter data in the blue cells)			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.015	
Height of Curb at Gutter Flow Line	H_{CURB} =	9.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	9.0	12.6	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

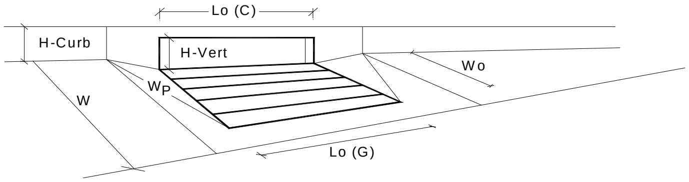
Maximum Capacity for 1/2 Street based On Allowable Spread		Minor Storm	Major Storm	
Water Depth without Gutter Depression (Eq. ST-2)	y =	3.60	4.08	inches
Vertical Depth between Gutter Lip and Gutter Flowline (usually 2")	d_C =	2.0	2.0	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	1.51	1.51	inches
Water Depth at Gutter Flowline	d =	5.11	5.59	inches
Allowable Spread for Discharge outside the Gutter Section W (T - W)	T_X =	13.0	15.0	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.397	0.350	
Discharge outside the Gutter Section W, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_T - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	31.2	46.2	ft
Theoretical Spread for Discharge outside the Gutter Section W (T - W)	$T_{X\ TH}$ =	29.2	44.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.186	0.123	
Theoretical Discharge outside the Gutter Section W, carried in Section $T_{X\ TH}$	$Q_{X\ TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section W, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	0.0	0.0	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Depth Safety Reduction Factor for Major & Minor (d $\geq$ 6") Storm	R =	SUMP	SUMP	
Max Flow Based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)	CDOT Type R Curb Opening	MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	0.00	0.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	5.9	8.0	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as a Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	Q_{Grate} =	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.31	1.31	
Clogging Factor for Multiple Units	Clog =	0.04	0.04	
Curb Opening as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	9.5	21.2	cfs
Interception with Clogging	Q_{wa} =	9.1	20.2	cfs
Curb Opening as an Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	20.8	26.8	cfs
Interception with Clogging	Q_{oa} =	19.8	25.7	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	13.1	22.2	cfs
Interception with Clogging	Q_{ma} =	12.5	21.2	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	Q_{Curb} =	**9.1**	**20.2**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	15.00	15.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	18.1	27.0	ft.>T-Crown
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.3	2.4	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.32	0.50	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.55	0.75	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.78	0.89	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	Q_a =	**9.1**	**20.2**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	Q PEAK REQUIRED =	9.1	20.2	cfs

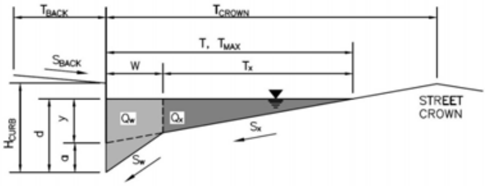
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Creekside Filing No. 1, Lorson Ranch, El Paso County, CO** **#100.045**

Inlet ID: **Inlet #DP-3**



Gutter Geometry (Enter data in the blue cells)				
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0		ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020		ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.015		
Height of Curb at Gutter Flow Line	H_{CURB} =	9.00		inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0		ft
Gutter Width	W =	2.00		ft
Street Transverse Slope	S_X =	0.020		ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083		ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000		ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016		
		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	9.0	12.6	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

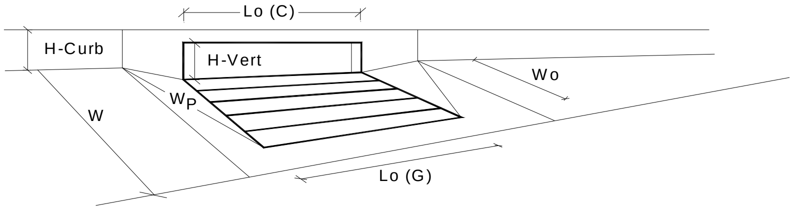
Maximum Capacity for 1/2 Street based On Allowable Spread		Minor Storm	Major Storm	
Water Depth without Gutter Depression (Eq. ST-2)	y =	3.60	4.08	inches
Vertical Depth between Gutter Lip and Gutter Flowline (usually 2")	d_C =	2.0	2.0	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	1.51	1.51	inches
Water Depth at Gutter Flowline	d =	5.11	5.59	inches
Allowable Spread for Discharge outside the Gutter Section W (T - W)	T_X =	13.0	15.0	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.397	0.350	
Discharge outside the Gutter Section W, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_T - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	Q_T =	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	31.2	46.2	ft
Theoretical Spread for Discharge outside the Gutter Section W (T - W)	$T_{X\ TH}$ =	29.2	44.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.186	0.123	
Theoretical Discharge outside the Gutter Section W, carried in Section $T_{X\ TH}$	$Q_{X\ TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section W, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	0.0	0.0	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Depth Safety Reduction Factor for Major & Minor (d $\geq$ 6") Storm	R =	SUMP	SUMP	
Max Flow Based on Allowable Depth (Safety Factor Applied)	Q_d =	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input) CDOT Type R Curb Opening ▾		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	0.00	0.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	5.2	7.1	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as a Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	Q_{Grate} =	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.31	1.31	
Clogging Factor for Multiple Units	Clog =	0.04	0.04	
Curb Opening as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	6.8	15.8	cfs
Interception with Clogging	Q_{wa} =	6.5	15.1	cfs
Curb Opening as an Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	18.5	24.5	cfs
Interception with Clogging	Q_{oa} =	17.7	23.4	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	10.4	18.3	cfs
Interception with Clogging	Q_{ma} =	10.0	17.5	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	Q_{Curb} =	**6.5**	**15.1**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	15.00	15.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	15.4	23.3	ft.>T-Crown
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	1.5	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.27	0.43	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.49	0.67	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.73	0.85	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	Q_a =	**6.5**	**15.1**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	6.0	13.3	cfs

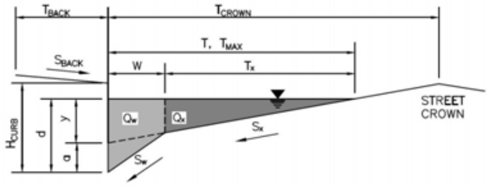
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Creekside Filing No. 1, Lorson Ranch, El Paso County, CO** **#100.045**

Inlet ID: **Inlet #DP-6**



Gutter Geometry (Enter data in the blue cells)

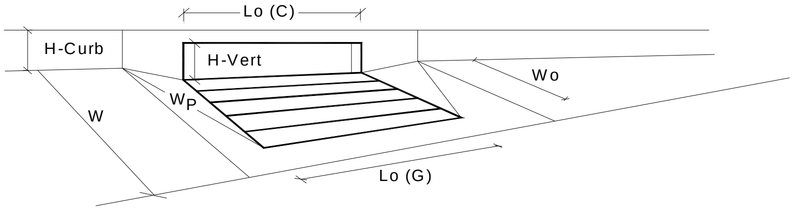
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.200	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.015	
Height of Curb at Gutter Flow Line	H_{CURB} =	9.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	9.0	12.6	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	0.00	0.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	5.1	6.9	inches
Grate Information		MINOR	MAJOR	☑ Override
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as a Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.25	1.25	
Clogging Factor for Multiple Units	Clog =	0.06	0.06	
Curb Opening as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	5.7	12.6	cfs
Interception with Clogging	Q_{wa} =	5.3	11.8	cfs
Curb Opening as an Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	12.1	16.0	cfs
Interception with Clogging	Q_{oa} =	11.3	15.0	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	7.7	13.2	cfs
Interception with Clogging	Q_{ma} =	7.2	12.4	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**5.3**	**11.8**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	10.00	10.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	15.0	22.5	ft.>T-Crown
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	1.3	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.26	0.41	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.48	0.65	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.88	0.98	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**5.3**	**11.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	5.3	11.8	cfs

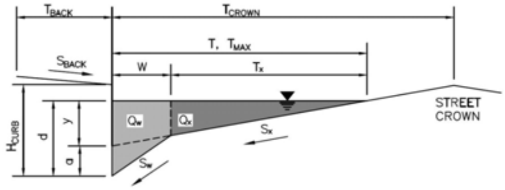
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Creekside Filing No. 1, Lorson Ranch, El Paso County, CO** **#100.045**

Inlet ID: **Inlet #DP-10**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.015	
Height of Curb at Gutter Flow Line	H_{CURB} =	9.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	9.0	12.6	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

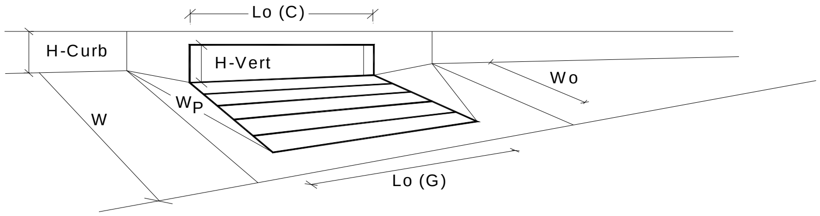
Maximum Capacity for 1/2 Street based On Allowable Spread		Minor Storm	Major Storm	
Water Depth without Gutter Depression (Eq. ST-2)	y =	3.60	4.08	inches
Vertical Depth between Gutter Lip and Gutter Flowline (usually 2")	d_C =	2.0	2.0	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	1.51	1.51	inches
Water Depth at Gutter Flowline	d =	5.11	5.59	inches
Allowable Spread for Discharge outside the Gutter Section W (T - W)	T_X =	13.0	15.0	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.397	0.350	
Discharge outside the Gutter Section W, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_T - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	31.2	46.2	ft
Theoretical Spread for Discharge outside the Gutter Section W (T - W)	$T_{X\ TH}$ =	29.2	44.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.186	0.123	
Theoretical Discharge outside the Gutter Section W, carried in Section $T_{X\ TH}$	$Q_{X\ TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section W, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	0.0	0.0	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Depth Safety Reduction Factor for Major & Minor (d $\geq$ 6") Storm	R =	SUMP	SUMP	
Max Flow Based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)		MINOR	MAJOR	
Type of Inlet CDOT Type R Curb Opening ▼	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	0.00	0.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	5.9	8.9	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as a Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	Q_{Grate} =	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.31	1.31	
Clogging Factor for Multiple Units	Clog =	0.04	0.04	
Curb Opening as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	9.7	27.2	cfs
Interception with Clogging	Q_{wa} =	9.3	26.0	cfs
Curb Opening as an Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	20.9	29.0	cfs
Interception with Clogging	Q_{oa} =	20.0	27.8	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	13.2	26.1	cfs
Interception with Clogging	Q_{ma} =	12.7	25.0	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	Q_{Curb} =	**9.3**	**25.0**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	15.00	15.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	18.3	30.8	ft.>T-Crown
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.3	3.3	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.33	0.58	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.56	0.84	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.78	0.93	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	Q_a =	**9.3**	**25.0**	**cfs**
WARNING: Inlet Capacity less than Q Peak for Minor Storm	$Q_{PEAK\ REQUIRED}$ =	9.7	21.5	cfs

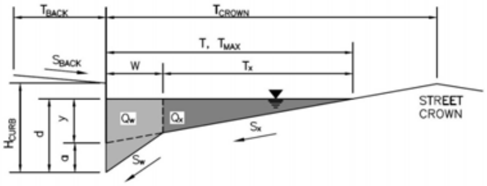
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Creekside Filing No. 1, Lorson Ranch, El Paso County, CO** **#100.045**

Inlet ID: **Inlet #DP-11**



Gutter Geometry (Enter data in the blue cells)

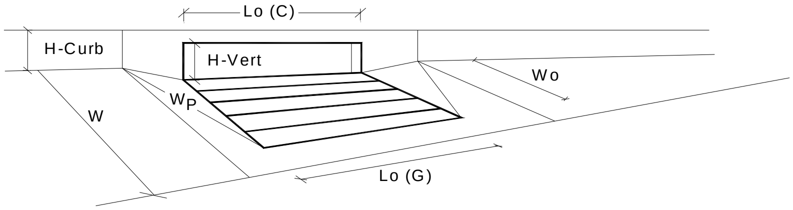
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.015	
Height of Curb at Gutter Flow Line	H_{CURB} =	9.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	9.0	12.6	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



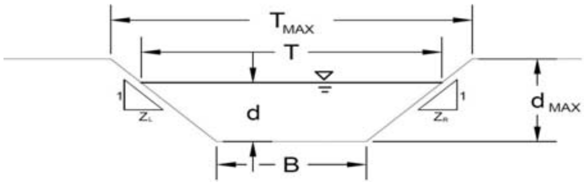
Design Information (Input) CDOT Type R Curb Op		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	0.00	0.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.4	6.2	inches
Grate Information		MINOR	MAJOR	☑ Override
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as a Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.31	1.31	
Clogging Factor for Multiple Units	Clog =	0.04	0.04	
Curb Opening as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	4.0	11.1	cfs
Interception with Clogging	Q_{wa} =	3.9	10.6	cfs
Curb Opening as an Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	15.3	21.8	cfs
Interception with Clogging	Q_{oa} =	14.6	20.9	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	7.3	14.5	cfs
Interception with Clogging	Q_{ma} =	7.0	13.9	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**3.9**	**10.6**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	15.00	15.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	12.0	19.5	ft.>T-Crown
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	0.6	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.20	0.35	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.42	0.58	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.67	0.80	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**3.9**	**10.6**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	3.7	8.3	cfs

Version 4.05 Released March 2017

AREA INLET IN A SWALE

Creekside Filing No. 1, Lorson Ranch, El Paso County, CO **#100.045**

Inlet #DP-12 (C1.17)



This worksheet uses the NRCS vegetal retardance method to determine Manning's n.

For more information see Section 7.2.3 of the USDCM.

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

NRCS Vegetal Retardance (A, B, C, D, or E)	A, B, C, D or E	C	
Manning's n (Leave cell D16 blank to manually enter an n value)	n =	see details below	
Channel Invert Slope	S_O =	0.0133	ft/ft
Bottom Width	B =	0.00	ft
Left Side Slope	Z1 =	30.00	ft/ft
Right Side Slope	Z2 =	30.00	ft/ft

Check one of the following soil types:

Soil Type:	Max. Velocity (V_{MAX})	Max Froude No. (F_{MAX})
Non-Cohesive	5.0 fps	0.60
Cohesive	7.0 fps	0.80
Paved	N/A	N/A

Choose One:
● **Non-Cohesive**
○ **Cohesive**
○ **Paved**

		Minor Storm	Major Storm	
Max. Allowable Top Width of Channel for Minor & Major Storm	T_{MAX} =	**60.00**	**60.00**	**feet**
Max. Allowable Water Depth in Channel for Minor & Major Storm	d_{MAX} =	**0.80**	**1.00**	**feet**

Maximum Channel Capacity Based On Allowable Top Width		Minor Storm	Major Storm	
Max. Allowable Top Width	T_{MAX} =	**60.00**	**60.00**	**ft**
Water Depth	d =	1.00	1.00	ft
Flow Area	A =	30.00	30.00	sq ft
Wetted Perimeter	P =	60.03	60.03	ft
Hydraulic Radius	R =	0.50	0.50	ft
Manning's n based on NRCS Vegetal Retardance	n =	0.215	0.215	
Flow Velocity	V =	0.50	0.50	fps
Velocity-Depth Product	VR =	0.25	0.25	ft^2/s
Hydraulic Depth	D =	0.50	0.50	ft
Froude Number	Fr =	0.13	0.13	
Max. Flow Based On Allowable Top Width	Q_T =	**15.1**	**15.1**	**cfs**

Maximum Channel Capacity Based On Allowable Water Depth		Minor Storm	Major Storm	
Max. Allowable Water Depth	d_{MAX} =	**0.80**	**1.00**	**feet**
Top Width	T =	48.00	60.00	feet
Flow Area	A =	19.20	30.00	square feet
Wetted Perimeter	P =	48.03	60.03	feet
Hydraulic Radius	R =	0.40	0.50	feet
Manning's n based on NRCS Vegetal Retardance	n =	0.430	0.215	
Flow Velocity	V =	0.22	0.50	fps
Velocity-Depth Product	VR =	0.09	0.25	ft^2/s
Hydraulic Depth	D =	0.40	0.50	feet
Froude Number	Fr =	0.06	0.13	
Max. Flow Based On Allowable Water Depth	Q_d =	**4.2**	**15.1**	**cfs**

Allowable Channel Capacity Based On Channel Geometry		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**4.2**	**15.1**	**cfs**
MAJOR STORM Allowable Capacity is based on Depth Criterion	d_{allow} =	**0.80**	**1.00**	**ft**

Water Depth in Channel Based On Design Peak Flow				
Design Peak Flow	Q_o =	**2.9**	**6.3**	**cfs**
Water Depth	**d =**	**0.70**	**0.91**	**feet**
Top Width	T =	41.91	54.69	feet
Flow Area	A =	14.64	24.92	square feet
Wetted Perimeter	P =	41.93	54.72	feet
Hydraulic Radius	R =	0.35	0.46	feet
Manning's n based on NRCS Vegetal Retardance	n =	0.430	0.402	
Flow Velocity	V =	0.20	0.25	fps
Velocity-Depth Product	VR =	0.07	0.12	ft^2/s
Hydraulic Depth	D =	0.35	0.46	feet
Froude Number	Fr =	0.06	0.07	

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Version 4.05 Released March 2017

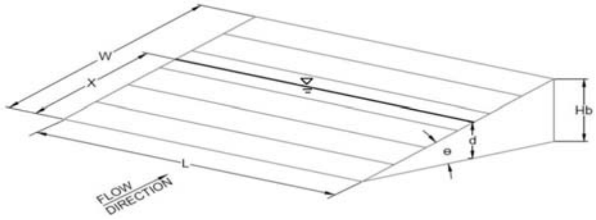
AREA INLET IN A SWALE

Creekside Filing No. 1, Lorson Ranch, El Paso County, CO **#100.045**

Inlet #DP-12 (C1.17)

Inlet Design Information (Input)

Type of Inlet CDOT Type C (Depressed) ▼ Inlet Type = CDOT Type C (Depressed)



Angle of Inclined Grate (must be <= 30 degrees)	θ =	0.00	degrees
Width of Grate	W =	3.00	feet
Length of Grate	L =	3.00	feet
Open Area Ratio	A_{RATIO} =	0.70	
Height of Inclined Grate	H_B =	0.00	feet
Clogging Factor	C_f =	0.50	
Grate Discharge Coefficient	C_d =	0.84	
Orifice Coefficient	C_o =	0.56	
Weir Coefficient	C_w =	1.81	

		MINOR	MAJOR	
Water Depth at Inlet (for depressed inlets, 1 foot is added for depression)	d =	1.70	1.91	
Grate Capacity as a Weir				
Submerged Side Weir Length	X =	3.00	3.00	feet
Inclined Side Weir Flow	Q_{ws} =	21.0	25.1	cfs
Base Weir Flow	Q_{wb} =	30.0	35.8	cfs
Interception without Clogging	Q_{wi} =	72.0	85.9	cfs
Interception with Clogging	Q_{wa} =	36.0	43.0	cfs
Grate Capacity as an Orifice				
Interception without Clogging	Q_{oi} =	37.1	39.3	cfs
Interception with Clogging	Q_{oa} =	18.5	19.7	cfs
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**18.5**	**19.7**	**cfs**
	Bypassed Flow, Q_b =	**0.0**	**0.0**	**cfs**
	Capture Percentage = Q_a/Q_o = C%	**100**	**100**	**%**

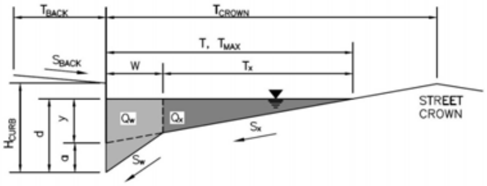
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Creekside Filing No. 1, Lorson Ranch, El Paso County, CO** **#100.045**

Inlet ID: **Inlet #DP-15**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.015	
Height of Curb at Gutter Flow Line	H_{CURB} =	9.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	9.0	12.6	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

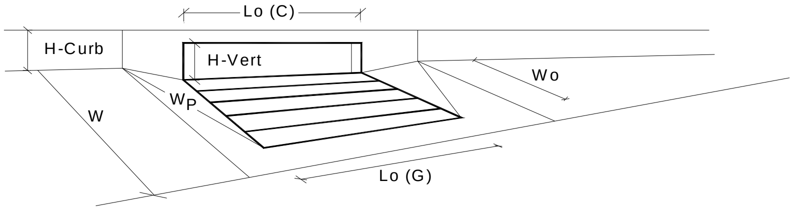
Maximum Capacity for 1/2 Street based On Allowable Spread		Minor Storm	Major Storm	
Water Depth without Gutter Depression (Eq. ST-2)	y =	3.60	4.08	inches
Vertical Depth between Gutter Lip and Gutter Flowline (usually 2")	d_C =	2.0	2.0	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	1.51	1.51	inches
Water Depth at Gutter Flowline	d =	5.11	5.59	inches
Allowable Spread for Discharge outside the Gutter Section W (T - W)	T_X =	13.0	15.0	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.397	0.350	
Discharge outside the Gutter Section W, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_T - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	31.2	46.2	ft
Theoretical Spread for Discharge outside the Gutter Section W (T - W)	$T_{X\ TH}$ =	29.2	44.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.186	0.123	
Theoretical Discharge outside the Gutter Section W, carried in Section $T_{X\ TH}$	$Q_{X\ TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section W, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	0.0	0.0	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Depth Safety Reduction Factor for Major & Minor (d $\geq$ 6") Storm	R =	SUMP	SUMP	
Max Flow Based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)	CDOT Type R Curb Opening	MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	0.00	0.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.2	5.1	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as a Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.00	1.00	
Clogging Factor for Multiple Units	Clog =	0.10	0.10	
Curb Opening as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	2.5	4.2	cfs
Interception with Clogging	Q_{wa} =	2.2	3.7	cfs
Curb Opening as an Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	4.8	6.1	cfs
Interception with Clogging	Q_{oa} =	4.3	5.5	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	3.2	4.7	cfs
Interception with Clogging	Q_{ma} =	2.9	4.2	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**2.2**	**3.7**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	5.00	5.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	11.3	15.1	ft
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	0.0	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.19	0.26	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.54	0.66	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	1.00	1.00	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**2.2**	**3.7**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	2.2	3.7	cfs

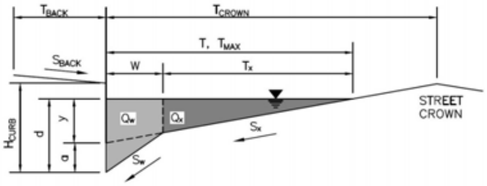
Version 4.05 Released March 2017

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Creekside Filing No. 1, Lorson Ranch, El Paso County, CO** **#100.045**

Inlet ID: **Inlet #DP-23**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	8.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.015	
Height of Curb at Gutter Flow Line	H_{CURB} =	9.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	17.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	15.0	17.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	9.0	12.6	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

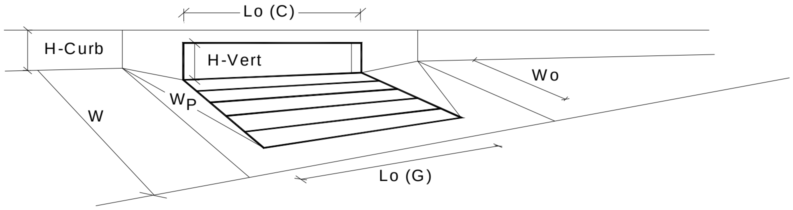
Maximum Capacity for 1/2 Street based On Allowable Spread		Minor Storm	Major Storm	
Water Depth without Gutter Depression (Eq. ST-2)	y =	3.60	4.08	inches
Vertical Depth between Gutter Lip and Gutter Flowline (usually 2")	d_C =	2.0	2.0	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	1.51	1.51	inches
Water Depth at Gutter Flowline	d =	5.11	5.59	inches
Allowable Spread for Discharge outside the Gutter Section W (T - W)	T_X =	13.0	15.0	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.397	0.350	
Discharge outside the Gutter Section W, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_T - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	Q_T =	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	31.2	46.2	ft
Theoretical Spread for Discharge outside the Gutter Section W (T - W)	$T_{X\,TH}$ =	29.2	44.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7)	E_O =	0.186	0.123	
Theoretical Discharge outside the Gutter Section W, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section W, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section W (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	0.0	0.0	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Depth Safety Reduction Factor for Major & Minor ($d \geq 6$") Storm	R =	SUMP	SUMP	
Max Flow Based on Allowable Depth (Safety Factor Applied)	Q_d =	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)	CDOT Type R Curb Opening	MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	0.00	0.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.2	8.4	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	20.00	20.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as a Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.33	1.33	
Clogging Factor for Multiple Units	Clog =	0.03	0.03	
Curb Opening as a Weir (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	14.0	29.8	cfs
Interception with Clogging	Q_{wa} =	13.5	28.8	cfs
Curb Opening as an Orifice (based on Modified HEC22 Method)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	29.0	37.0	cfs
Interception with Clogging	Q_{oa} =	28.1	35.7	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	18.7	30.8	cfs
Interception with Clogging	Q_{ma} =	18.1	29.8	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**13.5**	**28.8**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	20.00	20.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	19.5	28.5	ft.>T-Crown
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.6	2.8	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.35	0.53	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.58	0.79	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.80	0.91	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**13.5**	**28.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	Q PEAK REQUIRED =	10.8	23.1	cfs

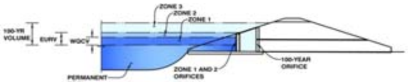
APPENDIX D – POND CALCULATIONS

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: **Creekside at Lorson Ranch**

Basin ID: **Pond C1-R**



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type = **EDB**
Watershed Area = 119.50 acres
Watershed Length = 3,000 ft
Watershed Slope = 0.009 ft/ft
Watershed Imperviousness = 55.00% percent
Percentage Hydrologic Soil Group A = 0.0% percent
Percentage Hydrologic Soil Group B = 20.0% percent
Percentage Hydrologic Soil Groups C/D = 80.0% percent
Desired WQCV Drain Time = 40.0 hours
Location for 1-hr Rainfall Depths = User Input
Water Quality Capture Volume (WQCV) = 2.195 acre-feet
Excess Urban Runoff Volume (EURV) = 6.428 acre-feet

Optional User Override 1-hr Precipitation

2-yr Runoff Volume (P1 = 1.19 in.) =	5.894 acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	8.460 acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	10.797 acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	14.428 acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	17.170 acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	20.616 acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 0 in.) =	0.000 acre-feet		inches

Approximate 2-yr Detention Volume = 5.526 acre-feet
Approximate 5-yr Detention Volume = 7.967 acre-feet
Approximate 10-yr Detention Volume = 9.326 acre-feet
Approximate 25-yr Detention Volume = 10.055 acre-feet
Approximate 50-yr Detention Volume = 10.414 acre-feet
Approximate 100-yr Detention Volume = 11.639 acre-feet

Stage-Storage Calculation

Zone 1 Volume (WQCV) = 2.195 acre-feet
Zone 2 Volume (EURV - Zone 1) = 4.233 acre-feet
Zone 3 (100yr + 1 / 2 WQCV - Zones 1 & 2) = 6.308 acre-feet
Total Detention Basin Volume = 12.736 acre-feet
Initial Surcharge Volume (ISV) = user ft^3
Initial Surcharge Depth (ISD) = user ft
Total Available Detention Depth (H_{total}) = user ft
Depth of Trickle Channel (H_{TC}) = user ft
Slope of Trickle Channel (S_{TC}) = user ft/ft
Slopes of Main Basin Sides (S_{main}) = user H:V
Basin Length-to-Width Ratio ($R_{L/W}$) = user

Initial Surcharge Area (A_{ISV}) = user ft^2
Surcharge Volume Length (L_{ISV}) = user ft
Surcharge Volume Width (W_{ISV}) = user ft
Depth of Basin Floor (H_{FLOOR}) = user ft
Length of Basin Floor (L_{FLOOR}) = user ft
Width of Basin Floor (W_{FLOOR}) = user ft
Area of Basin Floor (A_{FLOOR}) = user ft^2
Volume of Basin Floor (V_{FLOOR}) = user ft^3
Depth of Main Basin (H_{MAIN}) = user ft
Length of Main Basin (L_{MAIN}) = user ft
Width of Main Basin (W_{MAIN}) = user ft
Area of Main Basin (A_{MAIN}) = user ft^2
Volume of Main Basin (V_{MAIN}) = user ft^3
Calculated Total Basin Volume (V_{total}) = **user** acre-feet

Depth Increment = 0.2 ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	40	0.001		
5684	--	0.20	--	--	--	50	0.001	9	0.000
5685	--	1.20	--	--	--	11,456	0.263	5,648	0.130
5686	--	2.20	--	--	--	44,890	1.031	33,935	0.779
5687	--	3.20	--	--	--	82,996	1.905	97,877	2.247
5688	--	4.20	--	--	--	91,041	2.090	184,896	4.245
5689	--	5.20	--	--	--	99,130	2.276	279,981	6.427
5690	--	6.20	--	--	--	106,283	2.440	382,688	8.785
5691	--	7.20	--	--	--	113,531	2.606	492,595	11.308
5692	--	8.20	--	--	--	120,991	2.778	609,856	14.000
5693	--	9.20	--	--	--	128,724	2.955	734,713	16.867
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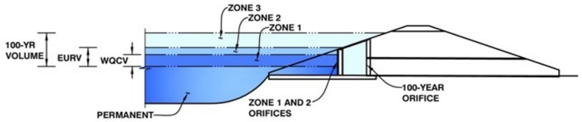
Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Creekside at Lorson Ranch
Basin ID: Pond C1-R



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.18	2.195	Orifice Plate
Zone 2 (EURV)	5.21	4.233	Rectangular Orifice
(100+1/2WQCV)	7.74	6.308	Weir&Pipe (Restrict)
		12.736	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	3.18	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	13.00	inches
Orifice Plate: Orifice Area per Row =	7.10	sq. inches (use rectangular openings)

Calculated Parameters for Plate

WQ Orifice Area per Row =	4.931E-02	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.10	2.20					
Orifice Area (sq. inches)	7.10	7.10	7.10					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Rectangular	Not Selected	
Invert of Vertical Orifice =	3.30	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	5.21	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Height =	8.00	N/A	inches
Vertical Orifice Width =	12.00		inches

Calculated Parameters for Vertical Orifice

	Zone 2 Rectangular	Not Selected	
Vertical Orifice Area =	0.67	N/A	ft^2
Vertical Orifice Centroid =	0.33	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	5.43	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	17.00	N/A	feet
Overflow Weir Slope =	7.00	N/A	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	7.00	N/A	feet
Overflow Grate Open Area % =	70%	N/A	%, grate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	6.43	N/A	feet
Over Flow Weir Slope Length =	7.07	N/A	feet
Grate Open Area / 100-yr Orifice Area =	6.06	N/A	should be $\geq$ 4
Overflow Grate Open Area w/o Debris =	84.15	N/A	ft^2
Overflow Grate Open Area w/ Debris =	42.07	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	54.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	44.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	13.88	N/A	ft^2
Outlet Orifice Centroid =	1.99	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	2.25	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	10.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	150.00	feet
Spillway End Slopes =	10.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

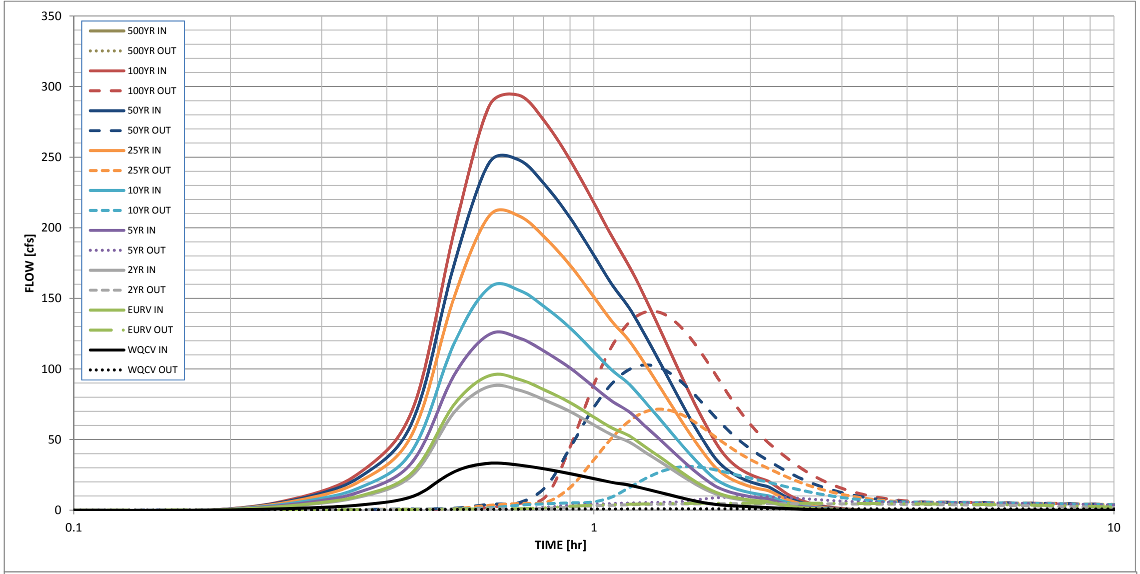
Spillway Design Flow Depth=	0.71	feet
Stage at Top of Freeboard =	11.71	feet
Basin Area at Top of Freeboard =	2.96	acres

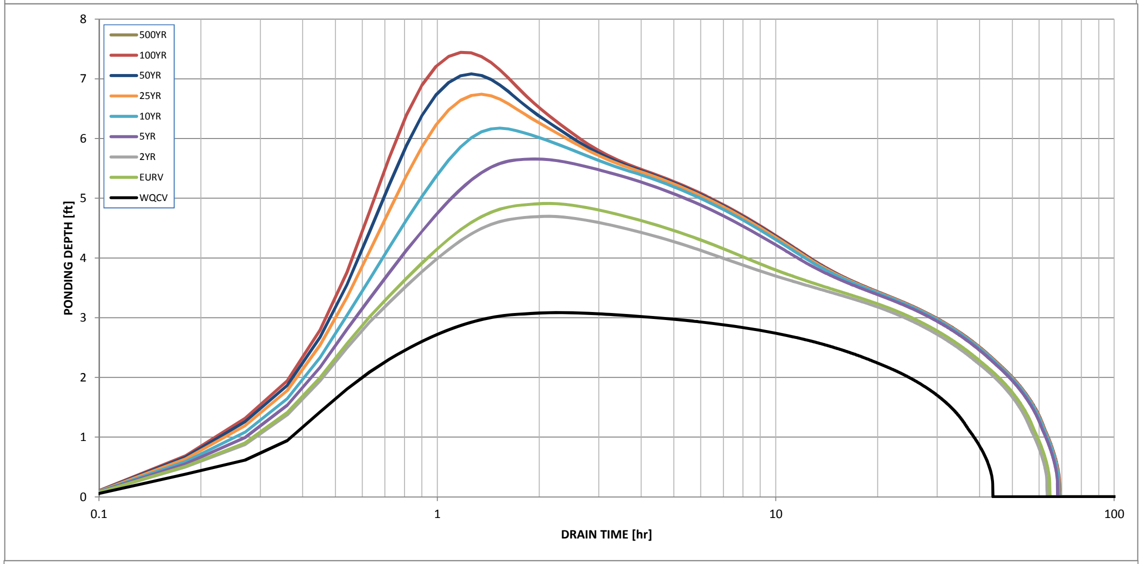
Routed Hydrograph Results

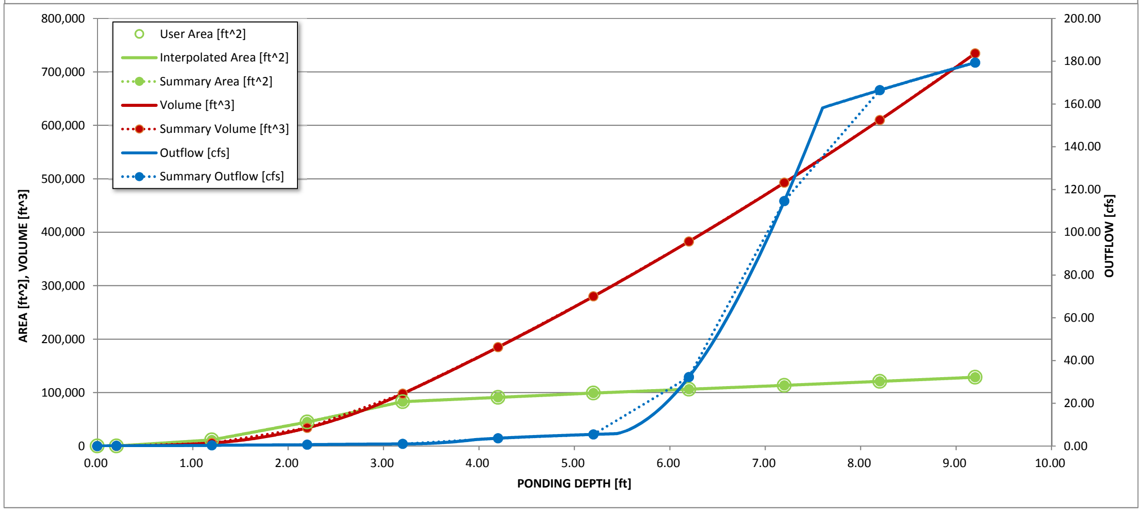
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	0.00
Calculated Runoff Volume (acre-ft) =	2.195	6.428	5.894	8.460	10.797	14.428	17.170	20.616	0.000
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	2.194	6.418	5.888	8.452	10.787	14.419	17.153	20.601	#N/A
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.10	0.29	0.71	0.95	1.25	0.00
Predevelopment Peak Q (cfs) =	0.0	0.0	1.6	11.4	34.4	85.2	113.5	149.1	0.0
Peak Inflow Q (cfs) =	33.2	95.3	87.7	124.6	157.9	208.7	247.8	293.7	#N/A
Peak Outflow Q (cfs) =	1.0	5.0	4.7	9.6	31.0	71.5	102.9	140.5	#N/A
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.8	0.9	0.8	0.9	0.9	#N/A
Structure Controlling Flow =	Plate	Vertical Orifice 1	Vertical Orifice 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	#N/A
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	0.0	0.3	0.8	1.1	1.6	#N/A
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	#N/A
Time to Drain 97% of Inflow Volume (hours) =	40	55	55	57	56	53	52	49	#N/A
Time to Drain 99% of Inflow Volume (hours) =	**42**	61	60	63	62	61	60	59	#N/A
Maximum Ponding Depth (ft) =	3.09	4.91	4.70	5.66	6.18	6.74	7.08	7.44	#N/A
Area at Maximum Ponding Depth (acres) =	1.80	2.22	2.18	2.35	2.43	2.53	2.59	2.65	#N/A
Maximum Volume Stored (acre-ft) =	2.025	5.775	5.291	7.468	8.712	10.127	10.997	11.939	#N/A

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	
5.40 min	
Hydrograph Constant	
0.925	

SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	#N/A
TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
0:05:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
0:10:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
0:16:12	1.42	3.77	3.51	4.72	5.71	7.07	7.97	8.77	#N/A
0:21:36	3.90	10.72	9.92	13.71	16.96	21.70	25.06	28.31	#N/A
0:27:00	10.01	27.54	25.47	35.22	43.59	55.82	64.52	73.20	#N/A
0:32:24	27.46	75.40	69.76	96.32	119.06	152.20	175.71	199.05	#N/A
0:37:48	33.23	95.34	87.66	124.62	157.85	208.71	246.42	286.74	#N/A
0:43:12	31.84	92.51	84.89	121.84	155.60	208.17	247.80	293.70	#N/A
0:48:36	28.97	84.56	77.53	111.69	143.04	192.11	229.31	273.83	#N/A
0:54:00	26.03	76.24	69.89	100.76	129.11	173.50	207.14	247.97	#N/A
0:59:24	22.65	66.91	61.29	88.61	113.76	153.23	183.20	220.75	#N/A
1:04:48	19.69	58.52	53.59	77.56	99.64	134.29	160.60	194.62	#N/A
1:10:12	17.85	52.64	48.25	69.60	89.20	119.84	143.04	172.60	#N/A
1:15:36	14.89	44.36	40.61	58.91	75.82	102.42	122.66	149.01	#N/A
1:21:00	12.29	36.84	33.71	48.98	63.10	85.34	102.28	125.39	#N/A
1:26:24	9.65	29.39	26.85	39.24	50.75	68.97	82.90	102.98	#N/A
1:31:48	7.37	22.83	20.84	30.59	39.66	54.08	65.12	82.24	#N/A
1:37:12	5.43	17.18	15.66	23.12	30.08	41.22	49.82	64.37	#N/A
1:42:36	4.11	12.79	11.68	17.13	22.20	30.49	36.95	48.79	#N/A
1:48:00	3.34	10.22	9.34	13.62	17.58	23.96	28.89	37.19	#N/A
1:53:24	2.82	8.58	7.85	11.42	14.72	19.98	24.02	30.44	#N/A
1:58:48	2.46	7.44	6.81	9.89	12.72	17.22	20.66	25.98	#N/A
2:04:12	2.21	6.64	6.08	8.81	11.32	15.30	18.33	22.90	#N/A
2:09:36	2.03	6.07	5.56	8.05	10.33	13.92	16.66	20.68	#N/A
2:15:00	1.50	4.59	4.19	6.14	7.97	10.90	13.16	16.56	#N/A
2:20:24	1.09	3.31	3.03	4.43	5.74	7.85	9.50	12.04	#N/A
2:25:48	0.80	2.45	2.24	3.29	4.26	5.82	7.03	8.84	#N/A
2:31:12	0.60	1.82	1.67	2.44	3.16	4.32	5.21	6.59	#N/A
2:36:36	0.43	1.34	1.22	1.80	2.34	3.20	3.87	4.91	#N/A
2:42:00	0.31	0.97	0.88	1.30	1.69	2.32	2.81	3.62	#N/A
2:47:24	0.22	0.70	0.64	0.94	1.22	1.68	2.03	2.63	#N/A
2:52:48	0.15	0.49	0.45	0.67	0.87	1.21	1.46	1.94	#N/A
2:58:12	0.10	0.32	0.29	0.44	0.58	0.81	0.99	1.36	#N/A
3:03:36	0.05	0.19	0.17	0.26	0.35	0.49	0.61	0.88	#N/A
3:09:00	0.02	0.09	0.08	0.13	0.17	0.25	0.32	0.50	#N/A
3:14:24	0.00	0.03	0.02	0.04	0.06	0.09	0.12	0.23	#N/A
3:19:48	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.06	#N/A
3:25:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
3:30:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
3:36:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
3:41:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
3:46:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
3:52:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
3:57:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:03:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:08:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:13:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:19:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:24:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:35:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:40:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:46:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:51:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
4:57:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:02:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:07:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:13:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:18:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:24:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:29:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:34:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:40:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:45:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:51:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
5:56:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
6:01:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
6:07:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
6:12:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
6:18:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
6:23:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
6:28:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]
	0.00	40	0.001	0	0.000	0.00
	0.20	50	0.001	9	0.000	0.10
	1.20	11,342	0.260	5,648	0.130	0.33
	2.20	44,890	1.031	33,935	0.779	0.60
	3.20	82,996	1.905	97,877	2.247	1.01
	4.20	91,041	2.090	184,896	4.245	3.66
	5.20	99,130	2.276	279,981	6.427	5.45
	6.20	106,283	2.440	382,688	8.785	32.27
	7.20	113,531	2.606	492,595	11.308	114.62
	8.20	120,991	2.778	609,856	14.000	166.46
	9.20	128,724	2.955	734,713	16.867	179.37

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 3

Designer: **Richard Schindler**
Company: **Core Engineering Group**
Date: **December 11, 2018**
Project: **Creekside at Lorson Ranch Filing No. 1**
Location: **Pond CR1**

1. Basin Storage Volume

A) Effective Imperviousness of Tributary Area, I_a — I_a = 55.0 %

B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$) — i = 0.550

C) Contributing Watershed Area — Area = 119.500 ac

D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm — d_6 = in

E) Design Concept
(Select EURV when also designing for flood control)

Choose One
● Water Quality Capture Volume (WQCV)
○ Excess Urban Runoff Volume (EURV)

F) Design Volume (WQCV) Based on 40-hour Drain Time
($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area)$ — V_{DESIGN}= 2.195 ac-ft

G) For Watersheds Outside of the Denver Region,
Water Quality Capture Volume (WQCV) Design Volume
($V_{WQCV\ OTHER} = (d_6*(V_{DESIGN}/0.43))$ — $V_{DESIGN\ OTHER}$= ac-ft

H) User Input of Water Quality Capture Volume (WQCV) Design Volume
(Only if a different WQCV Design Volume is desired) — $V_{DESIGN\ USER}$= ac-ft

I) NRCS Hydrologic Soil Groups of Tributary Watershed
i) Percentage of Watershed consisting of Type A Soils — HSG_A = %
ii) Percentage of Watershed consisting of Type B Soils — HSG_B = %
iii) Percentage of Watershed consisting of Type C/D Soils — $HSG_{C/D}$ = %

J) Excess Urban Runoff Volume (EURV) Design Volume
For HSG A: $EURV_A = 1.68 * i^{1.28}$
For HSG B: $EURV_B = 1.36 * i^{1.08}$
For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$
— $EURV_{DESIGN}$ = ac-f t

K) User Input of Excess Urban Runoff Volume (EURV) Design Volume
(Only if a different EURV Design Volume is desired) — $EURV_{DESIGN\ USER}$= ac-f t

2. Basin Shape: Length to Width Ratio
(A basin length to width ratio of at least 2:1 will improve TSS reduction.) — L : W = 2.0 : 1

3. Basin Side Slopes

A) Basin Maximum Side Slopes
(Horizontal distance per unit vertical, 4:1 or flatter preferred) — Z = 4.00 ft / ft

4. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations:

5. Forebay

A) Minimum Forebay Volume
(V_{FMIN} = 3% of the WQCV) — V_{FMIN} = 0.066 ac-ft

B) Actual Forebay Volume — V_F = 0.070 ac-ft

C) Forebay Depth
(D_F = 30 inch maximum) — D_F = 30.0 in

D) Forebay Discharge

i) Undetained 100-year Peak Discharge — Q_{100} = 288.00 cfs

ii) Forebay Discharge Design Flow
($Q_F = 0.02 * Q_{100}$) — Q_F = 5.76 cfs

E) Forebay Discharge Design

Choose One
● Berm With Pipe
○ Wall with Rect. Notch
○ Wall with V-Notch Weir

ROUND UP TO NEAREST PIPE SIZE

F) Discharge Pipe Size (minimum 8-inches) — Calculated D_P = 13 in

G) Rectangular Notch Width — Calculated W_N = in

Design Procedure Form: Extended Detention Basin (EDB)

Sheet 2 of 3

Designer: **Richard Schindler**
Company: **Core Engineering Group**
Date: **December 11, 2018**
Project: **Creekside at Lorson Ranch Filing No. 1**
Location: **Pond CR1**

6. Trickle Channel

A) Type of Trickle Channel

Choose One
○ Concrete
● Soft Bottom

F) Slope of Trickle Channel

S = 0.0050 ft / ft

PROVIDE A CONSISTENT LONGITUDINAL SLOPE FROM FOREBAY TO MICROPOOL WITH NO MEANDERING. RIPRAP AND SOIL RIPRAP LINED CHANNELS ARE NOT RECOMMENDED.
MINIMUM DEPTH OF 1.5 FEET

7. Micropool and Outlet Structure

A) Depth of Micropool (2.5-feet minimum)

D_M = 2.5 ft

B) Surface Area of Micropool (10 ft^2 minimum)

A_M = 65 sq ft

C) Outlet Type

Choose One
● Orifice Plate
○ Other (Describe):

D) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention)

$D_{orifice}$ = 2.60 inches

E) Total Outlet Area

A_{ot} = 20.34 square inches

8. Initial Surcharge Volume

A) Depth of Initial Surcharge Volume (Minimum recommended depth is 4 inches)

D_{IS} = 4 in

B) Minimum Initial Surcharge Volume (Minimum volume of 0.3% of the WQCV)

V_{IS} = 287 cu ft

C) Initial Surcharge Provided Above Micropool

V_s= 21.7 cu ft

9. Trash Rack

A) Water Quality Screen Open Area: $A_t = A_{ot} * 38.5*(e^{-0.095D})$

A_t = 612 square inches

B) Type of Screen (If specifying an alternative to the materials recommended in the USDCM, indicate "other" and enter the ratio of the total open are to the total screen are for the material specified.)

Other (Please describe below)

wellscreen stainless

Other (Y/N): y

C) Ratio of Total Open Area to Total Area (only for type 'Other')

User Ratio = 0.6

D) Total Water Quality Screen Area (based on screen type)

A_{total} = 1020 sq. in. **Based on type 'Other' screen ratio**

E) Depth of Design Volume (EURV or WQCV) (Based on design concept chosen under 1E)

H= 3.16 feet

F) Height of Water Quality Screen (H_{TR})

H_{TR}= 65.92 inches

G) Width of Water Quality Screen Opening ($W_{opening}$) (Minimum of 12 inches is recommended)

$W_{opening}$ = 15.5 inches

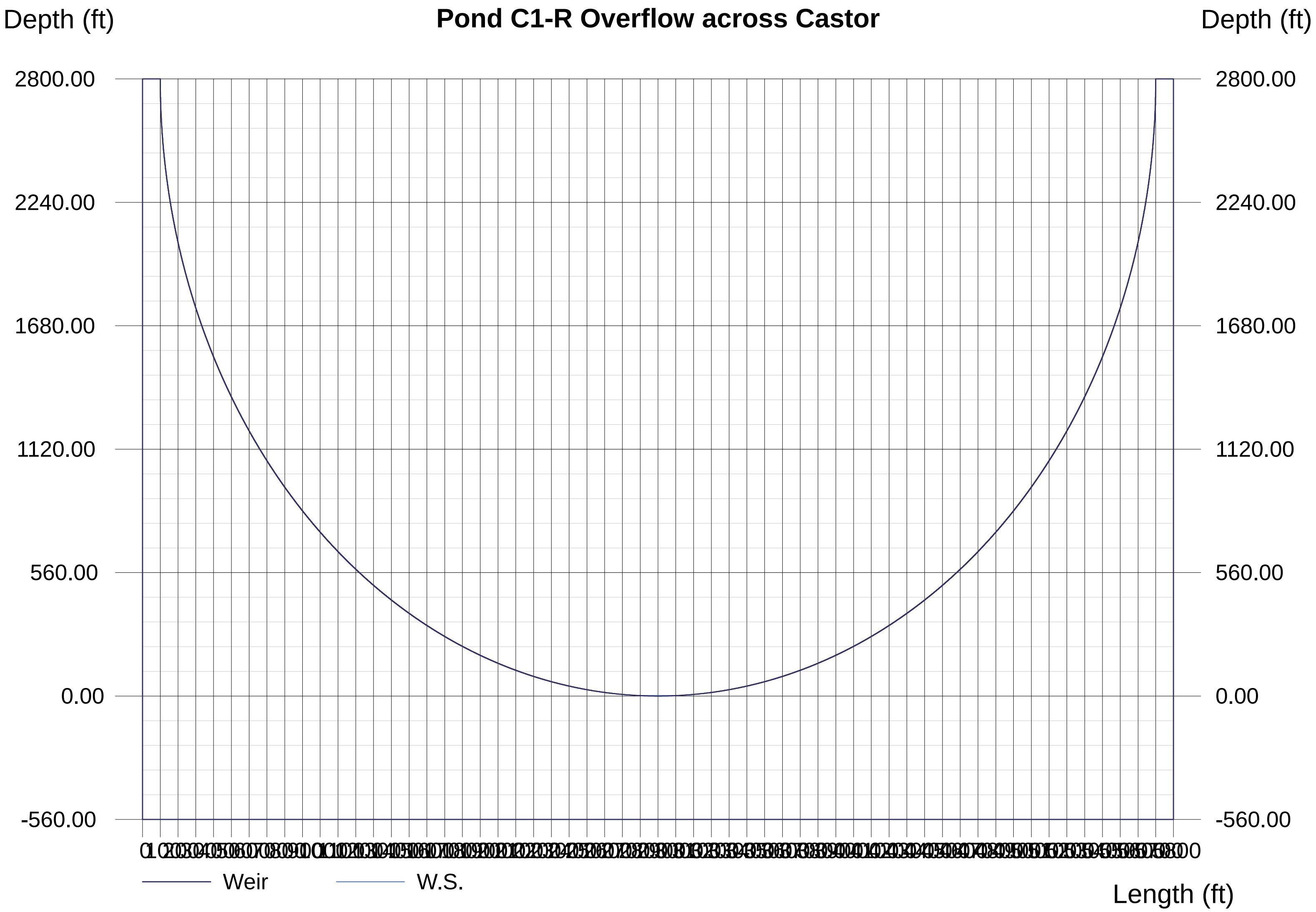
Weir Report

Pond C1-R Overflow across Castor

Circular Weir
Crest = Sharp
Diameter (ft) = 5600.00
Total Depth (ft) = 2800.00

Calculations
Weir Coeff. Cw = 3.33
Compute by: Known Q
Known Q (cfs) = 294.00

Highlighted
Depth (ft) = 0.88
Q (cfs) = 294.00
Area (sqft) = 141.09
Velocity (ft/s) = 2.08
Top Width (ft) = 167.97



CORE
ENGINEERING GROUP
212 North Wahsatch Avenue, Suite 206
Colorado Springs CO 80903
Phone: 719.570.1100
Fax: 719.570.1106

Project Creekside Job No. 100.045
Subject Rp Rap Size for C1-R overflow
By RLS Date
Sheet 1 of 1

Rip Rap Embankment Protection Sizing
Per DCM Vol #1, Equation 13-9

$$D_{50} = 5.23\ S^{0.43}\left(1.35\ C_f\ q\right)^{0.56}$$

$S = 0.25$, 4:1 Slope
$C_f = 2.0$
$q = 288\ \text{cfs}/140\ \text{ft} = 2\ \text{cfs/ft}$

$$D_{50} = 5.23 \times 0.25^{0.43}\left(1.35 \times 2 \times 2\right)^{0.43}$$

$$D_{50} = 5.95''$$

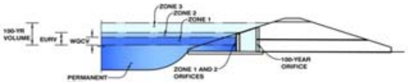
$\Longrightarrow$ We propose to use existing 12" D_{50} Rip Rap that will be removed/salvaged
The existing rip rap functioned as the overflow spillway for the existing pond.
Thickness of soil riprap = 24"

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: Creekside at Lorson Ranch

Basin ID: Pond CR2



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type = **EDB**
Watershed Area = 10.00 acres
Watershed Length = 1,000 ft
Watershed Slope = 0.013 ft/ft
Watershed Imperviousness = 52.00% percent
Percentage Hydrologic Soil Group A = 0.0% percent
Percentage Hydrologic Soil Group B = 100.0% percent
Percentage Hydrologic Soil Groups C/D = 0.0% percent
Desired WQCV Drain Time = 40.0 hours
Location for 1-hr Rainfall Depths = User Input
Water Quality Capture Volume (WQCV) = 0.176 acre-feet
Excess Urban Runoff Volume (EURV) = 0.558 acre-feet

Optional User Override 1-hr Precipitation

2-yr Runoff Volume (P1 = 1.19 in.) = 0.451 acre-feet 1.19 inches
5-yr Runoff Volume (P1 = 1.5 in.) = 0.615 acre-feet 1.50 inches
10-yr Runoff Volume (P1 = 1.75 in.) = 0.825 acre-feet 1.75 inches
25-yr Runoff Volume (P1 = 2 in.) = 1.135 acre-feet 2.00 inches
50-yr Runoff Volume (P1 = 2.25 in.) = 1.352 acre-feet 2.25 inches
100-yr Runoff Volume (P1 = 2.52 in.) = 1.636 acre-feet 2.52 inches
500-yr Runoff Volume (P1 = 0 in.) = 0.000 acre-feet inches
Approximate 2-yr Detention Volume = 0.422 acre-feet
Approximate 5-yr Detention Volume = 0.577 acre-feet
Approximate 10-yr Detention Volume = 0.760 acre-feet
Approximate 25-yr Detention Volume = 0.829 acre-feet
Approximate 50-yr Detention Volume = 0.866 acre-feet
Approximate 100-yr Detention Volume = 0.962 acre-feet

Stage-Storage Calculation

Zone 1 Volume (WQCV) = 0.176 acre-feet
Zone 2 Volume (EURV - Zone 1) = 0.381 acre-feet
Zone 3 (100yr + 1 / 2 WQCV - Zones 1 & 2) = 0.493 acre-feet
Total Detention Basin Volume = 1.051 acre-feet
Initial Surcharge Volume (ISV) = user ft^3
Initial Surcharge Depth (ISD) = user ft
Total Available Detention Depth (H_{total}) = user ft
Depth of Trickle Channel (H_{TC}) = user ft
Slope of Trickle Channel (S_{TC}) = user ft/ft
Slopes of Main Basin Sides (S_{main}) = user H:V
Basin Length-to-Width Ratio ($R_{L/W}$) = user

Initial Surcharge Area (A_{ISV}) = user ft^2
Surcharge Volume Length (L_{ISV}) = user ft
Surcharge Volume Width (W_{ISV}) = user ft
Depth of Basin Floor (H_{FLOOR}) = user ft
Length of Basin Floor (L_{FLOOR}) = user ft
Width of Basin Floor (W_{FLOOR}) = user ft
Area of Basin Floor (A_{FLOOR}) = user ft^2
Volume of Basin Floor (V_{FLOOR}) = user ft^3
Depth of Main Basin (H_{MAIN}) = user ft
Length of Main Basin (L_{MAIN}) = user ft
Width of Main Basin (W_{MAIN}) = user ft
Area of Main Basin (A_{MAIN}) = user ft^2
Volume of Main Basin (V_{MAIN}) = user ft^3
Calculated Total Basin Volume (V_{total}) = **user** acre-feet

Depth Increment = 0.2 ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	40	0.001		
5681.33	--	0.33	--	--	--	57	0.001	15	0.000
5682	--	1.00	--	--	--	500	0.011	198	0.005
5683	--	2.00	--	--	--	8,344	0.192	4,541	0.104
5684	--	3.00	--	--	--	10,785	0.248	14,189	0.326
5685	--	4.00	--	--	--	13,382	0.307	26,272	0.603
5686	--	5.00	--	--	--	16,130	0.370	41,028	0.942
5687	--	6.00	--	--	--	19,029	0.437	58,608	1.345
5688	--	7.00	--	--	--	22,079	0.507	79,162	1.817
5689	--	8.00	--	--	--	25,280	0.580	102,841	2.361
5690	--	9.00	--	--	--	28,675	0.658	129,819	2.980
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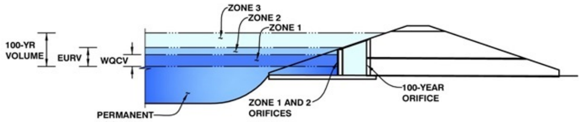
Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Creekside at Lorson Ranch

Basin ID: Pond CR2



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.35	0.176	Orifice Plate
Zone 2 (EURV)	3.85	0.381	Rectangular Orifice
(100+1/2WQCV)	5.29	0.493	Weir&Pipe (Restrict)
		1.051	**Total**

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	2.35	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	9.10	inches
Orifice Plate: Orifice Area per Row =	0.58	sq. inches (diameter = 7/8 inch)

Calculated Parameters for Plate

WQ Orifice Area per Row =	4.028E-03	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.78	1.57					
Orifice Area (sq. inches)	0.58	0.58	0.58					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Rectangular	Not Selected	
Invert of Vertical Orifice =	2.35	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	3.85	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Height =	2.10	N/A	inches
Vertical Orifice Width =	1.10		inches

Calculated Parameters for Vertical Orifice

	Zone 2 Rectangular	Not Selected	
Vertical Orifice Area =	0.02	N/A	ft^2
Vertical Orifice Centroid =	0.09	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.03	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	5.67	N/A	feet
Overflow Weir Slope =	3.00	N/A	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	3.00	N/A	feet
Overflow Grate Open Area % =	70%	N/A	%, grate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	5.03	N/A	feet
Over Flow Weir Slope Length =	3.16	N/A	feet
Grate Open Area / 100-yr Orifice Area =	12.45	N/A	should be ≥ 4
Overflow Grate Open Area w/o Debris =	12.55	N/A	ft^2
Overflow Grate Open Area w/ Debris =	6.28	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	10.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	1.01	N/A	ft^2
Outlet Orifice Centroid =	0.48	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	1.68	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	6.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	10.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	2.29	feet

Calculated Parameters for Spillway

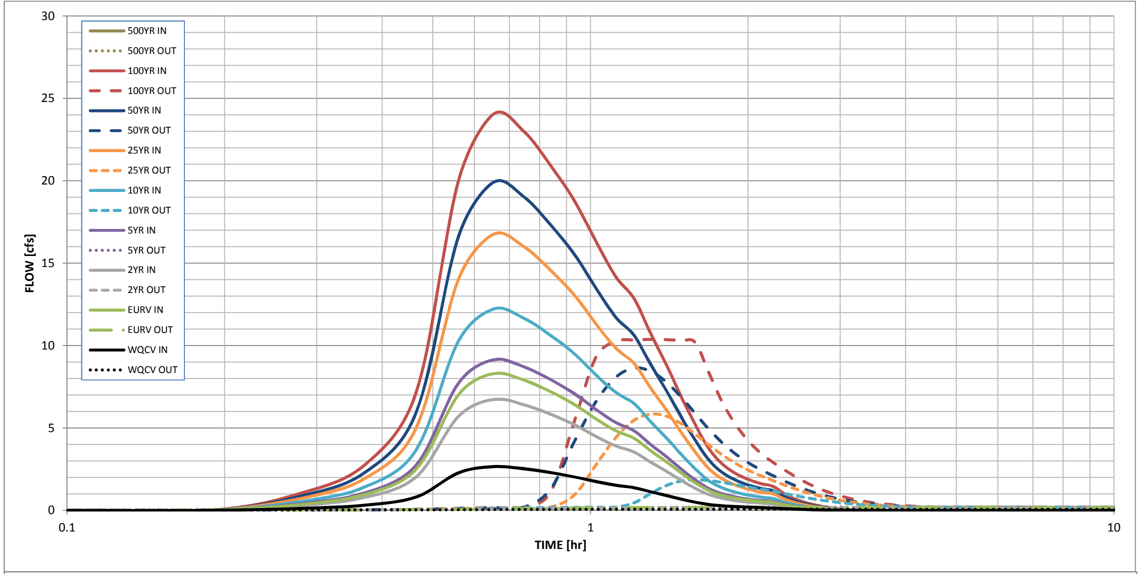
Spillway Design Flow Depth=	0.71	feet
Stage at Top of Freeboard =	9.00	feet
Basin Area at Top of Freeboard =	0.66	acres

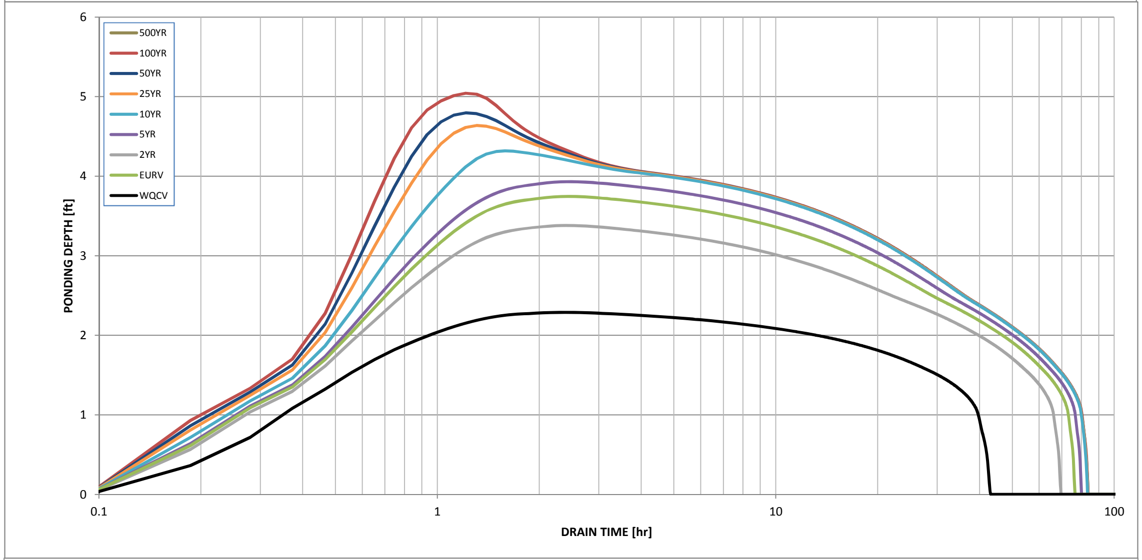
Routed Hydrograph Results

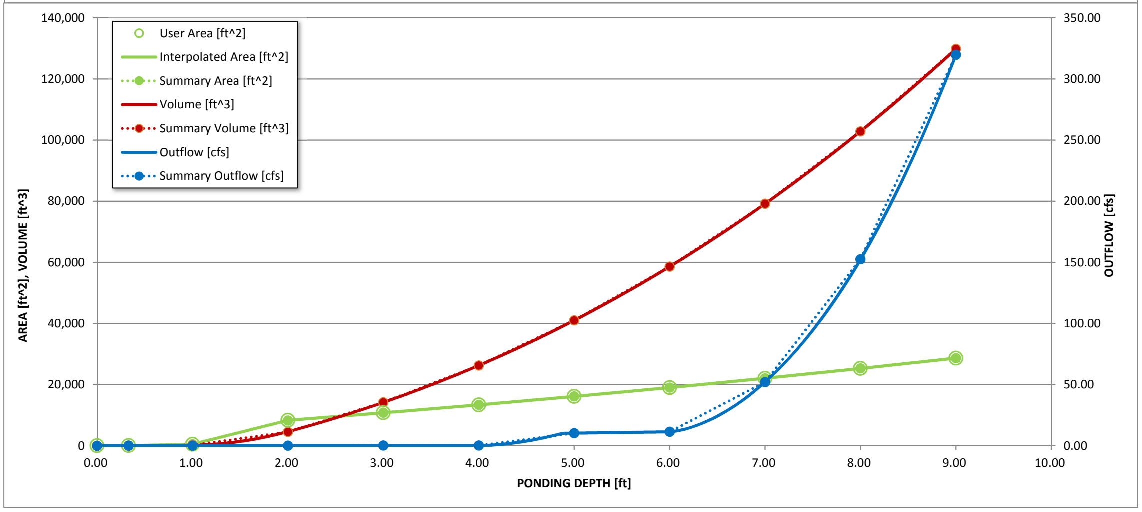
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	0.00
Calculated Runoff Volume (acre-ft) =	0.176	0.558	0.451	0.615	0.825	1.135	1.352	1.636	0.000
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.176	0.557	0.451	0.615	0.825	1.135	1.352	1.636	#N/A
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.17	0.58	0.81	1.10	0.00
Predevelopment Peak Q (cfs) =	0.0	0.0	0.1	0.2	1.7	5.8	8.1	11.0	0.0
Peak Inflow Q (cfs) =	2.7	8.3	6.7	9.1	12.2	16.8	19.9	24.0	#N/A
Peak Outflow Q (cfs) =	0.1	0.2	0.2	0.2	1.9	5.8	8.6	10.4	#N/A
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.1	1.1	1.0	1.1	0.9	#N/A
Structure Controlling Flow =	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Outlet Plate 1	#N/A
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.1	0.5	0.7	0.8	#N/A
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	#N/A
Time to Drain 97% of Inflow Volume (hours) =	39	69	63	72	73	70	69	66	#N/A
Time to Drain 99% of Inflow Volume (hours) =	**41**	73	66	76	79	78	77	76	#N/A
Maximum Ponding Depth (ft) =	2.29	3.75	3.38	3.93	4.32	4.64	4.80	5.04	#N/A
Area at Maximum Ponding Depth (acres) =	0.21	0.29	0.27	0.30	0.33	0.35	0.36	0.37	#N/A
Maximum Volume Stored (acre-ft) =	0.162	0.525	0.424	0.582	0.701	0.809	0.866	0.957	#N/A

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	#N/A
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.59 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	0:05:35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
Hydrograph Constant	0:11:11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	0:16:46	0.12	0.37	0.30	0.41	0.54	0.73	0.87	1.04	#N/A
0.895	0:22:22	0.32	0.99	0.81	1.09	1.45	1.98	2.35	2.83	#N/A
	0:27:57	0.83	2.55	2.07	2.80	3.73	5.10	6.04	7.27	#N/A
	0:33:32	2.27	7.00	5.69	7.70	10.26	14.00	16.59	19.97	#N/A
	0:39:08	2.66	8.29	6.73	9.13	12.22	16.75	19.91	24.04	#N/A
	0:44:43	2.52	7.91	6.41	8.72	11.67	16.01	19.04	23.01	#N/A
	0:50:19	2.29	7.20	5.84	7.93	10.62	14.58	17.33	20.94	#N/A
	0:55:54	2.04	6.43	5.20	7.08	9.50	13.06	15.53	18.79	#N/A
	1:01:29	1.74	5.54	4.48	6.11	8.21	11.31	13.47	16.31	#N/A
	1:07:05	1.52	4.83	3.91	5.33	7.15	9.84	11.71	14.17	#N/A
	1:12:40	1.38	4.38	3.54	4.83	6.48	8.92	10.62	12.85	#N/A
	1:18:16	1.12	3.61	2.91	3.98	5.36	7.40	8.83	10.70	#N/A
	1:23:51	0.90	2.94	2.37	3.25	4.39	6.07	7.26	8.81	#N/A
	1:29:26	0.68	2.26	1.81	2.50	3.39	4.72	5.65	6.89	#N/A
	1:35:02	0.49	1.68	1.34	1.86	2.54	3.55	4.27	5.23	#N/A
	1:40:37	0.36	1.22	0.98	1.35	1.83	2.58	3.12	3.83	#N/A
	1:46:13	0.29	0.94	0.76	1.05	1.42	1.98	2.38	2.92	#N/A
	1:51:48	0.24	0.78	0.62	0.86	1.16	1.62	1.95	2.38	#N/A
	1:57:23	0.20	0.66	0.53	0.73	0.99	1.38	1.65	2.01	#N/A
	2:02:59	0.18	0.58	0.47	0.64	0.87	1.20	1.44	1.76	#N/A
	2:08:34	0.16	0.52	0.42	0.58	0.78	1.08	1.30	1.58	#N/A
	2:14:10	0.15	0.48	0.39	0.53	0.72	1.00	1.19	1.45	#N/A
	2:19:45	0.11	0.35	0.29	0.39	0.53	0.73	0.88	1.07	#N/A
	2:25:20	0.08	0.26	0.21	0.29	0.39	0.54	0.64	0.78	#N/A
	2:30:56	0.06	0.19	0.15	0.21	0.28	0.39	0.47	0.57	#N/A
	2:36:31	0.04	0.14	0.11	0.15	0.21	0.29	0.35	0.42	#N/A
	2:42:07	0.03	0.10	0.08	0.11	0.15	0.21	0.25	0.31	#N/A
	2:47:42	0.02	0.07	0.06	0.08	0.11	0.15	0.18	0.22	#N/A
	2:53:17	0.01	0.05	0.04	0.06	0.08	0.11	0.13	0.16	#N/A
	2:58:53	0.01	0.03	0.03	0.04	0.05	0.07	0.09	0.11	#N/A
	3:04:28	0.01	0.02	0.02	0.02	0.03	0.04	0.05	0.07	#N/A
	3:10:04	0.00	0.01	0.01	0.01	0.01	0.02	0.03	0.04	#N/A
	3:15:39	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	#N/A
	3:21:14	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:26:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:32:25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:38:01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:43:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:49:11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:54:47	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:00:22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:05:58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:11:33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:17:08	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:22:44	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:28:19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:33:55	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:39:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:45:05	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:50:41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:56:16	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:01:52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:07:27	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:13:02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:18:38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:24:13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:29:49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:35:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:40:59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:46:35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:52:10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:57:46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	6:03:21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	6:08:56	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	6:14:32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	6:20:07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	6:25:43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	6:31:18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	6:36:53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	6:42:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]
	0.00	40	0.001	0	0.000	0.00
	0.33	56	0.001	15	0.000	0.01
	1.00	493	0.011	198	0.005	0.03
	2.00	8,266	0.190	4,541	0.104	0.06
	3.00	10,785	0.248	14,189	0.326	0.14
	4.00	13,382	0.307	26,272	0.603	0.20
	5.00	16,130	0.370	41,028	0.942	10.33
	6.00	19,029	0.437	58,608	1.345	11.41
	7.00	22,079	0.507	79,162	1.817	52.00
	8.00	25,280	0.580	102,841	2.361	152.48
	9.00	28,675	0.658	129,819	2.980	319.71

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 3

Designer: **Richard Schindler**
Company: **Core Engineering Group**
Date: **December 11, 2018**
Project: **Creekside at Lorson Ranch Filing No. 1**
Location: **Pond CR2**

1. Basin Storage Volume

A) Effective Imperviousness of Tributary Area, I_a
I_a = 52.0 %

B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$)
i = 0.520

C) Contributing Watershed Area
Area = 10.000 ac

D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm
d_6 = in

E) Design Concept
(Select EURV when also designing for flood control)

Choose One
● Water Quality Capture Volume (WQCV)
○ Excess Urban Runoff Volume (EURV)

F) Design Volume (WQCV) Based on 40-hour Drain Time
($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area)$)
V_{DESIGN}= 0.176 ac-ft

G) For Watersheds Outside of the Denver Region,
Water Quality Capture Volume (WQCV) Design Volume
($V_{WQCV\ OTHER} = (d_6*(V_{DESIGN}/0.43))$)
$V_{DESIGN\ OTHER}$= ac-ft

H) User Input of Water Quality Capture Volume (WQCV) Design Volume
(Only if a different WQCV Design Volume is desired)
$V_{DESIGN\ USER}$= ac-ft

I) NRCS Hydrologic Soil Groups of Tributary Watershed
i) Percentage of Watershed consisting of Type A Soils — HSG_A = %
ii) Percentage of Watershed consisting of Type B Soils — HSG_B = %
iii) Percentage of Watershed consisting of Type C/D Soils — $HSG_{C/D}$ = %

J) Excess Urban Runoff Volume (EURV) Design Volume
For HSG A: $EURV_A = 1.68 * i^{1.28}$
For HSG B: $EURV_B = 1.36 * i^{1.08}$
For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$
$EURV_{DESIGN}$ = ac-f t

K) User Input of Excess Urban Runoff Volume (EURV) Design Volume
(Only if a different EURV Design Volume is desired)
$EURV_{DESIGN\ USER}$= ac-f t

2. Basin Shape: Length to Width Ratio
(A basin length to width ratio of at least 2:1 will improve TSS reduction.)
L : W = 2.0 : 1

3. Basin Side Slopes

A) Basin Maximum Side Slopes
(Horizontal distance per unit vertical, 4:1 or flatter preferred)
Z = 4.00 ft / ft

4. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations:

5. Forebay

A) Minimum Forebay Volume
(V_{FMIN} = 3% of the WQCV)
V_{FMIN} = 0.005 ac-ft

B) Actual Forebay Volume
V_F = 0.005 ac-ft

C) Forebay Depth
(D_F = 18 inch maximum)
D_F = 18.0 in

D) Forebay Discharge

i) Undetained 100-year Peak Discharge
Q_{100} = 23.40 cfs

ii) Forebay Discharge Design Flow
($Q_F = 0.02 * Q_{100}$)
Q_F = 0.47 cfs

E) Forebay Discharge Design

Choose One
○ Berm With Pipe
● Wall with Rect. Notch
○ Wall with V-Notch Weir

Flow too small for berm w/ pipe

F) Discharge Pipe Size (minimum 8-inches)
Calculated D_P = in

G) Rectangular Notch Width
Calculated W_N = 4.5 in

Design Procedure Form: Extended Detention Basin (EDB)

Sheet 2 of 3

Designer: Richard Schindler
Company: Core Engineering Group
Date: December 11, 2018
Project: Creekside at Lorson Ranch Filing No. 1
Location: Pond CR2

6. Trickle Channel

A) Type of Trickle Channel

Choose One
● Concrete
○ Soft Bottom

F) Slope of Trickle Channel

$S =$ 0.0100 ft / ft

7. Micropool and Outlet Structure

A) Depth of Micropool (2.5-feet minimum)

$D_M =$ 2.5 ft

B) Surface Area of Micropool (10 ft^2 minimum)

$A_M =$ 56 sq ft

C) Outlet Type

Choose One
● Orifice Plate
○ Other (Describe):

D) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention)

$D_{orifice} =$ 0.57 inches

E) Total Outlet Area

$A_{ot} =$ 1.71 square inches

8. Initial Surcharge Volume

A) Depth of Initial Surcharge Volume
(Minimum recommended depth is 4 inches)

$D_{IS} =$ 4 in

B) Minimum Initial Surcharge Volume
(Minimum volume of 0.3% of the WQCV)

$V_{IS} =$ 23 cu ft

C) Initial Surcharge Provided Above Micropool

$V_s =$ 18.7 cu ft

9. Trash Rack

A) Water Quality Screen Open Area: $A_t = A_{ot} * 38.5*(e^{-0.095D})$

$A_t =$ 62 square inches

B) Type of Screen (If specifying an alternative to the materials recommended in the USDCM, indicate "other" and enter the ratio of the total open are to the total screen are for the material specified.)

Other (Please describe below)

wellscreen stainless

Other (Y/N): y

C) Ratio of Total Open Area to Total Area (only for type 'Other')

User Ratio = 0.6

D) Total Water Quality Screen Area (based on screen type)

$A_{total} =$ 104 sq. in. **Based on type 'Other' screen ratio**

E) Depth of Design Volume (EURV or WQCV)
(Based on design concept chosen under 1E)

$H =$ 2.23 feet

F) Height of Water Quality Screen (H_{TR})

$H_{TR} =$ 54.76 inches

G) Width of Water Quality Screen Opening ($W_{opening}$)
(Minimum of 12 inches is recommended)

$W_{opening} =$ 12.0 inches **VALUE LESS THAN RECOMMENDED MIN. WIDTH. WIDTH HAS BEEN SET TO 12 INCHES.**

Channel Report

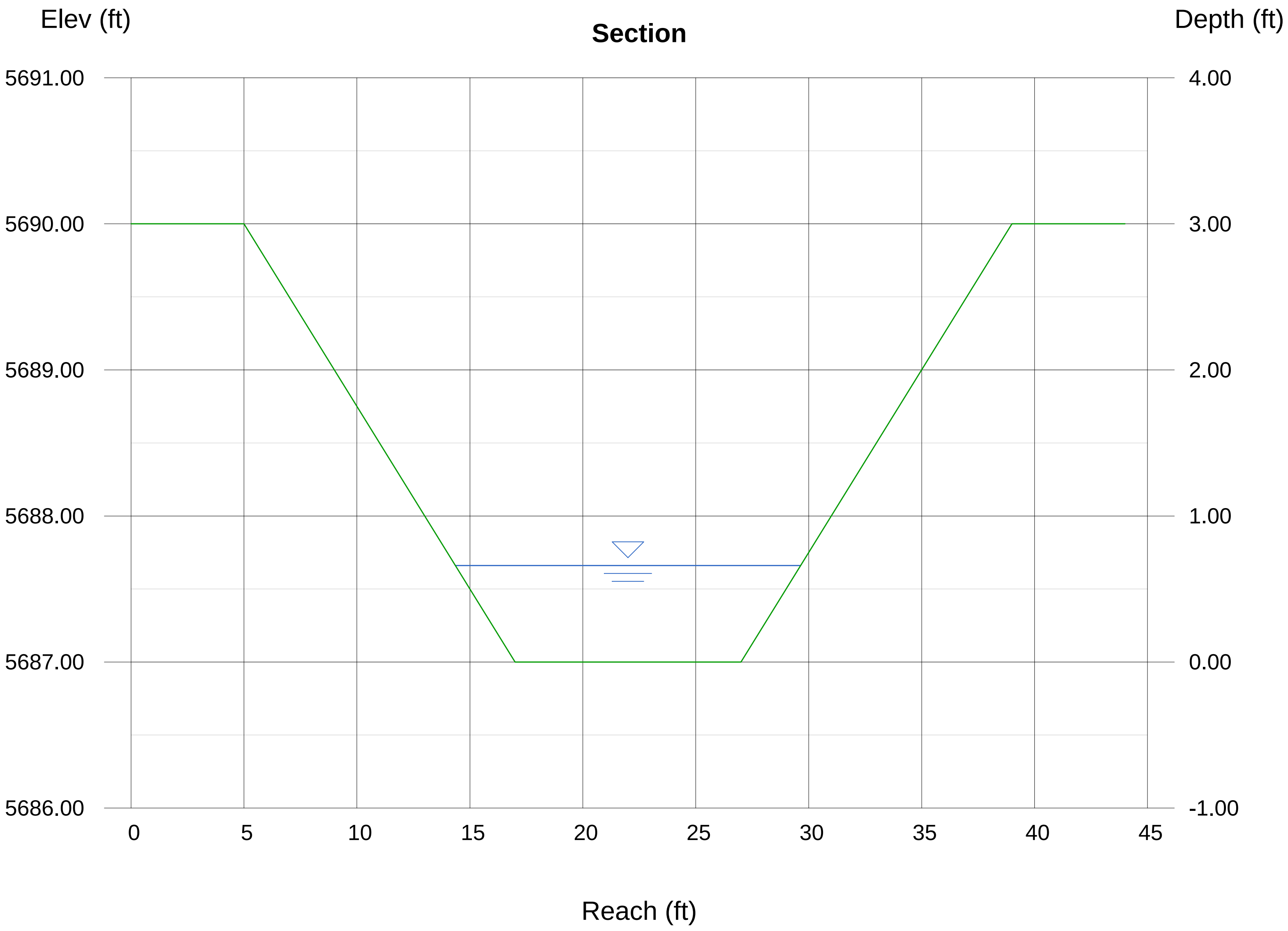
Hydraflow Express by Intelisolve

Monday, Jul 9 2018, 3:18 PM

POND CR2 OVERFLOW CHANNEL

Trapezoidal	
Botom Width (ft)	= 10.00
Side Slope (z:1)	= 4.00
Total Depth (ft)	= 3.00
Invert Elev (ft)	= 5687.00
Slope (%)	= 0.50
N-Value	= 0.025
Calculations	
Compute by:	Known Q
Known Q (cfs)	= 23.00

Highlighted	
Depth (ft)	= 0.66
Q (cfs)	= 24.00
Area (sqft)	= 8.34
Velocity (ft/s)	= 2.76
Wetted Perim (ft)	= 15.44
Crit Depth, Yc (ft)	= 0.52
Top Width (ft)	= 15.28
EGL (ft)	= 0.78

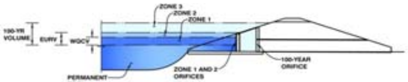


DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: **Creekside at Lorson Ranch**

Basin ID: **Pond CR3**



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	**SF**			
Watershed Area =	2.66	acres		
Watershed Length =	400	ft		
Watershed Slope =	0.025	ft/ft		
Watershed Imperviousness =	40.00%	percent		
Percentage Hydrologic Soil Group A =	0.0%	percent		
Percentage Hydrologic Soil Group B =	100.0%	percent		
Percentage Hydrologic Soil Groups C/D =	0.0%	percent		
Desired WQCV Drain Time =	12.0	hours		
Location for 1-hr Rainfall Depths =	User Input			
Water Quality Capture Volume (WQCV) =	0.032	acre-feet	Optional User Override 1-hr Precipitation	
Excess Urban Runoff Volume (EURV) =	0.112	acre-feet		
2-yr Runoff Volume (P1 = 1.19 in.) =	0.088	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	0.123	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	0.174	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	0.262	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	0.321	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	0.398	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 0 in.) =	0.000	acre-feet		inches
Approximate 2-yr Detention Volume =	0.082	acre-feet		
Approximate 5-yr Detention Volume =	0.115	acre-feet		
Approximate 10-yr Detention Volume =	0.158	acre-feet		
Approximate 25-yr Detention Volume =	0.177	acre-feet		
Approximate 50-yr Detention Volume =	0.186	acre-feet		
Approximate 100-yr Detention Volume =	0.213	acre-feet		

Stage-Storage Calculation

Zone 1 Volume (WQCV) =	0.032	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.080	acre-feet
Zone 3 (100yr + 1 / 2 WQCV - Zones 1 & 2) =	0.117	acre-feet
Total Detention Basin Volume =	0.229	acre-feet
Initial Surcharge Volume (ISV) =	N/A	ft^3
Initial Surcharge Depth (ISD) =	N/A	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	N/A	ft
Slope of Trickle Channel (S_{TC}) =	N/A	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

Depth Increment = 0.1 ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Media Surface	--	0.00	--	--	--	756	0.017		
5685	--	1.00	--	--	--	1,593	0.037	1,159	0.027
5686	--	2.00	--	--	--	2,541	0.058	3,216	0.074
5687	--	3.00	--	--	--	3,647	0.084	6,335	0.145
5688	--	4.00	--	--	--	5,041	0.116	10,679	0.245
5689	--	5.00	--	--	--	6,446	0.148	16,423	0.377
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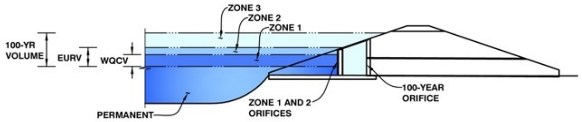
Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Creekside at Lorson Ranch
Basin ID: Pond CR3



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.13	0.032	Filtration Media
Zone 2 (EURV)	2.57	0.080	Rectangular Orifice
(100+1/2WQCV)	3.86	0.117	Weir&Pipe (Circular)
		0.229	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	1.97	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	0.69	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	0.0	ft^2
Underdrain Orifice Centroid =	0.03	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Rectangular	Not Selected	
Invert of Vertical Orifice =	1.13	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	2.57	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Height =	1.50	N/A	inches
Vertical Orifice Width =	0.70		inches

Calculated Parameters for Vertical Orifice

	Zone 2 Rectangular	Not Selected	
Vertical Orifice Area =	0.01	N/A	ft^2
Vertical Orifice Centroid =	0.06	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	3.00	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	3.00	N/A	feet
Overflow Weir Slope =	0.00	N/A	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	3.00	N/A	feet
Overflow Grate Open Area % =	70%	N/A	%, grate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	3.00	N/A	feet
Over Flow Weir Slope Length =	3.00	N/A	feet
Grate Open Area / 100-yr Orifice Area =	30.05	N/A	should be $\geq$ 4
Overflow Grate Open Area w/o Debris =	6.30	N/A	ft^2
Overflow Grate Open Area w/ Debris =	3.15	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Circular	Not Selected	
Depth to Invert of Outlet Pipe =	2.30	N/A	ft (distance below basin bottom at Stage = 0 ft)
Circular Orifice Diameter =	6.20	N/A	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Circular	Not Selected	
Outlet Orifice Area =	0.21	N/A	ft^2
Outlet Orifice Centroid =	0.26	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	N/A	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	4.50	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	6.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	0.40	feet

Calculated Parameters for Spillway

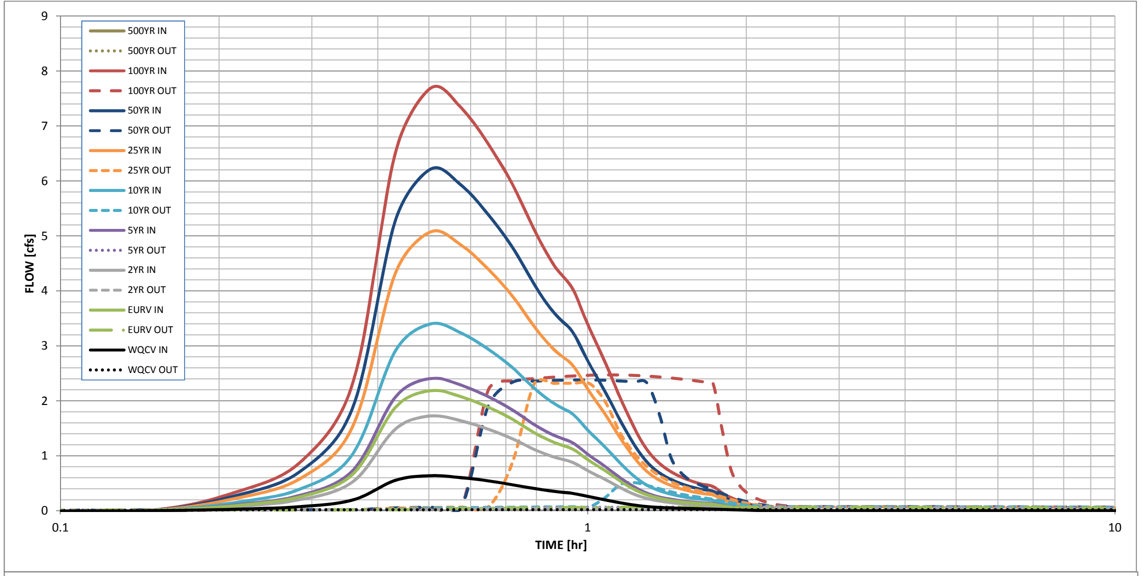
Spillway Design Flow Depth=	0.38	feet
Stage at Top of Freeboard =	5.28	feet
Basin Area at Top of Freeboard =	0.15	acres

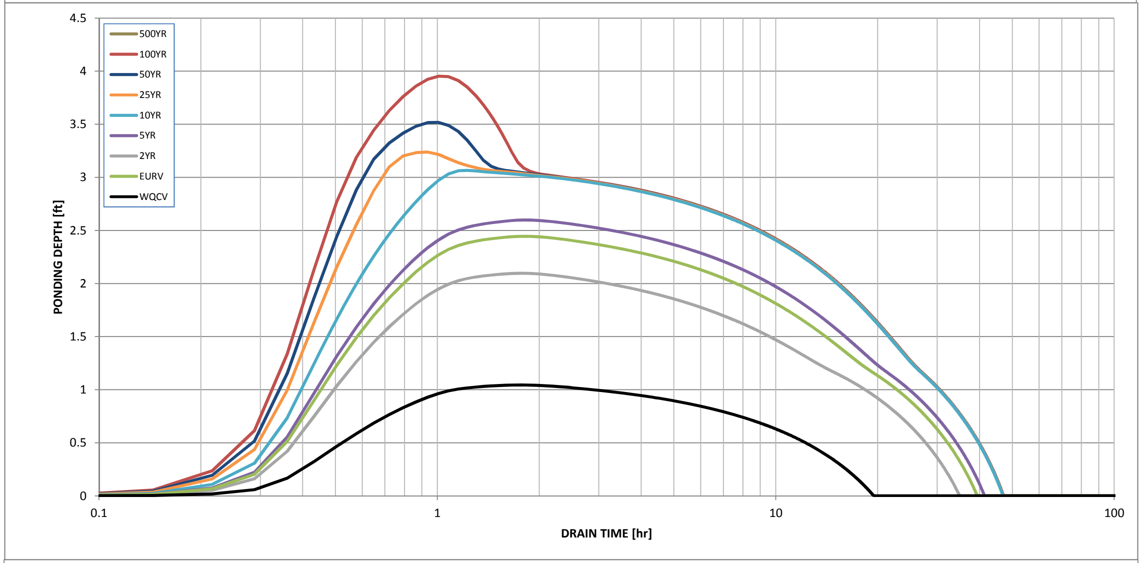
Routed Hydrograph Results

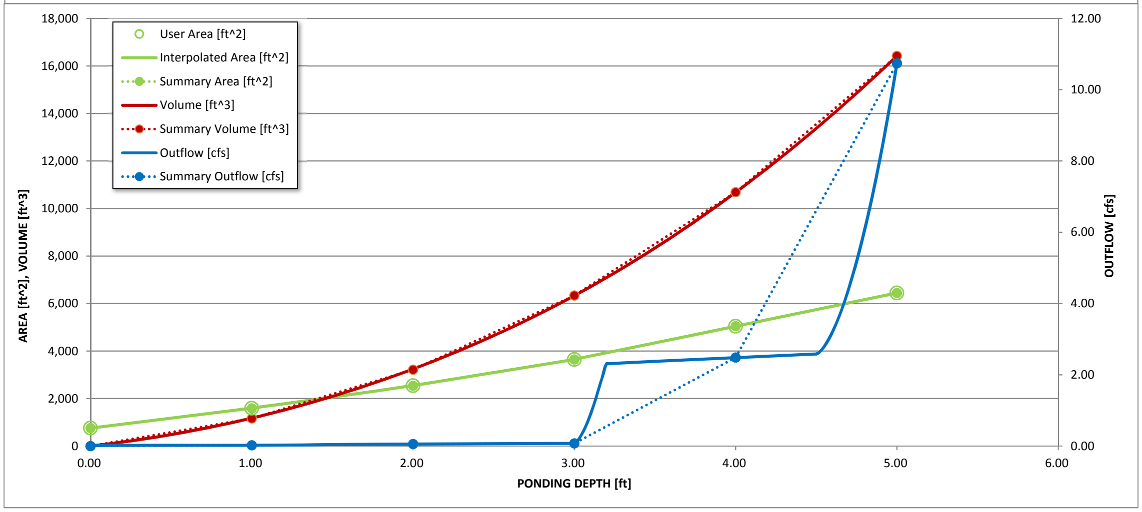
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	0.00
Calculated Runoff Volume (acre-ft) =	0.032	0.112	0.088	0.123	0.174	0.262	0.321	0.398	0.000
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.032	0.111	0.087	0.122	0.174	0.261	0.321	0.398	#N/A
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.03	0.25	0.79	1.10	1.46	0.00
Predevelopment Peak Q (cfs) =	0.00	0.00	0.04	0.067	0.7	2.1	2.9	3.9	0.0
Peak Inflow Q (cfs) =	0.6	2.2	1.7	2.4	3.4	5.1	6.2	7.7	#N/A
Peak Outflow Q (cfs) =	0.02	0.07	0.06	0.07	0.5	2.3	2.4	2.5	#N/A
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.0	0.8	1.1	0.8	0.6	#N/A
Structure Controlling Flow =	Filtration Media	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Grate 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1	#N/A
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.1	0.4	0.4	0.4	#N/A
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	#N/A
Time to Drain 97% of Inflow Volume (hours) =	19	37	33	39	43	42	41	39	#N/A
Time to Drain 99% of Inflow Volume (hours) =	**19**	39	34	41	46	45	45	44	#N/A
Maximum Ponding Depth (ft) =	1.04	2.45	2.10	2.60	3.07	3.24	3.52	3.95	#N/A
Area at Maximum Ponding Depth (acres) =	0.04	0.07	0.06	0.07	0.09	0.09	0.10	0.11	#N/A
Maximum Volume Stored (acre-ft) =	0.028	0.103	0.080	0.113	0.151	0.166	0.192	0.239	#N/A

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	#N/A
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
4.32 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	0:04:19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
Hydrograph	0:08:38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
Constant	0:12:58	0.03	0.10	0.08	0.11	0.15	0.23	0.28	0.34	#N/A
1.158	0:17:17	0.08	0.27	0.21	0.29	0.41	0.61	0.75	0.92	#N/A
	0:21:36	0.20	0.68	0.54	0.75	1.06	1.57	1.92	2.37	#N/A
	0:25:55	0.55	1.88	1.49	2.07	2.91	4.32	5.28	6.52	#N/A
	0:30:14	0.64	2.18	1.72	2.41	3.40	5.08	6.22	7.69	#N/A
	0:34:34	0.60	2.07	1.63	2.28	3.23	4.83	5.92	7.33	#N/A
	0:38:53	0.54	1.88	1.48	2.08	2.94	4.39	5.39	6.67	#N/A
	0:43:12	0.48	1.67	1.31	1.84	2.61	3.91	4.79	5.94	#N/A
	0:47:31	0.40	1.42	1.12	1.57	2.23	3.35	4.12	5.11	#N/A
	0:51:50	0.35	1.24	0.98	1.37	1.95	2.93	3.60	4.46	#N/A
	0:56:10	0.32	1.12	0.88	1.24	1.76	2.65	3.25	4.04	#N/A
	1:00:29	0.25	0.91	0.71	1.00	1.43	2.17	2.67	3.32	#N/A
	1:04:48	0.20	0.73	0.57	0.81	1.16	1.75	2.16	2.70	#N/A
	1:09:07	0.15	0.54	0.42	0.60	0.87	1.33	1.65	2.06	#N/A
	1:13:26	0.10	0.39	0.30	0.43	0.63	0.97	1.21	1.52	#N/A
	1:17:46	0.08	0.29	0.22	0.32	0.46	0.71	0.88	1.11	#N/A
	1:22:05	0.06	0.23	0.18	0.25	0.37	0.56	0.69	0.86	#N/A
	1:26:24	0.05	0.19	0.15	0.21	0.30	0.46	0.57	0.71	#N/A
	1:30:43	0.04	0.16	0.13	0.18	0.26	0.39	0.48	0.60	#N/A
	1:35:02	0.04	0.14	0.11	0.16	0.23	0.34	0.43	0.53	#N/A
	1:39:22	0.04	0.13	0.10	0.14	0.21	0.31	0.38	0.48	#N/A
	1:43:41	0.03	0.12	0.09	0.13	0.19	0.29	0.36	0.44	#N/A
	1:48:00	0.02	0.09	0.07	0.10	0.14	0.21	0.26	0.32	#N/A
	1:52:19	0.02	0.06	0.05	0.07	0.10	0.15	0.19	0.24	#N/A
	1:56:38	0.01	0.05	0.04	0.05	0.07	0.11	0.14	0.17	#N/A
	2:00:58	0.01	0.03	0.03	0.04	0.05	0.08	0.10	0.13	#N/A
	2:05:17	0.01	0.02	0.02	0.03	0.04	0.06	0.07	0.09	#N/A
	2:09:36	0.00	0.02	0.01	0.02	0.03	0.04	0.05	0.06	#N/A
	2:13:55	0.00	0.01	0.01	0.01	0.02	0.03	0.04	0.05	#N/A
	2:18:14	0.00	0.01	0.01	0.01	0.01	0.02	0.02	0.03	#N/A
	2:22:34	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.02	#N/A
	2:26:53	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01	#N/A
	2:31:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	2:35:31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	2:39:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	2:44:10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	2:48:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	2:52:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	2:57:07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:01:26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:05:46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:10:05	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:14:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:18:43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:23:02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:27:22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:31:41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:36:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:40:19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:44:38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:48:58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:53:17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	3:57:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:01:55	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:06:14	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:10:34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:14:53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:19:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:23:31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:27:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:32:10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:36:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:40:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:45:07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:49:26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:53:46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	4:58:05	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:02:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:06:43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A
	5:11:02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	#N/A

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]
	0.00	756	0.017	0	0.000	0.00
	1.00	1,585	0.036	1,159	0.027	0.02
	2.00	2,532	0.058	3,216	0.074	0.06
	3.00	3,647	0.084	6,335	0.145	0.07
	4.00	5,041	0.116	10,679	0.245	2.48
	5.00	6,446	0.148	16,423	0.377	10.74

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

Design Procedure Form: Sand Filter (SF)

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 2

Designer: **Richard Schindler**
Company: **Core Engineering**
Date: **October 16, 2018**
Project: **Creekside**
Location: **Pond CR3**

1. Basin Storage Volume

A) Effective Imperviousness of Tributary Area, I_a
(100% if all paved and roofed areas upstream of sand filter)
I_a = 40.0 %

B) Tributary Area's Imperviousness Ratio ($i = I_a/100$)
i = 0.400

C) Water Quality Capture Volume (WQCV) Based on 12-hour Drain Time
$WQCV= 0.8 * (0.91* i^3 - 1.19 * i^2 + 0.78 * i)$
WQCV = 0.14 watershed inches

D) Contributing Watershed Area (including sand filter area)
Area = 115,869 sq ft

E) Water Quality Capture Volume (WQCV) Design Volume
V_{WQCV} = WQCV / 12 * Area
V_{WQCV} = 1,389 cu ft

F) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm
d_6 = in

G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume
$V_{WQCV\ OTHER}$ = cu ft

H) User Input of Water Quality Capture Volume (WQCV) Design Volume
(Only if a different WQCV Design Volume is desired)
$V_{WQCV\ USER}$ = cu ft

2. Basin Geometry

A) WQCV Depth
D_{WQCV} = 1.13 ft

B) Sand Filter Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred). Use "0" if sand filter has vertical walls.
Z = 4.00 ft / ft

C) Minimum Filter Area (Flat Surface Area)
A_{Min} = 579 sq ft

D) Actual Filter Area
A_{Actual} = 756 sq ft

E) Volume Provided
V_T = 1393 cu ft

3. Filter Material

Choose One
● 18" CDOT Class B or C Filter Material
○ Other (Explain):

4. Underdrain System

A) Are underdrains provided?

Choose One
● YES
○ NO

B) Underdrain system orifice diameter for 12 hour drain time

i) Distance From Lowest Elevation of the Storage Volume to the Center of the Orifice
y = 1.8 ft

ii) Volume to Drain in 12 Hours
Vol_{12} = 1,389 cu ft

iii) Orifice Diameter, 3/8" Minimum
D_O = 7/8 in

Design Procedure Form: Sand Filter (SF)

Sheet 2 of 2

Designer: Richard Schindler
Company: Core Engineering
Date: October 16, 2018
Project: Creekside
Location: Pond CR3

5. Impermeable Geomembrane Liner and Geotextile Separator Fabric

A) Is an impermeable liner provided due to proximity of structures or groundwater contamination?

Choose One
○ YES ● NO

6. Inlet / Outlet Works

A) Describe the type of energy dissipation at inlet points and means of conveying flows in excess of the WQCV through the outlet

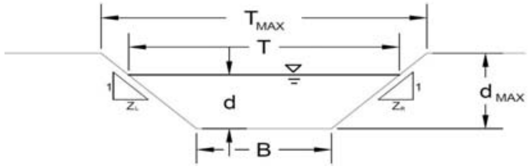
Notes:

Version 4.05 Released March 2017

AREA INLET IN A SWALE

Creekside

Pond CR3 type D Emergency Overflow Structure



This worksheet uses the NRCS vegetal retardance method to determine Manning's n.

For more information see Section 7.2.3 of the USDCM.

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

NRCS Vegetal Retardance (A, B, C, D, or E)	A, B, C, D or E	A	
Manning's n (Leave cell D16 blank to manually enter an n value)	n =	see details below	
Channel Invert Slope	S_O =	0.0050	ft/ft
Bottom Width	B =	27.00	ft
Left Side Slope	Z1 =	4.00	ft/ft
Right Side Slope	Z2 =	4.00	ft/ft

Check one of the following soil types:

Soil Type:	Max. Velocity (V_{MAX})	Max Froude No. (F_{MAX})
Non-Cohesive	5.0 fps	0.60
Cohesive	7.0 fps	0.80
Paved	N/A	N/A

Choose One:
● **Non-Cohesive**
○ **Cohesive**
○ **Paved**

		Minor Storm	Major Storm	
Max. Allowable Top Width of Channel for Minor & Major Storm	T_{MAX} =	**60.00**	**70.00**	**feet**
Max. Allowable Water Depth in Channel for Minor & Major Storm	d_{MAX} =	**0.70**	**1.00**	**feet**

Allowable Channel Capacity Based On Channel Geometry		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	**5.3**	**8.8**	**cfs**
MAJOR STORM Allowable Capacity is based on Depth Criterion	d_{allow} =	**0.70**	**1.00**	**ft**

Water Depth in Channel Based On Design Peak Flow

Design Peak Flow	Q_o =	**2.4**	**7.7**	**cfs**
Water Depth	d =	**0.41**	**0.91**	**feet**

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Version 4.05 Released March 2017

AREA INLET IN A SWALE

Creekside

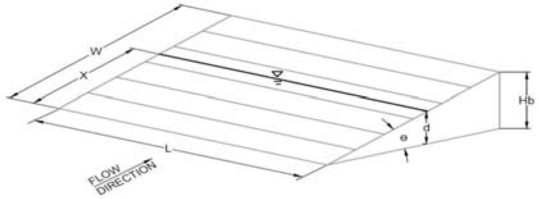
Pond CR3 type D Emergency Overflow Structure

Inlet Design Information (Input)

Type of Inlet: CDOT TYPE D (Parallel)

Inlet Type = CDOT TYPE D (Parallel)

Angle of Inclined Grate (must be <= 30 degrees)	θ =	0.00	degrees
Width of Grate	W =	6.00	feet
Length of Grate	L =	3.00	feet
Open Area Ratio	A_{RATIO} =	0.70	
Height of Inclined Grate	H_B =	0.00	feet
Clogging Factor	C_f =	0.38	
Grate Discharge Coefficient	C_d =	0.76	
Orifice Coefficient	C_o =	0.50	
Weir Coefficient	C_w =	1.62	



		MINOR	MAJOR	
Water Depth at Inlet (for depressed inlets, 1 foot is added for depression)	d =	0.41	0.91	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**6.8**	**22.3**	**cfs**
	Bypassed Flow, Q_b =	**0.0**	**0.0**	**cfs**
	Capture Percentage = Q_a/Q_o = C%	**100**	**100**	**%**

Warning 02: Depth (d) exceeds USDCM Volume I recommendation.

Calculation of Peak Runoff using Rational Method

Designer: Richard Schindler
Company: Core Engineering Group
Date: 12/11/2018
Project: Creekside at Lorson Ranch Fil No. 1
Location: Lorson Ranch

Version 2.00 released May 2017

Cells of this color are for required user-input
Cells of this color are for optional override values
Cells of this color are for calculated results based on overrides

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_i^{0.33}}$$

$$t_t = \frac{L_t}{60K\sqrt{S_t}} = \frac{L_t}{60V_t}$$

$$\text{Computed } t_c = t_i + t_t$$

$$\text{Regional } t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$

$t_{minimum}$ = 5 (urban)
$t_{minimum}$ = 10 (non-urban)

$$\text{Selected } t_c = \max\{t_{minimum}, \min(\text{Computed } t_c, \text{Regional } t_c)\}$$

Select UDFCD location for NOAA Atlas 14 Rainfall Depths from the pulldown list OR enter your own depths obtained from the NOAA website (click this link)

	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr
1-hour rainfall depth, P1 (in) =	0.83	1.09	1.33	1.69	1.99	2.31	3.14

	a	b	c
Rainfall Intensity Equation Coefficients =	28.50	10.00	0.786

$$I(in/hr) = \frac{a * P_1}{(b + t_c)^c}$$

$$Q(cfs) = CIA$$

Subcatchment Name	Area (ac)	NRCS Hydrologic Soil Group	Percent Imperviousness	Runoff Coefficient, C							Overland (Initial) Flow Time					Channelized (Travel) Flow Time							Time of Concentration			Rainfall Intensity, I (in/hr)							Peak Flow, Q (cfs)						
				2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr	Overland Flow Length L_i (ft)	U/S Elevation (ft) (Optional)	D/S Elevation (ft) (Optional)	Overland Flow Slope S_i (ft/ft)	Overland Flow Time t_i (min)	Channelized Flow Length L_t (ft)	U/S Elevation (ft) (Optional)	D/S Elevation (ft) (Optional)	Channelized Flow Slope S_t (ft/ft)	NRCS Conveyance Factor K	Channelized Flow Velocity V_t (ft/sec)	Channelized Flow Time t_t (min)	Computed t_c (min)	Regional t_c (min)	Selected t_c (min)	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr	500-yr
VQ for Grass Buffe	0.40	C	52.0	0.40	0.46	0.51	0.61	0.65	0.70	0.75	40.00			0.020	5.82	0.00			0.020	7	0.99	0.00	5.82	17.16	5.82	2.68	3.54	4.33	5.50	6.47	7.51	10.21	0.43	0.65	0.89	1.34	1.68	2.09	3.07

Design Procedure Form: Grass Buffer (GB)

UD-BMP (Version 3.06, November 2016)

Sheet 1 of 1

Designer:
Company: **Core Engineering Group**
Date: **December 11, 2018**
Project: **Creekside at Lorson Ranch East Filing No. 1**
Location: **Lorson Ranch**

1. Design Discharge

A) 2-Year Peak Flow Rate of the Area Draining to the Grass Buffer — Q_2 = 0.4 cfs

2. Minimum Width of Grass Buffer — W_G = 9 ft

3. Length of Grass Buffer (14' or greater recommended) — L_G = 45 ft

4. Buffer Slope (in the direction of flow, not to exceed 0.1 ft / ft) — S_G = 0.100 ft / ft

5. Flow Characteristics (sheet or concentrated)

A) Does runoff flow into the grass buffer across the entire width of the buffer?

Choose One
● Yes ○ No

B) Watershed Flow Length — F_L= 45 ft

C) Interface Slope (normal to flow) — S_I= 0.010 ft / ft

D) Type of Flow
Sheet Flow: $F_L * S_I \leq 1$
Concentrated Flow: $F_L * S_I > 1$

SHEET FLOW

6. Flow Distribution for Concentrated Flows

Choose One
○ None (sheet flow)
○ Slotted Curbing
○ Level Spreader
○ Other (Explain):

7 Soil Preparation
(Describe soil amendment)

4" topsoil

8 Vegetation (Check the type used or describe "Other")

Choose One
● Existing Xeric Turf Grass
○ Irrigated Turf Grass
○ Other (Explain):

9. Irrigation
(*Select None if existing buffer area has 80% vegetation AND will not be disturbed during construction.)

Choose One
○ Temporary
○ Permanent
● None*

10. Outflow Collection (Check the type used or describe "Other")

Choose One
○ Grass Swale
○ Street Gutter
○ Storm Sewer Inlet
● Other (Explain):

Etrib of Jimmy Camp Creek or Jimmy Camp Creek

Notes:

APPENDIX E– STORM SEWER SCHEMATIC AND HYDRAFLOW STORM SEWER CALCS

Hydraflow Plan View

Project File: Stm-1, Pond C1-R to DP-1, Alsea Dr, 5yr.stm	No. Lines: 2	10-17-2018

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L2 - 24" RCP	15.00	24 c	129.0	5686.90	5690.77	3.000	5688.90	5692.14	n/a	5692.14 j	End
2	L3 - 24" RCP	9.10	24 c	36.0	5691.27	5691.99	2.001	5692.67	5693.06	n/a	5693.06 j	1

Project File: Stm-1, Pond C1-R to DP-1, Alsea Dr, 5yr.stm	Number of lines: 2	Run Date: 12-11-2018

NOTES: c = cir; e = ellip; b = box; Return period = 5 Yrs. ; j - Line contains hyd. jump.

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L2 - 24" RCP	33.40	24 c	129.0	5686.90	5690.77	3.000	5688.81	5692.68	n/a	5692.68	End
2	L3 - 24" RCP	20.20	24 c	36.0	5691.27	5691.99	2.001	5693.73	5693.99	0.64	5694.63	1

Project File: Stm-1, Pond C1-R to DP-1, Alsea Dr, 100yr.stm	Number of lines: 2	Run Date: 12-11-2018

NOTES: c = cir; e = ellip; b = box; Return period = 100 Yrs.

Hydraflow Plan View

Project File: Stm-2, Pond C1-R to DP-10, Castor Dr, 5yr.stm	No. Lines: 2	10-17-2018

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L1 - 24" RCP	14.50	24 c	46.0	5684.63	5687.30	5.804	5685.98	5688.65	n/a	5688.65	End
2	L2 - 24" RCP	9.70	24 c	35.0	5687.80	5688.85	3.001	5689.14	5689.95	n/a	5689.95 j	1

Project File: Stm-2, Pond C1-R to DP-10, Castor Dr, 5yr.stm	Number of lines: 2	Run Date: 12-11-2018

NOTES: c = cir; e = ellip; b = box; Return period = 5 Yrs. ; j - Line contains hyd. jump.

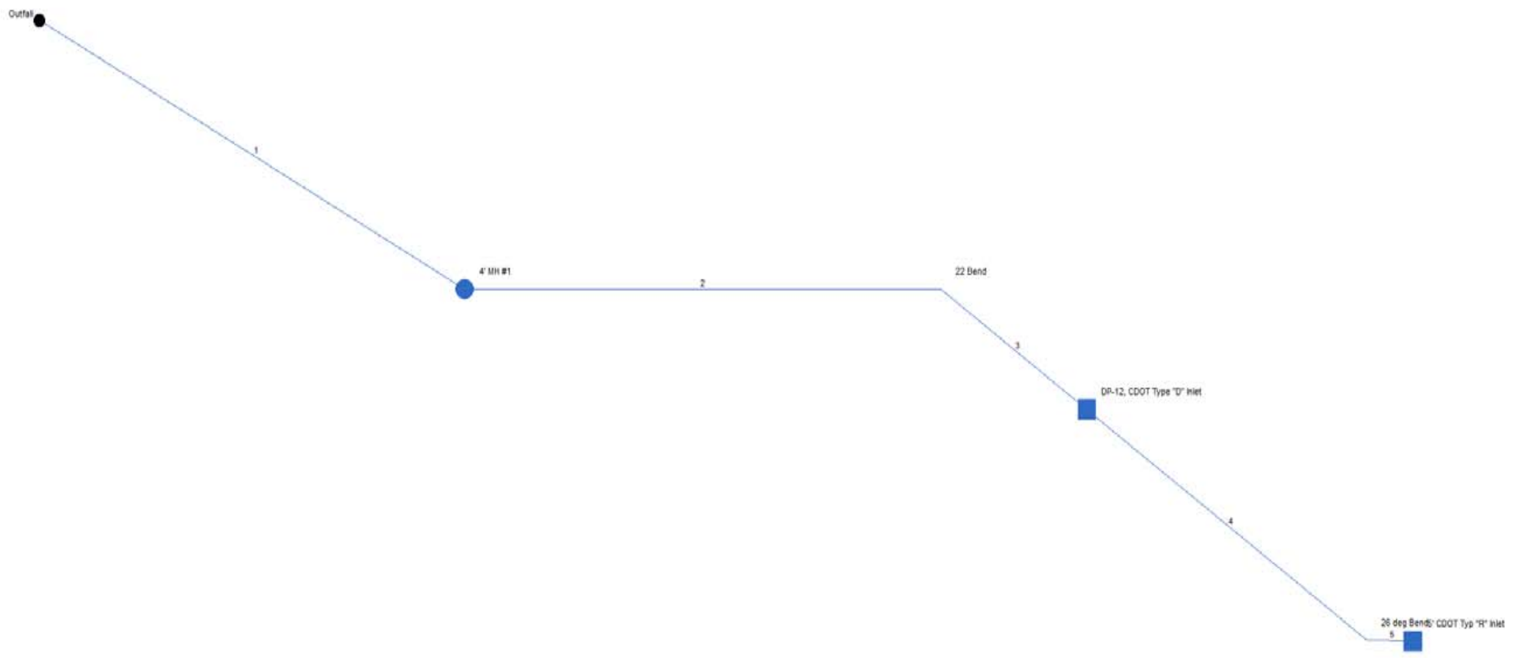
Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L1 - 24" RCP	32.10	24 c	46.0	5684.63	5687.30	5.804	5686.52	5689.19	n/a	5689.19	End
2	L2 - 24" RCP	21.50	24 c	35.0	5687.80	5688.85	3.001	5690.16	5690.49	n/a	5690.49 j	1

Project File: Stm-2, Pond C1-R to DP-10, Castor Dr, 100yr.stm	Number of lines: 2	Run Date: 12-11-2018

NOTES: c = cir; e = ellip; b = box; Return period = 100 Yrs. ; j - Line contains hyd. jump.

Hydraflow Plan View



Project File: Stm-2A, Pond C1-R to DP-11, Maidford Dr, 5yr.stm	No. Lines: 5	10-17-2018

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L1-18" RCP	6.30	24 c	223.0	5684.70	5687.82	1.399	5685.66	5688.71	n/a	5688.71 j	End
2	L2-18" RCP	6.30	24 c	216.0	5688.12	5690.28	1.000	5688.99	5691.17	0.22	5691.17	1
3	L3-18"RCP	6.30	24 c	83.0	5690.28	5691.11	1.000	5691.45	5692.00	n/a	5692.00 j	2
4	L4-18" RCP	3.70	18 c	159.0	5691.61	5693.20	1.000	5692.27	5693.94	0.18	5693.94	3
5	L5-18" RCP	3.70	18 c	21.0	5693.20	5693.41	1.000	5694.16	5694.15	n/a	5694.43 j	4

Project File: Stm-2A, Pond C1-R to DP-11, Maidford Dr, 5yr.stm

Number of lines: 5

Run Date: 12-11-2018

NOTES: c = cir; e = ellip; b = box; Return period = 5 Yrs. ; j - Line contains hyd. jump.

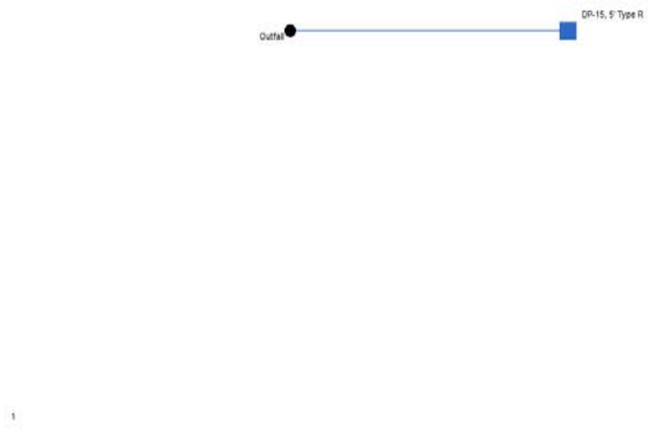
Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L1-18" RCP	14.10	24 c	223.0	5684.70	5687.82	1.399	5686.09	5689.15	n/a	5689.15 j	End
2	L2-18" RCP	14.10	24 c	216.0	5688.12	5690.28	1.000	5689.46	5691.61	n/a	5691.61 j	1
3	L3-18"RCP	14.10	24 c	83.0	5690.28	5691.11	1.000	5691.92	5692.44	n/a	5692.44 j	2
4	L4-18" RCP	8.30	18 c	159.0	5691.61	5693.20	1.000	5692.73	5694.30	n/a	5694.30 j	3
5	L5-18" RCP	8.30	18 c	21.0	5693.20	5693.41	1.000	5694.51	5694.51	n/a	5695.07 j	4

Project File: Stm-2A, Pond C1-R to DP-11, Maidford Dr, 100yr.stm	Number of lines: 5	Run Date: 12-11-2018

NOTES: c = cir; e = ellip; b = box; Return period = 100 Yrs. ; j - Line contains hyd. jump.

Hydraflow Plan View



Project File: Stm-3, Pond CR3 to DP-15, Yazoo Dr, 5yr.stm	No. Lines: 1	10-17-2018

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L1 - 18" RCP	2.20	18 c	141.0	5684.30	5689.94	4.000	5684.87	5690.51	n/a	5690.51 j	End

Project File: Stm-3, Pond CR3 to DP-15, Yazoo Dr, 5yr.stm	Number of lines: 1	Run Date: 10-17-2018

NOTES: c = cir; e = ellip; b = box; Return period = 5 Yrs. ; j - Line contains hyd. jump.

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L1 - 18" RCP	3.70	18 c	141.0	5684.30	5689.94	4.000	5685.03	5690.67	0.29	5690.67	End

Project File: Stm-3, Pond CR3 to DP-15, Yazoo Dr, 100yr.stm	Number of lines: 1	Run Date: 10-17-2018

NOTES: c = cir; e = ellip; b = box; Return period = 100 Yrs.

Hydraflow Plan View

Project File: Stm-4, Pond CR2 to DP-16, Castor Dr, 5yr.stm	No. Lines: 4	10-17-2018

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L4 - 24" RCP	12.40	24 c	40.0	5682.30	5683.50	3.000	5683.55	5684.75	n/a	5684.75	End
2	L2 - 18" RCP	2.10	18 c	103.0	5684.00	5685.85	1.796	5685.29	5686.40	n/a	5686.40 j	1
3	L3 - 18" RCP	2.10	18 c	247.0	5685.85	5690.30	1.802	5686.58	5690.85	n/a	5690.85 j	2
4	L4 - 18" RCP	2.10	18 c	33.0	5690.30	5690.89	1.789	5691.03	5691.44	n/a	5691.44 j	3

Project File: Stm-4, Pond CR2 to DP-16, Castor Dr, 5yr.stm

Number of lines: 4

Run Date: 12-11-2018

NOTES: c = cir; e = ellip; b = box; Return period = 5 Yrs. ; j - Line contains hyd. jump.

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	L4 - 24" RCP	26.70	24 c	40.0	5682.30	5683.50	3.000	5684.10	5685.30	0.90	5685.30	End
2	L2 - 18" RCP	4.60	18 c	103.0	5684.00	5685.85	1.796	5686.44	5686.67	n/a	5686.67 j	1
3	L3 - 18" RCP	4.60	18 c	247.0	5685.85	5690.30	1.802	5686.90	5691.12	n/a	5691.12 j	2
4	L4 - 18" RCP	4.60	18 c	33.0	5690.30	5690.89	1.789	5691.35	5691.71	0.34	5691.71	3

Project File: Stm-4, Pond CR2 to DP-16, Castor Dr, 100yr.stm

Number of lines: 4

Run Date: 12-11-2018

NOTES: c = cir; e = ellip; b = box; Return period = 100 Yrs. ; j - Line contains hyd. jump.

APPENDIX F–KIOWA ENGINEERING CHANNEL DESIGN REPORT

Final Channel Design Report

East Fork Jimmy Camp Creek
Creekside at Lorson Ranch Filing No. 1

CDR 192

El Paso County, Colorado

Prepared for:
Lorson Development
212 North Wahsatch Suite 301
Colorado Springs, Colorado 80903

Prepared by:
Kiowa
Engineering Corporation

1604 South 21st Street
Colorado Springs, Colorado 80904
(719) 630-7342

Kiowa Project No. 18020
May 14, 2020

List of Figures

Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Kiowa Engineering Corporation, 1604 South 21st Street, Colorado Springs, Colorado 80904

3/25/2020

Richard N. Wray
Registered Engineer #19310
For and on Behalf of Kiowa Engineering Corporation

Date

COLORADO REGISTERED PROFESSIONAL ENGINEER
RICHARD NORMAN WRAY
19310

Developer's Statement:

I, the Developer, have read and will comply with all of the requirements specified in this drainage report and plan.

BY: ______________________________

3/25/20
Date

JEFF MARK
Printed

ADDRESS: Lorson Development, LLC
212 North Wahsatch Suite 300
Colorado Springs, Colorado 80903

El Paso County:

Filed in accordance with the requirements of the Drainage Criteria Manual Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code, as amended.

____
Jennifer Irvine, P.E.
County Engineer/ECM Administrator

Approved
By: Elizabeth Nijkamp
Date:07/21/2020
El Paso County Planning & Community Development

I. General Location and Description

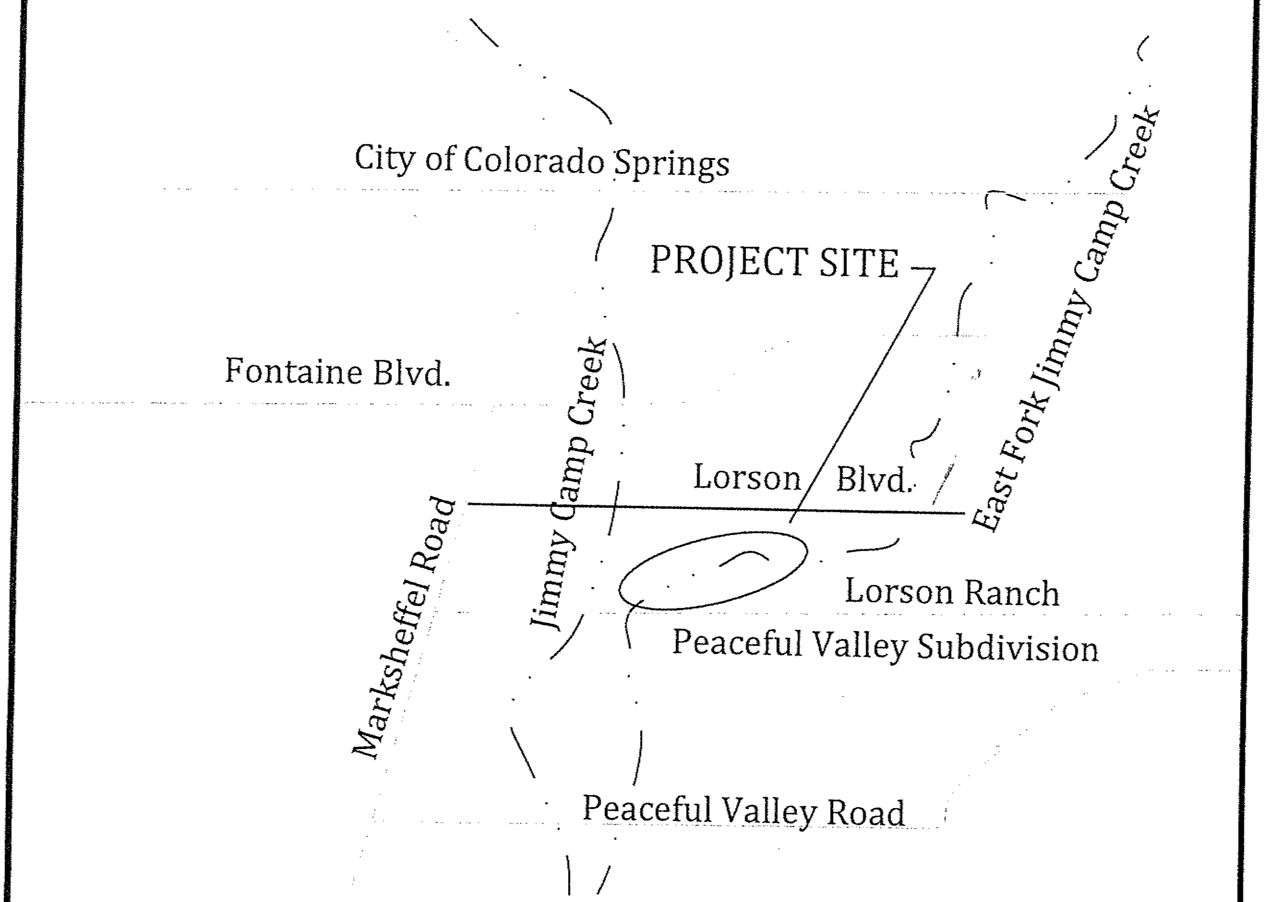
This report serves to summarize the design of the East Fork Jimmy Camp Creek (EFJCC), drainageway associated with the Creekside at Lorson Ranch Filing No. 1 subdivision. This design proposes to construct low flow boulder linings and soil/riprap banks at selective locations along a segment of EFJCC that begins at the south property line of Lorson Ranch and extends 3,900 feet upstream. At the upstream limit of the project an existing trapezoidal channel exists that was built as part of previous subdivision filings. The location of the site is shown on Figure 1.

Upon the completion of the drainageway facilities and acceptance by El Paso County and Lorson Ranch Metropolitan District, easements and or tracts will be dedicated within Creekside at Lorson Ranch Filing No. 1 for the purposes of maintenance access. Currently, the work will be completed within an un-plated parcel of land that encompasses the 100-year floodplain that commences at the south property line and extending north to Lorson Boulevard. Ownership, operation and maintenance of the drainageway will be the responsibility of the Lorson Ranch Metropolitan District.

Letter of Map Revision (LOMR), Case Number 19-08-0605P was approved in December 2019. The results of the LOMR become effective in April 2020. This LOMR reflects the post project condition of the channel improvements between Fontaine Boulevard to the north property line of Lorson Ranch, and new bridges at Fontaine Boulevard and Lorson Boulevard. The 100-year floodplain from the LOMR is shown on the design drawings and on the grading and erosion control plan. For the East Fork Jimmy Camp improvements south of Lorson Boulevard, encroachments of fill into the floodway have been avoided, and at a few locations the channel cross-section has been widened as compared to existing conditions. In this case, a no-rise determination has been prepared and the results included in Appendix D. Prior to commencing with the construction, a floodplain development permit will be processed through the Regional Administrator's office and the no-rise determination submitted. A Conditional Letter of Map Revision is therefore not required for the issuance of a floodplain development permit. The effective FIRM panel number 957G has been included within this report as Figure 2. The revised floodplain from the LOMR has been included as well and is presented as Figure 3.

A 404 permit has been issued for Lorson Ranch and covers all work proposed for East Fork Jimmy Camp Creek This permit has been included within Appendix B of this report. As with the construction for the bridges at Lorson Boulevard and Fontaine Boulevard, and the previous channel stabilization measures constructed for East Fork Jimmy Camp Creek, the condition of the permit require that the Corps of Engineers be notified when work authorized by the permit is anticipated to begin. Specifically, for the reach of East Fork Jimmy Camp Creek south of Lorson Boulevard, special condition 2 requires that that a stream preservation concept be advanced. The design as submitted with his report reflects the channel preservation concept whereby a "bankfull" low flow channel be constructed using un-grouted rock and channel benches stabilized with native vegetation. Once the initial review by El Paso County has been completed and the general design for the East Fork approved, a pre-construction meeting will be held with the Corps so that authorization under the Lorson Ranch 404 can proceed. This is the same process that was followed for the East Fork Jimmy Camp Creek north of Fontaine Boulevard. Based upon the initial review by El Paso County and a general acceptance of the proposed design, a wetland delineation will be updated in advance of a preconstruction notification.

Coordination with the Colorado Division of Parks and Wildlife, Department of Natural Resources (DNR), has been carried out as part of the design development. The proposed channel concept, specifically the low flow channel and overbank benched areas above the low flow, have been designed to address the concerns raised by the DNR during the review of the Creekside at Lorson



VICINITY MAP

SCALE: N.T.S.

National Flood Hazard Layer FIRMette

FIGURE 2.

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) Zone A, V, A99
- With BFE or Depth Zone AE, AO, AH, VE, AR
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Ar of 1% annual chance flood with averag depth less than one foot or with draina areas of less than one square mile Zon
- Future Conditions 1% Annual Chance Flood Hazard Zone X
- Area with Reduced Flood Risk due to Levee. See Notes. Zone X
- Area with Flood Risk due to Levee Zone

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard Zone X
- Effective LOMRs
- Area of Undetermined Flood Hazard Zo

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- B 20.2 17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
- 8 Coastal Transect
- 513 Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

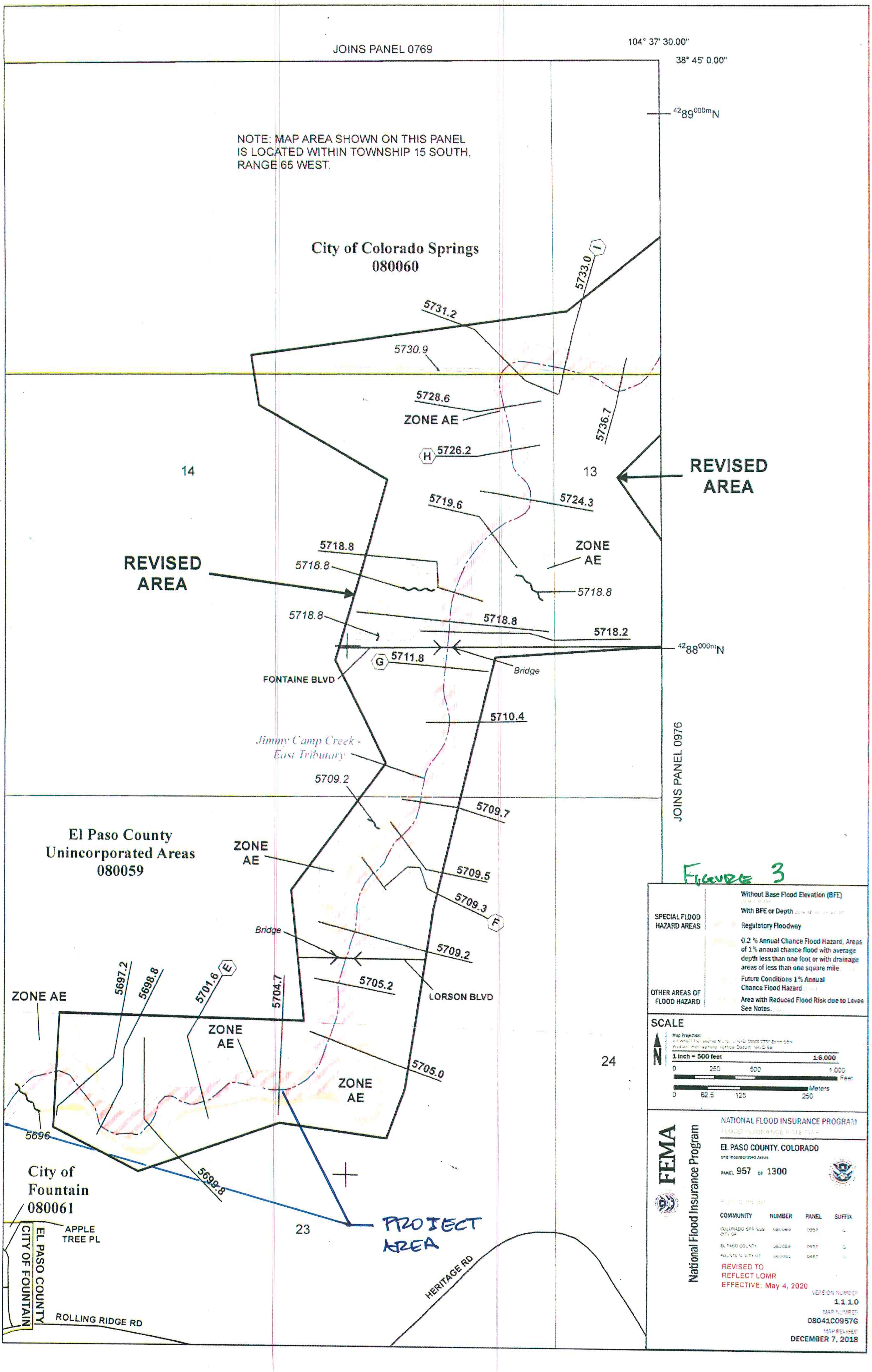
The pin displayed on the map is an approxima point selected by the user and does not repres an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 7/9/2019 at 12:54:29 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Figure 3


JOINS PANEL 0769
104° 37' 30.00"
38° 45' 0.00"
$^{42}89^{000\text{m}}$N
NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 15 SOUTH, RANGE 65 WEST.
City of Colorado Springs
080060
I
5733.0
5731.2
5730.9
5728.6
ZONE AE
5736.7
H
5726.2
14
13
REVISED AREA
5719.6
5724.3
ZONE AE
5718.8
5718.8
REVISED AREA
5718.8
5718.8
5718.8
5718.2
$^{42}88^{000\text{m}}$N
G
5711.8
Bridge
FONTAINE BLVD
5710.4
Jimmy Camp Creek - East Tributary
5709.2
5709.7
JOINS PANEL 0976
El Paso County
Unincorporated Areas
080059
ZONE AE
5709.5
5709.3
F
Bridge
5709.2
5705.2
LORSON BLVD
ZONE AE
5697.2
5698.8
5701.6
E
5704.7
ZONE AE
5705.0
ZONE AE
24
5696
5699.8
City of Fountain
080061
APPLE TREE PL
23
PROJECT AREA
HERITAGE RD
EL PASO COUNTY
CITY OF FOUNTAIN
ROLLING RIDGE RD
SPECIAL FLOOD HAZARD AREAS
Without Base Flood Elevation (BFE)
With BFE or Depth
Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD
0.2 % Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile
Future Conditions 1% Annual Chance Flood Hazard
Area with Reduced Flood Risk due to Levee See Notes.
SCALE
N
1 inch = 500 feet
1:6,000
0
250
500
1,000
Feet
0
62.5
125
250
Meters
FEMA
National Flood Insurance Program
NATIONAL FLOOD INSURANCE PROGRAM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY, COLORADO
and Incorporated Areas
PANEL 957 OF 1300
COMMUNITY
NUMBER
PANEL
SUFFIX
COLORADO SPRINGS, CITY OF
080060
0957
G
EL PASO COUNTY
080059
0957
G
FOUNTAIN, CITY OF
080061
0957
G
REVISED TO REFLECT LOMR
EFFECTIVE: May 4, 2020
VERSION NUMBER
1.1.1.0
MAP NUMBER
08041C0957G
MAP REVISED
DECEMBER 7, 2018

Ranch Filing No. 1 subdivision application. The documents related to the design coordination with the DNR has been included within Appendix E of this report.

The developer intends to request reimbursement for the cost to construct drainageway facilities, or request credit against future drainage and bridge fees. Reimbursement will be processed in accordance with sections 1.7 and 3.3 of the Drainage Criteria Manual (DCM). The drainageway facilities will be owned, operated and maintained by the Lorson Ranch Metropolitan District.

II. Project Background

EFJCC is a natural drainageway that was shown to be stabilized in the Lorson Ranch Master Development Drainage Plan (MDDP). The MDDP as last updated showed the EFJCC drainageway to be reconfigured into a benched channel section capable of conveying the 100-year discharge as defined in the Reference 6. The bankfull flow for this segment of East Fork Jimmy Camp Creek which typically has a recurrence interval of around the 1-3/4- to 2-year runoff event, was estimated at 110 cubic feet per second in Reference 2. The segment subject to design begins at the south property line and terminates at the existing trapezoidal channel that was constructed in 2015

In April 2015, the City of Colorado Springs adopted an update to the 1987 Jimmy Camp Creek DBPS. The primary findings and recommendations summarized in the updated 2015 DBPS regarding hydrology and the recommendation for implementation of full spectrum detention (FSD) within the overall Jimmy Camp Creek watershed. The long-term stable slope estimated in the Reference 2 was 0.09 percent. The segment subject to design presently has an average longitudinal slope of 0.25 percent. The segment subject to design will need vertical stabilization by means of grade controls. The 100-year discharge used in the design was obtained from References 6 and 7. The 100-year hydrology used for design reflects existing development conditions within the tributary watershed.

Another finding of the 2015 DBPS was that with the assumption of the maintenance of existing basin condition flow rates through the implementation of FSD, the low flow channel would still be needing stabilization because of the anticipation of continuous low flow once the basin develops into an urban watershed. The 2015 DBPS also called for the 100-year floodplain to be preserved for many segments of the natural drainageways within the Jimmy Camp Creek watershed, including the EFJCC drainageway subject to this design. Low flow stabilization was called for in the 2015 DBPS for the EFJCC, along with selective bank lining and the preservation of the 100-year floodplain.

Though the 2015 DBPS was never adopted by El Paso County, the County is now requiring development to provide for FSD, as in the City of Colorado Springs. The implementation of FSD is being accomplished in the County through the adoption of Chapter 6 and Section 3.2.1 of Chapter 13 of the City of Colorado Springs Drainage Criteria Manual, Volume 1.

III. Previous Reports and References

The basis for the development of the design has been developed from referencing the following reports:

1. ***Lorson Ranch Master Development Drainage Plan (MDDP), prepared by Core Engineering, latest version (not approved by El Paso County).***
2. ***Jimmy Camp Creek Drainage Basin Planning Study (DBPS), prepared by Kiowa Engineering, 2015 (not approved by El Paso County).***
3. ***City of Colorado Springs and El Paso County Drainage Criteria Manual, 1987.***
4. ***El Paso County Engineering Criteria Manual, most current version.***

5. *City of Colorado Springs Drainage Criteria Manual, Chapters 6 and 12, May 2014.*
6. *The City of Colorado Springs and El Paso County Flood Insurance Study (FIS), prepared by the Federal Emergency Management Agency, effective December 7, 2018.*
7. *East Fork Jimmy Camp Creek Letter of Map Revision, Case Number 19-08-0605P, Lorson Ranch Development, dated May 2019.*
8. *Design of Roadside Channels with Flexible Linings, Hydraulic Engineering Circular 15, October 1985.*

Reference 7 provides for the existing condition floodplain and floodway for the segment of EFJCC subject to this design. The 100-year existing condition floodplain has been shown on the design drawings. Construction of the channel improvements shown on the design plans will not alter the limits of the 100-year floodplain and floodway from those shown in Reference 7. Reference 7 is the post-project condition LOMR that reflects the bridges at Lorson Boulevard, Fontaine Boulevard and the drainageway stabilization measures from Fontaine Boulevard to the north property line of Lorson Ranch, all constructed as part of the Lorson East Subdivision. Reference 7 has been included in the Appendix. The LOMR is contained within Appendix D.

Chapter 6 and Section 3.2.1 of Chapter 12 of the City of Colorado Springs DCM (Reference 5), was made part of Reference 3 by El Paso County Board of County Commissioners Resolution 15-042.

IV. Site Description

The EFJCC floodplain within the design reach is vegetated with native and non-native grasses, herbs and shrubs that are in fair to good condition. The channel overbank is vegetated with trees and shrubs. There is very little evidence of active invert degradation or bank sloughing however there are some portions of the existing low flow channel that have formed nearly vertical banks. Current longitudinal slope along the project is ranges from 0.18 to 0.32 percent. There is presently a base flow in this segment. Where a low flow channel has formed, top widths range from 10 to 20-feet wide and ranges in depth from 2 to 3 feet. Topography used in the design was compiled at a two-foot contour interval and is dated 2015. The grading for the drainageway has been tied into the proposed grading for Creekside at Lorson Ranch Filing 1 as developed by Core Engineering. There are presently no encroachments into the floodplain or channel thread associated with man-made structures. There is presently an existing sanitary sewer outfall owned by Widefield Water and Sanitation that is aligned at the west bank of the floodplain. The Fountain Mutual Irrigation Company siphon crosses under the proposed drainageway near the south property line.

V. Hydrology

Hydrology for use in determining the typical channel sections shown on the plans were obtained from References 6 and 7. The 100-year discharge shown in Reference 7 (5,500 cubic feet per second), has been used in the hydraulic design of the channel banks and associated armoring. The HEC-RAS model developed for References 6 and 7 is contained within Appendix B. The 100-year water surface, depths and velocity were used in sizing the soil riprap bench and bank linings. Watershed area for the southern limit of the project is approximately 9.2 square miles (Reference 6). The watershed north of the Lorson Ranch development is presently undeveloped. Table 4 from Reference 6 has been included within Appendix A.

The assumption that FSD will be required for all future development is reflected in the use of the FIS discharges in this design. There is a good correlation between the FIS and DBPS 100-year discharges for the segment of EFJCC subject to this design. Use of the existing basin condition flow rates is consistent with the requirements set forth in the annexation agreement between the owners of Banning-Lewis Ranch and the City of Colorado Springs. The future FSDs within Banning-Lewis Ranch will be publicly operated and maintained facilities.

VI. Hydraulics

The hydraulic design of the drainageway and bridge as presented on the plans was carried out using the US Army Corps of Engineers HEC-RAS model compiled for References 6 and 7. The summary output for this model has been included within Appendix A. The results from the HEC-RAS model was used to determine the 100-year hydraulic grade line shown on the design profile. The 100-year profile from Reference 6 has been included in the Appendix A as well. The limits of the 100-year floodplain from Reference 7 has been presented on the design plans as well as on the grading and erosion control plan. The location for selected HEC-RAS cross-sections are shown on the design profile. The LOMR floodplain work maps from Reference 7 have been included within Appendix D.

The proposed drainageway design concepts put forth on the plans are 100-year selective bank lining with low flow stabilization. The bankfull channel will be constructed using un-grouted boulders. Above the bankfull channel will be soil and riprap benches that will be revegetated using native grasses and shrubs. At outside bends, soil and riprap bank linings with maximum side slopes of 3 to 1 is proposed that will extend to the height of the 100-year hydraulic grade line. The soil riprap benches were sized using the tractive force that would be developed during a 100-year flood event. Permissible shear stresses were obtained from Reference 8.

The effect of development within the watershed will be to increase the frequency and duration of base flows. Base flows will increase with the development because of discharges from future FSDs and irrigation return flows. Natural drainageways will eventually degrade along the invert in turn causing bank sloughing to occur. The bank full capacity as estimated in the DBPS represents rate of runoff that would form the low flow channel over time. The bank full capacity for most natural watersheds represents a flow rate usually between the 2- to 5-year recurrence intervals. In order to comply with DBPS criteria, the low flow channel capacity for this design was set at 110 cubic feet per second per Reference 2. The current DCM requires that the low flow channel be design for the 10-year discharge or 10 percent of the 100-year discharge. Using current County criteria, the low flow would be required to be sized to convey 550 cubic feet per second. Assuming a 2-foot depth the required top width would be 38 feet. As providing for this flow rate of conveyance would cause a significant reconfiguration of the existing low flow channel with resulting negative impacts upon the existing wetland vegetation and fish habitat. A deviation request will be submitted to allow for the sizing of the low flow channel to 110 cubic feet per second as determined in Reference 2.

A qualitative channel stability analysis was carried as part of developing the design for EFJCC. The analysis consisted of a field inspection, historic topographic mapping comparisons and the determination of existing channel slopes. Field observations revealed no indication of invert degradation along the entire length of the design reach. The long-term stable slope for this segment the East Fork Jimmy Camp Creek was estimated at 0.09 percent (Reference 2). The current slope ranges from 0.18 to 0.32 percent through the project reach. The design plans have been developed to address the potential for invert degradation should the channel seek the 0.09 percent long-term equilibrium slope of slope estimated in Reference 2. Should the invert reach a slope of 0.09 percent the bottom of the boulders as designed will not become exposed and therefore undermining of the low flow channel will be prevented. The boulders along the low flow channel will be situated so that the bottom of the boulder is at least two feet below the design invert shown on the profiles. Where the long-term invert would cause the channel to degrade to the bottom of the boulder lining, a grade control structure has been proposed. Five grade control structures have been designed and shown on the plan and profiles

Based upon the field observations regarding channel stability, the EFJCC low flow channel was designed to operate at normal depths of flow, thereby eliminating channel instability associated with super-critical flow conditions. The low flow channel lining is proposed to be a combination of soil/riprap bank and turf reinforcement mats depending upon velocity. The locations where selective 100-year soil/riprap lining are proposed was based upon the velocities returned by the HEC-RAS model. Velocities for the 100-year discharge range from 5.3 to 10.5 feet per second. The F100-yar Froude Number ranges from .37 to .73 which confirms that subcritical flow conditions exist even for the 100-year event. Calculations related to the sizing of the soil riprap banks, for the overbanks and low flow channel section are contained within the Appendix A of the report. The low flow is in normal depth conditions for the entire reach. Velocity within the low flow channel is ranges from 4.0 to 4.4 feet per second assuming a two-foot depth of flow and bottom widths ranging from 12 to 20-feet. The Froude Number for the low flow channel ranges from .52 to .54 which confirms the presence of normal flow conditions. At the outside channel bends of the floodplain, soil riprap is proposed as the bank lining material. Soil riprap is also specified for the channel bench above the low for channel

There was also an effort to realign portions of the low flow channel away the toe of outside bends of the drainageway. The intent of the repositioning of the low flow in these locations was to minimize disturbance to the vegetation on the benches of the 100-year floodplain that could occur during construction. Finally, shear stress calculations were carried out for the 100-year flow condition at each segment of the drainageway. Maximum 100-year shear stress on the bench was calculated at 1.4 pounds per square foot. Permissible shear stress for native vegetation with Class B retardance is 2.1 pounds per square foot for the vegetation that is present at the site. Channel design calculations are included in the Appendix A of this report. memorandum.

VII. Design Elements

Presented on the design plans associated with this report are the proposed drainageway conditions. Design criteria for the project are summarized as follows:

Channel design slope:	0.18-0.32 percent
Outside bend slopes- riprap	3 to 1 maximum
Low flow channel side slopes- riprap lined	vertical
Low flow channel depth	3 feet
Manning's n-values:	.025-.04
Minimum low flow channel radius	100 feet
Design shear stress: low flow channel	
Boulder linings	1.4 psf
Design shear stress: soil/riprap linings at outside bends and benches	
Type VL riprap	2.5 psf

The construction of the improvements shown on the plans will result in a long-term stable drainageway corridor and prevent damages that could arise from bank sloughing related to the erosion of the drainageway's invert. Because the low flow channel will be stabilized both horizontally and vertically the potential for negative impacts upon the existing vegetative habitat will be minimized. The preservation of the low flow channel and floodplain is consistent with the special condition 2 of the East Fork Jimmy Camp Creek 404 permit and with Reference 2. A stabilized floodplain corridor will result from the construction of the proposed drainageway structures and over the long term, the environmental quality of the corridor will be enhanced and preserved.

Maintenance access to the low flow channel and benches be provided via platted tracts within Creekside at Lorson Ranch Filing 1. The maintenance road will follow the existing outfall sewer that is shown on the design plans. The benches of the channel are relatively flat and will allow for access to the low flow channel, however an access trail to the benches is not recommended in order to limit disturbance to existing vegetation or that will be revegetated in the future. Maintenance access will have an all-weather surface and be a minimum of 12-feet in width.

VIII. Construction Permitting

The following permits are anticipated to allow for the construction of the project as shown on the design plans. A copy of the Lorson Ranch 404 Permit is included within the Appendix.

Notification of project in conformance with existing 404 permit - USACOE
Floodplain Development Permit – Regional Building Department
Grading and Erosion Control Permit (ESQCP) – El Paso County
Construction Stormwater Discharge Permit – CDPHE

IX. Drainage and Bridge Fees

The Lorson Ranch Development and specifically Lorson Ranch East lies wholly within the Jimmy Camp Creek drainage basin. Drainage and bridge fees have been established by the County for the Jimmy Camp Creek drainage basin for assessment against platted land within the watershed. The drainageway structures will be public and will be maintained by the Lorson Ranch Metropolitan District and are considered reimbursable or creditable, if a DBPS is approved, against drainage fees owed when land within Creekside at Lorson Ranch Filing 1 is platted pending approval through the DCM reimbursement process.

The current 2020 drainage and bridge fees for the Jimmy Camp Creek drainage basin are as follows:

Drainage Fee:	$19,084 per all impervious acres
Drainage Fee Escrow (BOCC Reas.18-470)	$7,285 per acre
Total Drainage Fee	$26,369 per acre
Bridge Fee:	$893 per acre

X. Phasing

Construction of the drainageway stabilization measures shown on the plans is to be completed all at once and no phasing of the construction is proposed. The construction will commence prior to or concurrent with the development of Creekside at Lorson Ranch Filing 1. Plans are to commence with construction in Summer 2020 with substantial completion in late Fall 2020.

XI. Conclusions

The development of the Creekside at Lorson Ranch Filing No. 1 requires that drainageway stabilization be implemented along East Fork Jimmy Camp Creek. The stabilization measures are intended to provide protection to the residential lots from long term erosion of the banks and invert of East Fork Jimmy Camp Creek. Stabilization of the drainageway to convey 100-year peak discharges will also promote better water quality to downstream reaches of East Fork Jimmy Camp Creek, Jimmy Camp Creek and Fountain Creek as the potential for sediment from bank sloughing to be conveyed downstream will be eliminated. Upon construction of the drainageway stabilization measures as shown on the design plans, adjacent and downstream properties will not be adversely impacted by the runoff from Creekside Filing No. 1 and upstream drainage basins that will be conveyed by East Fork Jimmy Camp Creek.

Appendix A

Hydrologic and Hydraulic Calculations

Table 4. Summary of Discharges (cont.)

Flooding Source and Location	Drainage Area (Square Miles)	Peak Discharges (Cubic Feet Per Second) 10-Year	50-Year	100-Year	500-Year
Jimmy Camp Creek					
At confluence with Fountain Creek	66.4	8,500	12,400	16,000	20,500
Jimmy Camp Creek – East Tributary					
At confluence with Jimmy Camp Creek	9.2	2,800	4,600	5,500	6,900
Jimmy Camp Creek – West Tributary					
At confluence with Jimmy Camp Creek	3.93	1,160	2,280	2,780	4,500
Kettle Creek					
At State Highway 83	16.3	2,600	6,600	9,300	19,300
Lower Big Springs					
At confluence with Black Squirrel Creek	--[1]	--[1]	--[1]	4,820	--[1]
Mesa Basin					
At confluence with Monument Creek	2.2	1,260	1,880	2,250	3,470
Mines Subtributary to Corral Tributary					
At confluence with Corral Tributary	9.4	2,200	2,500	4,300	5,400
Monument Creek					
At confluence with Fountain Creek	238.0	11,500	23,500	32,000	57,000
Upstream of City of Colorado Springs corporate limits	196.5	10,200	20,700	27,200	49,000
At northern boundary of U.S. Air Force Academy	74.7	7,400	15,300	19,900	37,200
Monument Creek Tributary					
At Cross Section C	1.4	890	1,620	1,880	2,880
190 feet downstream of Cross Section A	1.4	890	1,620	1,880	5,020[2]
North Beaver Creek					
At Confluence with Beaver Creek	3.5	--[1]	--[1]	1,932	--[1]

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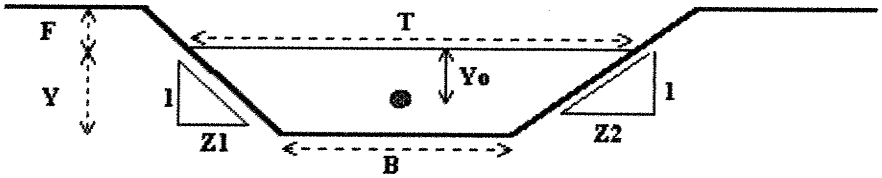
[1] Data not available

[2] Includes Monument Creek Bypass, only 500-year flood

Normal Flow Analysis - Trapezoidal Channel

Project: 18020 East Fork Sand Creek south of Lorson Blvd

Channel ID: Bankfull low flow Q=110 cfs 12-foot BW s=0.32%

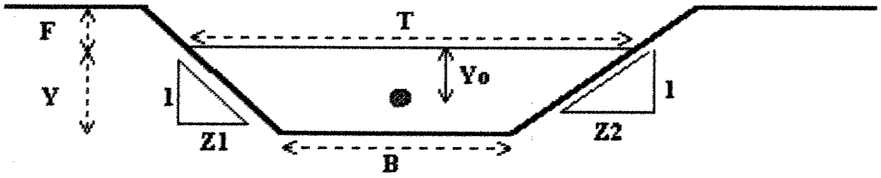


Design Information (Input)			
Channel Invert Slope	So =	0.0032	ft/ft
Manning's n	n =	0.025	
Bottom Width	B =	12.00	ft
Left Side Slope	Z1 =	0.10	ft/ft
Right Side Slope	Z2 =	0.10	ft/ft
Freeboard Height	F =	0.00	ft
Design Water Depth	Y =	**2.00**	ft

Normal Flow Condtion (Calculated)			
Discharge	**Q =**	**108.90**	cfs
Froude Number	**Fr =**	**0.56**	
Flow Velocity	**V =**	**4.46**	fps
Flow Area	A =	24.40	sq ft
Top Width	T =	12.40	ft
Wetted Perimeter	P =	16.02	ft
Hydraulic Radius	R =	1.52	ft
Hydraulic Depth	D =	1.97	ft
Specific Energy	Es =	2.31	ft
Centroid of Flow Area	Yo =	0.99	ft
Specific Force	Fs =	2.46	kip

Critical Flow Analysis - Trapezoidal Channel

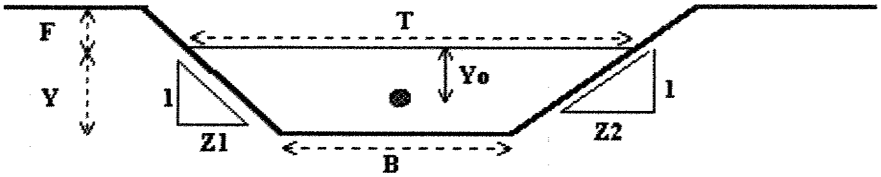
Project: 16031 East Fork Jimmy Camp Creek
Channel ID: Bankfull low flow Q=110 cfs 12-foot BW s=0.32%



Design Information (Input)			
Bottom Width	B =	12.00	ft
Left Side Slope	Z1 =	0.10	ft/ft
Right Side Slope	Z2 =	0.10	ft/ft
Design Discharge	Q =	**110.00**	cfs
Critical Flow Condition (Calculated)			
Critical Flow Depth	Y =	**1.37**	ft
Critical Flow Area	A =	16.63	sq ft
Critical Top Width	T =	12.27	ft
Critical Hydraulic Depth	D =	1.35	ft
Critical Flow Velocity	V =	6.62	fps
Froude Number	Fr =	**1.00**	
Critical Wetted Perimeter	P =	14.75	ft
Critical Hydraulic Radius	R =	1.13	ft
Critical (min) Specific Energy	Esc =	2.05	ft
Centroid on the Critical Flow Area	Yoc =	0.68	ft
Critical (min) Specific Force	Fsc =	2.12	kip

Normal Flow Analysis - Trapezoidal Channel

Project: **18020 East Fork Sand Creek south of Lorson Blvd**
Channel ID: **Bankfull low flow Q=110 cfs 20-foot BW S=0.18%**



Design Information (Input)

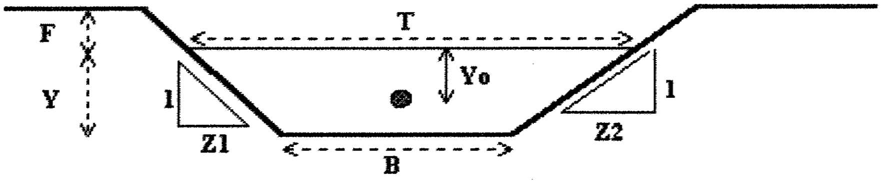
Channel Invert Slope	So =	0.0018	ft/ft
Manning's n	n =	0.025	
Bottom Width	B =	20.00	ft
Left Side Slope	Z1 =	0.10	ft/ft
Right Side Slope	Z2 =	0.10	ft/ft
Freeboard Height	F =	0.00	ft
Design Water Depth	Y =	**2.00**	ft

Normal Flow Condtion (Calculated)

Discharge	**Q =**	**144.48**	cfs
Froude Number	**Fr =**	**0.45**	
Flow Velocity	**V =**	**3.58**	fps
Flow Area	A =	40.40	sq ft
Top Width	T =	20.40	ft
Wetted Perimeter	P =	24.02	ft
Hydraulic Radius	R =	1.68	ft
Hydraulic Depth	D =	1.98	ft
Specific Energy	Es =	2.20	ft
Centroid of Flow Area	Yo =	1.00	ft
Specific Force	Fs =	3.51	kip

Critical Flow Analysis - Trapezoidal Channel

Project: 16031 East Fork Jimmy Camp Creek
Channel ID: Bankfull low flow Q=110 cfs 20-foot BW S=0.18%

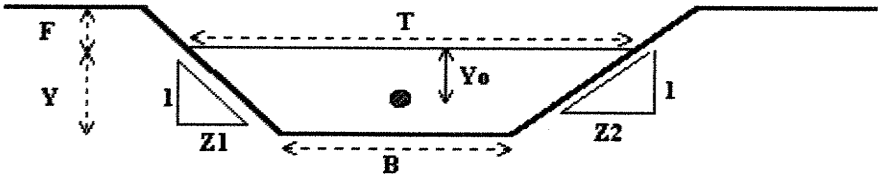


Design Information (Input)			
Bottom Width	B =	20.00	ft
Left Side Slope	Z1 =	0.10	ft/ft
Right Side Slope	Z2 =	0.10	ft/ft
Design Discharge	Q =	**100.00**	cfs
Critical Flow Condition (Calculated)			
Critical Flow Depth	**Y =**	**0.92**	ft
Critical Flow Area	A =	18.38	sq ft
Critical Top Width	T =	20.18	ft
Critical Hydraulic Depth	D =	0.91	ft
Critical Flow Velocity	V =	5.44	fps
Froude Number	**Fr =**	**1.00**	
Critical Wetted Perimeter	P =	21.84	ft
Critical Hydraulic Radius	R =	0.84	ft
Critical (min) Specific Energy	Esc =	1.37	ft
Centroid on the Critical Flow Area	Yoc =	0.46	ft
Critical (min) Specific Force	Fsc =	1.58	kip

Normal Flow Analysis - Trapezoidal Channel

Project: 18020 East Fork Sand Creek south of Lorson Blvd

Channel ID: Q=550 cfs 12-foot BW s=0.18%

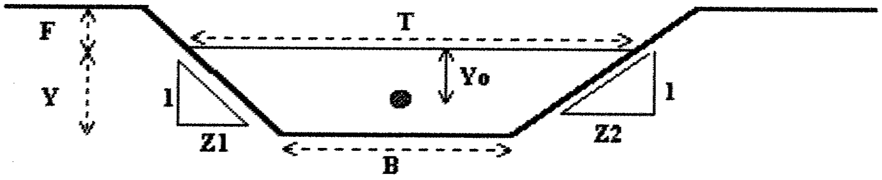


Design Information (Input)			
Channel Invert Slope	So =	0.0018	ft/ft
Manning's n	n =	0.025	
Bottom Width	B =	38.00	ft
Left Side Slope	Z1 =	0.10	ft/ft
Right Side Slope	Z2 =	0.10	ft/ft
Freeboard Height	F =	0.00	ft
Design Water Depth	Y =	**3.00**	ft

Normal Flow Condtion (Calculated)			
Discharge	**Q =**	**550.70**	cfs
Froude Number	**Fr =**	**0.49**	
Flow Velocity	**V =**	**4.79**	fps
Flow Area	A =	114.90	sq ft
Top Width	T =	38.60	ft
Wetted Perimeter	P =	44.03	ft
Hydraulic Radius	R =	2.61	ft
Hydraulic Depth	D =	2.98	ft
Specific Energy	Es =	3.36	ft
Centroid of Flow Area	Yo =	1.50	ft
Specific Force	Fs =	15.85	kip

Critical Flow Analysis - Trapezoidal Channel

Project: **16031 East Fork Jimmy Camp Creek**
Channel ID: **Q=550 cfs 12-foot BW s=0.18%**



Design Information (Input)

Bottom Width	B =	38.00	ft
Left Side Slope	Z1 =	0.50	ft/ft
Right Side Slope	Z2 =	0.50	ft/ft
Design Discharge	Q =	**550.00**	cfs

Critical Flow Condition (Calculated)

Critical Flow Depth	Y =	**1.85**	ft
Critical Flow Area	A =	72.01	sq ft
Critical Top Width	T =	39.85	ft
Critical Hydraulic Depth	D =	1.81	ft
Critical Flow Velocity	V =	7.64	fps
Froude Number	**Fr =**	**1.00**	
Critical Wetted Perimeter	P =	42.14	ft
Critical Hydraulic Radius	R =	1.71	ft
Critical (min) Specific Energy	Esc =	2.76	ft
Centroid on the Critical Flow Area	Yoc =	0.91	ft
Critical (min) Specific Force	Fsc =	12.24	kip

Project Name: EAST FORK JCC Project No: 18020
Description: Riprap Sizing
By: RDW Date: 1-20-20

Riprap Sizing:

Max Velocity in vicinity

10.4 fps X-SEC 5500

$S = .0032'/'$ $SG = 2.6$

$$\frac{VS^{.17}}{(2.6-1)^{.66}} = \frac{VS^{.17}}{1.36} = \frac{4.02}{1.36} = 2.96$$

High end of VL

∴ Per table 10-6 DCM Type L

$D_{50} = 9"$; min thickness = 18"

TABLE 10-6

RIPRAP REQUIREMENTS FOR CHANNEL LININGS **

$VS^{0.17}/(S_s-1)^{0.66}$* (ft^{1}/2/sec)	Rock Type ***
1.4 to 3.2	VL
3.3 to 3.9	L ←
4.0 to 4.5	M
4.6 to 5.5	H
5.6 to 6.4	VH

* where:

V = mean channel flow velocity, in fps;

S = longitudinal channel slope, in feet per foot (ft/ft); and

S_s = specific gravity of stone (minimum $S_s = 2.50$)

** Table valid only for Froude number of 0.8 or less and side slopes no steeper than 2h:1v.

*** Type VL and L riprap may be buried after placement to reduce vandalism.

Kiowa Engineering Corporation

Project Name: EAST FORK JCC
Project No: 18020
Description: Tractive Force
By: TRW
Date: 6/10/19

Tractive Force on channel benches.

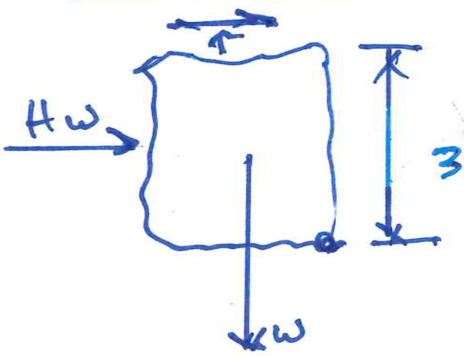
Typical depth over bench ≈ 9'
Typical slope = S = .25%

$\tau = \gamma d S = 62.4(9)(.0025) = 1.4$ psf

Permissible tractive Force (Des. of Roadside channels)
Class 'B' Vegetation = 2.1
Riprap: 6" D_{50} = 2.5 psf
Riprap: 12" D_{50} = 5.0 psf.

Use Type VL : D_{50} = 6" ; 18" thick.

Boulders



w = 130 pcf : 9 cf/ft
w = 1170 #/ft

$\tau = 62.4(10')(.0025) = 1.75$ #/ft

$H_w = \gamma h = 62.4(10' - 1.5) = 530$ psf/ft

Overturning 530(1.5) + 1.75(3) = 800 #/ft
Resisting 1170(1.5) = 1755 #/ft

FS = 1755/800 = 2.2 ∴ ok

DESIGN OF ROADSIDE CHANNELS WITH FLEXIBLE LININGS

Hydraulic Engineering Circular No. 15

Prepared By

Simons, Li & Associates, Inc.
3555 Stanford Road
P.O. Box 1816
Fort Collins, Colorado 80522

For

U.S. Department of Transportation
Federal Highway Administration

October 25, 1985

Table 4.1. Permissible Shear Stresses for Lining Materials.

Lining Category	Lining Type	Permissible Unit Shear Stress (lb/ft2)
Temporary	Woven Paper Net	0.15
	Jute Net	0.45
	Fiberglass Roving*	0.75
	Straw and Erosion Net	1.45
	Curled Wood Mat	1.55
	Nylon Mat	2.00
Vegetative	Class A	3.70
	Class B	2.10
	Class C	1.00
	Class D	0.60
	Class E	0.35
Gravel Riprap	1-inch	0.40
	2-inch	0.80
Rock Riprap	6-inch	2.50
	12-inch	5.00

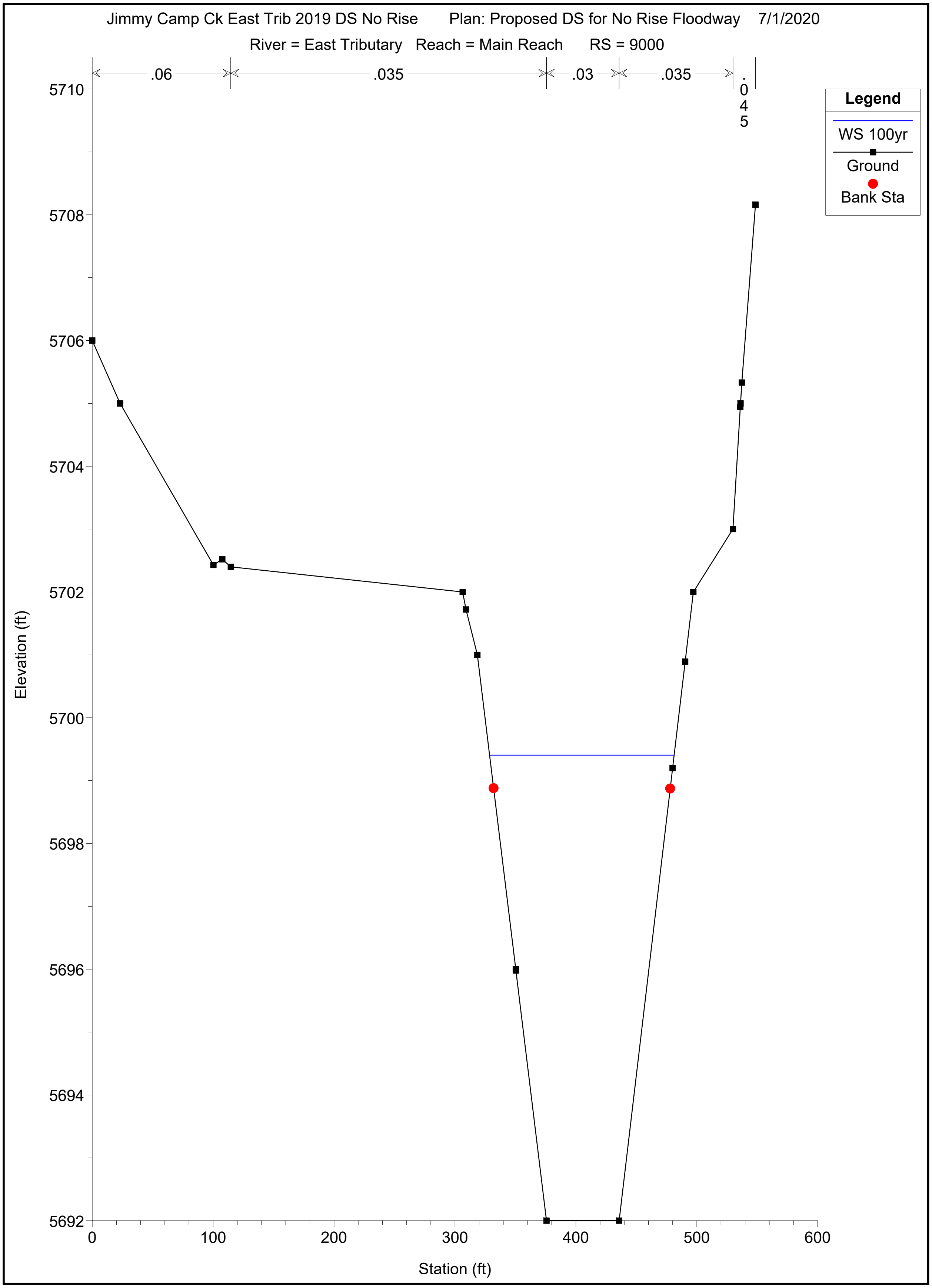
* single and double applications

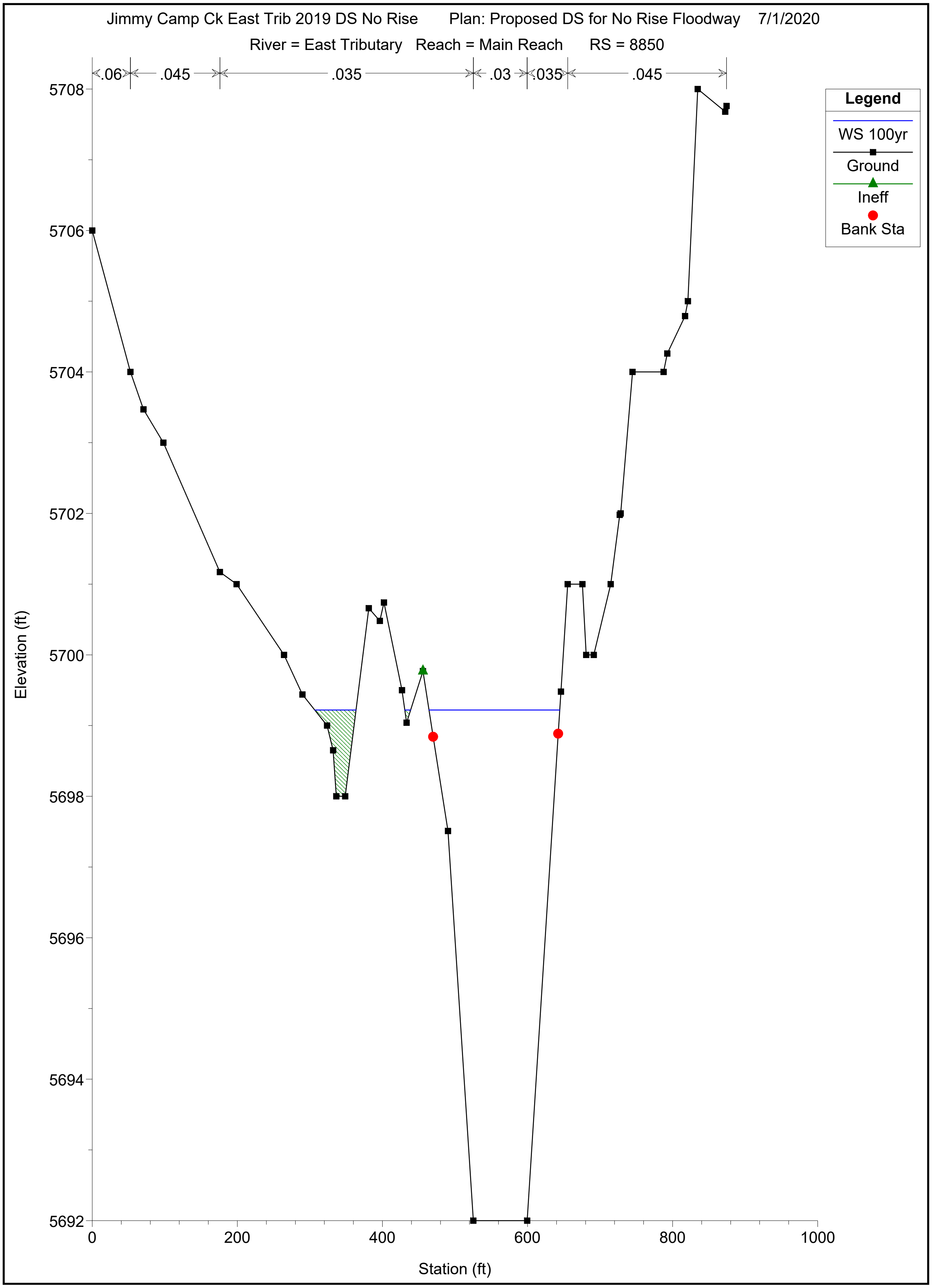
HEC-RAS Plan: Proposed No Rise Floodway River: East Tributary Reach: Main Reach

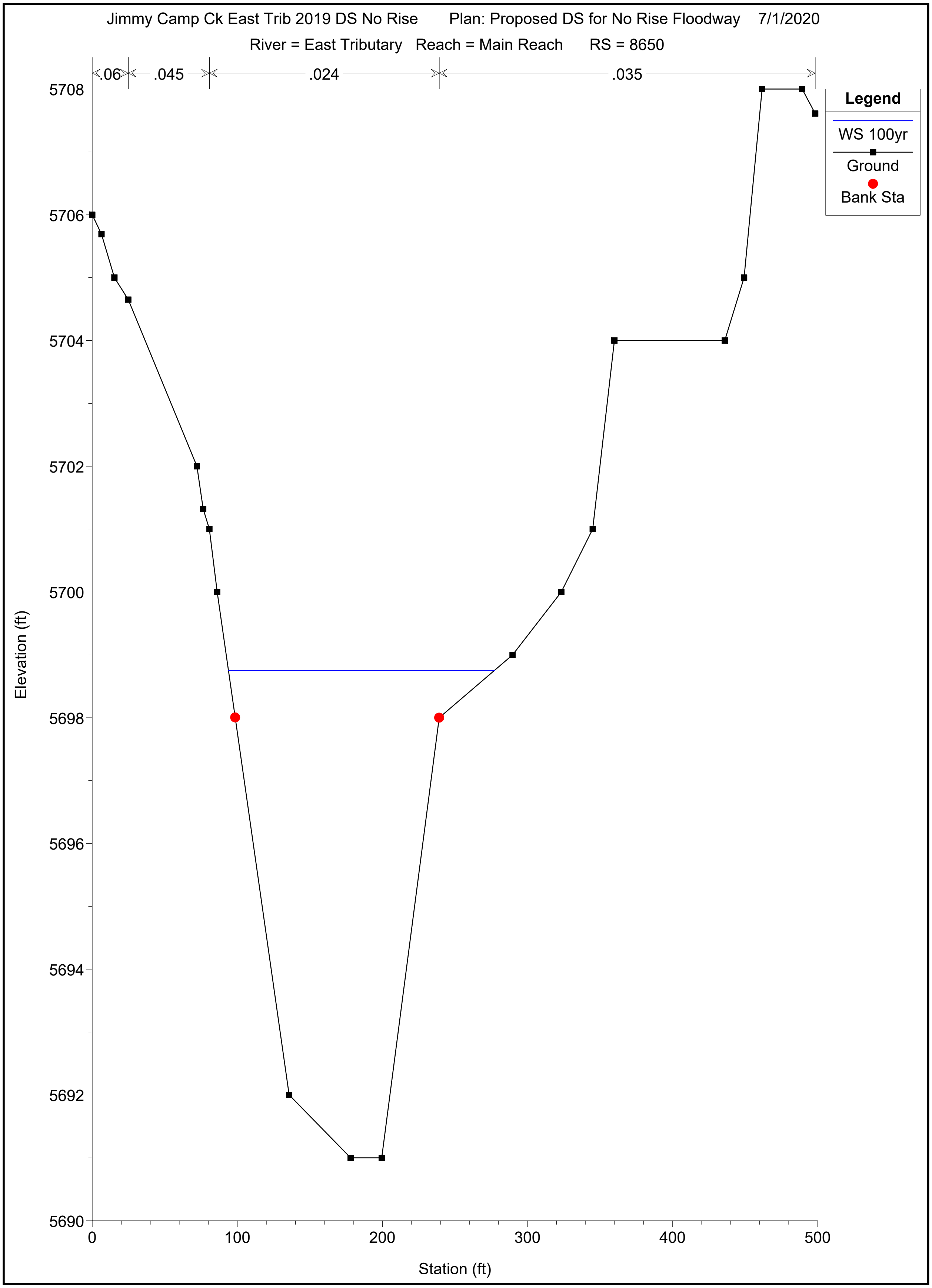
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Main Reach	9000	100yr	5500.00	5692.00	5699.40	5697.26	5700.17	0.002111	7.00	787.40	152.60	0.53
Main Reach	9000	550 cfs	550.00	5692.00	5694.15	5693.31	5694.34	0.002131	3.48	157.90	86.94	0.46
Main Reach	9000	110 cfs	110.00	5692.00	5692.99	5692.46	5693.03	0.001280	1.68	65.44	72.45	0.31
Main Reach	8850	100yr	5500.00	5692.00	5699.22	5696.82	5699.83	0.001742	6.29	875.73	244.91	0.49
Main Reach	8850	550 cfs	550.00	5692.00	5693.74	5693.16	5693.96	0.003039	3.72	148.04	95.98	0.53
Main Reach	8850	110 cfs	110.00	5692.00	5692.66	5692.40	5692.73	0.003435	2.14	51.39	82.33	0.48
Main Reach	8650	100yr	5500.00	5691.00	5698.75	5696.57	5699.52	0.001322	7.02	797.06	183.23	0.53
Main Reach	8650	550 cfs	550.00	5691.00	5693.07	5692.60	5693.36	0.002780	4.34	126.58	82.15	0.62
Main Reach	8650	110 cfs	110.00	5691.00	5691.96	5691.72	5692.06	0.003216	2.58	42.61	67.48	0.57
Main Reach	8521.53	100yr	5500.00	5690.25	5698.62	5696.19	5699.21	0.003318	6.17	897.93	204.98	0.49
Main Reach	8521.53	550 cfs	550.00	5690.25	5691.88	5691.88	5692.55	0.021563	6.56	83.80	63.28	1.01
Main Reach	8521.53	110 cfs	110.00	5690.25	5690.85	5690.85	5691.12	0.028638	4.19	26.25	48.17	1.00
Main Reach	8470	100yr	5500.00	5687.00	5698.56	5694.79	5699.06	0.001752	5.73	1016.35	194.28	0.37
Main Reach	8470	550 cfs	550.00	5687.00	5691.92	5690.28	5692.04	0.001814	2.80	196.50	79.07	0.31
Main Reach	8470	110 cfs	110.00	5687.00	5689.72	5688.38	5689.79	0.003891	2.15	51.14	53.12	0.39
Main Reach	8430	100yr	5500.00	5686.90	5698.28	5695.60	5698.96	0.002774	6.70	898.75	206.57	0.46
Main Reach	8430	550 cfs	550.00	5686.90	5691.64	5690.57	5691.91	0.005037	4.23	130.12	59.77	0.51
Main Reach	8430	110 cfs	110.00	5686.90	5689.42	5688.27	5689.59	0.006231	3.30	33.30	23.81	0.49
Main Reach	8350	100yr	5500.00	5686.50	5697.66	5695.58	5698.66	0.004147	8.07	738.58	179.51	0.56
Main Reach	8350	550 cfs	550.00	5686.50	5690.88	5690.28	5691.35	0.009496	5.53	99.38	48.25	0.68
Main Reach	8350	110 cfs	110.00	5686.50	5688.82	5687.87	5689.05	0.007282	3.84	28.66	17.48	0.53
Main Reach	8200	100yr	5500.00	5685.70	5697.57	5693.65	5698.10	0.001971	5.89	972.35	191.68	0.40
Main Reach	8200	550 cfs	550.00	5685.70	5690.74	5688.39	5690.85	0.001209	2.66	207.03	69.12	0.27
Main Reach	8200	110 cfs	110.00	5685.70	5688.79	5686.84	5688.81	0.000445	1.20	91.72	47.75	0.15
Main Reach	8000	100yr	5500.00	5685.40	5696.74	5693.99	5697.56	0.003438	7.30	783.34	207.95	0.51
Main Reach	8000	550 cfs	550.00	5685.40	5690.28	5688.87	5690.48	0.003071	3.59	153.25	62.10	0.40
Main Reach	8000	110 cfs	110.00	5685.40	5688.62	5686.77	5688.66	0.001455	1.71	64.46	44.62	0.25
Main Reach	7924	100yr	5500.00	5685.00	5696.14	5694.13	5697.22	0.005153	8.35	659.69	128.54	0.61
Main Reach	7924	550 cfs	550.00	5685.00	5689.71	5689.12	5690.10	0.008167	4.98	110.36	56.55	0.63
Main Reach	7924	110 cfs	110.00	5685.00	5688.33	5687.96	5688.45	0.007003	2.73	40.28	44.80	0.51
Main Reach	7750	100yr	5500.00	5684.50	5695.69	5692.42	5696.54	0.002555	7.77	886.03	173.34	0.46
Main Reach	7750	550 cfs	550.00	5684.50	5688.95	5687.99	5689.15	0.003515	3.55	157.14	77.68	0.43
Main Reach	7750	110 cfs	110.00	5684.50	5686.85	5685.88	5687.05	0.009066	3.61	30.50	25.60	0.58
Main Reach	7525	100yr	5500.00	5683.90	5695.33	5691.54	5695.93	0.002261	6.21	890.21	140.14	0.42
Main Reach	7525	550 cfs	550.00	5683.90	5688.51	5686.53	5688.63	0.001506	2.81	195.90	69.16	0.29
Main Reach	7525	110 cfs	110.00	5683.90	5686.40	5684.82	5686.43	0.001157	1.56	70.70	49.31	0.23
Main Reach	7375	100yr	5500.00	5683.70	5694.82	5691.40	5695.54	0.002795	6.84	867.43	212.42	0.46
Main Reach	7375	550 cfs	550.00	5683.70	5688.29	5686.34	5688.41	0.001441	2.82	195.37	66.20	0.29
Main Reach	7375	110 cfs	110.00	5683.70	5686.23	5684.60	5686.26	0.001101	1.50	73.38	52.27	0.22
Main Reach	7200	100yr	5500.00	5683.30	5693.72	5691.46	5694.91	0.003995	8.88	675.78	119.32	0.57
Main Reach	7200	550 cfs	550.00	5683.30	5687.91	5686.38	5688.08	0.002426	3.39	162.35	61.59	0.37
Main Reach	7200	110 cfs	110.00	5683.30	5685.94	5684.48	5686.00	0.002145	1.95	56.44	44.46	0.30
Main Reach	7075	100yr	5500.00	5683.00	5693.24	5691.29	5694.34	0.004783	8.45	661.13	122.79	0.59
Main Reach	7075	550 cfs	550.00	5683.00	5687.42	5686.47	5687.67	0.004619	4.02	136.86	63.76	0.48
Main Reach	7075	110 cfs	110.00	5683.00	5685.25	5684.38	5685.48	0.009779	3.91	28.11	21.56	0.60
Main Reach	6925	100yr	5500.00	5682.50	5693.05	5690.32	5693.63	0.002980	6.09	905.20	182.55	0.47
Main Reach	6925	550 cfs	550.00	5682.50	5686.86	5685.45	5687.06	0.003421	3.63	151.58	67.61	0.43
Main Reach	6925	110 cfs	110.00	5682.50	5684.64	5683.68	5684.74	0.002743	2.47	44.55	28.97	0.35
Main Reach	6746	100yr	5500.00	5682.00	5692.72	5689.18	5693.22	0.001642	5.94	1155.71	244.84	0.37
Main Reach	6746	550 cfs	550.00	5682.00	5686.46	5684.77	5686.58	0.001997	2.78	197.60	88.20	0.33
Main Reach	6746	110 cfs	110.00	5682.00	5684.31	5682.92	5684.37	0.001528	1.91	57.50	35.63	0.27
Main Reach	6561	100yr	5500.00	5681.40	5692.26	5689.27	5692.87	0.002120	6.65	1084.42	249.60	0.42
Main Reach	6561	550 cfs	550.00	5681.40	5685.98	5684.54	5686.15	0.002670	3.28	167.59	155.03	0.38
Main Reach	6561	110 cfs	110.00	5681.40	5683.86	5682.66	5683.95	0.003621	2.40	45.82	38.82	0.39
Main Reach	6448	100yr	5500.00	5681.25	5691.73	5689.48	5692.56	0.003274	7.64	905.79	234.30	0.51
Main Reach	6448	550 cfs	550.00	5681.25	5685.54	5684.36	5685.78	0.003775	3.97	138.67	128.12	0.45
Main Reach	6448	110 cfs	110.00	5681.25	5683.38	5682.46	5683.51	0.004096	2.94	37.42	59.68	0.42
Main Reach	6259	100yr	5500.00	5681.10	5690.35	5688.75	5691.71	0.005615	9.48	683.83	270.77	0.65
Main Reach	6259	550 cfs	550.00	5681.10	5684.83	5683.55	5685.06	0.003829	3.92	140.23	169.88	0.45
Main Reach	6259	110 cfs	110.00	5681.10	5682.90	5681.93	5682.97	0.001978	2.19	50.14	30.69	0.30

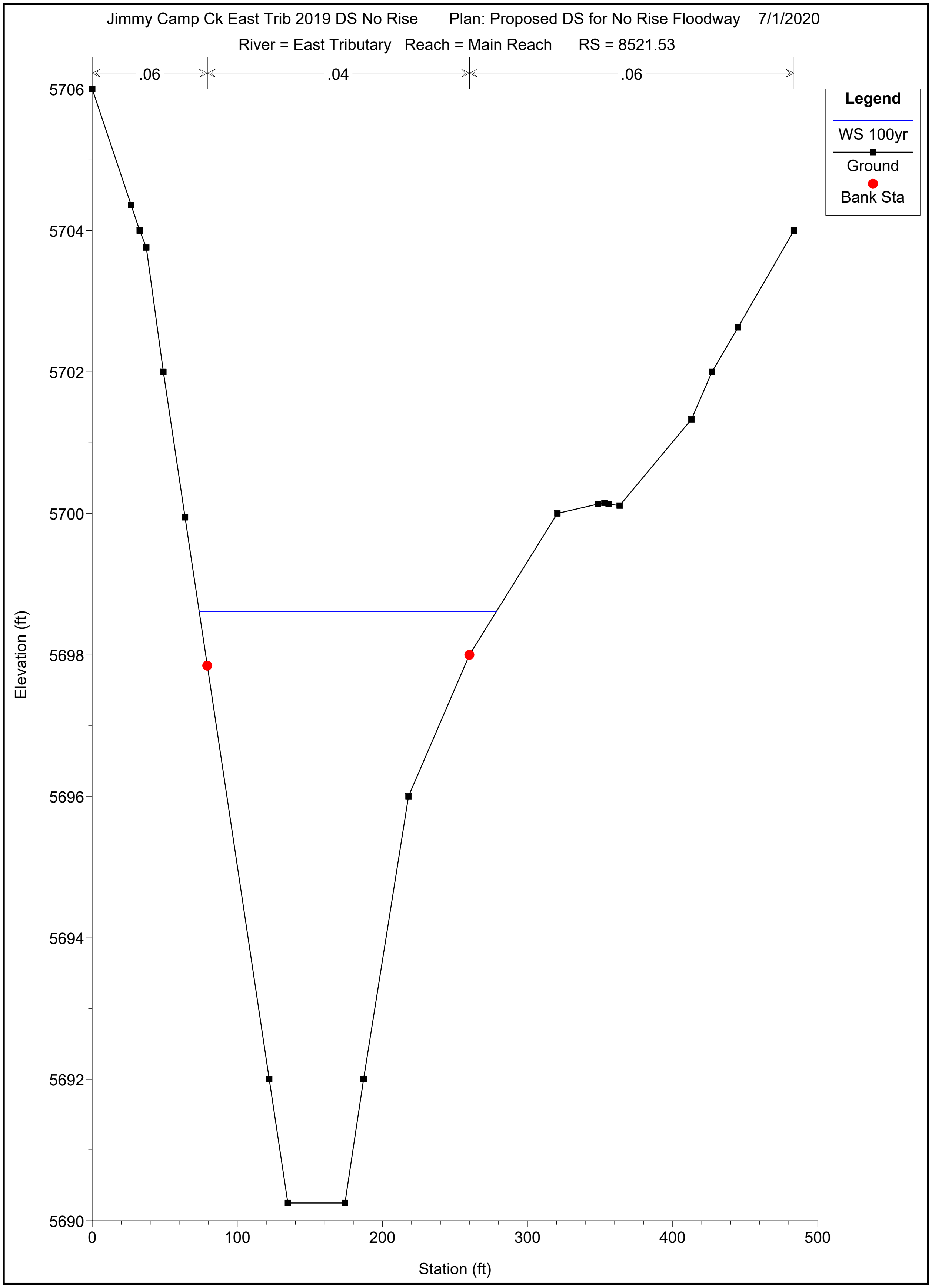
HEC-RAS Plan: Proposed No Rise Floodway   River: East Tributary   Reach: Main Reach (Continued)

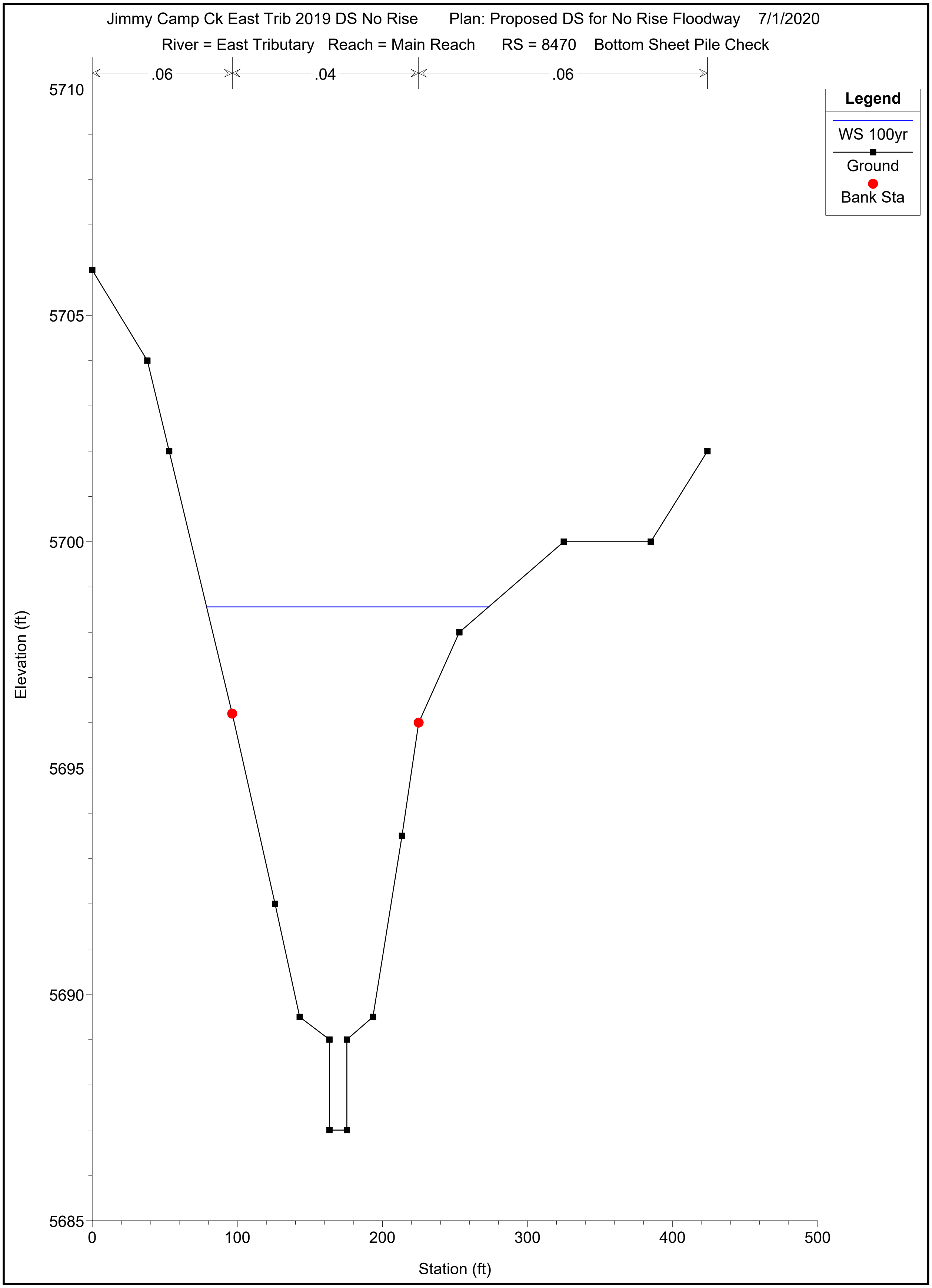
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Main Reach	6150	100yr	5500.00	5680.80	5690.22	5687.75	5691.11	0.003312	7.62	782.43	279.49	0.51
Main Reach	6150	550 cfs	550.00	5680.80	5684.59	5683.10	5684.73	0.002132	3.07	179.22	141.96	0.35
Main Reach	6150	110 cfs	110.00	5680.80	5682.69	5681.83	5682.74	0.002104	1.89	58.26	49.74	0.31
Main Reach	6000	100yr	5500.00	5680.20	5689.86	5687.31	5690.62	0.002729	7.58	958.89	199.31	0.47
Main Reach	6000	550 cfs	550.00	5680.20	5684.31	5682.22	5684.43	0.001780	2.85	195.87	84.31	0.31
Main Reach	6000	110 cfs	110.00	5680.20	5682.55	5680.89	5682.58	0.000636	1.35	81.32	42.86	0.17
Main Reach	5865	100yr	5500.00	5679.90	5688.91	5687.45	5690.09	0.004942	9.26	744.18	166.85	0.61
Main Reach	5865	550 cfs	550.00	5679.90	5683.86	5682.87	5684.08	0.004060	3.70	148.54	71.57	0.45
Main Reach	5865	110 cfs	110.00	5679.90	5682.32	5681.09	5682.39	0.004308	2.18	50.36	55.33	0.40
Main Reach	5710	100yr	5500.00	5679.40	5688.33	5686.36	5689.34	0.004148	8.25	757.46	193.68	0.56
Main Reach	5710	550 cfs	550.00	5679.40	5683.30	5682.33	5683.47	0.003589	3.35	164.35	84.91	0.42
Main Reach	5710	110 cfs	110.00	5679.40	5681.66	5680.41	5681.75	0.004053	2.40	45.81	40.76	0.40
Main Reach	5436	100yr	5500.00	5678.40	5687.69	5685.11	5688.34	0.002671	6.59	913.11	181.17	0.45
Main Reach	5436	550 cfs	550.00	5678.40	5682.39	5681.27	5682.54	0.003179	3.09	177.91	95.04	0.40
Main Reach	5436	110 cfs	110.00	5678.40	5680.91	5679.38	5680.95	0.002134	1.76	62.62	56.43	0.29
Main Reach	5236	100yr	5500.00	5677.70	5686.92	5684.65	5687.74	0.003345	7.37	818.51	160.27	0.51
Main Reach	5236	550 cfs	550.00	5677.70	5681.71	5680.73	5681.87	0.003500	3.28	167.53	87.46	0.42
Main Reach	5236	110 cfs	110.00	5677.70	5680.20	5679.08	5680.28	0.006083	2.23	49.43	69.39	0.46
Main Reach	5032	100yr	5500.00	5677.10	5686.58	5683.78	5687.19	0.001768	6.85	1057.87	202.01	0.45
Main Reach	5032	550 cfs	550.00	5677.10	5681.22	5680.10	5681.35	0.001840	2.98	206.22	113.63	0.37
Main Reach	5032	110 cfs	110.00	5677.10	5679.46	5678.11	5679.53	0.002426	2.23	49.38	45.05	0.38
Main Reach	4836	100yr	5500.00	5676.60	5686.67	5682.36	5686.90	0.000546	3.96	1644.01	311.71	0.26
Main Reach	4836	550 cfs	550.00	5676.60	5681.13	5679.33	5681.17	0.000455	1.60	342.68	154.32	0.19
Main Reach	4836	110 cfs	110.00	5676.60	5679.10	5677.61	5679.13	0.001671	1.44	76.43	106.49	0.30
Main Reach	4500	100yr	5500.00	5676.10	5686.60	5682.49	5686.69	0.000494	3.01	2650.83	482.65	0.20
Main Reach	4500	550 cfs	550.00	5676.10	5680.66	5679.11	5680.81	0.004107	3.23	205.50	217.10	0.45
Main Reach	4500	110 cfs	110.00	5676.10	5678.16	5677.43	5678.29	0.004099	2.87	38.31	27.64	0.43

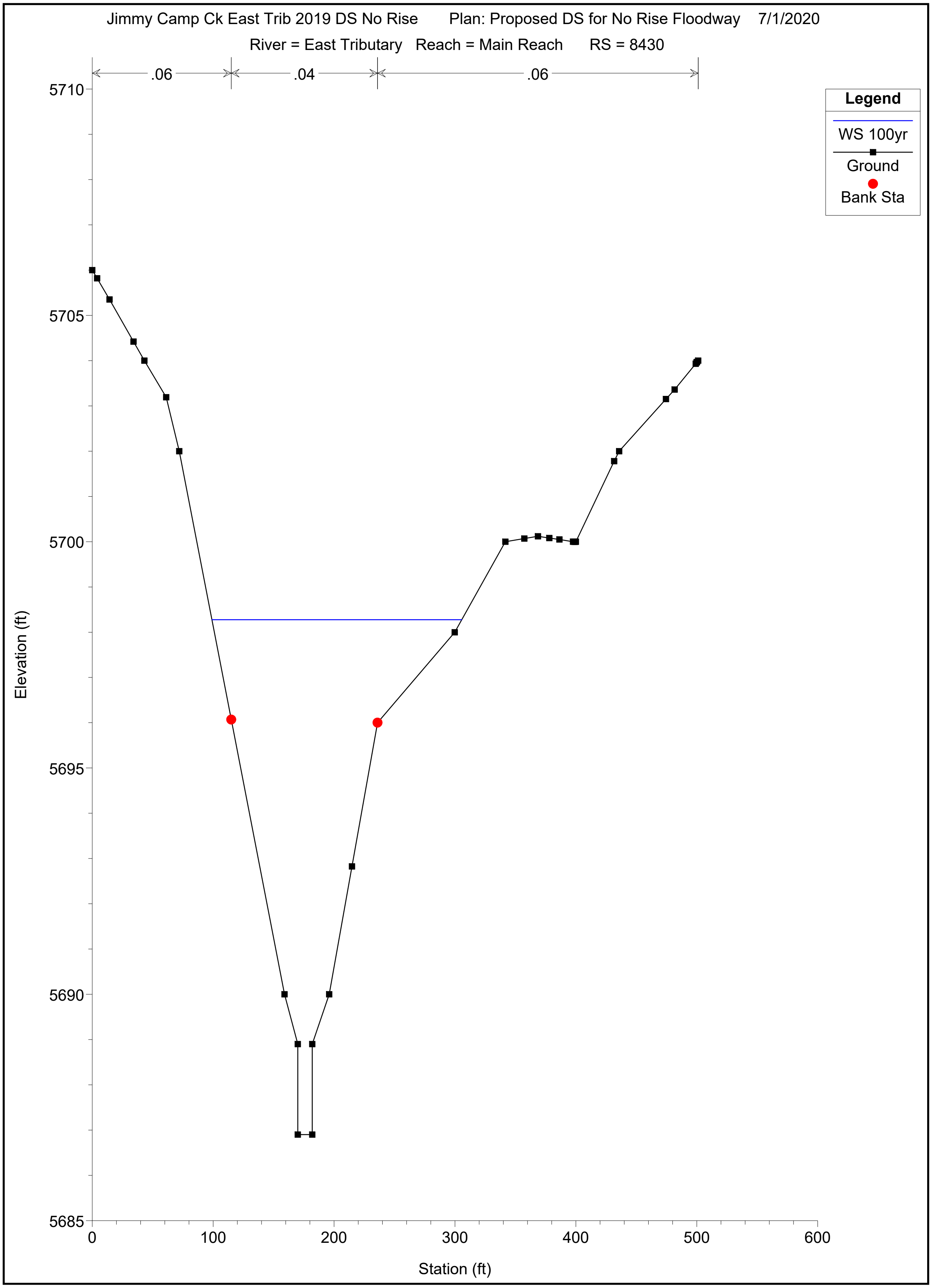

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 9000
.06
.035
.03
.035
.045
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5710
5708
5706
5704
5702
5700
5698
5696
5694
5692
0
100
200
300
400
500
600
Station (ft)

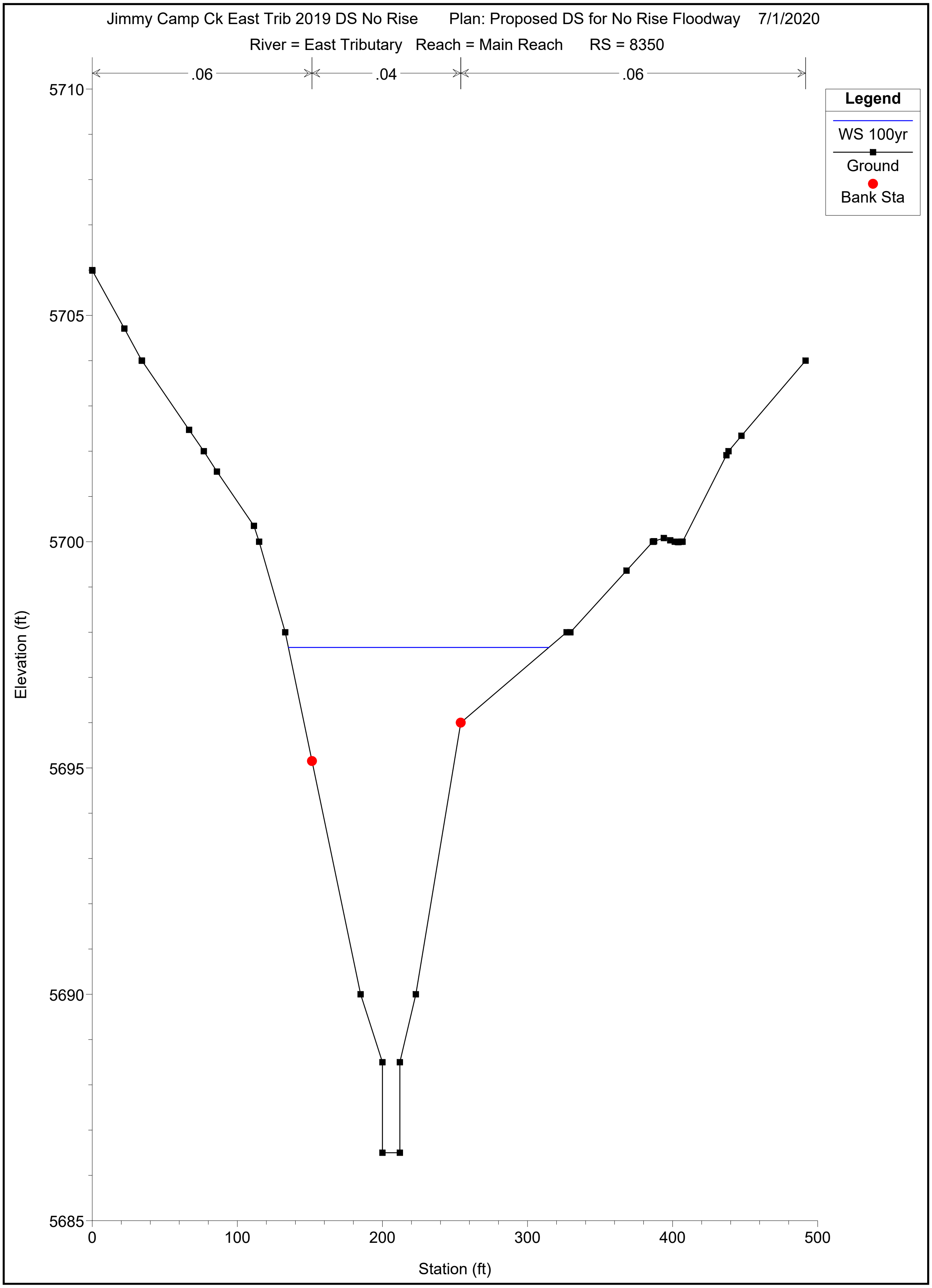

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 8850
.06
.045
.035
.03
.035
.045
Legend
WS 100yr
Ground
Ineff
Bank Sta
Elevation (ft)
5708
5706
5704
5702
5700
5698
5696
5694
5692
0
200
400
600
800
1000
Station (ft)

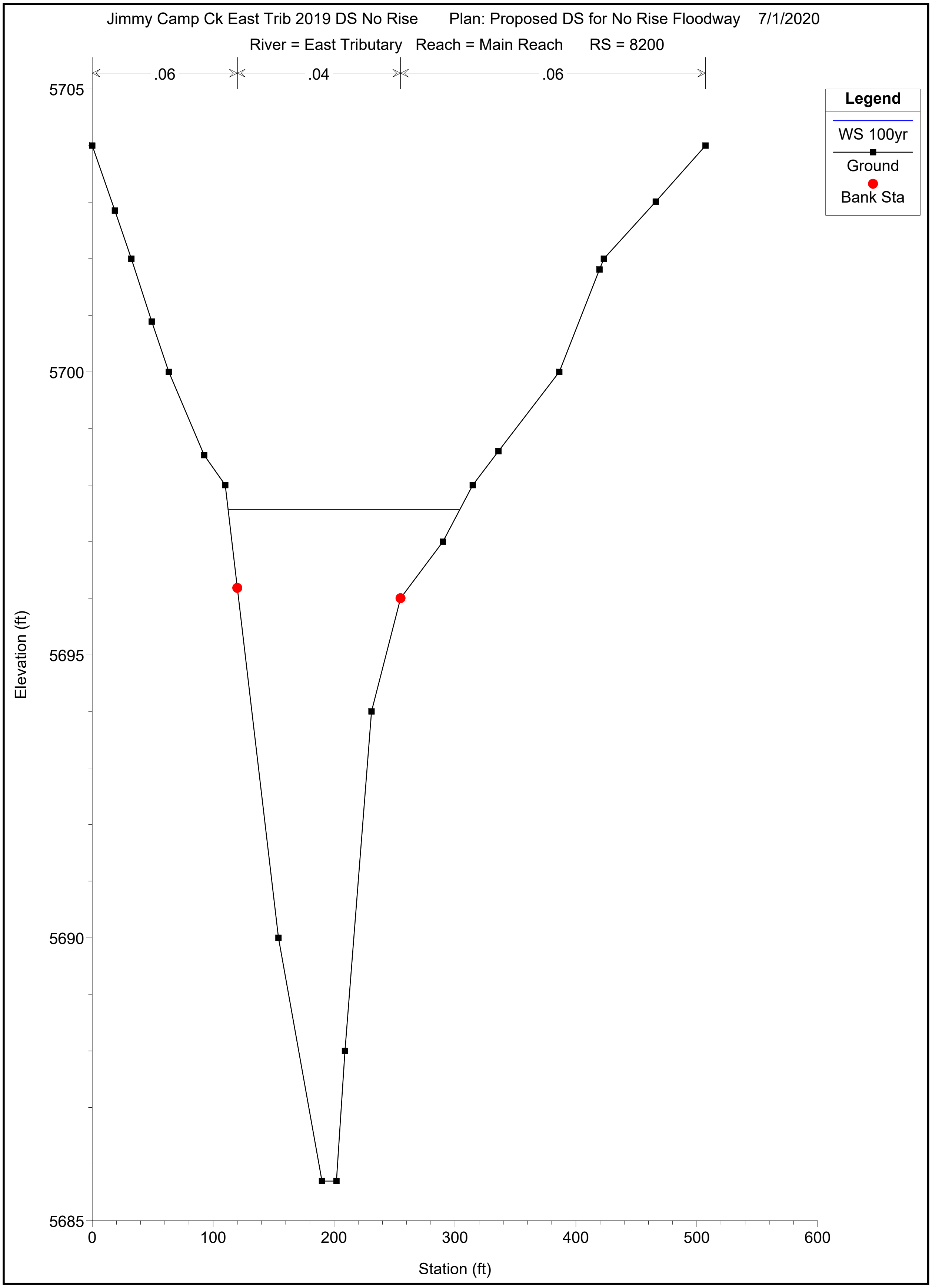

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 8650
.06
.045
.024
.035
Legend
WS 100yr
Ground
Bank Sta
5708
5706
5704
5702
5700
5698
5696
5694
5692
5690
Elevation (ft)
0
100
200
300
400
500
Station (ft)

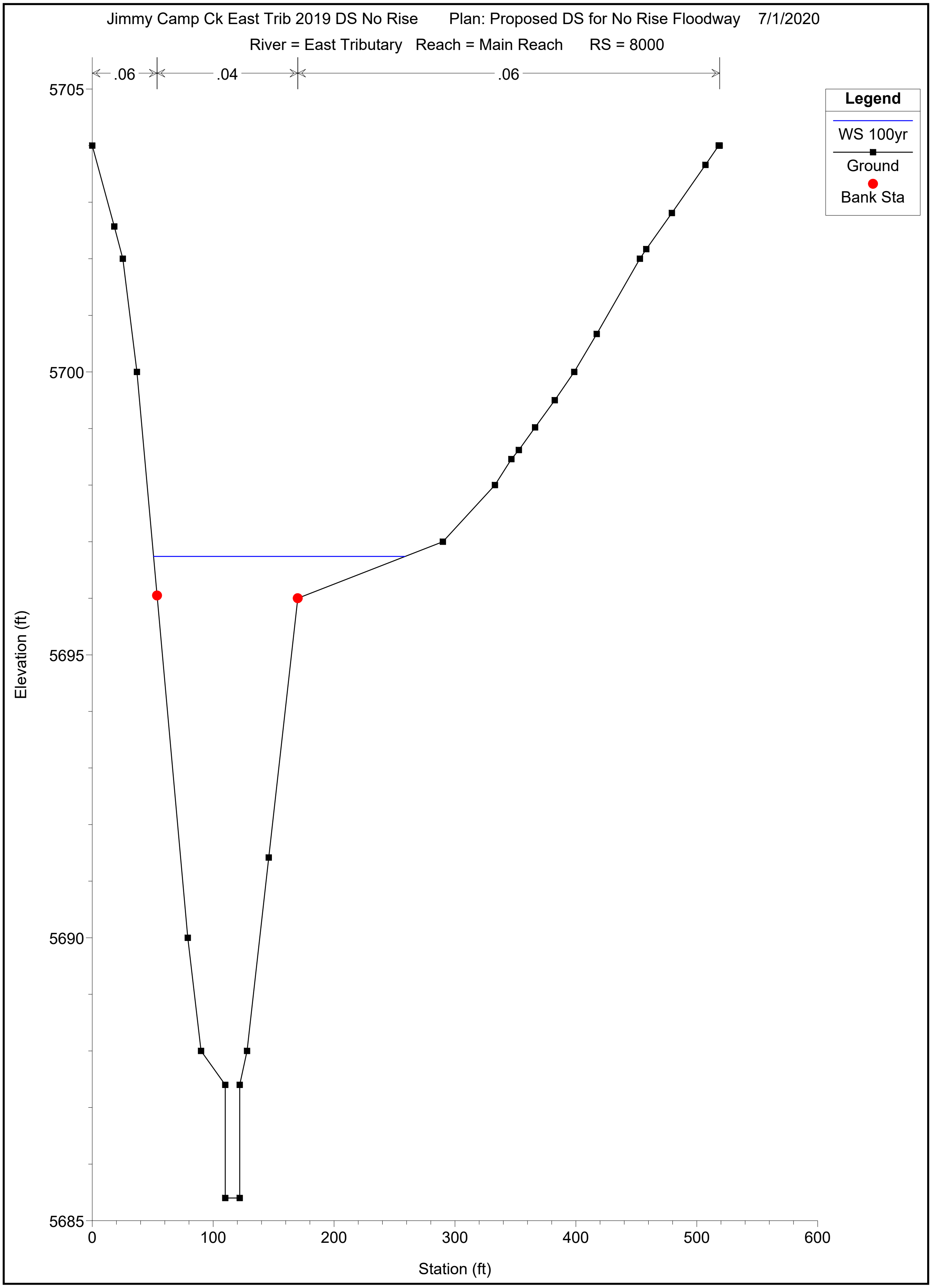
Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 8521.53
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5706
5704
5702
5700
5698
5696
5694
5692
5690
0
100
200
300
400
500
Station (ft)

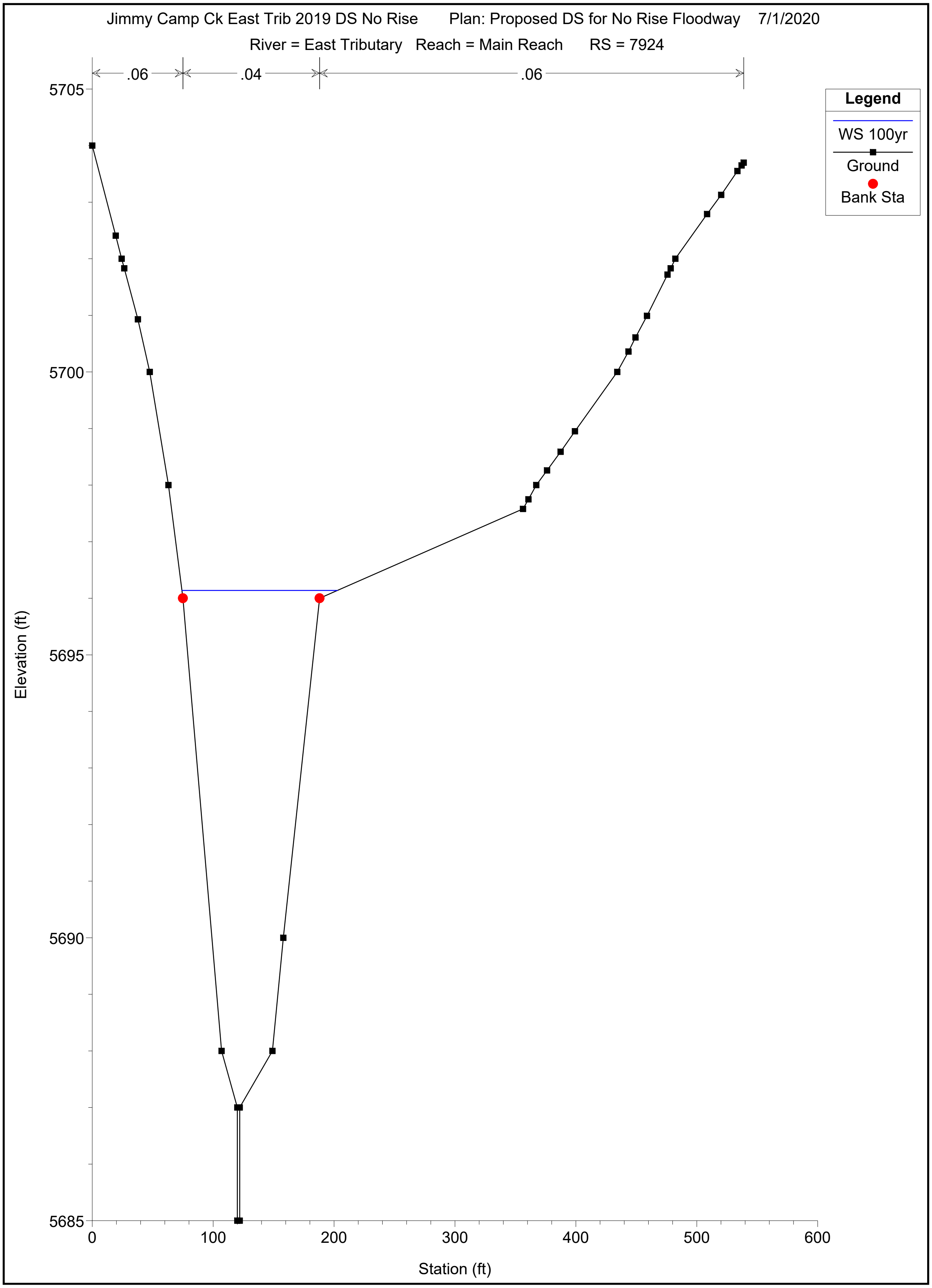

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 8470 Bottom Sheet Pile Check
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5710
5705
5700
5695
5690
5685
Station (ft)
0
100
200
300
400
500

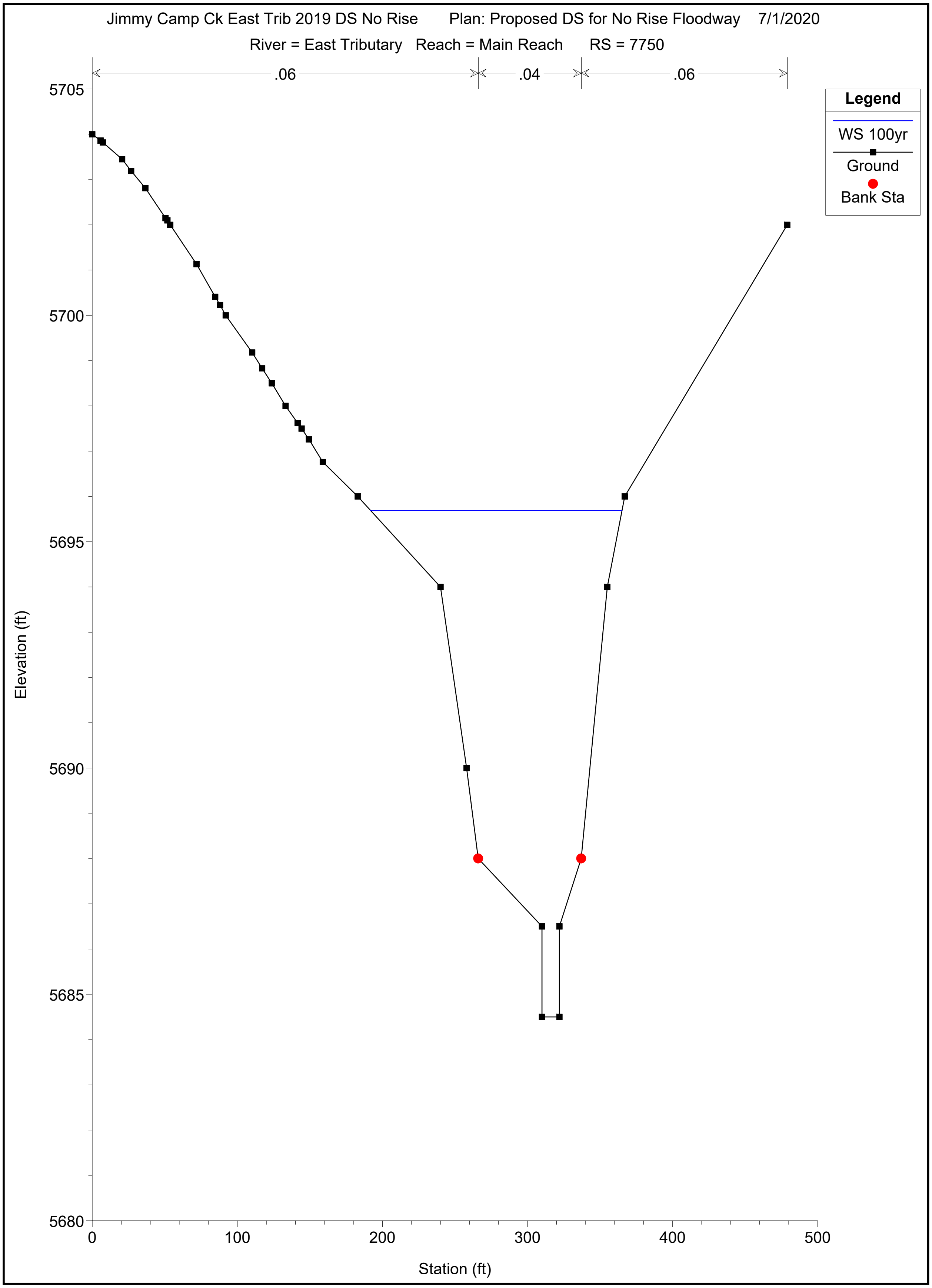
Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 8430
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5710
5705
5700
5695
5690
5685
0
100
200
300
400
500
600
Station (ft)

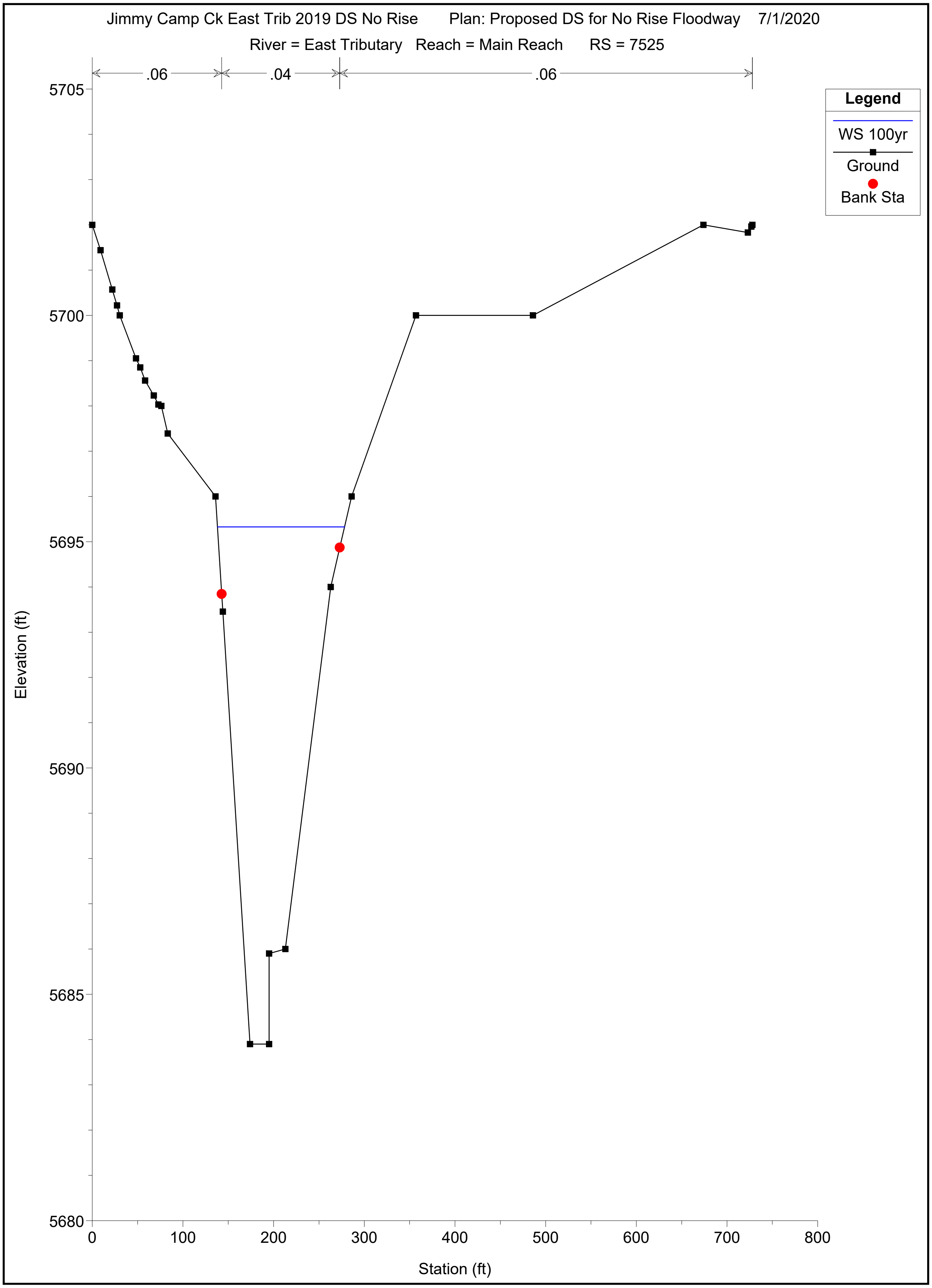

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 8350
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5710
5705
5700
5695
5690
5685
0
100
200
300
400
500
Station (ft)

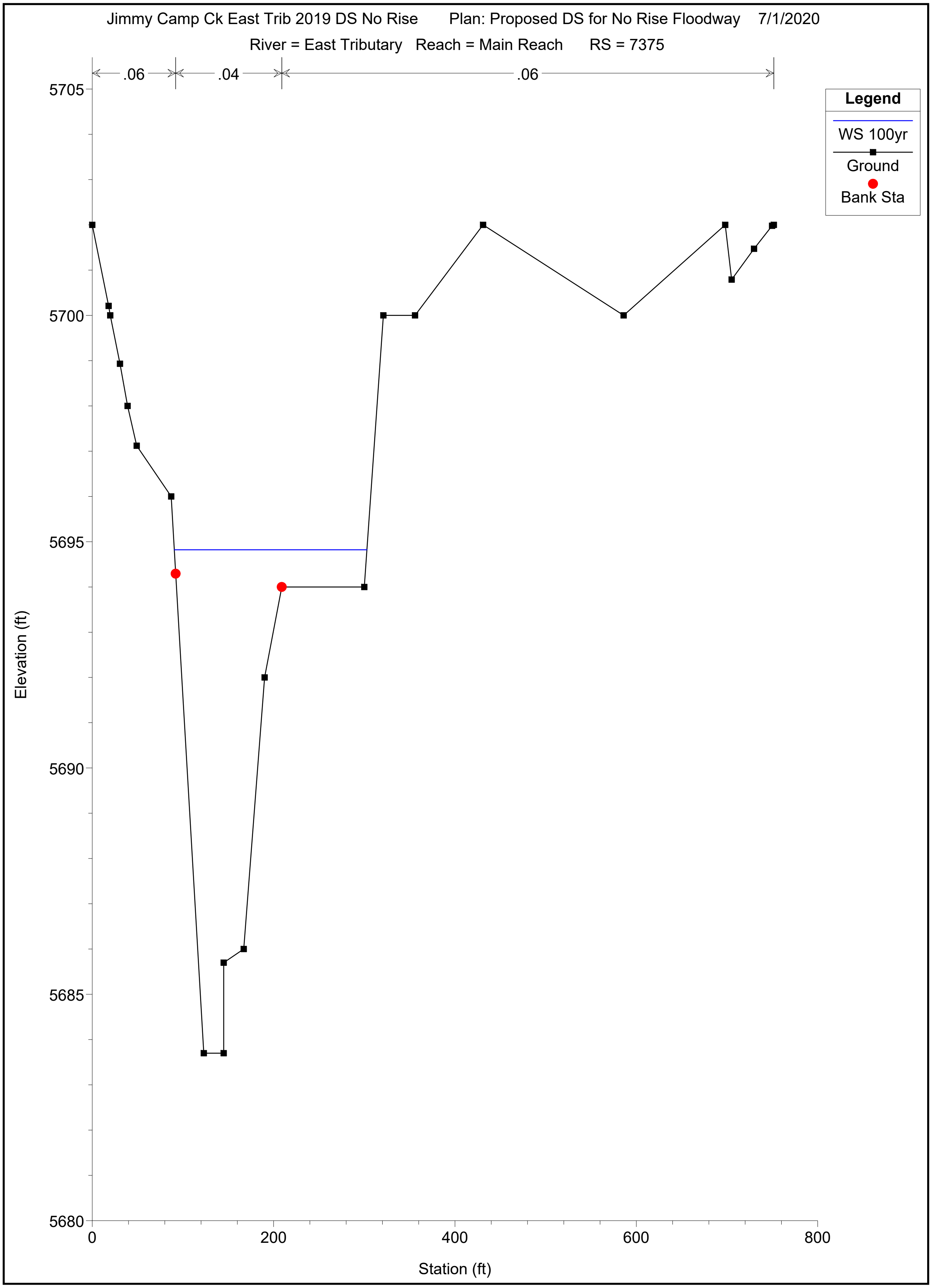

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 8200
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
5705
5700
5695
5690
5685
Elevation (ft)
0
100
200
300
400
500
600
Station (ft)

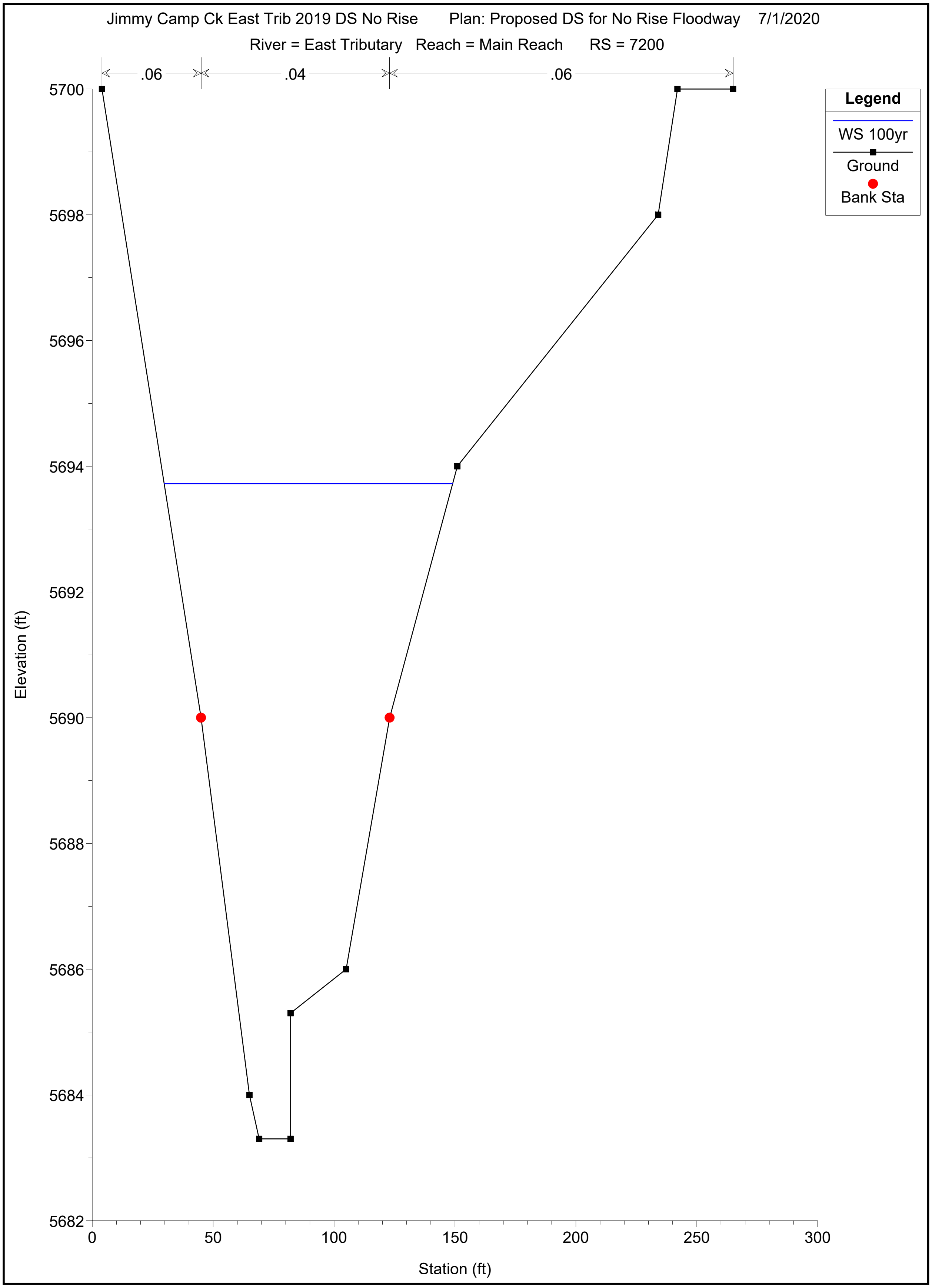

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 8000
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5705
5700
5695
5690
5685
0
100
200
300
400
500
600
Station (ft)

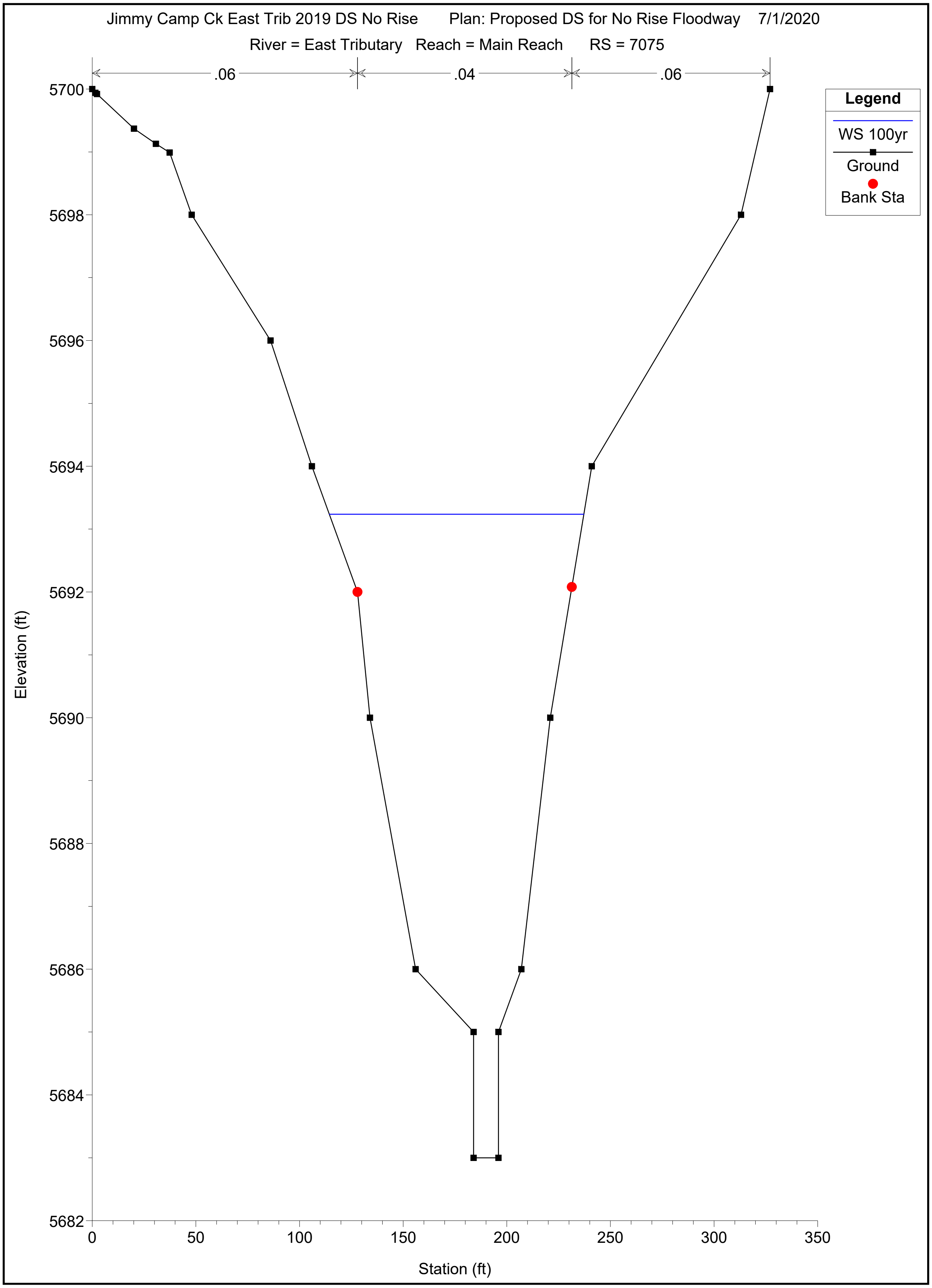

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 7924
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5705
5700
5695
5690
5685
0
100
200
300
400
500
600
Station (ft)

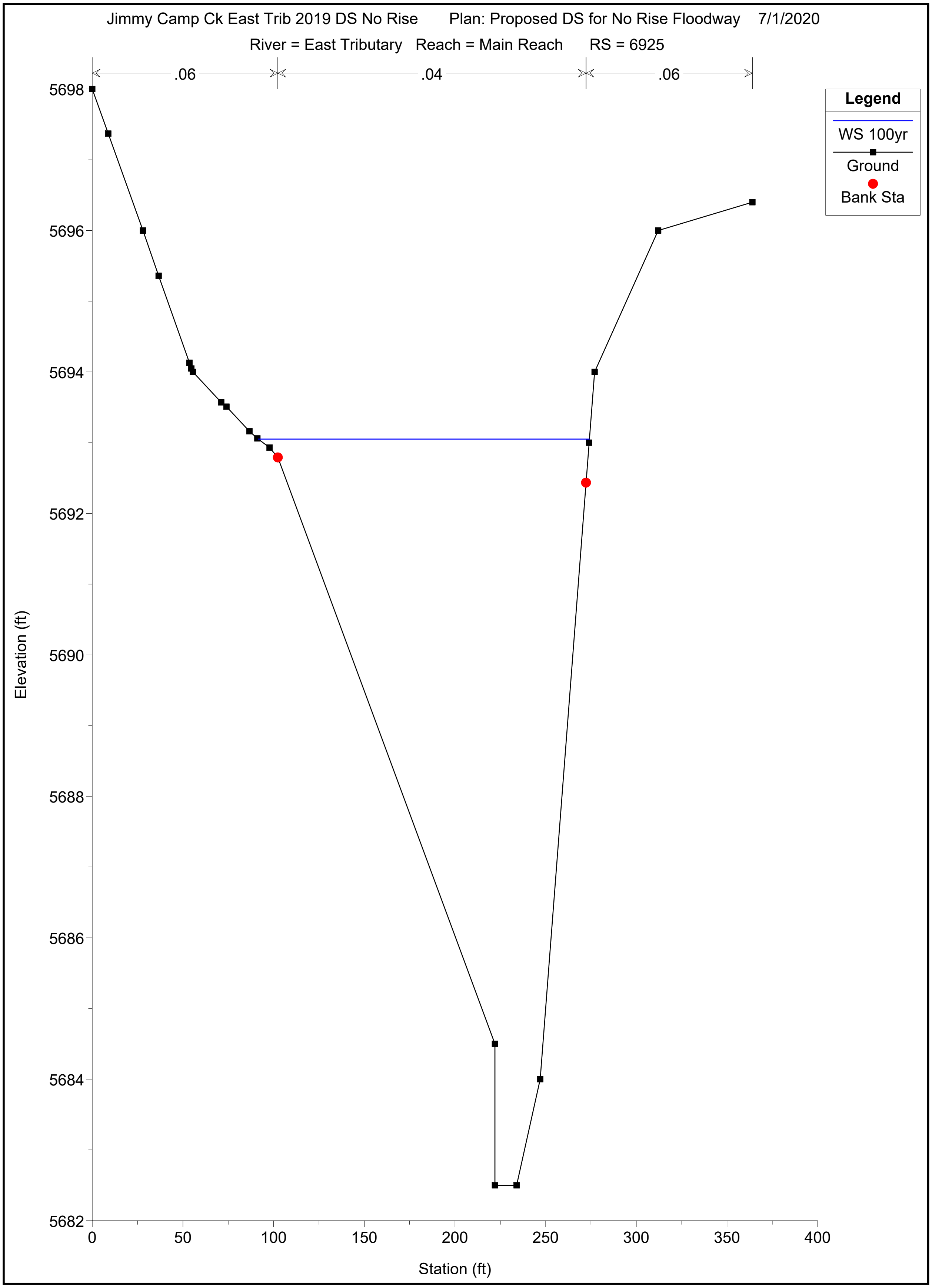

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 7750
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5705
5700
5695
5690
5685
5680
0
100
200
300
400
500
Station (ft)

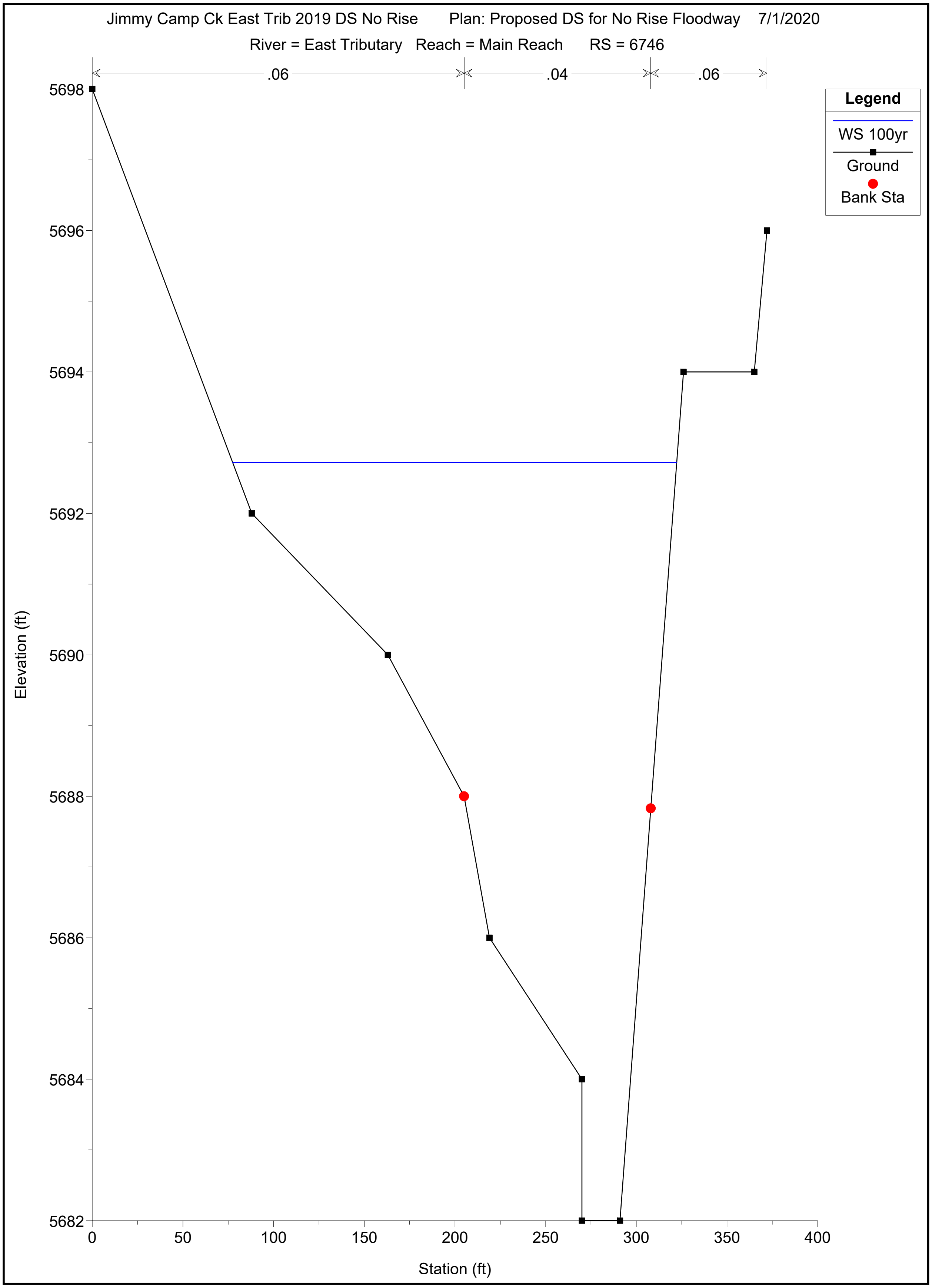

Jimmy Camp Ck East Trib 2019 DS No Rise   Plan: Proposed DS for No Rise Floodway   7/1/2020
River = East Tributary   Reach = Main Reach   RS = 7525
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
5705
5700
5695
5690
5685
5680
Elevation (ft)
0
100
200
300
400
500
600
700
800
Station (ft)


Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 7375
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
5705
5700
5695
5690
5685
5680
Elevation (ft)
0
200
400
600
800
Station (ft)

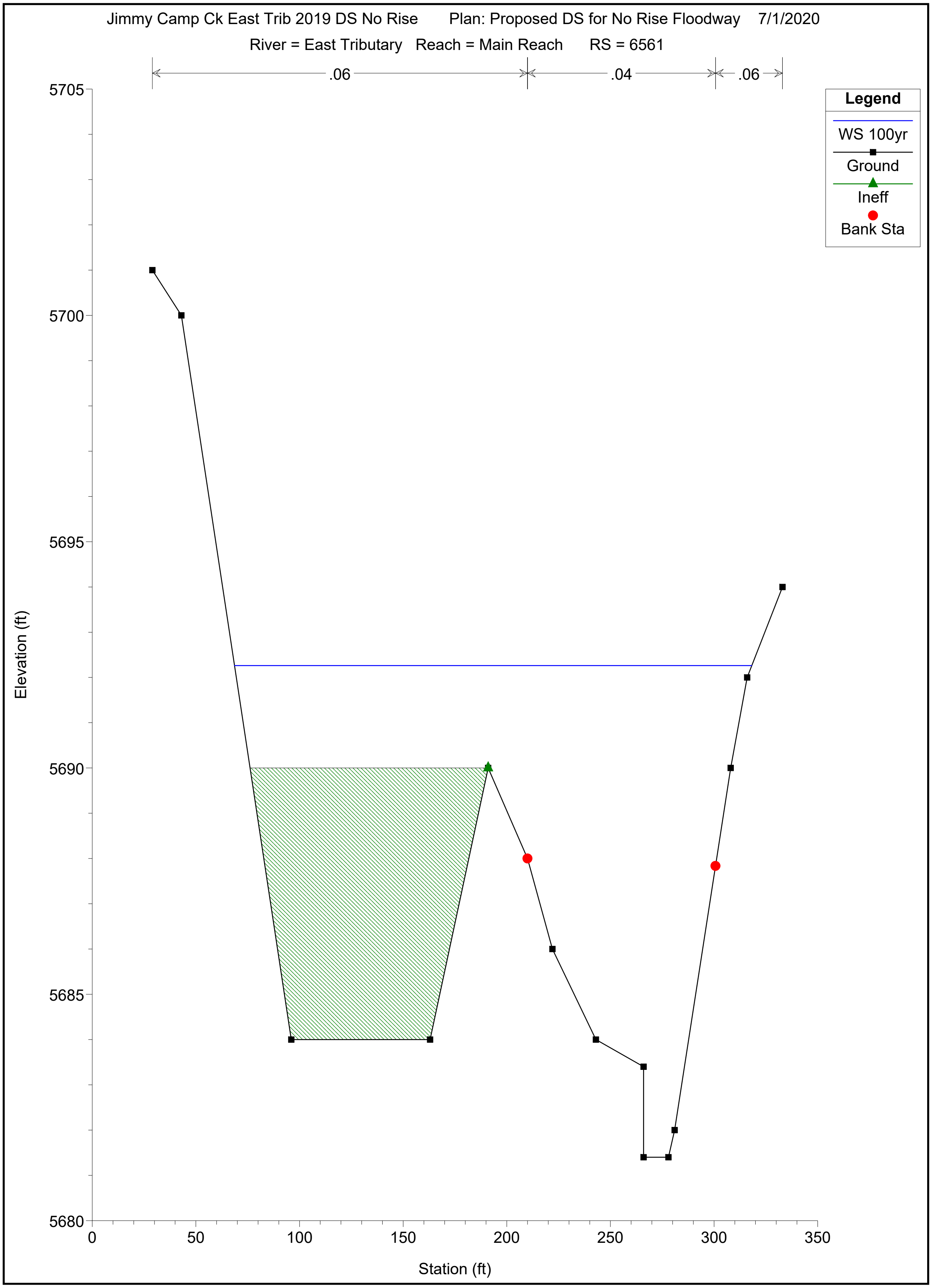
Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 7200
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5700
5698
5696
5694
5692
5690
5688
5686
5684
5682
0
50
100
150
200
250
300
Station (ft)

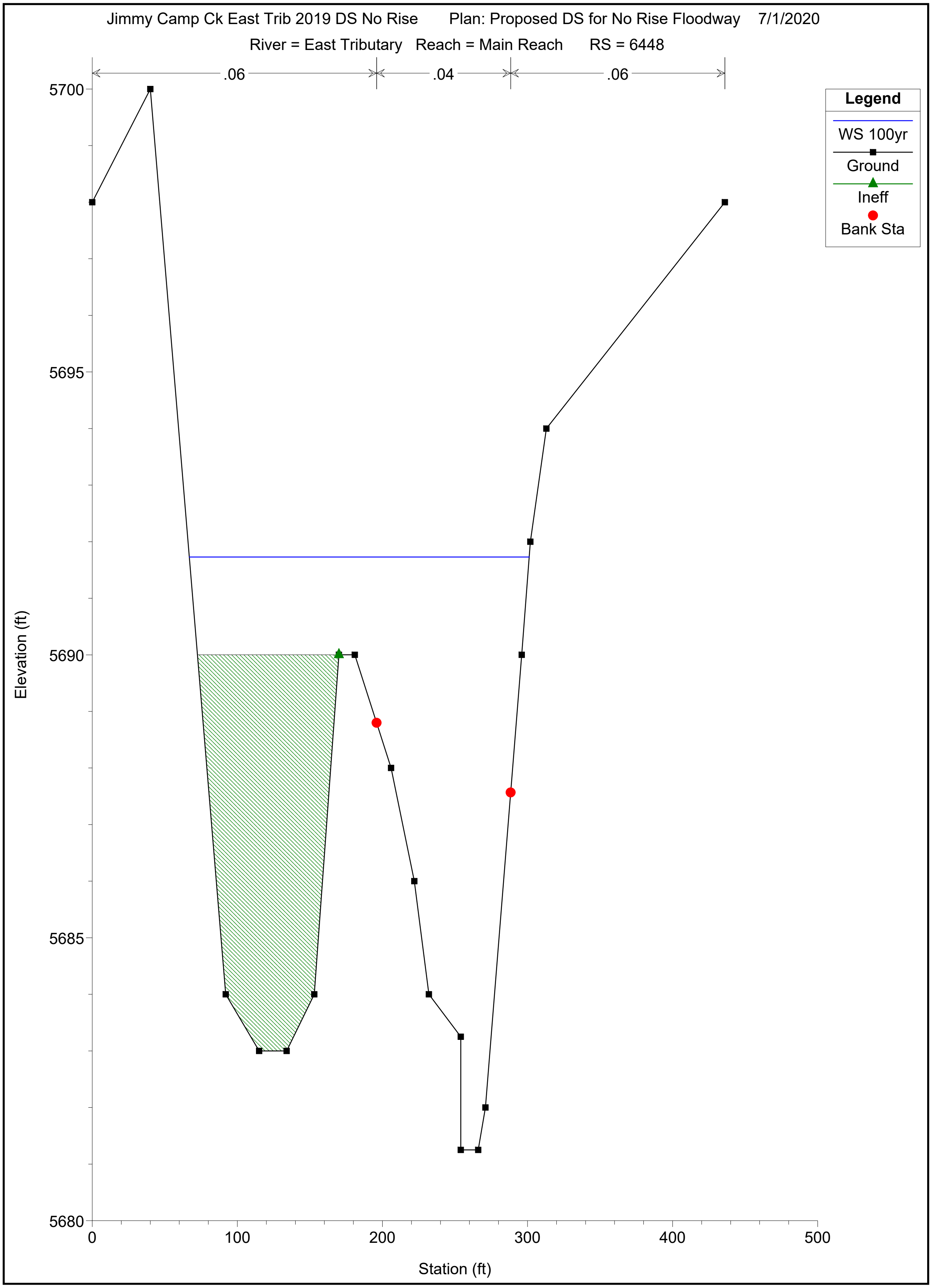

Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 7075
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5700
5698
5696
5694
5692
5690
5688
5686
5684
5682
0
50
100
150
200
250
300
350
Station (ft)


Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 6925
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5698
5696
5694
5692
5690
5688
5686
5684
5682
0
50
100
150
200
250
300
350
400
Station (ft)



Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 6746
.06
.04
.06
Legend
WS 100yr
Ground
Bank Sta
Elevation (ft)
5698
5696
5694
5692
5690
5688
5686
5684
5682
0
50
100
150
200
250
300
350
400
Station (ft)


Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 6561
.06
.04
.06
Legend
WS 100yr
Ground
Ineff
Bank Sta
Elevation (ft)
5705
5700
5695
5690
5685
5680
Station (ft)
0
50
100
150
200
250
300
350


Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
River = East Tributary Reach = Main Reach RS = 6448
.06
.04
.06
Legend
WS 100yr
Ground
Ineff
Bank Sta
5700
5695
5690
5685
5680
Elevation (ft)
0
100
200
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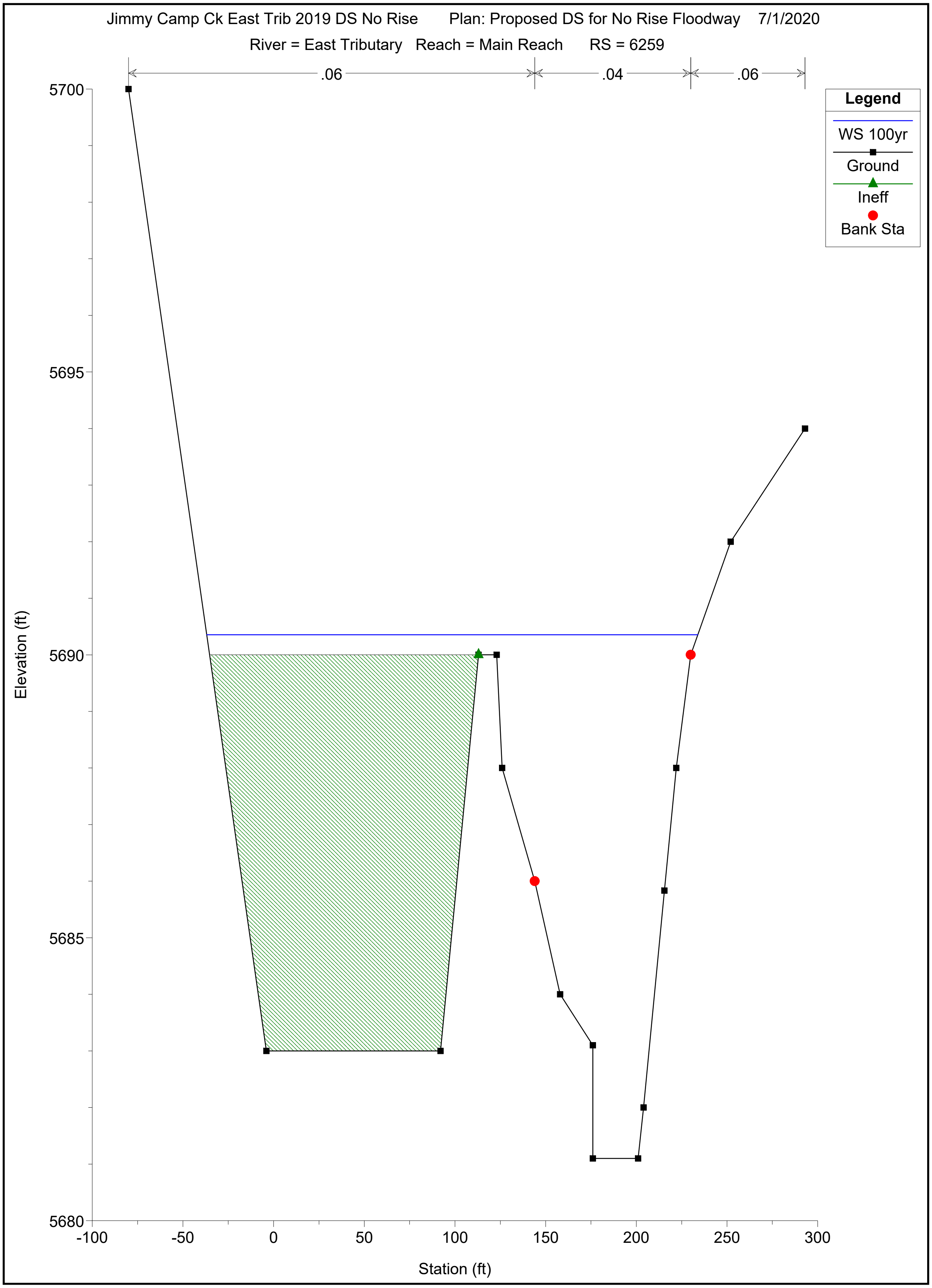
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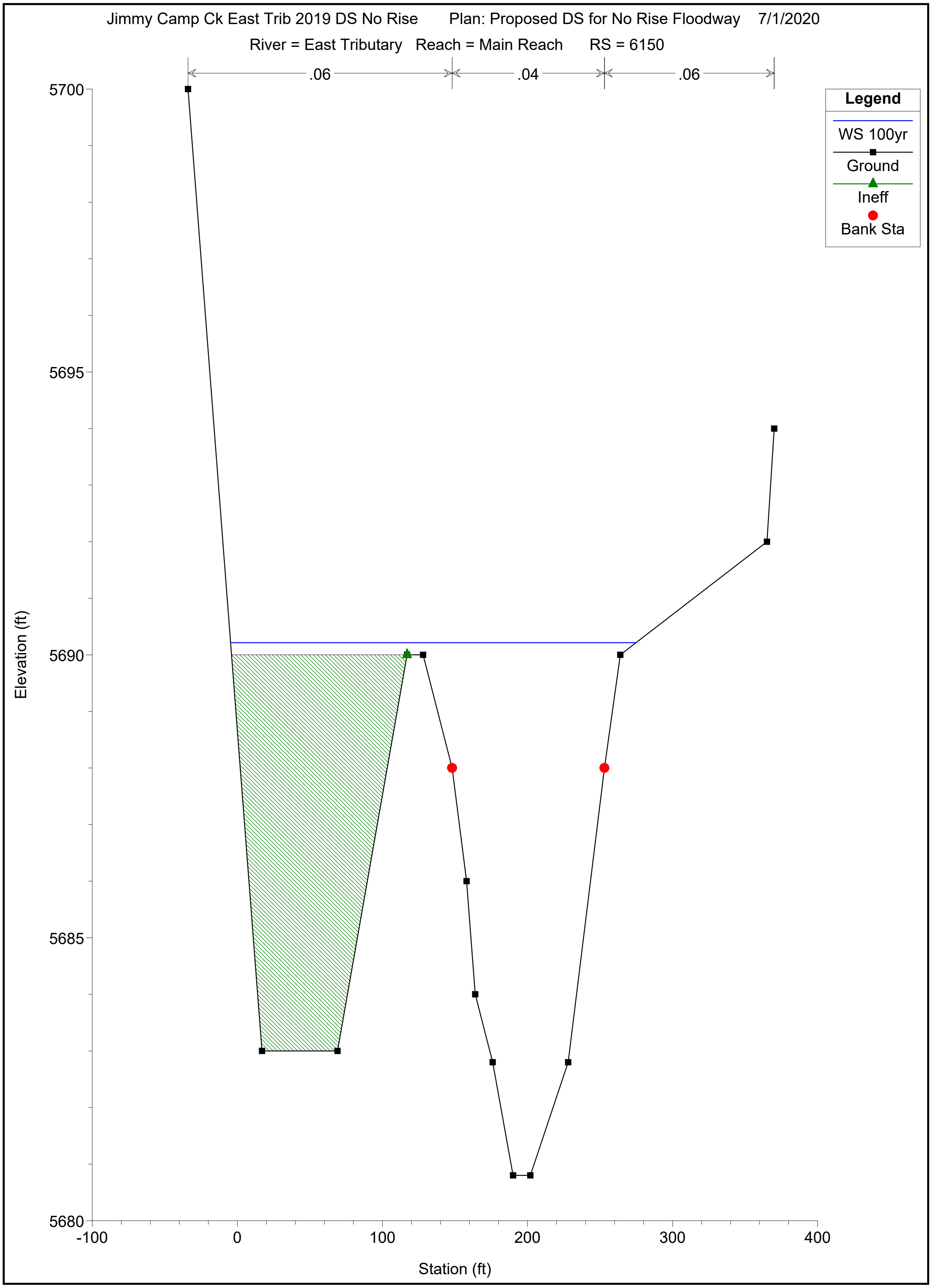
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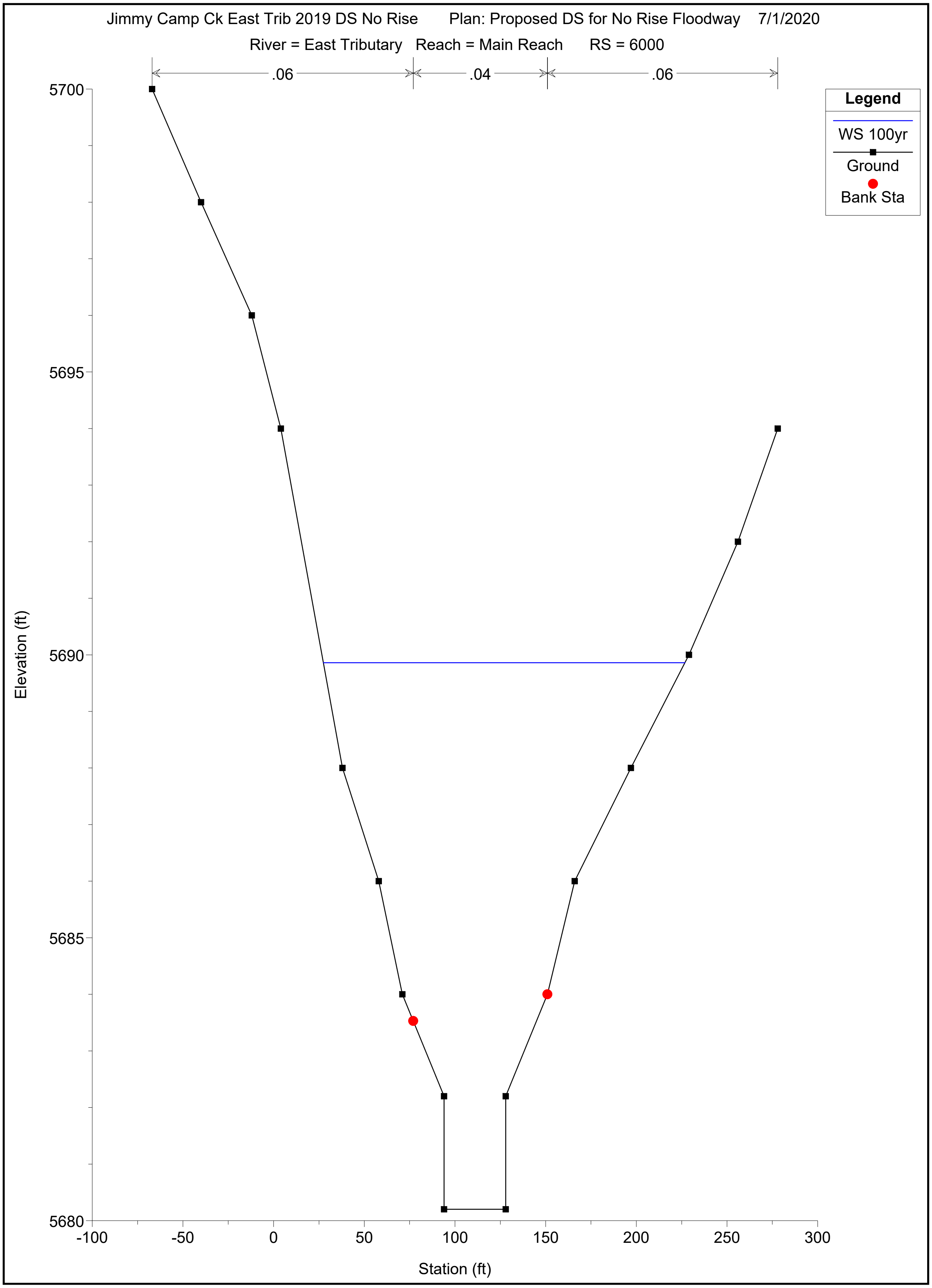Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
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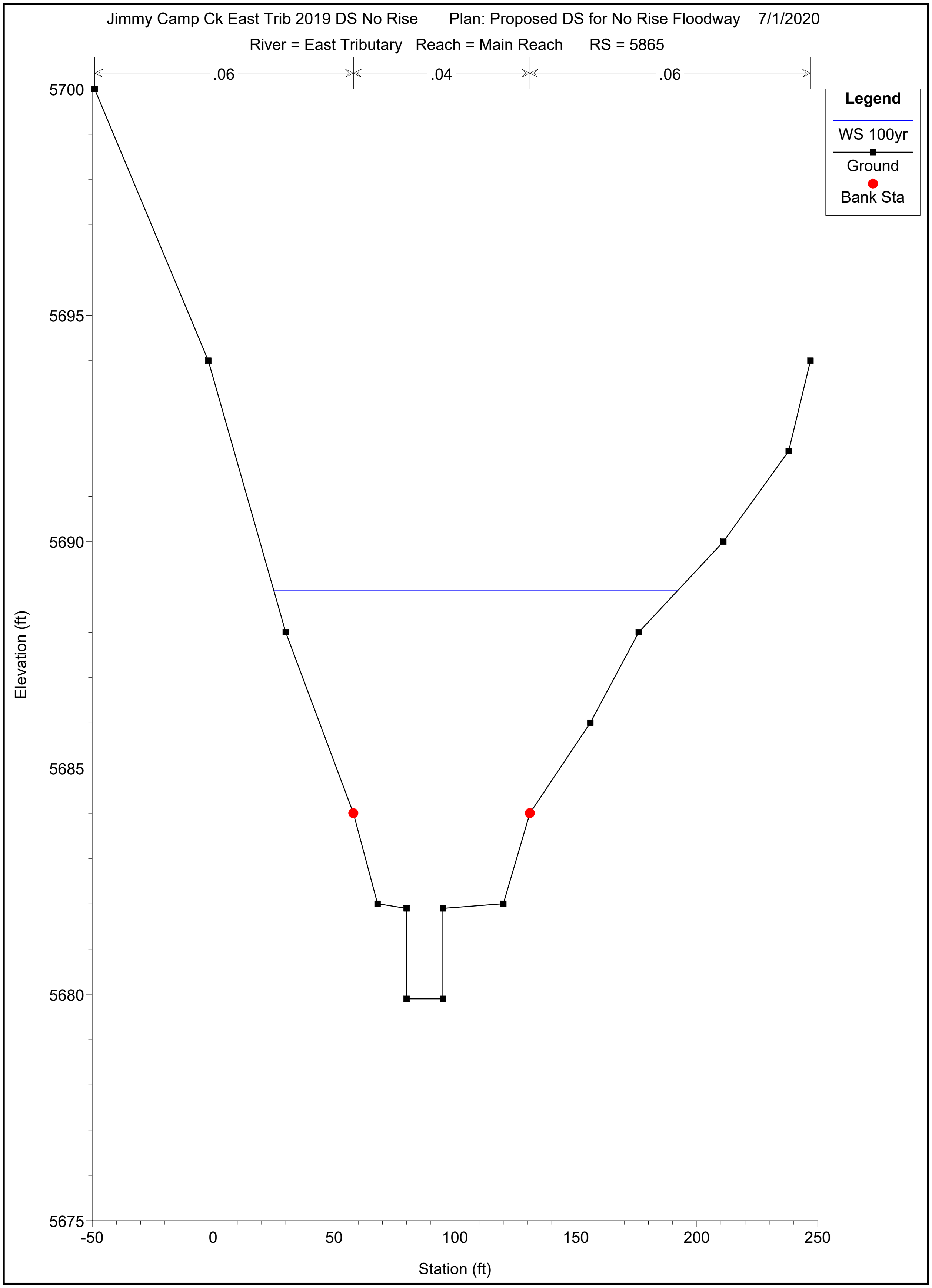
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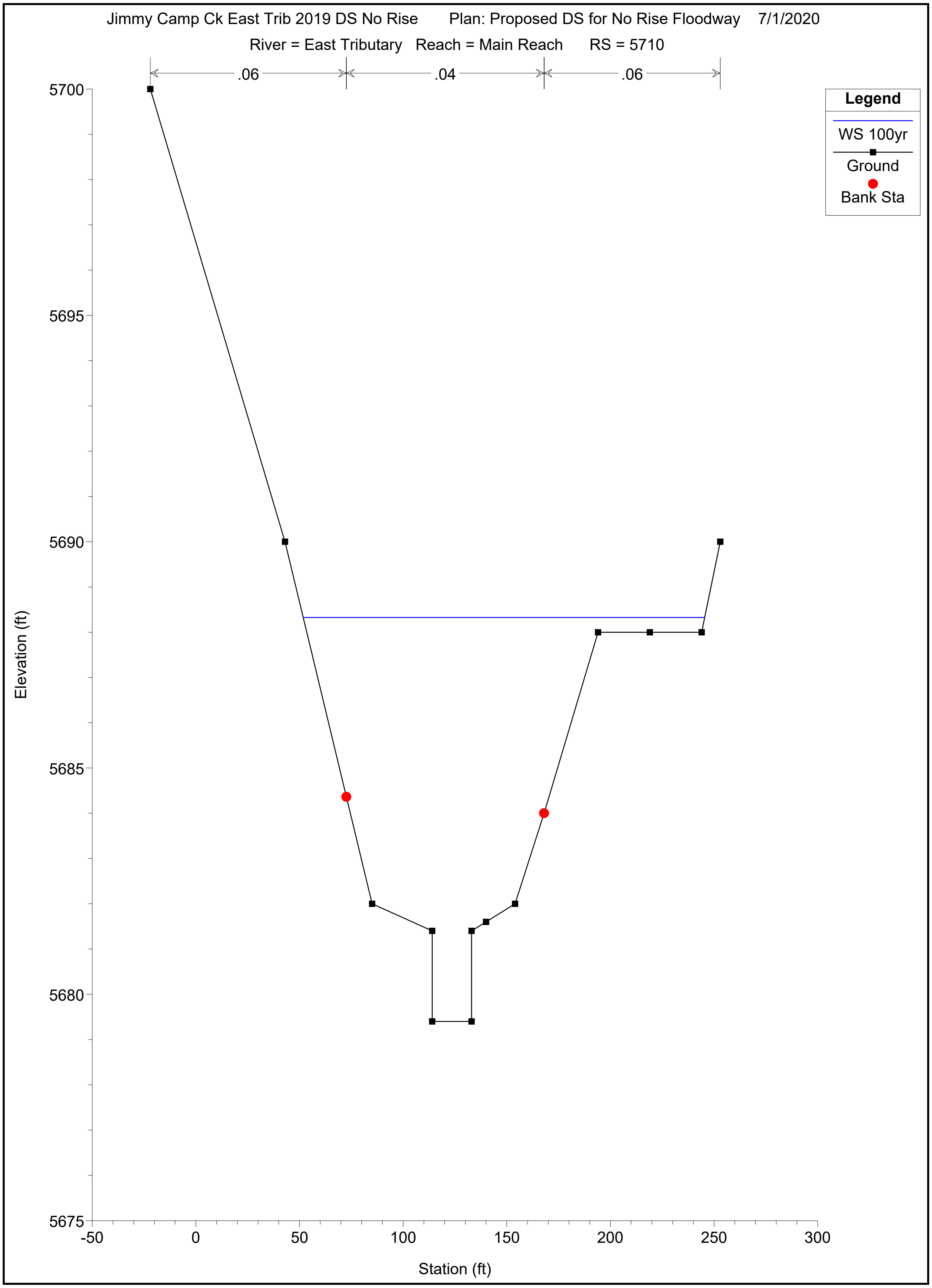
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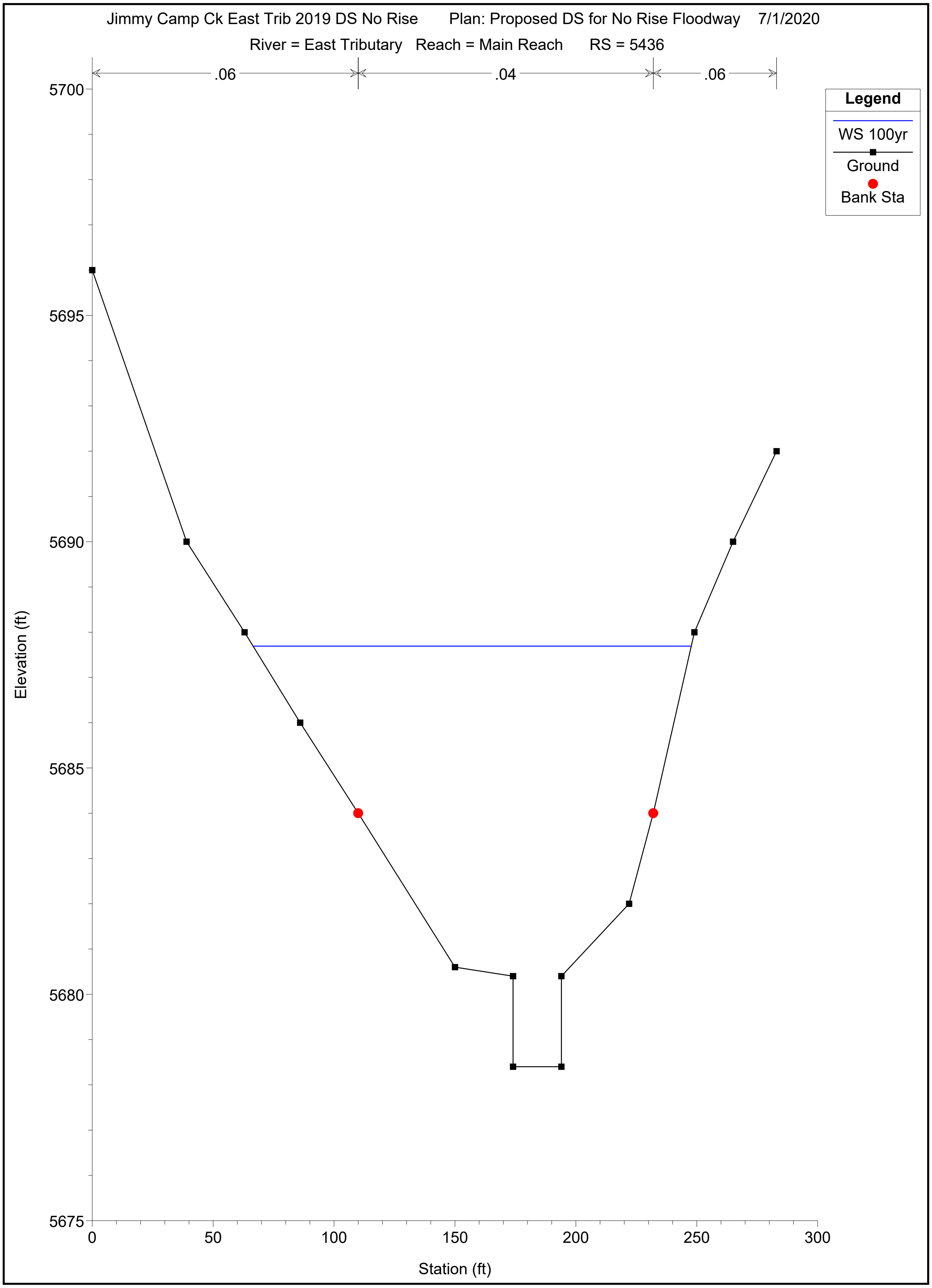
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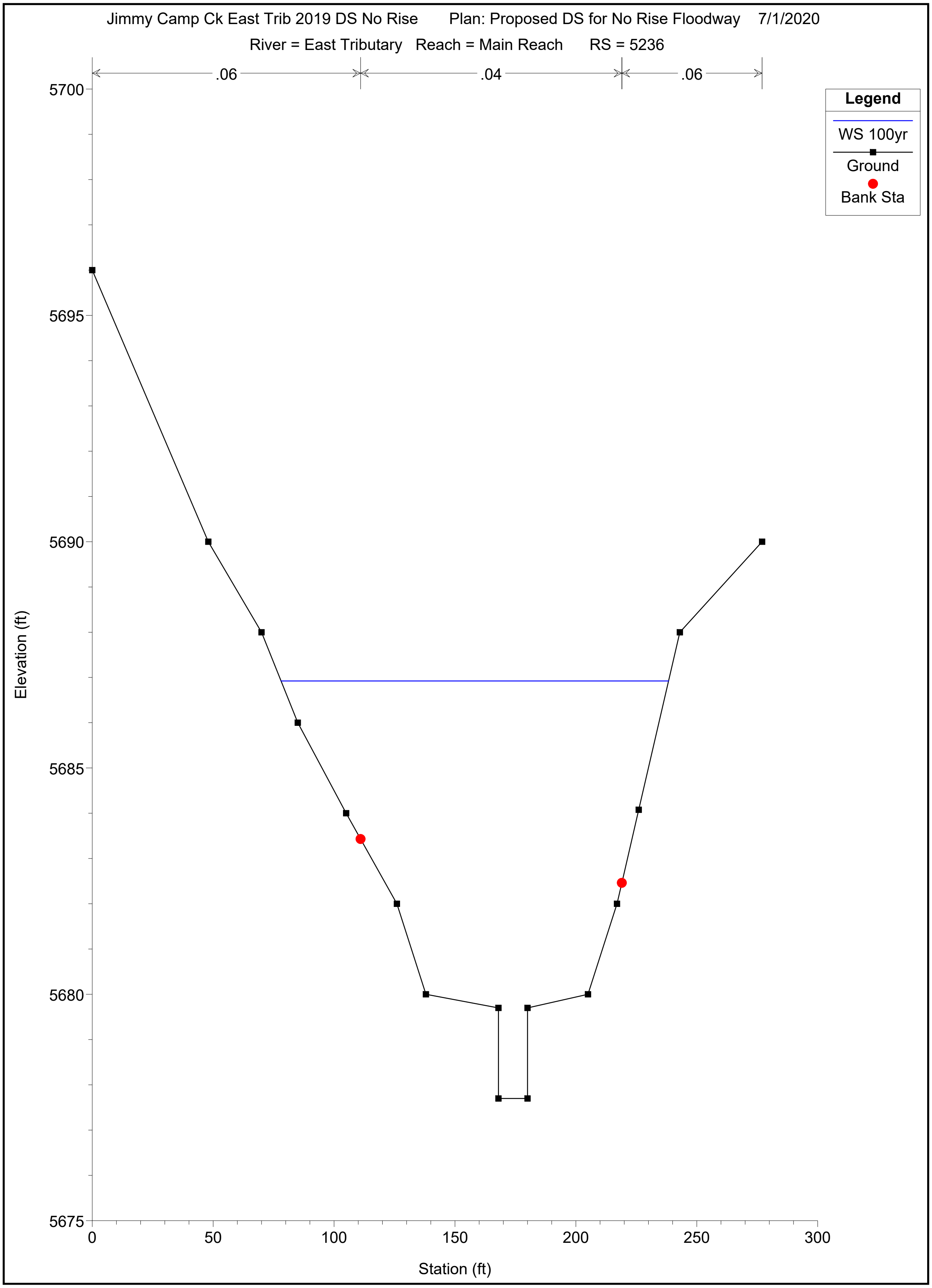
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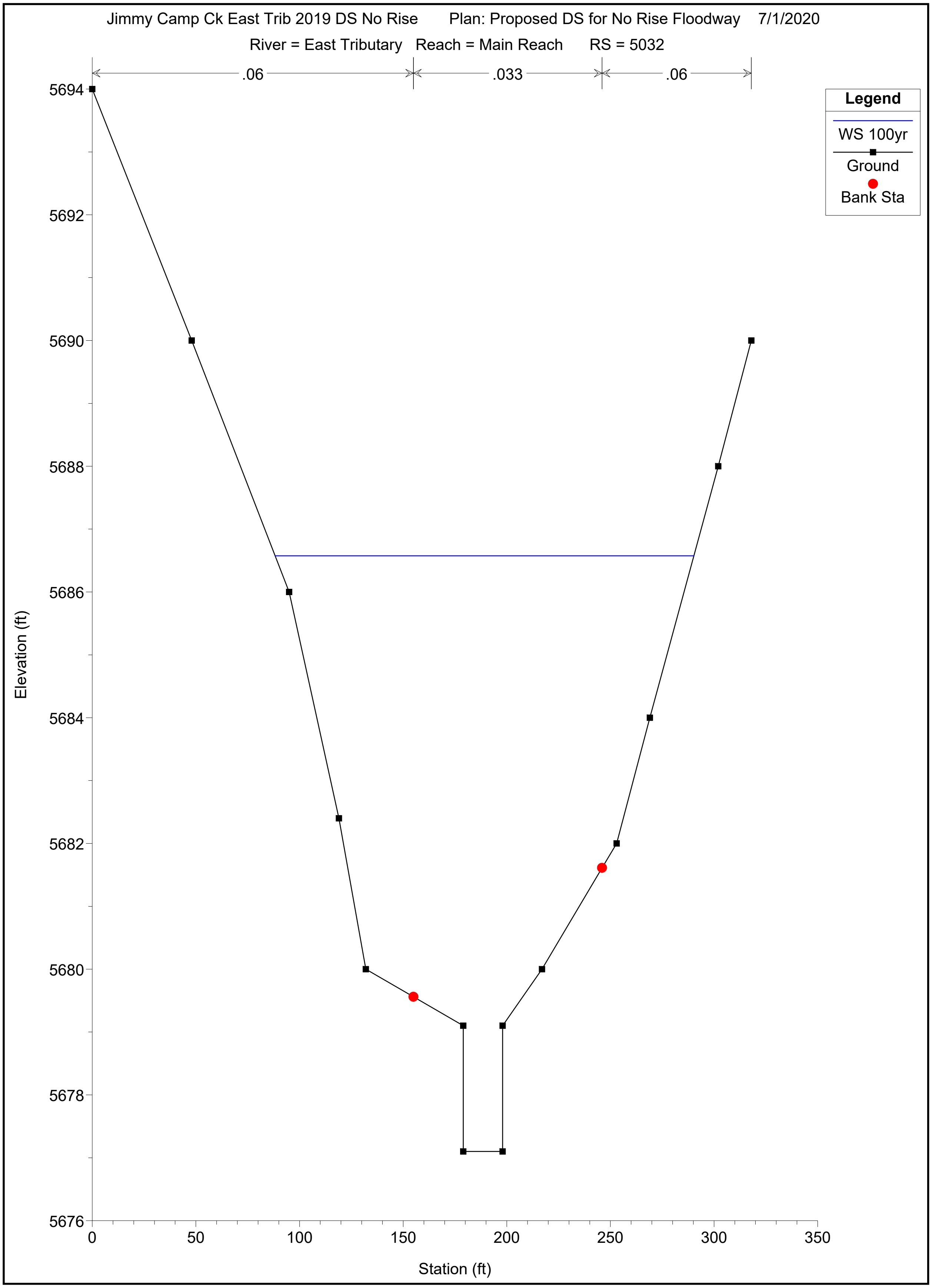
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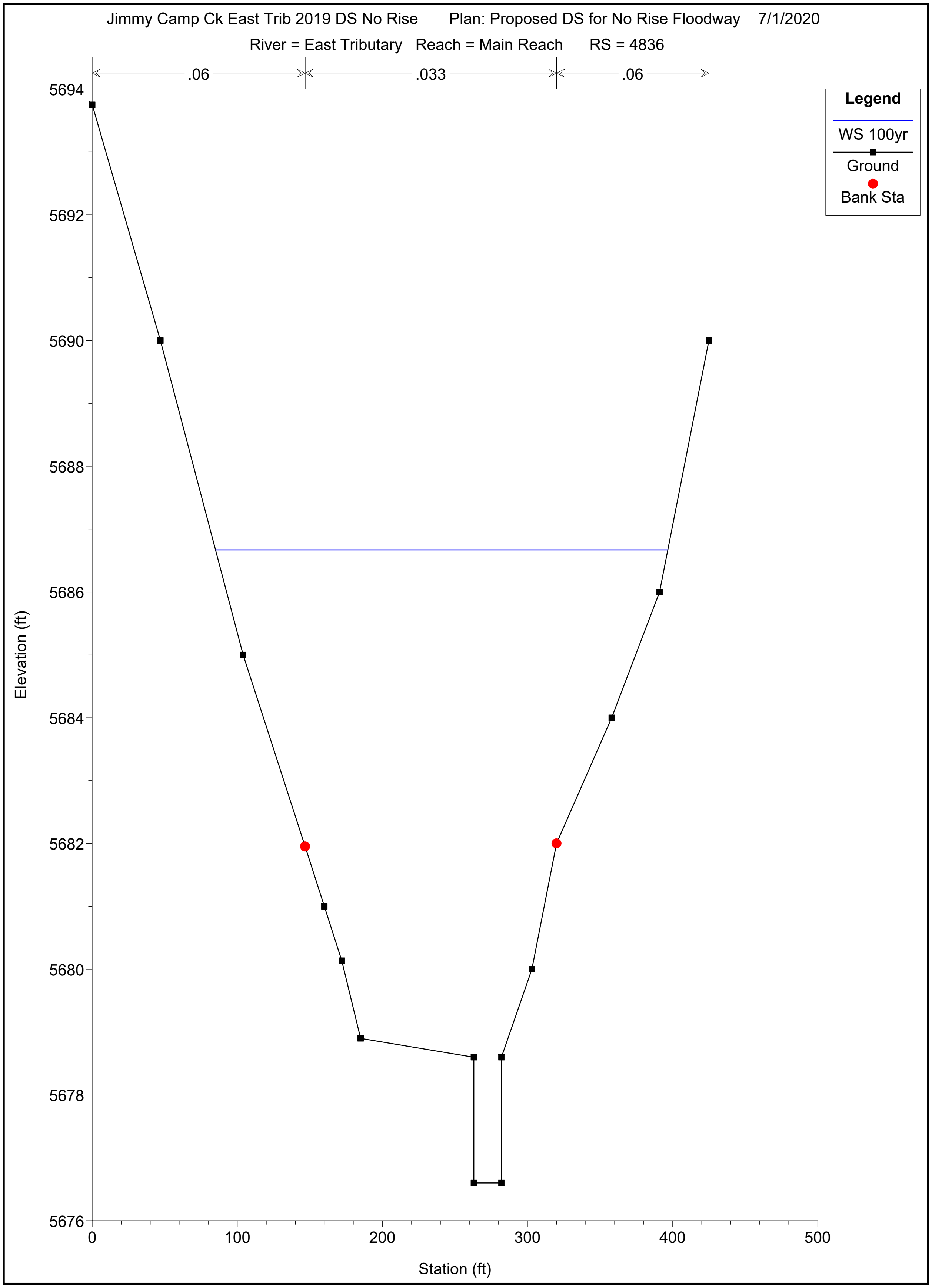Jimmy Camp Ck East Trib 2019 DS No Rise Plan: Proposed DS for No Rise Floodway 7/1/2020
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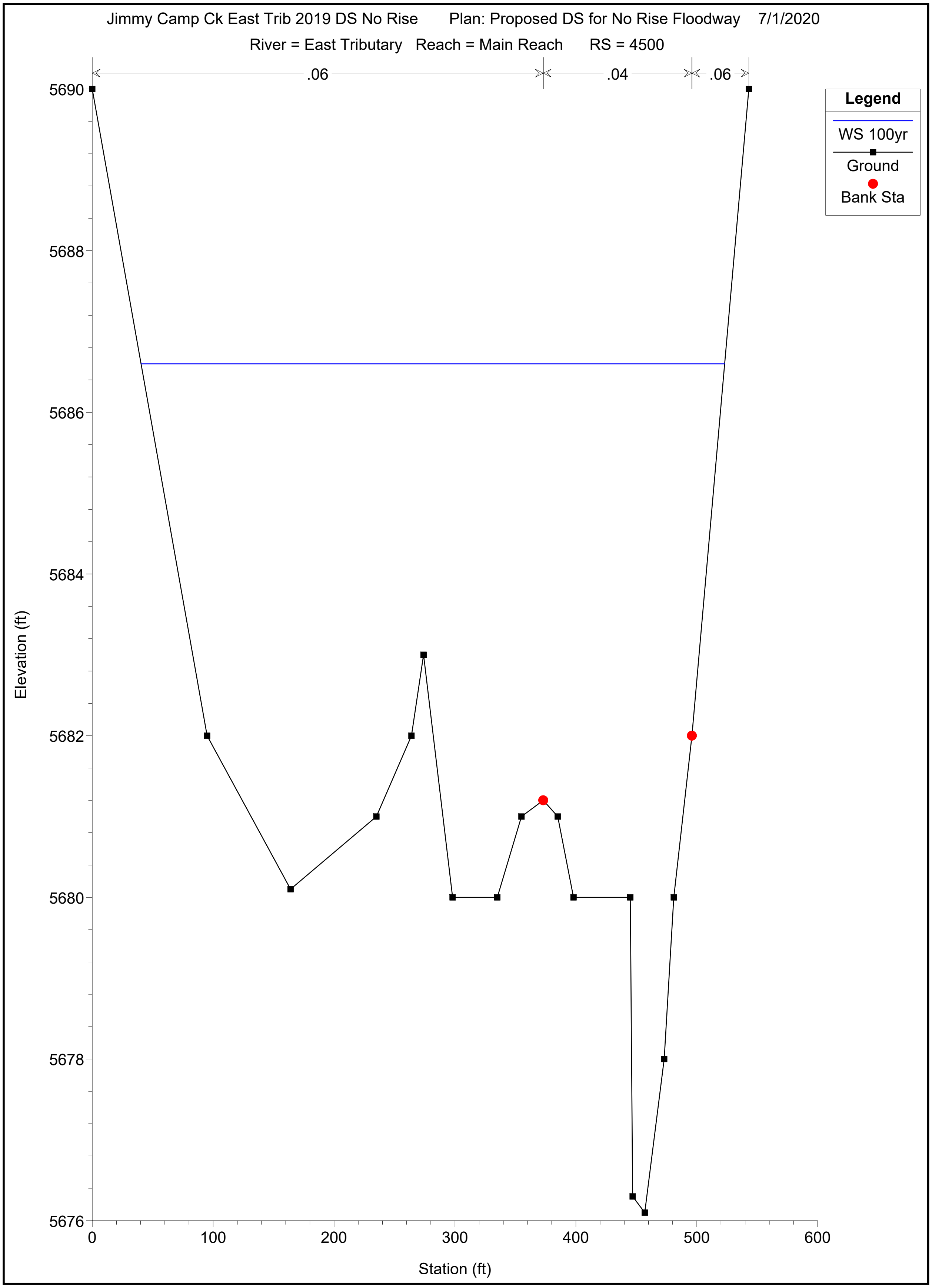

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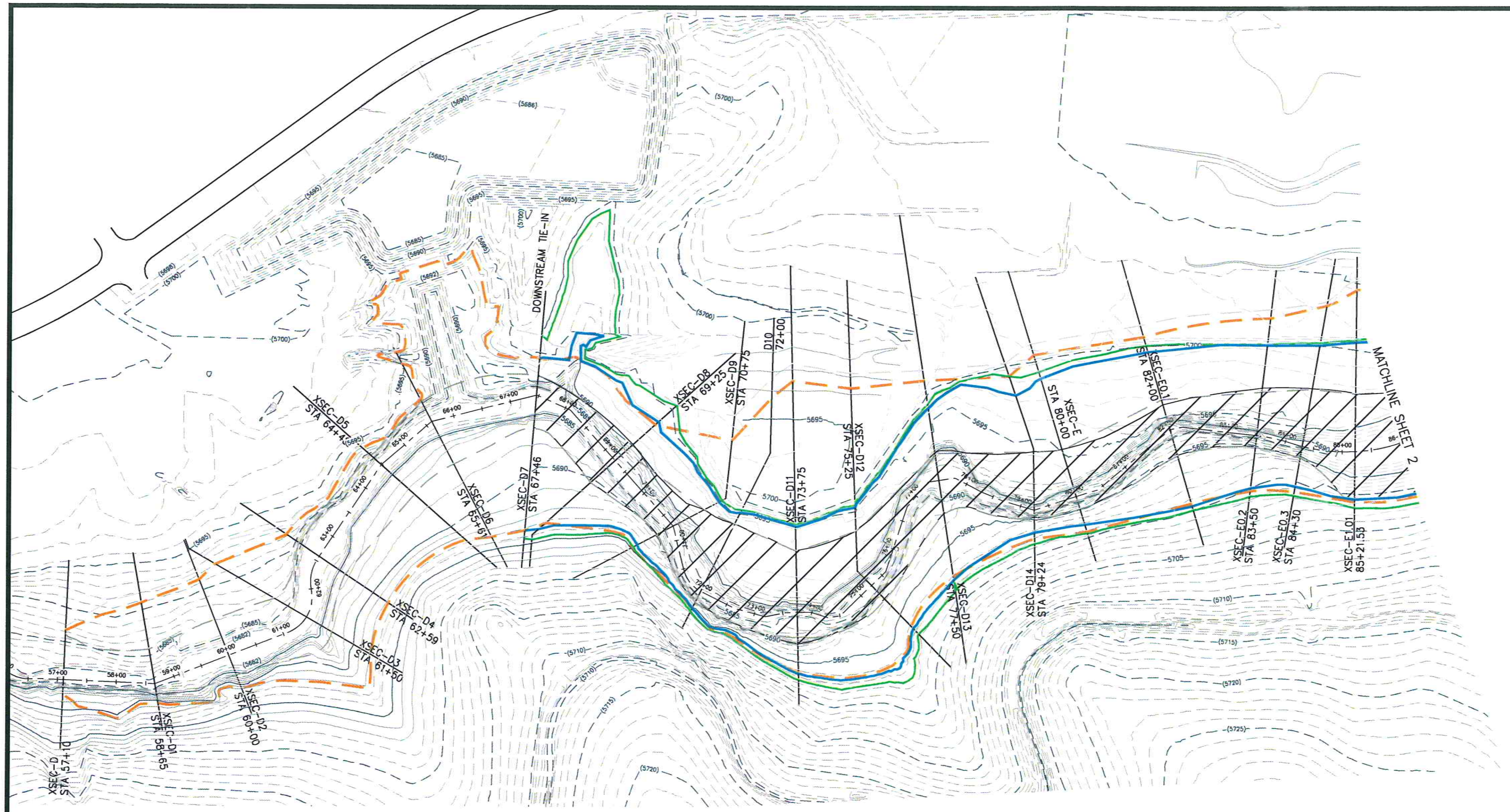
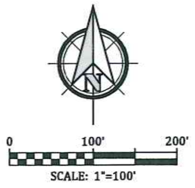


LEGEND

🟧 — —	EFFECTIVE 100-YEAR FLOODPLAIN
— —	EFFECTIVE 500-YEAR FLOODPLAIN
— —	EFFECTIVE FLOODWAY
🟦 ———	POST PROJECT 100-YEAR FLOODPLAIN
🟩 ———	POST PROJECT 500-YEAR FLOODPLAIN
▨	POST PROJECT FLOODWAY
—(6020)—	EXISTING CONTOURS



TOPOGRAPHY DOWNSTREAM OF SECTION 84+30 IS BASED UPON AERIAL MAPPING PROVIDED BY CORE ENGINEERING INC., COLORADO STATE PLANE COORDINATES 1983, AND NATIONAL GEODETIC VERTICAL DATUM OF 1929.

THE TOPOGRAPHY WAS COMPILED IN ACCORDANCE WITH NATIONAL MAPPING STANDARDS FOR 1"=200' & 2' CONTOUR INTERVAL DETAIL.

TOPOGRAPHIC MAPPING UPSTREAM OF SECTION 84+30 IS BASED ON FIELD SURVEY BY M & S CIVIL CONSULTANTS , COLORADO STATE PLANE COORDINATES 1983, AND NATIONAL GEODETIC VERTICAL DATUM OF 1929.

SITE BENCHMARK: FIMS MONUMENT NO. F204

STEPHEN A. BROWN 40190

STEPHEN A. BROWN
COLORADO LIC. 40190

7/31/19
DATED

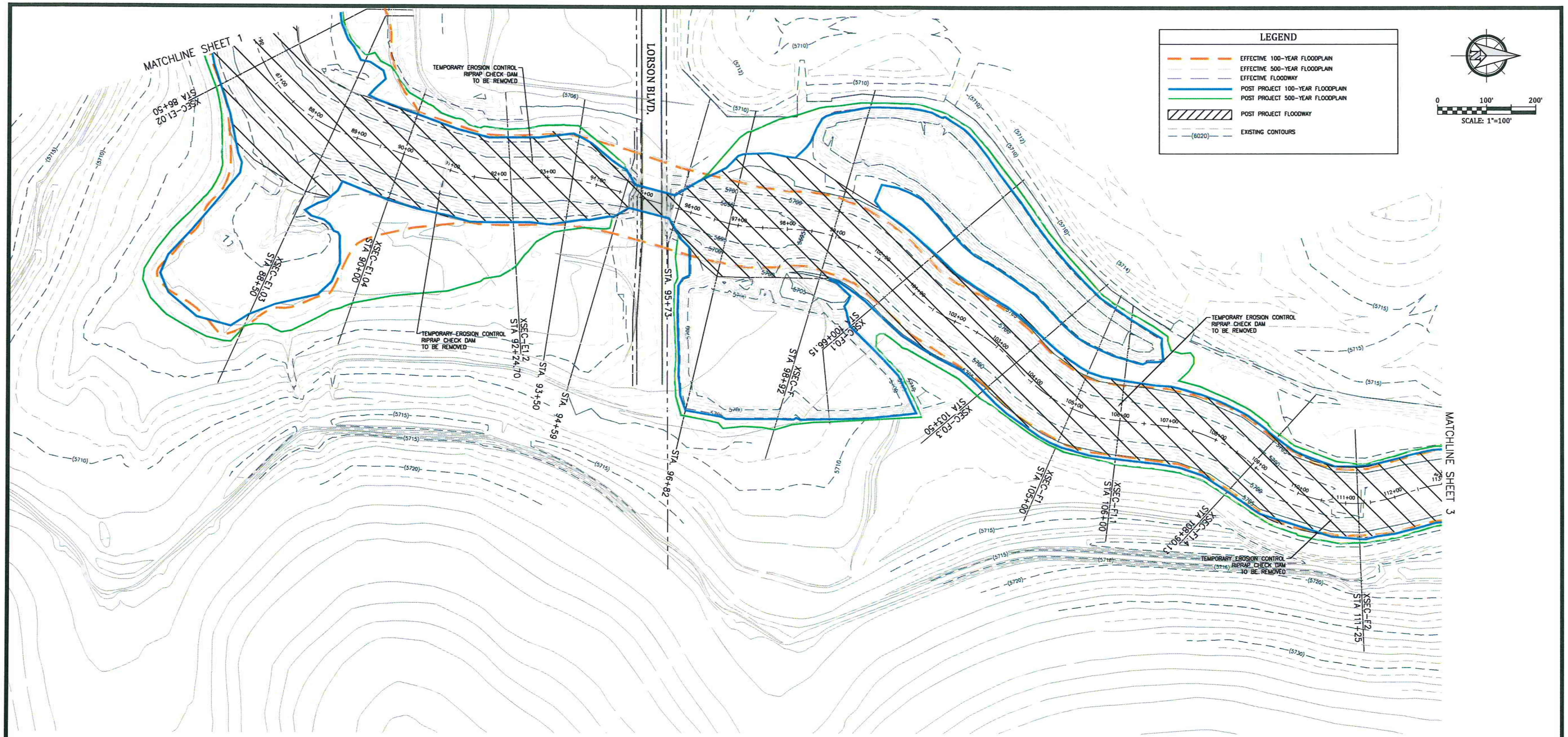
Kiowa
Engineering Corporation
Celebrating 30 years
1604 South 21st Street
Colorado Springs, Colorado 80904
(719) 630-7342

EAST TRIBUTARY JIMMY CAMP CREEK LOMR
LORSON RANCH
POST PROJECT CONDITIONS FLOODPLAIN MAP
EL PASO COUNTY, COLORADO

Project No.:	19001
Date:	JULY 31, 2019
Design:	SAB
Drawn:	SAB
Check:	RNW
Revisions:	

SHEET
1
OF 4 SHEETS

19001 LOMR Workmap FINAL.dwg/Aug 01, 2019



TOPOGRAPHIC MAPPING IS BASED ON FIELD SURVEY BY M & S CIVIL CONSULTANTS , COLORADO STATE PLANE COORDINATES 1983, AND NATIONAL GEODETIC VERTICAL DATUM OF 1929.

SITE BENCHMARK: FIMS MONUMENT NO. F204

COLORADO LICENSED
STEPHEN A. BROWN
40190
PROFESSIONAL ENGINEER

7031/19
DATED

STEPHEN A. BROWN
COLORADO LIC. 40190

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EAST TRIBUTARY JIMMY CAMP CREEK LOMR
LORSON RANCH
POST PROJECT CONDITIONS FLOODPLAIN MAP
EL PASO COUNTY, COLORADO

Project No.: 19001
Date: JULY 31, 2019
Design: SAB
Drawn: SAB
Check: RNW
Revisions:

SHEET
2
OF 4 SHEETS

19001 LOMR Workmap FINAL.dwg/Jul 31, 2019

Appendix B

Lorson Ranch 404 Permit

Kiowa
Engineering Corporation

April 1, 2020

Mr. Van Truan
U. S. Army Corps of Engineers
200 South Santa Fe Avenue Suite 301
Pueblo, Colorado 81003

Re: **SPA Action No. 2005 00757 Modification Amendment No. 2 Concurrence Request**
Lorson Ranch East Fork Jimmy Camp Creek Creekside Development
El Paso County, Colorado
(Kiowa Project No. 18020)

Dear Van:

Following Permit Modification No. 1 of August 2017, we are submitting a Permit Modification Amendment No. 2 for the above-mentioned project on behalf of Lorson Development and are requesting your concurrence at this time. Your office issued concurrence of Permit Modification No. 1 on September 7, 2017.

The purpose of this Modification Amendment is to address Special Condition 2 in the permit and will complete all activities that were originally authorized. Construction is proposed to commence in June 2020.

Permit Summary

Project impacts for the East Fork Jimmy Camp Creek on the Lorson Ranch were originally authorized under the above-mentioned permit by the Pueblo Regulatory Office on September 22, 2006. The permit authorized channel bank linings, grade control structures and two roadway crossings for three segments for the entire length of the East Fork Jimmy Camp Creek on the Lorson Ranch. See Figure 2, Permit Modification Amendment No. 2 Map (included in Wetland Delineation) for location of existing and proposed activities discussed here.

The central stream segment, designed as a reconfigured reach (Item#1 on Figure 2) was completed in about 2007 or 2008. Subsequently, a construction standstill in 2009 occurred and activity lapsed for about ten years. At that time, about 3,600 linear feet of reconfigured trapezoidal channel consisting of 100-Year riprap bank linings and grouted grade control structures were completed. The bottom width was designed at about 60-feet wide and the top width was about 180-feet wide. Currently, the reconfigured channel is vegetated with upland vegetation with areas of exposed rock on the bank linings and grouted drops structures.

Items 2, 3, and 4, the upper reach of the East Fork of Jimmy Camp Creek, the Fontaine Boulevard and Lorson Boulevard Bridges were completed between 2017 and 2018 and have been restored according to the Erosion Control Plan. No mitigation was required as these improvements had no loss to wetland or waters of the U.S.

Special Condition 2 applies to the lower stream preservation reach (Item #5 on Figure 2) and is addressed in this Permit Modification No. 2. The Lorson Ranch has been delineated twice, first by Savage and Savage in 2002 for the overall project for both the Mainstem and the East Fork under

1604 South 21st Street, Colorado Springs, Colorado 80904-4208
Ph: (719) 630-7342 Fax: (719) 630-0406 www.kiowaengineering.com

Action No. SPA-2002-00701. The East Fork Jimmy Camp Creek on the Lorson Ranch was again delineated in March 7, 2006 by AG Environmental Services, Inc. under Action No. SPA-2005-00757. The existing delineations for this reach were reviewed and verified for current conditions and are submitted within the Wetland Delineation (enclosed).

Lower Lorson East Fork Jimmy Camp Creek Stream Preservation Plan

This reach will be about 3,640 linear feet of discontinuous stream improvements in the Creekside Development on the Lorson Ranch. The design concept for this reach is to retain the stream alignment and sinuosity, to maintain the channel invert and bottom width, and to lay back the steep banks to three-to-one or four-to one with as minimal modifications as possible. An ungrouted boulder lining in selective locations will be placed along the low flow channel to prevent bank degradation.

Project Impacts

Project impacts within the ordinary high water are ungrouted boulder low flow channel linings in selective locations. A shallow overbank terrace of varying widths will be formed and revegetated to allow for riparian restoration. The lower terrace will be about three vertical feet. Stabilized outer banks, also in select locations, will be three-to-one revegetated rock/soil bank linings. The remainder of the outer banks will be regraded to four-to-one and will be revegetated. The outer banks will be outside of the Waters of the U.S. Two sheet pile grade control structures will ensure invert stability of this low gradient waterway. The bottom width of the channel will be about 12- to 20-feet.

The existing Waters of the U.S. measure 1.4 acres and will be replaced with approximately 1.5 acres of Waters of the U.S. All wetlands found to be present in 2020 are included within the Waters of the U.S. We anticipate that the pools and intermittent wetland channel vegetation replace in situ with no net fill to Waters of the U.S. and disturbance to be of a temporary impact.

Please let us know if you need more information.

Sincerely,
KIOWA ENGINEERING CORPORATION
Liz Klein
Elizabeth Klein

Encs. Wetland Delineation and 404 Permitting Update East Fork Jimmy Camp Creek Channel Design Creekside at Lorson Filing No.1

East Fork Jimmy Camp Creek Channel Design Creekside at Lorson Filing No 1. Final Design Plans

cc: Jeff Mark, Lorson Development
Richard Schindler, Core Engineering

Wetland Delineation and 404 Permitting Update
East Fork Jimmy Camp Creek Channel Design
Creekside at Lorson Filing No.1

U.S.A.C.E. Action No. SPA-2005-00757
Colorado Springs, Colorado

Prepared for:

Lorson Development
212 N. Wahsatch Ave., Ste 301
Colorado Springs, CO 80903

Prepared by:

Kiowa
Engineering Corporation

1604 South 21st Street
Colorado Springs, Colorado 80904
(719) 630-7342

Kiowa Project No. 18020

March 30, 2020

Introduction and Project History

The proposed East Fork Jimmy Camp Creek Channel Design Project is located on the East Fork of Jimmy Camp Creek in El Paso County Colorado, in Section 24 Township 15 South and Range 65 West of the 6th Principal Meridian (see Vicinity Map on Figure 1). GPS coordinates for the center of the project are approximately 38.732° Lat. and -104.637° Long.

Project impacts for the East Fork Jimmy Camp Creek on the Lorson Ranch were originally authorized by the Pueblo Regulatory Office of the U.S. Army Corps of Engineers on September 22, 2006 under USACE Action No. 2005-00757 with an expiration date of December 31, 2009. The permit authorized channel bank linings, grade control structures and two roadway crossings for three segments for the entire length of the East Fork Jimmy Camp Creek on the Lorson Ranch from the north boundary to the south boundary.

Attached Figure 2, Permit Modification Amendment No. 1 Map (Revised 2020) shows locations of jurisdictional activities authorized under this permit. The central stream segment, designed as a reconfigured reach (Item#1 on Figure 2), was completed in about 2007 or 2008. Subsequently, a construction standstill in 2009 occurred with no further activity for a number of years. It appears that the permit was extended twice, first to September 2001 and then to September 2021.

In August 2017, Kiowa Engineering consulted with the USACE to update the existing permit per Modification Amendment No. 1. The purpose of this Modification Amendment was to address and clarify Special Conditions in the permit and summarize all future activities that were originally authorized in this permit. The agency concurred with Modification Amendment No. 1 in September 2017 (see Appendix).

Following Modification Amendment No. 1, Item 2 (Upper Reach Channel Improvements), Item 3 (Fontaine Boulevard Bridge) and Item 4 (Lorson Boulevard Bridge) have been completed.

Item 5, the lower reach of the East Fork Jimmy Camp Creek from the existing trapezoidal channel to the south property line is proposed to be completed in the near future. The East Fork Jimmy Camp Creek Channel Design Creekside at Lorson Filing No 1 Final Design Plans are included in the Appendix.

Per Special Condition 2 of the Permit Modification, the lower stream preservation reach (Item #5 on Figure 2) will be about 3,600 linear feet of revegetated three-to-one soil/riprap bank linings in selective locations on the overbanks with two steel sheet pile grade control structures. The low flow channel with be lined with boulders in selective locations to prevent over bank degradation and retain a bottom width similar to the existing channel width. The design concept for this reach retains the stream sinuosity and alignment as much as possible, avoids future channel incision and lays back steep over banks to three-to-one and four-to-one. The proposed invert remains similar the same as the existing.

This Wetland Delineation Update is being submitted to satisfy Permit Modification #1 of August 2017 to review the current Waters of the U.S. and adjacent wetlands. This will be the last phase of the East Fork Jimmy Camp Creek IP-SPA-2005-00757 and will finalize the above-mentioned permit.

Wetland Assessments

The Lorson Ranch has been delineated twice during the permitting process. The original delineation by Savage and Savage in 2002 for the overall project delineated both the Mainstem Jimmy Camp Creek and the East Fork Jimmy Camp Creek. Subsequently, the Mainstem Jimmy Camp Creek was permitted and completed under Action No. 2002 00701. The East Fork Jimmy Camp Creek in the Lorson Ranch was again delineated in March 7, 2006 by AG Environmental Services, Inc. under Action No. 2005 00757.

The existing delineations for this reach were verified for current conditions in March 2020 and are presented in this document.

Vegetation

The dominant upland vegetation community in the project area is short grass prairie dominated by smooth brome grass where disturbed and blue grama in undisturbed areas. A poorly developed riparian forest/shrubland is present along the waterway. Small pockets of wetland vegetation are present within the Ordinary High Water Mark. As the project site had been historically used for grazing to the extent it was overgrazed, large areas of weedy species are present.

Wetland plant species found were soft stemmed bulrush (*Scirpus sp.* OBL), American three Square (*Schoenoplectus pungens.* FACW), sandbar willow (*Salix exigua* OBL), and cattails (*Typhus angustifolia* and *latifolia* OBL). On the whole, wetland species were sparse and interspersed with communities of upland species.

Riparian forest/shrubland species present on the site are dominated by non-native Russian olive trees (*Eleagnus angustifolia,* FACU), snowberry (*Symphoriocarpus occidentalis* UPL), wildrose (*Rosa woodsii* FACU) and elm (*Ulmus pumila* NI). In general, the riparian species present along the waterway were non-native undesirable native species that may shade the water but not promote water quality. Only a few native cottonwoods were found and one peachleaf willow. Fortunately, no tamarisk was found.

Upland herb species encountered include smooth broom (*Bromus inermis* NI), blue grama (*Bouteloua gracilis* NI), filaree (*Erodium cicutarium* NI), licorice (*Glycerrhiza* *lepidota,* FACU), milkweed (*Asclepias speciosa* FAC), annual brome grass (*Bromus tectorum* FACU), mullein (*Verbascum thaspus* NI), and Canada thistle (*Cirsium arvense* FACU). Other weedy species present were blue mustard (*Chorispora tenuella* NI), kochia (*Kochia sp* UPL), and flixweed (*Descurainia sp.* NI).

Hydrology

The East Fork Jimmy Camp Creek is a 'blue-line' drainageway as depicted on the Fountain Quadrangle Map (1994) and appears to have an intermittent flow and as such is an assumed jurisdictional waterway. Channel banks are nearly vertical fairly shallow and range from 2- to over 10-feet of height. The channel varies from vegetated to unvegetated. The channel has very shallow slopes of less than 0.5% to nearly flat.

The Fountain Mutual Irrigation Canal (FMIC) formerly traversed around the East Fork Jimmy Camp Creek channel and contributed to the historic hydrology. More than 15 years ago, the FMIC was realigned and subsequently caused adjacent wetlands to dry out. A large area of former wetland body downgradient of the FMIC was decommissioned by the U.S.A.C.E. in the delineation of 2006.

Seasonal snowfall for this location is 149% of normal per Colorado Springs climate data for October 2019 through March 30, 2020. The flow at the time of delineation is substantially higher than usual with standing water of three-to four inches present on the channel bottom in many locations. During the average growing season, this stream segment is often dry with only small pools remaining. The hydrology present during this field review is much greater than can be expected during the average growing season due to high seasonal snows.

Soils

The native soils within the project area per the Soil Survey of El Paso County are the Blendon sandy loam and the Ellicott loamy coarse sand. Neither of these soils is considered hydric, although both could have pockets of hydric soil inclusions.

The Blendon sandy loam, 0-3% slopes, is a deep well drained soil with a depth of the water table anticipated to be greater than 6.5 feet, run-off is anticipated to be low, frequency of flooding and/or ponding is none. This upland soil was formed in alluvium and residuum and is derived from arkosic sedimentary rock.

The Ellicott loamy coarse sand, 0-5% slopes are somewhat excessively drained soils, with an expected depth to the water table to be greater than 6.5 feet. Runoff is anticipated to be very low, frequency of flooding is frequent, but ponding is none. This soil is formed on floodplains and terraces.

Hydric soils were only found on the bottom of the channel bed.

Wetland Resources and Jurisdictional Waters of the U.S.

Waters of the U.S. were found within the existing low flow channel with a depth of about two vertical feet and are presented on Exhibit 1. Small pockets of mixed emergent wetlands were found within the existing channel bottom, but rapidly transitioned to upland vegetation with where a clear boundary between wetland vegetation and upland vegetation shows on the bank. The wetland vegetation is contained within the Waters of the U.S. and measures approximately 1.4 acres.

Summary

Existing Waters of the U.S. and adjacent wetlands (1.4 acres) will be replaced with an intermittent wetland/pool channel of similar area (1.5 acres). Channel width, length, and invert will remain the same as the existing condition allowing intermittent pools to exist. Sinuosity will also be preserved and overbanks will be stabilized compatible with the stream preservation design concept.

References

El Paso County, Colorado, Comprehensive Hydric Soils List. U.S. Department of Agriculture, Soil Conservation Survey.

Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Great Plains Region ver 2.0, March 2010, ERDC/EL TR-10-1.

Munsell Color Charts. Macbeth: a Division of Kollmorgan Corporation.

National Wetland Inventory, Fountain Quadrangle, 1975. U. S. Department of the Interior, U.S. Fish and Wildlife Service.

National Wetland Inventory Wetlands Mapper August, 2019. U. S. Department of the Interior, U.S. Fish and Wildlife Service.

Reed, P.B. 1996. National List of Plants That Occur in Wetlands: Colorado. U.S. Fish and Wildlife Service, Washington D.C. NERC-88/18.06.

Rocky Mountain Group Engineers, August 2018. Geology and Soils Report Creekside at Lorson Ranch, Filing No. 1, El Paso County CO.

Custom Soils Report for Lower Lorson East Fork Jimmy Camp Creek, of El Paso County, Colorado. July 25, 2019. U.S. Department of Agriculture, Natural Resource Conservation Service.

Weber, W. A. 1996. Flora of Colorado: East Slope. University Press of Colorado, Boulder, Colorado.

U.S.G.S. 7.5-minute Fountain Quadrangle Map. El Paso County, Colorado. 1994. U. S. Department of the Interior.

Appendix

Photographs
USACE Concurrence Letter
Figure 1 Vicinity Map
Figure 2 Permit Modification Map – Revised in 2020
Exhibit 1 Waters of the U.S. Map 2020 Update
East Fork Jimmy Camp Creek Channel Design Creekside at Lorson Filing No 1. Final Design Plans

Photograph 1: DP 1 Near downstream property line near FMIC siphon.

Photograph 2: DP2 Near old ranch road crossings.

Photograph 1: DP 3 Standing water in channel bed.

Photograph 2: DP4 Located downstream of existing trapezoidal channel.

WETLAND DETERMINATION DATA FORM – Great Plains Region

Project/Site: Lower Lurson Ranch City/County: EL PASO Sampling Date: 3/24/20
Applicant/Owner: LURSON DEVELOPMENT State: CO Sampling Point: DP 1
Investigator(s): LIZ KLEIN Section, Township, Range: S 24 T 15 S R 65 W
Landform (hillslope, terrace, etc.): Plain Local relief (concave, convex, none): ENTRENCHED CHANNEL Slope (%): 0 - 5 %
Subregion (LRR): LRR G Lat: 38.731 Long: -104.634 Datum: ____
Soil Map Unit Name: MANZANOLA Clay Loam NWI classification: PEM1C
Are climatic / hydrologic conditions on the site typical for this time of year? Yes ____ No ✓ (If no, explain in Remarks.)
Are Vegetation ____, Soil ____, or Hydrology ____ significantly disturbed? Are "Normal Circumstances" present? Yes ✓ No ____
Are Vegetation ____, Soil ____, or Hydrology ____ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

Hydrophytic Vegetation Present? Yes ✓ No ____
Hydric Soil Present? Yes ✓ No ____
Wetland Hydrology Present? Yes ✓ No ____

Is the Sampled Area within a Wetland? Yes ✓ No ____
ABOUT 20' WIDE

Remarks: SEASONAL SNOWFALL AT 149%
Located DOWNSTREAM OF FMIC SIPHON

VEGETATION – Use scientific names of plants.

	Absolute % Cover	Dominant Species?	Indicator Status
Tree Stratum (Plot size: 30')			
1. Ulmus pumila	20	Y	NI
2. ELEAGNUS ANGUSTIFOLIA	20	Y	FACU
3.			
4.			
	40 = Total Cover		
Sapling/Shrub Stratum (Plot size: 15')			
1. Symphoricarpos Carpus	40	Y	UPL
2. Salix exigua	5	N	FACW
3.			
4.			
5.			
	45 = Total Cover		
Herb Stratum (Plot size: 5')			
1. BRomus inermis	30	Y	NI
2. Rumex crispus	2	N	FAC
3. * Schoenoplectus pungens	30	Y	OBL
4.			
5.			
6.			
7. * INDICATIVE OF LOW FLOW AREA +			
8. thus used for Rapid DOMINANCE TEST			
9.			
10.			
	= Total Cover		
Woody Vine Stratum (Plot size: ____)			
1.			
2.			
	= Total Cover		

% Bare Ground in Herb Stratum ____

Dominance Test worksheet:

Number of Dominant Species That Are OBL, FACW, or FAC (excluding FAC−): 1 (A)
Total Number of Dominant Species Across All Strata: 1 (B)
Percent of Dominant Species That Are OBL, FACW, or FAC: 100 (A/B)

Prevalence Index worksheet:

Total % Cover of:		Multiply by:
OBL species		x 1 =
FACW species		x 2 =
FAC species		x 3 =
FACU species		x 4 =
UPL species		x 5 =
Column Totals:	(A)	(B)

Prevalence Index = B/A = ____

Hydrophytic Vegetation Indicators:

- ____ 1 - Rapid Test for Hydrophytic Vegetation
- ✓ 2 - Dominance Test is >50%
- ____ 3 - Prevalence Index is ≤3.0[1]
- ____ 4 - Morphological Adaptations[1] (Provide supporting data in Remarks or on a separate sheet)
- ____ Problematic Hydrophytic Vegetation[1] (Explain)

[1]Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Hydrophytic Vegetation Present? Yes ✓ No ____

Remarks:
ALL WETLAND VEGETATION LOCATED WITHIN WUS
IN A NARROW BAND. TRANSITIONS to upland AT 2' Elevation

SOIL

Sampling Point: [illegible]

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

Depth	Matrix		Redox Features					
(inches)	Color (moist)	%	Color (moist)	%	Type[1]	Loc[2]	Texture	Remarks
0-12"	10YR 2/2		—					

[1]Type: C=Concentration, D=Depletion, RM=Reduced Matrix, CS=Covered or Coated Sand Grains. [2]Location: PL=Pore Lining, M=Matrix.

Hydric Soil Indicators: (Applicable to all LRRs, unless otherwise noted.)

___ Histosol (A1)
___ Histic Epipedon (A2)
___ Black Histic (A3)
___ Hydrogen Sulfide (A4)
___ Stratified Layers (A5) (LRR F)
___ 1 cm Muck (A9) (LRR F, G, H)
___ Depleted Below Dark Surface (A11)
✓ Thick Dark Surface (A12)
___ Sandy Mucky Mineral (S1)
___ 2.5 cm Mucky Peat or Peat (S2) (LRR G, H)
___ 5 cm Mucky Peat or Peat (S3) (LRR F)
___ Sandy Gleyed Matrix (S4)
___ Sandy Redox (S5)
___ Stripped Matrix (S6)
___ Loamy Mucky Mineral (F1)
___ Loamy Gleyed Matrix (F2)
___ Depleted Matrix (F3)
___ Redox Dark Surface (F6)
___ Depleted Dark Surface (F7)
___ Redox Depressions (F8)
✓ High Plains Depressions (F16)
(MLRA 72 & 73 of LRR H)

Indicators for Problematic Hydric Soils[3]:

___ 1 cm Muck (A9) (LRR I, J)
___ Coast Prairie Redox (A16) (LRR F, G, H)
___ Dark Surface (S7) (LRR G)
___ High Plains Depressions (F16)
(LRR H outside of MLRA 72 & 73)
___ Reduced Vertic (F18)
___ Red Parent Material (TF2)
___ Very Shallow Dark Surface (TF12)
___ Other (Explain in Remarks)

[3]Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if present):

Type: ______

Depth (inches): ______

Hydric Soil Present? Yes ✓* No ______

Remarks: *wetland soils on inchannel bottom, upland soil on bank 10YR 4/2

HYDROLOGY

Wetland Hydrology Indicators:

Primary Indicators (minimum of one required; check all that apply)

✓ Surface Water (A1)
___ High Water Table (A2)
___ Saturation (A3)
___ Water Marks (B1)
___ Sediment Deposits (B2)
___ Drift Deposits (B3)
___ Algal Mat or Crust (B4)
___ Iron Deposits (B5)
___ Inundation Visible on Aerial Imagery (B7)
___ Water-Stained Leaves (B9)
___ Salt Crust (B11)
___ Aquatic Invertebrates (B13)
___ Hydrogen Sulfide Odor (C1)
___ Dry-Season Water Table (C2)
___ Oxidized Rhizospheres on Living Roots (C3)
(where not tilled)
___ Presence of Reduced Iron (C4)
___ Thin Muck Surface (C7)
___ Other (Explain in Remarks)

Secondary Indicators (minimum of two required)

___ Surface Soil Cracks (B6)
___ Sparsely Vegetated Concave Surface (B8)
___ Drainage Patterns (B10)
___ Oxidized Rhizospheres on Living Roots (C3)
(where tilled)
___ Crayfish Burrows (C8)
___ Saturation Visible on Aerial Imagery (C9)
___ Geomorphic Position (D2)
___ FAC-Neutral Test (D5)
___ Frost-Heave Hummocks (D7) (LRR F)

Field Observations:

Surface Water Present? Yes ✓ No ______ Depth (inches): 0-3"
Water Table Present? Yes ______ No ______ Depth (inches): ______
Saturation Present? (includes capillary fringe) Yes ______ No ______ Depth (inches): ______

Wetland Hydrology Present? Yes ✓ No ______

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks: CHANNEL varies FROM 2' to 12'

WETLAND DETERMINATION DATA FORM – Great Plains Region

Project/Site: LOWER LOESON RANCH City/County: EL PASO Sampling Date: 3/24/20
Applicant/Owner: LOESON DEVELOPMENT State: CO Sampling Point: DP 2
Investigator(s): LIZ KLEIN Section, Township, Range: S 24 T 15 S R 65 W
Landform (hillslope, terrace, etc.): plains Local relief (concave, convex, none): ENTRENCHED CHANNEL Slope (%): 0-5%
Subregion (LRR): LRR G Lat: 38.732 Long: 104.640 Datum: ____
Soil Map Unit Name: BLENDON sandy loam NWI classification: PEM1C
Are climatic / hydrologic conditions on the site typical for this time of year? Yes ____ No ✓ (If no, explain in Remarks.)
Are Vegetation ____, Soil ____, or Hydrology ____ significantly disturbed? Are "Normal Circumstances" present? Yes ✓ No ____
Are Vegetation ____, Soil ____, or Hydrology ____ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

Hydrophytic Vegetation Present?	Yes ✓ No ____	Is the Sampled Area within a Wetland? Yes ✓ No ____ BUT ONLY IN CHANNEL
Hydric Soil Present?	Yes ____ No ____	
Wetland Hydrology Present?	Yes ✓ No ____	

Remarks: SEASONAL SNOWFALL AT 149%
LOCATED NEAR OLD RANCH ROAD CROSSING

VEGETATION – Use scientific names of plants.

	Absolute % Cover	Dominant Species?	Indicator Status
Tree Stratum (Plot size: 30')			
1. ELEAGNUS ANGUSTIFOLIA	30	Y	FACU
2.			
3.			
4.			
	____ = Total Cover		
Sapling/Shrub Stratum (Plot size: 15')			
1. Symphoricarpus occidentalis	15	Y	UPL
2.			
3.			
4.			
5.			
	____ = Total Cover		
Herb Stratum (Plot size: 5')			
1. Typha latifolia	50	Y	OBL
2. BROMUS INERMIS	50	Y	UPL
3. SCIRPUS SP	5	N	OBL
4.			
5.			
6.			
7.			
8.			
9.			
10.			
	____ = Total Cover		
Woody Vine Stratum (Plot size: ____)			
1.			
2.			
	____ = Total Cover		
% Bare Ground in Herb Stratum ____			

Dominance Test worksheet: * SEE BELOW

Number of Dominant Species That Are OBL, FACW, or FAC (excluding FAC−): 1 (A)
Total Number of Dominant Species Across All Strata: 1 (B)
Percent of Dominant Species That Are OBL, FACW, or FAC: 100% (A/B)

Prevalence Index worksheet:

Total % Cover of:		Multiply by:
OBL species	____	x 1 = ____
FACW species	____	x 2 = ____
FAC species	____	x 3 = ____
FACU species	____	x 4 = ____
UPL species	____	x 5 = ____
Column Totals:	____ (A)	____ (B)

Prevalence Index = B/A = ____

Hydrophytic Vegetation Indicators:

___ 1 - Rapid Test for Hydrophytic Vegetation
___ 2 - Dominance Test is >50%
___ 3 - Prevalence Index is ≤3.0[1]
___ 4 - Morphological Adaptations[1] (Provide supporting data in Remarks or on a separate sheet)
___ Problematic Hydrophytic Vegetation[1] (Explain)

[1] Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Hydrophytic Vegetation Present? Yes ✓ No ____

Remarks: *WETLAND VEG. FOUND IN CHANNEL BOTTOM
DETERMINATION BASED ON CHANNEL AREA ONLY

SOIL

Sampling Point: ___

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

Depth (inches)	Matrix		Redox Features				Texture	Remarks
	Color (moist)	%	Color (moist)	%	Type[1]	Loc[2]		
0-6"	10YR4/2							
6-9	10YR3/2							

[1]Type: C=Concentration, D=Depletion, RM=Reduced Matrix, CS=Covered or Coated Sand Grains. [2]Location: PL=Pore Lining, M=Matrix.

Hydric Soil Indicators: (Applicable to all LRRs, unless otherwise noted.)

- ___ Histosol (A1)
- ___ Histic Epipedon (A2)
- ___ Black Histic (A3)
- ___ Hydrogen Sulfide (A4)
- ___ Stratified Layers (A5) (LRR F)
- ___ 1 cm Muck (A9) (LRR F, G, H)
- ___ Depleted Below Dark Surface (A11)
- ___ Thick Dark Surface (A12)
- ___ Sandy Mucky Mineral (S1)
- ___ 2.5 cm Mucky Peat or Peat (S2) (LRR G, H)
- ___ 5 cm Mucky Peat or Peat (S3) (LRR F)
- ___ Sandy Gleyed Matrix (S4)
- ___ Sandy Redox (S5)
- ___ Stripped Matrix (S6)
- ___ Loamy Mucky Mineral (F1)
- ___ Loamy Gleyed Matrix (F2)
- ___ Depleted Matrix (F3)
- ___ Redox Dark Surface (F6)
- ___ Depleted Dark Surface (F7)
- ___ Redox Depressions (F8)
- ___ High Plains Depressions (F16)
  (MLRA 72 & 73 of LRR H)

Indicators for Problematic Hydric Soils[3]:

- ___ 1 cm Muck (A9) (LRR I, J)
- ___ Coast Prairie Redox (A16) (LRR F, G, H)
- ___ Dark Surface (S7) (LRR G)
- ___ High Plains Depressions (F16)
  (LRR H outside of MLRA 72 & 73)
- ___ Reduced Vertic (F18)
- ___ Red Parent Material (TF2)
- ___ Very Shallow Dark Surface (TF12)
- ___ Other (Explain in Remarks)

[3]Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if present):

Type: ___

Depth (inches): ___

Hydric Soil Present? Yes ___ No ✓

Remarks: Immediate overbank not hydric soil, but assumed in channel

HYDROLOGY

Wetland Hydrology Indicators:

Primary Indicators (minimum of one required; check all that apply)

- ✓ Surface Water (A1)
- ___ High Water Table (A2)
- ___ Saturation (A3)
- ___ Water Marks (B1)
- ___ Sediment Deposits (B2)
- ___ Drift Deposits (B3)
- ___ Algal Mat or Crust (B4)
- ___ Iron Deposits (B5)
- ___ Inundation Visible on Aerial Imagery (B7)
- ___ Water-Stained Leaves (B9)
- ___ Salt Crust (B11)
- ___ Aquatic Invertebrates (B13)
- ___ Hydrogen Sulfide Odor (C1)
- ___ Dry-Season Water Table (C2)
- ___ Oxidized Rhizospheres on Living Roots (C3)
  (where not tilled)
- ___ Presence of Reduced Iron (C4)
- ___ Thin Muck Surface (C7)
- ___ Other (Explain in Remarks)

Secondary Indicators (minimum of two required)

- ___ Surface Soil Cracks (B6)
- ___ Sparsely Vegetated Concave Surface (B8)
- ___ Drainage Patterns (B10)
- ___ Oxidized Rhizospheres on Living Roots (C3)
  (where tilled)
- ___ Crayfish Burrows (C8)
- ___ Saturation Visible on Aerial Imagery (C9)
- ___ Geomorphic Position (D2)
- ___ FAC-Neutral Test (D5)
- ___ Frost-Heave Hummocks (D7) (LRR F)

Field Observations:

Surface Water Present? Yes ✓ No ___ Depth (inches): ___

Water Table Present? Yes ___ No ___ Depth (inches): ___

Saturation Present? (includes capillary fringe) Yes ___ No ___ Depth (inches): ___

Wetland Hydrology Present? Yes ✓ No ___

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks: standing water

WETLAND DETERMINATION DATA FORM – Great Plains Region

Project/Site: LOWER LOESON RANCH City/County: EL PASO Sampling Date: 3/24/20
Applicant/Owner: LOESON DEVELOPMENT State: CO Sampling Point: DP 3
Investigator(s): LIZ KLEIN Section, Township, Range: S 24 T 15 R 65 W
Landform (hillslope, terrace, etc.): PLAINS Local relief (concave, convex, none): ENTRENCHED CHANNEL Slope (%): 0-5%
Subregion (LRR): L REG Lat: 38.732 Long: -104.637 Datum: ____
Soil Map Unit Name: MANZANOLA CLAY LOAM NWI classification: PEM1C
Are climatic / hydrologic conditions on the site typical for this time of year? Yes ___ No ✓ (If no, explain in Remarks.)
Are Vegetation ___, Soil ___, or Hydrology ___ significantly disturbed? Are "Normal Circumstances" present? Yes ✓ No ___
Are Vegetation ___, Soil ___, or Hydrology ___ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

Hydrophytic Vegetation Present? Yes ___ No ✓
Hydric Soil Present? Yes ___ No ✓
Wetland Hydrology Present? Yes ✓ No ___

Is the Sampled Area within a Wetland? Yes ___ No ✓

Remarks: SEASONAL SNOWFALL AT 149%

VEGETATION – Use scientific names of plants.

	Absolute % Cover	Dominant Species?	Indicator Status
Tree Stratum (Plot size: 30')			
1. Elaeagnus angustifolia	30	Y	NI
2.			
3.			
4.			
= Total Cover			
Sapling/Shrub Stratum (Plot size: 15')			
1.			
2.			
3.			
4.			
5.			
= Total Cover			
Herb Stratum (Plot size: 5')			
1. Bromus inermis	40	Y	
2. Kochia sp	30	Y	NI
3.			
4.			
5.			
6.			
7.			
8.			
9.			
10.			
= Total Cover			
Woody Vine Stratum (Plot size: ___)			
1.			
2.			
= Total Cover			

% Bare Ground in Herb Stratum ____

Remarks: upland veg to WATER

Dominance Test worksheet:

Number of Dominant Species That Are OBL, FACW, or FAC (excluding FAC−): 6 (A)
Total Number of Dominant Species Across All Strata: 2 (B)
Percent of Dominant Species That Are OBL, FACW, or FAC: 0 (A/B)

Prevalence Index worksheet:

Total % Cover of:		Multiply by:
OBL species		x 1 =
FACW species		x 2 =
FAC species		x 3 =
FACU species		x 4 =
UPL species		x 5 =
Column Totals:	(A)	(B)

Prevalence Index = B/A = ____

Hydrophytic Vegetation Indicators:

✓ 1 - Rapid Test for Hydrophytic Vegetation
__ 2 - Dominance Test is >50%
__ 3 - Prevalence Index is ≤3.0[1]
__ 4 - Morphological Adaptations[1] (Provide supporting data in Remarks or on a separate sheet)
__ Problematic Hydrophytic Vegetation[1] (Explain)

[1] Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Hydrophytic Vegetation Present? Yes ___ No ✓

SOIL

Sampling Point: DC

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

Depth (inches)	Matrix		Redox Features					
	Color (moist)	%	Color (moist)	%	Type[1]	Loc[2]	Texture	Remarks
0-3'	10YR 5/3							
3-12''	10yR 5/3							

[1]Type: C=Concentration, D=Depletion, RM=Reduced Matrix, CS=Covered or Coated Sand Grains. [2]Location: PL=Pore Lining, M=Matrix.

Hydric Soil Indicators: (Applicable to all LRRs, unless otherwise noted.)

___ Histosol (A1)
___ Histic Epipedon (A2)
___ Black Histic (A3)
___ Hydrogen Sulfide (A4)
___ Stratified Layers (A5) (LRR F)
___ 1 cm Muck (A9) (LRR F, G, H)
___ Depleted Below Dark Surface (A11)
___ Thick Dark Surface (A12)
___ Sandy Mucky Mineral (S1)
___ 2.5 cm Mucky Peat or Peat (S2) (LRR G, H)
___ 5 cm Mucky Peat or Peat (S3) (LRR F)

___ Sandy Gleyed Matrix (S4)
___ Sandy Redox (S5)
___ Stripped Matrix (S6)
___ Loamy Mucky Mineral (F1)
___ Loamy Gleyed Matrix (F2)
___ Depleted Matrix (F3)
___ Redox Dark Surface (F6)
___ Depleted Dark Surface (F7)
___ Redox Depressions (F8)
___ High Plains Depressions (F16)
(MLRA 72 & 73 of LRR H)

Indicators for Problematic Hydric Soils[3]:

___ 1 cm Muck (A9) (LRR I, J)
___ Coast Prairie Redox (A16) (LRR F, G, H)
___ Dark Surface (S7) (LRR G)
___ High Plains Depressions (F16)
(LRR H outside of MLRA 72 & 73)
___ Reduced Vertic (F18)
___ Red Parent Material (TF2)
___ Very Shallow Dark Surface (TF12)
___ Other (Explain in Remarks)

[3]Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if present):

Type: ____________________

Depth (inches): ____________________

Hydric Soil Present? Yes ______ No ✓

Remarks: SOIL DRY ON BANK

HYDROLOGY

Wetland Hydrology Indicators:

Primary Indicators (minimum of one required; check all that apply)

✓ Surface Water (A1)
___ High Water Table (A2)
___ Saturation (A3)
___ Water Marks (B1)
___ Sediment Deposits (B2)
___ Drift Deposits (B3)
___ Algal Mat or Crust (B4)
___ Iron Deposits (B5)
___ Inundation Visible on Aerial Imagery (B7)
___ Water-Stained Leaves (B9)

___ Salt Crust (B11)
___ Aquatic Invertebrates (B13)
___ Hydrogen Sulfide Odor (C1)
___ Dry-Season Water Table (C2)
___ Oxidized Rhizospheres on Living Roots (C3)
(where not tilled)
___ Presence of Reduced Iron (C4)
___ Thin Muck Surface (C7)
___ Other (Explain in Remarks)

Secondary Indicators (minimum of two required)

___ Surface Soil Cracks (B6)
___ Sparsely Vegetated Concave Surface (B8)
___ Drainage Patterns (B10)
___ Oxidized Rhizospheres on Living Roots (C3)
(where tilled)
___ Crayfish Burrows (C8)
___ Saturation Visible on Aerial Imagery (C9)
___ Geomorphic Position (D2)
___ FAC-Neutral Test (D5)
___ Frost-Heave Hummocks (D7) (LRR F)

Field Observations:

Surface Water Present? Yes ✓ No _____ Depth (inches): 3"
Water Table Present? Yes _____ No _____ Depth (inches): __________
Saturation Present? (includes capillary fringe) Yes _____ No _____ Depth (inches): __________

ON CHANNEL BOTTOM ONLY

Wetland Hydrology Present? Yes ✓ No ______

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks: STANDING WATER BANK TO BANK

WETLAND DETERMINATION DATA FORM – Great Plains Region

Project/Site: LOWER LORSON Ranch City/County: EL PASO Sampling Date: 3/24/20
Applicant/Owner: LORSON DEVELOPMENT State: CO Sampling Point: DP 4
Investigator(s): Liz KLEIN Section, Township, Range: S 24 T15S R65W
Landform (hillslope, terrace, etc.): plains Local relief (concave, convex, none): ______ Slope (%): ______
Subregion (LRR): LRRG Lat: 38.733 Long: -104.634 Datum: ______
Soil Map Unit Name: MANZANOLA clay loam NWI classification: PEM1C
Are climatic / hydrologic conditions on the site typical for this time of year? Yes ______ No ✓ (If no, explain in Remarks.)
Are Vegetation ______, Soil ______, or Hydrology ______ significantly disturbed? Are "Normal Circumstances" present? Yes ✓ No ______
Are Vegetation ______, Soil ______, or Hydrology ______ naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

Hydrophytic Vegetation Present? Yes ✓ No ______
Hydric Soil Present? Yes ✓ No ______
Wetland Hydrology Present? Yes ✓ No ______

Is the Sampled Area within a Wetland? NARROW CHANNEL Yes ✓ No ______

Remarks:
SEASONAL SNOWfall at 149%
DOWNSTREAM OF TRAPEZOIDAL CHANNEL

VEGETATION – Use scientific names of plants.

	Absolute % Cover	Dominant Species?	Indicator Status
Tree Stratum (Plot size: 30')			
1.			
2.			
3.			
4.			
= Total Cover			
Sapling/Shrub Stratum (Plot size: 15')			
1. ELEAGNUS Augustifolia	15	Y	NI
2. Sumphoria [illegible]	50	Y	UPL
3.			
4.			
5.			
= Total Cover			
Herb Stratum (Plot size: 5')			
* 1. SCIRPUS sp	50	Y	OBL
2. GLYCERRUZA lepidota	10	N	FACU
* 3. Schoenoplectus pungens	50	Y	FACW
4.			
5.			
6.			
7.			
8.			
9.			
10.			
= Total Cover			
Woody Vine Stratum (Plot size: ______)			
1.			
2.			
= Total Cover			

% Bare Ground in Herb Stratum ______

Dominance Test worksheet: see herb * stratum only

Number of Dominant Species That Are OBL, FACW, or FAC (excluding FAC–): 2 (A)
Total Number of Dominant Species Across All Strata: 2 (B)
Percent of Dominant Species That Are OBL, FACW, or FAC: 100% (A/B)

Prevalence Index worksheet:

Total % Cover of:		Multiply by:
OBL species	______	x 1 = ______
FACW species	______	x 2 = ______
FAC species	______	x 3 = ______
FACU species	______	x 4 = ______
UPL species	______	x 5 = ______
Column Totals:	______ (A)	______ (B)

Prevalence Index = B/A = ______

Hydrophytic Vegetation Indicators:

- ___ 1 - Rapid Test for Hydrophytic Vegetation
- ___ 2 - Dominance Test is >50%
- ___ 3 - Prevalence Index is ≤3.0[1]
- ___ 4 - Morphological Adaptations[1] (Provide supporting data in Remarks or on a separate sheet)
- ___ Problematic Hydrophytic Vegetation[1] (Explain)

[1] Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Hydrophytic Vegetation Present? Yes ✓ No ______

Remarks: VEGETATION OUTSIDE OF CHANNEL DISREGARDED

SOIL

Sampling Point: ______

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

Depth (inches)	Matrix		Redox Features				Texture	Remarks
	Color (moist)	%	Color (moist)	%	Type[1]	Loc[2]		
0-5	10YR 5/2		none					
5-9	10YR 5/3							

[1]Type: C=Concentration, D=Depletion, RM=Reduced Matrix, CS=Covered or Coated Sand Grains. [2]Location: PL=Pore Lining, M=Matrix.

Hydric Soil Indicators: (Applicable to all LRRs, unless otherwise noted.)

___ Histosol (A1)
___ Histic Epipedon (A2)
___ Black Histic (A3)
___ Hydrogen Sulfide (A4)
___ Stratified Layers (A5) (LRR F)
___ 1 cm Muck (A9) (LRR F, G, H)
___ Depleted Below Dark Surface (A11)
___ Thick Dark Surface (A12)
___ Sandy Mucky Mineral (S1)
___ 2.5 cm Mucky Peat or Peat (S2) (LRR G, H)
___ 5 cm Mucky Peat or Peat (S3) (LRR F)
___ Sandy Gleyed Matrix (S4)
___ Sandy Redox (S5)
___ Stripped Matrix (S6)
___ Loamy Mucky Mineral (F1)
___ Loamy Gleyed Matrix (F2)
___ Depleted Matrix (F3)
___ Redox Dark Surface (F6)
___ Depleted Dark Surface (F7)
___ Redox Depressions (F8)
___ High Plains Depressions (F16) (MLRA 72 & 73 of LRR H)

Indicators for Problematic Hydric Soils[3]:

___ 1 cm Muck (A9) (LRR I, J)
___ Coast Prairie Redox (A16) (LRR F, G, H)
___ Dark Surface (S7) (LRR G)
___ High Plains Depressions (F16) (LRR H outside of MLRA 72 & 73)
___ Reduced Vertic (F18)
___ Red Parent Material (TF2)
___ Very Shallow Dark Surface (TF12)
___ Other (Explain in Remarks)

[3]Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if present):
Type: ______
Depth (inches): ______

Hydric Soil Present? Yes ✓ No ______

Remarks: ASSUMED wetland soil IN channel only
QUICKLY TRANSITIONS to upland

HYDROLOGY

Wetland Hydrology Indicators:

Primary Indicators (minimum of one required; check all that apply)

✓ Surface Water (A1)
___ High Water Table (A2)
✓ Saturation (A3)
___ Water Marks (B1)
___ Sediment Deposits (B2)
___ Drift Deposits (B3)
___ Algal Mat or Crust (B4)
___ Iron Deposits (B5)
___ Inundation Visible on Aerial Imagery (B7)
___ Water-Stained Leaves (B9)
___ Salt Crust (B11)
___ Aquatic Invertebrates (B13)
___ Hydrogen Sulfide Odor (C1)
___ Dry-Season Water Table (C2)
___ Oxidized Rhizospheres on Living Roots (C3) (where not tilled)
___ Presence of Reduced Iron (C4)
___ Thin Muck Surface (C7)
___ Other (Explain in Remarks)

Secondary Indicators (minimum of two required)

___ Surface Soil Cracks (B6)
___ Sparsely Vegetated Concave Surface (B8)
___ Drainage Patterns (B10)
___ Oxidized Rhizospheres on Living Roots (C3) (where tilled)
___ Crayfish Burrows (C8)
___ Saturation Visible on Aerial Imagery (C9)
___ Geomorphic Position (D2)
___ FAC-Neutral Test (D5)
___ Frost-Heave Hummocks (D7) (LRR F)

Field Observations:

Surface Water Present? Yes ✓ No ______ Depth (inches): 2"
Water Table Present? Yes ______ No ______ Depth (inches): ______
Saturation Present? (includes capillary fringe) Yes ______ No ______ Depth (inches): ______

Wetland Hydrology Present? Yes ✓ No ______

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks: saturation present in a NARROW Band
AT TOE OF SLOPE

DEPARTMENT OF DEFENSE
UNITED STATES OF AMERICA

REPLY TO
ATTENTION OF

DEPARTMENT OF THE ARMY
ALBUQUERQUE DISTRICT, U.S. ARMY CORPS OF ENGINEERS
SOUTHERN COLORADO REGULATORY OFFICE
200 S. SANTA FE AVENUE, SUITE 301
PUEBLO, COLORADO 81003

September 7, 2017

Regulatory Division

SUBJECT: Action No. SPA-2005-00757; Modification to the Lorson Ranch Permit in El Paso County, Colorado

Elizabeth Klein
Kiowa Engineering
1604 South 21st Street
Colorado Springs, CO 80904

Ms. Klein:

The U.S. Army Corps of Engineers (Corps) is in receipt of your letter dated August 3, 2017, requesting a modification to the Department of the Army permit for the discharge of dredged and fill material into waters of the United States associated with Lorson Ranch. This includes the bridge construction and stream configurations and updating delineation for upland swale in the Lorson ranch development, Fountain, El Paso County, Colorado.

We have reviewed and hereby approve your request. Action Number SPA-2005-00757 is modified as follows: This includes approval of the Special Condition 1 - Lorson Blvd. & Fontaine Blvd. bridge design and stream configuration, Special Condition 2 – no action required; and Upper Reach Item #2 Stabilization – No permit required.

Replace the project description on page one of your permit with: Insert the approved designs into the Permit as an attachment to the Special Condition 1.

The expiration date of your is still September 30, 2021.

This modification is effective immediately. All other terms and conditions of the original permit remain in full force and effect.

If you have any questions concerning this letter, please contact me at (719) 543-6915 or by e-mail at Van.A.Truan@usace.army.mil.

Sincerely,

TRUAN.VAN.A
LLAN.123142
2150

Digitally signed by TRUAN.VAN.ALLAN.1231422150
DN: c=US, o=U.S. Government, ou=DoD, ou=PKI, ou=USA, cn=TRUAN.VAN.ALLAN.1231422150
Date: 2017.09.07 09:15:45 -06'00'

Van Truan
Chief, Southern Colorado
Regulatory Branch

DEPARTMENT OF THE ARMY PERMIT

Permittee Lorson LLC nominee for Lorson Conservation Investment 1, LLLP

Permit No. 2005 00757

Issuing Office Albuquerque District Corps of Engineers

NOTE: The term "you" and its derivatives, as used in this permit, means the permittee or any future transferee. The term "this office" refers to the appropriate district or division office of the Corps of Engineers having jurisdiction over the permitted activity or the appropriate official of that office acting under the authority of the commanding officer.

You are authorized to perform work in accordance with the terms and conditions specified below.

Project Description: The work includes modifying the lower 3,110 linear feet of stream with bank protection while preserving the stream alignment (stream preservation reach), and reconfiguring the upper 5,825 linear feet of the stream (reconfiguration reach). Specifically:

In the lower stream preservation reach, about 3,110 linear feet will be treated on one or both banks by regrading the overbank to 3H:1V and treating with concrete or synthetic matting with seeded topsoil beneath the mat. About 350 linear feet will be treated with stone toe protection with soil coir lifts. One or two grade control structures may be built to provide protection from future channel incision.

In the upper reconfiguration reach, a breached stock pond dam will be removed. About 4,025 linear feet of the upper channel will be reconstructed with a bottom width of about 40 feet, side slopes no steeper than 6H:1V, and a natural channel bottom. The new channel side slopes will be protected with a mat material that will provide stability while allowing establishment of vegetation. Eleven boulder grade control structures will be built.

The upper 1,800 linear feet of the channel is actually an upland swale and is not a water of the U.S. However, it's channel design is included in the permit for clarity.

Two road crossings will be built in the upper reach for Lorson Boulevard and Fontaine Boulevard. These structures will be two or three concrete arch, natural bottom spans. A temporary construction crossing may be built in the upper stream portion.

The project will be constructed in accordance with the attached drawings, entitled, "Lorson Ranch channel modification in East Tributary of Jimmy Camp Creek near Fountain, El Paso County, Colorado, Application by: Lorson LLC, Application No. 2005 00757," sheets 1 through 16, dated May 17, 2006.

Project Location: In the East Tributary of Jimmy Camp Creek and adjacent wetlands in the east portion of the Lorson Ranch development located east of the intersection of Fountaine Boulevard and Marksheffel Road near Fountain, El Paso County, Colorado, Sections 13, 14 and 23, Township 15S, Range 65W (38° 44.1' N Latitude, 104° 37.9' W Longitude).

Permit Conditions:

General Conditions:

1. The time limit for completing the work authorized ends on December 31, 2009. If you find that you need more time to complete the authorized activity, submit your request for a time extension to this office for consideration at least one month before the above date is reached.

2. You must maintain the activity authorized by this permit in good condition and in conformance with the terms and conditions of this permit. You are not relieved of this requirement if you abandon the permitted activity, although you may make a good faith transfer to a third party in compliance with General Condition 4 below. Should you wish to cease to maintain the authorized activity or should you desire to abandon it without a good faith transfer, you must obtain a modification of this permit from this office, which may require restoration of the area.

3. If you discover any previously unknown historic or archeological remains while accomplishing the activity authorized by this permit, you must immediately notify this office of what you have found. We will initiate the Federal and state coordination required to determine if the remains warrant a recovery effort or if the site is eligible for listing in the National Register of Historic Places.

4. If you sell the property associated with this permit, you must obtain the signature of the new owner in the space provided and forward a copy of the permit to this office to validate the transfer of this authorization.

5. If a conditioned water quality certification has been issued for your project, you must comply with the conditions specified in the certification as special conditions to this permit. For your convenience, a copy of the certification is attached if it contains such conditions.

6. You must allow representatives from this office to inspect the authorized activity at any time deemed necessary to ensure that it is being or has been accomplished in accordance with the terms and conditions of your permit.

Special Conditions:

After a detailed and careful review of all of the conditions contained in this permit, the permittee acknowledges that, although said conditions were required by the Corps of Engineers, nonetheless the permittee agreed to those conditions voluntarily to facilitate issuance of the permit; the permittee will comply fully with all the terms of all the permit conditions.

1. Final bridge designs for Fontaine Boulevard and Lorson Boulevard will be submitted to the Corps of Engineers for review and approval 60 days prior to start of each bridge construction. Project construction of each structure may begin upon the Corps of Engineers' issuance of a start-of-work authorization.

2. The bank armoring for the stream preservation (lower) reach will be ungrouted stone toe with coir fabric lifts or similar materials. A final design for the stream preservation reach, including vegetation species list, will be submitted to the Corps of Engineers for review and

approval 60 days prior to start of bank armoring construction. Project construction may begin upon the Corps of Engineers' issuance of a start-of-work authorization.

3. The bank armoring for the reconfiguration (upper) reach will be armorflex, geogrid, or similar materials. The bank armoring will be covered with at least 6 inches of topsoil and seeded with grasses. The boulder grade control structures will be ungrouted. A final design for the reconfigured channel reach, including vegetation species list, will be submitted to the Corps of Engineers for review and approval 60 days prior to start of channel construction. Project construction may begin upon the Corps of Engineers' issuance of a start-of-work authorization.

4. Sloping boulder grade control structures will be ungrouted and designed to allow passage of small fish. For the stream preservation (lower) reach, the location of grade control structures and their design will be submitted to the Corps of Engineers for review and approval 60 days prior to the start of grade control structure construction.

5. Erosion control measures will be implemented to prevent upland erosion into the East Tributary of Jimmy Camp Creek. All upland areas disturbed by the permittee or their (sub)contractors located within 200 feet of the stream will be treated with erosion control measures including placing topsoil, seeding, and mulching within 21 calendar days after final grading or final earth disturbance or in accordance with the erosion control plan required by El Paso County. An erosion control plan or a summary of the County's approved plan will be provided to the Corps of Engineers within 60 days of permit issuance.

6. Noxious weeds will be controlled in all project-disturbed areas within 200 feet of the stream during the 5-year maintenance period. A plan for such control will be provided to the Corps of Engineers within 60 days of permit issuance, for review and approval.

7. A detailed mitigation plan will be provided to the Corps of Engineers within 60 days of permit issuance, for review and approval prior to start of project construction. Project construction may begin upon the Corps of Engineers' issuance of a start-of-work authorization. The plan will provide for the mitigation of the loss of 4.56 acres of wetland shrubs and the loss of riparian trees. The mitigation work will begin in the spring following winter construction (or in the fall following summer construction) and be completed within 6 months of project construction. The plan will include, but is not limited to, the following items:

- A typical cross section showing the area to be planted with shrubs and trees,
- Planting densities and number and species of trees,
- Methods and times of year for planting. (If willow stakes are used, they must be planted with no more than 6 inches of the stake exposed above the ground.) And,
- A plan for short and long term management and maintenance of the mitigation sites, including supplemental tree watering if needed,

replacement of failed plantings before the end of the 5-year monitoring period, and other contingency needs.

8. The mitigation efforts must be maintained for at least 5 years including 5 growing seasons or until the Corps of Engineers has determined that the mitigation efforts have been successful. Tree plantings will be deemed successful when 80% of the planted trees are alive at the end of the 5-year period. Willow shrub plantings will be deemed successful when 50% of the planted shrubs are alive at the end of the 5-year period.

9. An annual monitoring report of mitigation activities is required and will be sent to the Corps of Engineers by October 31 of each year. The monitoring report will include as a minimum:

- A drawing or sketch showing photographic monitoring points,
- Before and after photographs from fixed photographic location(s),
- A brief discussion of the overall success, any bare or problem areas, and a plan to remedy any problem areas.

10. A letter of intent from the local governing authority will be provided as financial assurances for construction, and for contingency and monitoring of the mitigation for the 5-year monitoring period. The assurances of the mitigation effort will be provided sufficient to hire an independent contractor to complete the proposed mitigation should the permittee default. The financial assurance for construction of the mitigation project will in an amount equal to 115 percent of the estimated cost of construction. The financial assurance for contingency and monitoring of the mitigation for the 5-year monitoring period will be in an amount equal to 25% of the construction costs and will be to assure the success of the mitigation. The letter of intent will be submitted to the Corps of Engineers, for approval, within 90 days of permit issuance.

11. Any changes to the project must be approved by the Corps of Engineers through a permit modification prior to the changes being implemented.

Further information:

1. Congressional Authorities: You have been authorized to undertake the activity described above pursuant to:

( ) Section 10 of the Rivers and Harbors Act of 1899 (33 U.S.C. 403).

(XX) Section 404 of the Clean Water Act (33 U.S.C. 1344).

( ) Section 103 of the Marine Protection, Research and Sanctuaries Act of 1972 (33 U.S.C. 1413).

2. Limits of this authorization.

a. This permit does not obviate the need to obtain other Federal, state, or local authorizations required by law.

b. This permit does not grant any property rights or exclusive privileges.

c. This permit does not authorize any injury to the property or rights of others.

d. This permit does not authorize interference with any existing or proposed Federal project.

3. Limits of Federal Liability. In issuing this permit, the Federal Government does not assume any liability for the following:

a. Damages to the permitted project or uses thereof as a result of other permitted or unpermitted activities or from natural causes.

b. Damages to the permitted project or uses thereof as a result of current or future activities undertaken by or on behalf of the United States in the public interest.

c. Damages to persons, property, or to other permitted or unpermitted activities or structures caused by the activity authorized by this permit.

d. Design or construction deficiencies associated with the permitted work.

e. Damage claims associated with any future modification, suspension, or revocation of this permit.

4. Reliance on Applicant's Data: The determination of this office that issuance of this permit is not contrary to the public interest was made in reliance on the information you provided.

5. Reevaluation of Permit Decision. This office may reevaluate its decision on this permit at any time the circumstances warrant. Circumstances that could require a reevaluation include, but are not limited to, the following:

a. You fail to comply with the terms and conditions of this permit.

b. The information provided by you in support of your permit application proves to have been false, incomplete, or inaccurate (See 4 above).

c. Significant new information surfaces which this office did not consider in reaching the original public interest decision.

Such a reevaluation may result in a determination that it is appropriate to use the suspension, modification, and revocation procedures contained in 33 CFR 325.7 or enforcement procedures such as those contained in 33 CFR 326.4 and 326.5. The referenced enforcement procedures provide for the issuance of an administrative order requiring you to comply with the terms and conditions of your permit and for the initiation of legal action where appropriate. You will be required to pay for any corrective measures ordered by this office, and if you fail to comply with such directive, this office may in certain situations (such as those specified in 33 CFR 209.170) accomplish the corrective measures by contract or otherwise and bill you for the cost.

6. Extensions. General condition 1 establishes a time limit for the completion of the activity authorized by this permit. Unless there are circumstances requiring either a prompt completion of the authorized activity or a reevaluation of the public interest decision, the Corps will normally give favorable consideration to a request for an extension of this time limit.

Your signature below, as permittee, indicates that you accept and agree to comply with the terms and conditions of this permit.

(PERMITTEE)

11 September [illegible]

(DATE)

This permit becomes effective when the Federal official, designated to act for the Secretary of the Army, has signed below.

Van A. Truan
Chief, Southern Colorado Regulatory Office
(for the DISTRICT ENGINEER)

22 September 2006
(DATE)

When the structures or work authorized by this permit are still in existence at the time the property is transferred, the terms and conditions of this permit will continue to be binding on the new owner(s) of the property. To validate the transfer of this permit and the associated liabilities associated with compliance with its terms and conditions, have the transferee sign and date below.

(TRANSFERREE)

(DATE)

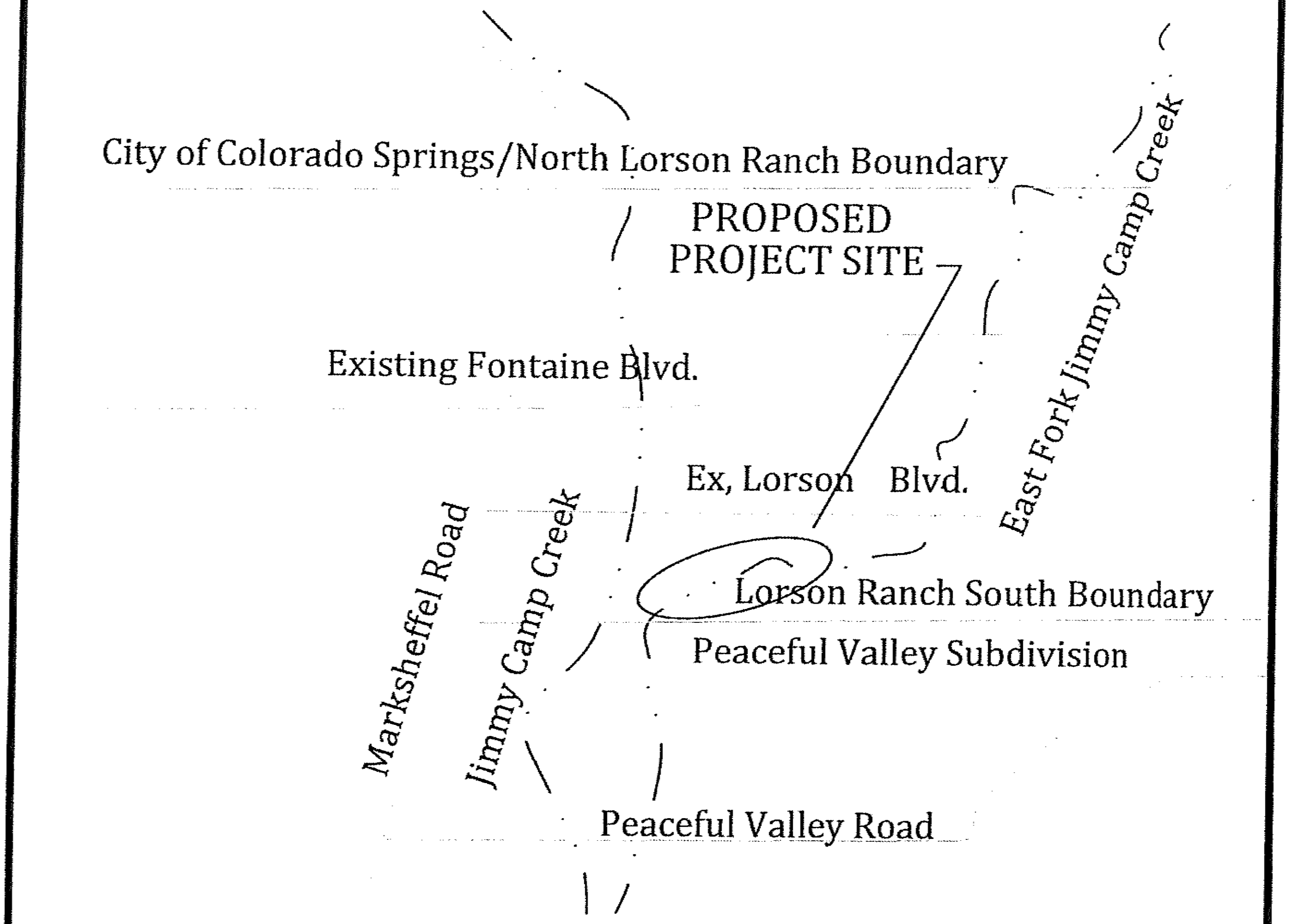
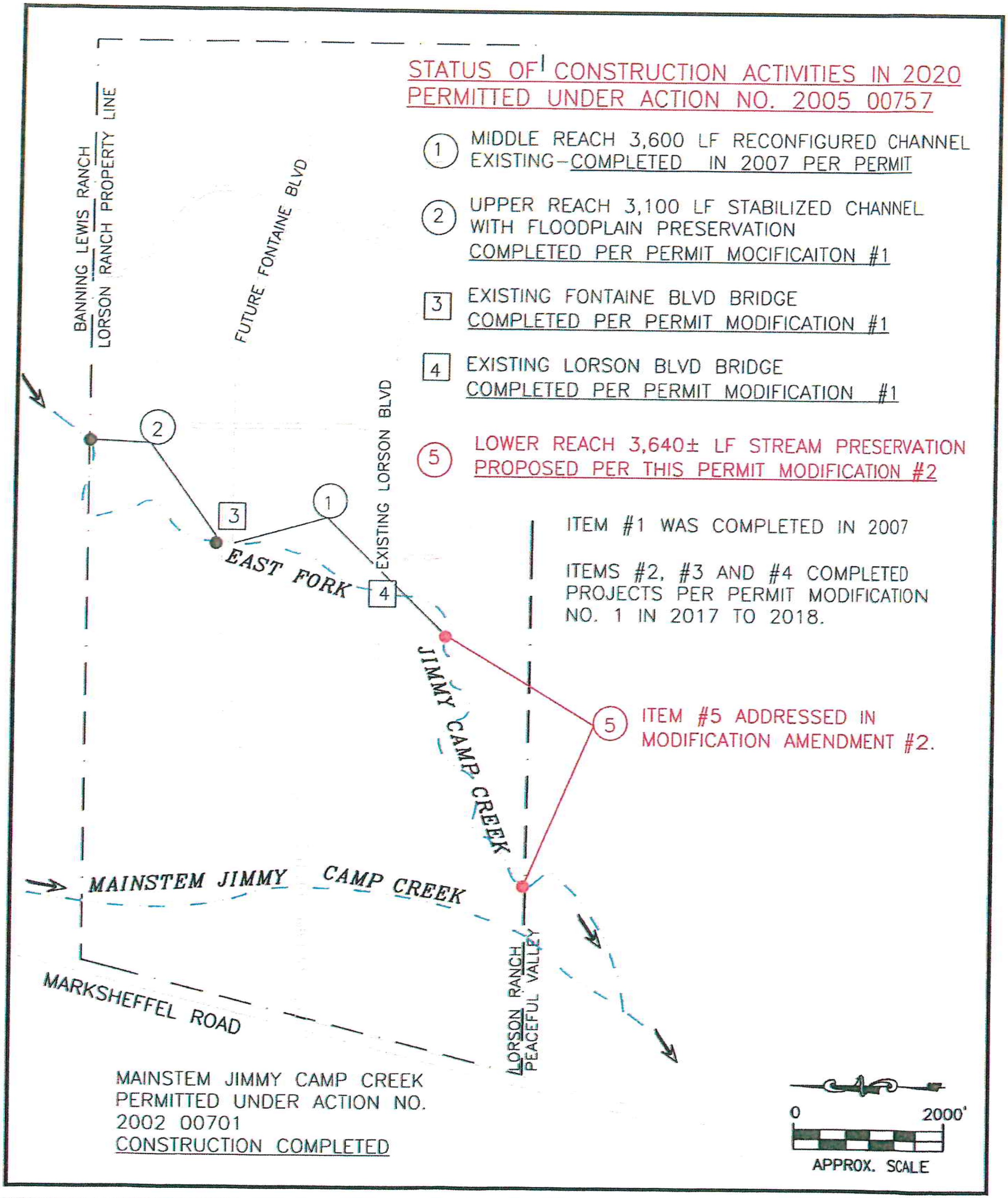


FIG. 1 VICINITY MAP

SCALE: N.T.S.



STATUS OF CONSTRUCTION ACTIVITIES IN 2020
PERMITTED UNDER ACTION NO. 2005 00757

① MIDDLE REACH 3,600 LF RECONFIGURED CHANNEL EXISTING-COMPLETED IN 2007 PER PERMIT

② UPPER REACH 3,100 LF STABILIZED CHANNEL WITH FLOODPLAIN PRESERVATION COMPLETED PER PERMIT MOCIFICAITON #1

$\boxed{3}$ EXISTING FONTAINE BLVD BRIDGE COMPLETED PER PERMIT MODIFICATION #1

$\boxed{4}$ EXISTING LORSON BLVD BRIDGE COMPLETED PER PERMIT MODIFICATION #1

⑤ LOWER REACH 3,640± LF STREAM PRESERVATION PROPOSED PER THIS PERMIT MODIFICATION #2

ITEM #1 WAS COMPLETED IN 2007

ITEMS #2, #3 AND #4 COMPLETED PROJECTS PER PERMIT MODIFICATION NO. 1 IN 2017 TO 2018.

ITEM #5 ADDRESSED IN MODIFICATION AMENDMENT #2.

MAINSTEM JIMMY CAMP CREEK PERMITTED UNDER ACTION NO. 2002 00701 CONSTRUCTION COMPLETED

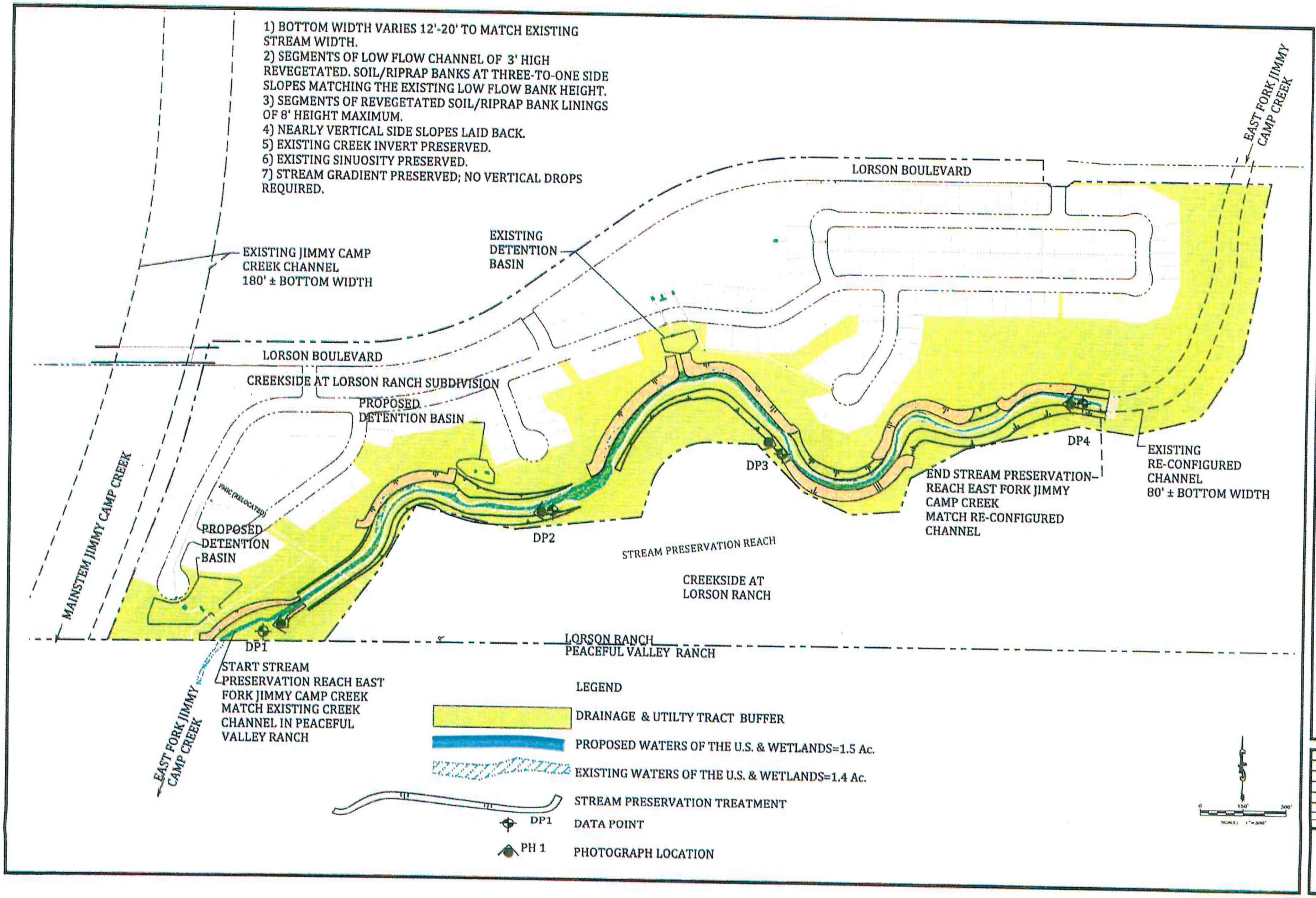
LORSON RANCH
PERMIT MODIFICATION AMENDMENT NO. 2 MAP
ACTION NO. 2005 00757
EL PASO COUNTY, COLORADO

DATE: 04/1/20
PROJECT NO. 18020

FIGURE 2

Kiowa
Engineering Corporation
1604 South 21st Street
Colorado Springs, Colorado 80904
(719) 630-7342

1) BOTTOM WIDTH VARIES 12'-20' TO MATCH EXISTING STREAM WIDTH.
2) SEGMENTS OF LOW FLOW CHANNEL OF 3' HIGH REVEGETATED. SOIL/RIPRAP BANKS AT THREE-TO-ONE SIDE SLOPES MATCHING THE EXISTING LOW FLOW BANK HEIGHT.
3) SEGMENTS OF REVEGETATED SOIL/RIPRAP BANK LININGS OF 8' HEIGHT MAXIMUM.
4) NEARLY VERTICAL SIDE SLOPES LAID BACK.
5) EXISTING CREEK INVERT PRESERVED.
6) EXISTING SINUOSITY PRESERVED.
7) STREAM GRADIENT PRESERVED; NO VERTICAL DROPS REQUIRED.



LEGEND

🟩 DRAINAGE & UTILTY TRACT BUFFER

PROPOSED WATERS OF THE U.S. & WETLANDS=1.5 Ac.

▨ EXISTING WATERS OF THE U.S. & WETLANDS=1.4 Ac.

STREAM PRESERVATION TREATMENT

DP1 DATA POINT

PH 1 PHOTOGRAPH LOCATION

Kiowa
Engineering Corporation
1604 South 21st Street
Colorado Springs, Colorado 80904
(719) 630-7342

LORSON RANCH
CREEKSIDE DEVELOPMENT
EAST FORK JIMMY CAMP CREEK
WATERS OF THE U.S. & WETLANDS
EL PASO COUNTY, COLORADO

Project No.: 18020
Date: 3/2020
Design: EAK
Drawn: EAK
Check:
Revisions:

Exh.1

18020 Cbw Exhibit.dwg/Apr 01, 2020/11:08am

Appendix C

Creekside at Lorson Ranch Filing No., 1- Geotechnical Report

NCRS Soil Survey

ARCHITECTS
RMG
ENGINEERS
ROCKY MOUNTAIN GROUP

GEOLOGY AND SOILS REPORT

Creekside at Lorson Ranch, Filing No. 1
El Paso County, Colorado

PREPARED FOR:

Lorson Ranch Metropolitan District No.1
212 N. Wahsatch Ave, Ste. 301
Colorado Springs, CO 80903

JOB NO. 164808

August 10, 2018

Respectfully Submitted,	**Reviewed by,**
RMG – Rocky Mountain Group	**RMG – Rocky Mountain Group**
Kelli Zigler	
Kelli Zigler **Project Geologist**	**Geoff Webster, P.E.** **Sr. Geotechnical Project Manager**

COLORADO LICENSED PROFESSIONAL ENGINEER
GEOFF WEBSTER
45046
8/10/18

TABLE OF CONTENTS

FIGURES

1.0 GENERAL SITE AND PROJECT DESCRIPTION

1.1 Project Location

The project lies in the northeast portion of Section 23, Township 15 South, Range 65 West of the 6th Principal Meridian in El Paso County, Colorado. The approximate location of the site is shown on the Site Vicinity Map, Figure 1.

1.2 Existing Land Use

The site currently consists of portions of ree parcels. The combined total area of the proposed site is to be 83.085 acres. The three parcels included are:

- Schedule No. 5500000265 which consists of 48.88 acres and is located on the northern portion of the site. The parcel is currently not developed.
- Schedule No. 5500000267 which consists of 18.87 acres and is located along the northern portion of Jimmy Camp Creek "east tributary". The parcel is currently not developed.
- A portion of Schedule No. 5500000406 which consists of 15.335 acres and is located along the southern bank of Jimmy Camp Creek "east tributary". The parcel is currently not developed.

The parcels are zoned "PUD" (Planned Unit Development).

The Jimmy Camp Creek "east tributary" is included in this development, but is to be platted outside of the buildable lots.

1.3 Project Description

The majority of the site is to be developed as a single-family residential subdivision and is proposed to contain 235 single family lots. The proposed development will consist of the replat of portions of the three existing parcels into one parcel with 83.085 acres.

Rocky Mountain Group - RMG was retained to explore the subsurface conditions at the site and develop geotechnical engineering recommendations for the proposed land development operations.

2.0 QUALIFICATIONS OF PREPARERS

This Geology and Soils report was prepared by a professional geologist as defined by Colorado Revised Statutes section 34-1-201(3) and by a qualified geotechnical engineer as defined by policy statement 15, "Engineering in Designated Natural Hazards Areas" of the Colorado State Board of Registration for Professional Engineers and Professional Land Surveyors. (Ord. 96-74; Ord. 01-42)

The principle investigators for this study are Kelli Zigler, P.G. and Geoff G. Webster, P.E. Ms. Zigler is a Professional Geologist as defined by State Statute (C.R.S 34-1-201) with over18 years of experience in the geological and geotechnical engineering field. Ms. Kelli Zigler holds a B.S. in Geology from the University of Tulsa. Ms. Zigler has supervised and performed numerous geological and geotechnical field investigations in Colorado.

Geoff Webster, P.E. is a licensed Professional Engineer with over 33 years of experience in the structural and geotechnical engineering fields. Mr. Webster is a professional engineer and holds a Master's degree from the University of Central Florida. Mr. Webster has supervised and performed numerous geological and geotechnical field investigation programs in Colorado and other states.

3.0 STUDY OVERVIEW

The purpose of this investigation is to characterize the general geotechnical and geologic site conditions, and present our opinions of the potential effect of these conditions on the proposed development of single-family residences within the referenced site. As such, our services exclude evaluation of the environmental and/or human, health-related work products or recommendations previously prepared, by others, for this project.

Revisions to the conclusions presented in this report may be issued based upon submission of the development plan. This study has been prepared in accordance with the requirements outlined in the El Paso County Land Development Code (LDC) specifically Chapter 8 last updated 01/06/2015 applicable sections include 8.4.9. and the Engineering Criteria Manual (ECM), specifically Appendix C last updated July 29, 2015.

This report presents the findings of the study performed by RMG relating to the geotechnical and geologic conditions of the above-referenced site. Revisions and modifications to the conclusions and recommendations presented in this report may be issued subsequently by RMG based upon additional observations made during grading and construction which may indicate conditions that require re-evaluation of some of the criteria presented in this report.

3.1 Scope and Objective

This report presents the findings of our Geology and Soils Investigation for the Creekside at Lorson Ranch, Filing No. 1 development located in southern El Paso County, Colorado.

The purpose of our report is to adhere to the guidelines outlined in Appendix C of the ECM and Chapter 8.4.9 of the LDC. The occurrences of potential geologic hazards were evaluated and our opinions of the observed conditions on the proposed development with the respect to the intended usage are outlined in this report.

This report presents the findings of the study performed by RMG-Rocky Mountain Group (RMG) relating to the geology and soil conditions of the above-referenced site.

3.2 Site Evaluation Techniques

The information included in this report has been compiled from:

- Field reconnaissance
- Geologic and topographic maps
- Review of selected publicly available, pertinent reports
- Available aerial photographs
- Exploratory borings
- Laboratory testing of representative site soil and rock samples

- Geologic research and analysis
- Site development plans prepared by others

Geophysical investigations were not considered necessary for characterization of the site geology. Monitoring programs, which typically include instrumentation and/or observations for changes in groundwater, surface water flows, slope stability, subsidence, and similar conditions, are not known to exist and were not considered applicable for the scope of this report.

3.3 Previous Studies and Field Investigation

Reports of previous geotechnical engineering/geologic investigations for this site were available for our review and are listed below:

1. *Preliminary Site Grading and Erosion Control plans for Creekside at Lorson Ranch, Filing No. 1, El Paso County, Colorado,* prepared by Core Engineering Group, LLC, Project No. 100.045 dated August, 2018.
2. *FIRM, Flood Insurance Rate Map, El Paso County, Colorado and Incorporated Areas*, Parcel 957 of 1300, Map No. 08041CO957F and 08041C1000F dated March 17, 1997, modified per LOMR Case No. 14-08-0534P.
3. *Preliminary Drainage Plan for Creekside at Lorson Ranch, Filing No. 1, El Paso County, Colorado*, prepared by Core Engineering Group, LLC, Project No. 100.045, August, 2018.
4. *PUD and Preliminary Plan, Creekside at Lorson Ranch, Filing No. 1, El Paso County, Colorado*, prepared by Thomas and Thomas.

4.0 SITE CONDITIONS

4.1 Proposed Land Use and Zoning

It is our understanding that the project is to consist of single-family residential construction on 235 lots at the Creekside at Lorson Ranch, Filing No. 1 subdivision. The residential structures are anticipated to be one to two-stories in height with multi-car garages. The homes may be constructed with or without basements.

Figure 2 presents the general boundaries of our investigation.

4.2 Topography

Based on our site observations, the ground surface generally slopes gently down to the south and southwest across the entire site. The elevation difference across the site from northeast to southwest is approximately 16 to 20 feet. The Jimmy Camp Creek "east tributary" runs along the southern property line and Jimmy Camp Creek runs parallel to the western property line. The Jimmy Camp Creek "east tributary" was dry at the time of the site reconnaissance on July 23, 2018.

4.3 Vegetation

The majority of the site consists of tall native grasses and weeds. Deciduous trees and vegetation are denser along the Jimmy Camp Creek "east tributary".

5.0 FIELD INVESTIGATION

5.1 Drilling

The subsurface conditions within the property were explored by drilling twelve exploratory borings on June 25, 2018 extending to depths of approximately 25 to 30 feet below the existing ground surface. The test borings were performed to explore the subsurface soils underlying the site. The number of borings is in excess of the minimum one test boring per 10 acres of development up to 100 acres and one additional boring for every 25 acres of development above 100 acres as required by the ECM, Section C.3.3.

The test borings were drilled with a power-driven, continuous-flight auger drill rig. Samples were obtained during drilling of the test borings in general accordance with ASTM D-1586 utilizing a 2-inch O.D. Split Barrel sampler. Results of the penetration tests are shown on the drilling logs. The Test Boring are presented in Figures 6 through 11.

5.2 Laboratory Testing

Soil laboratory testing was performed as part of this investigation. The laboratory tests included moisture content, dry density, grain-size analyses, Atterberg Limits and Swell/Consolidation tests. A Summary of Laboratory Test Results is presented in Figure 12. Soils Classification Data is presented in Figures 13 and 15. Swell/Consolidation Test Results are presented in Figures 16 through 18.

6.0 GEOLOGIC AND SUBSURFACE CONDITIONS

6.1 Geologic Conditions

Based upon review of the *Geologic Map of the Fountain Quadrangle, El Paso County, Colorado* the site reconnaissance and exploratory drilling, the site and surrounding area generally consists of a silty to clayey sand and sandy clay overlying the Pierre Shale Formation. The Pierre Shale was not encountered in the Test Borings at the time of drilling.

6.2 General Geology

Our field investigation included a site reconnaissance with consideration given to geologic features and significant surficial deposits. The general geology of the area is typically stream terrace deposits and alluvium soils overlying the Pierre Shale. Three general geology units were mapped in the vicinity of the site and are identified (Morgan, et al., 2003) as:

- af: Man-placed fill – associated with the removal of the existing structures after the Black Forest fire.

- al: alluvium is loose, unconsolidated (not cemented together into a solid rock) soil or sediments, which has been eroded, reshaped by water in some form, and redeposited in a non-marine setting. Alluvium is typically made up of a variety of materials, including fine particles of silt and clay and larger particles of sand and gravel.

- Kp: Pierre Shale – (Upper Cretaceous) Underlain by the Piney Creek Alluvium. Permeability is generally low, excavation and compaction generally easy. Foundation stability is less than fair. The majority of the formation has low to high swell potential. Slope stability is generally poor and slopes steeper than 5 degrees may slide, if the toe of the slope is removed.

  The General Geology is presented in the Geologic Conditions Map, Figure 21.

6.3 U.S. Soil Conservation Service

The U.S. Soil Conservation Service along with United States Department of Agriculture (USDA) has identified the soils on the property as:

- 10 – Blendon sandy loam, 0 to 3% slopes. Properties of the sandy loam include, well-drained soils, depth of the water table is anticipated to be greater than 6.5 feet, run-off is anticipated to be low, frequency of flooding and/or ponding is none, and landforms include alluvial fans and terraces.

- 40 – Ellicott loamy coarse sand, 0 to 5% slopes. Properties of the loamy sand include, somewhat excessively drained soils, depth of the water table is anticipated to be greater than 6.5 feet, run-off is anticipated to be very low, frequency of flooding is frequent and ponding is none, and landforms include flood plains and stream terraces.

- 52 – Manzanst clay loam, 0 to 3 percent slopes. Properties of the clay loam include, well-drained soils, depth of the water table is anticipated to be greater than 6.5 feet, runoff is anticipated to be low, frequency of flooding and/or ponding is none, and landforms include terraces and drainage-ways.

  The USDA Soil Survey Map is presented in Figure 19.

6.4 Subsurface Materials

The subsurface materials encountered in the test borings were classified using the Unified Soils Classification System (USCS) and the materials were grouped into the general categories of silty to clayey sand (SM and SC), sandy silt (ML) and sandy clay (CL and CH).

Additional descriptions and the interpreted distribution (approximate depths) of the subsurface materials are presented on the Test Boring Logs presented in Figures 6 through 11. The classifications shown on the logs are based upon the engineer's classification of the samples at the depths indicated. Stratification lines shown on the logs represent the approximate boundaries between material types and the actual transitions may be gradual and vary with location.

6.5 Bedrock Conditions

Bedrock was not encountered in the test borings for this investigation. The bedrock beneath the site is considered to be part of the Pierre Shale Formation and consists of sandy claystone, silty sandstone and shale.

6.6 Structural Features

Structural features such as schistocity, folds, zones of contortion or crushing, joints, shear zones or faults were not observed on the site, surrounding the site or in the soil samples collected for laboratory testing.

6.7 Surficial (Unconsolidated) Deposits

Various lake and pond sediments, swamp accumulations, sand dunes, marine and non-marine terrace deposits, talus accumulations, creep or slope wash were not observed along the Jimmy Camp Creek "east tributary" or elsewhere on the site. Slump and slide debris were not observed on the site.

6.8 Drainage of Water and Groundwater

The overall topography of the site slopes down to the south and west towards Jimmy Camp Creek "east tributary". Groundwater was encountered in all twelve of the test borings at depths ranging from approximately 14 to 26 feet at the time of drilling. When checked 29 days subsequent to drilling groundwater was encountered in at depths ranging from approximately 12 to 23 feet below the existing ground surface.

The Jimmy Camp Creek "east tributary" is currently a defined drainage way located along the southern property line of the property. Review of the historical photos provided by Google Earth depict that the Jimmy Camp Creek "east tributary" adjacent to the site has remained in its native state since at least 1999.

6.9 Features of Special Significance

Features of special significance such as accelerated erosion, (advancing gully head, badlands or cliff reentrants) were not observed on the property. Features indicating settlement or subsidence such as fissures, scarplets and offset reference features were also not observed on the property.

Features indicating creep, slump or slide masses in bedrock and surficial deposits were also not observed on the property.

6.10 Engineering Geology

The Engineering Geology is presented below. Charles Robinson and Associates have mapped two environmental engineering units the site as:

- 2A: Stable alluvium, colluvium and bedrock on gentle to moderate slopes (5-12%).
- 7A: Physiographic floodplain where erosion and deposition presently occur and is generally subject to recurrent flooding. Includes 100-year along major streams where floodplain studies have been conducted and Base Flood Elevations have been determined.

The Engineering Geology is presented in the Geologic Conditions Map in Figure 20.

6.11 Mineral Resources

Under the provision of House Bill 1529, it was made a policy by the State of Colorado to preserve for extraction commercial mineral resources located in a populous county. Review of the *Master Plan for*

Mineral Extraction, *Map 2* indicates the site is not identified as an aggregate resource. Extraction of the sand and sandstone resources are not considered to be economical compared to materials available elsewhere within the county.

6.12 Permeability

The permeability of a soil measures how well air and water can flow within the soil. Soil permeability varies according to the type of soil and other factors.

The infiltration rate of a soil refers to how much water a type of soil can absorb over a specific time period. Infiltration rates are determined by soil permeability and surface conditions, and usually are measured in inches per hour.

The soils encountered in the test borings, at the time of drilling were silty to clayey sand and sandy clay. The permeability of the sands is anticipated to be moderate to high. The permeability of the clay is anticipated to be low.

7.0 POTENTIAL GEOLOGIC CONDITIONS

The El Paso County Engineering Criteria Manual recognizes and delineates the difference between hazards and constraints. A geologic hazard is one of several types of adverse geologic conditions capable of causing significant damage or loss of property and life. Geologic hazards are defined in Section C.2.2 Sub-section E.1 of the ECM. A geologic constraint is one of several types of adverse geologic conditions capable of limiting or restricting construction on a particular site. Geologic constraints are defined in Section C.2.2 Sub-section E.2 of the ECM. The following sections discuss potential geologic conditions that commonly exist within El Paso County, Colorado.

7.1 Landslides

Landslides are a form of mass wasting slope failure that consists of relatively rapid downward sliding, falling, or flowing of a mass of soil, rock, or a mixture of the two. Landslides typically have one or more distinct failure surfaces. They typically occur on slope sides where the shear strength of a material is exceeded by the driving mass or weight of the material and may be induced by the presence of groundwater, heavy precipitation, and seismic events.

The entire area appears to lie outside the mapped areas of previous landslide and/or unstable slopes according to the electronic (online) version of the Colorado Landside Inventory map prepared by the Colorado Geological Survey (CGS) located at:

https://cologeosurvey.maps.arcgis.com/apps/webappviewer/index.html?id=9dd73db7fbc34139abe51599396e2648

Neither unstable slopes nor apparent signs of ongoing slope movement were observed on the property.

7.2 Rockfall

Rockfall is the falling of a newly detached mass of rock from a cliff or down a very steep slope, and is considered to be a type of landslide with a very rapid rate of down-slope movement. It usually occurs on mountainsides or other steep slopes during periods of abundant moisture and frequent freeze-thaw cycles, and is caused by the loss of support from underneath or detachment from a larger rock mass. Ice wedging, root growth, or ground shaking, erosion or chemical weathering may start the fall. The rocks may freefall, bounce, tumble, roll, or slide down slope and can vary considerably in size.

The subject site does not have steep slopes with large boulders above or around it to generate rockfall. The subject property is not considered to be prone to rockfall.

7.3 Debris Flow and Debris Fans

Debris flows consist of water with a high sediment load of sand, cobbles and boulders flowing down a stream, ravine, canyon, arroyo or gully, and are typically activated by heavy or long-term rains or snowmelts which cause rapid erosion and transport of surficial materials down slope of drainages. Debris fans are created when debris flows reach a valley with a much lower gradient. As the energy level drops, the sediment load is deposited creating the fan shape.

The potential for the development of significant debris flows was not observed on the surface of the property.

7.4 Faults and Seismicity

Review of the *Geologic Map of the Colorado Springs Quadrangle* and *Map of Areas Susceptible to Differential Heave in Expansive, Steeply Dipping Bedrock, City of Colorado Springs, Colorado* indicates the Ute Pass Fault lies approximately 10 miles to the west of the proposed residential development. According to the CGS, these faults are not considered to be recently active. However, they have been active during geologic times and could affect the site if they did rupture.

Information presented by the CGS indicates that several recent earthquakes have occurred in the vicinity of the Ute Pass Fault near Colorado Springs and Woodland Park. The earthquakes, with magnitudes in the range of 3.0 to 3.9, occurred approximately from 1962 to 2007.

Earthquakes felt at this site will most likely result from minor shifting of the granite mass within the Pikes Peak Batholith which includes pull from minor movements along faults found in the Denver basin. Ground motions resulting from small earthquakes are more likely to affect structures at this site and will likely only affect slopes stability to a minimal degree.

In accordance with the International Building Code, 2012/2015, seismic design parameters have been determined for this site. The Seismic Site Class has been interpreted from the results of the soil test boring drilled within the project site. The USGS seismic design tool has been used to determine the seismic response acceleration parameters. USGS output is presented in Appendix B. The soil on this site is not considered susceptible to liquefaction. The following recommended Seismic Design Parameters are based upon Seismic Site Class D, and a 2 percent probability of exceedance in 50 years. The Seismic Design Category is “B”.

Period (sec)	Mapped MCE Spectral Response Acceleration (g)		Site Coefficients		Adjusted MCE Spectral Response Acceleration (g)		Design Spectral Response Acceleration (g)	
0.2	S_s	0.168	F_a	1.6	S_{ms}	0.268	S_{ds}	0.179
1.0	S_1	0.059	F_v	2.4	S_{m1}	0.142	S_{d1}	0.095

Notes: MCE = Maximum Considered Earthquake
g = acceleration due to gravity

The USGS Seismic Output is presented in Appendix B.

7.5 Steeply Dipping Bedrock

Steeply dipping bedrock is a geological hazard common along the Rocky Mountain Front Range piedmont where uplifted sedimentary formations containing thin layers of moderately to highly expansive shale are encountered near the ground surface e.g., Noe and Dodson 1995; Noe 1997. Problematic formations in the region, most notably the Pierre Shale, are characterized by relatively thin vertically oriented beds that can exhibit dissimilar swelling characteristics from one particular bed to the next.

The site is lies outside of the mapped zone of areas susceptible to differential heave in expansive steeply dipping bedrock. Bedrock was not encountered in the test borings drilled for this investigation. Indications of dipping bedrock were not observed in the soil samples collected. The site is generally not considered to be prone to steeply dipping bedrock.

7.6 Unstable or Potentially Unstable Slopes

Slope stability is the potential of soil covered slopes to withstand and undergo movement. The stability of a slope is determined by the balance of shear stress and shear strength. Previously stable slopes may initially be affected by preparatory factors, making the slope conditionally unstable. Factors that may trigger a slope failure may be climatic events that can make a slope actively unstable, leading to mass movements. Mass movements can be caused by an increase in shear stress, such as loading, lateral pressure, and transient forces. Alternatively, shear strength may be decreased by weathering, changes in pore water pressure, and organic material.

According to the LDC, Chapter 8.4.2 Section B.3 Unsuitable Building Areas, areas that are identified as having certain characteristics "*... shall be deemed unsuitable for building and shall be identified as no build areas on the plat.*" One such characteristic is *"Areas where slopes are greater than 30%."* These areas have typically been designated as "No Build" areas in the recent past.

Unstable slopes greater than 30 percent or apparent signs of ongoing slope movement were not observed around or on the property. The subject site is also not in an area identified as containing unstable slopes in the Colorado Landslide Inventory map referenced in section 7.1 of this report.

Mitigation

Long term fill slopes should be limited to areas supported by foundation walls or other engineered components, unless adequately benched into the bedrock. Long term cut slopes in the upper soil should be limited to no steeper than 3:1 (horizontal:vertical).

We believe the surficial soils will classify as Type C materials as defined by OSHA in 29CFR Part 1926, date January 2, 1990. OSHA requires temporary slopes made in Type C materials be laid back at ratios no steeper than 1.5:1 (horizontal to vertical) unless the excavation is shored or braced. Flatter slopes will likely be necessary should groundwater conditions occur.

7.7 Ground Subsidence

Subsidence is the motion of the ground surface (usually, the Earth's surface) as it shifts downward relative to a datum such as sea-level.

Common causes of land subsidence from human activity are pumping water, oil, and gas from underground reservoirs; dissolution of limestone aquifers (sinkholes); collapse of underground mines; drainage of organic soils; and initial wetting of dry soils (hydrocompaction).

The presence of sinkholes and collapse were not observed on the site. The site lies outside of the Colorado Springs Subsidence Investigation report (Dames and Moore, 1985). Evidence of underground mining in the presence of coal was not encountered in the test boring samples. The site is generally not considered to be prone to ground subsidence.

7.8 Hydrocompactive and Potentially Expansive Soils (Moisture Sensitive Soils)

The subsurface materials at the site generally consist of silty to clayey sand and sandy clay. Based on the test borings performed on site, the silty to clayey sand and sandy clay generally possess low swell potential. Expansive bedrock was not identified on this site. It is anticipated that if these materials are encountered can readily be mitigated with typical construction practices common to this region of El Paso County, Colorado.

Mitigation

Shallow foundations are anticipated for structures within this development. Foundation design and construction are typically adjusted for expansive soils. Mitigation of expansive soils are typically accomplished by overexcavation and replacement with structural fill, subexcavation and/or replacement with on-site moisture-conditioned soils. If loose sands are encountered, mitigation of hydrocompactive soils can be accomplished by overexcavation and replacement with structural fill, subexcavation and replacement with on-site moisture-conditioned soils, and/or the use of a geogrid reinforced fill.

7.9 Radon

*"**Radon Act 51** passed by Congress set the natural outdoor level of radon gas (0.4 pCi/L) as the target radon level for indoor radon levels.*

The 80925 zip code located in El Paso County, has an EPA assigned Radon Zone of 1. A radon zone of 1 predicts an average indoor radon screening level greater than 4 pCi/L, which is above the recommended levels assigned by the EPA. Black Forest is located in a high risk area of the country. *The EPA recommends you take corrective measures to reduce your exposure to radon gas.*

Most of Colorado is generally considered to have the potential of high levels of radon gas, based on the information provided at: http://county-radon.info/CO/El_Paso.html. There is not believed to be unusually hazardous levels of radon from naturally occurring sources at this site.

Mitigation
Radon hazards are best mitigated at the building design and construction phases. Providing increased ventilation of basements, crawlspaces, creating slightly positive pressures within structures, and sealing of joints and cracks in the foundations and below-grade walls can help mitigate radon hazards.

7.10 Flooding and Surface Drainage

The Jimmy Camp Creek "east tributary" resides along the southern property boundary. The Flood Insurance Study report and Flood Insurance Rate Map for FEMA Map Number 08041C0957 dated March 17, 1997, has been modified per LOMR Case No. 14-08-0534P.

The Jimmy Camp Creek "east tributary" resides in Zone AE, which is defined by FEMA as areas subject to inundation by the 1-percent-annual chance-flood event determined by detailed methods. This area is shown hatched on the Geologic Conditions Map, Figure 21

The remainder of the site now lies in the Zone X. Zone X is defined by FEMA as an area of minimal flood hazard that is determined to be outside the Special Flood Hazard Area and higher than the elevation of the 0.2-percent-annual-chance (or 500-year) flood.

7.11 Springs and High Groundwater

Based on the site observations, review of the Fountain Quadrangle of El Paso County, 7.5 minute series (Topographic) dated 2000, and Google Earth images dating back to September 1999, springs do not appear to originate on the subject site. Groundwater was encountered at depths ranging from 12 to 23 feet in the test borings for this investigation at the time of drilling and when checked 29 days subsequent to drilling.

Fluctuations in groundwater and subsurface moisture conditions may occur due to variations in rainfall and other factors not readily apparent at this time. Development of the property and adjacent properties may also affect groundwater levels.

Mitigation:
If shallow groundwater conditions are encountered during the Site Specific Soils Investigations and Open Excavation Observations, mitigations can include a combination of surface and subsurface drainage systems, vertical drainboard, etc.

In general, if groundwater was encountered within 4 to 6 feet of the proposed basement slab elevation, an underslab drain should be anticipated in conjunction with the perimeter drain. Perimeter drains are anticipated for each individual lot to prevent the infiltration of water and to help control wetting of potentially expansive and hydrocompactive soils in the immediate vicinity of foundation elements. It must be understood that the drain is designed to intercept some types of subsurface moisture and not others. Therefore, the drain could operate properly and not mitigate all moisture problems relating to foundation performance or moisture intrusion into the basement area.

7.12 Erosion and Corrosion

The upper sands encountered at the site are susceptible to erosion by wind and flowing water. The sandstone at this site typically has low resistivity values (less than 2,000 ohm-cm) and is likely to be potentially corrosive to buried, ferrous metal piping and other structures.

Mitigation:
Due to the nature of the soils on the site it is anticipated that the majority of the surficial soils (silty to clayey sand) is subject to erosion by wind or water. The majority of the site has low lying vegetation that is reducing the potential for erosion. During construction disturbance of the site most likely will occur around the buildings site and may require regrading and revegetation. Further recommendations for Erosion Control are discussed in section 7.15

7.13 Surface Grading and Drainage

The ground surface should be sloped from the buildings with a minimum gradient of 10 percent for the first 10 feet. This is equivalent to 12 inches of fall across this 10-foot zone. If a 10-foot zone is not possible on the upslope side of the structure, then a well-defined swale should be created a minimum 5 feet from the foundation and sloped parallel with the wall with a minimum slope of 2 percent to intercept the surface water and transport it around and away from the structure. Roof drains should extend across backfill zones and landscaped areas to a region that is graded to direct flow away from the structure. Homeowners should maintain the surface grading and drainage recommended in this report to help prevent water from being directed toward and/or ponding near the foundations.

Landscaping should be selected to reduce irrigation requirements. Plants used close to foundation walls should be limited to those with low moisture requirements and irrigated grass should not be located within 5 feet of the foundation. To help control weed growth, geotextiles should be used below landscaped areas adjacent to foundations. Impervious plastic membranes are not recommended.

Irrigation devices should not be placed within 5 feet of the foundation. Irrigation should be limited to the amount sufficient to maintain vegetation. Application of more water will increase the likelihood of slab and foundation movements.

The recommendations listed in this report are intended to address normal surface drainage conditions, assuming the presence of groundcover (established vegetation, paved surfaces, and/or structures) throughout the regions upslope from this structure. However, groundcover may not be present due to a variety of factors (ongoing construction/development, wildfires, etc.). During periods when groundcover is not present in the "upslope" regions, higher than normal surface drainage conditions may occur, resulting in perched water tables, excess runoff, flash floods, etc. In these cases, the surface drainage recommendations presented herein (even if properly maintained) may not mitigate all groundwater problems or moisture intrusion into the structure. We recommend that the site plan be prepared with consideration of increased runoff during periods when groundcover is not present on the upslope areas.

7.14 Fill Soils

Fill soils were not encountered at the time of drilling. Fill soils could include (but are not limited to) non-engineered fills, fill soils containing trash or debris, contaminated, fill soils that appear to have been improperly placed and/or compacted, etc. If unsuitable soils are encountered during the Site Specific

Soils Investigation and/or the Open Excavation Observation, they may require removal (overexcavation) and replacement with compacted structural fill. The anticipated fill areas (af) are hatched on the Geologic Condition Map, Figure 20.

Mitigation

If any man-placed fill is encountered, it is considered unsuitable for support of foundations. If unsuitable fill soils are encountered during construction, they should be removed (overexcavated) and replaced with compacted structural fill. If contaminated soils from the septic fields are encountered all soils should be removed and disposed of properly. The zone of overexcavation shall extend to the bottom of the unsuitable fill zone and shall extend at least that same distance beyond the building perimeter (or lateral extent of any fill, if encountered first). Provided that this recommendation is implemented, the presence of this fill is not considered to pose a risk to the proposed new structures.

7.15 Proposed Grading, Erosion Control, Cuts and Masses of Fill

Preliminary grading plans were provided (referenced above) and reviewed at the time the report was issued. It is assumed based on the test borings for this investigation that the excavations will encounter silty to clayey sands and/or sandy clay. The on-site soils can be used as site grading fill.

The on-site soils are mildly susceptible to wind and water erosion. Minor wind erosion and dust may be an issue for a short time during and immediately after construction. Should the problem be considered severe during construction, watering of the cut areas may be required. Once construction is complete, vegetation should be re-established.

Prior to placement of overlot fill or removal and recompaction of the existing materials, topsoil, low-density native soil, fill and organic matter should be removed from the fill area. The subgrade should be scarified, moisture conditioned to within 2% of the optimum moisture content, and recompacted to the same degree as the overlying fill to be placed. The placement and compaction of fill should be periodically observed and tested by a representative of RMG during construction.

7.16 Onsite Wastewater Treatment Systems

It is our understanding that on-site wastewater treatment systems are not proposed. Based on the Preliminary Plan by Thomas and Thomas, sewer services will be dedicated to Widefield Water and Sanitation District.

7.17 Special Recommendations

The Jimmy Camp Creek "east tributary" extends along the southern boundary of the site. Based on the relative elevation of these water features to the proposed structures and the conditions encountered in the subsurface soil investigation and the open excavation observation for each lot, additional drainage features may be recommended. It appears the current Jimmy Camp Creek "east tributary" alignment and existing detention pond (C1-R) will remain undisturbed during construction.

8.0 BEARING OF GEOLOGIC CONDITIONS UPON PROPOSED DEVELOPMENT

Geologic hazards (as described in section 7.0 of this report) and geologic constraints (also as described in section 7.0 of this report) were found to be present at this site.

The geologic hazards anticipated to affect this site are Faults/Seismicity and Radioactivity/Radon Gas.

The most significant geologic constraints to development recognized at this site are *potential for expansive and hydrocompactive soils.* It may be necessary to design and implement mitigation alternatives at the site.

The geologic conditions encountered at this site are relatively common to the immediate area and mitigation can be accomplished by implementing common engineering and construction practices.

9.0 BURIED UTILITIES

Based upon the conditions encountered in the exploratory test borings, we anticipate that the soils encountered in the utility trench excavations will consist of silty to clayey sands, (SM and SC) sandy silt (ML) and sandy clay (CL and CH). It is anticipated that the sands will be encountered at loose to medium dense relative densities, the clays at medium stiff to very stiff consistencies. Depending on the depth of excavations, temporary shoring and hydraulic water pumps may be required to prevent the collapse of trenches and the accumulation of water at the bottom of the excavation.

We believe the sand and clays will classify as Type C materials as defined by OSHA in 29 CFR Part 1926. OSHA requires that temporary excavations made in Type B and C materials be laid back at ratios no steeper than 1:1 (horizontal to vertical) and 1½:1 (horizontal to vertical), respectively, unless the excavation is shored and braced. Excavations deeper than 20 feet, or when water is present, should always be braced or the slope designed by a professional engineer.

Utility mains such as water and sanitary sewer lines are typically placed beneath paved roadways. The settlement of the utility trench backfill can have a detrimental effect on pavements and roadway surfaces. We recommend that utility trench backfill be placed in thin loose lifts, moisture conditioned as required and compacted to the recommendations outlined in the **Backfill** section of this report. The placement and compaction of utility trench backfill should be observed and tested by a representative of RMG Engineers during construction.

It is a common local practice for underdrains to be placed at the bottom of sanitary sewer trenches within drive lanes. Underdrains placed in the sanitary sewer trenches in areas where groundwater is anticipated will likely be the "active" type, which uses a perforated drain pipe. In areas where groundwater is not anticipated, “passive” type underdrains may be used. Typical underdrain details are presented in Figures 22 and 23. If an underdrain system is used, it will likely necessitate construction and maintenance of a pumping station to collect and redirect the discharge from the underdrain system. At this time an underdrain system is not anticipated. One potential alternative to this approach would be to provide individual sump pits and pumps for each residence to collect and redirect discharge water from all recommended subsurface foundation drains. If this option is selected, care should be taken to ensure that the sump pumps have outfall to a location that is graded to direct the discharge water away from the surrounding structures and to a suitable collection or drainage area.

10.0 PAVEMENTS

Preliminary Roadway Layout plans were provided prior to the report issue date. Roadways throughout the proposed development are anticipated to be classified as Urban/Residential, Local and Residential Collectors and 2-lane Minor Arterials in accordance with Appendix D of the ECM. ***The actual pavement section design for individual streets will be completed following overlot grading and rough cutting of the street subgrade.***

For preliminary planning purposes, estimated full-depth pavement sections have been evaluated based on current design criteria. For purposes of this report, we anticipate the subgrade soils will primarily have an American Association of State Highway and Transportation Officials (AASHTO) Soil Classification of A-2-4, A-4, A-6, A-7-5, and A-7-6 with an estimated California Bearing Ratio (CBR) value of approximately 3 to 10.

The above value is for preliminary planning purposes and may vary upon final design, dependent upon the soil material used for subgrade construction.

11.0 ANTICIPATED FOUNDATION SYSTEMS

Based on the information presented previously, conventional shallow foundation systems consisting of standard spread footings/stemwalls are anticipated to be suitable for the proposed residential structures. It is assumed that the deepest excavation cuts will be approximately 6 to 8 feet below the final ground surface not including overexcavation which may be required on a lot-by-lot basis.

Due to its swell potential, the sandy clay is generally not suitable for support of spread footing foundations or floor slabs. Where expansive soils are encountered near spread footing foundation or floor slab levels, they should be removed and replaced with granular, non-expansive structural fill. Foundation systems which may reduce or eliminate the need for overexcavation include (but are not limited to) post-tension slabs-on-grade, integral stiffened (ribbed) slab foundations, driller pier (caisson) foundations with or without a structural floor, etc.

If loose or hydrocompactive sands are encountered, they may require additional compaction. In some cases, removal and recompaction may be required for loose soils. Similarly, if shallow groundwater conditions result in unstable soils, unsuitable for bearing of residential foundations, these soils may require stabilization or overexcavation and replacement prior to construction of foundation components.

The foundation system for each lot should be designed and constructed based upon recommendations developed in a detailed Subsurface Soil Investigation completed after site development activities are complete. The recommendations presented in the Subsurface Soil Investigation should be verified by an Open Excavation Observation following the excavation on each lot.

11.1 Subexcavation and Moisture-Conditioned Fill

Based upon the field exploration and laboratory testing, subexcavation and replacement is not anticipated. However, prior to performing excavation and/or filling operations, vegetation, organic and deleterious material shall be cleared and disposed of in accordance with applicable requirements. The excavation should extend to a minimum depth below and laterally beyond the bottom of foundations as determined based on final grading plans.

11.2 Foundation Stabilization

Groundwater and loose soils were encountered at the time of drilling, if moisture conditions encountered at the time of the foundation excavation result in water flow into the excavation and/or destabilization of the foundation bearing soils, stabilization techniques should be implemented. Various stabilization methods can be employed, and can be discussed at the time of construction. However, a method that affords potentially a reduced amount of overexcavation (versus other methods) and provides increased performance under moderately to severely unstable conditions is the use of a layered geogrid and structural fill system.

Additionally, dependent upon the rate of groundwater flow into the excavation, a geosynthetic vertical drain and an overexcavation perimeter drain may be required around the lower portions of the excavation to allow for installation of the layered geogrid and structural fill system.

11.3 Foundations Drains

A subsurface perimeter drain is recommended around portions of the structure which will have habitable or storage space located below the finished ground surface. This includes crawlspace areas but not the walkout trench, if applicable.

Groundwater conditions were encountered in the test borings at the time of field exploration. The proposed detention ponds appear to be located at proposed basement foundation elevations. Depending on the conditions encountered during the lot specific Subsurface Soil Investigation and the conditions observed at the time of the Open Excavation Observation, additional subsurface drainage systems may be recommended.

One such system is an underslab drainage layer to help intercept groundwater before it enters the slab area should the groundwater levels rise. In general, if groundwater was encountered within 4 to 6 feet of the proposed basement slab elevation, an underslab drain should be anticipated. Another such system would consist of a subsurface drain and/or vertical drain board placed around the perimeter of the overexcavation to help intercept groundwater and allow for proper placement and compaction of the replacement structural fill. Careful attention should be paid to grade and discharge of the drain pipes of these systems.

It must be understood that the drain systems are designed to intercept some types of subsurface moisture and not others. Therefore, the drains could operate properly and not mitigate all moisture problems relating to foundation performance or moisture intrusion into the basement area.

11.4 Structural Fill

Areas to receive structural fill should have topsoil, organic material, or debris removed. The upper 6 inches of the exposed surface soils should be scarified and moisture conditioned to facilitate compaction (usually within 2 percent of the optimum moisture content) and compacted to a minimum of 95 percent of the maximum dry density as determined by the Standard Proctor test (ASTM D-698) or to a minimum of 92 percent of the maximum dry density as determined by the Modified Proctor test (ASTM D-1557) prior to placing structural fill.

Structural fill placed on slopes should be benched into the slope. Maximum bench heights should not exceed 4 feet, and bench widths should be wide enough to accommodate compaction equipment.

Structural fill shall consist of granular, non-expansive material. It should be placed in loose lifts not exceeding 8 to 12 inches, moisture conditioned to facilitate compaction (usually within 2 percent of the optimum moisture content) and compacted to a minimum of 92 percent of the maximum dry density as determined by the Modified Proctor test, ASTM D-1557. The materials should be compacted by mechanical means.

Materials used for structural fill should be approved by RMG prior to use. Structural fill should not be placed on frozen subgrade or allowed to freeze during moisture conditioning and placement.

11.5 Design Parameters

The allowable bearing pressure of the subsurface soils should be determined by a detailed site specific Subsurface Soil Investigation and verified by and Open Excavation Observation, as noted above.

12.0 DETENTION STORAGE CRITERIA

The purpose of this investigation is to characterize the subsurface soils pertinent to embankment construction, and to provide recommendations regarding embankment construction. This report has been prepared in accordance with the requirements outlined in the El Paso County Land Development Code (LDC), the Engineering Criteria Manual (ECM) Section 2.2.6 and Appendix C.3.2.B, and the El Paso County (EPC) Drainage Criteria Manual, Volume 1 Section 11.3.3.

2.1 Detention Storage Criteria

Detention pond embankments that impound water above the natural grade of the land are considered dams under rules and regulation promulgated by the State of Colorado Department of Natural Resources. Rules and Regulations for Dam Safety and Dam Construction have been developed to provide guidance to design engineers and constructors. Dams are regulated as jurisdictional dams or non-jurisdictional dams. In accordance with El Paso County Drainage Criteria Manual, Volume 1, Section 6.6, embankments associated with Creekside at Lorson Ranch, Filing No. 1 detention ponds CR2 and CR3 **do not** include features that can be considered dams and are not subject to the State dam rules and regulations. Based upon the Creekside at Lorson Ranch Filing No. 1 Early Grading and Erosion Plans, these ponds will be cut into the existing natural terrain and will not impound water above the natural ground level.

The purpose of our report is to comply with the referenced guidelines and provide pertinent geotechnical information upon which to base the design and construction of pond embankments. This report presents the findings of the investigation performed by RMG and our recommendations regarding detention pond construction.

12.2 Embankment Recommendations

In the event that embankments become necessary the following general construction recommendations are applicable. Embankments should be constructed in accordance with applicable sections of the El Paso County Engineering Criteria Manual, the El Paso County Drainage Criteria Manual, and the El Paso County Land Development Manual. The following recommendations are in accordance with the El Paso county DCM Volume 2, Extended Detention Basin (EDB), Design Procedure and Criteria, paragraph 8.

The ground area to receive embankments should be cleared and grubbed to a minimum depth of two-feet to remove grass, shrubs, trees, roots, stumps, and other organic material. The exposed soil should be moisture conditioned to facilitate compaction (usually within 2 percent of the optimum moisture content) and compacted to a minimum of 95 percent of the maximum dry density as determined by the Modified Proctor test (ASTM D-1557). The prepared surface should present a firm and stable condition.

Embankment should be constructed as structural fill on a prepared stable base. On-site native soil when screened of all deleterious material and cobbles greater than 6-inches in any dimension is suitable for embankment construction. Structural fill should be placed in 10-inch loose lifts and moisture conditioned to facilitate compaction (usually within 2 percent of the optimum moisture content) and compacted to a minimum of 95 percent of the maximum dry density as determined by the Modified Proctor test (ASTM D-1557).

Structural fill placed on slopes should be benched into the slope. Maximum bench heights should not exceed 4 feet, and bench widths should be wide enough to accommodate compaction equipment. Structural fill should not be placed on frozen subgrade or allowed to freeze during moisture conditioning and placement. To verify the condition of the compacted soils, density tests should be performed during placement. The first density tests should be conducted when 24 inches of fill have been placed.

13.0 ADDITIONAL STUDIES

The findings, conclusions and recommendations presented in this report were provided to evaluate the suitability of the site for future development. Unless indicated otherwise, the test borings, laboratory test results, conclusions and recommendations presented in this report are not intended for use for design and construction. ***A site specific Subsurface Soil Investigation will be required for all proposed structures including (but not limited to) residences, retaining walls and pumphouses, commercial buildings, etc.***

To develop recommendations for construction of the proposed roadways, a pavement design investigation should be performed. This investigation should consist of additional test borings, soil laboratory testing and specific recommendations for the design and construction of roadway pavement sections.

14.0 CONCLUSIONS

Based upon our evaluation of the geologic conditions, it is our opinion that the proposed development is feasible. The potential for hydrocompactive and expansive soils and flooding, the geologic hazards identified are not considered unusual for the Front Range region of Colorado. Mitigation of geologic hazards is most effectively accomplished by avoidance. However, where avoidance is not a practical or acceptable alternative, geologic hazards should be mitigated by implementing appropriate planning, engineering, and local construction practices.

Potential mitigation alternatives include (but are not limited to) overexcavation and replacement of unsuitable soils and the design and construction of surface and subsurface drainage systems which are commonly used in the El Paso County vicinity.

Revisions and modifications to the conclusions and recommendations presented in this report may be issued subsequently by RMG based upon additional observations made during grading and construction which may indicate conditions that require re-evaluation of some of the criteria presented in this report.

15.0 CLOSING

This report is for the exclusive purpose of providing geologic hazards information and preliminary geotechnical engineering recommendations. The scope of services did not include, either specifically or by implication, evaluation of wild fire hazards, environmental assessment of the site, or identification of contaminated or hazardous materials or conditions. Development of recommendations for the mitigation of environmentally related conditions, including but not limited to, biological or toxicological issues, are beyond the scope of this report. If the owner is concerned about the potential for such contamination or conditions, other studies should be undertaken.

This report has been prepared for **Lorson Ranch Metro District No. 1** in accordance with generally accepted geotechnical engineering and engineering geology practices. The conclusions and recommendations in this report are based in part upon data obtained from review of available topographic and geologic maps, review of available reports of previous studies conducted in the site vicinity, a site reconnaissance, and research of available published information, soil test borings, soil laboratory testing, and engineering analyses. The nature and extent of variations may not become evident until construction activities begin. If variations then become evident, RMG should be retained to re-evaluate the recommendations of this report, if necessary.

Our professional services were performed using that degree of care and skill ordinarily exercised, under similar circumstances, by geotechnical engineers and engineering geologists practicing in this or similar localities. RMG does not warrant the work of regulatory agencies or other third parties supplying information which may have been used during the preparation of this report. No warranty, express or implied, is made by the preparation of this report. Third parties reviewing this report should draw their own conclusions regarding site conditions and specific construction techniques to be used on this project.

If we can be of further assistance in discussing the contents of this report or analysis of the proposed development, from a geotechnical engineering and/or geologic hazards point-of-view, please feel free to contact us.

16.0 REFERENCES

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FIGURES

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SITE VICINITY MAP

CREEKSIDE AT LORSON RANCH
FILING NO.1
EL PASO COUNTY, COLORADO
LORSON RANCH METRO DISTRICT NO. 1

JOB No. 164808

FIG No. 1

DATE 8-10-2018

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▨ INDICATES THE APPROXIMATE LIMITS OF THIS INVESTIGATION

BASE MAP PROVIDED BY: GOOGLE 2018

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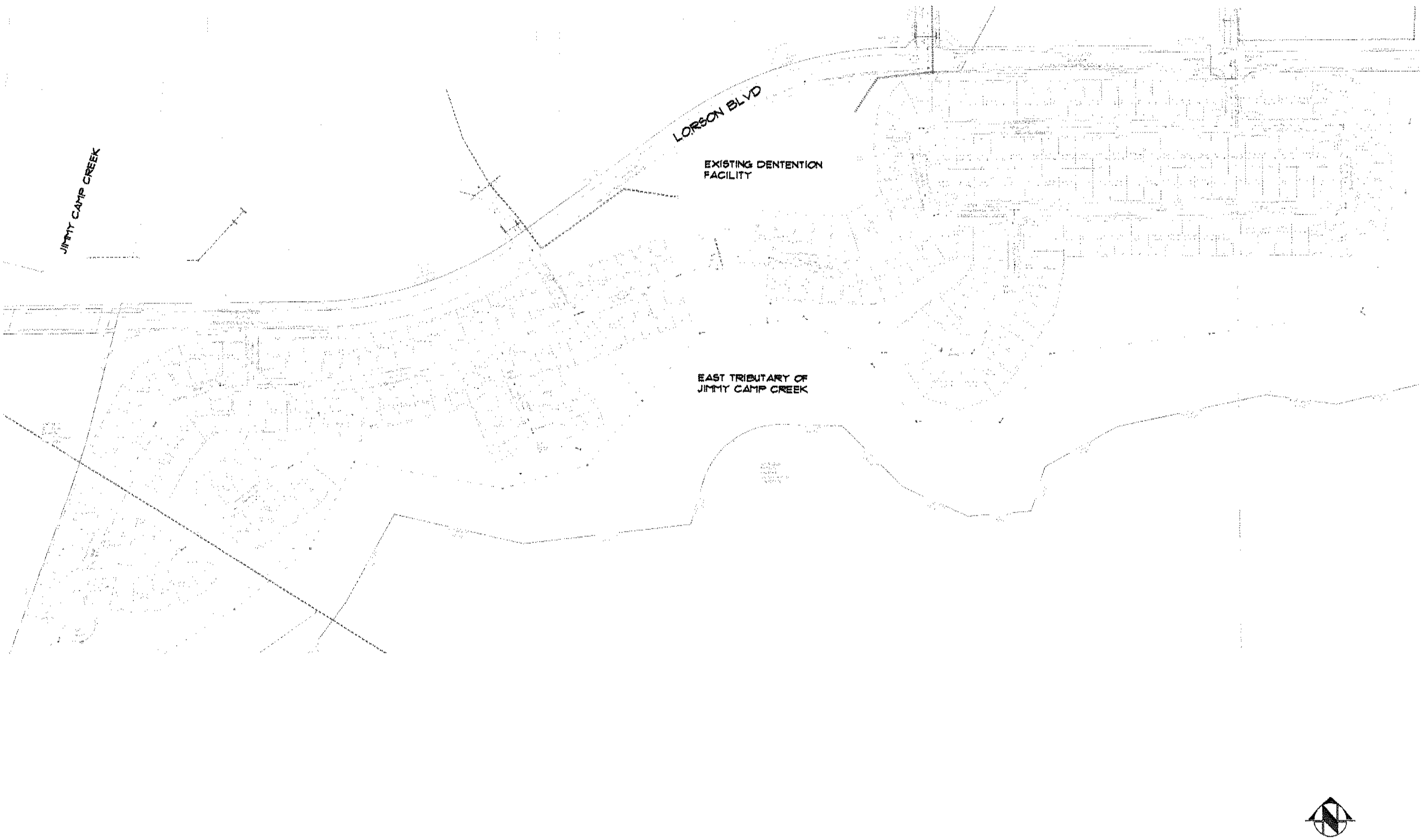
2018 AERIAL PHOTOGRAPH

CREEKSIDE AT LORSON RANCH
FILING NO.1
EL PASO COUNTY, COLORADO
LORSON RANCH METRO DISTRICT NO.1

JOB No. 164808

FIG No. 2

DATE 8-10-2018



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BASE MAP PROVIDED BY
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DISTRICT, NO. 1

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DRAWN BY:	KZ
CHECKED BY:	GW
ISSUED:	8-10-2018
	DATE:
REVISION:	JOB #:

PRELIMINARY
CONCEPT PLAN

SHEET No.
FIG-3

● DENOTES APPROXIMATE
LOCATION OF TEST BORINGS

JOB No. 164808

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CHECKED BY:	GW
ISSUED:	8-10-2018
	DATE:
REVISION:	JOB #:

PRELIMINARY
CONCEPT PLAN WITH
TEST BORING LOCATIONS

SHEET No.
FIG-4

SOILS DESCRIPTION

▧	CLAYEY SAND
▨	SANDY CLAY
▥	SILTY SAND

UNLESS NOTED OTHERWISE, ALL LABORATORY
TESTS PRESENTED HEREIN WERE PERFORMED BY:
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2910 AUSTIN BLUFFS PARKWAY
COLORADO SPRINGS, COLORADO

SYMBOLS AND NOTES

■	XX	STANDARD PENETRATION TEST - MADE BY DRIVING A SPLIT-BARREL SAMPLER INTO THE SOIL BY DROPPING A 140 LB. HAMMER 30", IN GENERAL ACCORDANCE WITH ASTM D-1586. NUMBER INDICATES NUMBER OF HAMMER BLOWS PER FOOT (UNLESS OTHERWISE INDICATED).
◩	XX	UNDISTURBED CALIFORNIA SAMPLE - MADE BY DRIVING A RING-LINED SAMPLER INTO THE SOIL BY DROPPING A 140 LB. HAMMER 30", IN GENERAL ACCORDANCE WITH ASTM D-3550. NUMBER INDICATES NUMBER OF HAMMER BLOWS PER FOOT (UNLESS OTHERWISE INDICATED).
▽		FREE WATER TABLE
⊠		DEPTH AT WHICH BORING CAVED
⧗	BULK	DISTURBED BULK SAMPLE
⦚	AUG	AUGER "CUTTINGS"
4.5		WATER CONTENT (%)

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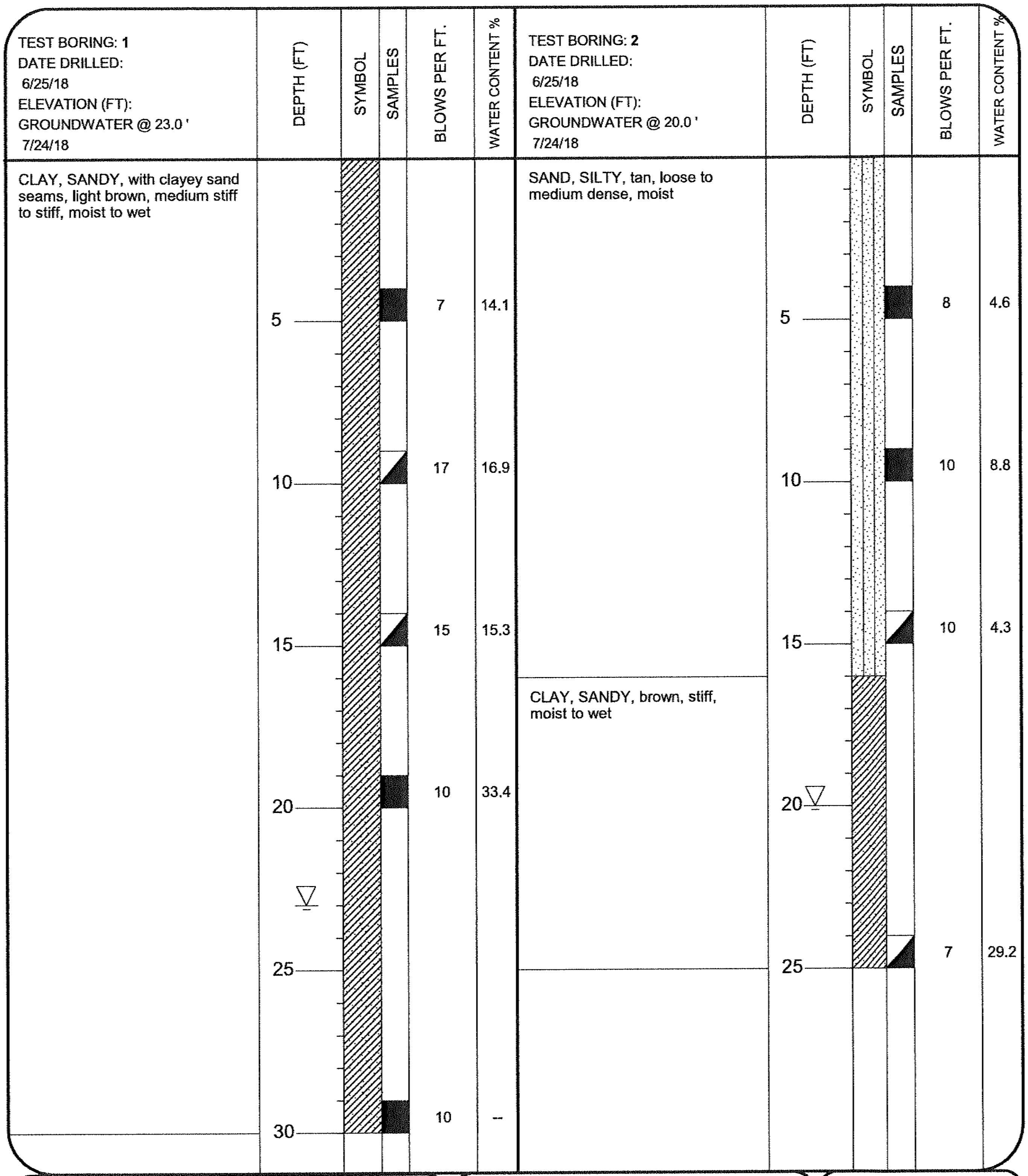
EXPLANATION OF TEST BORING LOGS

JOB No. 164808

FIGURE No. 5

DATE 8/10/18



TEST BORING: **1**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 23.0 '
7/24/18

CLAY, SANDY, with clayey sand seams, light brown, medium stiff to stiff, moist to wet

DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
5	7	14.1
10	17	16.9
15	15	15.3
20	10	33.4
25		
30	10	--

TEST BORING: **2**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 20.0 '
7/24/18

SAND, SILTY, tan, loose to medium dense, moist

CLAY, SANDY, brown, stiff, moist to wet

DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
5	8	4.6
10	10	8.8
15	10	4.3
20		
25	7	29.2

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TEST BORING LOG

JOB No. 164808

FIGURE No. 6

DATE 8/10/18

TEST BORING: **3**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 20.0 '
7/24/18

	DEPTH (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
CLAY, SANDY, with sandy silt seams, light brown, stiff to very stiff, moist to wet		▨			
	5	▨	◢	10	17.1
	10	▨	■	15	26.3
	15	▨	◢	21	15.3
	20 ▽	▨	■	14	31.8
	25	▨	■	11	34.8

TEST BORING: **4**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 19.0 '
7/24/18

	DEPTH (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SAND, SILTY, tan, medium dense, moist		░			
	5	░	■	10	7.6
CLAY, SANDY, light brown, medium stiff to stiff, moist to wet	10	▨	◢	11	28.7
	15	▨	◢	7	20.1
	▽ 20	▨			
	25	▨	■	7	32.4

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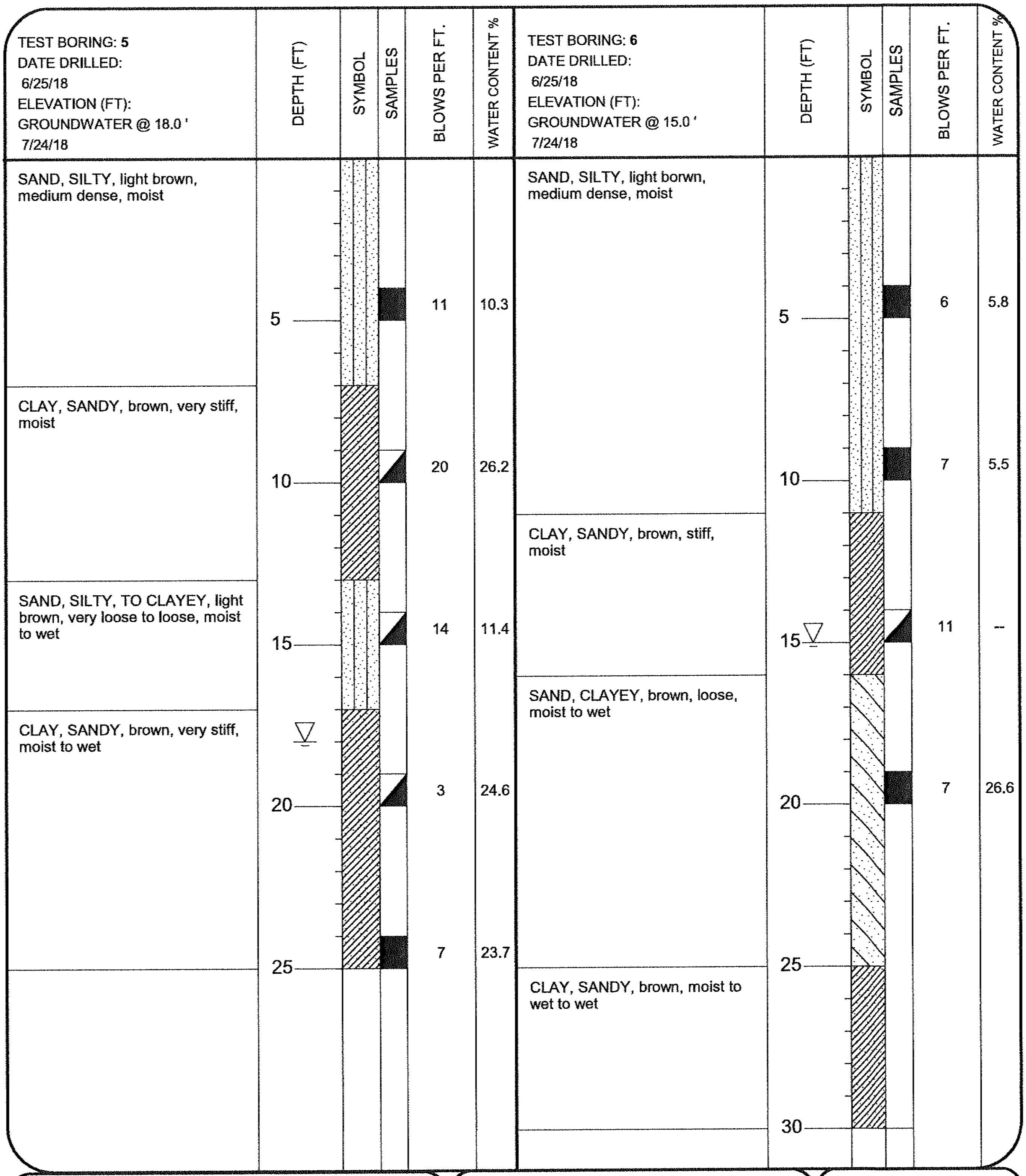
TEST BORING LOG

JOB No. 164808

FIGURE No. 7

DATE 8/10/18



TEST BORING: **5**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 18.0 '
7/24/18

	DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
SAND, SILTY, light brown, medium dense, moist	5	11	10.3
CLAY, SANDY, brown, very stiff, moist	10	20	26.2
SAND, SILTY, TO CLAYEY, light brown, very loose to loose, moist to wet	15	14	11.4
CLAY, SANDY, brown, very stiff, moist to wet	20	3	24.6
	25	7	23.7

TEST BORING: **6**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 15.0 '
7/24/18

	DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
SAND, SILTY, light borwn, medium dense, moist	5	6	5.8
	10	7	5.5
CLAY, SANDY, brown, stiff, moist	15	11	--
SAND, CLAYEY, brown, loose, moist to wet	20	7	26.6
CLAY, SANDY, brown, moist to wet	25		
	30		

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TEST BORING LOG

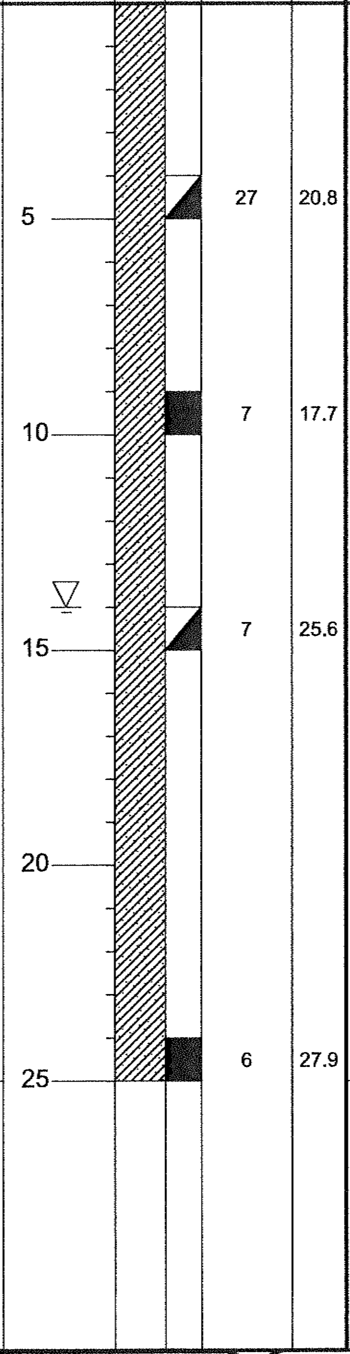
JOB No. 164808

FIGURE No. 8

DATE 8/10/18

TEST BORING: **7**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 14.0 '
7/24/18

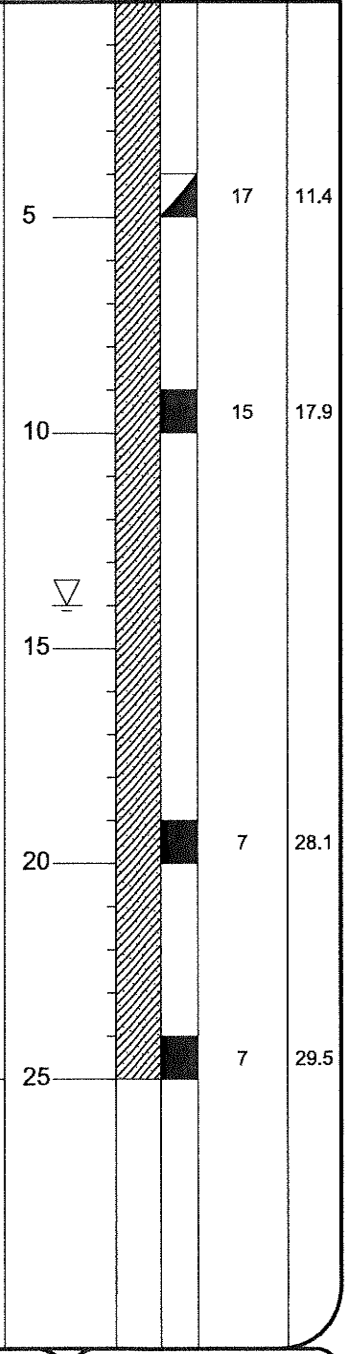
CLAY, SANDY, dark brown, medium stiff to very stiff, moist to wet



DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
5	27	20.8
10	7	17.7
15	7	25.6
20		
25	6	27.9

TEST BORING: **8**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 14.0 '
7/24/18

CLAY, SANDY, light brown, medium stiff to stiff, moist to wet



DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
5	17	11.4
10	15	17.9
15		
20	7	28.1
25	7	29.5

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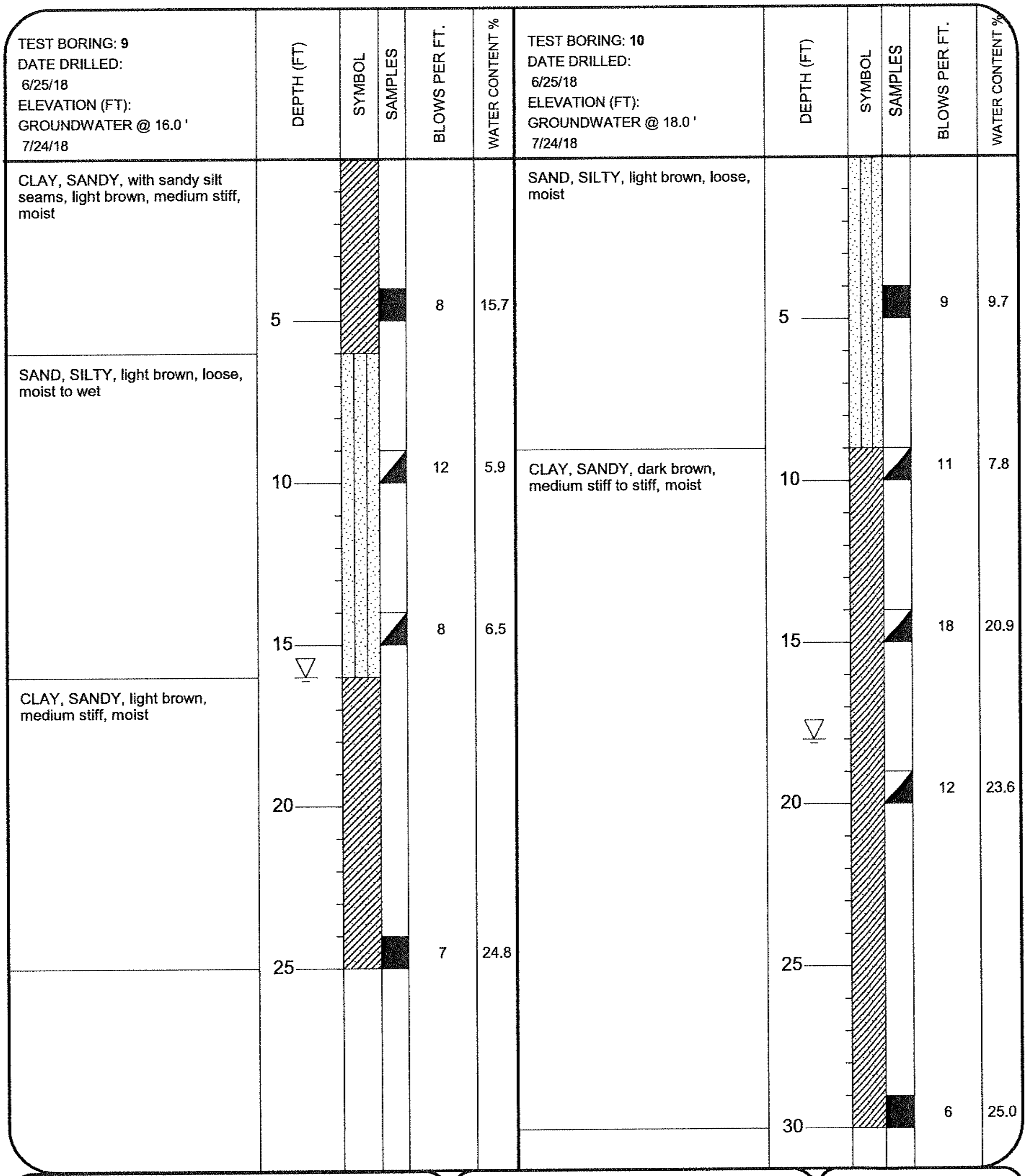
FIGURE No. 9

DATE 8/10/18



TEST BORING: **9**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 16.0 '
7/24/18

	DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
CLAY, SANDY, with sandy silt seams, light brown, medium stiff, moist	5	8	15.7
SAND, SILTY, light brown, loose, moist to wet	10	12	5.9
	15	8	6.5
CLAY, SANDY, light brown, medium stiff, moist	20		
	25	7	24.8

TEST BORING: **10**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 18.0 '
7/24/18

	DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
SAND, SILTY, light brown, loose, moist	5	9	9.7
CLAY, SANDY, dark brown, medium stiff to stiff, moist	10	11	7.8
	15	18	20.9
	20	12	23.6
	25		
	30	6	25.0

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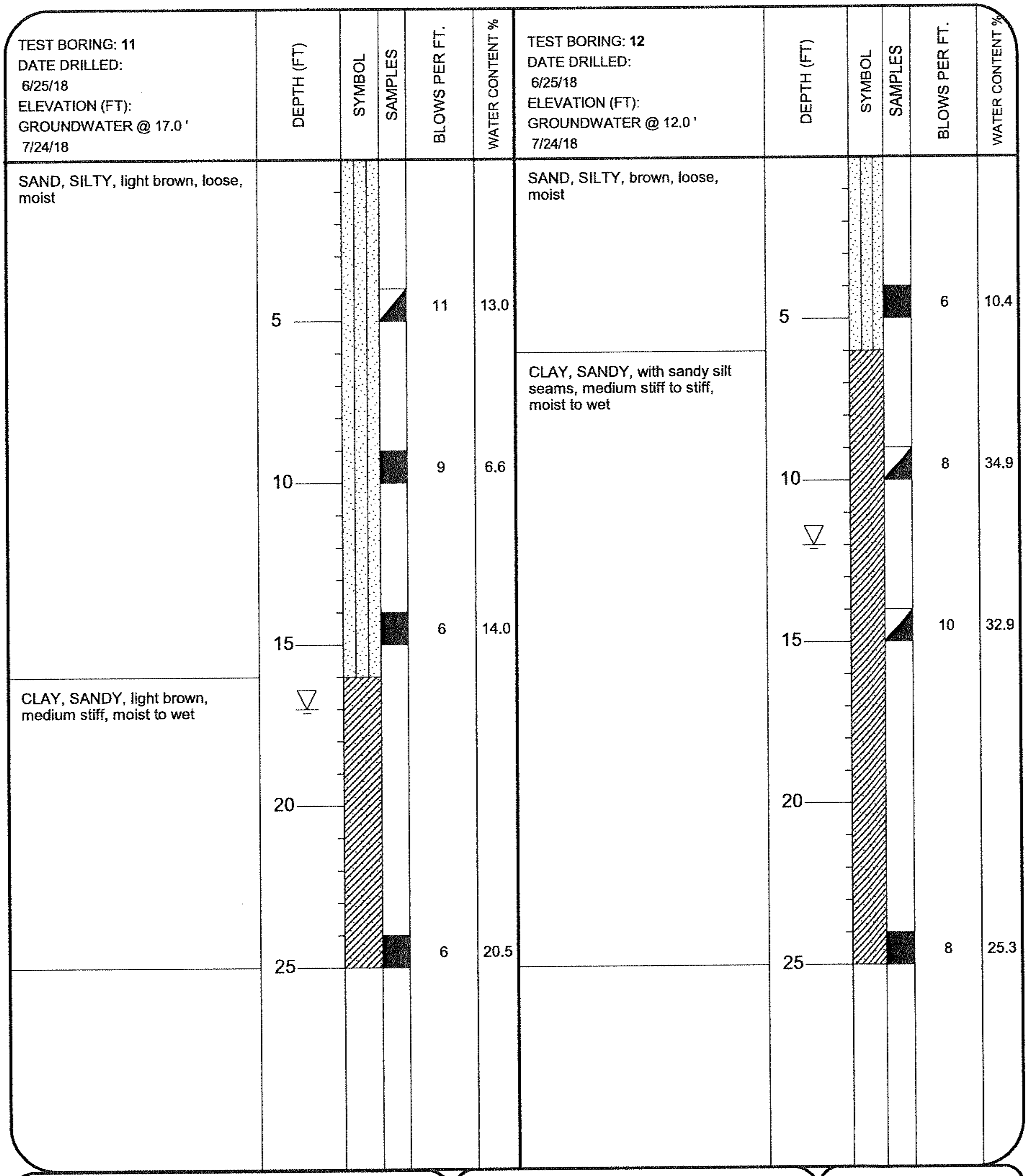
FIGURE No. 10

DATE 8/10/18



TEST BORING: **11**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 17.0 '
7/24/18

SAND, SILTY, light brown, loose, moist

CLAY, SANDY, light brown, medium stiff, moist to wet

DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
5	11	13.0
10	9	6.6
15	6	14.0
20		
25	6	20.5

TEST BORING: **12**
DATE DRILLED:
6/25/18
ELEVATION (FT):
GROUNDWATER @ 12.0 '
7/24/18

SAND, SILTY, brown, loose, moist

CLAY, SANDY, with sandy silt seams, medium stiff to stiff, moist to wet

DEPTH (FT)	BLOWS PER FT.	WATER CONTENT %
5	6	10.4
10	8	34.9
15	10	32.9
20		
25	8	25.3

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TEST BORING LOG

JOB No. 164808

FIGURE No. 11

DATE 8/10/18

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	FHA Expansion Pressure (psf)	% Swell/ Collapse	USCS Classification
1	4.0	14.1								
1	9.0	16.9	90.3	42	17		38.9		0.5	SC
1	14.0	15.3								
1	19.0	33.4								
2	4.0	4.6								
2	9.0	8.8								
2	14.0	4.3	106.7	NP	NP		30.5		- 2.0	SM
2	24.0	29.2								
3	4.0	17.1		39	12		93.0			ML
3	9.0	26.3								
3	14.0	15.3	108.4						3.2	
3	19.0	31.8								
3	24.0	34.8								
4	4.0	7.6								
4	9.0	28.7		59	29		99.0			CH
4	14.0	20.1								
4	24.0	32.4								
5	4.0	10.3								
5	9.0	26.2								
5	14.0	11.4	93.9	NP	NP		35.1		- 1.5	SM
5	19.0	24.6								
5	24.0	23.7								
6	4.0	5.8								
6	9.0	5.5		NP	NP	0.0	18.1			SM
6	19.0	26.6								
6	24.0	26.0								
6	29.0	22.2								
7	4.0	20.8								
7	9.0	17.7		32	13		65.3			CL
7	14.0	25.6								
7	24.0	27.9								
8	4.0	11.4								
8	9.0	17.9								
8	19.0	28.1		35	19		94.3			CL

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SUMMARY OF LABORATORY TEST RESULTS

JOB No. 164808
FIGURE No. 12
PAGE 1 OF 2
DATE 8/10/18

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	FHA Expansion Pressure (psf)	% Swell/ Collapse	USCS Classification
8	24.0	29.5								
9	4.0	15.7		NP	NP	0.0	82.5			ML
9	9.0	5.9								
9	14.0	6.5								
9	24.0	24.8								
10	4.0	9.7								
10	9.0	7.8								
10	14.0	20.9	77.4	46	24		62.5		0.0	CL
10	19.0	23.6								
10	29.0	25.0								
11	4.0	13.0								
11	9.0	6.6		NP	NP	0.0	24.1			SM
11	14.0	14.0								
11	24.0	20.5								
12	4.0	10.4								
12	9.0	34.9	85.0	NP	NP		95.3		0.7	ML
12	14.0	32.9								
12	24.0	25.3								

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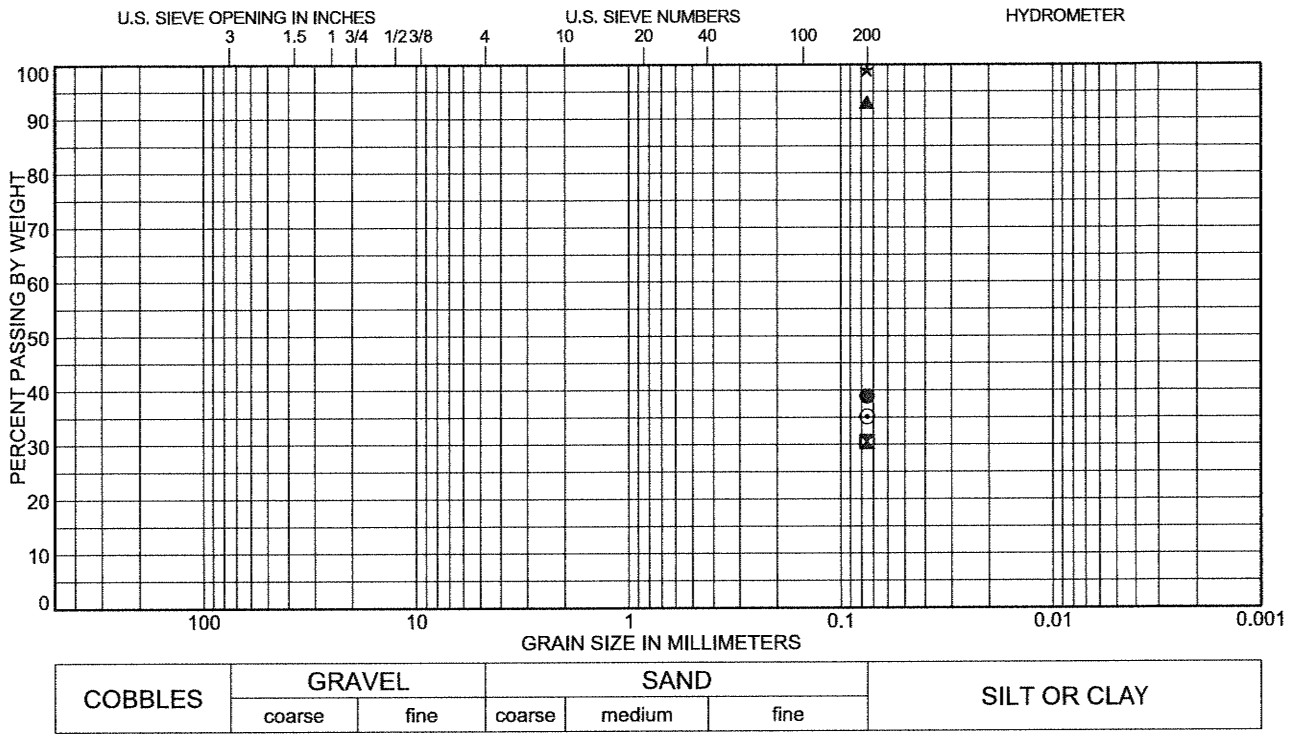
SUMMARY OF LABORATORY TEST RESULTS

JOB No. 164808
FIGURE No. 12
PAGE 2 OF 2
DATE 8/10/18



Test Boring		Depth (ft)	Classification	LL	PL	PI
●	1	9.0	**CLAYEY SAND(SC)**	42	25	17
☒	2	14.0	**SILTY SAND(SM)**	NP	NP	NP
▲	3	4.0	**SILT(ML)**	39	27	12
★	4	9.0	**FAT CLAY(CH)**	59	30	29
⊙	5	14.0	**SILTY SAND(SM)**	NP	NP	NP

Test Boring		Depth (ft)	%Gravel	%Sand	%Silt	%Clay
●	1	9.0			38.9	
☒	2	14.0			30.5	
▲	3	4.0			93.0	
★	4	9.0			99.0	
⊙	5	14.0			35.1	

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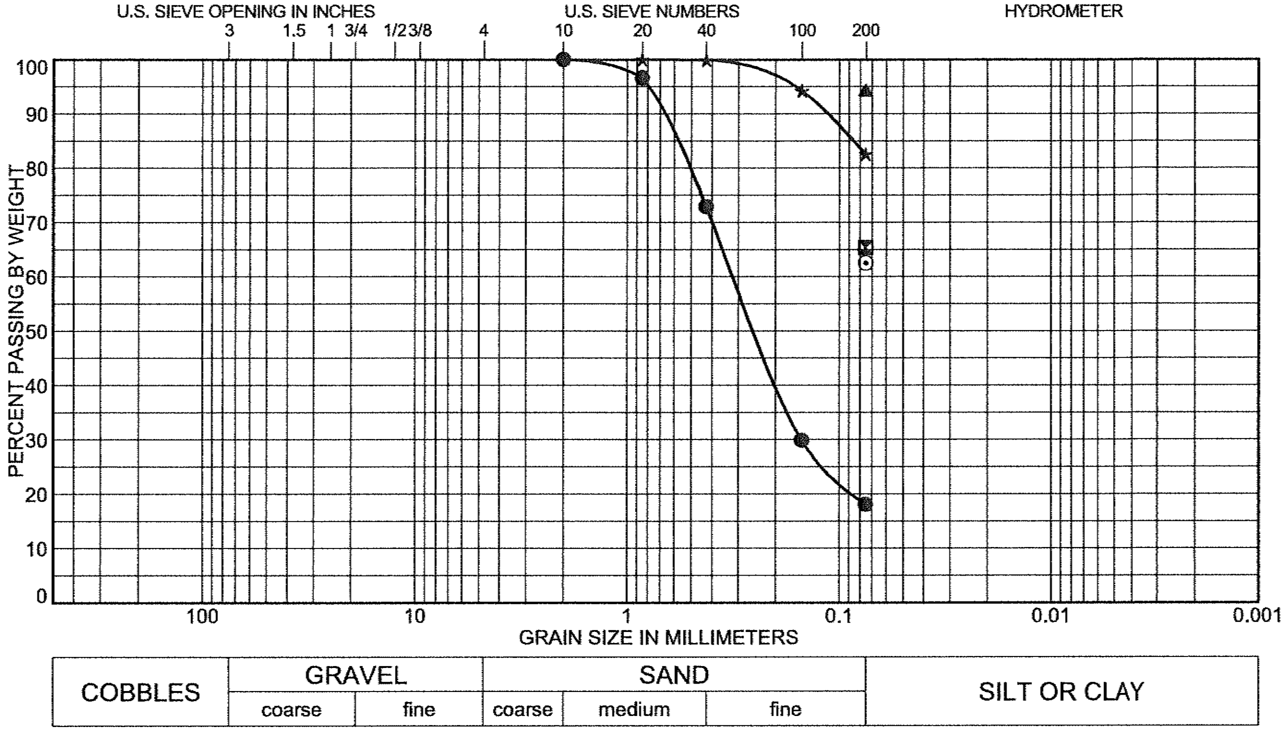
SOIL CLASSIFICATION DATA

JOB No. 164808

FIGURE No. 13

DATE 8/10/18



Test Boring	Depth (ft)	Classification	LL	PL	PI
● **6**	**9.0**	**SILTY SAND(SM)**	**NP**	**NP**	**NP**
⊠ **7**	**9.0**	**SANDY LEAN CLAY(CL)**	**32**	**19**	**13**
▲ **8**	**19.0**	**LEAN CLAY(CL)**	**35**	**16**	**19**
★ **9**	**4.0**	**SILT with SAND(ML)**	**NP**	**NP**	**NP**
⊙ **10**	**14.0**	**SANDY LEAN CLAY(CL)**	**46**	**22**	**24**

Test Boring	Depth (ft)	%Gravel	%Sand	%Silt	%Clay
● **6**	**9.0**	**0.0**	**81.9**	**18.1**	
⊠ **7**	**9.0**			**65.3**	
▲ **8**	**19.0**			**94.3**	
★ **9**	**4.0**	**0.0**	**17.5**	**82.5**	
⊙ **10**	**14.0**			**62.5**	

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SOIL CLASSIFICATION DATA

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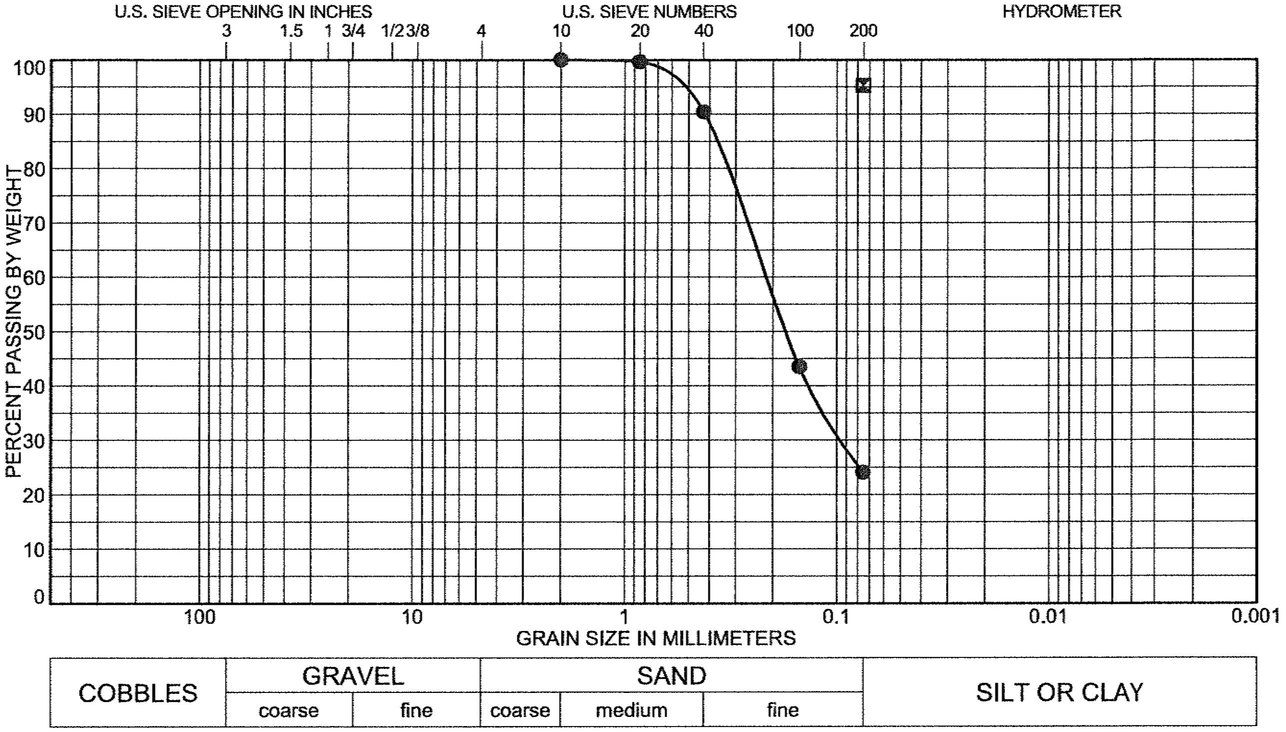
FIGURE No. 14

DATE 8/10/18



	Test Boring	Depth (ft)	Classification	LL	PL	PI
●	**11**	**9.0**	**SILTY SAND(SM)**	**NP**	**NP**	**NP**
☒	**12**	**9.0**	**SILT(ML)**	**NP**	**NP**	**NP**

	Test Boring	Depth (ft)	%Gravel	%Sand	%Silt	%Clay
●	**11**	**9.0**	**0.0**	**75.9**	**24.1**	
☒	**12**	**9.0**			**95.3**	

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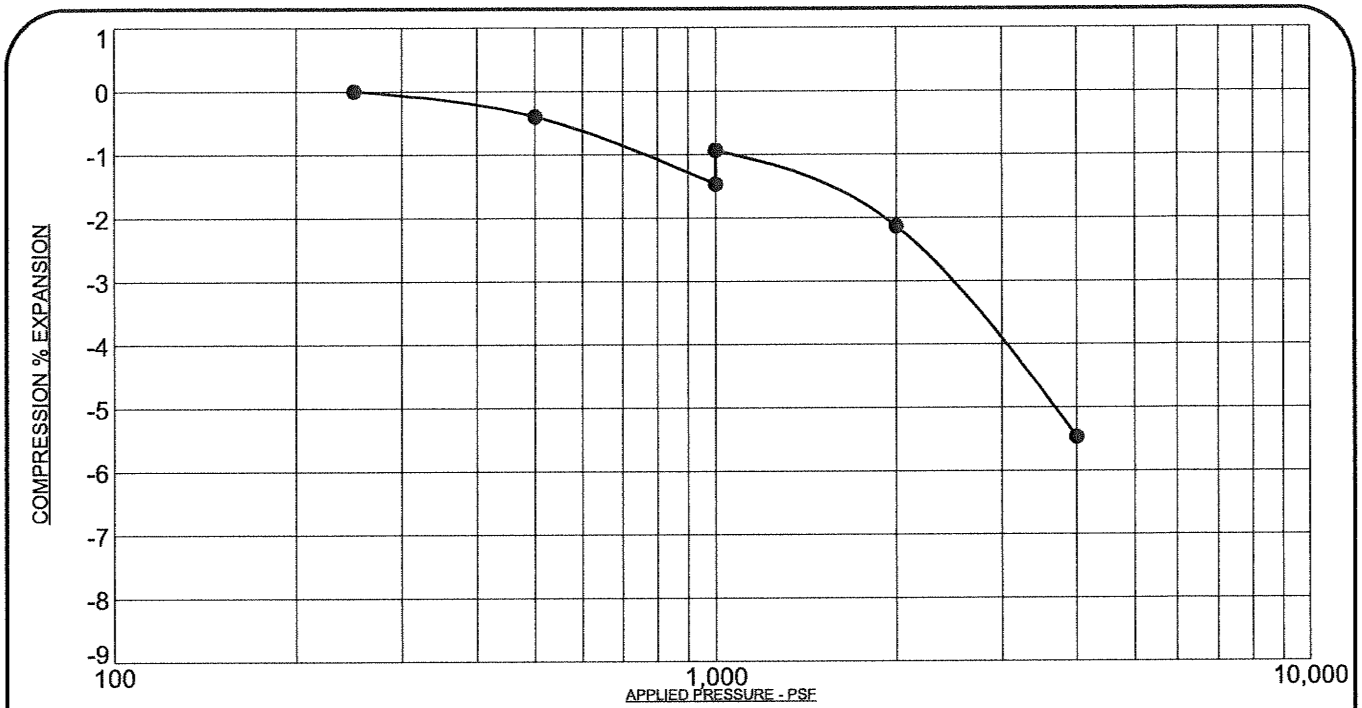
SOIL CLASSIFICATION DATA

JOB No. 164808

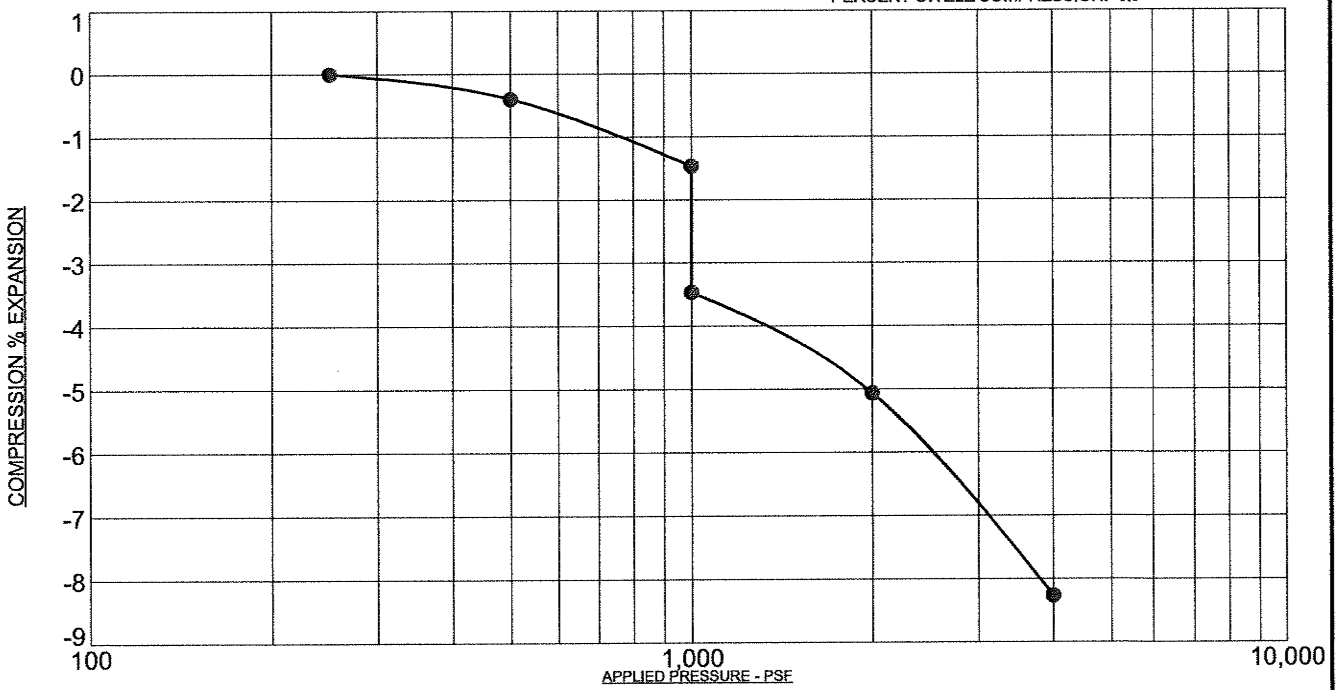
FIGURE No. 15

DATE 8/10/18



PROJECT: **Creekside at Lorson Ranch, Filing No. 1 El Paso County, Colorado**
SAMPLE DESCRIPTION: **SAND, CLAYEY**
NOTE: SAMPLE WAS INUNDATED WITH WATER AT 1,000 PSF

SAMPLE LOCATION: **1 @ 9 FT**
NATURAL DRY UNIT WEIGHT: **90.3 PCF**
NATURAL MOISTURE CONTENT: **16.8%**
PERCENT SWELL/COMPRESSION: **0.5**



PROJECT: **Creekside at Lorson Ranch, Filing No. 1 El Paso County, Colorado**
SAMPLE DESCRIPTION: **SAND, SILTY**
NOTE: SAMPLE WAS INUNDATED WITH WATER AT 1,000 PSF

SAMPLE LOCATION: **2 @ 14 FT**
NATURAL DRY UNIT WEIGHT: **106.7 PCF**
NATURAL MOISTURE CONTENT: **4.3%**
PERCENT SWELL/COMPRESSION: **- 2.0**

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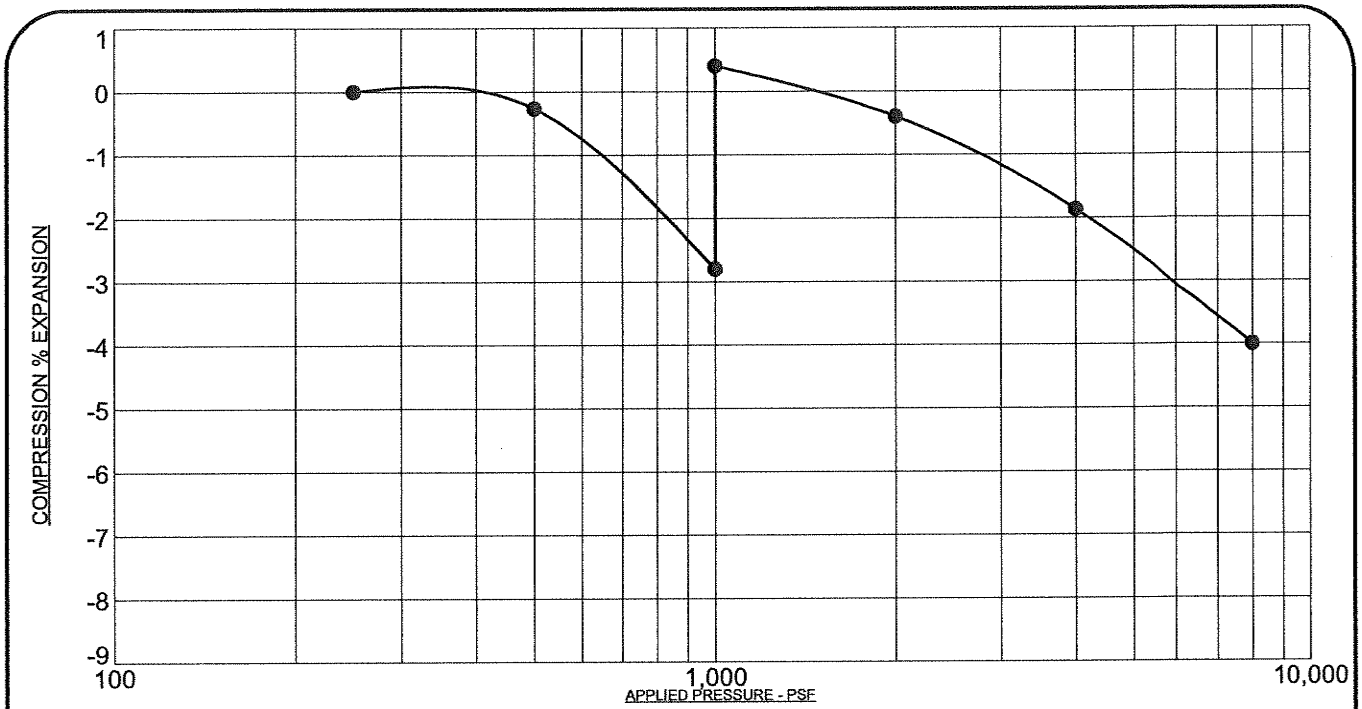
SWELL/CONSOLIDATION TEST RESULTS

JOB No. 164808

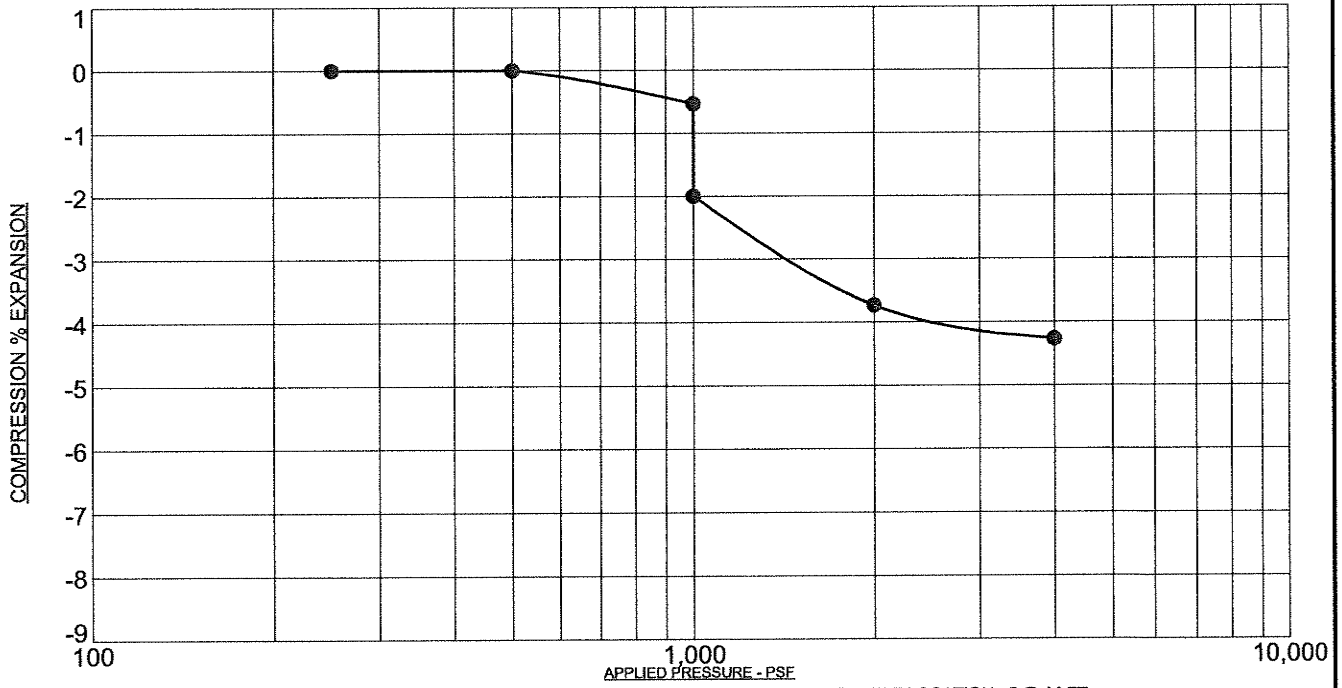
FIGURE No. 16

DATE 8/10/18



PROJECT: **Creekside at Lorson Ranch, Filing No. 1 El Paso County, Colorado**
SAMPLE DESCRIPTION: **CLAY, SANDY**
NOTE: SAMPLE WAS INUNDATED WITH WATER AT 1,000 PSF

SAMPLE LOCATION: **3 @ 14 FT**
NATURAL DRY UNIT WEIGHT: **108.4 PCF**
NATURAL MOISTURE CONTENT: **15.3%**
PERCENT SWELL/COMPRESSION: **3.2**



PROJECT: **Creekside at Lorson Ranch, Filing No. 1 El Paso County, Colorado**
SAMPLE DESCRIPTION: **SAND, SILTY**
NOTE: SAMPLE WAS INUNDATED WITH WATER AT 1,000 PSF

SAMPLE LOCATION: **5 @ 14 FT**
NATURAL DRY UNIT WEIGHT: **93.9 PCF**
NATURAL MOISTURE CONTENT: **11.4%**
PERCENT SWELL/COMPRESSION: **- 1.5**

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SWELL/CONSOLIDATION TEST RESULTS

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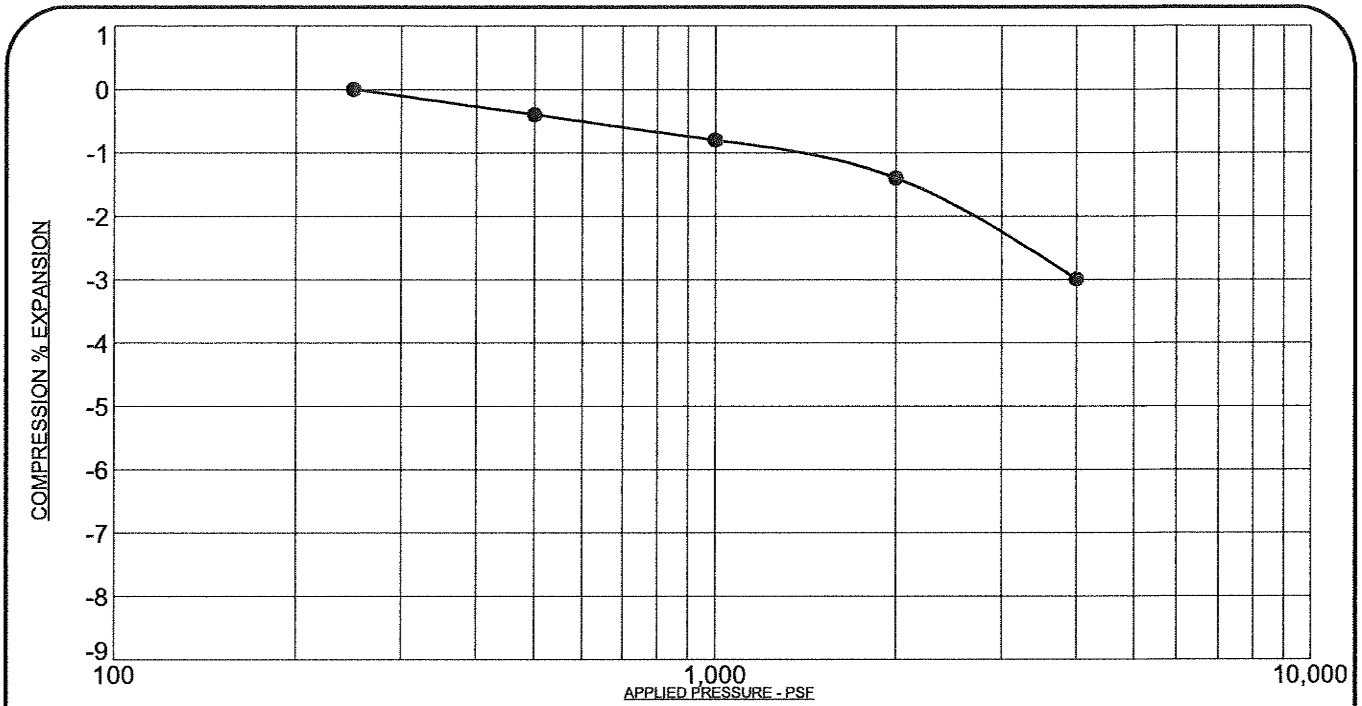
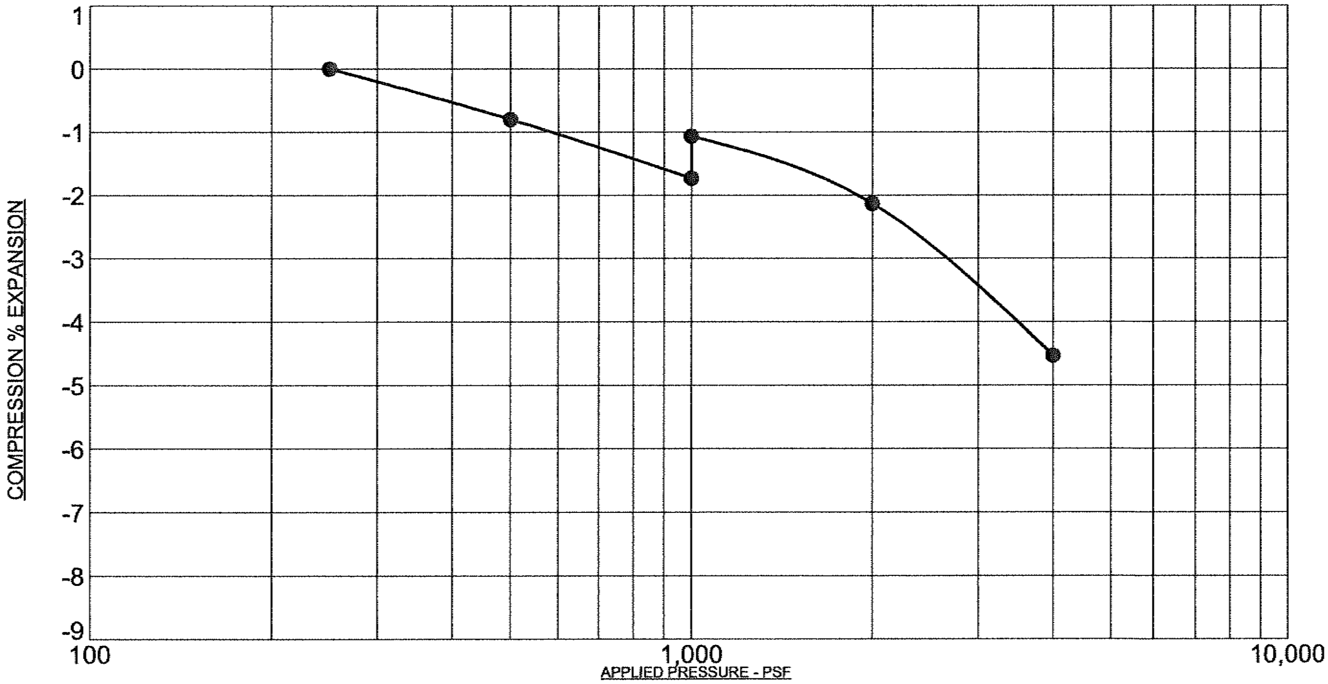
FIGURE No. 17

DATE 8/10/18



PROJECT: **Creekside at Lorson Ranch, Filing No. 1 El Paso County, Colorado**
SAMPLE DESCRIPTION: **CLAY, SANDY**
NOTE: SAMPLE WAS INUNDATED WITH WATER AT 1,000 PSF

SAMPLE LOCATION: **10 @ 14 FT**
NATURAL DRY UNIT WEIGHT: **77.4 PCF**
NATURAL MOISTURE CONTENT: **20.9%**
PERCENT SWELL/COMPRESSION: **0.0**



PROJECT: **Creekside at Lorson Ranch, Filing No. 1 El Paso County, Colorado**
SAMPLE DESCRIPTION: **CLAY, SANDY**
NOTE: SAMPLE WAS INUNDATED WITH WATER AT 1,000 PSF

SAMPLE LOCATION: **12 @ 9 FT**
NATURAL DRY UNIT WEIGHT: **85.0 PCF**
NATURAL MOISTURE CONTENT: **34.8%**
PERCENT SWELL/COMPRESSION: **0.7**

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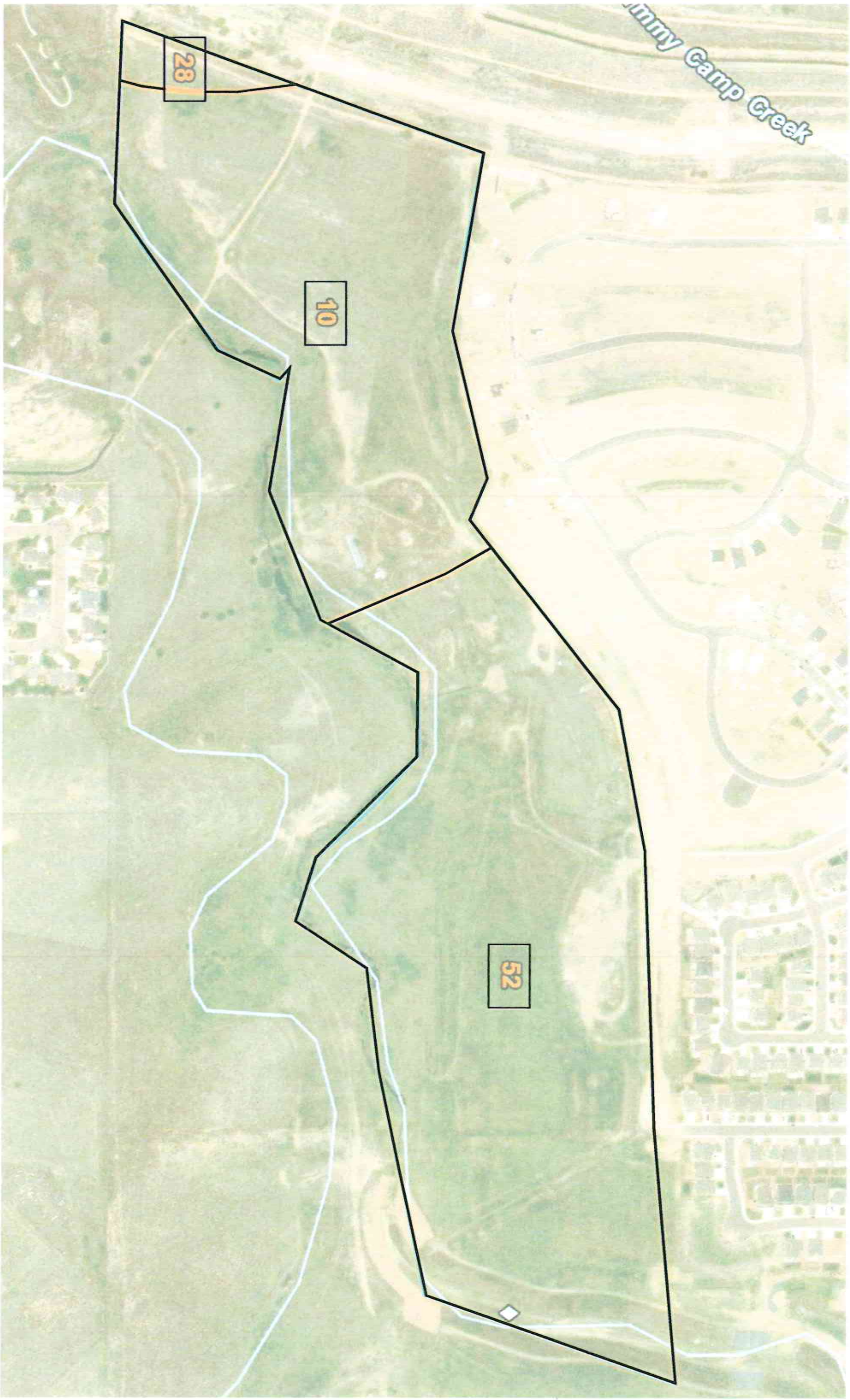
SWELL/CONSOLIDATION TEST RESULTS

JOB No. 164808

FIGURE No. 18

DATE 8/10/18



REFER TO SECTION 6.3, PAGE 8 OF THE GEOLOGY AND SOILS REPORT FOR SOIL EXPLANATIONS

NOT TO SCALE

BASE MAP PROVIDED BY: U.S. SOIL CONSERVATION SERVICE

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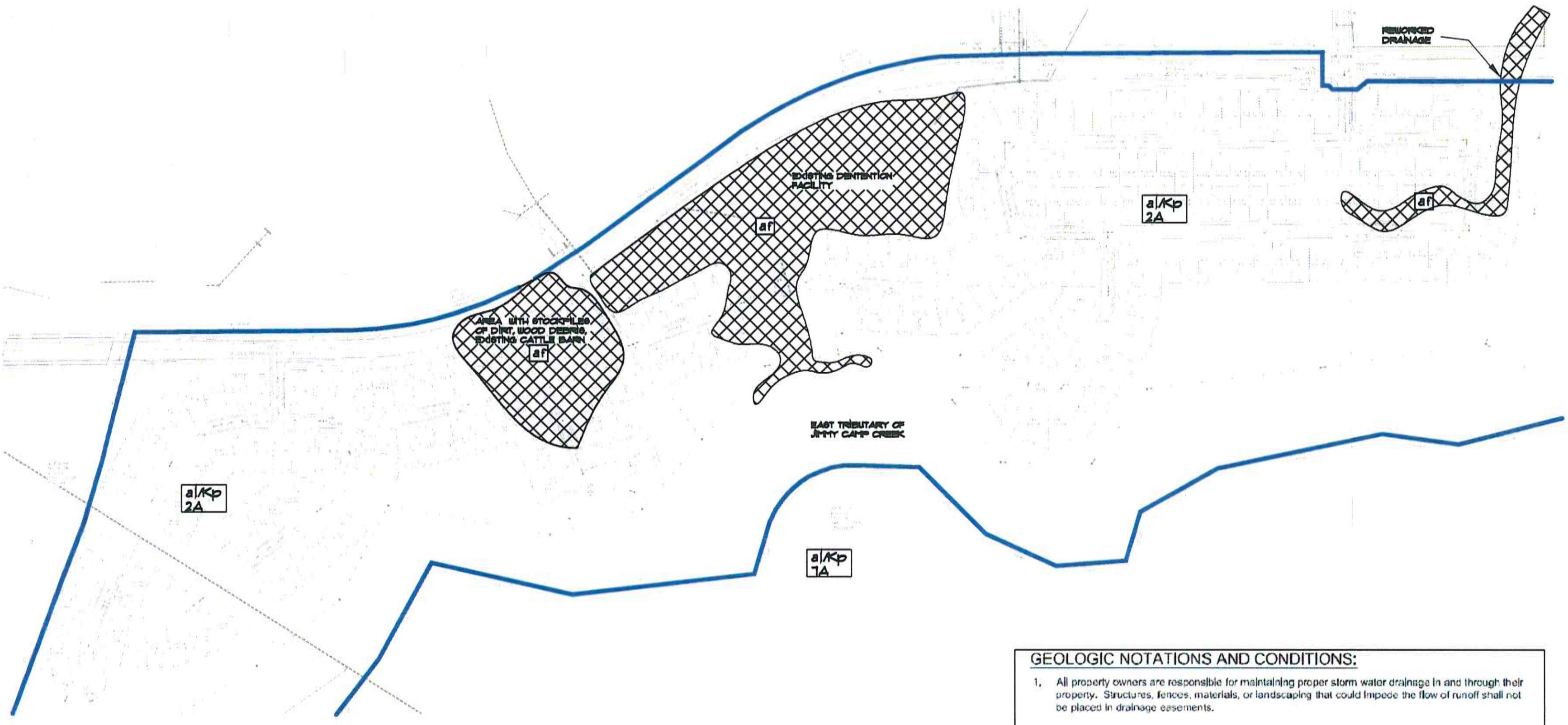
USDA SOIL TYPES

CREEKSIDE AT LORSON RANCH
FILING NO. 1
EL PASO COUNTY, COLORADO
LORSON RANCH METRO DISTRICT NO. 1

JOB No. 164808

FIG No. 19

DATE 8-10-2018



GENERAL GEOLOGY

af - artificial fill
al - Stable alluvium
Kp - Pierre Shale

ENGINEERING GEOLOGY

2A - Stable alluvium, colluvium and bedrock on gentle to moderate slopes (5-12%)

7A - Physiographic floodplain

NOT TO SCALE

GEOLOGIC NOTATIONS AND CONDITIONS:

1. All property owners are responsible for maintaining proper storm water drainage in and through their property. Structures, fences, materials, or landscaping that could impede the flow of runoff shall not be placed in drainage easements.

2. Developer shall comply with local federal and state laws, regulations, ordinances, review and permit requirements, and other agency requirements, if any, of applicable agencies including, but not limited to, the Colorado Division of Wildlife, Colorado Department of Transportation, U.S. Army Corps of Engineers and the U.S. Fish and Wildlife Service regarding the Endangered Species Act.

3. A 'Soils and Geology Report for Lorson Ranch East, El Paso County, Colorado' was completed by Rocky Mountain Group (RMG). There are no significant geological hazards; however, the potential for geologic hazards or constraints do exists related to the potential for expansive or hydrocompactive soils. The geologic conditions are considered relatively common to the immediate area and mitigation can be accomplished by implementing common engineering and construction practices.

4. Site specific subsurface soil investigations shall be conducted prior to construction on all lots. In addition to providing anticipated foundation design recommendations, these investigations should also consider lot-specific recommendations relating to the following geologic conditions:
   a. Mitigation for loose and/ or expansive soil conditions (if encountered), and
   b. Potential shallow groundwater conditions and feasibility of below-grade construction.

5. At a minimum, separate subsurface perimeter drains should be provided around the below-grade (habitable) portions of each foundation. Additional drainage measures may also be required as determined by the lot-specific subsurface soil investigation and/ or the lot-specific excavation observation performed at the time of construction.

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Monument Office:
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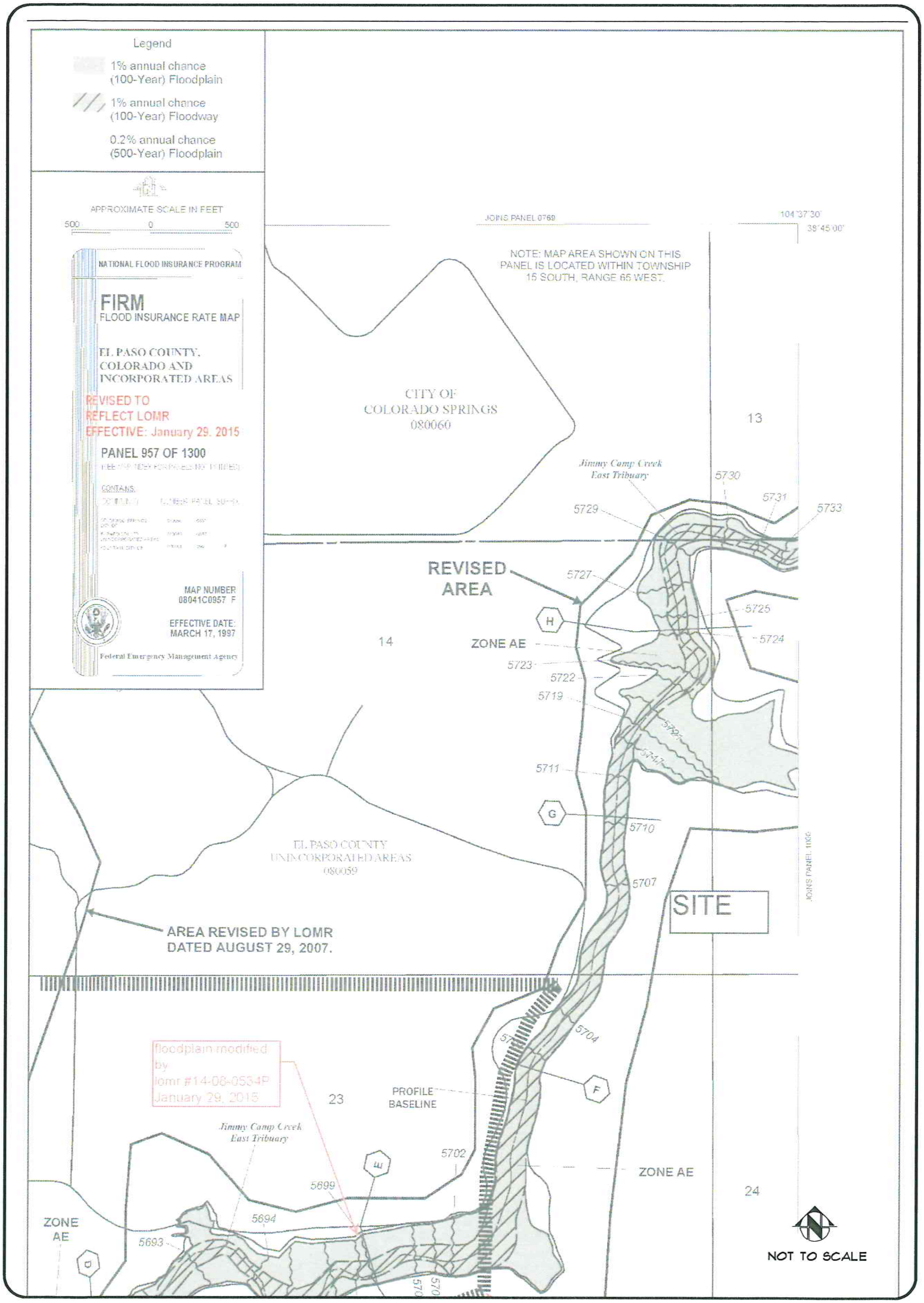
CREEKSIDE AT
LORSON, FILING NO. 1

EL PASO COUNTY, CO
LORSON RANCH METRO
DISTRICT, NO. 1

ENGINEER:	GW
DRAWN BY:	KZ
CHECKED BY:	GW
ISSUED:	8-10-2018
REVISION:	DATE:
	JOB #:

GENERAL GEOLOGIC
CONDITIONS MAP

SHEET No.
FIG-20



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CREEKSIDE AT LORSON, FILING NO. 1

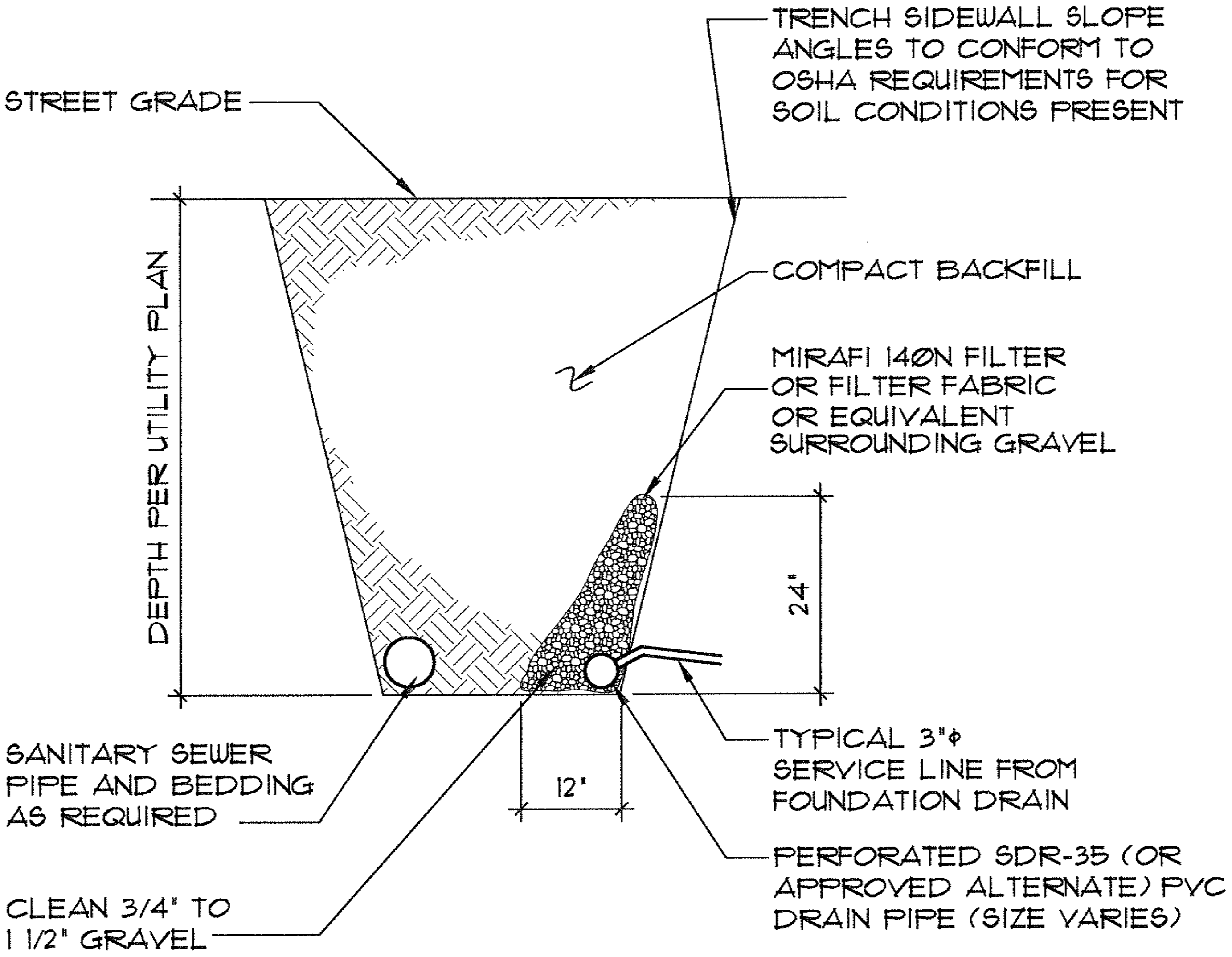
EL PASO COUNTY, CO
LORSON RANCH METRO DISTRICT, NO. 1

ENGINEER: GW
DRAWN BY: KZ
CHECKED BY: GW
ISSUED: 8-10-2018
DATE:
JOB #:
REVISION:

REVISED FEMA MAP TO REFLECT LOMR

SHEET No.
FIG-21

NOTE: TO BE USED IN CASES WHERE GROUNDWATER IS FOUND DURING TRENCHING OR WHERE SHALLOW GROUNDWATER IS KNOWN TO EXIST



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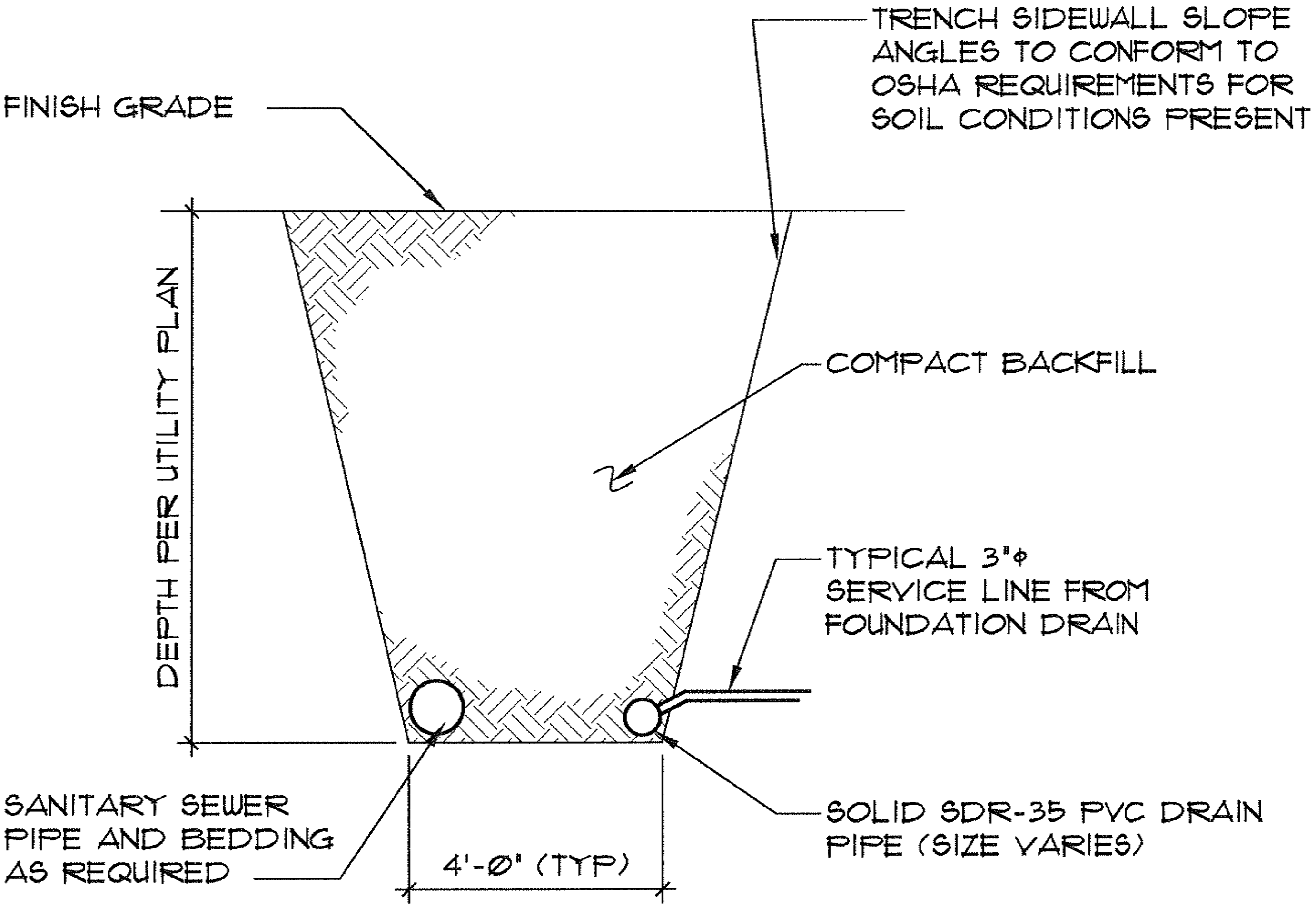
ACTIVE UNDERDRAIN IN SANITARY SEWER TRENCH

JOB. No. 164808

FIG. No. 22

DATE 8-7-18

NOTE: TO BE USED WHERE NO SHALLOW GROUNDWATER IS KNOWN TO EXIST



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PASSIVE UNDERDRAIN IN SANITARY SEWER TRENCH

JOB. No. 164808
FIG. No. 23
DATE 8-7-18

APPENDIX A
GUIDELINE SITE GRADING SPECIFICATIONS

Guideline Site Grading Specifications

Description: Unless specified otherwise by local or state regulatory agencies, these guideline specifications are for the excavation, placement and compaction of material from locations indicated on the plans, or staked by the Engineer, as necessary to achieve the required elevations. These specifications shall also apply to compaction of materials that may be placed outside of the project.

General: The Geotechnical Engineer shall approve fill materials, method of placement, moisture contents and percent compactions, and shall give written approval of the compacted fill.

Clearing Site: The Contractor shall remove trees, brush, rubbish, vegetation, topsoil and existing structures before excavation or fill placement is commenced. The Contractor shall dispose of the cleared material to provide the Owner with a clean job site. Cleared material shall not be placed in areas to receive fill or where the material will support structures. Clearing shall also include removal of existing fills that do not meet the requirements of this specification and existing structures.

Preparation of Slopes or Drainage Areas to Receive Fill: Natural slopes or slopes of drainage gullies where grades are 20 percent (5:1, horizontal to vertical) or steeper shall be benched prior to fill placement. Benches shall be at least 10 feet wide. Benches may require additional width to accommodate excavation or compaction equipment. At least one bench shall be provided for each 5 feet or less of vertical elevation difference. The bench surface shall be essentially horizontal perpendicular to the slope or at a slight incline into the slope.

Scarifying: Topsoil and vegetation shall be removed from the ground surface in areas to receive fill. The surface shall be plowed or scarified a minimum of 12 inches until the surface is free from ruts, hummocks or other uneven features which would prevent uniform compaction by the equipment to be used.

Compacting Area to Receive Fill: After the area to receive fill has been cleared and scarified, it shall be disked or bladed until it is free from large clods, moisture conditioned to a proper moisture content and compacted to the maximum density as specified for the overlying fill. Areas to receive fill shall be worked, stabilized, or removed and replaced, if necessary, in accordance with the Geotechnical Engineer's recommendations in preparation for fill.

Fill Materials: Fill material shall be free from organic material or other deleterious substances, and shall not contain rocks or lumps having a diameter greater than six inches. Fill materials shall be obtained from cut areas shown on the plans or staked in the field by the Engineer or imported to the site and shall be approved by the Geotechnical Engineer prior to placement. It is recommended that the fill materials have nil to low expansion potential, i.e., consist of silty to slightly clayey sand.

- The moisture-conditioned materials should be placed in maximum 6" compacted lifts. These materials should be compacted to a minimum of 92 percent of the maximum Modified Proctor dry density or 95 percent of the maximum Standard Proctor dry density. Material not meeting the above requirements shall be reprocessed.

Materials used for moisture-conditioned structural fill should be approved by RMG prior to use. Moisture-conditioned structural fill should not be placed on frozen subgrade or allowed to freeze during moisture conditioning and placement.

Moisture Content: Fill materials shall be moisture conditioned to within limits of optimum moisture content specified. Sufficient laboratory compaction tests shall be made to determine the optimum moisture content for the various soils encountered in borrow areas or imported to the site.

The contractor may be required to add moisture to the excavation materials in the borrow area if, in the opinion of the Geotechnical Engineer, it is not possible to obtain uniform moisture content by adding water to the fill material during placement. The Contractor may be required to rake or disk the fill soils to provide uniform moisture content through the soils.

The application of water to embankment materials shall be made with watering equipment, approved by the Geotechnical Engineer, which will give the desired results. Water jets from the spreader shall not be directed at the embankment with such force that fill materials are eroded.

Should too much water be added to the fill, such that the material is too wet to permit the desired compaction to be obtained, compacting and work on that section of the fill shall be delayed until the material has been allowed to dry to the required moisture content. The Contractor will be permitted to rework the wet material in an approved manner to hasten its drying.

Compaction of Fill Areas: Selected fill material shall be placed and mixed in evenly spread layers. After each fill layer has been placed, it shall be uniformly compacted to not less than the specified percentage of maximum density. Fill materials shall be placed such that the thickness of loose material does not exceed 10 inches and the compacted lift thickness does not exceed 6 inches.

Compaction, as specified above, shall be obtained by the use of sheepsfoot rollers, multiple-wheel pneumatic-tired rollers, or other equipment approved by the Geotechnical Engineer. Granular fill shall be compacted using vibratory equipment or other equipment approved by the Geotechnical Engineer. Compaction shall be accomplished while the fill material is at the specified moisture content. Compaction of each layer shall be continuous over the entire area.

Moisture Content and Density Criteria:

A. Fill placed in roadways and utility trenches should be moisture conditioned and compacted in accordance with El Paso County Specifications.

B. Fill placed outside of roadways and utility trenches should be compacted to at least 92% of the maximum Modified Proctor density (ASTM D-1557) or at least 95% of the maximum Standard Proctor density (ASTM D-698) at a moisture content within 2% of optimum.

Compaction of Slopes: Fill slopes shall be compacted by means of sheepsfoot rollers or other suitable equipment. Compaction operations shall be continued until slopes are stable, but not too dense for planting, and such that there is no appreciable amount of loose soil on the slopes. Compaction of slopes may be done progressively in increments of three to five feet in height or after the fill is brought to its total height. Permanent fill slopes shall not exceed 3:1 (horizontal to vertical).

Density Testing: Field density testing shall be performed by the Geotechnical Engineer at locations and depths of his choosing. Where sheepsfoot rollers are used, the soil may be disturbed to a depth of several inches. Density tests shall be taken in compacted material below the disturbed surface. When density tests indicate the density or moisture content of any layer of fill or portion thereof is below that required, the particular layer or portion shall be reworked until the required density or moisture content has been achieved.

Observation and Testing of Fill: Observation by the Geotechnical Engineer shall be sufficient during the placement of fill and compaction operations so that he can declare the fill was placed in general conformance with Specifications. All observations necessary to test the placement of fill and observe compaction operations will be at the expense of the Owner.

Seasonal Limits: No fill material shall be placed, spread or rolled while it is frozen, thawing, or during unfavorable weather conditions. When work is interrupted by heavy precipitation, fill operations shall not be resumed until the Geotechnical Engineer indicates the moisture content and density of previously placed materials are as specified.

Reporting of Field Density Tests: Density tests made by the Geotechnical Engineer shall be submitted progressively to the Owner. Dry density, moisture content, percent compaction, and approximate location shall be reported for each test taken.

APPENDIX B

USGS Seismic Data

USGS Design Maps Summary Report

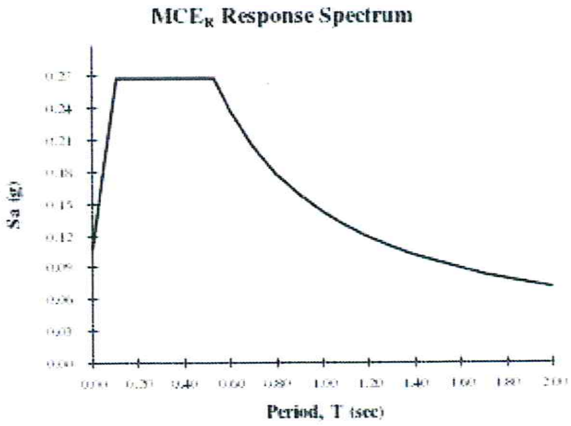
User–Specified Input

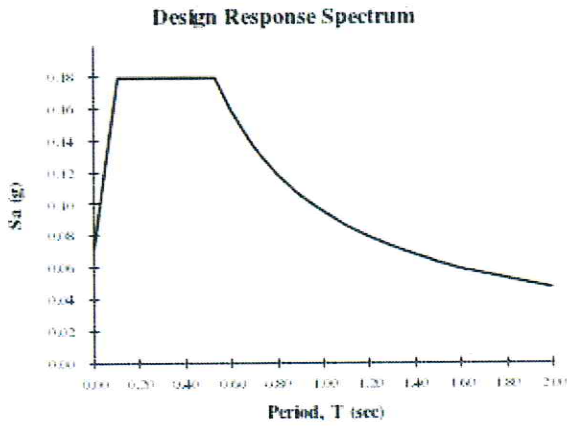
Report Title	Creekside at Lorson Ranch, Filing No. 1 Tue August 7, 2018 21:05:46 UTC
Building Code Reference Document	2012/2015 International Building Code (which utilizes USGS hazard data available in 2008)
Site Coordinates	38.73373°N, 104.64357°W
Site Soil Classification	Site Class D – "Stiff Soil"
Risk Category	I/II/III

USGS–Provided Output

S_S =	0.168 g	S_{MS} =	0.268 g	S_{DS} =	0.179 g
S_1 =	0.059 g	S_{M1} =	0.142 g	S_{D1} =	0.095 g

For information on how the SS and S1 values above have been calculated from probabilistic (risk-targeted) and deterministic ground motions in the direction of maximum horizontal response, please return to the application and select the "2009 NEHRP" building code reference document.





United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for

El Paso County Area, Colorado

Lorson Ranch

April 26, 2017

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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Contents

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

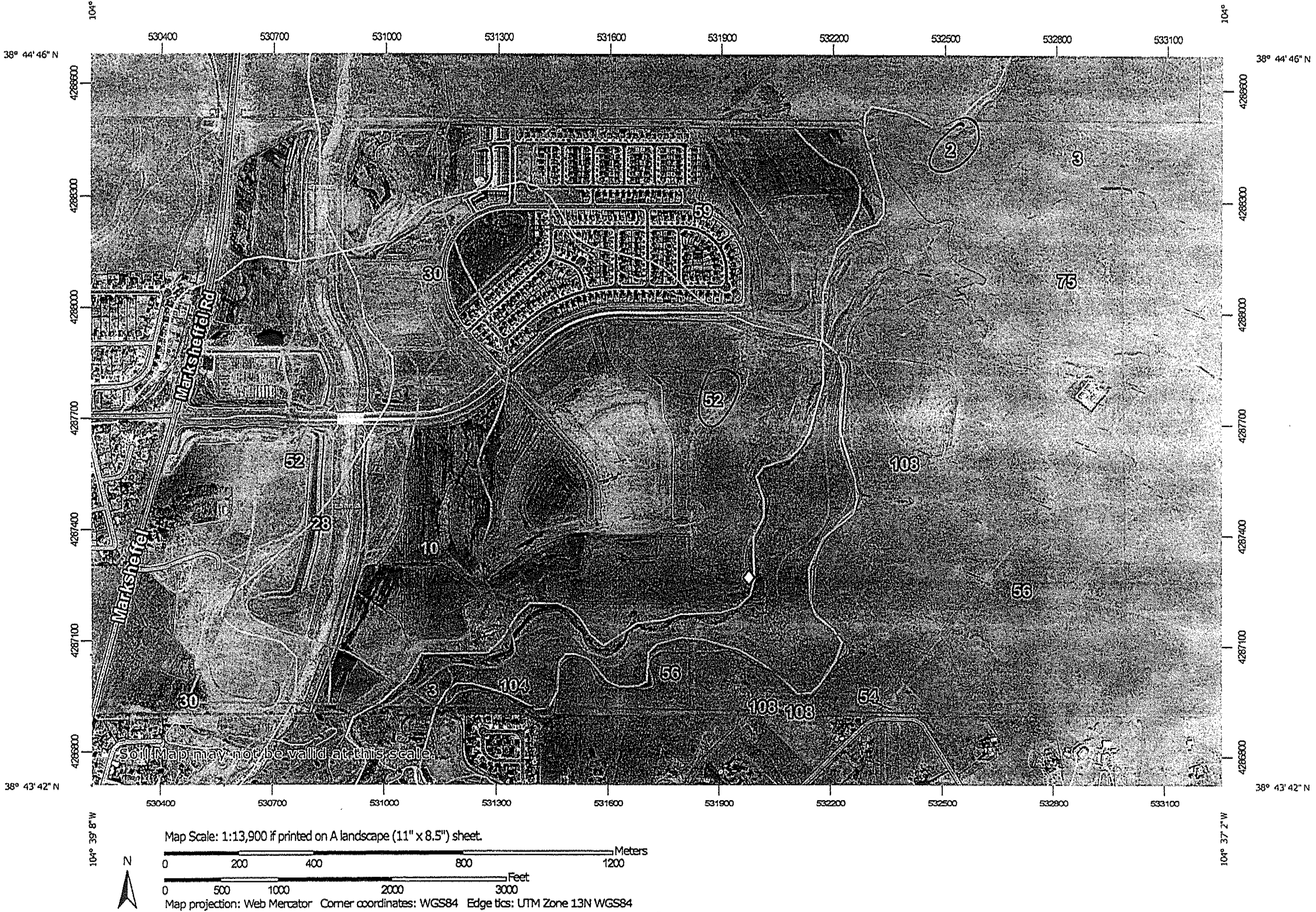
Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report
Soil Map



Soil Map may not be valid at this scale.

Map Scale: 1:13,900 if printed on A landscape (11" x 8.5") sheet.

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)
- ☐ Area of Interest (AOI)

Soils
- ☐ Soil Map Unit Polygons
- Soil Map Unit Lines
- ⊡ Soil Map Unit Points

Special Point Features
- Blowout
- ⊠ Borrow Pit
- ✱ Clay Spot
- ◇ Closed Depression
- ⚒ Gravel Pit
- ∴ Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- ◎ Miscellaneous Water
- ◎ Perennial Water
- ∨ Rock Outcrop
- + Saline Spot
- ∴ Sandy Spot
- Severely Eroded Spot
- ◇ Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- △ Other
- Special Line Features

Water Features
- ~ Streams and Canals

Transportation
- ┼┼┼ Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- ■ Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 14, Sep 23, 2016

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 15, 2011—Sep 22, 2011

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

El Paso County Area, Colorado (CO625)			
Map Unit Symbol	**Map Unit Name**	**Acres in AOI**	**Percent of AOI**
2	Ascalon sandy loam, 1 to 3 percent slopes	12.5	1.5%
3	Ascalon sandy loam, 3 to 9 percent slopes	11.0	1.3%
10	Blendon sandy loam, 0 to 3 percent slopes	70.2	8.2%
28	Ellicott loamy coarse sand, 0 to 5 percent slopes	75.7	8.9%
30	Fort Collins loam, 0 to 3 percent slopes	24.8	2.9%
52	Manzanst clay loam, 0 to 3 percent slopes	315.6	37.0%
54	Midway clay loam, 3 to 25 percent slopes	3.7	0.4%
56	Nelson-Tassel fine sandy loams, 3 to 18 percent slopes	129.4	15.2%
59	Nunn clay loam, 0 to 3 percent slopes	85.4	10.0%
75	Razor-Midway complex	25.8	3.0%
104	Vona sandy loam, warm, 0 to 3 percent slopes	9.7	1.1%
108	Wiley silt loam, 3 to 9 percent slopes	89.2	10.5%
Totals for Area of Interest		**852.7**	**100.0%**

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

2—Ascalon sandy loam, 1 to 3 percent slopes

Map Unit Setting

National map unit symbol: 367q
Elevation: 5,500 to 6,500 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 47 to 50 degrees F
Frost-free period: 130 to 150 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Ascalon and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Ascalon

Setting

Landform: Flats
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Mixed alluvium and/or eolian deposits

Typical profile

A - 0 to 8 inches: sandy loam
Bt - 8 to 21 inches: sandy clay loam
BC - 21 to 27 inches: sandy loam
Ck1 - 27 to 48 inches: sandy loam
Ck2 - 48 to 60 inches: loamy sand

Properties and qualities

Slope: 1 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 10 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: Moderate (about 7.1 inches)

Interpretive groups

Land capability classification (irrigated): 3e
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: B
Ecological site: Sandy Plains LRU's A & B (R069XY026CO)
Other vegetative classification: SANDY PLAINS (069BY026CO)
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:
Hydric soil rating: No

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

3—Ascalon sandy loam, 3 to 9 percent slopes

Map Unit Setting

National map unit symbol: 2tlny
Elevation: 3,870 to 5,960 feet
Mean annual precipitation: 13 to 18 inches
Mean annual air temperature: 46 to 54 degrees F
Frost-free period: 95 to 155 days
Farmland classification: Not prime farmland

Map Unit Composition

Ascalon and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Ascalon

Setting

Landform: Interfluves
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Wind-reworked alluvium and/or calcareous sandy eolian deposits

Typical profile

Ap - 0 to 6 inches: sandy loam
Bt1 - 6 to 12 inches: sandy clay loam
Bt2 - 12 to 19 inches: sandy clay loam
Bk1 - 19 to 35 inches: fine sandy loam
Bk2 - 35 to 80 inches: fine sandy loam

Properties and qualities

Slope: 3 to 9 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 5.98 in/hr)

Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 10 percent
Salinity, maximum in profile: Nonsaline (0.1 to 1.9 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 1.0
Available water storage in profile: Moderate (about 7.1 inches)

Interpretive groups

Land capability classification (irrigated): 6e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: B
Ecological site: Sandy Plains (R067BY024CO)
Hydric soil rating: No

Minor Components

Olnest

Percent of map unit: 10 percent
Landform: Interfluves
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: Sandy Plains (R067BY024CO)
Hydric soil rating: No

Vona

Percent of map unit: 5 percent
Landform: Interfluves
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: Sandy Plains (R067BY024CO)
Hydric soil rating: No

10—Blendon sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 3671
Elevation: 6,000 to 6,800 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 46 to 48 degrees F
Frost-free period: 125 to 145 days
Farmland classification: Not prime farmland

Map Unit Composition

Blendon and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Blendon

Setting

Landform: Alluvial fans, terraces
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Sandy alluvium derived from arkose

Typical profile

A - 0 to 10 inches: sandy loam
Bw - 10 to 36 inches: sandy loam
C - 36 to 60 inches: gravelly sandy loam

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 2 percent
Available water storage in profile: Moderate (about 6.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: B
Ecological site: Sandy Foothill (R049BY210CO)
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:
Hydric soil rating: No

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

28—Ellicott loamy coarse sand, 0 to 5 percent slopes

Map Unit Setting

National map unit symbol: 3680
Elevation: 5,500 to 6,500 feet
Mean annual precipitation: 13 to 15 inches
Mean annual air temperature: 47 to 50 degrees F

Frost-free period: 125 to 145 days
Farmland classification: Not prime farmland

Map Unit Composition

Ellicott and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Ellicott

Setting

Landform: Flood plains, stream terraces
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Sandy alluvium

Typical profile

A - 0 to 4 inches: loamy coarse sand
C - 4 to 60 inches: stratified coarse sand to sandy loam

Properties and qualities

Slope: 0 to 5 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: Frequent
Frequency of ponding: None
Available water storage in profile: Low (about 4.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7w
Hydrologic Soil Group: A
Ecological site: Sandy Bottomland LRU's A & B (R069XY031CO)
Other vegetative classification: SANDY BOTTOMLAND (069AY031CO)
Hydric soil rating: No

Minor Components

Fluvaquentic haplaquoll

Percent of map unit:
Landform: Swales
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

30—Fort Collins loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 3683
Elevation: 5,200 to 6,500 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 135 to 155 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Fort collins and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Fort Collins

Setting

Landform: Flats
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Loamy alluvium

Typical profile

A - 0 to 9 inches: loam
Bt - 9 to 16 inches: clay loam
Bk - 16 to 21 inches: clay loam
Ck - 21 to 60 inches: loam

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: High (about 10.1 inches)

Interpretive groups

Land capability classification (irrigated): 2e
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: B
Ecological site: Loamy Plains (R067BY002CO)
Other vegetative classification: LOAMY PLAINS (069AY006CO)

Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

52—Manzanst clay loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2w4nr
Elevation: 4,060 to 6,660 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 50 to 54 degrees F
Frost-free period: 130 to 170 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Manzanst and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Manzanst

Setting

Landform: Terraces, drainageways
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear, concave
Parent material: Clayey alluvium derived from shale

Typical profile

A - 0 to 3 inches: clay loam
Bt - 3 to 12 inches: clay
Btk - 12 to 37 inches: clay
Bk1 - 37 to 52 inches: clay
Bk2 - 52 to 79 inches: clay

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches

Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Gypsum, maximum in profile: 3 percent
Salinity, maximum in profile: Slightly saline (4.0 to 7.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 10.0
Available water storage in profile: High (about 9.0 inches)

Interpretive groups

Land capability classification (irrigated): 3e
Land capability classification (nonirrigated): 4c
Hydrologic Soil Group: C
Ecological site: Saline Overflow (R067BY037CO)
Hydric soil rating: No

Minor Components

Ritoazul

Percent of map unit: 7 percent
Landform: Drainageways, interfluves
Landform position (three-dimensional): Rise
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: Clayey Plains (R067BY042CO)
Hydric soil rating: No

Arvada

Percent of map unit: 6 percent
Landform: Drainageways, interfluves
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: Salt Flat (R067XY033CO)
Hydric soil rating: No

Wiley

Percent of map unit: 2 percent
Landform: Interfluves
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: Loamy Plains (R067BY002CO)
Hydric soil rating: No

54—Midway clay loam, 3 to 25 percent slopes

Map Unit Setting

National map unit symbol: 368y
Elevation: 5,200 to 6,200 feet
Mean annual precipitation: 12 to 14 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 135 to 155 days
Farmland classification: Not prime farmland

Map Unit Composition

Midway and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Midway

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Slope alluvium over residuum weathered from shale

Typical profile

A - 0 to 4 inches: clay loam
C - 4 to 13 inches: clay
Cr - 13 to 17 inches: weathered bedrock

Properties and qualities

Slope: 3 to 25 percent
Depth to restrictive feature: 6 to 20 inches to paralithic bedrock
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Gypsum, maximum in profile: 15 percent
Salinity, maximum in profile: Very slightly saline to moderately saline (2.0 to 8.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 15.0
Available water storage in profile: Very low (about 2.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7e
Hydrologic Soil Group: D
Ecological site: Shaly Plains LRU's A & B (R069XY046CO)
Other vegetative classification: SHALY PLAINS (069AY046CO)
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:
Hydric soil rating: No

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

56—Nelson-Tassel fine sandy loams, 3 to 18 percent slopes

Map Unit Setting

National map unit symbol: 3690
Elevation: 5,600 to 6,400 feet
Mean annual precipitation: 12 to 14 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 135 to 155 days
Farmland classification: Not prime farmland

Map Unit Composition

Nelson and similar soils: 45 percent
Tassel and similar soils: 30 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Nelson

Setting

Landform: Hills
Landform position (three-dimensional): Crest, side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Calcareous residuum weathered from interbedded sedimentary rock

Typical profile

A - 0 to 5 inches: fine sandy loam
Ck - 5 to 23 inches: fine sandy loam
Cr - 23 to 27 inches: weathered bedrock

Properties and qualities

Slope: 3 to 12 percent
Depth to restrictive feature: 20 to 40 inches to paralithic bedrock
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.06 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 10 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: Very low (about 2.8 inches)

Interpretive groups

Land capability classification (irrigated): 4e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: B
Ecological site: Shaly Plains (R067BY045CO)

Other vegetative classification: SHALY PLAINS (069AY046CO)
Hydric soil rating: No

Description of Tassel

Setting

Landform: Hills
Landform position (three-dimensional): Crest, side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Calcareous slope alluvium over residuum weathered from sandstone

Typical profile

A - 0 to 4 inches: fine sandy loam
C - 4 to 10 inches: fine sandy loam
Cr - 10 to 14 inches: weathered bedrock

Properties and qualities

Slope: 3 to 18 percent
Depth to restrictive feature: 6 to 20 inches to paralithic bedrock
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 10 percent
Available water storage in profile: Very low (about 1.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6s
Hydrologic Soil Group: D
Ecological site: Shaly Plains (R067BY045CO)
Other vegetative classification: SHALY PLAINS (069AY046CO)
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:
Hydric soil rating: No

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

59—Nunn clay loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 3693
Elevation: 5,400 to 6,500 feet
Mean annual precipitation: 13 to 15 inches
Mean annual air temperature: 46 to 50 degrees F
Frost-free period: 135 to 155 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Nunn and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Nunn

Setting

Landform: Terraces, fans
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Mixed alluvium

Typical profile

A - 0 to 12 inches: clay loam
Bt - 12 to 26 inches: clay loam
BC - 26 to 30 inches: clay loam
Bk - 30 to 58 inches: sandy clay loam
C - 58 to 72 inches: clay

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Gypsum, maximum in profile: 2 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: High (about 9.8 inches)

Interpretive groups

Land capability classification (irrigated): 2e
Land capability classification (nonirrigated): 3c
Hydrologic Soil Group: C

Ecological site: Clayey Plains LRU's A & B (R069XY042CO)
Other vegetative classification: CLAYEY PLAINS (069AY042CO)
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:
Hydric soil rating: No

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

75—Razor-Midway complex

Map Unit Setting

National map unit symbol: 369p
Elevation: 5,300 to 6,100 feet
Mean annual precipitation: 12 to 14 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 135 to 155 days
Farmland classification: Not prime farmland

Map Unit Composition

Razor and similar soils: 50 percent
Midway and similar soils: 30 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Razor

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Concave, linear
Across-slope shape: Linear
Parent material: Clayey slope alluvium over residuum weathered from shale

Typical profile

A - 0 to 4 inches: stony clay loam
Bw - 4 to 22 inches: cobbly clay loam
Bk - 22 to 29 inches: cobbly clay
Cr - 29 to 33 inches: weathered bedrock

Properties and qualities

Slope: 3 to 15 percent
Depth to restrictive feature: 20 to 40 inches to paralithic bedrock
Natural drainage class: Well drained
Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Gypsum, maximum in profile: 5 percent
Salinity, maximum in profile: Moderately saline to strongly saline (8.0 to 16.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 15.0
Available water storage in profile: Low (about 4.7 inches)

Interpretive groups

Land capability classification (irrigated): 6e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: D
Ecological site: Alkaline Plains LRU's A & B (R069XY047CO)
Other vegetative classification: ALKALINE PLAINS (069AY047CO)
Hydric soil rating: No

Description of Midway

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Slope alluvium over residuum weathered from shale

Typical profile

A - 0 to 4 inches: clay loam
C - 4 to 13 inches: clay
Cr - 13 to 17 inches: weathered bedrock

Properties and qualities

Slope: 3 to 25 percent
Depth to restrictive feature: 6 to 20 inches to paralithic bedrock
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Gypsum, maximum in profile: 15 percent
Salinity, maximum in profile: Very slightly saline to moderately saline (2.0 to 8.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 15.0
Available water storage in profile: Very low (about 2.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7e
Hydrologic Soil Group: D
Ecological site: Shaly Plains LRU's A & B (R069XY046CO)
Other vegetative classification: SHALY PLAINS (069AY045CO)

Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

104—Vona sandy loam, warm, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2t516
Elevation: 3,590 to 6,000 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 50 to 54 degrees F
Frost-free period: 130 to 170 days
Farmland classification: Not prime farmland

Map Unit Composition

Vona, warm, and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Vona, Warm

Setting

Landform: Sand sheets
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Eolian sands

Typical profile

A - 0 to 5 inches: sandy loam
Bt1 - 5 to 12 inches: sandy loam
Bt2 - 12 to 17 inches: sandy loam
Bk - 17 to 41 inches: sandy loam
BCk - 41 to 79 inches: loamy sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat excessively drained

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Gypsum, maximum in profile: 2 percent
Salinity, maximum in profile: Nonsaline to slightly saline (0.5 to 4.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 2.0
Available water storage in profile: Moderate (about 7.2 inches)

Interpretive groups

Land capability classification (irrigated): 4e
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: A
Ecological site: Sandy Plains (R067BY024CO)
Other vegetative classification: Loamy, Dry (G067BW019CO), Sandy Plains #24 (067XY024CO_2)
Hydric soil rating: No

Minor Components

Valent, warm

Percent of map unit: 5 percent
Landform: Sand sheets
Landform position (two-dimensional): Shoulder, backslope
Landform position (three-dimensional): Crest, side slope
Down-slope shape: Convex
Across-slope shape: Convex
Ecological site: Deep Sand (R067BY015CO)
Other vegetative classification: Sandy, Dry (G067BW026CO), Deep Sands #15 (067XY015CO_3)
Hydric soil rating: No

Olnest, warm

Percent of map unit: 5 percent
Landform: Hillslopes
Landform position (two-dimensional): Summit, backslope
Landform position (three-dimensional): Crest, side slope
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: Sandy Plains (R067BY024CO)
Other vegetative classification: Loamy, Dry (G067BW019CO)
Hydric soil rating: No

Otero

Percent of map unit: 5 percent
Landform: Hillslopes
Landform position (two-dimensional): Shoulder, backslope
Landform position (three-dimensional): Side slope, head slope
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: Sandy Plains (R067BY024CO)
Other vegetative classification: Loamy, Dry (G067BW019CO), SANDY PLAINS (067XY024CO_1)
Hydric soil rating: No

108—Wiley silt loam, 3 to 9 percent slopes

Map Unit Setting

National map unit symbol: 367b
Elevation: 5,200 to 6,200 feet
Mean annual precipitation: 12 to 14 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 135 to 155 days
Farmland classification: Not prime farmland

Map Unit Composition

Wiley and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Wiley

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Calcareous silty eolian deposits

Typical profile

A - 0 to 4 inches: silt loam
Bt - 4 to 16 inches: silt loam
Bk - 16 to 60 inches: silt loam

Properties and qualities

Slope: 3 to 9 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 15 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: High (about 11.5 inches)

Interpretive groups

Land capability classification (irrigated): 4e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: B
Ecological site: Loamy Plains (R067BY002CO)
Other vegetative classification: LOAMY PLAINS (069AY006CO)
Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

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Appendix D

East Fork Jimmy Camp Creek Letter of Map Revision

Case No. 19-08-0605P

No Rise Determination Calculations

Federal Emergency Management Agency
Washington, D.C. 20472

December 18, 2019

CERTIFIED MAIL
RETURN RECEIPT REQUESTED

The Honorable John Suthers
Mayor, City of Colorado Springs
30 South Nevada Avenue, Suite 601
Colorado Springs, CO 80903

IN REPLY REFER TO:
Case No.: 19-08-0605P
Follows Conditional
Case No.: 17-08-1043R
Community Name: City of Colorado Springs, CO
Community No.: 080060
Effective Date of
This Revision: May 4, 2020

Dear Mayor Suthers:

The Flood Insurance Study (FIS) Report and Flood Insurance Rate Map (FIRM) for your community have been revised by this Letter of Map Revision (LOMR). Please use the enclosed annotated map panel(s) revised by this LOMR for floodplain management purposes and for all flood insurance policies and renewals issued in your community.

Additional documents are enclosed that provide information regarding this LOMR. Please see the List of Enclosures below to determine which documents are included. Other enclosures specific to this request may be included as referenced in the Determination Document. If you have any questions regarding floodplain management regulations for your community or the National Flood Insurance Program (NFIP) in general, please contact the Consultation Coordination Officer for your community. If you have any technical questions regarding this LOMR, please contact the Director, Mitigation Division of the Department of Homeland Security's Federal Emergency Management Agency (FEMA) in Denver, Colorado, at (303) 235-4830, or the FEMA Map Information eXchange (FMIX) toll free at 1-877-336-2627 (1-877-FEMA MAP). Additional information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Sincerely,

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

List of Enclosures:

Letter of Map Revision Determination Document
Annotated Flood Insurance Rate Map
Annotated Flood Insurance Study Report

cc: The Honorable Mark Waller
President, El Paso County
Board of Commissioners

Mr. Keith Curtis, P.E., CFM
Floodplain Administrator
City of Colorado Springs and El Paso County

Mr. Stephen A. Brown, P.E.
Principal
Kiowa Engineering Corp.

Page 1 of 5	**Issue Date: December 18, 2019**	**Effective Date: May 4, 2020**	**Case No.: 19-08-0605P**	LOMR-APP

Follows Conditional Case No.: 17-08-1043R

Federal Emergency Management Agency
Washington, D.C. 20472

LETTER OF MAP REVISION DETERMINATION DOCUMENT

COMMUNITY AND REVISION INFORMATION		**PROJECT DESCRIPTION**	**BASIS OF REQUEST**
COMMUNITY	**City of Colorado Springs El Paso County Colorado**	**BRIDGE CHANNELIZATION EXCAVATION FILL**	**BASE MAP CHANGES FLOODWAY HYDRAULIC ANALYSIS UPDATED TOPOGRAPHIC DATA**
	COMMUNITY NO.: 080060		
IDENTIFIER	Jimmy Camp Creek East Tributary	**APPROXIMATE LATITUDE & LONGITUDE:** 38.732, -104.636 **SOURCE:** USGS QUADRANGLE **DATUM:** NAD 83	

ANNOTATED MAPPING ENCLOSURES	**ANNOTATED STUDY ENCLOSURES**
TYPE: FIRM* NO.: 08041C0976G DATE: December 7, 2018 TYPE: FIRM* NO.: 08041C0957G DATE: December 7, 2018	DATE OF EFFECTIVE FLOOD INSURANCE STUDY: December 7, 2018 PROFILE: 213P FLOODWAY DATA TABLE: 8

Enclosures reflect changes to flooding sources affected by this revision.
* FIRM - Flood Insurance Rate Map

FLOODING SOURCE AND REVISED REACH

Jimmy Camp Creek East Tributary - From approximately 2,760 feet downstream of Lorson Boulevard to approximately 4,260 feet upstream of Fontaine Boulevard

SUMMARY OF REVISIONS

Flooding Source	**Effective Flooding**	**Revised Flooding**	**Increases**	**Decreases**
Jimmy Camp Creek East Tributary	Zone AE	Zone AE	YES	YES
	Zone X (shaded)	Zone X (shaded)	YES	YES
	BFEs*	BFEs	YES	YES
	Floodway	Floodway	YES	YES

* BFEs - Base Flood Elevations

DETERMINATION

This document provides the determination from the Department of Homeland Security's Federal Emergency Management Agency (FEMA) regarding a request for a Letter of Map Revision (LOMR) for the area described above. Using the information submitted, we have determined that a revision to the flood hazards depicted in the Flood Insurance Study (FIS) report and/or National Flood Insurance Program (NFIP) map is warranted. This document revises the effective NFIP map, as indicated in the attached documentation. Please use the enclosed annotated map panels revised by this LOMR for floodplain management purposes and for all flood insurance policies and renewals in your community.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Information eXchange toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMC Clearinghouse, 3601 Eisenhower Avenue, Suite 500, Alexandria, VA 22304-6426. Additional Information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

OTHER COMMUNITIES AFFECTED BY THIS REVISION

CID Number: 080059 **Name:** El Paso County, Colorado

AFFECTED MAP PANELS	AFFECTED PORTIONS OF THE FLOOD INSURANCE STUDY REPORT
TYPE: FIRM* NO.: 08041C0957G DATE: December 7, 2018 TYPE: FIRM* NO.: 08041C0976G DATE: December 7, 2018	DATE OF EFFECTIVE FLOOD INSURANCE STUDY: December 7, 2018 PROFILE(S): 210P, 211P, 212P, and 213P FLOODWAY DATA TABLE: 8

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Information eXchange toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMC Clearinghouse, 3601 Eisenhower Avenue, Suite 500, Alexandria, VA 22304-6426. Additional Information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

U.S. DEPARTMENT OF HOMELAND SECURITY

Federal Emergency Management Agency
Washington, D.C. 20472

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

COMMUNITY INFORMATION

APPLICABLE NFIP REGULATIONS/COMMUNITY OBLIGATION

We have made this determination pursuant to Section 206 of the Flood Disaster Protection Act of 1973 (P.L. 93-234) and in accordance with the National Flood Insurance Act of 1968, as amended (Title XIII of the Housing and Urban Development Act of 1968, P.L. 90-448), 42 U.S.C. 4001-4128, and 44 CFR Part 65. Pursuant to Section 1361 of the National Flood Insurance Act of 1968, as amended, communities participating in the NFIP are required to adopt and enforce floodplain management regulations that meet or exceed NFIP criteria. These criteria, including adoption of the FIS report and FIRM, and the modifications made by this LOMR, are the minimum requirements for continued NFIP participation and do not supersede more stringent State/Commonwealth or local requirements to which the regulations apply.

We provide the floodway designation to your community as a tool to regulate floodplain development. Therefore, the floodway revision we have described in this letter, while acceptable to us, must also be acceptable to your community and adopted by appropriate community action, as specified in Paragraph 60.3(d) of the NFIP regulations.

NFIP regulations Subparagraph 60.3(b)(7) requires communities to ensure that the flood-carrying capacity within the altered or relocated portion of any watercourse is maintained. This provision is incorporated into your community's existing floodplain management ordinances; therefore, responsibility for maintenance of the altered or relocated watercourse, including any related appurtenances such as bridges, culverts, and other drainage structures, rests with your community. We may request that your community submit a description and schedule of maintenance activities necessary to ensure this requirement.

COMMUNITY REMINDERS

We based this determination on the 1-percent-annual-chance flood discharges computed in the FIS for your community without considering subsequent changes in watershed characteristics that could increase flood discharges. Future development of projects upstream could cause increased flood discharges, which could cause increased flood hazards. A comprehensive restudy of your community's flood hazards would consider the cumulative effects of development on flood discharges subsequent to the publication of the FIS report for your community and could, therefore, establish greater flood hazards in this area.

Your community must regulate all proposed floodplain development and ensure that permits required by Federal and/or State/Commonwealth law have been obtained. State/Commonwealth or community officials, based on knowledge of local conditions and in the interest of safety, may set higher standards for construction or may limit development in floodplain areas. If your State/Commonwealth or community has adopted more restrictive or comprehensive floodplain management criteria, those criteria take precedence over the minimum NFIP requirements.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Information eXchange toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMC Clearinghouse, 3601 Eisenhower Avenue, Suite 500, Alexandria, VA 22304-6426. Additional Information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

We will not print and distribute this LOMR to primary users, such as local insurance agents or mortgage lenders; instead, the community will serve as a repository for the new data. We encourage you to disseminate the information in this LOMR by preparing a news release for publication in your community's newspaper that describes the revision and explains how your community will provide the data and help interpret the NFIP maps. In that way, interested persons, such as property owners, insurance agents, and mortgage lenders, can benefit from the information.

This revision has met our criteria for removing an area from the 1-percent-annual-chance floodplain to reflect the placement of fill. However, we encourage you to require that the lowest adjacent grade and lowest floor (including basement) of any structure placed within the subject area be elevated to or above the Base (1-percent-annual-chance) Flood Elevation.

We have designated a Consultation Coordination Officer (CCO) to assist your community. The CCO will be the primary liaison between your community and FEMA. For information regarding your CCO, please contact:

Ms. Jeanine D. Petterson
Director, Mitigation Division
Federal Emergency Management Agency, Region VIII
Denver Federal Center, Building 710
P.O. Box 25267
Denver, CO 80225-0267
(303) 235-4830

STATUS OF THE COMMUNITY NFIP MAPS

We will not physically revise and republish the FIRM and FIS report for your community to reflect the modifications made by this LOMR at this time. When changes to the previously cited FIRM panel(s) and FIS report warrant physical revision and republication in the future, we will incorporate the modifications made by this LOMR at that time.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Information eXchange toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMC Clearinghouse, 3601 Eisenhower Avenue, Suite 500, Alexandria, VA 22304-6426. Additional Information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

PUBLIC NOTIFICATION OF REVISION

A notice of changes will be published in the *Federal Register*. This information also will be published in your local newspaper on or about the dates listed below, and through FEMA's Flood Hazard Mapping website at https://www.floodmaps.fema.gov/fhm/bfe_status/bfe_main.asp

LOCAL NEWSPAPER	Name: *The Colorado Springs Gazette*
	Dates: December 27, 2019 and January 3, 2020

Within 90 days of the second publication in the local newspaper, any interested party may request that we reconsider this determination. Any request for reconsideration must be based on scientific or technical data. Therefore, this letter will be effective only after the 90-day appeal period has elapsed and we have resolved any appeals that we receive during this appeal period. Until this LOMR is effective, the revised flood hazard determination presented in this LOMR may be changed.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Information eXchange toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMC Clearinghouse, 3601 Eisenhower Avenue, Suite 500, Alexandria, VA 22304-6426. Additional information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

Federal Emergency Management Agency
Washington, D.C. 20472

December 18, 2019

CERTIFIED MAIL
RETURN RECEIPT REQUESTED

IN REPLY REFER TO:
Case No.: 19-08-0605P
Follows Conditional
Case No.: 17-08-1043R
Community Name: El Paso County, CO
Community No.: 080059
Effective Date of
This Revision: May 4, 2020

The Honorable Mark Waller
President, El Paso County
Board of Commissioners
200 South Cascade Avenue, Suite 100
Colorado Springs, CO 80903

Dear Mr. Waller:

The Flood Insurance Study (FIS) Report and Flood Insurance Rate Map (FIRM) for your community have been revised by this Letter of Map Revision (LOMR). Please use the enclosed annotated map panel(s) revised by this LOMR for floodplain management purposes and for all flood insurance policies and renewals issued in your community.

Additional documents are enclosed that provide information regarding this LOMR. Please see the List of Enclosures below to determine which documents are included. Other enclosures specific to this request may be included as referenced in the Determination Document. If you have any questions regarding floodplain management regulations for your community or the National Flood Insurance Program (NFIP) in general, please contact the Consultation Coordination Officer for your community. If you have any technical questions regarding this LOMR, please contact the Director, Mitigation Division of the Department of Homeland Security's Federal Emergency Management Agency (FEMA) in Denver, Colorado, at (303) 235-4830, or the FEMA Map Information eXchange (FMIX) toll free at 1-877-336-2627 (1-877-FEMA MAP). Additional information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Sincerely,

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

List of Enclosures:

Letter of Map Revision Determination Document
Annotated Flood Insurance Rate Map
Annotated Flood Insurance Study Report

cc: The Honorable John Suthers
Mayor, City of Colorado Springs

Mr. Keith Curtis, P.E., CFM
Floodplain Administrator
El Paso County and City of Colorado Springs

Mr. Stephen A. Brown, P.E.
Principal
Kiowa Engineering Corp.

Page 1 of 5	**Issue Date: December 18, 2019**	**Effective Date: May 4, 2020**	**Case No.: 19-08-0605P**	LOMR-APP

Follows Conditional Case No.: 17-08-1043R

Federal Emergency Management Agency
Washington, D.C. 20472

LETTER OF MAP REVISION
DETERMINATION DOCUMENT

COMMUNITY AND REVISION INFORMATION		PROJECT DESCRIPTION	BASIS OF REQUEST
COMMUNITY	**El Paso County** **Colorado** **(Unincorporated Areas)**	BRIDGE CHANNELIZATION EXCAVATION FILL	BASE MAP CHANGES FLOODWAY HYDRAULIC ANALYSIS UPDATED TOPOGRAPHIC DATA
	COMMUNITY NO.: 080059		
IDENTIFIER	Jimmy Camp Creek East Tributary	**APPROXIMATE LATITUDE & LONGITUDE:** 38.732, -104.636 **SOURCE:** USGS QUADRANGLE **DATUM:** NAD 83	

ANNOTATED MAPPING ENCLOSURES	ANNOTATED STUDY ENCLOSURES
TYPE: FIRM* NO.: 08041C0957G DATE: December 7, 2018 TYPE: FIRM* NO.: 08041C0976G DATE: December 7, 2018	DATE OF EFFECTIVE FLOOD INSURANCE STUDY: December 7, 2018 PROFILE(S): 210P, 211P, 212P, and 213P FLOODWAY DATA TABLE: 8

Enclosures reflect changes to flooding sources affected by this revision.
* FIRM - Flood Insurance Rate Map

FLOODING SOURCE AND REVISED REACH

Jimmy Camp Creek East Tributary - From approximately 2,760 feet downstream of Lorson Boulevard to approximately 4,260 feet upstream of Fontaine Boulevard

SUMMARY OF REVISIONS

Flooding Source	Effective Flooding	Revised Flooding	Increases	Decreases
Jimmy Camp Creek East Tributary	Zone AE	Zone AE	YES	YES
	Zone X (shaded)	Zone X (shaded)	YES	YES
	BFEs*	BFEs	YES	YES
	Floodway	Floodway	YES	YES

* BFEs - Base Flood Elevations

DETERMINATION

This document provides the determination from the Department of Homeland Security's Federal Emergency Management Agency (FEMA) regarding a request for a Letter of Map Revision (LOMR) for the area described above. Using the information submitted, we have determined that a revision to the flood hazards depicted in the Flood Insurance Study (FIS) report and/or National Flood Insurance Program (NFIP) map is warranted. This document revises the effective NFIP map, as indicated in the attached documentation. Please use the enclosed annotated map panels revised by this LOMR for floodplain management purposes and for all flood insurance policies and renewals in your community.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Information eXchange toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMC Clearinghouse, 3601 Eisenhower Avenue, Suite 500, Alexandria, VA 22304-6426. Additional Information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

Federal Emergency Management Agency
Washington, D.C. 20472

LETTER OF MAP REVISION
DETERMINATION DOCUMENT (CONTINUED)

OTHER COMMUNITIES AFFECTED BY THIS REVISION

CID Number: 080060 **Name:** City of Colorado Springs, Colorado

AFFECTED MAP PANELS	AFFECTED PORTIONS OF THE FLOOD INSURANCE STUDY REPORT
TYPE: FIRM* NO.: 08041C0976G DATE: December 7, 2018 TYPE: FIRM* NO.: 08041C0957G DATE: December 7, 2018	DATE OF EFFECTIVE FLOOD INSURANCE STUDY: December 7, 2018 PROFILE(S): 213P FLOODWAY DATA TABLE: 8

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Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

U.S. DEPARTMENT OF HOMELAND SECURITY

Federal Emergency Management Agency
Washington, D.C. 20472

LETTER OF MAP REVISION DETERMINATION DOCUMENT (CONTINUED)

COMMUNITY INFORMATION

APPLICABLE NFIP REGULATIONS/COMMUNITY OBLIGATION

We have made this determination pursuant to Section 206 of the Flood Disaster Protection Act of 1973 (P.L. 93-234) and in accordance with the National Flood Insurance Act of 1968, as amended (Title XIII of the Housing and Urban Development Act of 1968, P.L. 90-448), 42 U.S.C. 4001-4128, and 44 CFR Part 65. Pursuant to Section 1361 of the National Flood Insurance Act of 1968, as amended, communities participating in the NFIP are required to adopt and enforce floodplain management regulations that meet or exceed NFIP criteria. These criteria, including adoption of the FIS report and FIRM, and the modifications made by this LOMR, are the minimum requirements for continued NFIP participation and do not supersede more stringent State/Commonwealth or local requirements to which the regulations apply.

We provide the floodway designation to your community as a tool to regulate floodplain development. Therefore, the floodway revision we have described in this letter, while acceptable to us, must also be acceptable to your community and adopted by appropriate community action, as specified in Paragraph 60.3(d) of the NFIP regulations.

NFIP regulations Subparagraph 60.3(b)(7) requires communities to ensure that the flood-carrying capacity within the altered or relocated portion of any watercourse is maintained. This provision is incorporated into your community's existing floodplain management ordinances; therefore, responsibility for maintenance of the altered or relocated watercourse, including any related appurtenances such as bridges, culverts, and other drainage structures, rests with your community. We may request that your community submit a description and schedule of maintenance activities necessary to ensure this requirement.

COMMUNITY REMINDERS

We based this determination on the 1-percent-annual-chance flood discharges computed in the FIS for your community without considering subsequent changes in watershed characteristics that could increase flood discharges. Future development of projects upstream could cause increased flood discharges, which could cause increased flood hazards. A comprehensive restudy of your community's flood hazards would consider the cumulative effects of development on flood discharges subsequent to the publication of the FIS report for your community and could, therefore, establish greater flood hazards in this area.

Your community must regulate all proposed floodplain development and ensure that permits required by Federal and/or State/Commonwealth law have been obtained. State/Commonwealth or community officials, based on knowledge of local conditions and in the interest of safety, may set higher standards for construction or may limit development in floodplain areas. If your State/Commonwealth or community has adopted more restrictive or comprehensive floodplain management criteria, those criteria take precedence over the minimum NFIP requirements.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Information eXchange toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMC Clearinghouse, 3601 Eisenhower Avenue, Suite 500, Alexandria, VA 22304-6426. Additional Information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

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Engineering Services Branch
Federal Insurance and Mitigation Administration

LETTER OF MAP REVISION
DETERMINATION DOCUMENT (CONTINUED)

We will not print and distribute this LOMR to primary users, such as local insurance agents or mortgage lenders; instead, the community will serve as a repository for the new data. We encourage you to disseminate the information in this LOMR by preparing a news release for publication in your community's newspaper that describes the revision and explains how your community will provide the data and help interpret the NFIP maps. In that way, interested persons, such as property owners, insurance agents, and mortgage lenders, can benefit from the information.

This revision has met our criteria for removing an area from the 1-percent-annual-chance floodplain to reflect the placement of fill. However, we encourage you to require that the lowest adjacent grade and lowest floor (including basement) of any structure placed within the subject area be elevated to or above the Base (1-percent-annual-chance) Flood Elevation.

We have designated a Consultation Coordination Officer (CCO) to assist your community. The CCO will be the primary liaison between your community and FEMA. For information regarding your CCO, please contact:

Ms. Jeanine D. Petterson
Director, Mitigation Division
Federal Emergency Management Agency, Region VIII
Denver Federal Center, Building 710
P.O. Box 25267
Denver, CO 80225-0267
(303) 235-4830

STATUS OF THE COMMUNITY NFIP MAPS

We will not physically revise and republish the FIRM and FIS report for your community to reflect the modifications made by this LOMR at this time. When changes to the previously cited FIRM panel(s) and FIS report warrant physical revision and republication in the future, we will incorporate the modifications made by this LOMR at that time.

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Information eXchange toll free at 1-877-336-2627 (1-877-FEMA MAP) or by letter addressed to the LOMC Clearinghouse, 3601 Eisenhower Avenue, Suite 500, Alexandria, VA 22304-6426. Additional Information about the NFIP is available on our website at https://www.fema.gov/national-flood-insurance-program.

Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

Page 5 of 5	**Issue Date: December 18, 2019**	**Effective Date: May 4, 2020**	**Case No.: 19-08-0605P**	LOMR-APP

Federal Emergency Management Agency
Washington, D.C. 20472

LETTER OF MAP REVISION
DETERMINATION DOCUMENT (CONTINUED)

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Patrick "Rick" F. Sacbibit, P.E., Branch Chief
Engineering Services Branch
Federal Insurance and Mitigation Administration

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Jimmy Camp Creek - East Tributary								
A	1,672	240	1,535	3.6	5679.4	5678.2[2]	5679.0[2]	0.8
B	1,962	700	862	6.4	5684.7	5684.7	5685.7	1.0
C	4,362	340	2,188	2.5	5690.0	5690.0	5690.7	0.7
D	5,693	80	660	8.3	5692.9	5692.9	5693.4	0.5
E	7,983	111	672	10.1	5701.6	5701.6	5701.8	0.2
F	10,049	172	1090	4.8	5709.3	5709.3	5709.3	0.0
G	11,455	90	443	11.8	5711.8	5711.8	5711.8	0.0
H	13,024	158	513	10.5	5726.2	5726.2	5726.2	0.0
I	13,933	148	511	8.8	5733.0	5733.0	5733.0	0.0
J	17,377	220	518	8.5	5763.6	5763.6	5763.6	0.0
K	18,757	260	735	5.0	5775.4	5775.4	5775.9	0.5
L	20,057	670	647	5.6	5789.1	5789.1	5789.2	0.1
M	23,137	600	1,259	2.9	5816.4	5816.4	5816.4	0.0
N	24,867	450	596	6.1	5831.5	5831.5	5832.2	0.7
O	26,127	400	900	3.7	5847.0	5847.0	5848.0	1.0
P	28,097	195	346	7.8	5870.0	5870.0	5870.0	0.0
Q	30,037	190	550	4.9	5886.2	5886.2	5886.5	0.3
R	30,257	200	613	4.4	5888.1	5888.1	5888.1	0.0
S	30,299	200	580	4.7	5890.6	5890.6	5890.6	0.0
T	30,399	280	996	2.7	5890.6	5890.6	5890.6	0.0
U	31,759	230	435	5.5	5901.7	5901.7	5902.4	0.7
V	32,839	350	518	4.6	5913.8	5913.8	5914.0	0.2
W	33,919	150	318	7.6	5925.1	5925.1	5925.1	0.0
X	35,939	130	255	7.9	5957.7	5957.7	5957.7	0.0

REVISED DATA

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[1] Feet Above Confluence with Jimmy Camp Creek.

[2] Elevation Computed without Consideration of Backwater Effects from Jimmy Camp Creek.

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY

EL PASO COUNTY, CO AND INCORPORATED AREAS

FLOODWAY DATA

REVISED TO
REFLECT LOMR
EFFECTIVE: May 4, 2020

JIMMY CAMP CREEK - EAST TRIBUTARY

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Jimmy Camp Creek - East Tributary								
Y	38,039	70	221	9.0	5989.9	5989.9	5989.9	0.0
Z	40,529	110	187	7.2	6028.7	6028.7	6028.8	0.1
AA	41,739	198	240	5.6	6062.4	6062.4	6062.4	0.0
AB	42,659	100	178	7.6	6084.3	6084.3	6084.3	0.0

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[1] Feet Above Confluence with Jimmy Camp Creek.

TABLE 8

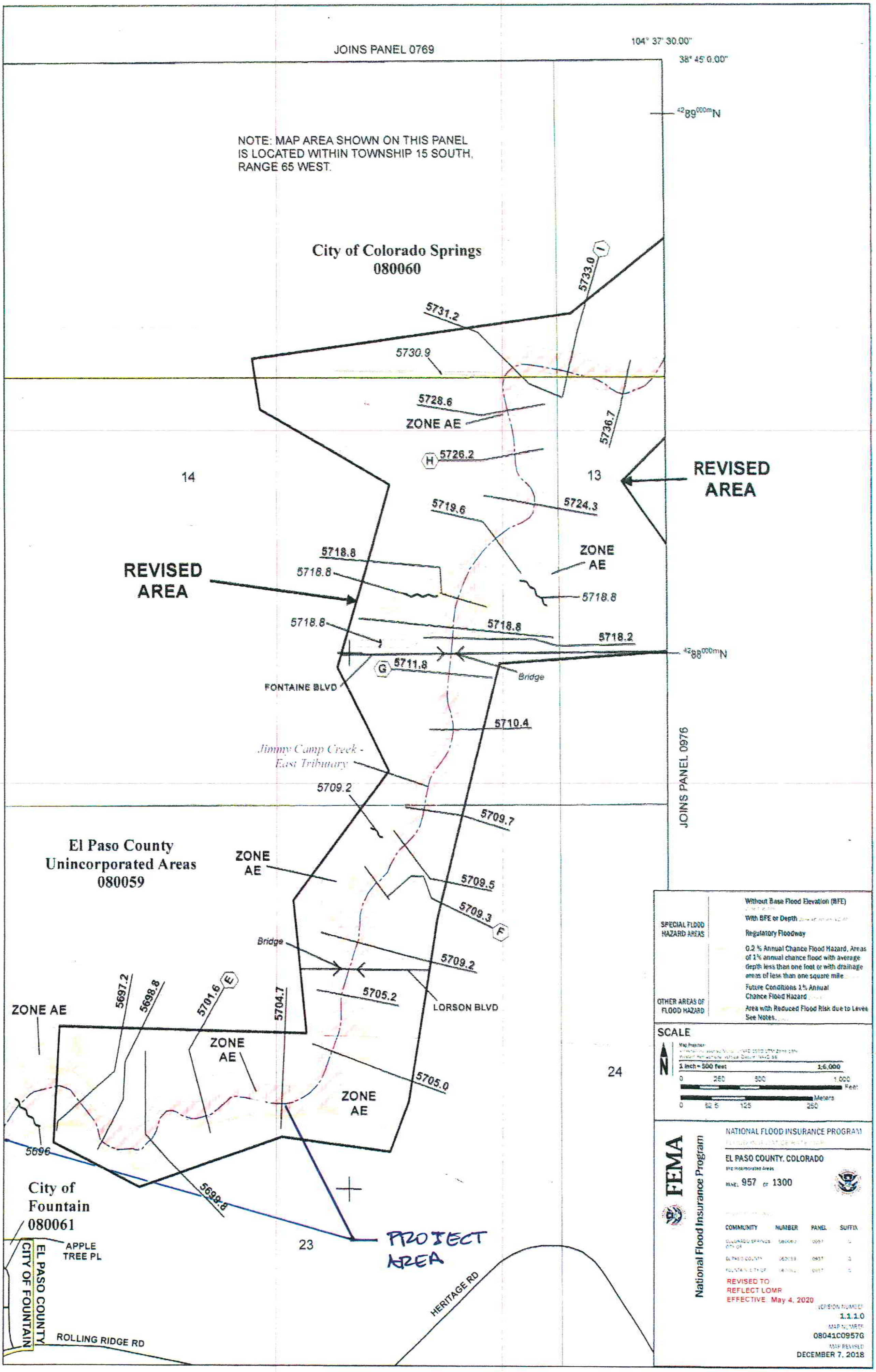
FEDERAL EMERGENCY MANAGEMENT AGENCY

EL PASO COUNTY, CO
AND INCORPORATED AREAS

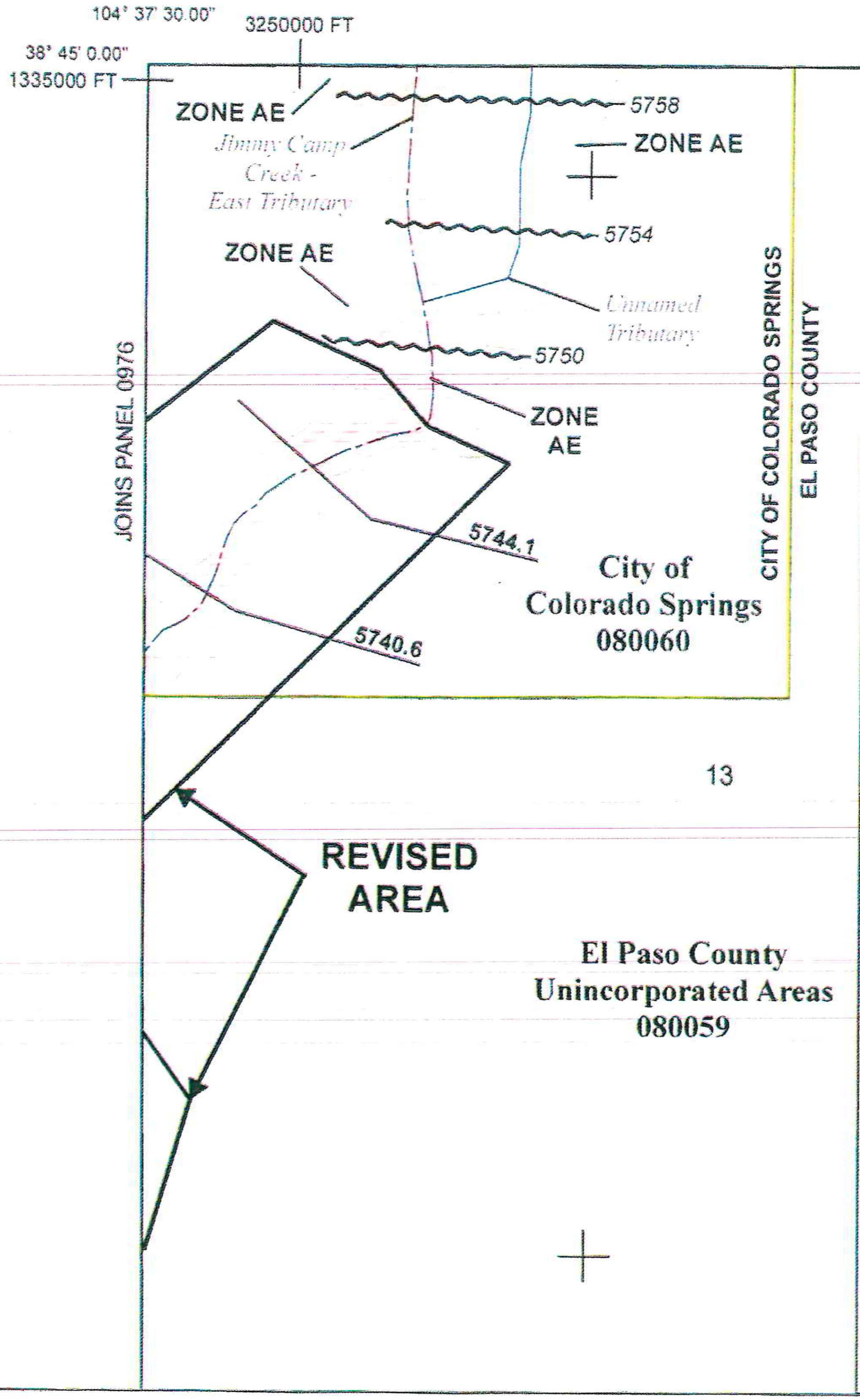
FLOODWAY DATA

REVISED TO
REFLECT LOMR
EFFECTIVE: May 4, 2020

JIMMY CAMP CREEK - EAST TRIBUTARY


JOINS PANEL 0769
104° 37' 30.00"
38° 45' 0.00"
4289000mN
NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 15 SOUTH, RANGE 65 WEST.
City of Colorado Springs
080060
I
5733.0
5731.2
5730.9
5728.6
ZONE AE
5736.7
H
5726.2
14
13
REVISED AREA
5724.3
5719.6
ZONE AE
5718.8
REVISED AREA
5718.8
5718.8
5718.8
5718.8
5718.2
4288000mN
G
5711.8
Bridge
FONTAINE BLVD
5710.4
Jimmy Camp Creek - East Tributary
5709.2
JOINS PANEL 0976
5709.7
El Paso County
Unincorporated Areas
080059
ZONE AE
5709.5
5709.3
F
Bridge
5709.2
E
5697.2
5698.8
5701.6
5704.7
5705.2
LORSON BLVD
ZONE AE
ZONE AE
24
5705.0
ZONE AE
5696
City of Fountain
080061
5699.8
APPLE TREE PL
23
PROJECT AREA
EL PASO COUNTY
CITY OF FOUNTAIN
HERITAGE RD
ROLLING RIDGE RD
SPECIAL FLOOD HAZARD AREAS
Without Base Flood Elevation (BFE)
With BFE or Depth
Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD
0.2 % Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile
Future Conditions 1% Annual Chance Flood Hazard
Area with Reduced Flood Risk due to Levee See Notes.
SCALE
N
Map Projection
1 inch = 500 feet
1:6,000
0
250
500
1,000
Feet
Meters
0
62.5
125
250
FEMA
National Flood Insurance Program
NATIONAL FLOOD INSURANCE PROGRAM
EL PASO COUNTY, COLORADO
and Incorporated Areas
PANEL 957 OF 1300
COMMUNITY
NUMBER
PANEL
SUFFIX
EL PASO COUNTY
0957
REVISED TO REFLECT LOMR EFFECTIVE May 4, 2020
VERSION NUMBER
1.1.1.0
MAP NUMBER
08041C0957G
MAP REVISED
DECEMBER 7, 2018

NOTE: MAP AREA SHOWN ON THIS PANEL
IS LOCATED WITHIN TOWNSHIP 15 SOUTH,
RANGE 64 WEST AND TOWNSHIP 15 SOUTH,
RANGE 65 WEST.



SPECIAL FLOOD HAZARD AREAS	Without Base Flood Elevation (BFE)
	With BFE or Depth
	Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD	0.2 % Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile
	Future Conditions 1% Annual Chance Flood Hazard
	Area with Reduced Flood Risk due to Levee See Notes.

SCALE

Map Projection:

1 inch = 500 feet 1:6,000

0 250 500 1,000 Feet

0 62.5 125 250 Meters

FEMA

National Flood Insurance Program

NATIONAL FLOOD INSURANCE PROGRAM

EL PASO COUNTY, COLORADO
and Incorporated Areas

PANEL 976 OF 1300

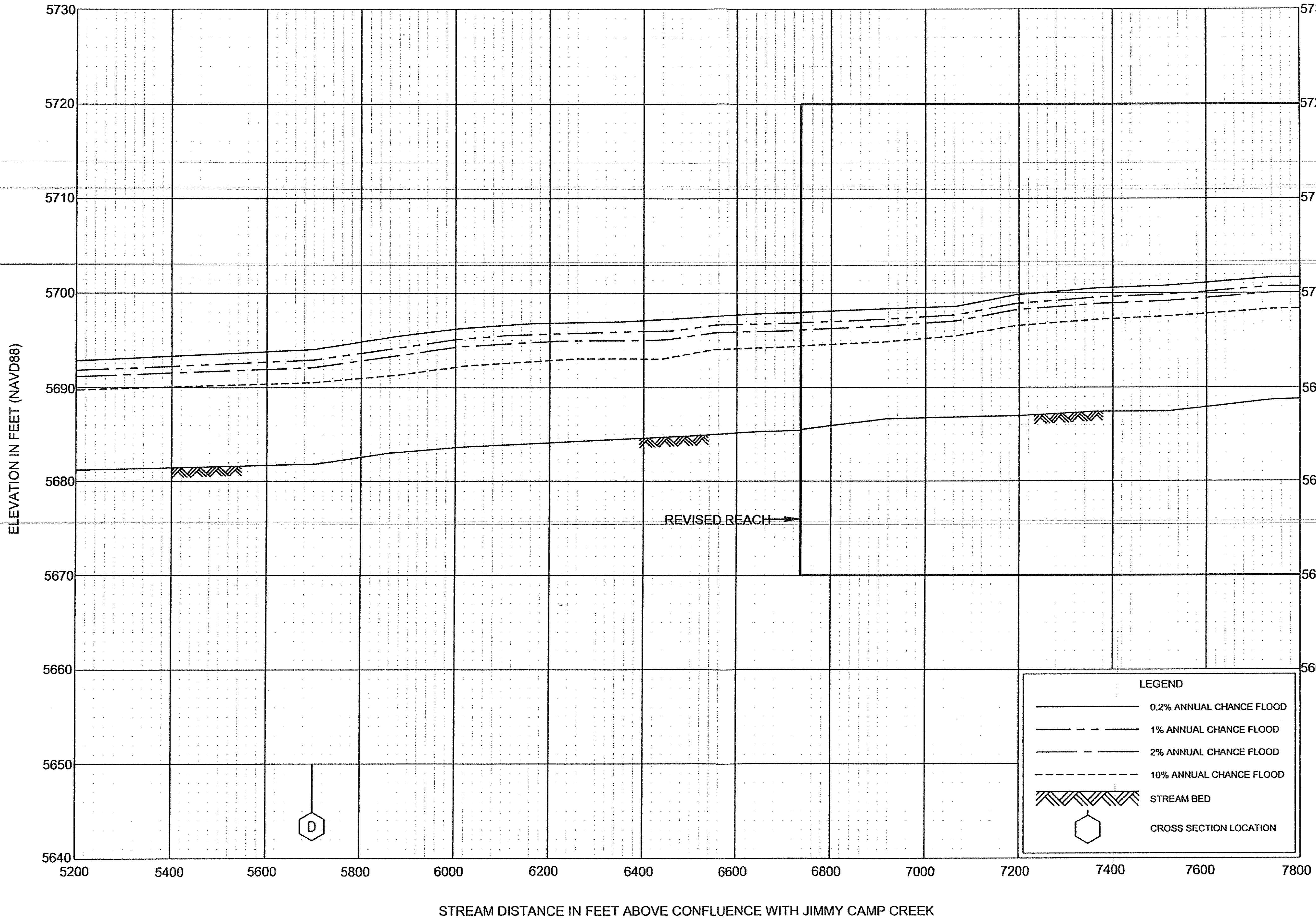
COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	080060	0976	G
EL PASO COUNTY	080059	0976	G

REVISED TO
REFLECT LOMR
EFFECTIVE: May 4, 2020

VERSION NUMBER
1.1.1.0

MAP NUMBER
08041C0976G

MAP REVISED
DECEMBER 7, 2018



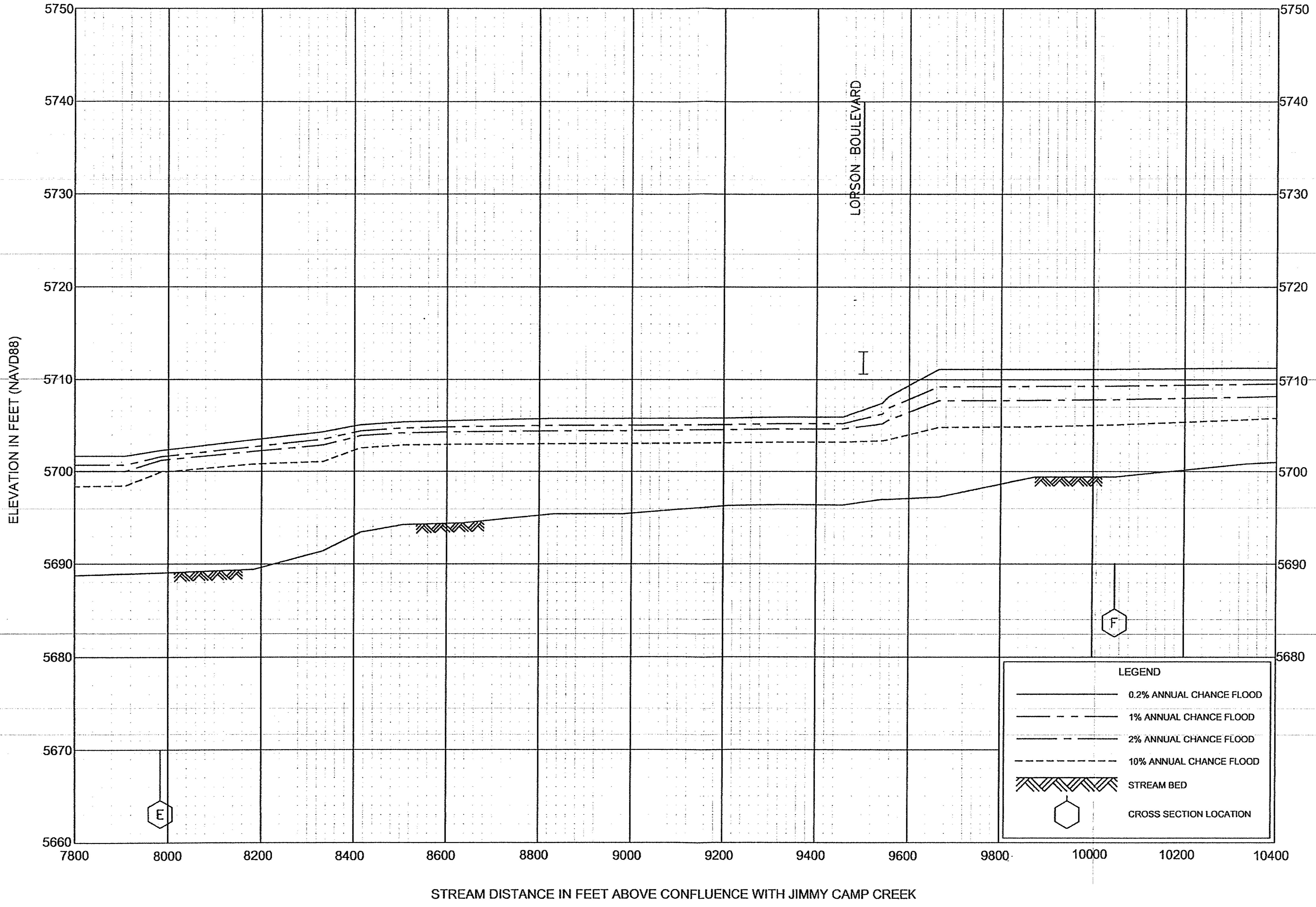
FLOOD PROFILES

REVISED TO
REFLECT LOMR
EFFECTIVE: May 4, 2020

JIMMY CAMP CREEK - EAST TRIBUTARY

FEDERAL EMERGENCY MANAGEMENT AGENCY

EL PASO COUNTY, CO
(AND INCORPORATED AREAS)



FLOOD PROFILES

JIMMY CAMP CREEK - EAST TRIBUTARY

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EL PASO COUNTY, CO
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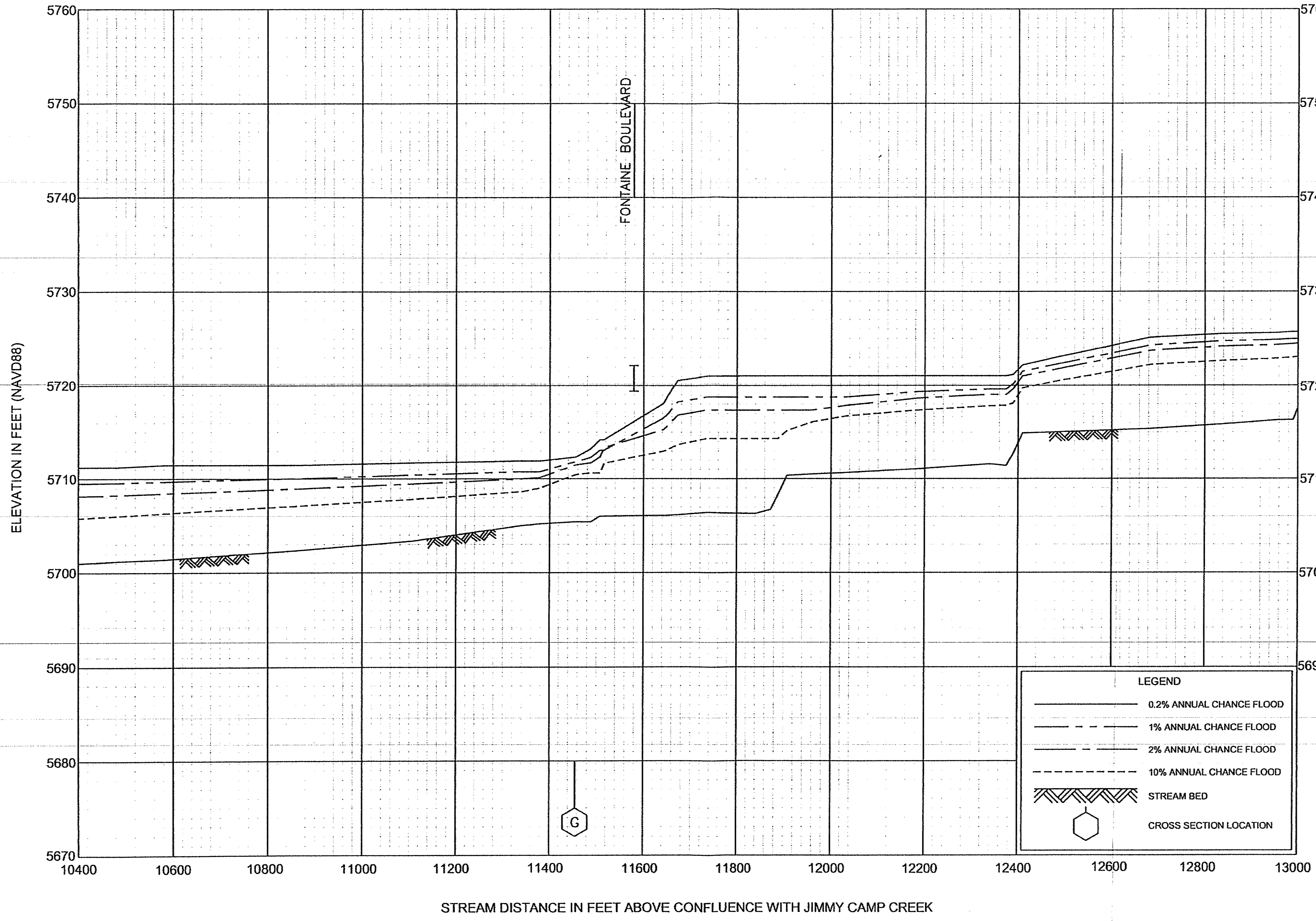

FLOOD PROFILES

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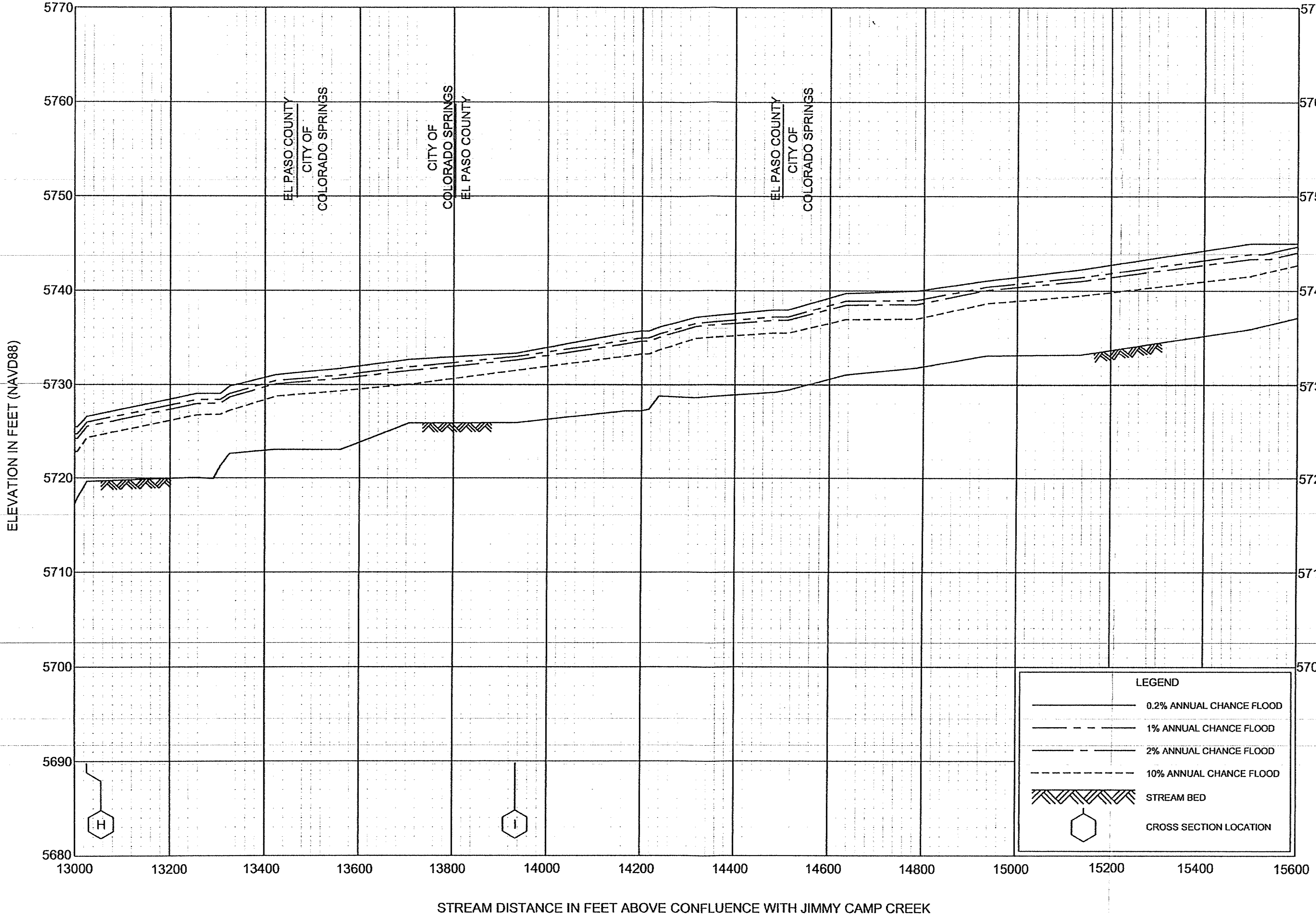
JIMMY CAMP CREEK - EAST TRIBUTARY

FEDERAL EMERGENCY MANAGEMENT AGENCY

EL PASO COUNTY, CO
(AND INCORPORATED AREAS)



FLOOD PROFILES

JIMMY CAMP CREEK - EAST TRIBUTARY

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FEDERAL EMERGENCY MANAGEMENT AGENCY

EL PASO COUNTY, CO
(AND INCORPORATED AREAS)

17-Mar
SAB

	Proposed Revised			Effective	PP Rev-Eff
River Sta	100yr W.S. Elev (ft)	550 cfs	110 cfs	100yr W.S. Elev (ft)	100yr Difference (ft)
9000	5699.47	5694.15	5692.96	5702.15	-2.7
8850	5699.43	5693.85	5692.70	5702.21	-2.8
8650	5698.74	5693.14	5692.08	5702.03	-3.3
8521.53	5698.59	5691.88	5690.85	5701.88	-3.3
8470	5698.58	5691.92	5689.72	5701.62	-3.0
8430	5698.27	5691.64	5689.42	5701.29	-3.0
8350	5697.69	5690.88	5688.82	5699.93	-2.2
8200	5697.56	5690.74	5688.79	5699.08	-1.5
8000	5696.73	5690.28	5688.62	5698.13	-1.4
7924	5696.13	5689.71	5688.33	5697.13	-1.0
7750	5695.67	5688.95	5686.85	5696.44	-0.8
7525	5695.29	5688.51	5686.40	5695.39	-0.1
7375	5694.81	5688.29	5686.23	5695.42	-0.6
7200	5693.63	5687.91	5685.94	5695.02	-1.4
7075	5693.09	5687.42	5685.25	5693.97	-0.9
6925	5692.84	5686.86	5684.64	5693.69	-0.8
6746	5692.44	5686.46	5684.31	5693.47	-1.0
6561	5691.95	5685.98	5683.86	5693.18	-1.2
6448	5691.70	5685.55	5683.38	5692.57	-0.9
6259	5690.49	5684.85	5682.90	5692.37	-1.9
6150	5690.22	5684.61	5682.69	5692.25	-2.0
6000	5689.88	5684.33	5682.55	5691.69	-1.8
5865	5689.19	5683.87	5682.32	5690.77	-1.6
5710	5688.21	5683.30	5681.66	5688.58	-0.4
5436	5687.49	5682.43	5680.89	5687.83	-0.3
5236	5686.65	5681.85	5680.34	5687.18	-0.5
5032	5684.84	5681.29	5679.62	5684.85	0.0
4836	5684.65	5681.16	5679.16	5684.75	-0.1
4500	5683.70	5680.62	5678.16	5683.70	0.0

Appendix E
Correspondence with Colorado Parks and Wildlife
Department of Natural Resources

COLORADO
Parks and Wildlife
Department of Natural Resources

Area 14
4255 Sinton Road
Colorado Springs, CO 80907
P 719.227.5200 | F 719.227.5297

September 17, 2018

Thomas and Thomas Planning Group
ATTN: Jason Alwine
702 N. Tejon Street
Colorado Springs, CO

Re: Creekside at Lorson Ranch PUDSP Plan/ PUDSP- Combined PUD/Preliminary Plan

Dear Mr. Alwine:

Thank you for the opportunity to comment on the Creekside at Lorson Ranch PUDSP Plan. Colorado Parks and Wildlife (CPW) has reviewed the project materials and visited the site. CPW has commented on previous phases of this development, and offers the following comments on this phase.

The vegetation is comprised of short grass prairie species. This habitat type will sustain numerous wildlife species including antelope, deer, coyote, fox, raptors, songbirds and numerous small mammals.

Construction even near riparian habitats can have downstream effects, such as increased sedimentation and erosion. If bank stabilization is not completely necessary in an area, we recommend leaving it in its natural state. Disturbance to soil can lead to introduction of invasive plant species which, among other things, can reduce the amount of quality forage for wildlife and cattle as well as possibly create an increased fire hazard. CPW recommends the development and implementation of a noxious weed control plan for the site. CPW recommends that in places where vegetation is removed, a native seed blend is used that matches the surrounding vegetation types as accurately as possible. All disturbed soils should be monitored for noxious weeds and noxious weeds should be actively controlled until native plant re-vegetation and reclamation is achieved. All landscaping in the developed area should be comprised of native species, and CPW recommends against using non-native plants or noxious weeds. Some care should be taken with species choice to prevent the attraction of unwanted wildlife into the development area. Information on plant species consumption by specific wildlife species is available through CPW.

By using native species with high food and cover values in an open space area large enough to maintain a viable movement corridor, and native plants with little food and cover value in the developed area, wildlife will be concentrated in areas that minimize conflict and optimize wildlife watching opportunities. Native species provide an aesthetically pleasing landscape that requires little maintenance and are frequently more drought-tolerant than non-native species.

CPW has identified current and past raptor nesting in the area. CPW recommends the use of preconstruction surveys, as well as continuation of those surveys during construction, to identify raptor nests within the project area and implement appropriate restrictions. CPW recommends adherence to the recommended buffer distances and timing stipulations identified in the attached document "Recommended Buffer Zones and Seasonal Restrictions for Colorado Raptors". Removal or relocation of any active raptor nests will require consultation with CPW and US Fish and Wildlife Service prior to moving. Both active and potential nest sites, winter night roosts should be considered when evaluating disturbance during construction.

Jimmy Camp Creek contains a population of Arkansas darters, a state threatened and federal candidate species. The Jimmy Camp Creek population of Arkansas darters is an important population in the Arkansas Basin. Arkansas darters are a high priority Tier 1 species in the CPW State Wildlife Action Plan. One of the conservation actions of CPW is securing habitat quality for existing populations. Although no Arkansas Darters were located during a stream survey conducted in 2005, the East Tributary of Jimmy Camp Creek (ETJC) does provide potential darter habitat.

In 2006, then, Colorado Division of Wildlife (CDOW) wrote a comment letter advising against straightening the ETJC. Reduction in sinuosity (the way a stream channel bends) can cause negative impacts to the riparian wildlife habitat associated with this stream. As streams are straightened, the slope of the channel tends to steepen, thus increasing water flows and sedimentation. Riparian areas and flood plains slow flood waters, provide habitat for wildlife, and decreases potential damage to any structures that end up being built near the creek channel. A stream with higher sinuosity allows for willows and other plants to establish along the banks and create a complex root system, thus strengthening the integrity of the stream channel. Although some sinuosity was left, the channel has undergone a drastic change and is for the most part straight; the channel is perfectly "U" shaped which further increases water velocity during high flow/flood events. ETJC also no longer has a riparian/flood plain as it goes through the development. Since 2006, several hundred acres of short grass prairie have been developed creating a large amount of impervious surface. The proposed addition will add an additional approximate 83.08 acres of impervious surface. This increase in impervious surface combined with the new straightened and channelized nature of the creek will increase erosion, siltation and water velocity during heavy rain events which could have a negative impact on the surrounding environment as well as manmade structures. Jimmy Camp Creek's hydrograph already has a flow pattern dominated by flood pulse events that is sharply amplified by the already constructed developments both up stream and down from the development's future location. CPW is concerned about the possible addition to the amplitude of flows that could result from the impacts listed above.

Conflicts may arise between homeowners and wildlife. The following is a list of general recommendations that CPW would also like to be taken into consideration in order to avoid nuisance conflicts with wildlife. Coyotes, foxes, cottontail rabbits, and raccoons are several species that have adapted well to living within city limits. Open space, as well as developed areas, may become suitable habitat for many wildlife species. Coyote sightings are common within the city and few interactions are negative for the coyote. While coyotes will not usually approach people, in places where they see us often, they become less fearful. Coyotes feed near homes, yards, trails, and roads in order to survive in urban areas. Homeowners can do their part by *not* inviting wildlife into their yard. Many times these conditions can be enforced **through the local Homeowner's Association or through covenants.**

1. Pets should not be allowed to roam free and fences should be installed to decrease or eliminate this problem. Dogs and cats chase or prey on various wildlife species. One benefit to keeping animals under control is that they are less likely to bother other people, be in roadways or become prey for coyotes, foxes or owls.

2. Trash should be kept indoors until the morning of trash pickup. CPW recommends using bear resistant trash containers. Skunks, raccoons, bears, and neighborhood dogs are attracted to garbage and do become habituated.

3. Feeding of all wildlife should be prohibited, with the exception of songbirds. The use of bird feeders, suet feeders, and hummingbird feeders are discouraged. However, if feeders are used, they should be placed so they are inaccessible to raccoons or skunks and other wildlife species that might cause damage or threaten human safety. It is illegal to feed big game including deer, elk, antelope, moose, bear and lion as well as coyote and fox.

4. Pets should be fed inside or if pets are fed outside, feeding should occur only for a specified period of time and food bowls returned afterwards to a secure site for storage. Pet food left outside attracts various wildlife species which in turn attracts predators.

5. When landscaping lots, it is strongly recommended that native vegetation be used that wildlife is less likely to be attracted to. Planting of trees and shrubs that are attractive to native ungulates should incorporate the use of materials that will prevent access and damage (fencing, tree guards, trunk guards, etc.).

6. Fences, other than those around the immediate domicile and serving to protect landscaped trees and shrubs, should be designed so as not to impair wildlife movements. Ornamental fences with sharp vertical points or projections extending beyond the top rail should be strongly discouraged. Wildlife friendly design recommendations can be provided upon request.

CPW has further resources available to developers and residents on our website at **CPW's homepage**.

CPW believes that the development as proposed will lead to increased nuisance wildlife conflicts as well as erosion concerns on the East Tributary of Jimmy Camp Creek similar to those seen in many other Colorado Springs streams. The proximity of human development on both sides of the ETJC as well as the main channel limits the effectiveness of these streams as wildlife corridors. To preserve the ETJC as outlined in the 2003 Highway 94 Comprehensive plan CPW recommends increasing the size of the open space surrounding the creek.

We appreciate being given the opportunity to comment. Please feel free to contact District Wildlife Manager Philip Gurule, should you have any questions or require additional information at 719-227-5283 or via email at Philip.gurule@state.co.us.

Sincerely,

Frank McGee
Area Wildlife Manager

Cc: Philip Gurule DWM
SE Regional File
Area 14 File

Rich Wray

rom: Rich Wray
Sent: Monday, March 11, 2019 11:11 AM
To: Philip Gurule - DNR
Subject: Creekside/east fork jimmy camp creek
Attachments: 18020 rev efjcc cross-sections.pdf

Philip: I am following up on our recent channel design drawings submitted to your office last January. Having not heard from your office regarding the latest channel sections we are proceeding with our submittal to the County Planning office using the attached low flow detail. If you can provide any further comments it would be appreciated.

Thanks for your help on this.

Rich Wray

Richard Wray, PE
Principal
Celebrating 50 years
Kiowa
Engineering Corporation
1604 South 21st Street
Colorado Springs, Colorado 80904-4208
'hone: (719) 630-7342
Email: rwray@kiowaengineering.com

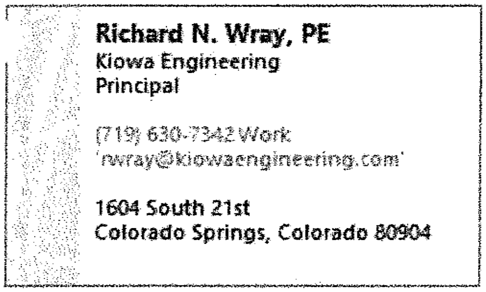
Rich Wray

From: Rich Wray
Sent: Friday, January 11, 2019 12:49 PM
To: Philip Gurule - DNR
Cc: 'Jason Alwine'
Subject: creek side at lorson ranch
Attachments: 18020 rev efjcc cross-sections.pdf

Phillip: sorry it has taken so long to get back to you. Regarding your email dated November 28th, I have revised the typical peal sections transmitted previously for your review. The new low flow section accommodates a 2-foot deep bankfull channel created out of boulders and a 2-foot deep overbank channel. Combined the bankfull channel and overbank channel can carry the required low flow capacity of 560 cfs per county criteria. The bankfull flow of 110 cfs (2yr frequency +/-), was derived by Kiowa when the Jimmy Camp Creek drainage basin planning study was completed in 2014. The bankfull channel as shown carries 113 cfs. This two stage approach is I believe what you were explaining in your email. Velocities are around 6 feet per second well within the erosive tolerance of the proposed vegetated bench.

Let me know your thoughts as if this appears to meet the goals of DNR than I will take this concept to the County and begin the design review process.

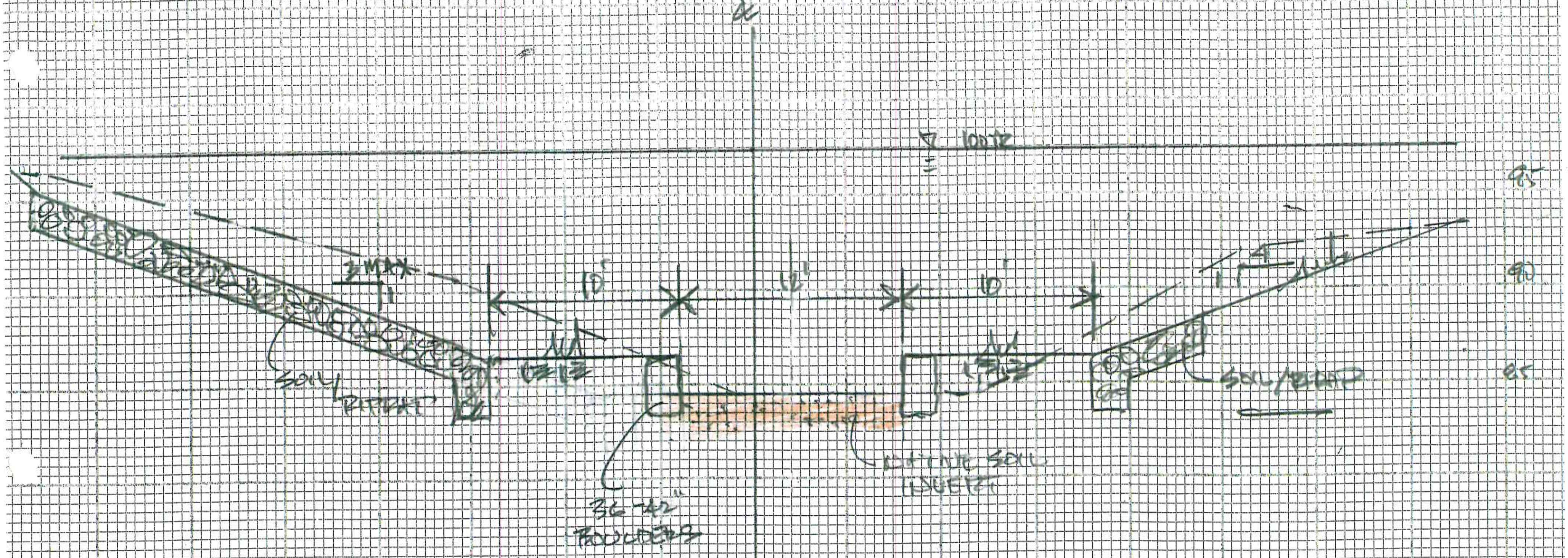
Rich



Richard N. Wray, PE
Kiowa Engineering
Principal

(719) 630-7342 Work
'rwray@kiowaengineering.com'

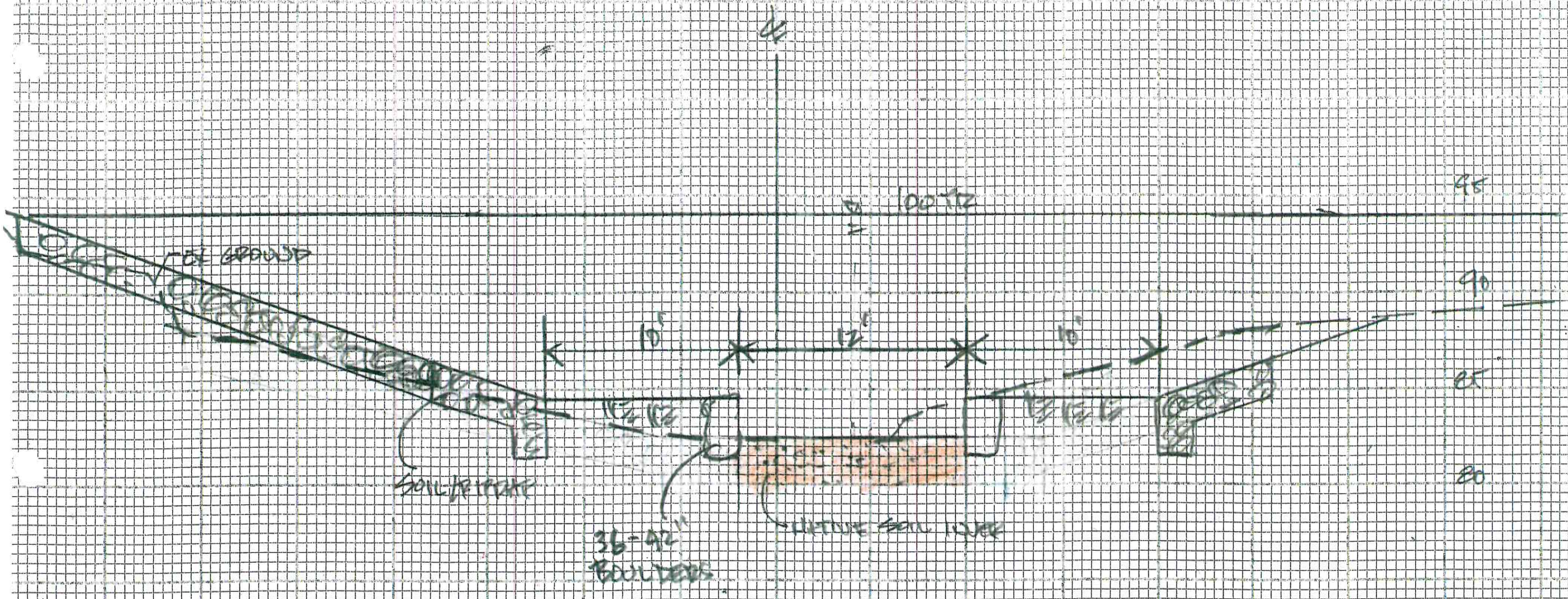
1604 South 21st
Colorado Springs, Colorado 80904



XSEC 41+50

1" = 5' H+V

Rev. 1/10/19



X SEC 33+00

1" = 5' H+V

RCW 1/10/19

Rich Wray

From: Jason Alwine <jalwine@ttplan.net>
Sent: Thursday, December 6, 2018 12:19 PM
To: Rich Wray; Liz Klein
Subject: FW: Creekside at Lorson Ranch
Attachments: image001.jpg

Rich,

Did you response to Philip about his question? Seems like this is getting deeper than it needs to be but then again what do I know 😉

Jason

From: Gurule - DNR, Philip <philip.gurule@state.co.us>
Sent: Wednesday, November 28, 2018 11:14 AM
To: Jason Alwine <jalwine@ttplan.net>; eklein@kiowaengineering.com; Rich Wray <rwray@kiowaengineering.com>
Cc: Paul Foutz - DNR <paul.foutz@state.co.us>; Cory Noble - DNR <cory.noble@state.co.us>
Subject: Re: Creekside at Lorson Ranch

Good afternoon everyone,

Thank you so much for getting those cross sections sent over! We really appreciate the willingness to work with us. Very seldom do we find folks who will it down with us and discuss the project more in depth. As we looked at the cross section, we saw some areas where we feel that enhancements could be made. Such as, adding a two stage channel design that would have a stabilized lower stage channel which can hold and carry a bankfull flow and the incorporation of native woody vegetation. This would be beneficial to the stabilization of the creek as well as enhance the area for wildlife. I will be typing up a formal letter for the addition of these elements. If you have any questions in the meantime, don't hesitate to reach out to me! Thanks!

On Mon, Nov 26, 2018 at 1:29 PM Jason Alwine <jalwine@ttplan.net> wrote:

Philip,

Attached are some cross sections that indicate the minimal improvements to the existing channel for the Creeside at LR project. Please let us know of any questions, thank you.

Jason

Jason Alwine, PLA

Rich Wray

om: Rich Wray
ent: Monday, November 26, 2018 12:50 PM
To: 'Jason Alwine'
Subject: efjcc cross-sections
Attachments: 18020 efjcc cross-sections.pdf

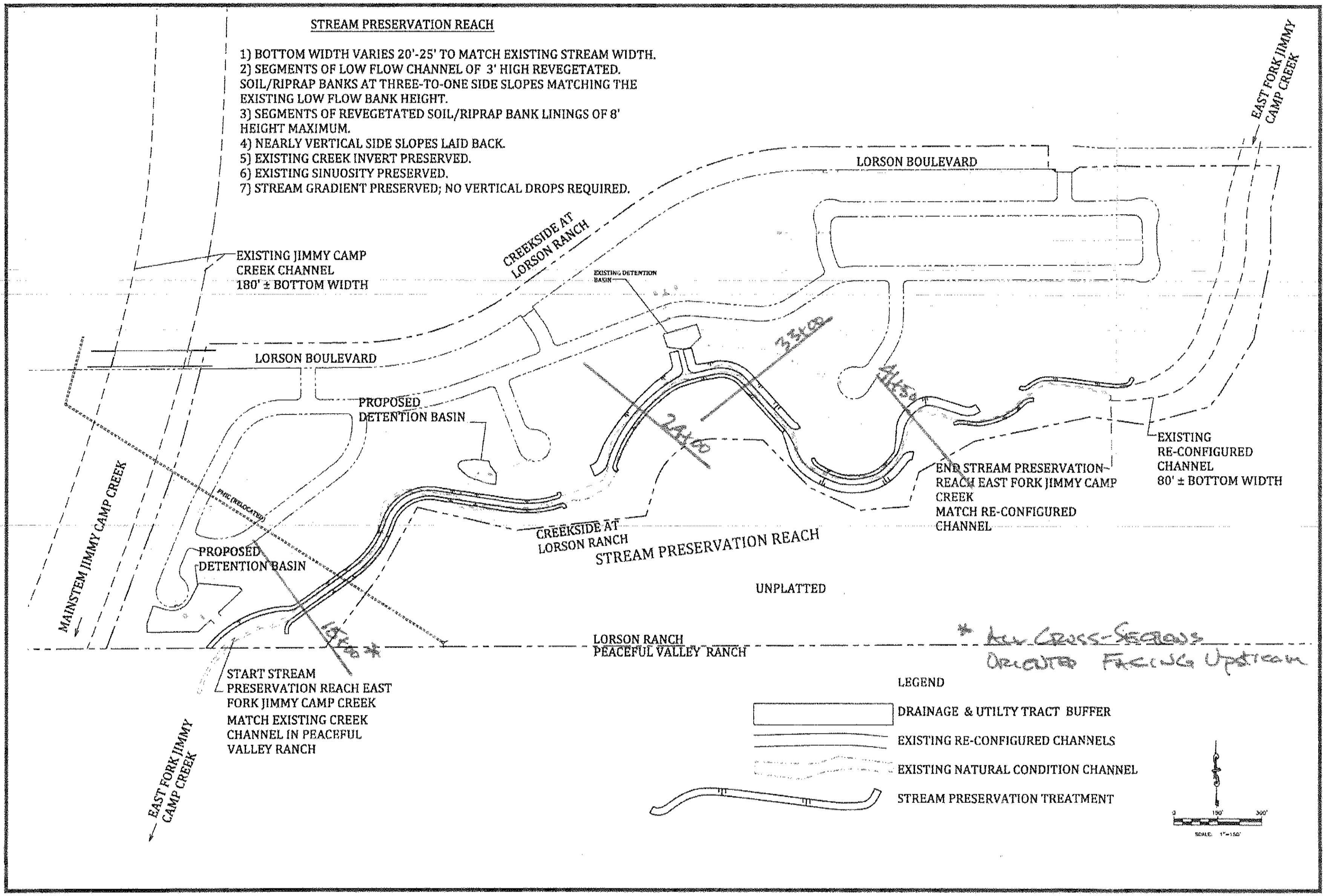
Jason: attached are cross-sections per our meeting with USFW.

Rich

Richard N. Wray, PE
Kiowa Engieering
Principal

(719) 630-7342 Work
'rwray@kiowaengineering.com'

1604 South 21st
Colorado Springs, Colorado 80904



STREAM PRESERVATION REACH

1) BOTTOM WIDTH VARIES 20'-25' TO MATCH EXISTING STREAM WIDTH.
2) SEGMENTS OF LOW FLOW CHANNEL OF 3' HIGH REVEGETATED. SOIL/RIPRAP BANKS AT THREE-TO-ONE SIDE SLOPES MATCHING THE EXISTING LOW FLOW BANK HEIGHT.
3) SEGMENTS OF REVEGETATED SOIL/RIPRAP BANK LININGS OF 8' HEIGHT MAXIMUM.
4) NEARLY VERTICAL SIDE SLOPES LAID BACK.
5) EXISTING CREEK INVERT PRESERVED.
6) EXISTING SINUOSITY PRESERVED.
7) STREAM GRADIENT PRESERVED; NO VERTICAL DROPS REQUIRED.

* ALL CROSS-SECTIONS ORIENTED FACING UPSTREAM

LEGEND

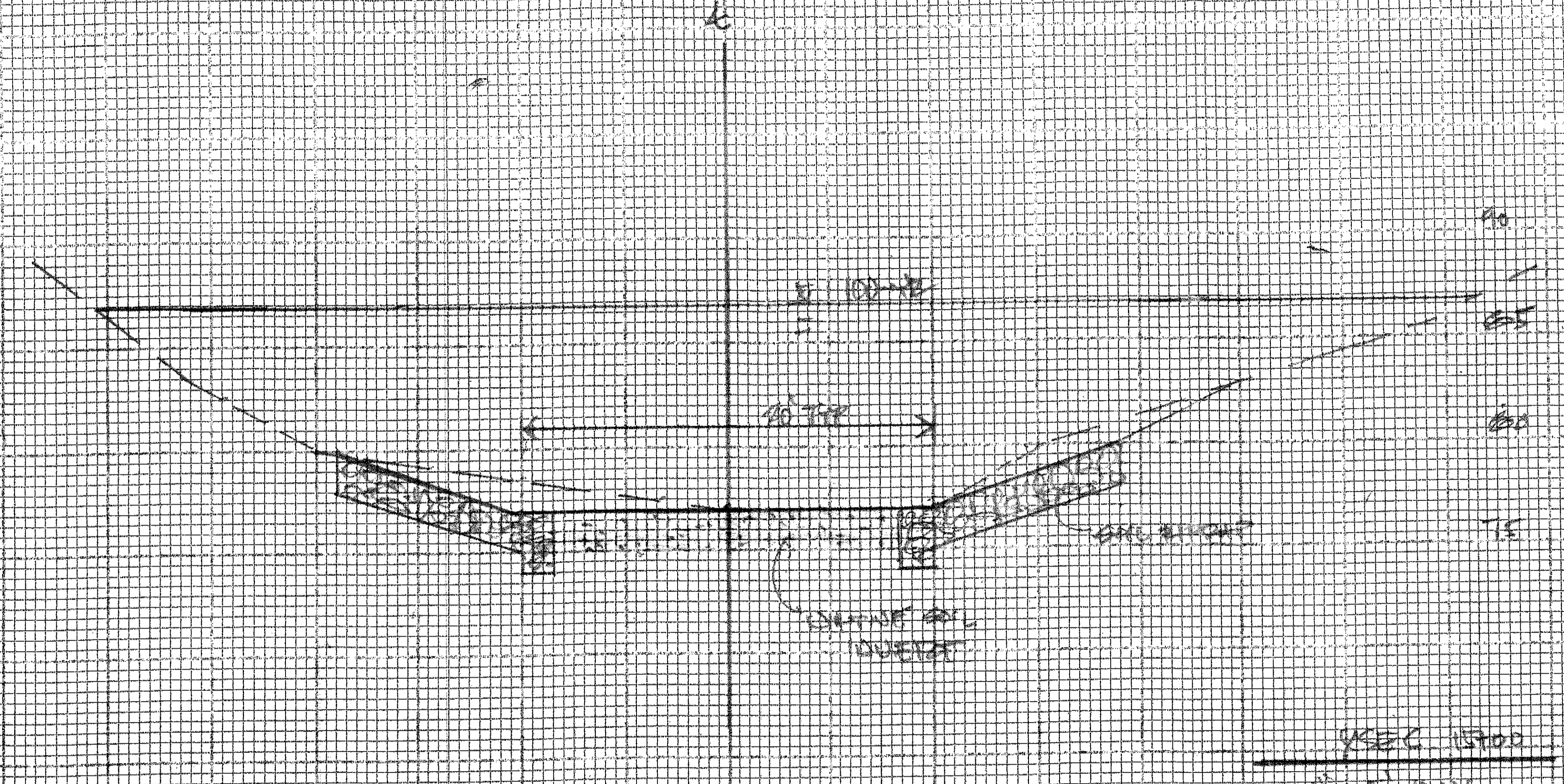
▭	DRAINAGE & UTILTY TRACT BUFFER
═	EXISTING RE-CONFIGURED CHANNELS
┈	EXISTING NATURAL CONDITION CHANNEL
═	STREAM PRESERVATION TREATMENT

Kiowa
Engineering Corporation
1604 South 21st Street
Colorado Springs, Colorado 80904
(719) 630-7342

LORSON RANCH
CREEKSIDE DEVELOPMENT
EAST FORK JIMMY CAMP CREEK
CONCEPTUAL DESIGN of LOWER LORSON EAST FORK JIMMY CAMP CREEK
EL PASO COUNTY, COLORADO

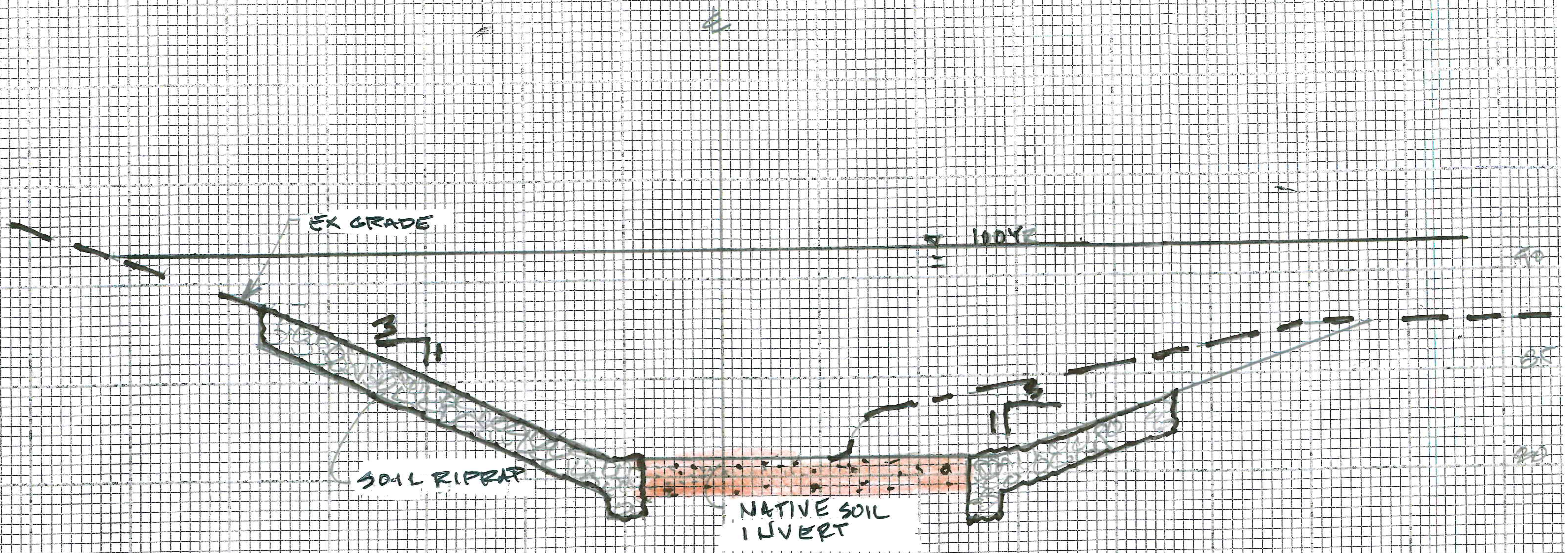
Project No.: 16020
Date: 10/24/18
Design:
Drawn:
Check:
Revisions:

Exh.1



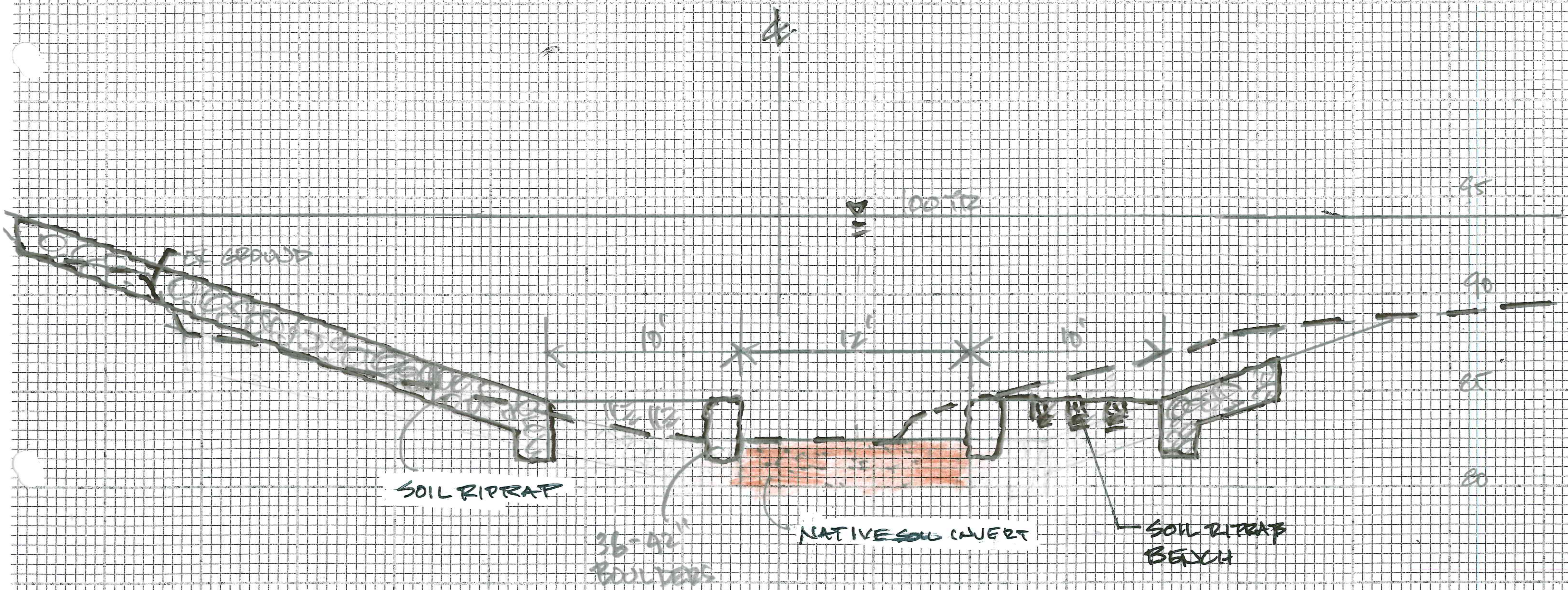
XSEC 15+00

1" = 5' H+V



XSEC 28+00

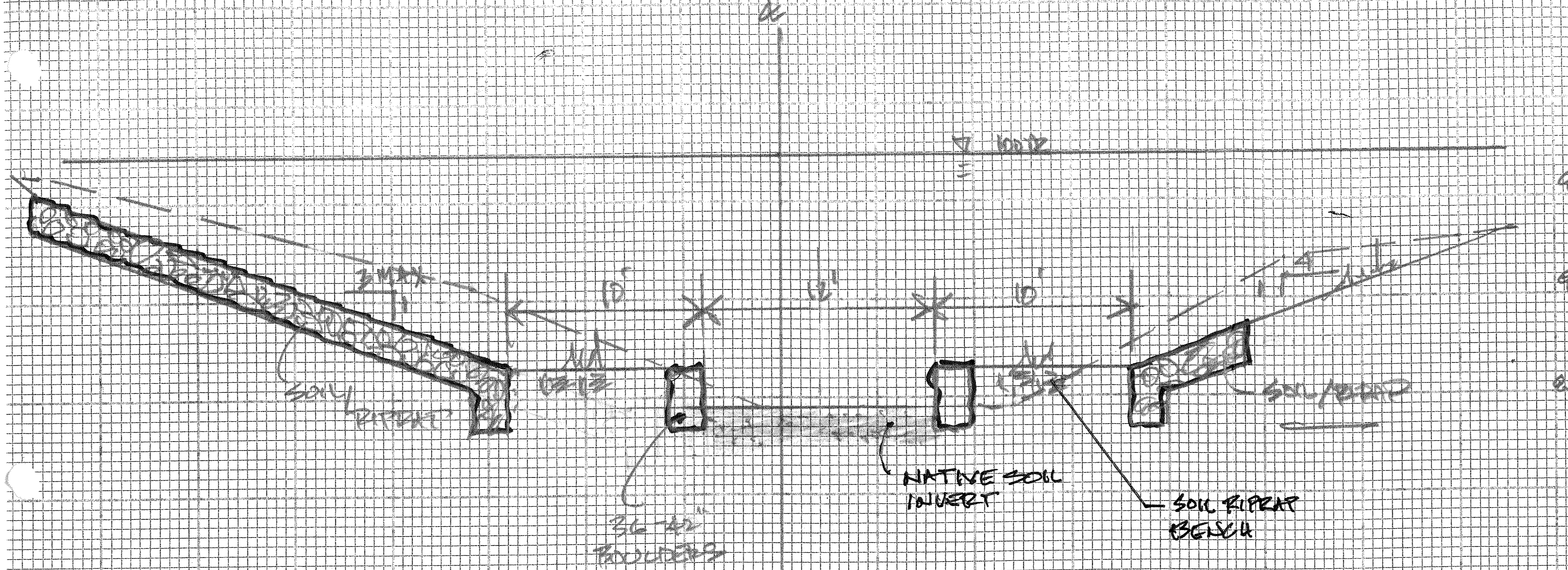
1"=5' H&V



XSEC 33+00

1"=5' H+V

Rev 1/10/19



XSEC 41+50

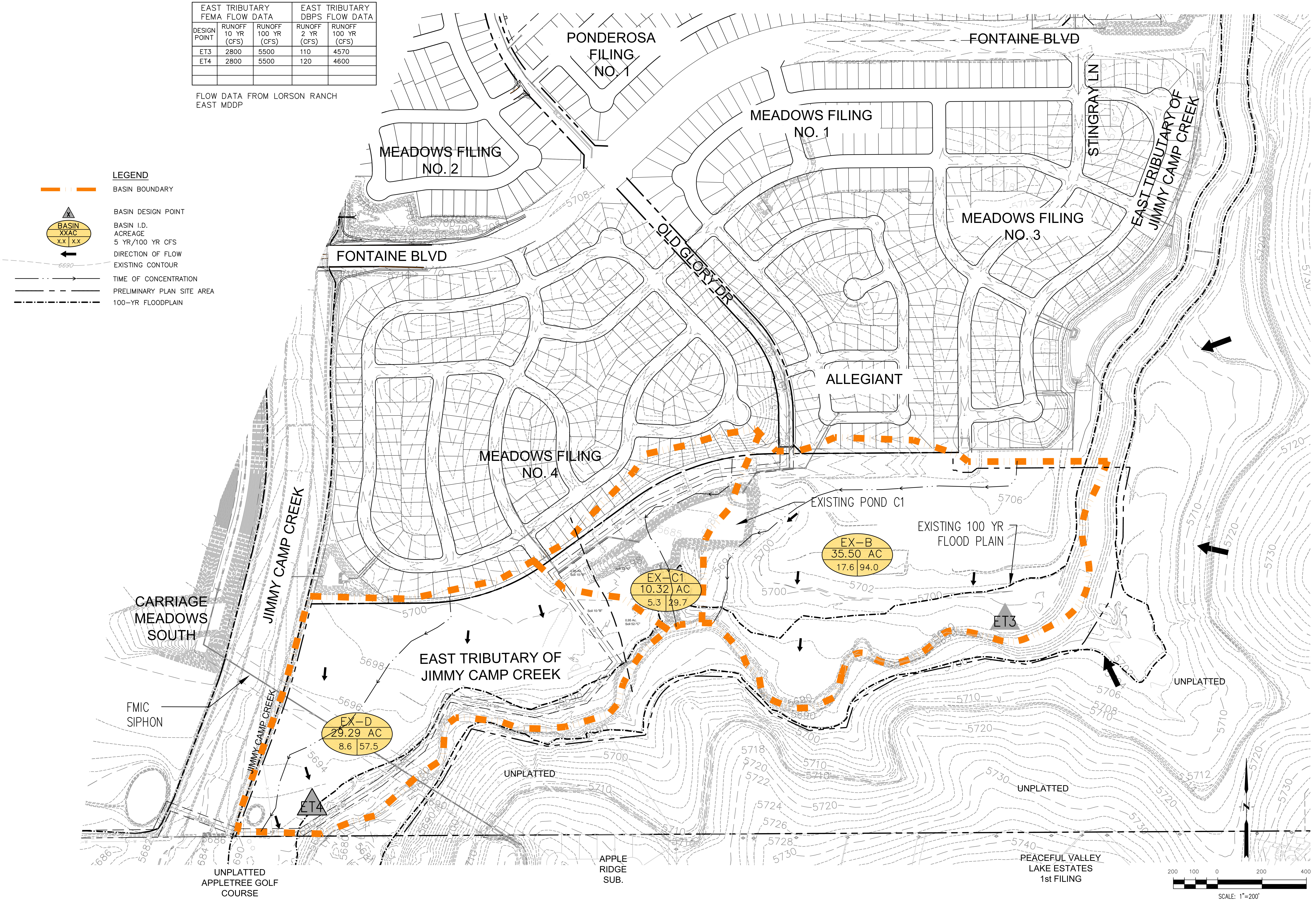
1"=5' H+V

Rev. 1/10/19

MAP POCKET

EAST TRIBUTARY FEMA FLOW DATA			EAST TRIBUTARY DBPS FLOW DATA	
DESIGN POINT	RUNOFF 10 YR (CFS)	RUNOFF 100 YR (CFS)	RUNOFF 2 YR (CFS)	RUNOFF 100 YR (CFS)
ET3	2800	5500	110	4570
ET4	2800	5500	120	4600

FLOW DATA FROM LORSON RANCH EAST MDDP



CORE
ENGINEERING GROUP
15004 1ST AVENUE S.
BURNSVILLE, MN 55306
PH: 719.570.1100
CONTACT: RICHARD L. SCHINDLER, P.E.
EMAIL: Rich@ceg1.com

NO.	DESCRIPTION	DATE

PREPARED FOR:
LORSON LLC
212 NORTH WAHSATCH AVE, SUITE 301
COLORADO SPRINGS, COLORADO 80903 (719) 635-3200
CONTACT: JEFF MARK

PROJECT:
CREEKSIDE FILING NO. 1
LORSON BLVD – OLD GLORY DRIVE
EL PASO COUNTY, COLORADO

DRAWN: LAB
DESIGNED: LAB
CHECKED: RLS

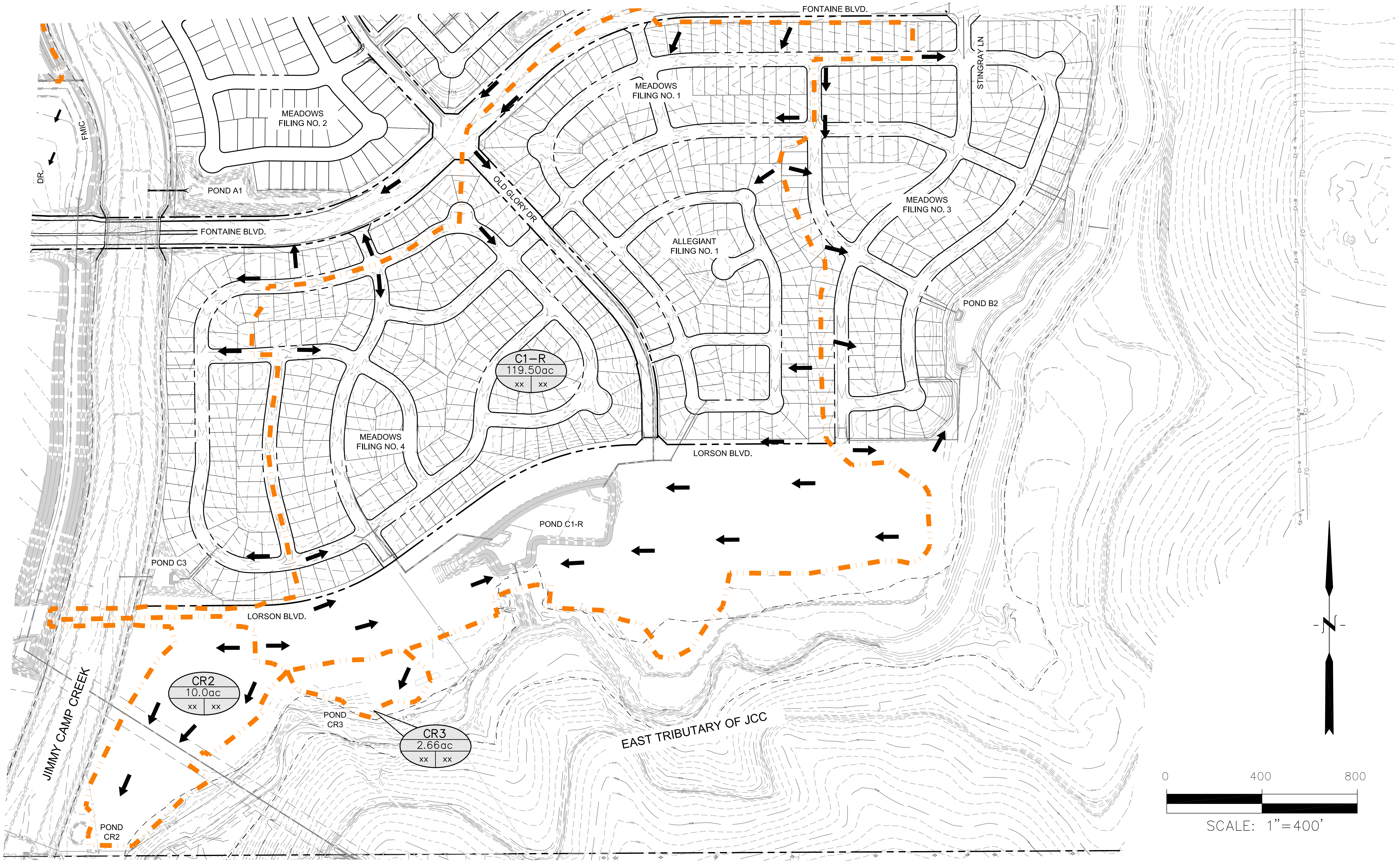
EXISTING CONDITIONS
CREEKSIDE AT LORSON RANCH FIL. NO. 1

DATE:
SEPT 19, 2019

PROJECT NO.
100.045

SHEET NUMBER
1

TOTAL SHEETS: 1



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EMAIL: Rich@ceg1.com

CREEKSIDE AT LORSON RANCH FILING NO. 1
WATER QUALITY & POND TRIBUTARY AREAS

SCALE:	DATE:	FIGURE NO.
NTS	SEPT 19, 2019	1

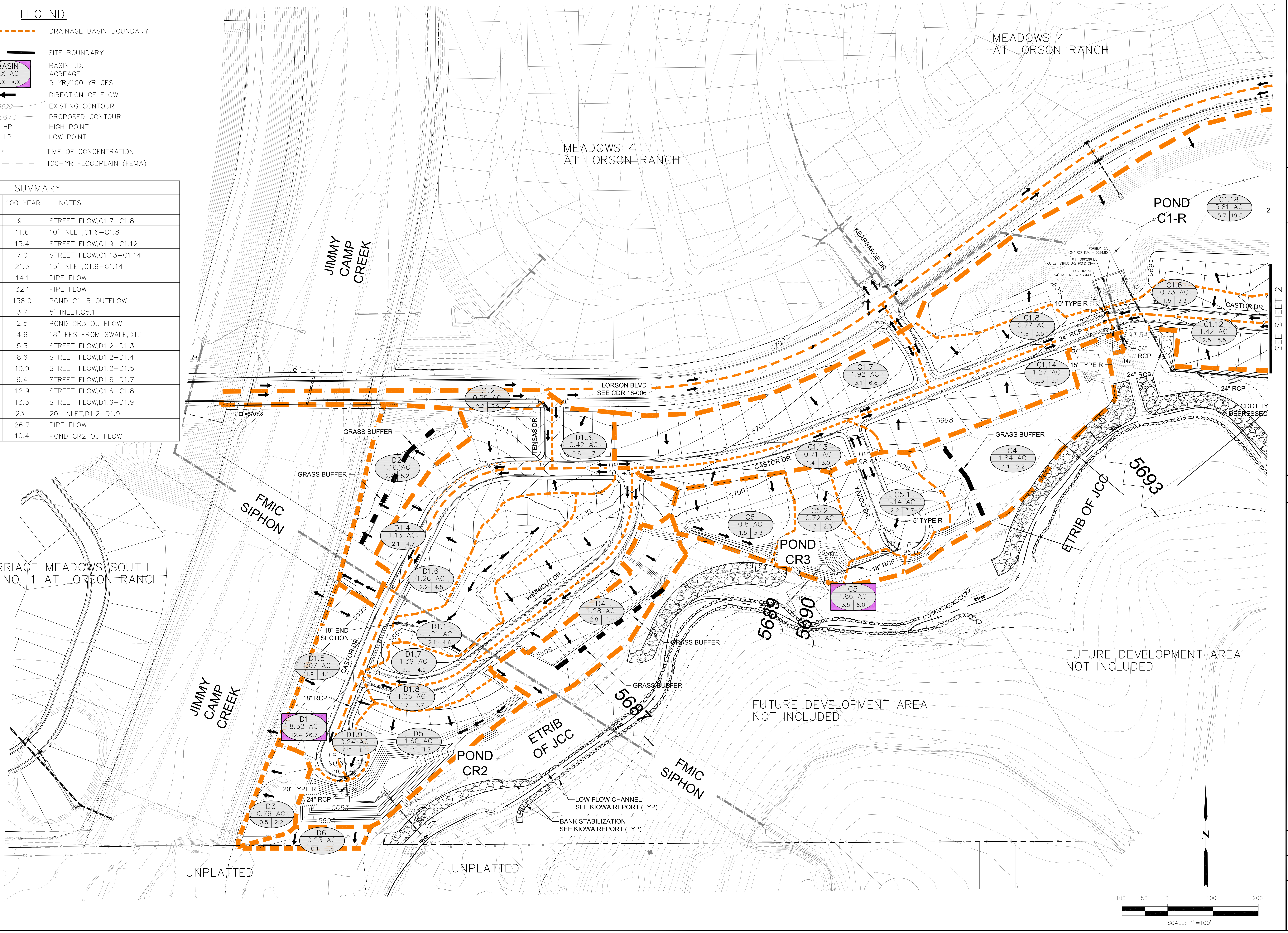
LEGEND

- - - - -	DRAINAGE BASIN BOUNDARY
— - —	SITE BOUNDARY
BASIN XX AC X.X | X.X	BASIN I.D. ACREAGE 5 YR/100 YR CFS
←	DIRECTION OF FLOW
6690	EXISTING CONTOUR
6670	PROPOSED CONTOUR
HP	HIGH POINT
LP	LOW POINT
—·→—	TIME OF CONCENTRATION
— · —	100-YR FLOODPLAIN (FEMA)

DENOTES OVERALL BASIN

RUNOFF SUMMARY

DESIGN POINT	5 YEAR	100 YEAR	NOTES
5	4.1	9.1	STREET FLOW,C1.7–C1.8
6	5.3	11.6	10' INLET,C1.6–C1.8
8	6.9	15.4	STREET FLOW,C1.9–C1.12
9	3.2	7.0	STREET FLOW,C1.13–C1.14
10	9.7	21.5	15' INLET,C1.9–C1.14
13	6.3	14.1	PIPE FLOW
14	14.5	32.1	PIPE FLOW
14a	10.0	138.0	POND C1–R OUTFLOW
15	2.2	3.7	5' INLET,C5.1
15a	0.07	2.5	POND CR3 OUTFLOW
16	2.1	4.6	18" FES FROM SWALE,D1.1
17	2.8	5.3	STREET FLOW,D1.2–D1.3
18	4.2	8.6	STREET FLOW,D1.2–D1.4
19	5.2	10.9	STREET FLOW,D1.2–D1.5
20	4.3	9.4	STREET FLOW,D1.6–D1.7
21	5.9	12.9	STREET FLOW,C1.6–C1.8
22	6.0	13.3	STREET FLOW,D1.6–D1.9
23	10.8	23.1	20' INLET,D1.2–D1.9
24	12.4	26.7	PIPE FLOW
24a	0.2	10.4	POND CR2 OUTFLOW



CORE ENGINEERING GROUP

15004 1ST AVE. S.
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CONTACT: RICHARD L. SCHINDLER, P.E.
EMAIL: Rich@ceg1.com

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COLORADO SPRINGS, COLORADO 80903
(719) 635-3200
CONTACT: JEFF MARK

PROJECT:
CREEKSIDE FIL NO. 1
LORSON BLVD – OLD GLORY DRIVE
EL PASO COUNTY, COLORADO

DRAWN: RLS
DESIGNED: LAB
CHECKED: LAB

DEVELOPED CONDITIONS CREEKSIDE FILING NO. 1

DATE
SEPT 19, 2019

PROJECT NO.
100.045

SHEET NUMBER
1

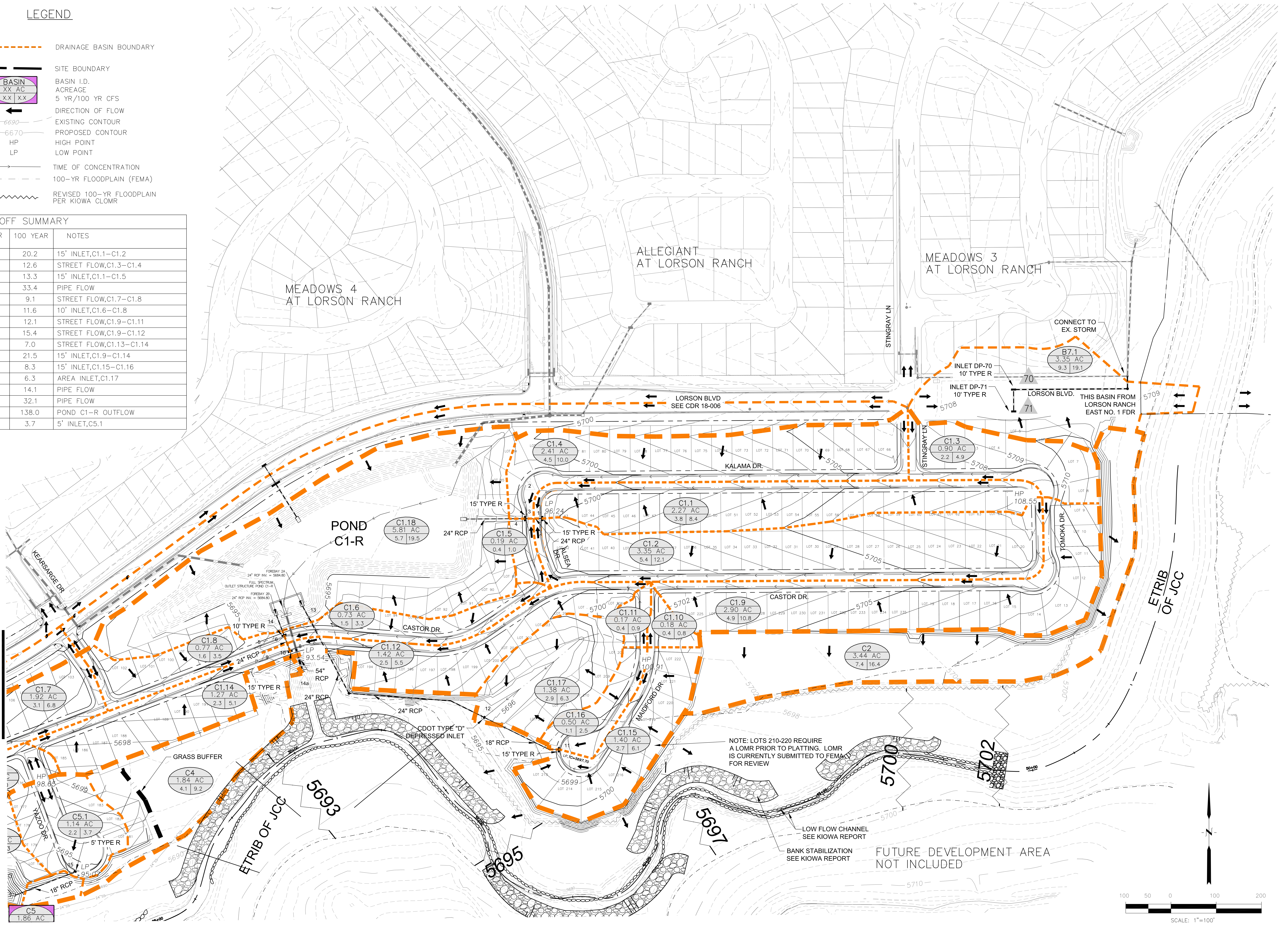
TOTAL SHEETS: 2

LEGEND

🟧 - - -	DRAINAGE BASIN BOUNDARY
▬ — · —	SITE BOUNDARY
DENOTES OVERALL BASIN: BASIN / XX AC / X.X X.X	BASIN I.D. ACREAGE 5 YR/100 YR CFS
←	DIRECTION OF FLOW
6690	EXISTING CONTOUR
6670	PROPOSED CONTOUR
HP	HIGH POINT
LP	LOW POINT
—·—→—	TIME OF CONCENTRATION
— · —	100–YR FLOODPLAIN (FEMA)
∿∿∿	REVISED 100–YR FLOODPLAIN PER KIOWA CLOMR

RUNOFF SUMMARY

DESIGN POINT	5 YEAR	100 YEAR	NOTES
1	9.1	20.2	15' INLET,C1.1–C1.2
2	5.7	12.6	STREET FLOW,C1.3–C1.4
3	6.0	13.3	15' INLET,C1.1–C1.5
4	15.0	33.4	PIPE FLOW
5	4.1	9.1	STREET FLOW,C1.7–C1.8
6	5.3	11.6	10' INLET,C1.6–C1.8
7	5.4	12.1	STREET FLOW,C1.9–C1.11
8	6.9	15.4	STREET FLOW,C1.9–C1.12
9	3.2	7.0	STREET FLOW,C1.13–C1.14
10	9.7	21.5	15' INLET,C1.9–C1.14
11	3.7	8.3	15' INLET,C1.15–C1.16
12	2.9	6.3	AREA INLET,C1.17
13	6.3	14.1	PIPE FLOW
14	14.5	32.1	PIPE FLOW
14a	10.0	138.0	POND C1–R OUTFLOW
15	2.2	3.7	5' INLET,C5.1



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EL PASO COUNTY, COLORADO

DRAWN: RLS
DESIGNED: LAB
CHECKED: LAB

DEVELOPED CONDITIONS CREEKSIDE FILING NO. 1

DATE
SEPT 19, 2019

PROJECT NO.
100.045

SHEET NUMBER
2

TOTAL SHEETS: 2