



March 28, 2024

El Paso County
Planning & Community Development
2880 International Circle, Suite 110
Colorado Springs, CO 80910

Attn.: Project Manager

RE: Paint Brush Hills Filing No. 13E
Private Detention/Stormwater Quality Pond 'D'

Dear Project Manager:

Per the approved construction drawings for "Paint Brush Hills Filing 13E" improvements were made to construct a water quality facility in compliance with the current El Paso County Drainage Criteria and with the approved Final Drainage Report for this project.

Based upon this information and periodic site visits to the project during significant/key phases of the stormwater BMP installation, M&S Civil Consultants, Inc. is of the opinion that the stormwater BMPs have been constructed in general compliance with the approved design plans, and specifications as filed with El Paso County.

Statement Of Engineer In Responsible Charge

To the best of my knowledge, information and belief, for the referenced project above, the improvements have been constructed in general compliance with the approved design plans and specifications as filed with El Paso County to provide the required storage volume and meet the required release rates documented by the SDI design form, the stage areas, elevations and outlet dimensions. In addition, to the best of my knowledge, information and belief, for the referenced project above, the site and adjacent properties (as affected by work performed under the County permit) are stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and that the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements.

Virgil A. Sanchez
Colorado P.E. No.37160
For and on behalf of M&S Civil
Consultants, Inc.



LINE	LENGTH	BEARING
L19	35.61'	N82°36'44"E
L20	162.27'	N89°37'00"E
L21	64.00'	S62°10'18"E
L22	26.67'	N18°52'36"E

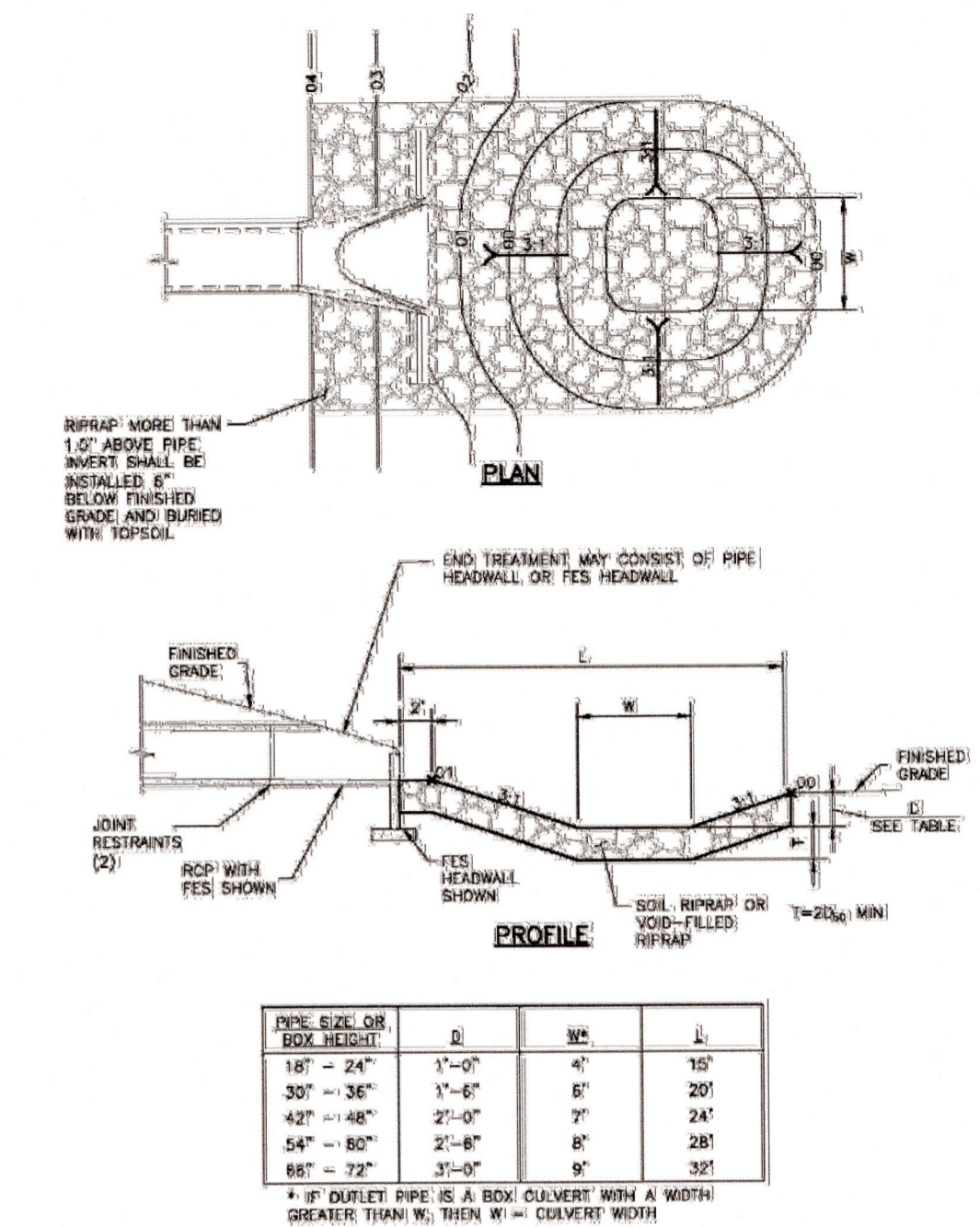
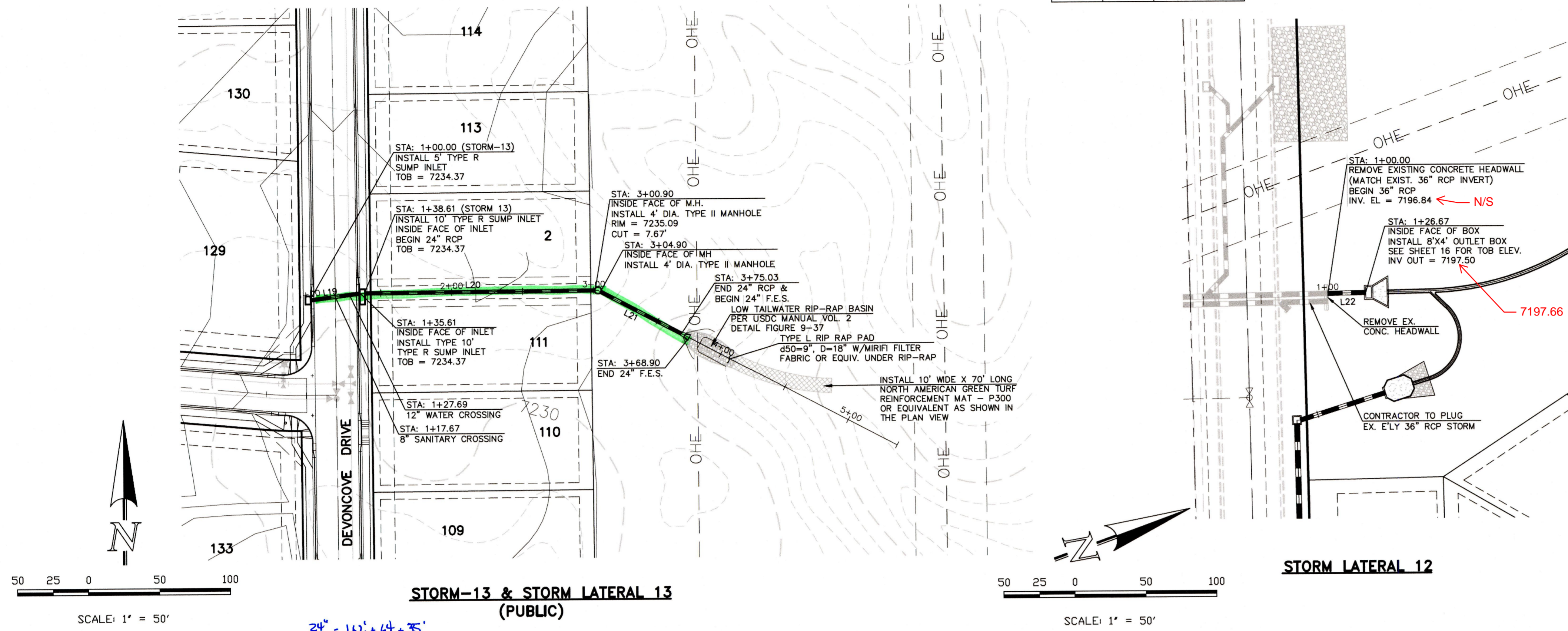
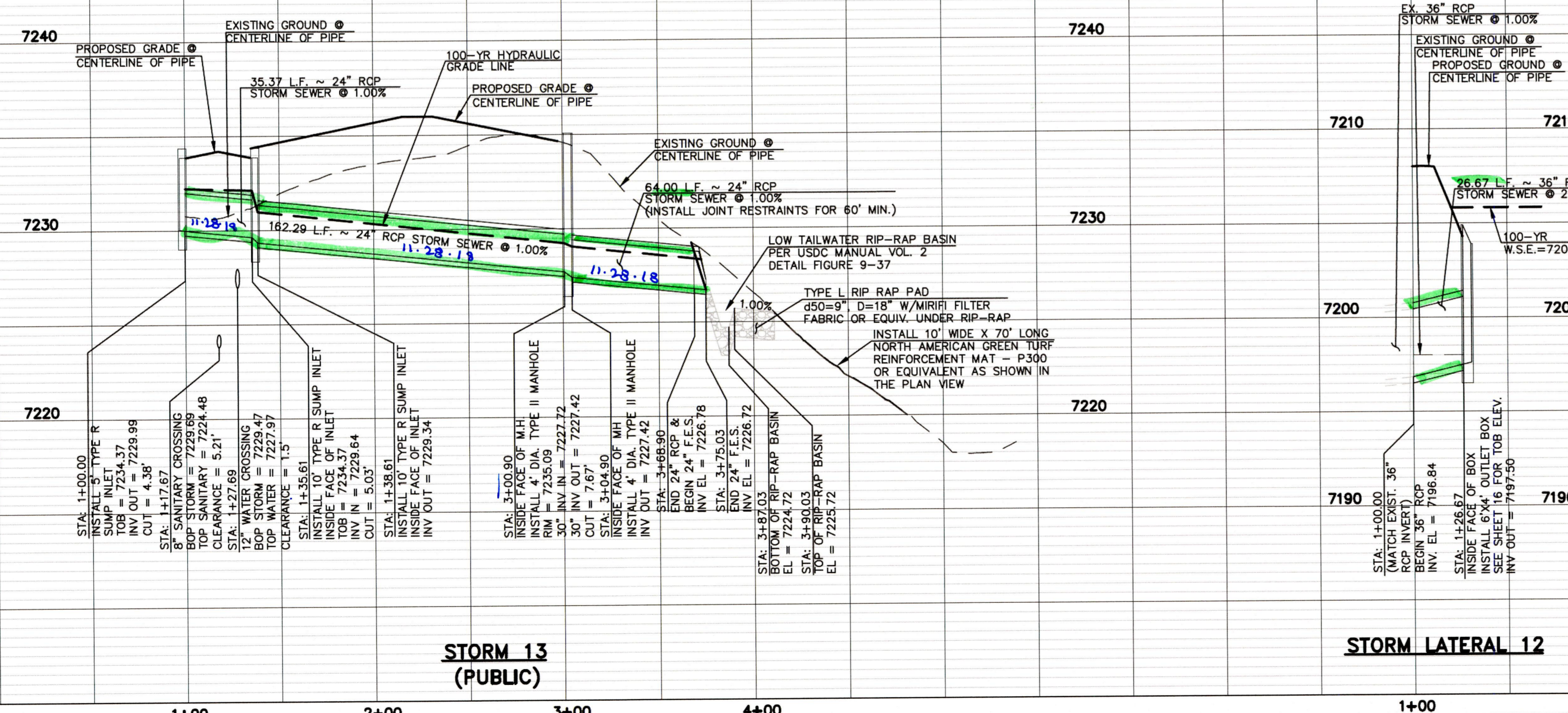


Figure 9-37. Low tailwater riprap basin

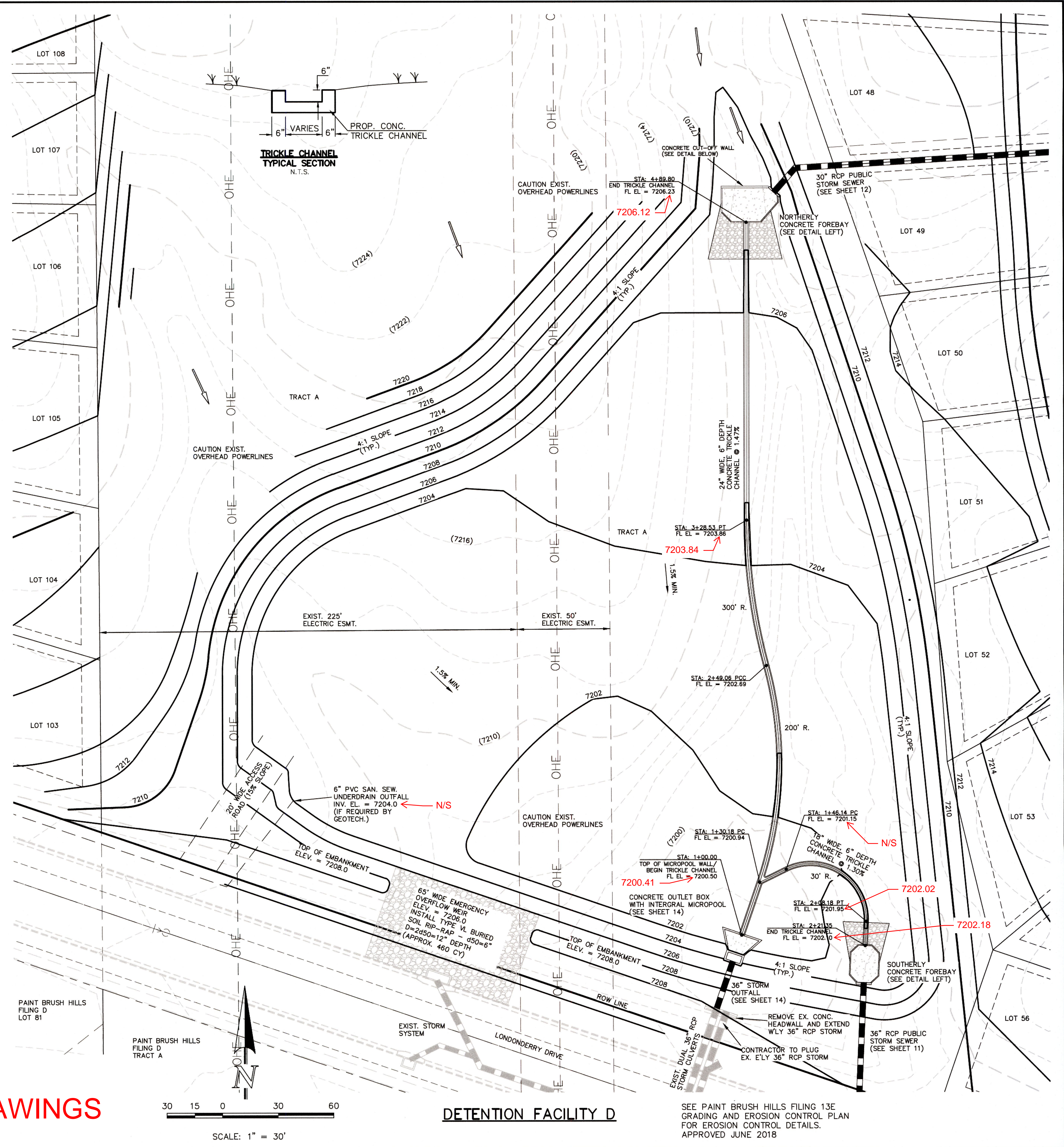
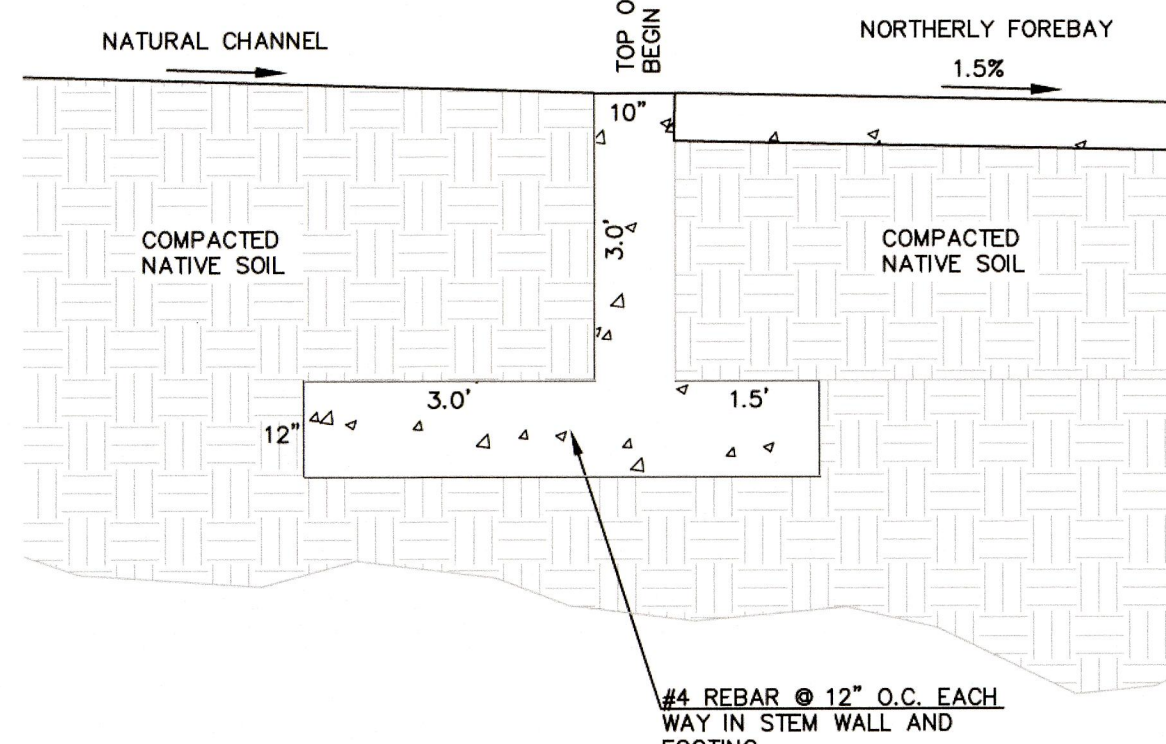
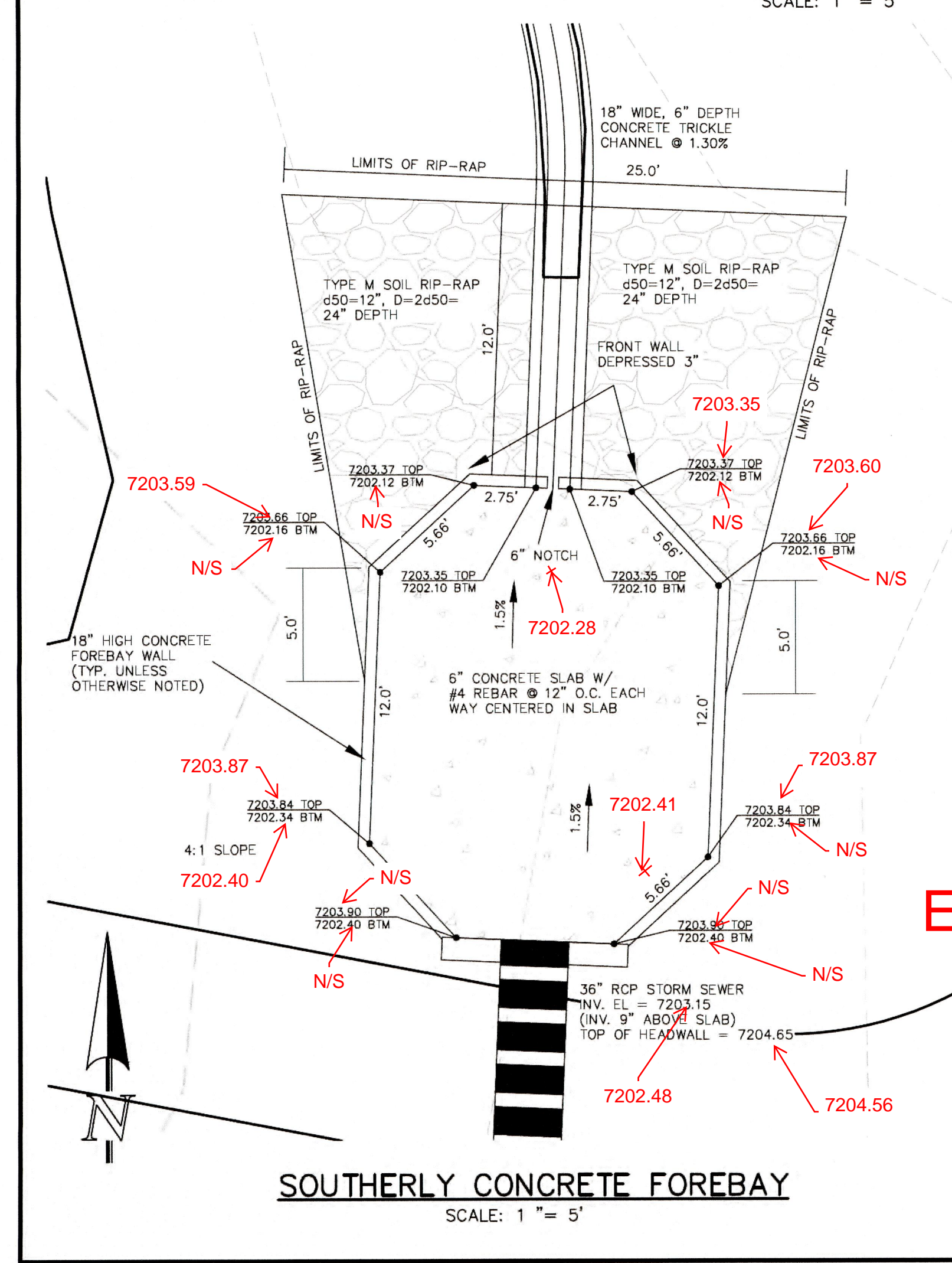
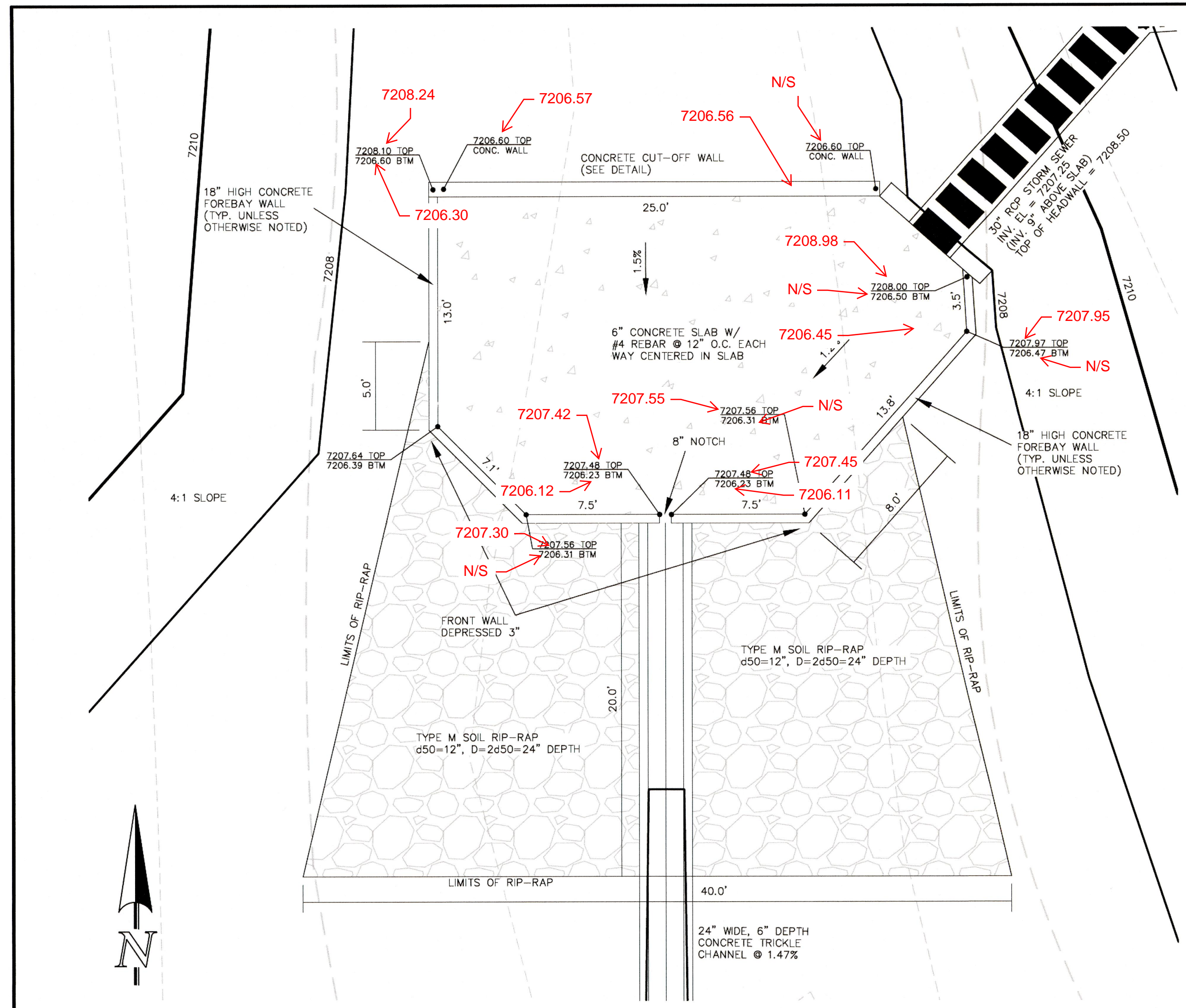
AS-BUILT ENGINEERING RECORD DRAWINGS



<p>48 HOURS BEFORE YOU DIG, CALL UTILITY LOCATORS</p> <p>811 UTILITY NOTIFICATION CENTER OF COLORADO IT'S THE LAW</p> <p>THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE CAUSED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.</p>		<table border="1"> <thead> <tr> <th>NO.</th> <th>REVISION</th> <th>DATE</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>REVISED PER COUNTY COMMENTS</td> <td>9-26-18</td> </tr> <tr> <td>2</td> <td>REVISED PER COUNTY COMMENTS</td> <td>10-18-18</td> </tr> </tbody> </table>	NO.	REVISION	DATE	1	REVISED PER COUNTY COMMENTS	9-26-18	2	REVISED PER COUNTY COMMENTS	10-18-18	<p>REVIEW:</p> <p>PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF CLASSIC CONSULTING ENGINEERS AND SURVEYORS, LLC</p> <p>MARC A. WHORTON, COLORADO P.E. #37155</p> <p>10/19/18</p>	<p>PAINT BRUSH HILLS FILING NO. 13E CONSTRUCTION PLANS STORM SEWER PLAN</p> <p>DESIGNED BY: ESO SCALE: DATE: 8-10-12</p> <p>DRAWN BY: ESO (H) 1" = 50' SHEET 14 OF 47-18</p> <p>CHECKED BY: (V) 1" = 5' JOB NO. 2053.50</p>
NO.	REVISION	DATE											
1	REVISED PER COUNTY COMMENTS	9-26-18											
2	REVISED PER COUNTY COMMENTS	10-18-18											



N:\205350\DRAWINGS\CONSTRUCTION\Construction Plans\STREET AND STORM_CDS\205350-STMA-SHEET 14.dwg, 10/19/2018 11:34:45 AM, 1:1



AS-BUILT ENGINEERING RECORD DRAWINGS



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NO.	REVISION	DATE
1	REVISED PER COUNTY COMMENTS	9-20-18

REVIEW: PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF CLASSIC CONSULTING ENGINEERS AND SURVEYORS, LLC

MARC A. WHORTON, COLORADO P.E. #37155
DATE: 10/19/18



PAINT BRUSH HILLS FILING 13A CONSTRUCTION PLANS

DETENTION FACILITY D POND PLAN

DESIGNED BY	MAW	SCALE	DATE
7-12-18			

DRAWN BY	MAW	(H) 1" = 30'	SHEET 15 OF 18

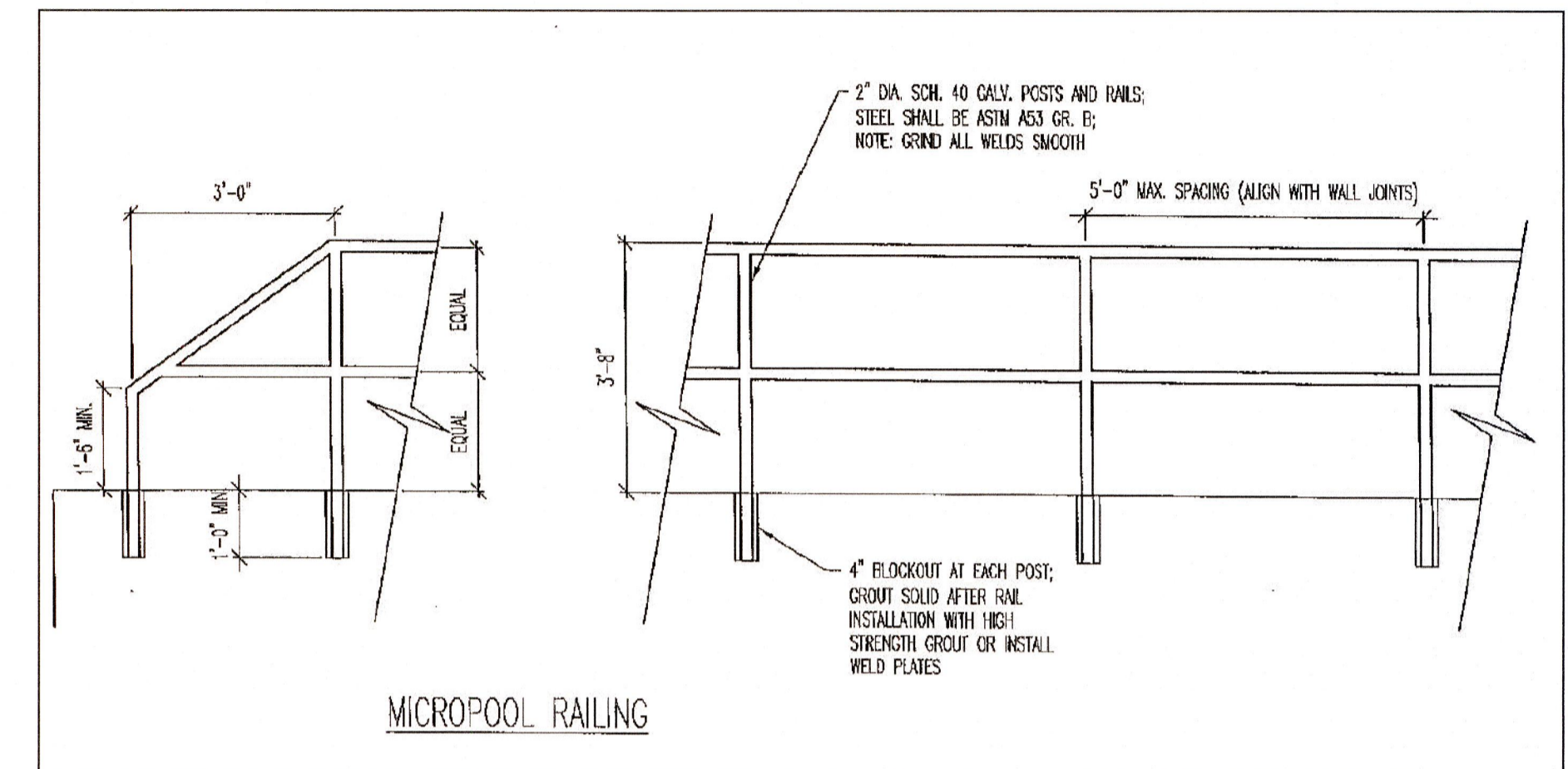
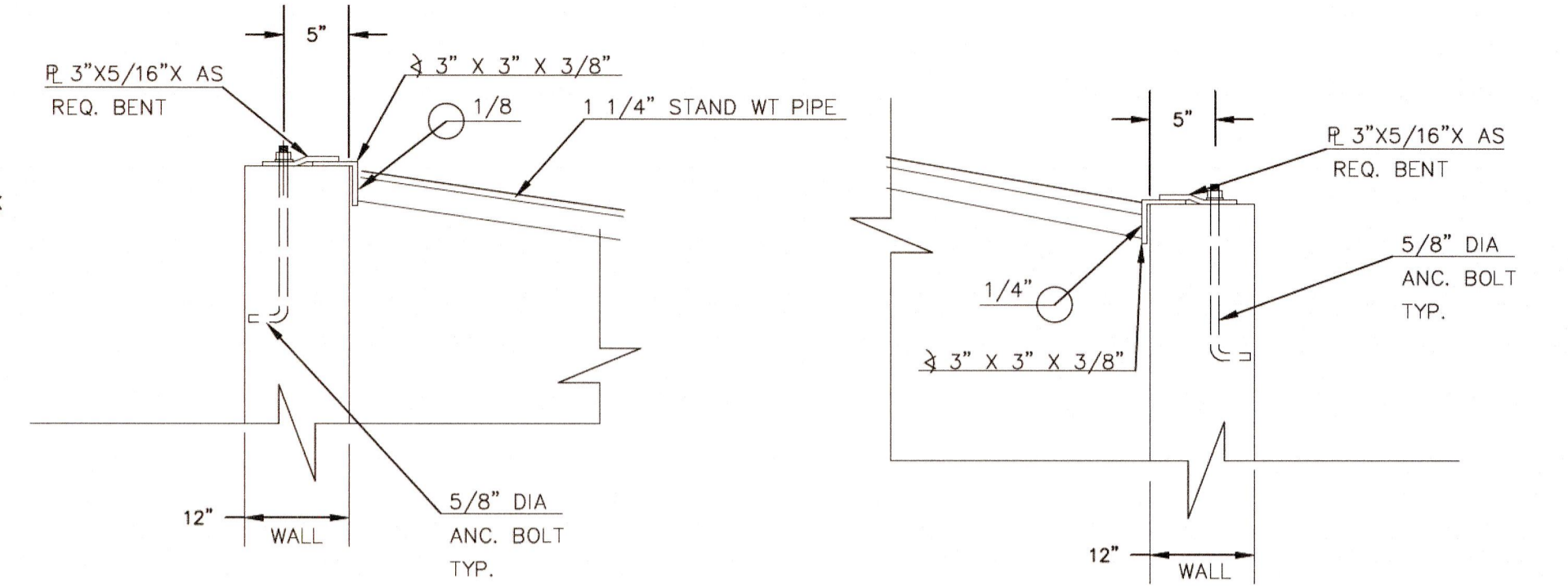
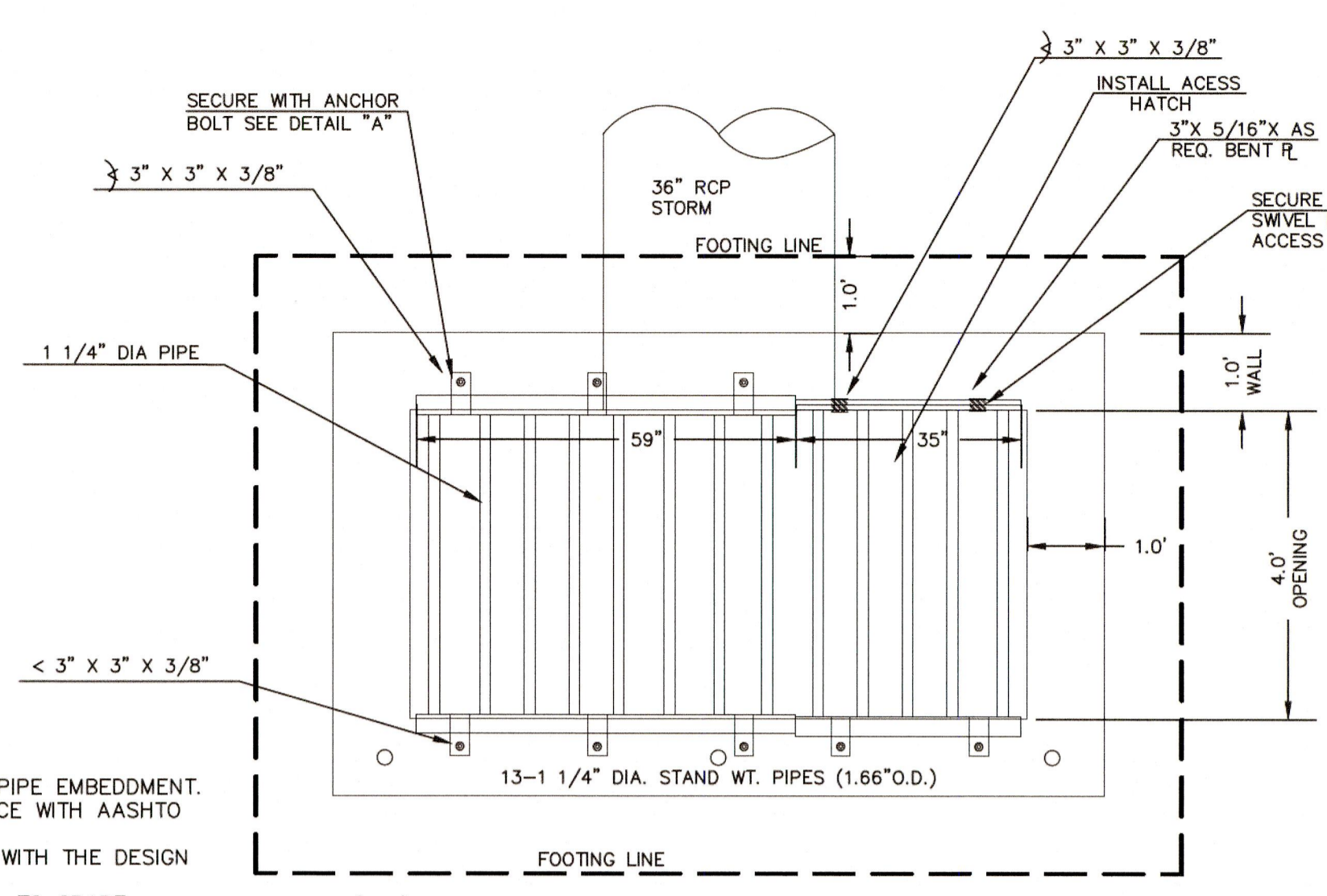
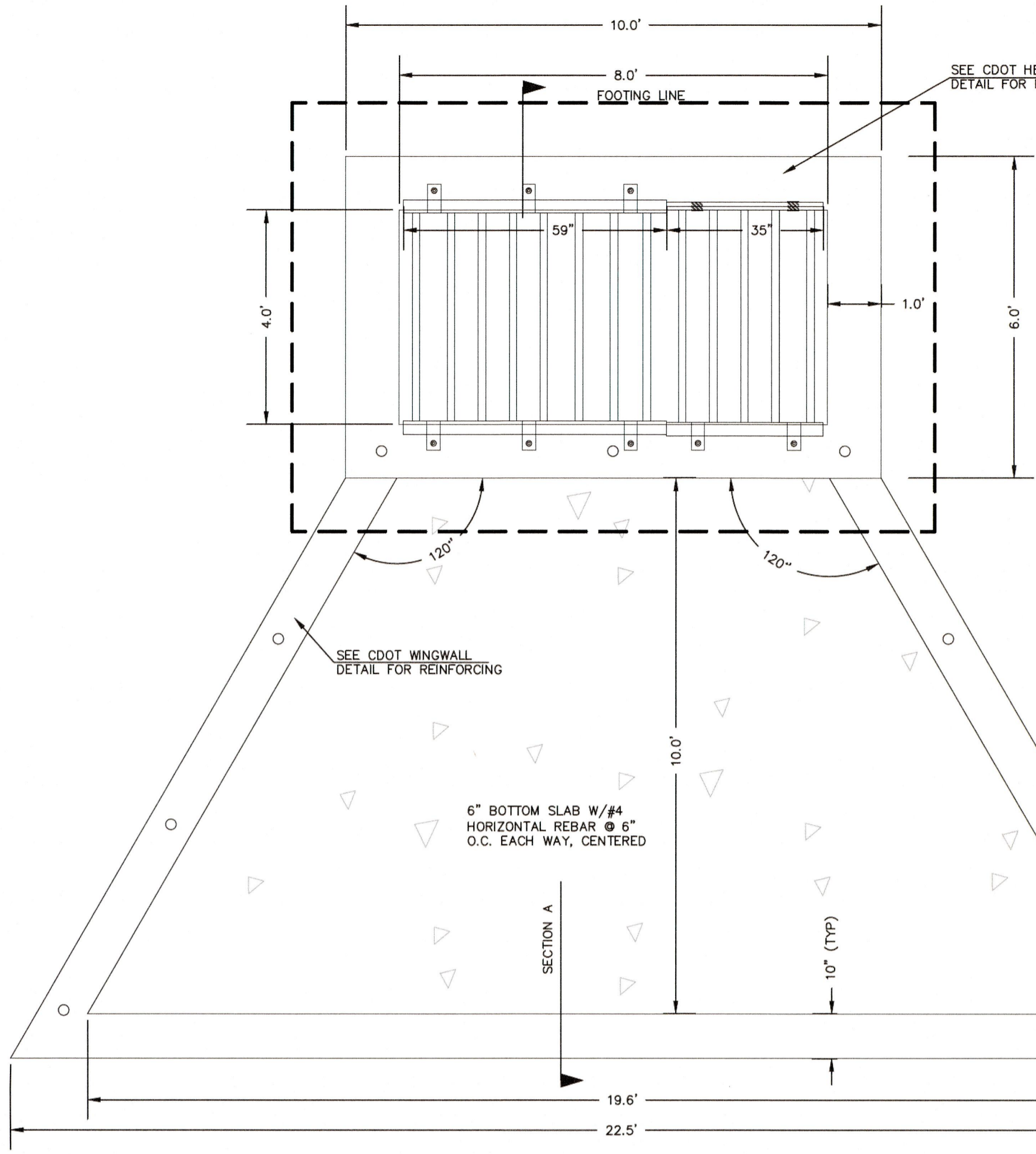
CHECKED BY	(V) 1" = N/A	JOB NO.	2053.50

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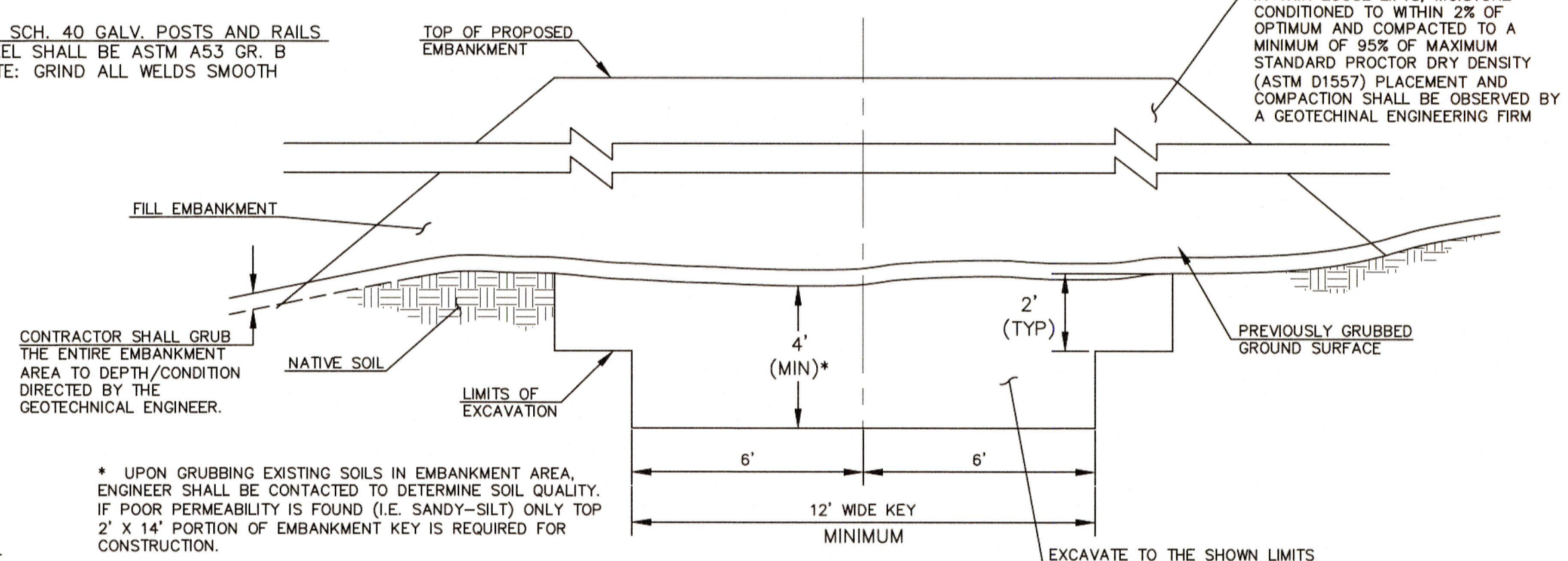
AS-BUILT ENGINEERING RECORD DRAWINGS

NOTES:

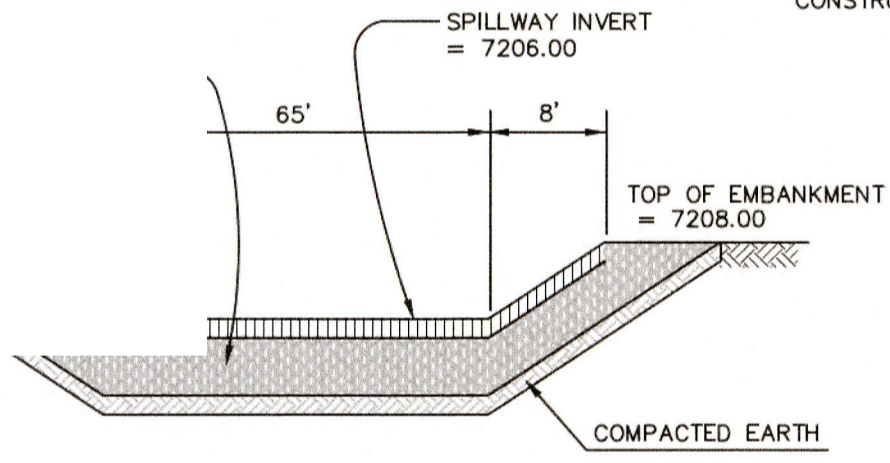
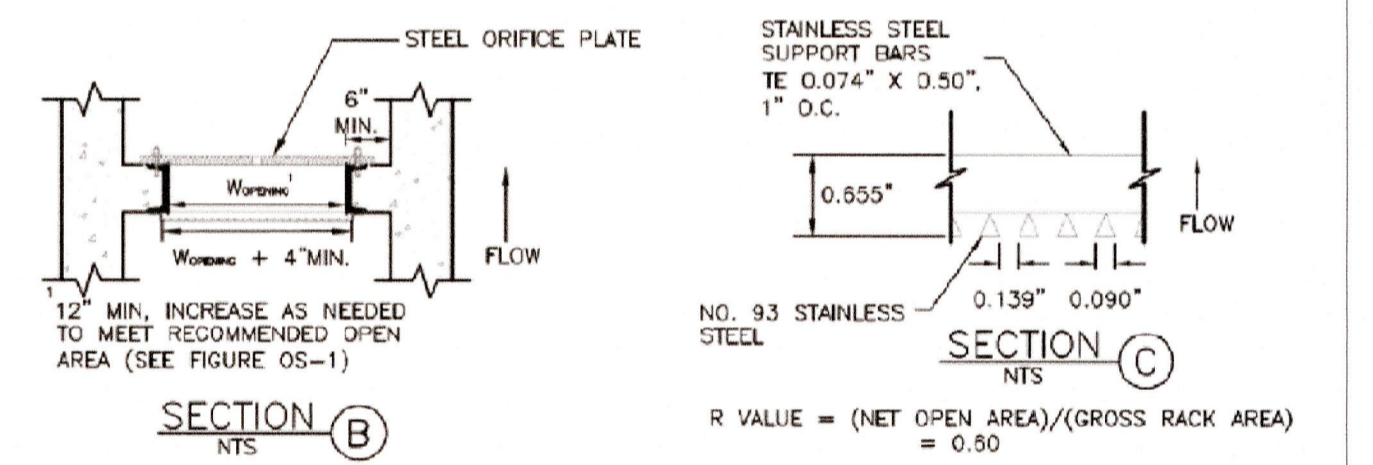
1. WELD PLATES MAY BE SUBSTITUTED FOR PIPE EMBEDMENT. DESIGN CRITERIA SHALL BE IN ACCORDANCE WITH AASHTO STANDARDS.
2. HANDRAIL DESIGN SHALL BE COMPATIBLE WITH THE DESIGN OF THE WINGWALLS AND HEADWALLS.
3. RAILING POSTS SHALL BE SET TO NORMAL TO GRADE. RAILS SHALL RUN PARALLEL TO THE SLOPES OF TOPS OF THE WALLS.
4. ALL RAILS SHALL HAVE EXPANSION JOINTS SPACED AT 40'-0" MAX. JOINT ENDS SHALL BE FREE OF ANY SHARP EDGES OR CORNERS.



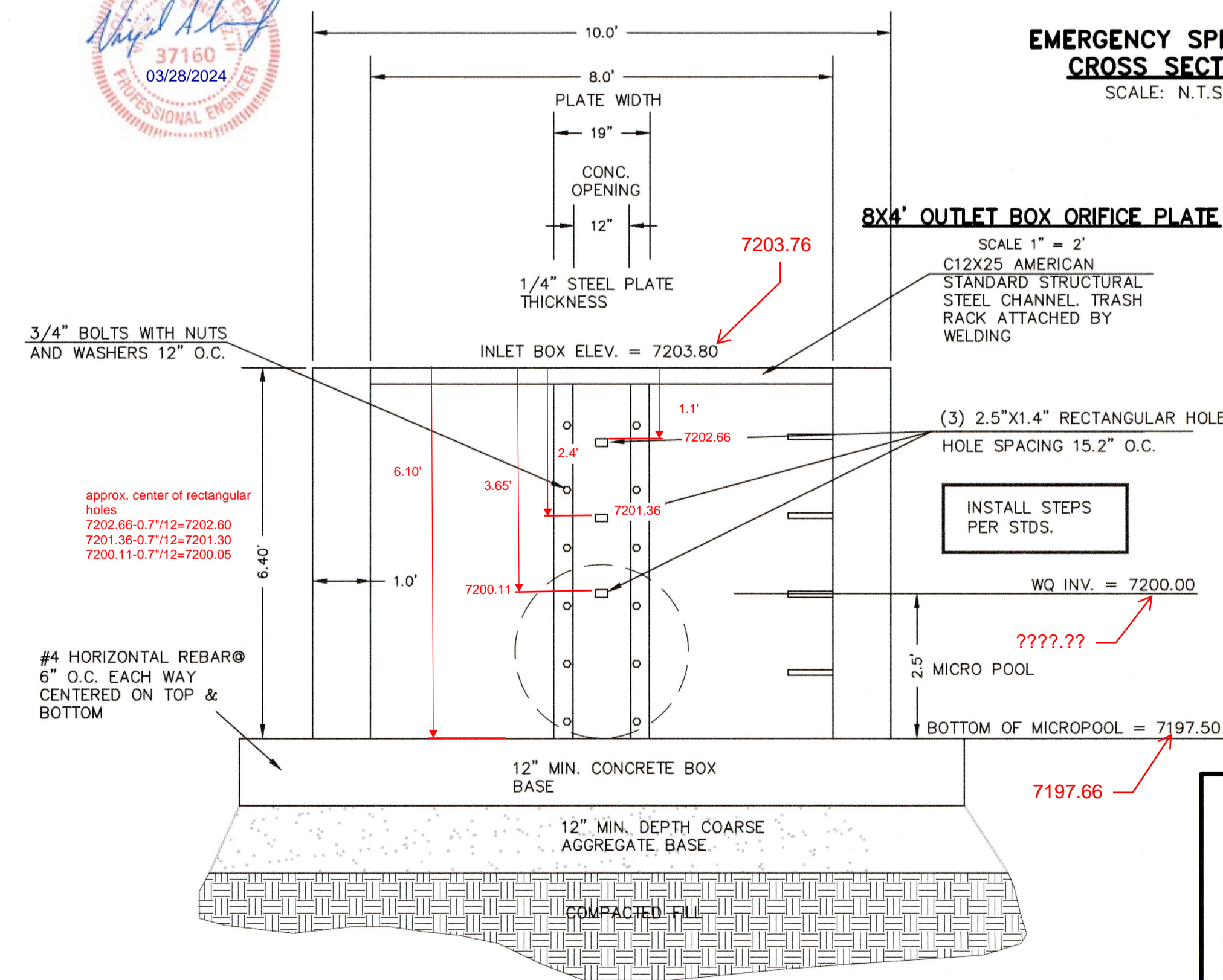
2" SCH. 40 GALV. POSTS AND RAILS. STEEL SHALL BE ASTM A53 GR. B. NOTE: GRIND ALL WELDS SMOOTH



**SECTION A
TYPICAL EMBANKMENT
WITH SOIL KEYWAY**
SCALE: N.T.S.



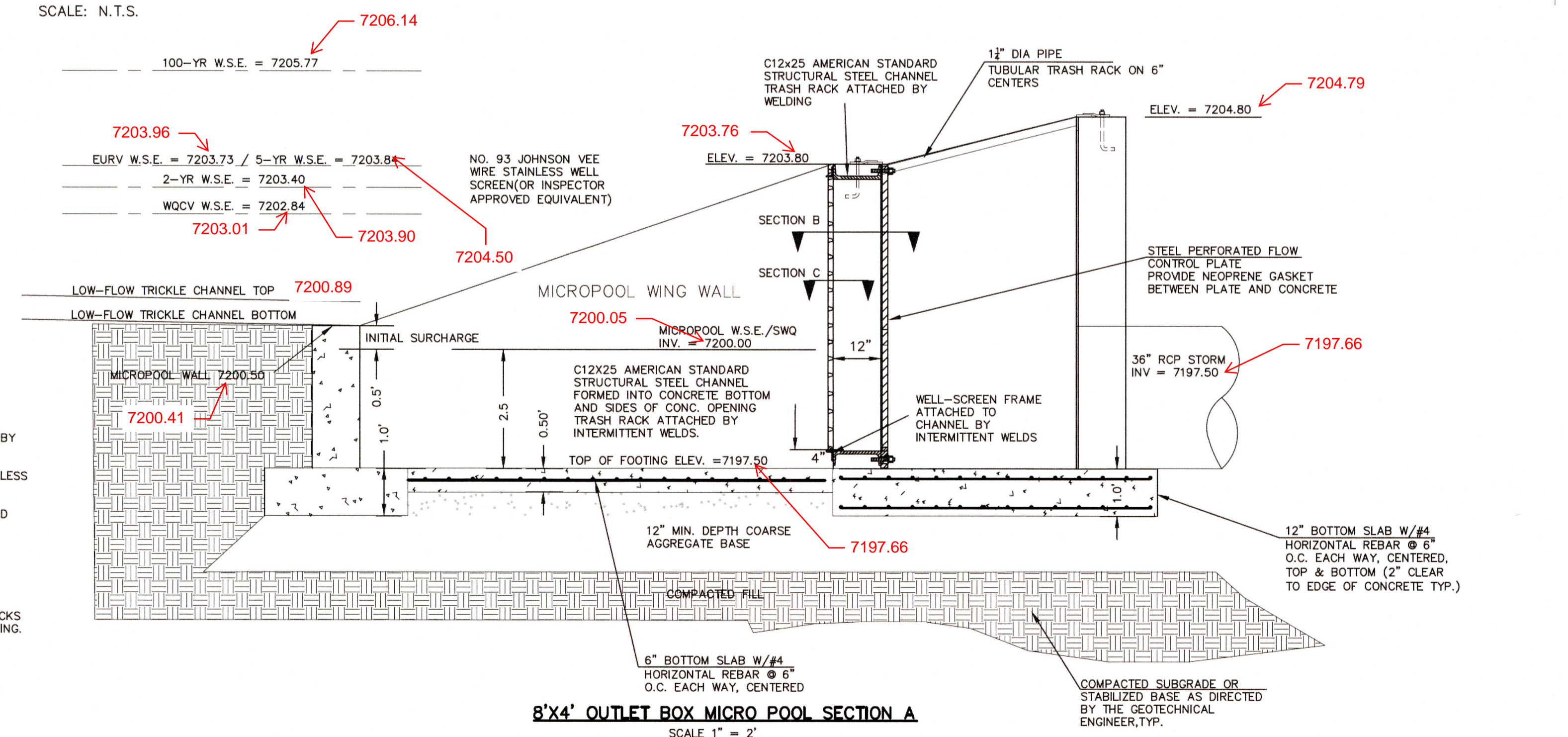
**EMERGENCY SPILLWAY
CROSS SECTION**
SCALE: N.T.S.



(ALL MATERIALS PER EL PASO COUNTY SPECIFICATIONS)

ORIFICE PLATE NOTES:

1. INSTALL HOLES AS SHOWN ON DETAIL TO RIGHT.
 2. PROVIDE GASKET MATERIAL BETWEEN THE ORIFICE PLATE AND CONCRETE
 3. WELL-SCREEN TRASH RACKS SHALL BE STAINLESS STEEL AND SHALL BE ATTACHED BY INTERMITTENT WELDS ALONG THE EDGE OF THE MOUNTING FRAME.
 4. BAR GRATE TRASH RACKS SHALL BE ALUMINUM AND SHALL BE BOLTED USING STAINLESS STEEL HARDWARE.
 5. STRUCTURAL DESIGN OF TRASH RACKS SHALL BE BASED ON FULL HYDROSTATIC HEAD WITH ZERO HEAD DOWNSTREAM OF RACK
- OVERFLOW TRASH RACKS:**
1. ALL TRASH RACKS SHALL BE MOUNTED USING STAINLESS STEEL HARDWARE AND PROVIDED WITH HINGED AND LOCKABLE OR BOLTABLE ACCESS PANELS
 2. TRASH RACKS SHALL BE STAINLESS STEEL, ALUMINUM, OR STEEL. STEEL TRASH RACKS SHALL BE HOT DIP GALVANIZED AND MAY BE HOT POWDER COATED AFTER GALVANIZING.
 3. TRASH RACKS SHALL BE DESIGNED SUCH THAT THE DIAGONAL DIMENSION OF EACH OPENING IS SMALLER THAN THE DIAMETER OF THE OUTLET PIPE.
 4. STRUCTURAL DESIGN OF THE TRASH RACKS SHALL BE BASED ON FULL HYDROSTATIC HEAD WITH ZERO HEAD DOWNSTREAM OF THE RACK.



8'X4' OUTLET BOX MICRO POOL SECTION A
SCALE 1" = 2"

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NO.	REVISION	DATE
1	REVISED PER COUNTY COMMENTS	9-21-18
2	REVISED PER COUNTY COMMENTS	10-18-18

REVIEW:
PREPARED UNDER INDIRECT SUPERVISION FOR AND ON BEHALF OF
CLASSIC CONSULTING ENGINEERS AND SURVEYORS, LLC

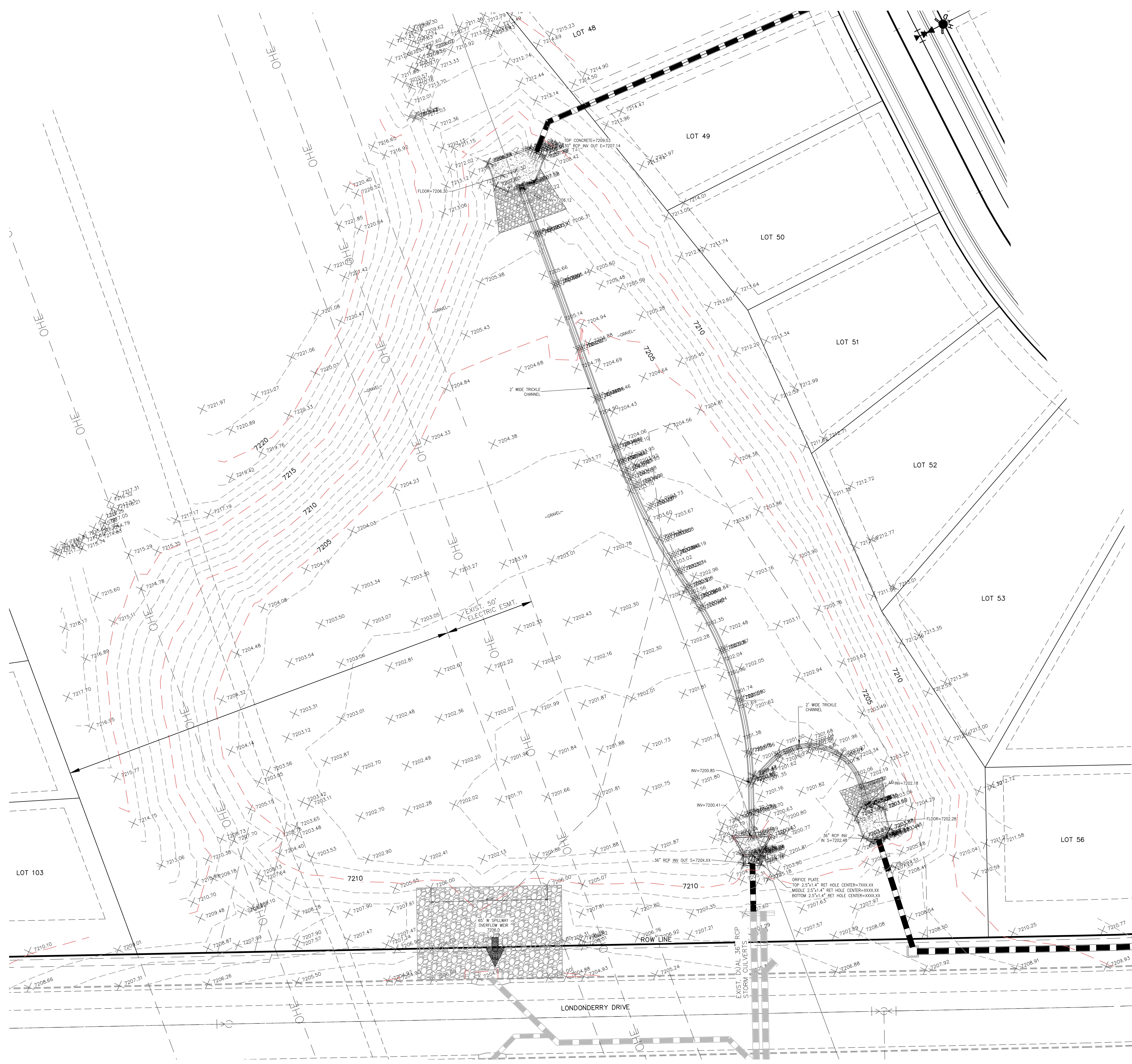
MARC A. WORTON, COLORADO P.E. #37155
DATE 10/19/18



PAINT BRUSH HILLS FILING 13A
CONSTRUCTION PLANS
DETENTION FACILITY D
OUTLET BOX DETAILS

DESIGNED BY MAW SCALE DATE 7-12-18
DRAWN BY MAW (H) 1" = N/A SHEET 16 OF 47-18
CHECKED BY (V) 1" = N/A JOB NO. 2053.50





LEGEND

- EXIST MAJ CONT
- EXIST MIN CONT
- UNDERGROUND STORM SEWER LINE(S)
- EMERGENCY OVERTFLOW/SPILLWAY
- RIPRAP

POND CERTIFICATION

FOREBAY INLETS

DESIGNED INVERT 30" IN (NE) = 7207.25', SURVEY INVERT 30" IN (NE) = 7207.14'
 DESIGNED INVERT 36" IN (S) = 7202.80', SURVEY INVERT 36" IN (S) = 7202.48'

OUTLET STRUCTURE

DESIGNED LF CHANNEL INVERT IN = 7200.50', SURVEY INVERT IN = 7200.41'
 DESIGNED 36" INVERT OUT (S) = 7197.50', SURVEY 36" INVERT OUT (S) = 7197.66'
 DESIGNED TOP CONC. BOX/GRATE (FRONT) = 7203.80', SURVEY TOP CONC. GRATE = 7203.76'
 DESIGNED TOP CONC. BOX/GRATE (BACK) = 7204.80', SURVEY TOP CONC. GRATE = 7204.79'

SPILLWAY (EDB)

DESIGNED SPILLWAY ELEVATION = 7206.00', SURVEY SPILLWAY ELEVATION = 7206.00'

VOLUME

DESIGNED VOLUME = 5.941 ACRE FEET @ 7205.77' (100 YR SURFACE)
 PROVIDED VOLUME = 6.211 ACRE FEET @ 7206.09' (100 YR SURFACE)

PAINT BRUSH HILLS FILING 13A	
PERMANENT BMP PLAN	
PROJECT NO. 43-124	DATE: 03/28/2024
DESIGNED BY: DLM	HORIZONTAL: 1"=20'
DRAWN BY: DLM	VERTICAL: N/A
CHECKED BY: DLM	BMPAB

212 N. WAHATCH AVE, STE 305
 COLORADO SPRINGS, CO 80903
 PHONE 719.555.5885

CIVIL CONSULTANTS, INC.

FOR AND ON BEHALF OF
 MRS. CIVIL CONSULTANTS, INC.

VIRGIL A. SANCHEZ, COLORADO, P.E. NO. 371160

NO.	DATE	BY	DESCRIPTION

FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES

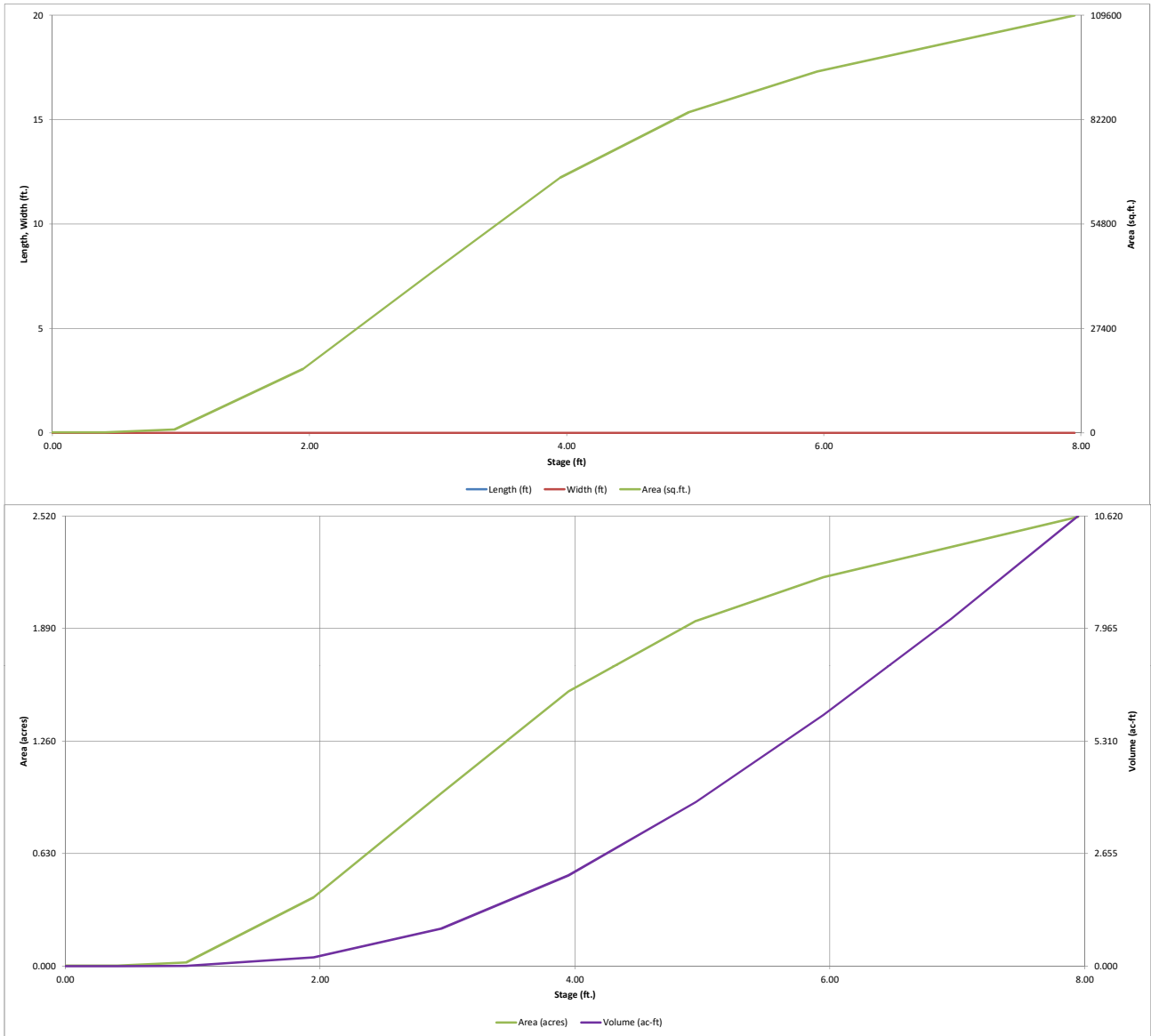
FOR BURIED UTILITY INFORMATION
 48 HRS BEFORE YOU DIG
 CALL 1-800-922-1987

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CAUTION

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

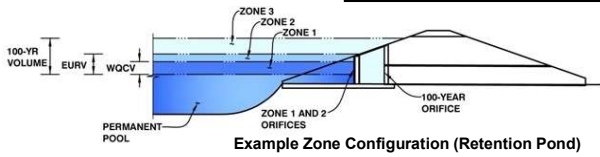
MHFD-*Detention*, Version 4.06 (July 2022)



DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: PAINT BRUSH HILLS FILING 13E
Basin ID: POND D (AS-BUILT)



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.96	0.886	Orifice Plate
Zone 2 (EURV)	3.91	1.175	Orifice Plate
Zone 3 (100-year)	5.36	2.612	Weir&Pipe (Restrict)
Total (all zones)		4.674	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft ²
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	3.96	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	sq. inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft ²
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.25	2.61					
Orifice Area (sq. inches)	3.51	3.51	3.51					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft ²
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	3.71	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	8.00	N/A	feet
Overflow Weir Grate Slope =	4.00	N/A	H:V
Horiz. Length of Weir Sides =	4.00	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H _u =	4.71	N/A	feet
Overflow Weir Slope Length =	4.12	N/A	feet
Grate Open Area / 100-yr Orifice Area =	3.25	N/A	
Overflow Grate Open Area w/o Debris =	22.96	N/A	ft ²
Overflow Grate Open Area w/ Debris =	11.48	N/A	ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	2.39	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	36.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	36.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	7.07	N/A	ft ²
Outlet Orifice Centroid =	1.50	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	3.14	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	5.95	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	65.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth =	0.70	feet
Stage at Top of Freeboard =	7.65	feet
Basin Area at Top of Freeboard =	2.46	acres
Basin Volume at Top of Freeboard =	9.87	acre-ft

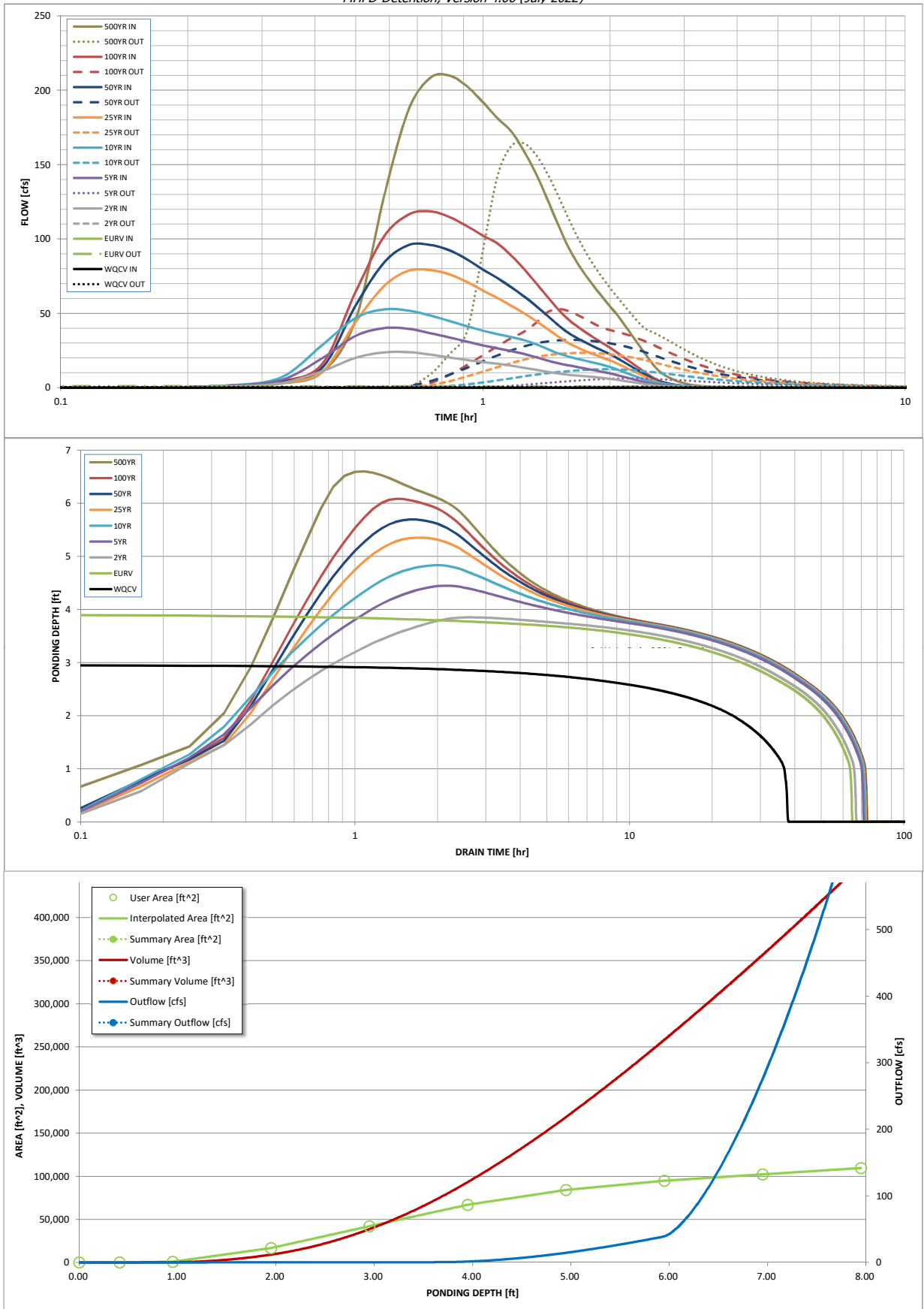
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.85
One-Hour Rainfall Depth (in) =	0.886	2.062	2.127	3.558	4.887	6.936	8.464	10.524	19.067
CUHP Runoff Volume (acre-ft) =	N/A	N/A	2.127	3.558	4.887	6.936	8.464	10.524	19.067
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	7.6	21.3	32.4	57.9	72.7	92.6	173.1
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.10	0.28	0.43	0.77	0.97	1.23	2.31
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A	23.9	40.0	52.6	79.0	95.9	118.6	209.7
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.9	6.2	12.4	23.4	32.0	52.9	164.4
Peak Inflow Q (cfs) =	N/A	N/A	N/A	0.3	0.4	0.4	0.4	0.6	0.9
Peak Outflow Q (cfs) =	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Spillway	Spillway
Ratio Peak Outflow to Predevelopment Q =	N/A	0.03	0.02	0.2	0.5	1.0	1.4	1.8	2.5
Structure Controlling Flow =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Max Velocity through Grate 1 (fps) =	35	59	62	63	62	60	58	56	46
Max Velocity through Grate 2 (fps) =	37	63	65	68	68	67	66	65	61
Time to Drain 97% of Inflow Volume (hours) =	2.96	3.91	3.85	4.45	4.83	5.35	5.69	6.09	6.60
Time to Drain 99% of Inflow Volume (hours) =	0.97	1.51	1.48	1.73	1.88	2.03	2.11	2.20	2.29
Area at Maximum Ponding Depth (acres) =	0.895	2.076	1.986	2.938	3.642	4.664	5.368	6.211	7.354
Maximum Volume Stored (acre-ft) =									

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.09	0.01
	0:15:00	0.00	0.00	0.76	1.24	1.54	1.04	1.35	1.29	2.79
	0:20:00	0.00	0.00	3.02	5.21	6.94	3.09	3.72	4.40	10.53
	0:25:00	0.00	0.00	11.11	19.84	28.71	10.96	13.44	16.04	45.90
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	0:40:00	0.00	0.00	23.94	39.53	51.73	78.60	95.80	116.31	208.20
	0:45:00	0.00	0.00	22.26	36.64	48.49	79.03	95.89	118.55	209.73
	0:50:00	0.00	0.00	20.32	33.74	44.78	76.18	92.29	114.69	202.60
	0:55:00	0.00	0.00	18.64	30.96	41.27	71.07	86.21	108.64	191.89
	1:00:00	0.00	0.00	17.18	28.41	38.27	65.30	79.37	102.23	181.03
	1:05:00	0.00	0.00	16.04	26.37	35.97	60.31	73.58	96.94	172.38
	1:10:00	0.00	0.00	14.76	24.60	33.96	55.17	67.50	88.89	159.14
	1:15:00	0.00	0.00	13.39	22.67	31.96	50.16	61.49	79.97	144.37
	1:20:00	0.00	0.00	12.04	20.53	29.29	44.99	55.09	70.67	127.60
	1:25:00	0.00	0.00	10.73	18.40	26.15	39.92	48.76	61.68	110.98
	1:30:00	0.00	0.00	9.57	16.52	23.25	34.90	42.54	53.40	96.16
	1:35:00	0.00	0.00	8.71	15.19	21.14	30.51	37.32	46.64	84.71
	1:40:00	0.00	0.00	8.12	13.99	19.51	27.29	33.45	41.56	75.73
	1:45:00	0.00	0.00	7.61	12.81	18.06	24.67	30.25	37.33	67.99
	1:50:00	0.00	0.00	7.14	11.70	16.73	22.40	27.44	33.57	61.06
	1:55:00	0.00	0.00	6.54	10.64	15.37	20.32	24.87	30.13	54.67
	2:00:00	0.00	0.00	5.92	9.62	13.85	18.39	22.47	26.92	48.65
	2:05:00	0.00	0.00	5.18	8.40	12.07	16.18	19.69	23.45	42.06
	2:10:00	0.00	0.00	4.43	7.15	10.26	13.91	16.85	20.05	35.54
	2:15:00	0.00	0.00	3.72	5.95	8.53	11.71	14.10	16.76	29.23
	2:20:00	0.00	0.00	3.03	4.80	6.91	9.59	11.45	13.57	23.19
	2:25:00	0.00	0.00	2.38	3.73	5.41	7.56	8.93	10.50	17.46
	2:30:00	0.00	0.00	1.79	2.76	4.07	5.62	6.56	7.61	12.54
	2:35:00	0.00	0.00	1.34	2.08	3.19	3.89	4.64	5.36	9.31
	2:40:00	0.00	0.00	1.05	1.67	2.60	2.84	3.45	3.91	7.03
	2:45:00	0.00	0.00	0.85	1.37	2.13	2.13	2.62	2.88	5.27
	2:50:00	0.00	0.00	0.70	1.12	1.74	1.62	2.00	2.10	3.91
	2:55:00	0.00	0.00	0.58	0.92	1.42	1.23	1.53	1.52	2.86
	3:00:00	0.00	0.00	0.47	0.74	1.14	0.96	1.18	1.09	2.07
	3:05:00	0.00	0.00	0.39	0.60	0.92	0.74	0.92	0.79	1.52
	3:10:00	0.00	0.00	0.32	0.48	0.73	0.58	0.72	0.60	1.19
	3:15:00	0.00	0.00	0.26	0.38	0.56	0.46	0.56	0.48	0.93
	3:20:00	0.00	0.00	0.21	0.29	0.44	0.36	0.44	0.39	0.74
	3:25:00	0.00	0.00	0.17	0.22	0.34	0.28	0.34	0.30	0.58
	3:30:00	0.00	0.00	0.13	0.16	0.26	0.22	0.26	0.23	0.44
	3:35:00	0.00	0.00	0.09	0.12	0.19	0.16	0.19	0.17	0.31
	3:40:00	0.00	0.00	0.06	0.08	0.13	0.11	0.14	0.12	0.21
	3:45:00	0.00	0.00	0.04	0.05	0.08	0.08	0.09	0.08	0.13
	3:50:00	0.00	0.00	0.02	0.03	0.05	0.05	0.05	0.04	0.06
	3:55:00	0.00	0.00	0.01	0.02	0.02	0.02	0.02	0.02	0.02
	4:00:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	

















