

FINAL DRAINAGE REPORT FOR DWIRE STORAGE YARD FILING NO. 1 EL PASO COUNTY, COLORADO

MAY 2020

Prepared for:
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Prepared by:



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Project #43-117

PCD Project No. MS-20-002

**FINAL DRAINAGE REPORT FOR
DWIRE STORAGE YARD FILING NO. 1**

DRAINAGE PLAN STATEMENTS

ENGINEERS STATEMENT

The attached drainage plan and report was prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omission on my part in preparing this report.

Virgil A. Sanchez, P.E. #37160
For and on Behalf of M&S Civil Consultants, Inc

DEVELOPER'S STATEMENT

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

BY: _____

TITLE: _____

DATE: _____

ADDRESS: Dwire Earthmoving, Inc.
 6799 Bismark Road, Suite C
 Colorado Springs, CO 80922

EL PASO COUNTY'S STATEMENT

Filed in accordance with the requirements of El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Manual, as amended.

BY: _____

Jennifer Irvine, P.E.
County Engineer

DATE: _____

CONDITIONS:

FINAL DRAINAGE REPORT FOR DWIRE STORAGE YARD FILING NO. 1

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FINAL DRAINAGE REPORT FOR DWIRE STORAGE YARD FILING NO. 1

PURPOSE

This document is intended to serve as the Final Drainage Report for the DWIRE Storage Yard Filing No. 1. The purpose of this document is to identify and analyze the on and offsite drainage patterns and to ensure that post development runoff is routed through the site safely and in a manner that satisfies the requirements set forth by the El Paso County Drainage Criteria Manual. The development plan for the site will consist of a gravel storage yard with office/warehouse buildings, asphalt, curb, lighting, a Full Spectrum Detention Pond and landscaping. The parcel is zoned "I-3" and the proposed use is permissible within the Industrial zoning criteria.

GENERAL LOCATION AND DESCRIPTION

DWIRE Storage Yard Filing No. 1 is located in the southeast quarter of the southwest quarter of Section 28, Township 13 South, Range 65 West of the 6th P.M. in El Paso County, Colorado. The parcel is bound to the north by an existing and abandoned railroad embankment approximately 100 feet from the northern boundary and the East Fork Sand Creek Sub-tributary, to the west by Transit Mix property and East Fork Sand Creek Sub-tributary, to the south by Lot 7 of the Marksheffel Industrial Park, Capitol Drive, and to the east by Timberline Storage Yard. As shown on the enclosed FIRM panel, a channel known as the East Fork of Sand Creek Sub-tributary flows from north to south approximately 85 feet from the western boundary of the site. The site is located within the greater Sand Creek Drainage Basin and is tributary to the Sand Creek Channel via the East Fork Sand Creek Sub-Tributary. A vicinity map showing the location of the proposed development has been provided in the appendix of this report.

The proposed storage yard development and improvements will be constructed on approximately 14.6 acres of the 19.362 acre parcel. The site is currently zoned "I-3" which is associated with industrial development. In the existing condition, both the parcel and offsite contributing watershed lands are sparsely vegetated, with ground cover consisting primarily of native grasses ranging in density from fair to good. The proposed development will consist of a gravel storage yard with two office/warehouse buildings, gravel and asphalt parking areas, lighting, landscaping, and an access road. Slopes across the development typically range between 2% to 5%. Offsite flows reaching development are contributed in part from areas of Timberline Storage Yard along the eastern boundary and from small portions of unplatted property to the north. Flows produced within the development will be collected by proposed storm sewer improvements, swales, riprap rundowns, and will be routed to a proposed full spectrum detention (FSD) pond located at the southern boundary of the development.

SOILS

Soils for this project are delineated by the map in the appendix as Blendon Sandy Loam (10) which is characterized as Hydrologic Soil Types "B". Soils in the study area are shown as mapped by Soil Conservation Service in the "Soils Survey of El Paso County Area".

HYDROLOGIC CALCULATIONS

Hydrologic calculations were performed using the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual and where applicable the Urban Storm Drainage Criteria Manual. The Rational Method was used to estimate stormwater runoff anticipated from design storms with 5-year and 100-year recurrence intervals.

HYDRAULIC CALCULATIONS

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual. The relevant data sheets are included in the appendix of this report.

FLOODPLAIN STATEMENT

A portion of the site lies within the 100 year floodplain according to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 08041C0543 F, effective date March 17, 1997 and the more recent FIRM Panel No. 08041C0543 G, effective date December 7, 2018. Base Flood Elevation (BFE) lines from FIRM Panel No. 08041C0543 F (NGVD29) are used for hydraulic calculations, drainage maps, and a discussion within this report. No development is anticipated to occur within the floodplain located at the northwest corner of the site. See Proposed Drainage Map and the FIRM Panels located in the appendix of this report for details. Tract A is provided on the plat for the portion of the lot encumbered by the floodplain. The Floodplain application and Floodplain permit are included in this report, in the Floodplain Map section in the appendix. No portion of the developed lots are within the 100 year flood zone. Additional work will be at the discretion of the local floodplain administrator in accordance with FEMA policy.

DRAINAGE CRITERIA

This drainage analysis has been prepared in accordance with the current El Paso County Drainage Criteria Manual and where applicable the City of Colorado Springs DCM Volume 1 dated May 2014 effective January 2015. Hydrologic calculations were performed to determine runoff quantities for the 5-year and 100-year frequency storms for developed conditions using the Rational Method as required for basins having areas less than 130 acres (in accordance with Chapter 6 of the City of Colorado Springs DCM Volume 1). Full spectrum detention facilities have been designed in accordance with Section 3.2.1. of Chapter 13 of the City of Colorado Springs DCM Volume 1, dated May 2014, effective January 31, 2015 and Urban Drainage and Flood Control District Manuals dated January 2016.

FOUR STEP PROCESS

Step 1: Employ Runoff Reduction Practices. – Approx. 1.6 acres of the proposed development 19.362 acres is being set aside for a Full Spectrum Detention (FSD) Pond. Whenever possible, runoff produced within developable area containing impervious surfaces will be routed through landscaped areas or earthen swales (grass-lined where slope exceeds 2%) to minimize direct connection of impervious surfaces.

Step 2: Stabilize drainage ways –The DWIRE Storage Yard site proposes a Full Spectrum Detention (FSD) Pond to control developed runoff that is discharging to the East Fork Sand Creek Sub-Tributary located approximately 85-170 feet from the northern and western boundaries of the subject site. The FSD outlet structure has been designed to drain the water quality event storm in 40 hours, while reducing the 100 year peak discharge to approximately 90% of the predevelopment conditions. The development of this site is not anticipated to have negative effects on downstream drainageways. The existing channel embankment has been stabilized at the FSD pond's emergency spillway and where the outlet pipe from the pond enters the channel. Per the DBPS the existing channel is to remain, and no improvements are necessary for this reach of the channel (See "Referenced Reports" in the Appendix, Sheets EF-27 & EF-28).

Step 3: Provide water quality capture volume. – A Full Spectrum Detention Pond is proposed to reduce peak discharge rates and provide water quality treatment. The WQCV will be released over a 40 hour period while larger event storms will be released in periods of times between 64-80 hours.

Step 4: Consider Need for Industrial and Commercial BMP's – This submittal provides a final grading and erosion control plans with BMPs in place. The proposed project will use silt fence, a vehicle tracking control pad, concrete washout area, mulching and reseeded to mitigate the potential for erosion across the site.

EXISTING DRAINAGE CONDITIONS

The DWIRE Storage Yard Filing No. 1 site consists of 19.362 acres situated south and east of the East Fork Sub-tributary of Sand Creek. There are no existing structures within the site. In accordance with the Engineering Criteria Manual (ECM) and Drainage Criteria Manual (DCM) an existing conditions hydrologic analysis was performed to determine existing flow quantities entering and exiting the subject site so a comparison to post development discharge rates could be made. As shown on the enclosed Existing Drainage Map (located in the appendix of this report) the existing site terrain within the parcel generally slopes from north to south at grades that vary between 2% to 5%. An existing and abandoned railroad embankment lies approximately 100 feet from the northern boundary of the site and protects the site from the East Fork Sand Creek Sub-tributary and offsite runoff. The East Fork Sand Creek Sub-Tributary continues from north to south approximately 85 feet from the western boundary of the site. A site visit was made and it was observed that existing channel banks appear to be stable with established vegetation and minimal scour. Per the DBPS the existing channel is to remain, and no improvements are necessary for this reach of the channel (See "Reference Reports" in the Appendix, Sheets EF-27 & EF-28). An overlay of the 100 yr floodplain (Zone AE) is shown on the Existing Drainage Map and the Floodplain Map, of which 0.28 acres overlaps the northwest corner of the site. Refer to the enclosed Existing Drainage Map for specific flow rates associated with Existing Basins and Design Points.

Design Point 1 consists of native grass covered un-platted land located along the east parcel property boundary. Runoff produced by **Basins OS1, OS2 and A**, has been calculated to be 0.8 cfs in the 5-year storm event and 5.10 cfs in the 100-year storm event. Runoff from these basins is conveyed as sheet flow and earthen swale to the east towards **Design Point 1** and are treated for water quality by the Timberline Storage Yard Full Spectrum Detention Pond 1. The "Amendment No.1 to the Final Drainage Report for Timberline Storage Yard", dated March 2020, by M&S Civil Consultants, Inc. accounts for these flows and

the Full Spectrum Detention Pond 1 was designed with consideration to this tributary area (See Timberline Storage Yard Proposed Drainage Map, Basins OS2).

Design Point 2 consists of grass covered un-platted land and a portion of Timberline Storage Yard (Basin OS3). Runoff produced by **Basins A1 and OS3**, has been calculated to be 3.6 cfs in the 5-year storm event and 7.5 cfs in the 100-year storm event. Runoff from these basins is conveyed as sheet flow to the east towards **Design Point 2**. Runoff from Basin OS3 and A1 continues south on existing Capital Drive.

Design Point 3 consists of native grass covered un-platted land located central portion of the property and along the south and west property boundary. Runoff produced by **Basins OS4, OS5 and B**, has been calculated to be 2.6 cfs in the 5-year storm event and 17.2 cfs in the 100-year storm event. Runoff from these basins is conveyed as sheet flow to the south and is captured by an existing swale on the southern property boundary and routed west towards **Design Point 3**. This runoff outfalls into the East Fork Sand Creek Sub-tributary.

Design Point 4 consists of native grass covered un-platted land located at the northwest portion of the property boundary. Runoff produced by **Basins OS6 and C**, has been calculated to be 0.4 cfs in the 5-year storm event and 2.4 cfs in the 100-year storm event. Runoff from these basins is conveyed as sheet flow to the northwest towards **Design Point 4**. This runoff outfalls into the East Fork Sand Creek Sub-tributary.

Design Point 5 consists of native grass covered un-platted land located at the west portion of the property boundary. Runoff produced by **Basins OS7, D, Design Point 3 and Design Point 4**, has been calculated to be 4.5 cfs in the 5-year storm event and 29.3 cfs in the 100-year storm event. Runoff from these basins is conveyed as sheet flow to the southwest towards **Design Point 5**. This runoff outfalls into the East Fork Sand Creek Sub-tributary. The cumulative runoff values are from the onsite flows and do not include the East Fork Sand Creek Sub-tributary upstream flows. The values provided by FEMA for Sand Creek East Fork Subtributary at confluence with Sand Creek East Fork are 1970 cfs for the 100 year event.

PROPOSED DRAINAGE CHARACTERISTICS

General Proposed Conditions Drainage Discussion

The proposed storage yard development and improvements will be constructed on approximately 14.6 acres of the 19.362 acre parcel. The majority of the site has been accounted for as a storage yard and the remaining northern portion being considered as pastureland/undeveloped is shown on the Proposed Drainage Map. Refer to the Proposed Drainage Map for weighted runoff coefficients for the site. Proposed drainage patterns generally remain consistent with those in the existing condition with surface runoff traveling north to south. A swale is proposed on the western boundary of the site to capture and route runoff south to the Full Spectrum Detention (FSD) pond. The combined onsite runoff, offsite runoff from Timberline Storage Yard, and adjacent Basins OS3 & OS4 are conveyed to the proposed FSD pond. The runoff reaching the pond will be detained and discharged via a staged outlet structure and proposed 24" RCP storm system to the East Fork Sand Creek Sub-Tributary below historic rates. The outfall into the East Fork Sand Creek Sub-Tributary channel is armored with a proposed riprap pad and is grading away from main flows within the channel. Type M riprap protection is also proposed to stabilize the emergency spillway bank and all proposed grading around the outfall. Refer to the Proposed Drainage Map in the appendix for an illustration of the proposed site drainage patterns. A detailed description of the proposed drainage characteristics follows:

Proposed Conditions Detailed Drainage Discussion

Design Point 1, 2.59 acres, consists of offsite **Basins OS1** and onsite **Basin A**. Surface runoff, from these contributing areas, generally flows from north to south and exits the site at **Design Point 1**, where it combines with flows from neighboring Timberline Storage Yard and ultimately enters the Full Spectrum Detention Pond 1 on the Timberline Storage Yard site. The Proposed Drainage Map overlays Timberline Basin OS2 boundary in a gray dashed line type. Any discrepancy is minor, and within tolerance, with no significant drainage affects and changes to runoff values. The "Amendment No.1 for the Final Drainage Report for Timberline Storage Yard", dated March 2020, by M&S Civil Consultants, Inc. accounts for these flows and the Full Spectrum Detention Pond 1 was designed with consideration to this undeveloped tributary area (See Timberline Storage Yard Proposed Drainage Map, Basin OS2, referenced in the appendix). Onsite Basin A is to remain undeveloped at this time and runoff values are $Q_5=4.1$ cfs and $Q_{100}=9.3$ cfs have been calculated for **DP1**. The Timberline pond has been built.

Design Point 2, 2.89 acres, consists of offsite **Basin OS2** and onsite **Basin B**. Developed runoff of $Q_5=7.2$ cfs and $Q_{100}=14.4$ cfs has been calculated for **DP2**. Runoff from the existing neighboring Timberline Storage Yard improvements (parking lot, gravel lot, building) within offsite Basin OS3 travels as sheet flow west to onsite Basin B. All tributary runoff will be routed via a 6' wide pan, curb and gutter and collected by a 10' CDOT Type R sump inlet located at a low point at **DP2**. Captured flows are routed to the east forebay of the FSD pond and are treated before being discharged to the East Fork Sand Creek Sub-Tributary.

Design Point 3, 2.74 acres, consists of onsite **Basin B1**. Developed runoff of $Q_5=5.9$ cfs and $Q_{100}=11.7$ cfs has been calculated for **DP3**. Existing and proposed grading conveys runoff south (to **DP3**) where a localized swale captures all sheet and concentrated flows conveying them south into the FSD pond via a 3' wide trapezoidal riprap rundown. Energy associated with the runoff is dissipated by the trapezoidal rundown and outfalls into a concrete forebay located at the bottom of the rundown. All runoff tributary to **DP3** is treated within the FSD pond prior to being discharged to the East Fork Sand Creek Sub-Tributary.

Design Point 4, 10.67 acres, consists of onsite **Basins B2** and **Basin D**. Developed runoff of $Q_5=20.2$ cfs and $Q_{100}=40.2$ cfs has been calculated for **DP4**. A proposed swale along the western property boundary prevents any runoff produced within **Basin D** from traveling offsite. A proposed swale located along the north side of the larger proposed building routes runoff west where it combines with flows in the aforementioned swale prior to reaching **DP4**. At **DP4**, a 3' wide trapezoidal riprap rundown conveys concentrated flows south into the FSD pond. Energy associated with the runoff is dissipated by the trapezoidal rundown and outfalls into a concrete forebay located at the bottom of the rundown. All runoff tributary to **DP4** is treated within the FSD pond prior to being discharged to the East Fork Sand Creek Sub-Tributary.

Design Point 5, 1.34 acres, consists of **Basin OS4** and **Basin B3**. Developed runoff from Design Points **DP2**, **DP3**, **DP4**, offsite **Basins OS4** and **Basin B3** contribute to the proposed FSD pond at **DP5** at a peak flow rate of $Q_5=31.7$ cfs, $Q_{100}=64.7$ cfs. The proposed full spectrum detention pond was sized utilizing the UDFCD MFHD-Detention Worksheet, Ver 4.03. Based upon the contributing watershed acreage, characteristics, and imperviousness of the site, the pond requires a minimum of 2.296 acre feet of storage in the 100-year event and was limited to a peak discharge of 17.4cfs. A proposed outlet structure with a 24" RCP storm pipe and restrictor plate is recommended to meet the allowable discharge rate. All treated

discharge from the FSD pond is released to the East Fork Sand Creek Sub-Tributary via a flared end section and riprap pad to dissipate energy and maintain integrity of the outfall. The low tailwater basin, $D_{50} = 12''$, riprap and flared end section have been graded into the channel bank away from higher velocity main channel flows. In the case that the outlet structure were to become clogged, 100 year flows will be released through a 25' wide emergency spillway and rundown armored with Type M riprap, $D_{50} = 12''$. The emergency spillway is designed with a foot of freeboard in the 100-year event and has a crest elevation of 6533.13. This crest elevation is 5' above the 100-year FEMA water surface elevation of 6528.0. A hydraulic analysis for the FSD pond outlet structure and 24" RCP was performed in StormCAD and is enclosed in the appendix. The FEMA 100-year water surface elevation of 6528.0 was used as the starting hydraulic grade line (HGL) elevation. The results show that the FSD pond remains functional in the 100-year event and the outlet structure is able to discharge flows to the East Fork Sand Creek Sub-Tributary.

Design Point 6, 1.44 acres, consists of offsite **Basin OS5** and **Basin C**. Developed runoff of $Q_5 = 0.5$ cfs and $Q_{100} = 3.2$ cfs has been calculated for **DP6**. As shown on the Proposed Drainage Map, runoff from **DP6** enters the East Fork Sand Creek Sub-Tributary channel and continues south. The FEMA 100 yr floodplain, Zone AE, has been overlayed on the Proposed Drainage Map and crosses the northwest corner of the site within Basin C. Tract A within the plat has been dedicated to this area and no development is anticipated to occur within the floodplain, per plat restriction.

Design Point 7, 4.64 acres, consists of **Basin OS6**. Developed runoff from Design Point **DP6** and Pipe Run **PR2** contribute to the existing East Fork Sand Creek Sub-tributary at **DP7** at a peak flow rate of $Q_5 = 2.3$ cfs, $Q_{100} = 29.7$ cfs. The flows released do not exceed the flows calculated in the historic condition. The cumulative runoff values are from the onsite flows and do not include the East Fork Sand Creek Sub-tributary upstream flows. The FEMA 100 yr floodplain, Zone AE, has been overlayed on the Proposed Drainage Map along with the associated cross section IDs and water surface elevation markers. No grading is proposed within the floodplain with the exception of the emergency spillway from the proposed FSD pond and some cut around the FSD pond outfall to grade in the 24" RCP flared end section and riprap pad. Riprap is proposed in these areas to protect the FSD pond and all storm improvements discharging from the FSD pond to the East Fork Sand Creek Sub-Tributary channel. A floodplain development permit is included in the appendix on this report.

Basin OS3, 0.46 acres, consists of 4:1 vegetated slope with runoff of $Q_5 = 0.2$ cfs and $Q_{100} = 1.1$ cfs has been calculated for **Basin OS3**. As shown on the Proposed Drainage Map, runoff from **Basin OS3** is released as sheet flow.

WATER QUALITY PROVISIONS AND MAINTENANCE

The proposed full spectrum detention (FSD) pond functions to provide detention and water quality for the proposed development. This full spectrum detention pond will function to treat approximately 19.362 acres of tributary area by providing 0.453 acre-feet of storage for the water quality event, 1.519 acre feet of storage at the EURV event storm and 2.296 acre-feet of storage in the 100-year event. The 25' wide emergency spillway is designed with a foot of freeboard in the 100-year event and has a crest elevation of 6533.13. This crest elevation is 5' above the 100-year FEMA water surface elevation of 6528.0. A hydraulic analysis for the FSD pond outlet structure and 24" RCP was performed and is enclosed in the appendix (Storm Cad). The FEMA 100-year water surface elevation of 6528.0 was used as the starting

hydraulic grade line (HGL) elevation. The results show that the FSD pond remains functional in the 100-year event and the outlet structure is able to discharge flows to the East Fork Sand Creek Sub-Tributary. The sizing for the full spectrum detention facility has been determined using the guidelines set forth in the Urban Drainage and Flood Control District Criteria Manual. Refer to the UDFCD MHFD-Detention, Version 4.03, Excel Workbook located within the appendix of this report for calculations.

The proposed FSD pond will be private and shall be maintained by the property owner. Access shall be granted to the owner and El Paso County for access and maintenance of the private WQCV facility. A private maintenance agreement document shall accompany this report submittal.

EROSION CONTROL

It is the policy of the El Paso County that we submit a grading and erosion control plan with the drainage report. Proposed silt fence, vehicle traffic control, and concrete washout area are proposed as erosion control measures. The costs for these measures have been provided on the Grading and Erosion Control plan.

CONSTRUCTION COST OPINION

Private Drainage Facilities (**NON-Reimbursable**):

| Item | Description | Quantity | Unit Cost | Cost |
|-----------------|--|----------|--------------|--------------------|
| 1. | 18" RCP | 22 LF | \$40 /LF | \$880.00 |
| 2. | 24" RCP | 42 LF | \$50 /LF | \$2,100.00 |
| 3. | 18" RCP FES | 1 EA | \$800 /EA | \$800.00 |
| 4. | 24" RCP FES | 1 EA | \$900 /EA | \$900.00 |
| 5. | 10' CDOT Type R Inlet | 1 EA | \$5,000 /EA | \$5,000.00 |
| 6. | Type M riprap, 2' deep | 255 CY | \$65 /CY | \$16,575.00 |
| 7. | FSD Pond (Including Outlet Struct, Access Road, Trickle Channel, Forebays) | 1 EA | \$23,000 /EA | \$23,000.00 |
| Total \$ | | | | \$49,255.00 |

M & S Civil Consultants, Inc. (M & S) cannot and does not guarantee the construction cost will not vary from these opinions of probable costs. These opinions represent our best judgment as design professionals familiar with the construction industry and this development in particular. The above and below is only an estimate of the facility cost and drainage basin fee amounts in 2020.

DRAINAGE & BRIDGE FEES – DWIRE STORAGE YARD

This site is within the Sand Creek Drainage Basin. The 2020 (Resolution 19-441) Drainage and Bridge Fees per El Paso County for the DWIRE Storage Yard Filing No. 1 site are as follows:

Per DWIRE Storage Yard Filing No. 1 Plat –
Tract A (No build per Plat restriction, within floodplain)-

19.362 Acres
-0.432 Acres
Total Area 18.93 Acres

DWIRE STORAGE YARD FILING NO. 1 FEES:

| | | | | | | | |
|-----------------------|-------|---|-------|----|-----------|--------------|----------------------|
| Drainage Fees: | 18.93 | x | 67.1% | \$ | 19,698.00 | = | \$ 250204.59 |
| Bridge Fees: | 18.93 | x | 67.1% | \$ | 8,057.00 | = | <u>\$ 102,340.26</u> |
| | | | | | | Total | \$ 352,544.85 |

SUMMARY

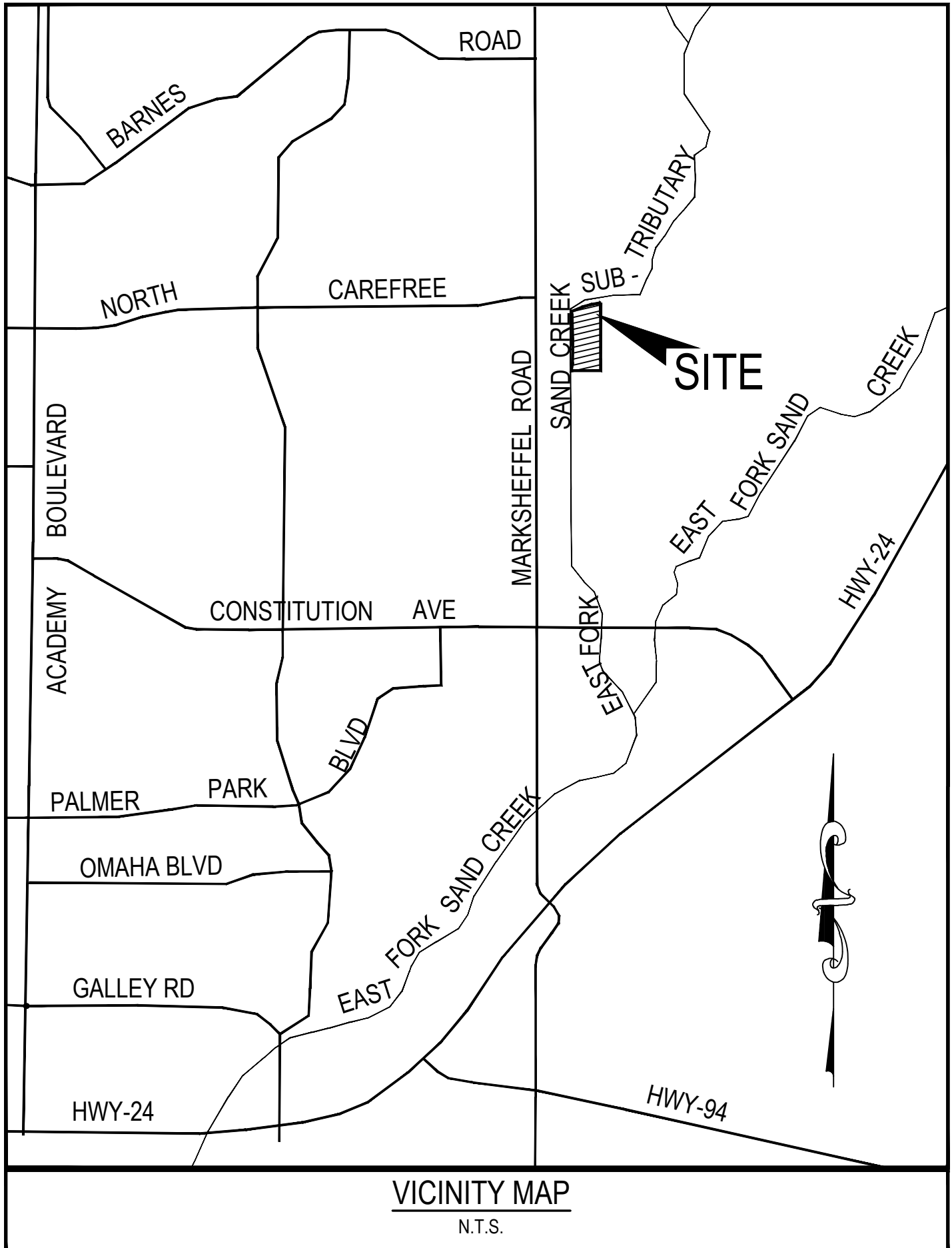
Per this final drainage report, the proposed drainage facilities recommended within this report will adequately convey, detain and route runoff from the planned development to the East Fork Sand Creek Sub-Tributary drainage way at peak flow rates which are below existing with no negative impacts on surrounding developments. All drainage facilities described herein and shown on the included Proposed Drainage Map (See Appendix) are subject to change due to formal design considerations during the construction document preparation stage (simultaneous review of Proposed Drainage Report and Construction Documents). Care will be taken to accommodate overland emergency flow routes on site and temporary drainage conditions. The development of the DWIRE Storage Yard Filing No.1 site will not adversely affect adjacent or downstream properties.

REFERENCES

- 1.) "El Paso County and City of Colorado Springs Drainage Criteria Manuals".
- 2.) "Urban Storm Drainage Criteria Manual"
- 3.) SCS Soils Map for El Paso County.
- 4.) Flood Insurance Rate Map (FIRM), Federal Emergency Management Agency (Map No. 08041C0543F), Effective date March 17, 1997.
- 5.) Flood Insurance Rate Map (FIRM), Federal Emergency Management Agency (Map No. 08041C0543G), Effective date December 7, 2018.
- 6.) "Amendment No.1 to the Final Drainage Report for Timberline Storage Yard", dated March 2020, by M&S Civil Consultants, Inc.
- 7.) "Sand Creek Drainage Basin Planning Study, Preliminary Design Report", Revised March 1996, by Kiowa Engineering Corporation.

APPENDIX

VICINITY MAP



SOILS MAP



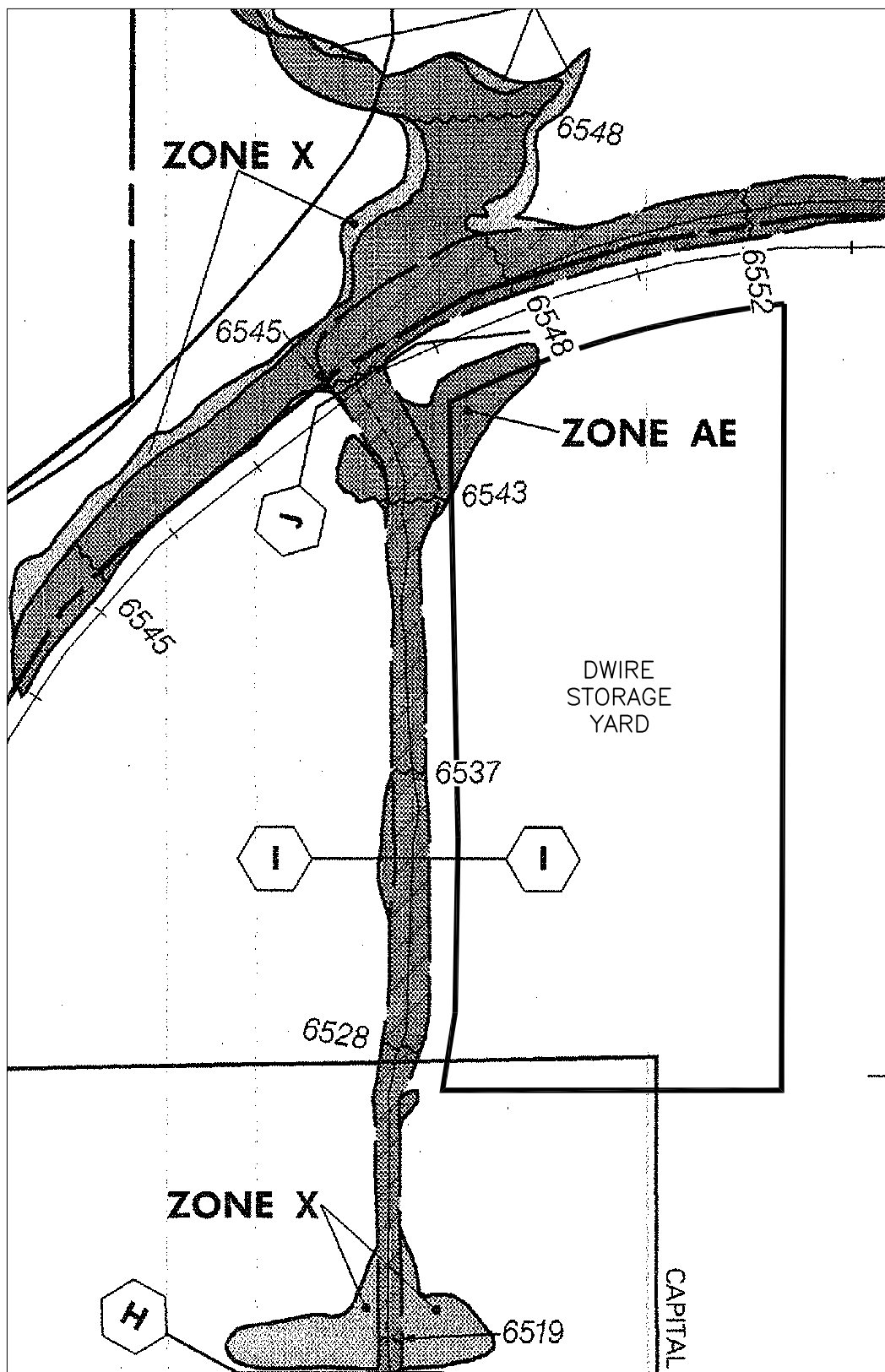
NOT TO SCALE



Tables — Hydrologic Soil Group — Summary By Map Unit

| Summary by Map Unit — El Paso County Area, Colorado (CO625) | | |
|---|---|--------|
| Summary by Map Unit — El Paso County Area, Colorado (CO625) | | |
| Map unit symbol | Map unit name | Rating |
| 10 | Blendon sandy loam, 0 to 3 percent slopes | B |

FIRM PANEL



NOT TO SCALE

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

EL PASO COUNTY,
COLORADO AND
INCORPORATED AREAS




PANEL 543 OF 1300
(SEE MAP INDEX FOR PANELS NOT PRINTED)

| CONTAINS: | NUMBER | PANEL | SUFFIX |
|--------------------------|--------|-------|--------|
| COLORADO SPRINGS CITY OF | 08001 | 0849 | F |
| EL PASO COUNTY | 08008 | 0853 | F |
| UNINCORPORATED AREAS | 08009 | 0853 | F |

MAP NUMBER
08041C0543 F

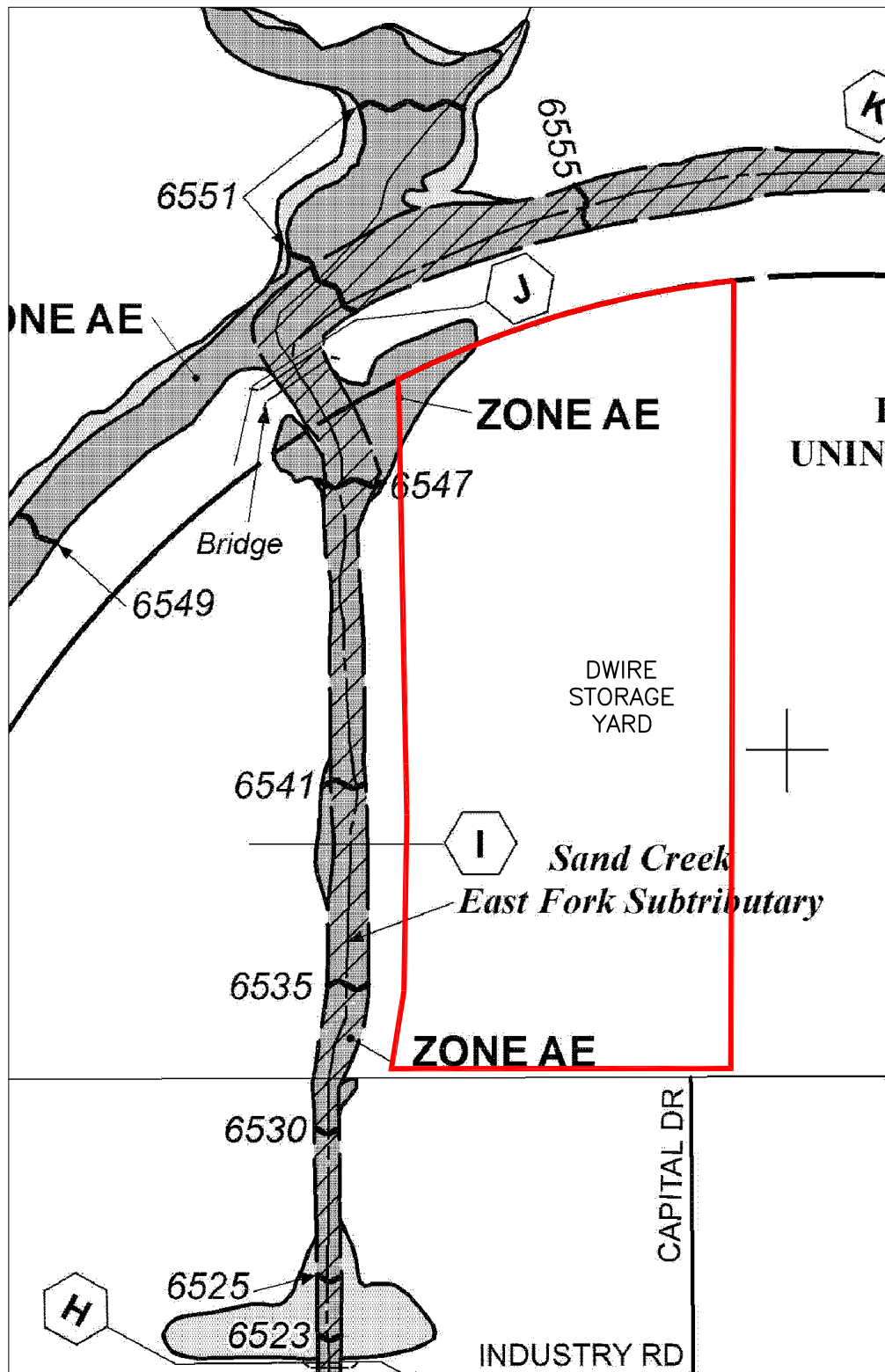
EFFECTIVE DATE:
MARCH 17, 1997

Federal Emergency Management Agency

-  ZONE 'X' - 500 YEAR FLOODPLAIN
-  ZONE 'AE' - 100 YEAR FLOODPLAIN
-  FLOODWAY AREAS IN ZONE 'AE'

DWIRE STORAGE YARD
FLOODPLAIN MAP





NOT TO SCALE

FIRM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY,
COLORADO
AND INCORPORATED AREAS

PANEL 543 OF 1300

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

| COMMUNITY | NUMBER | PANEL | SUFFIX |
|---------------------------|--------|-------|--------|
| COLORADO SPRINGS, CITY OF | 00000 | 0543 | 0 |
| EL PASO COUNTY | 00000 | 0543 | 0 |




Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.



MAP NUMBER
08041C0543G

MAP REVISED
DECEMBER 7, 2018

Federal Emergency Management Agency

-  ZONE 'X' — 500 YEAR FLOODPLAIN
-  ZONE 'AE' — 100 YEAR FLOODPLAIN
-  FLOODWAY AREAS IN ZONE 'AE'

DWIRE STORAGE YARD
FLOODPLAIN MAP





COLORADO
Division of Water Resources
Department of Natural Resources
Water Division 2 - Main Office

January 31, 2020

Tim Emick
8110 Opportunity View
Colorado Springs, CO 80939

Via Email temick@timberlinelandscaping.com

When replying, please refer to:
**DWIRE Storage Yard DAM Non-Jurisdictional
Water Division 2 Water District 10**

SUBJECT: Signed Notice of Intent to Construct a Non-Jurisdictional Water Impoundment Structure

Dear Mr. Emick:

Our office is in receipt of a Notice of Intent (NOI) to Construct a Non-Jurisdictional Water Impoundment Structure for the subject dam. The impoundments are to be located adjacent to East Fork of Sand Creek tributary to Fountain Creek, with the filling source to be Stormwater for Temporary Detention.

In accordance with Rule 11.1 of the Colorado Rules and Regulations for Dam Safety and Dam Construction, the hazard of this dam has been assessed as Low based on the construction drawing plans submitted with the NOI. A copy of the signed NOI is attached. An electronic copy will be maintained with the Division of Water Resources.

Please note the following:

- Location information has been corrected on your submission.
- This structure must be designed and constructed to standards outlined in 37-92-602(8) for stormwater detention facilities.
- Because this structure is located on a tributary to Fountain Creek, the structure can only operate pursuant a Colorado Discharge Permit System Municipal, Separate Storm Sewer System Permit issued by the Department of Public Health and Environment Pursuant to Article 8 of Title 25, C.R.S.
- In the event groundwater is encountered during construction of the pond, the pond must be backfilled so as not to expose groundwater until such time as: 1) a well permit has been obtained for the groundwater pond pursuant to CRS §37-90-137, or 2) the pond is lined in accordance with the document, ["State Engineer Guidelines for Lining Criteria for Gravel Pits,"](#) dated August 1999.



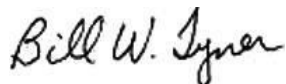
The requirements and recommendations provided herein are based on our review of the safety and water administration aspects of the proposed dam and the information provided in the submitted NOI. These requirements and recommendations create no liability for the State of Colorado should the dam fail for any reason. Please be aware that it is in the owner's best interest to construct, operate, and maintain the structure in a safe manner, as he or she may be held liable in civil court for any downstream damages resulting from failure of the dam. A copy of Specifications for Construction of Non-Jurisdictional Dams is provided to assist you in the construction of a sound structure.

Finally, please be aware of any other permitting or regulatory requirements associated with the construction of a water impoundment structure, including but not limited to county and/or municipal regulations and wetland permitting through the U.S. Army Corps of Engineers (see www.usace.army.mil for regional contact information).

The plans reviewed in this determination are submitted as part of the Developmental Approval process. Prior to the operation of this structure, please provide notice of completion of construction and as-constructed plans in PDF form including as constructed Stormwater Detention and Infiltration Data Sheet. Additionally, prior to the operation of this structure, notice must be provided pursuant to 37-92-602(8)(d) to the substitute water supply plan notification list maintained by the state engineer pursuant to section 37-92-308 (6) for the water division in which the facility is located.

If you have any questions regarding this approval, please contact Water Commissioner, Doug Hollister, at (719) 227-5291 or via email to doug.hollister@state.co.us, or Dam Safety Engineer, John Hunyadi, at (719)-227-5294, or via email to john.hunyadi@state.co.us,

Sincerely,



Bill W. Tyner, P.E.
Division Engineer, Division 2

Enc: Signed Notice of Intent to Construct a Non-Jurisdictional Water
Impoundment Structure
Specifications for Construction of Non-Jurisdictional Dams
Completion of Construction Form

ec: John Hunyadi, P.E., Dam Safety Engineer
Doug Hollister, District 10 Water Commissioner

Laserfiche File



COLORADO
Division of Water Resources
Department of Natural Resources

COMPLETION OF CONSTRUCTION

DAM ID. 100541

Upon the completion of the DWIRE Storage Yard DAM, Subject of the Notice of Intent to Construct a Non-Jurisdictional Dam under the Receipt above, location in Sec. 28 Twp. 13 S Rng. 65 W; UTM NAD83 , Northing 4304006, Easting 527860.

Indicate in the blank at the bottom of this form, the date of completion of construction and return to:

Colorado Division of Water Resources
Division 2
310 East Abriendo Ave, Suite B
Pueblo, CO 81004

Date of Completion: _____

Signature of Owner Date





NON-JURISDICTIONAL WATER IMPOUNDMENT STRUCTURE¹

This notice is required per Section 37-87-125, C.R.S. (1998) and must be submitted to the Division Engineer's Office a minimum of 45 days prior to construction.

OWNER INFORMATION

Name: Tim Enick Telephone/E-Mail: (719) 499-1291 / TEMICK@TIMBERLINELANDSCAPING.COM
Address: 8110 OPPORTUNITY VIEW Colorado Springs CO 80939
Street / P.O. Box/ Rural Route City State Zip Code
Responsible Person: Tim Enick Telephone/E-Mail: () SAME /
Address: SAME
Street / P.O. Box/ Rural Route City State Zip Code
Contractor: SAME Telephone/E-Mail: () /

STRUCTURE INFORMATION

Name of Dam: DWIRE STORAGE YARD Water Division: 2 Water District: CS

Location: (Provide Section, Township, Range, and GPS Point taken at crest of dam above streamline/outlet)

- Section: 28, Township: 13 SOUTH, Range: 65 WEST P.M.
- Northing: 527860 meters, Easting: 4304006 meters (Datum should be UTM, NAD 83)

Dam Dimensions:

- Vertical Height²: 5.08 ft., Length: 85 ft., Crest Width: 17 ft., Slopes: U/S: 4:1 (H:1V), D/S: 4:1 (H:1V)

Reservoir:

- Surface Area¹: 0.7 acres, Capacity¹: 1.619 acre-feet, Drainage Area*: 19.362 acres
*(If drainage area is unknown leave blank and a spillway size will be assigned):

Emergency Spillway: (See Table 1, Spillway Sizing Guidelines)

- Bottom Width: 15' ft., Side Slopes: 4:1 H:1V, Freeboard³: 0.95 ft

Outlet Conduit Type: RCP, Size: 24" inches, Location: WESTSIDE

Stream Name or Water Source⁴: SAND CREEK E. TRIB Proposed Water Use: STORM WATER

Water Court Case or WDID : _____
(Water District Identification Number)

Signature of Owner

Date

Office Use Only

DIVISION ENGINEER'S REQUIREMENTS:

Dam I.D. 100541

Signature of Division Engineer

Date

¹ A "Non-Jurisdictional Structure" is a dam creating a reservoir with a capacity of 100 acre-feet or less and a surface area of 20 acres or less and a vertical height (footnote 2) of 10 feet or less. Non-jurisdictional size dams are regulated and subject to the authority of the State Engineer consistent with sections 37-87-102 and 37-87-105 C.R.S.

² "Vertical Height" is measured from the elevation of the lowest point of the natural surface of the ground or the invert of the outlet conduit (whichever is lower) where that point occurs along the longitudinal centerline of the dam up to the crest of the emergency spillway of the dam.

³ "Freeboard" is the vertical distance from the bottom of spillway to the crest of the dam. Minimum Freeboard is 3 feet.

⁴ If construction in reservoir intercepts groundwater, a well permit is required. (Well permit applications can be found at www.water.state.co.us)

HYDROLOGIC CALCULATIONS

DWIRE STORAGE YARD

FINAL DRAINAGE REPORT DRAINAGE CALCULATIONS

(Existing Conditions Area Runoff Coefficient Summary)

| | | | PAVEMENT | | | ROOF | | | STORAGE YARD (GRAVEL) | | | LANDSCAPED/PASTURELAND | | | WEIGHTED | |
|-------|--------------------|--------------------|--------------|----------------|------------------|--------------|----------------|------------------|-----------------------|----------------|------------------|------------------------|----------------|------------------|----------------|------------------|
| BASIN | TOTAL AREA (Sq Ft) | TOTAL AREA (Acres) | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | C ₅ | C ₁₀₀ |
| OS1 | 39125.8 | 0.90 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.90 | 0.09 | 0.35 | 0.09 | 0.35 |
| OS2 | 6346.6 | 0.15 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.15 | 0.09 | 0.35 | 0.09 | 0.35 |
| OS3 | 55083.7 | 1.26 | 0.54 | 0.90 | 0.96 | 0.28 | 0.73 | 0.81 | 0.45 | 0.30 | 0.50 | 0.00 | 0.09 | 0.35 | 0.65 | 0.76 |
| OS4 | 7008.0 | 0.16 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.16 | 0.09 | 0.35 | 0.09 | 0.35 |
| OS5 | 15582.7 | 0.36 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.36 | 0.09 | 0.35 | 0.09 | 0.35 |
| OS6 | 64655.6 | 0.78 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.78 | 0.09 | 0.35 | 0.09 | 0.35 |
| OS7 | 198141.1 | 4.55 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 4.55 | 0.09 | 0.35 | 0.09 | 0.35 |
| A | 107676.5 | 2.47 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 2.47 | 0.09 | 0.35 | 0.09 | 0.35 |
| A1 | 13107.4 | 0.30 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.30 | 0.09 | 0.35 | 0.09 | 0.35 |
| B | 550809.9 | 12.64 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 12.64 | 0.09 | 0.35 | 0.09 | 0.35 |
| C | 28780.7 | 0.66 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.66 | 0.09 | 0.35 | 0.09 | 0.35 |
| D | 139029.7 | 3.19 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 3.19 | 0.09 | 0.35 | 0.09 | 0.35 |

Calculated by: DLM

Date: 11/19/2018

Checked by: VAS

DWIRE STORAGE YARD
FINAL DRAINAGE REPORT DRAINAGE CALCULATIONS
(Existing Conditions - Area Drainage Summary)

| From Area Runoff Coefficient Summary | | | | OVERLAND | | | | STREET / CHANNEL FLOW | | | | Time of Travel | | INTENSITY * | | TOTAL FLOWS | |
|--------------------------------------|---------------|----------------|------------------|----------------|--------|--------|----------------|-----------------------|-------|----------|----------------|----------------|-------|----------------|------------------|----------------|------------------|
| BASIN | AREA TOTAL | C ₅ | C ₁₀₀ | C ₅ | Length | Height | T _C | Length | Slope | Velocity | T _t | TOTAL | CHECK | I ₅ | I ₁₀₀ | Q ₅ | Q ₁₀₀ |
| | | | | | | | | | | | | | | | | | |
| OS1 | 0.90 | 0.09 | 0.35 | 0.09 | 150 | 4 | 17.1 | 300 | 1.0% | 1.0 | 5.0 | 22.1 | 12.5 | 2.9 | 4.9 | 0.2 | 1.6 |
| OS2 | 0.15 | 0.09 | 0.35 | 0.09 | 100 | 3 | 13.4 | 350 | 1.4% | 1.2 | 4.9 | 18.3 | 12.5 | 3.2 | 5.4 | 0.0 | 0.3 |
| OS3 | 1.26 | 0.65 | 0.76 | 0.65 | 50 | 3 | 5.1 | 540 | 1.0% | 2.0 | 4.5 | 9.6 | 13.3 | 4.2 | 7.0 | 3.4 | 6.8 |
| OS4 | 0.16 | 0.09 | 0.35 | 0.09 | 50 | 1 | 10.9 | 300 | 0.7% | 0.8 | 6.1 | 17.0 | 11.9 | 3.3 | 5.6 | 0.0 | 0.3 |
| OS5 | 0.36 | 0.09 | 0.35 | 0.09 | 100 | 1 | 19.3 | 325 | 3.7% | 1.9 | 2.8 | 22.1 | 12.4 | 2.9 | 4.9 | 0.1 | 0.6 |
| OS6 | 0.78 | 0.09 | 0.35 | 0.09 | 100 | 1 | 19.3 | 480 | 4.8% | 2.2 | 3.7 | 23.0 | 13.2 | 2.9 | 4.8 | 0.2 | 1.3 |
| OS7 | 4.55 | 0.09 | 0.35 | 0.09 | 100 | 1.5 | 16.9 | 1200 | 1.2% | 1.1 | 18.5 | 35.4 | 17.2 | 2.2 | 3.7 | 0.9 | 6.0 |
| A | 2.47 | 0.09 | 0.35 | 0.09 | 150 | 5 | 15.9 | 885 | 1.1% | 1.1 | 13.9 | 29.8 | 15.8 | 2.5 | 4.2 | 0.6 | 3.6 |
| AI | 0.30 | 0.09 | 0.35 | 0.09 | 50 | 1.5 | 9.5 | 460 | 3.7% | 2.9 | 2.7 | 12.2 | 12.8 | 3.8 | 6.4 | 0.1 | 0.7 |
| B | 12.64 | 0.09 | 0.35 | 0.09 | 180 | 6 | 17.4 | 1450 | 1.8% | 1.3 | 18.0 | 35.5 | 19.1 | 2.2 | 3.7 | 2.5 | 16.6 |
| C | 0.66 | 0.09 | 0.35 | 0.09 | 50 | 1 | 10.9 | 60 | 10.0% | 3.2 | 0.3 | 11.2 | 10.6 | 4.0 | 6.7 | 0.2 | 1.5 |
| D | 3.19 | 0.09 | 0.35 | 0.09 | 150 | 1.5 | 23.7 | 750 | 1.6% | 1.3 | 9.9 | 33.5 | 15.0 | 2.3 | 3.9 | 0.7 | 4.3 |

Calculated by: DLM
Date: 11/19/2018
Checked by: VAS

DWIRE STORAGE YARD
FINAL DRAINAGE REPORT DRAINAGE CALCULATIONS
(Existing Conditions - Basin Routing Summary)

| From Area Runoff Coefficient Summary | | | | OVERLAND | | | | CHANNEL FLOW | | | | Time of Travel (T _t) | INTENSITY * | | TOTAL FLOWS | | COMMENTS |
|---|------------------------|-----------------|-------------------|----------------|--------|--------|----------------|-------------------|-------|----------|----------------|----------------------------------|----------------|------------------|----------------|------------------|----------|
| DESIGN POINT | CONTRIBUTING BASINS | CA ₅ | CA ₁₀₀ | C _s | Length | Height | T _c | Length | Slope | Velocity | T _t | TOTAL | I ₅ | I ₁₀₀ | Q ₅ | Q ₁₀₀ | |
| | | | | | (ft) | (ft) | (min) | (ft) | (%) | (fps) | (min) | (min) | (in/hr) | (in/hr) | (c.f.s.) | (c.f.s.) | |
| PROPOSED DRAINAGE BASIN ROUTING SUMMARY | | | | | | | | | | | | | | | | | |
| 1 | OS1 OS2 A | 0.08 | 0.31 | | | | | | | | | | | | | | |
| | | 0.01 | 0.05 | | | | | | | | | | | | | | |
| | | 0.22 | 0.87 | | | | | | | | | | | | | | |
| | | 0.32 | 1.23 | | | | | Tc From Basin A | | | | 29.8 | 2.5 | 4.2 | 0.8 | 5.1 | |
| 2 | A1 OS3 | 0.03 | 0.11 | | | | | | | | | | | | | | |
| | | 0.82 | 0.97 | | | | | | | | | | | | | | |
| | | 0.85 | 1.07 | | | | | Tc From Basin OS3 | | | | 9.6 | 4.2 | 7.0 | 3.6 | 7.5 | |
| 3 | OS4 OS5 B | 0.01 | 0.06 | | | | | | | | | | | | | | |
| | | 0.03 | 0.13 | | | | | | | | | | | | | | |
| | | 1.14 | 4.43 | | | | | | | | | | | | | | |
| | | 1.18 | 4.61 | | | | | Tc From Basin B | | | | 35.5 | 2.2 | 3.7 | 2.6 | 17.2 | |
| 4 | OS6 C | 0.07 | 0.27 | | | | | | | | | | | | | | |
| | | 0.06 | 0.23 | | | | | | | | | | | | | | |
| | | 0.13 | 0.50 | | | | | Tc From Basin OS6 | | | | 23.0 | 2.9 | 4.8 | 0.4 | 2.4 | |
| 5 | DP3 DP4 OS7 D | 1.18 | 4.61 | | | | | | | | | | | | | | |
| | | 0.13 | 0.50 | | | | | | | | | | | | | | |
| | | 0.41 | 1.59 | | | | | | | | | | | | | | |
| | | 0.29 | 1.12 | | | | | | | | | | | | | | |
| | | 2.01 | 7.82 | | | | | Tc From Basin OS7 | | | | 35.4 | 2.2 | 3.7 | 4.5 | 29.3 | |

Calculated by: DLM
Date: 11/19/2018
Checked by: VAS

DWIRE STORAGE YARD
FINAL DRAINAGE REPORT DRAINAGE CALCULATIONS
(Proposed Conditions Area Runoff Coefficient Summary)

| BASIN | TOTAL AREA (Sq Ft) | TOTAL AREA (Acres) | PAVEMENT/CONC | | | ROOF | | | STORAGE YARD | | | PASTURELAND/UNDEVELOPED | | | WEIGHTED | |
|------------|--------------------------|--------------------------|-----------------|----------------|------------------|-----------------|----------------|------------------|-----------------|----------------|------------------|-------------------------|----------------|------------------|----------------|------------------|
| | | | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | C ₅ | C ₁₀₀ |
| OS1 | 6346.6 | 0.15 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.15 | 0.09 | 0.36 | 0.09 | 0.36 |
| OS2 | 65383.0 | 1.50 | 0.85 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.55 | 0.30 | 0.50 | 0.10 | 0.09 | 0.36 | 0.63 | 0.75 |
| OS3 | 19864.6 | 0.46 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.46 | 0.09 | 0.36 | 0.09 | 0.36 |
| OS4 | 5124.1 | 0.12 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.12 | 0.09 | 0.36 | 0.09 | 0.36 |
| OS5 | 64655.6 | 0.78 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.78 | 0.09 | 0.36 | 0.09 | 0.36 |
| OS6 | 202148.8 | 4.64 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 4.64 | 0.09 | 0.36 | 0.09 | 0.36 |
| A | 106469.9 | 2.44 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 1.90 | 0.59 | 0.70 | 0.54 | 0.09 | 0.36 | 0.48 | 0.62 |
| B | 60512.9 | 1.39 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 1.39 | 0.59 | 0.70 | 0.00 | 0.09 | 0.36 | 0.59 | 0.70 |
| B1 | 119490.8 | 2.74 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 2.74 | 0.59 | 0.70 | 0.00 | 0.09 | 0.36 | 0.59 | 0.70 |
| B2 | 325993.3 | 7.48 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 7.48 | 0.59 | 0.70 | 0.00 | 0.09 | 0.36 | 0.59 | 0.70 |
| B3 | 53002.6 | 1.22 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.59 | 0.70 | 1.22 | 0.16 | 0.41 | 0.16 | 0.41 |
| C | 28780.7 | 0.66 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 0.00 | 0.59 | 0.70 | 0.66 | 0.09 | 0.36 | 0.09 | 0.36 |
| D | 139029.7 | 3.19 | 0.00 | 0.90 | 0.96 | 0.00 | 0.73 | 0.81 | 3.19 | 0.59 | 0.70 | 0.00 | 0.09 | 0.36 | 0.59 | 0.70 |

Calculated by: GT
Date: 5/18/2020
Checked by: VAS

DWIRE STORAGE YARD
FINAL DRAINAGE REPORT DRAINAGE CALCULATIONS
(Proposed Conditions - Area Drainage Summary)

| From Area Runoff Coefficient Summary | | | | OVERLAND | | | | STREET / CHANNEL FLOW | | | | Time of Travel | | INTENSITY * | | TOTAL FLOWS | |
|--------------------------------------|---------------|----------------|--------------------|----------------|--------|--------|----------------|-----------------------|-------|----------|----------------|----------------|-------|----------------|------------------|----------------|------------------|
| BASIN | AREA TOTAL | C ₅ | C ₁₀₀ | C ₅ | Length | Height | T _C | Length | Slope | Velocity | T _t | TOTAL | CHECK | I ₅ | I ₁₀₀ | Q ₅ | Q ₁₀₀ |
| | | (Acres) | From DCM Table 6-6 | | | | | | | | | | | | | | |
| OS1 | 0.15 | 0.09 | 0.36 | 0.09 | 100 | 4 | 11.5 | 44 | 4.5% | 2.1 | 0.3 | 11.9 | 10.8 | 4.0 | 6.7 | 0.1 | 0.4 |
| OS2 | 1.50 | 0.63 | 0.75 | 0.63 | 50 | 1 | 4.8 | 47 | 4.0% | 4.0 | 0.2 | 5.0 | 10.5 | 5.2 | 8.7 | 4.9 | 9.8 |
| OS3 | 0.46 | 0.09 | 0.36 | 0.09 | 50 | 1 | 10.3 | 0 | 0.0% | 0.0 | 0.0 | 10.3 | 10.3 | 4.1 | 6.9 | 0.2 | 1.1 |
| OS4 | 0.12 | 0.09 | 0.36 | 0.09 | 100 | 1 | 18.2 | 200 | 2.5% | 1.6 | 2.1 | 20.3 | 11.7 | 3.9 | 6.5 | 0.0 | 0.3 |
| OS5 | 0.78 | 0.09 | 0.36 | 0.09 | 100 | 1 | 18.2 | 480 | 4.8% | 2.2 | 3.7 | 21.9 | 13.2 | 3.7 | 6.2 | 0.3 | 1.7 |
| OS6 | 4.64 | 0.09 | 0.36 | 0.09 | 100 | 1.5 | 16.0 | 1200 | 1.2% | 1.1 | 18.5 | 34.5 | 17.2 | 3.3 | 5.6 | 1.4 | 9.3 |
| A | 2.44 | 0.48 | 0.62 | 0.48 | 100 | 7 | 5.9 | 830 | 2.0% | 1.4 | 9.7 | 15.6 | 15.2 | 3.5 | 5.9 | 4.1 | 9.0 |
| B | 1.39 | 0.59 | 0.70 | 0.59 | 100 | 3 | 6.4 | 440 | 1.6% | 1.9 | 3.9 | 10.3 | 13.0 | 4.1 | 6.9 | 3.4 | 6.7 |
| B1 | 2.74 | 0.59 | 0.70 | 0.59 | 100 | 1 | 9.2 | 636 | 1.8% | 1.3 | 7.9 | 17.1 | 14.1 | 3.6 | 6.1 | 5.9 | 11.7 |
| B2 | 7.48 | 0.59 | 0.70 | 0.59 | 100 | 6 | 5.1 | 1435 | 1.9% | 2.1 | 11.6 | 16.7 | 18.5 | 3.2 | 5.4 | 14.1 | 28.2 |
| B3 | 1.22 | 0.16 | 0.41 | 0.16 | 30 | 6 | 3.5 | 447 | 0.5% | 1.4 | 5.3 | 8.7 | 12.7 | 4.3 | 7.3 | 0.8 | 3.6 |
| C | 0.66 | 0.09 | 0.36 | 0.09 | 53 | 2 | 8.6 | 54 | 25.0% | 5.0 | 0.2 | 8.7 | 10.6 | 4.3 | 7.3 | 0.3 | 1.7 |
| D | 3.19 | 0.59 | 0.70 | 0.59 | 100 | 5 | 5.4 | 905 | 1.7% | 1.3 | 11.7 | 17.1 | 15.6 | 3.5 | 5.8 | 6.5 | 13.0 |

Calculated by: GT
Date: 5/18/2020
Checked by: VAS

MARKSHEFFEL & CONSTITUTION
FINAL DRAINAGE REPORT DRAINAGE CALCULATIONS
(Proposed Conditions - Basin Routing Summary)

| From Area Runoff Coefficient Summary | | | | OVERLAND | | | | PIPE / CHANNEL FLOW | | | | Time of Travel (T _t) | INTENSITY * | | TOTAL FLOWS | | COMMENTS | |
|---|--------------------------------|-----------------|-------------------|----------------|--------|--------|----------------|---------------------|-------|----------|----------------|----------------------------------|----------------|------------------|----------------|------------------|----------|------|
| DESIGN POINT | CONTRIBUTING BASINS | CA ₅ | CA ₁₀₀ | C ₅ | Length | Height | T _c | Length | Slope | Velocity | T _t | TOTAL | I ₅ | I ₁₀₀ | Q ₅ | Q ₁₀₀ | | |
| | | | | | (ft) | (ft) | (min) | | | | | | | | | | | (ft) |
| PROPOSED DRAINAGE BASIN ROUTING SUMMARY | | | | | | | | | | | | | | | | | | |
| 1 | OS1 A | 0.01 | 0.05 | | | | | | | | | | | | | | | |
| | | 1.17 | 1.53 | | | | | | | | | | | | | | | |
| | | 1.18 | 1.58 | | | | | Tc From Basin A | | | | 15.2 | 3.5 | 5.9 | 4.1 | 9.3 | | |
| 2 | OS2 B | 0.94 | 1.13 | | | | | | | | | | | | | | | |
| | | 0.82 | 0.97 | | | | | | | | | | | | | | | |
| | | 1.76 | 2.10 | | | | | Tc From Basin B | | | | 10.3 | 4.1 | 6.9 | 7.2 | 14.4 | | |
| 3 | B1 | 1.62 | 1.92 | | | | | | | | | | | | | | | |
| | | 1.62 | 1.92 | | | | | Tc From Basin B1 | | | | 14.1 | 3.6 | 6.1 | 5.9 | 11.7 | | |
| | | | | | | | | | | | | | | | | | | |
| 4 | B2 D | 4.42 | 5.24 | | | | | | | | | | | | | | | |
| | | 1.88 | 2.23 | | | | | | | | | | | | | | | |
| | | 6.30 | 7.47 | | | | | Tc From Basin B2 | | | | 18.5 | 3.2 | 5.4 | 20.2 | 40.2 | | |
| 5 | DP2 DP3 DP4 OS4 B3 | 1.76 | 2.10 | | | | | | | | | | | | | | | |
| | | 1.62 | 1.92 | | | | | | | | | | | | | | | |
| | | 6.30 | 7.47 | | | | | | | | | | | | | | | |
| | | 0.01 | 0.04 | | | | | | | | | | | | | | | |
| | | 0.19 | 0.50 | | | | | | | | | | | | | | | |
| | | 9.88 | 12.03 | | | | | Tc From Basin DP4 | | | | 18.5 | 3.2 | 5.4 | 31.7 | 64.7 | | |
| 6 | OS5 C | 0.07 | 0.28 | | | | | | | | | | | | | | | |
| | | 0.06 | 0.24 | | | | | | | | | | | | | | | |
| | | 0.13 | 0.52 | | | | | Tc From Basin OS5 | | | | 13.2 | 3.7 | 6.2 | 0.5 | 3.2 | | |
| 7 | DP6 OS6 | 0.13 | 0.52 | | | | | | | | | | | | | | | |
| | | 0.42 | 1.67 | | | | | | | | | | | | | | | |
| | | 0.55 | 2.19 | | | | | Tc From Basin OS6 | | | | 17.2 | 3.3 | 5.6 | 1.8 | 12.2 | | |
| | PR2 | | | | | | | | | | | | | | | 0.5 | 17.5 | |
| | | | | | | | | | | | | | | | | 2.3 | 29.7 | |
| | | | | | | | | | | | | | Total | | | | | |

Calculated by: GT

Date: 5/18/2020

Checked by: VAS

DWIRE STORAGE YARD
FINAL DRAINAGE REPORT DRAINAGE CALCULATIONS
(Storm Sewer Routing Summary)

| <i>PIPE RUN</i> | <i>Contributing Pipes/Design Points</i> | <i>Equivalent CA₅</i> | <i>Equivalent CA₁₀₀</i> | <i>Maximum T_C</i> | <i>Intensity*</i> | | <i>Flow</i> | | PIPE SIZE |
|----------------------------|--|---|---|---|-----------------------------|-------------------------------|-----------------------------|-------------------------------|------------------|
| | | | | | <i>I₅</i> | <i>I₁₀₀</i> | <i>Q₅</i> | <i>Q₁₀₀</i> | |
| <i>1</i> | DP2 | 1.76 | 2.10 | 10.3 | 4.1 | 6.9 | <i>7.2</i> | <i>14.4</i> | 18" RCP |
| <i>2</i> | FSD Outlet Structure | UD-Detention, V 3.07, Routed Hydrograph Results | | | | | <i>0.5</i> | <i>17.4</i> | 24" RCP |

* Intensity equations assume a minimum travel time of 5 minutes.

DP - Design Point

EX - Existing Design Point

FB- Flow By from Design Point

INT- Intercepted Flow from Design Point

Calculated by: GT

Date: 5/18/2020

Checked by: VAS

HYDRAULIC CALCULATIONS / FSD POND CALCULATIONS

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

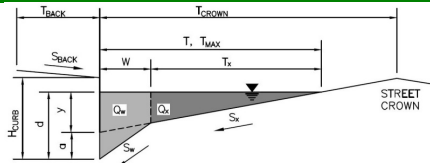
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Dwire Storage Yard

Inlet ID:

Inlet at DP2

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

 $T_{BACK} = 7.5$ ft

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

 $S_{BACK} = 0.020$ ft/ft

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

 $n_{BACK} = 0.018$

Height of Curb at Gutter Flow Line

 $H_{CURB} = 6.00$ inches

Distance from Curb Face to Street Crown

 $T_{CROWN} = 21.0$ ft

Gutter Width

 $W = 2.00$ ft

Street Transverse Slope

 $S_X = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

 $S_W = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

 $S_O = 0.000$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $n_{STREET} = 0.012$

Max. Allowable Spread for Minor & Major Storm

| | Minor Storm | Major Storm | |
|-------------|-------------|-------------|--------|
| $T_{MAX} =$ | 21.0 | 21.0 | ft |
| $d_{MAX} =$ | 5.6 | 7.8 | inches |

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

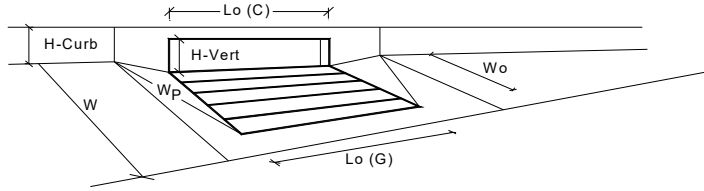
Check boxes are not applicable in SUMP conditions

**MINOR STORM Allowable Capacity is based on Depth Criterion****MAJOR STORM Allowable Capacity is based on Depth Criterion**

| | Minor Storm | Major Storm | |
|---------------|-------------|-------------|-----|
| $Q_{allow} =$ | SUMP | SUMP | cfs |

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



Design Information (Input)

Type of Inlet CDOT Type R Curb Opening

Local Depression (additional to continuous gutter depression 'a' from above)

Number of Unit Inlets (Grate or Curb Opening)

Water Depth at Flowline (outside of local depression)

Grate Information

Length of a Unit Grate

Width of a Unit Grate

Area Opening Ratio for a Grate (typical values 0.15-0.90)

Clogging Factor for a Single Grate (typical value 0.50 - 0.70)

Grate Weir Coefficient (typical value 2.15 - 3.60)

Grate Orifice Coefficient (typical value 0.60 - 0.80)

Curb Opening Information

Length of a Unit Curb Opening

Height of Vertical Curb Opening in Inches

Height of Curb Orifice Throat in Inches

Angle of Throat (see USDCM Figure ST-5)

Side Width for Depression Pan (typically the gutter width of 2 feet)

Clogging Factor for a Single Curb Opening (typical value 0.10)

Curb Opening Weir Coefficient (typical value 2.3-3.7)

Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)

Low Head Performance Reduction (Calculated)

Depth for Grate Midwidth

Depth for Curb Opening Weir Equation

Combination Inlet Performance Reduction Factor for Long Inlets

Curb Opening Performance Reduction Factor for Long Inlets

Grated Inlet Performance Reduction Factor for Long Inlets

Total Inlet Interception Capacity (assumes clogged condition)

Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)

| | MINOR | MAJOR | |
|-----------------------|--------------------------|-------|---|
| Type = | CDOT Type R Curb Opening | | |
| a_{local} = | 3.00 | 3.00 | inches |
| No = | 2 | 2 | |
| Ponding Depth = | 5.6 | 7.8 | inches |
| | MINOR | MAJOR | <input checked="" type="checkbox"/> Override Depths |
| $L_o (G)$ = | N/A | N/A | feet |
| W_o = | N/A | N/A | feet |
| A_{ratio} = | N/A | N/A | |
| $C_r (G)$ = | N/A | N/A | |
| $C_w (G)$ = | N/A | N/A | |
| $C_o (G)$ = | N/A | N/A | |
| | MINOR | MAJOR | |
| $L_o (C)$ = | 5.00 | 5.00 | feet |
| H_{vert} = | 6.00 | 6.00 | inches |
| H_{throat} = | 6.00 | 6.00 | inches |
| Theta = | 63.40 | 63.40 | degrees |
| W_p = | 2.00 | 2.00 | feet |
| $C_r (C)$ = | 0.10 | 0.10 | |
| $C_w (C)$ = | 3.60 | 3.60 | |
| $C_o (C)$ = | 0.67 | 0.67 | |
| | MINOR | MAJOR | |
| d_{Grate} = | N/A | N/A | ft |
| d_{Curb} = | 0.30 | 0.48 | ft |
| $RF_{Combination}$ = | 0.53 | 0.74 | |
| RF_{Curb} = | 0.91 | 1.00 | |
| RF_{Grate} = | N/A | N/A | |
| | MINOR | MAJOR | |
| Q_a = | 8.7 | 18.7 | cfs |
| $Q_{PEAK REQUIRED}$ = | 7.0 | 14.3 | cfs |

18" RCP INDEX MAP

Forebay into FSD Pond

PR1, Prop. 18" RCP

CDOT Type R Sump Inlet



Conduit FlexTable: POND 1- 18inch OUTFALL 100-YR

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) | Upstream Structure Hydraulic Grade Line (In) (ft) |
|---|---|----------------------------------|-------------------------------|------------------------------|-----------------------|--------------------|------------------------|---------------------|-----------------------|-----------------------------|------------------------------|--------------------------------|---------------------------------|---------------|---|
| PR1, Prop. 18" RCP | 88 | CDOT Type R Sump Inlet | 14.40 | 67.4 | 27.8 | 8.15 | 2.630 | 0.90 | 1.39 | 6,534.68 | 6,534.16 | 6,533.65 | 6,533.13 | 0.52 | 6,535.20 |
| Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description | | | | | | | | |
| 8.15 | 1.500 | 1.55 | 6,536.58 | 6,530.10 | 6,532.00 | 6,530.85 | Circle - 18.0 in | | | | | | | | |

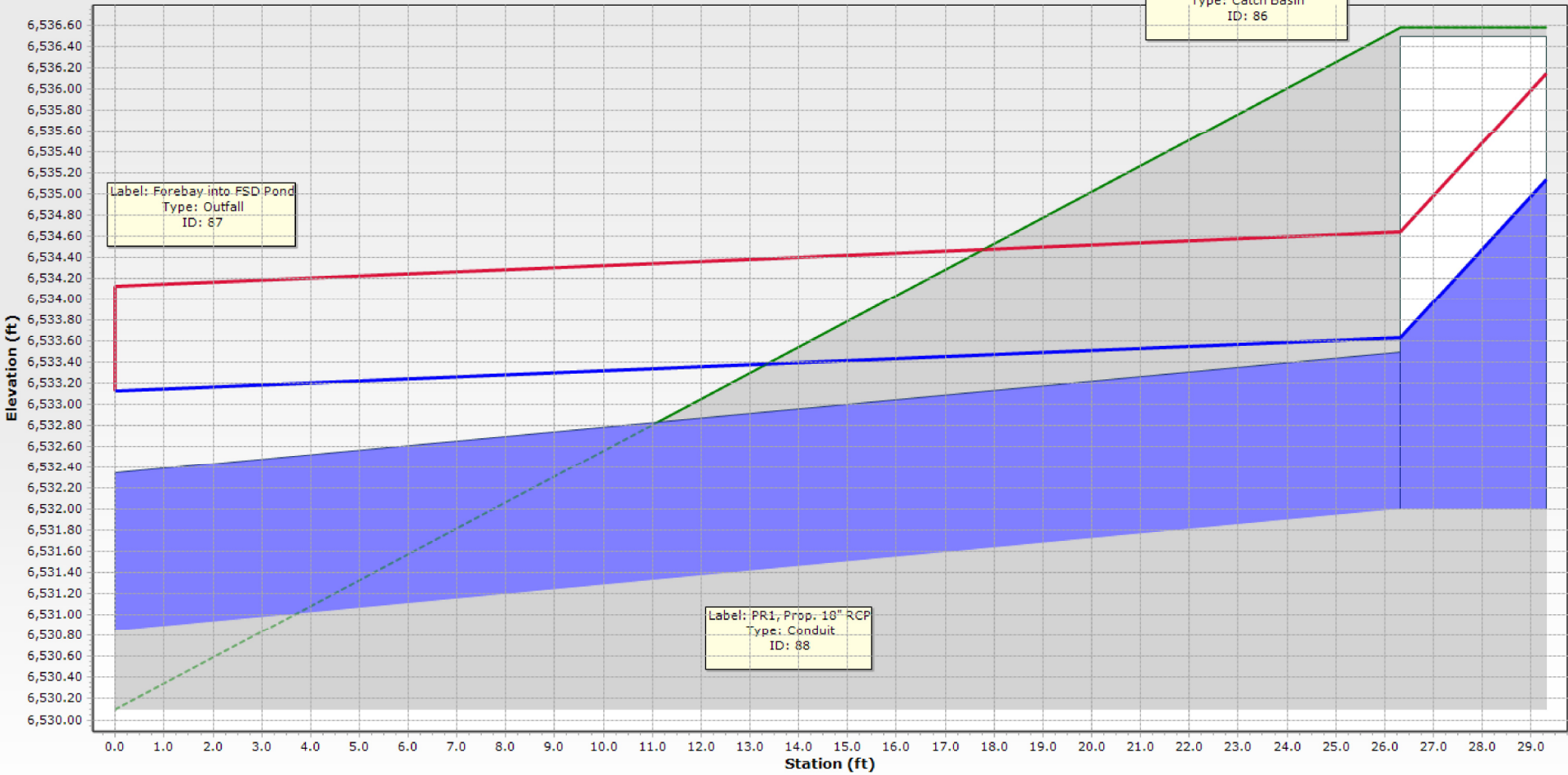
Pipe Run 1, 18" RCP - 100 yr Event

Label: CDOT Type R Sump Inlet
Type: Catch Basin
ID: 86

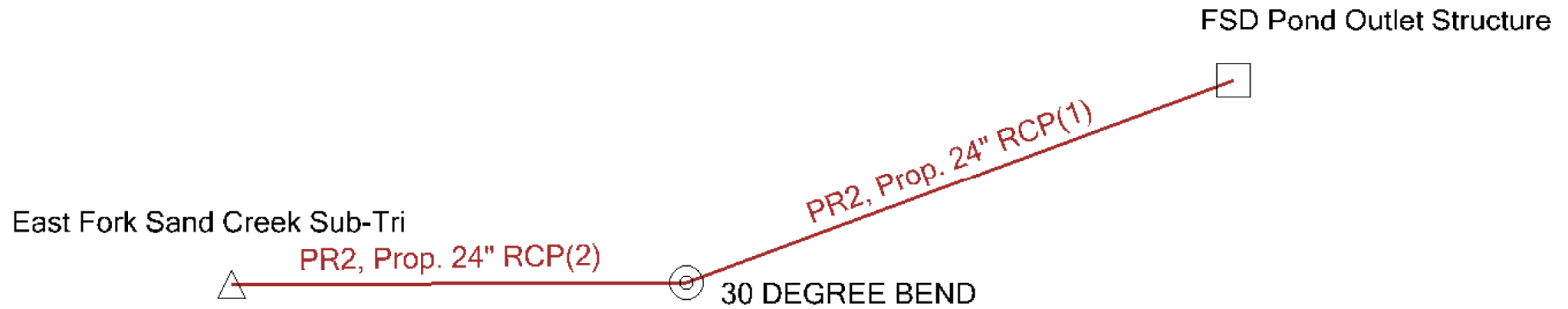
Label: Forebay into FSD Pond
Type: Outfall
ID: 87

Label: PR1, Prop. 18" RCP
Type: Conduit
ID: 88

EGL
HGL



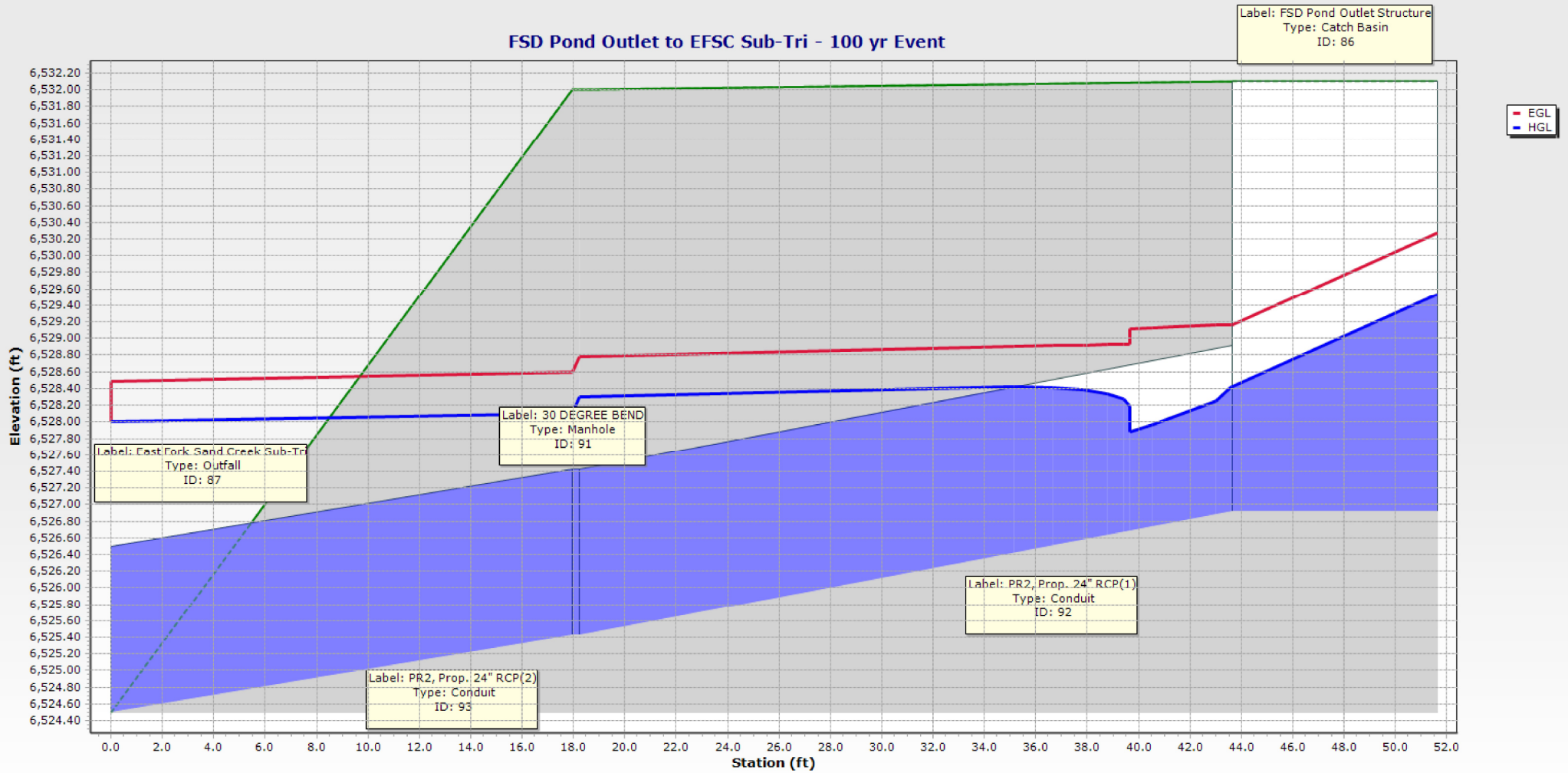
24" RCP INDEX MAP



Conduit FlexTable: POND 1- 24inch OUTFALL 100-YR

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) | Upstream Structure Hydraulic Grade Line (In) (ft) |
|---|---|----------------------------------|-------------------------------|------------------------------|-----------------------|--------------------|------------------------|---------------------|-----------------------|-----------------------------|------------------------------|--------------------------------|---------------------------------|---------------|---|
| PR2, Prop. 24" RCP(1) | 92 | FSD Pond Outlet Structure | 17.50 | 34.3 | 29.5 | 14.71 | 3.332 | 0.81 | 1.51 | 6,529.17 | 6,528.78 | 6,528.43 | 6,528.30 | 0.13 | 6,529.53 |
| PR2, Prop. 24" RCP(2) | 93 | 30 DEGREE BEND | 17.50 | 34.3 | 18.1 | 5.57 | 3.335 | 0.81 | 1.51 | 6,528.59 | 6,528.48 | 6,528.11 | 6,528.00 | 0.11 | 6,528.30 |
| Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description | | | | | | | | |
| 6.89 | 1.500 | 1.11 | 6,532.10 | 6,532.00 | 6,526.92 | 6,525.42 | Circle - 24.0 in | | | | | | | | |
| 5.57 | 0.400 | 0.19 | 6,532.00 | 6,524.50 | 6,525.42 | 6,524.50 | Circle - 24.0 in | | | | | | | | |

FSD Pond Outlet to EFSC Sub-Tri - 100 yr Event



| <i>Weighted Percent Imperviousness of Dwire Storage Yard Site</i> | | | | |
|---|-------------------------|----------------------|-------------------------|--------------------|
| <i>Contributing Onsite Basins</i> | <i>Area (Acres)</i> | <i>C_s</i> | <i>Impervious % (I)</i> | <i>(Acres)*(I)</i> |
| <i>A</i> | 2.44 | 0.48 | 40 | 97.77 |
| <i>B</i> | 1.39 | 0.59 | 80 | 111.13 |
| <i>B1</i> | 2.74 | 0.59 | 80 | 219.45 |
| <i>B2</i> | 7.48 | 0.59 | 80 | 598.70 |
| <i>B3</i> | 1.22 | 0.16 | 13 | 15.82 |
| <i>C</i> | 0.66 | 0.09 | 2 | 1.32 |
| <i>D</i> | 3.19 | 0.59 | 80 | 255.33 |
| <i>Totals</i> | 19.36 | | | 1299.53 |
| <i>Imperviousness of Site %</i> | 67.1 | | | |

MHFD-Detention, Version 4.03 (May 2020)

Basin ID: FSD Pond 1



| | | |
|---|-------|-----------------|
| Zone 1 Volume (WQCV) = | 0.453 | acre-feet |
| Zone 2 Volume (EURV - Zone 1) = | 1.065 | acre-feet |
| Zone 3 Volume (100-year - Zones 1 & 2) = | 0.746 | acre-feet |
| Total Detention Basin Volume = | 2.264 | acre-feet |
| Initial Surcharge Volume (ISV) = | user | ft ³ |
| Initial Surcharge Depth (ISD) = | user | ft |
| Total Available Detention Depth (H_{total}) = | user | ft |
| Depth of Trickle Channel (H_{TC}) = | user | ft |
| Slope of Trickle Channel (S_{TC}) = | user | ft/ft |
| Slopes of Main Basin Sides (S_{main}) = | user | H:V |
| Basin Length-to-Width Ratio ($R_{L/W}$) = | user | |

| Optional User Overrides | |
|-------------------------|-----------|
| 0.453 | acre-feet |
| 1.518 | acre-feet |
| 1.19 | inches |
| 1.50 | inches |
| 1.75 | inches |
| 2.00 | inches |
| 2.25 | inches |
| 2.52 | inches |
| | inches |

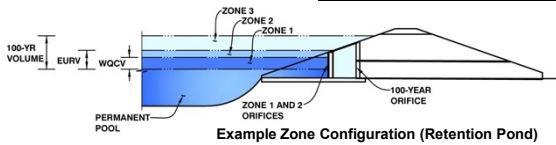
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DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.03 (May 2020)

Project: DWIRE Storage Yard

Basin ID: FSD Pond 1



Example Zone Configuration (Retention Pond)

| | Estimated Stage (ft) | Estimated Volume (ac-ft) | Outlet Type |
|-------------------|----------------------|--------------------------|----------------------|
| Zone 1 (WQCV) | 2.98 | 0.453 | Orifice Plate |
| Zone 2 (EURV) | 4.84 | 1.065 | Orifice Plate |
| Zone 3 (100-year) | 5.86 | 0.746 | Weir&Pipe (Restrict) |
| Total (all zones) | | 2.264 | |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = ft²
Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = inches
Orifice Plate: Orifice Area per Row = sq. inches (diameter = 1-9/16 inches)

Calculated Parameters for Plate
WQ Orifice Area per Row = ft²
Elliptical Half-Width = feet
Elliptical Slot Centroid = feet
Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (required) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|--------------------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|
| Stage of Orifice Centroid (ft) | 0.00 | 1.61 | 3.23 | | | | | |
| Orifice Area (sq. inches) | 1.91 | 1.91 | 1.91 | | | | | |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|--------------------------------|------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Stage of Orifice Centroid (ft) | | | | | | | | |
| Orifice Area (sq. inches) | | | | | | | | |

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = Not Selected Not Selected ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice = Not Selected Not Selected ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter = Not Selected Not Selected inches

Calculated Parameters for Vertical Orifice
Vertical Orifice Area = Not Selected Not Selected ft²
Vertical Orifice Centroid = Not Selected Not Selected feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

Overflow Weir Front Edge Height, H_o = Zone 3 Weir Not Selected ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length = 4.85 N/A feet
Overflow Weir Grate Slope = 8.00 N/A H:V
Horiz. Length of Weir Sides = 0.00 N/A feet
Overflow Grate Open Area % = 3.50 N/A %, grate open area/total area
Debris Clogging % = 70% N/A %

Calculated Parameters for Overflow Weir
Height of Grate Upper Edge, H_u = Zone 3 Weir Not Selected feet
Overflow Weir Slope Length = 4.85 N/A feet
Grate Open Area / 100-yr Orifice Area = 3.50 N/A feet
Overflow Grate Open Area w/o Debris = 12.82 N/A ft²
Overflow Grate Open Area w/ Debris = 19.60 N/A ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = Zone 3 Restrictor Not Selected ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter = 0.25 N/A inches
Restrictor Plate Height Above Pipe Invert = 24.00 N/A inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate
Outlet Orifice Area = Zone 3 Restrictor Not Selected ft²
Outlet Orifice Centroid = 1.53 N/A feet
Half-Central Angle of Restrictor Plate on Pipe = 0.56 N/A radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 6.00 ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = 25.00 feet
Spillway End Slopes = 4.00 H:V
Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway
Spillway Design Flow Depth = 0.78 feet
Stage at Top of Freeboard = 7.78 feet
Basin Area at Top of Freeboard = 1.01 acres
Basin Volume at Top of Freeboard = 3.99 acre-ft

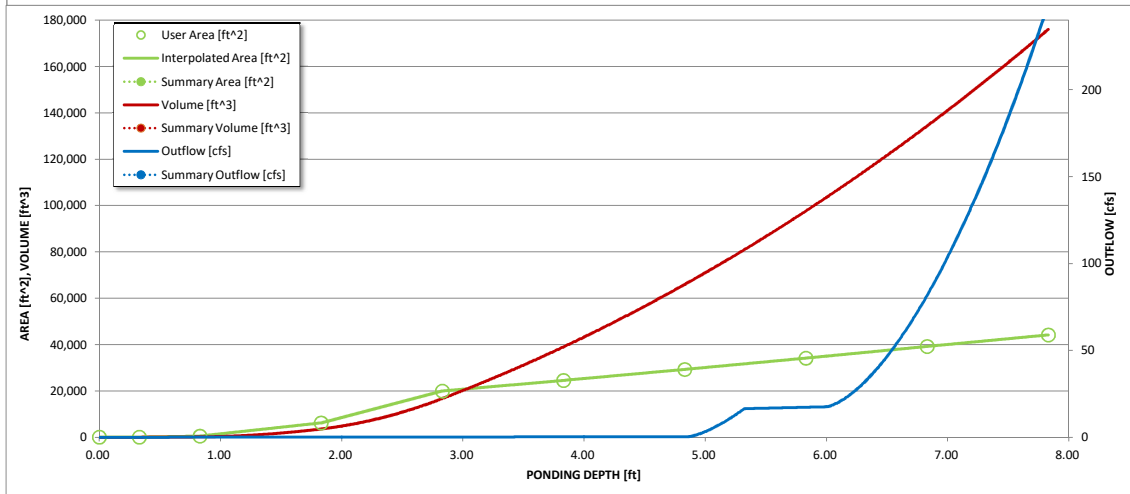
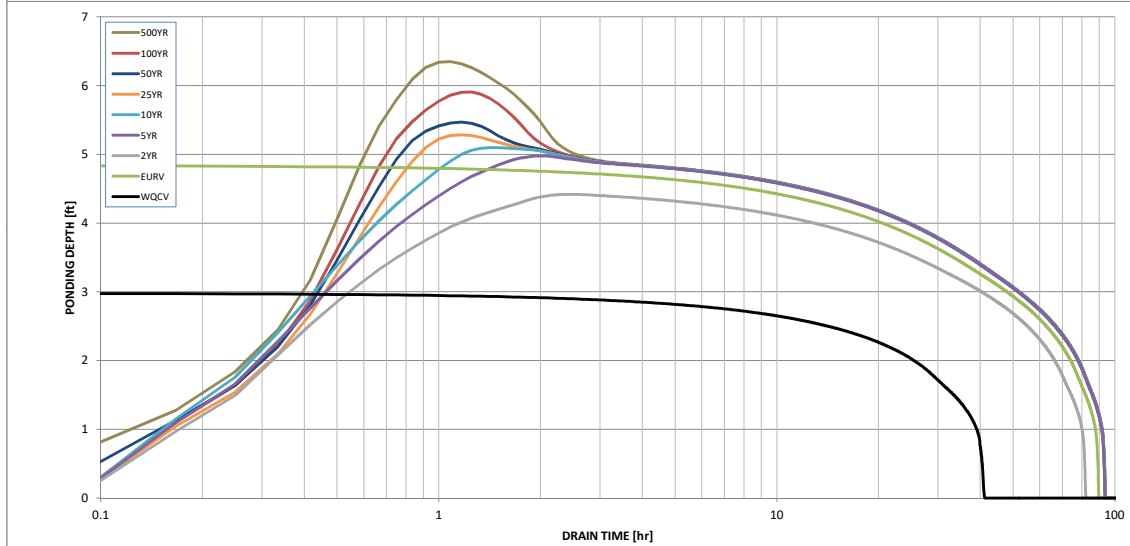
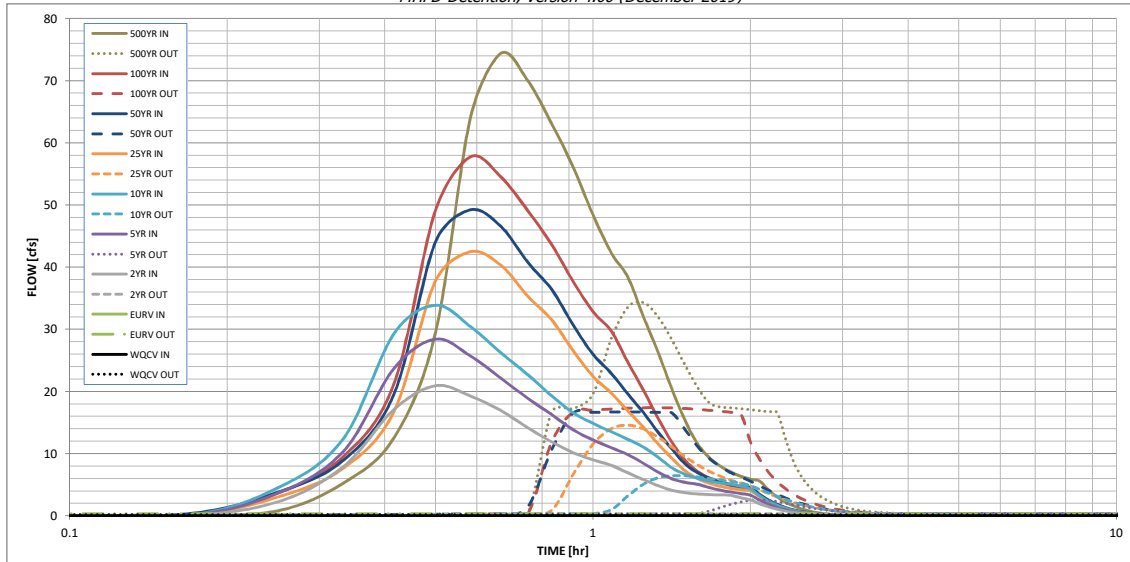
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

| | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year | 500 Year |
|---|-------|-------|--------|-----------------|-----------------|-----------------|----------------|----------------|----------|
| Design Storm Return Period | N/A | N/A | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 | 3.14 |
| One-Hour Rainfall Depth (in) | 0.453 | 1.518 | 1.297 | 1.760 | 2.153 | 2.626 | 3.040 | 3.537 | 4.589 |
| CUHP Runoff Volume (acre-ft) | N/A | N/A | 1.297 | 1.760 | 2.153 | 2.626 | 3.040 | 3.537 | 4.589 |
| Inflow Hydrograph Volume (acre-ft) | N/A | N/A | 1.8 | 5.1 | 7.8 | 14.1 | 17.7 | 22.6 | 31.6 |
| CUHP Predevelopment Peak Q (cfs) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| OPTIONAL Override Predevelopment Peak Q (cfs) | N/A | N/A | 0.10 | 0.27 | 0.40 | 0.73 | 0.91 | 1.17 | 1.63 |
| Predevelopment Unit Peak Flow, q (cfs/acre) | N/A | N/A | 20.9 | 28.4 | 33.9 | 42.5 | 49.2 | 57.8 | 74.4 |
| Peak Inflow Q (cfs) | 0.2 | 0.3 | 0.3 | 2.6 | 6.4 | 14.6 | 16.7 | 17.4 | 34.3 |
| Peak Outflow Q (cfs) | N/A | N/A | N/A | 0.5 | 0.8 | 1.0 | 0.9 | 0.8 | 1.1 |
| Ratio Peak Outflow to Predevelopment Q | Plate | Plate | Plate | Overflow Weir 1 | Overflow Weir 1 | Overflow Weir 1 | Outlet Plate 1 | Outlet Plate 1 | Spillway |
| Structure Controlling Flow | N/A | N/A | N/A | 0.1 | 0.3 | 0.7 | 0.8 | 0.9 | 0.9 |
| Max Velocity through Grate 1 (fps) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Max Velocity through Grate 2 (fps) | 38 | 82 | 75 | 84 | 83 | 81 | 80 | 79 | 76 |
| Time to Drain 97% of Inflow Volume (hours) | 40 | 87 | 79 | 90 | 89 | 88 | 87 | 86 | |
| Time to Drain 99% of Inflow Volume (hours) | 2.98 | 4.84 | 4.42 | 4.98 | 5.10 | 5.28 | 5.47 | 5.91 | 6.35 |
| Maximum Ponding Depth (ft) | 0.47 | 0.67 | 0.63 | 0.69 | 0.70 | 0.72 | 0.74 | 0.79 | 0.84 |
| Area at Maximum Ponding Depth (acres) | 0.453 | 1.519 | 1.240 | 1.608 | 1.691 | 1.826 | 1.958 | 2.296 | 2.656 |
| Maximum Volume Stored (acre-ft) | | | | | | | | | |

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.00 (December 2019)



| S-A-V-D Chart Axis Override | X-axis | Left Y-Axis | Right Y-Axis |
|-----------------------------|--------|-------------|--------------|
| minimum bound | | | |
| maximum bound | | | |

Stormwater Detention and Infiltration Design Data Sheet

Worksheet Protected

| | |
|-------------------|-------|
| Watershed Slope = | 0.021 |
|-------------------|-------|

| |
|-------|
| 0.021 |
|-------|

1535

19.36

67.1%

| |
|------|
| 0.0% |
|------|

| |
|--------|
| 100.0% |
|--------|

| |
|------|
| 0.0% |
|------|

User Input

[illegible]

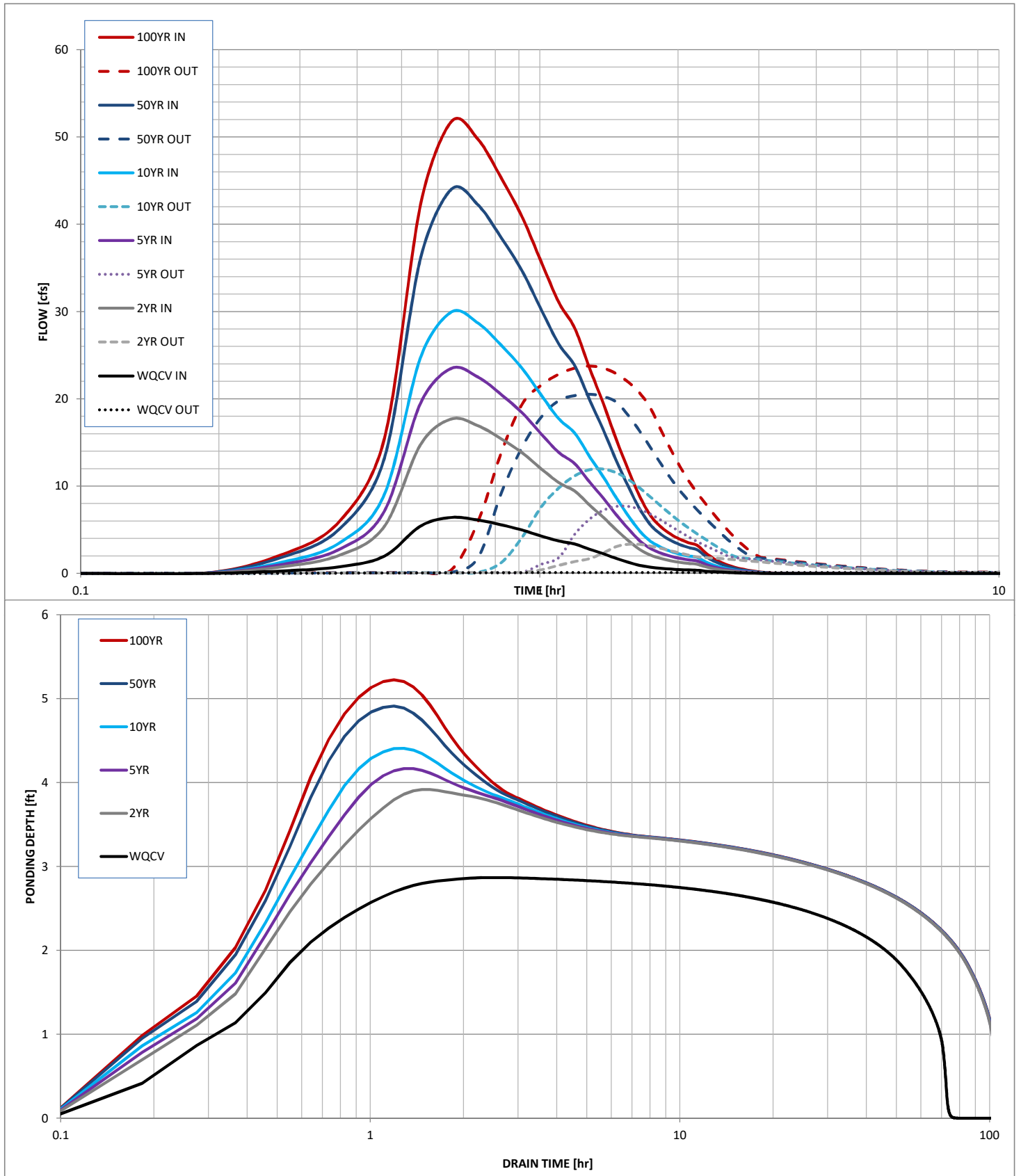
<https://maperture.digitaldataservices.com/gvh/?viewer=cswdif>

attach the pdf of this worksheet to th

| | |
|------|--------|
| WQCV | 2 Year |
|------|--------|

| | | | | | | | |
|--------------------------------------|-------|--------|--------|---------|---------|----------|---------|
| Design Storm Return Period = | WQCV | 2 Year | 5 Year | 10 Year | 50 Year | 100 Year | |
| One-Hour Rainfall Depth = | 0.53 | 1.19 | 1.50 | 1.75 | 2.25 | 2.52 | in |
| Calculated Runoff Volume = | 0.424 | 1.180 | 1.574 | 2.011 | 2.972 | 3.506 | acre-ft |
| OPTIONAL Override Runoff Volume = | | | | | | | acre-ft |
| Inflow Hydrograph Volume = | 0.423 | 1.180 | 1.574 | 2.010 | 2.972 | 3.505 | acre-ft |
| Time to Drain 97% of Inflow Volume = | 68.1 | 94.9 | 92.3 | 89.6 | 83.8 | 81.0 | hours |
| Time to Drain 99% of Inflow Volume = | 70.8 | 101.1 | 100.2 | 99.1 | 96.8 | 95.6 | hours |
| Maximum Ponding Depth = | 2.87 | 3.92 | 4.17 | 4.41 | 4.91 | 5.23 | ft |
| Maximum Poned Area = | 0.47 | 0.58 | 0.62 | 0.64 | 0.68 | 0.71 | acres |
| Maximum Volume Stored = | 0.405 | 0.966 | 1.113 | 1.267 | 1.597 | 1.817 | acre-ft |

Stormwater Detention and Infiltration Design Data Sheet

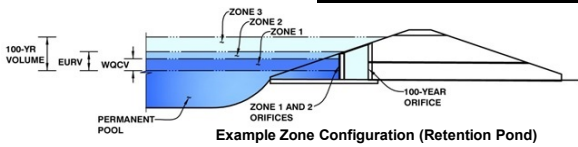


Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Timberline Storage (Amended)

Basin ID: FSD Pond 1



| | Stage (ft) | Zone Volume (ac-ft) | Outlet Type |
|-------------------|------------|---------------------|----------------------|
| Zone 1 (WQCV) | 3.29 | 0.668 | Orifice Plate |
| Zone 2 (EURV) | 4.41 | 0.737 | Orifice Plate |
| Zone 3 (100-year) | 6.84 | 1.928 | Weir&Pipe (Restrict) |
| | | 3.333 | Total |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = ft²
Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = inches
Orifice Plate: Orifice Area per Row = inches

Calculated Parameters for Plate

WQ Orifice Area per Row = ft²
Elliptical Half-Width = feet
Elliptical Slot Centroid = feet
Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (required) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|--------------------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|
| Stage of Orifice Centroid (ft) | 0.00 | 1.47 | 2.94 | | | | | |
| Orifice Area (sq. inches) | 2.51 | 2.30 | 1.50 | | | | | |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|--------------------------------|------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Stage of Orifice Centroid (ft) | | | | | | | | |
| Orifice Area (sq. inches) | | | | | | | | |

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice = ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter = inches

Calculated Parameters for Vertical Orifice

Vertical Orifice Area = ft²
Vertical Orifice Centroid = feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

Overflow Weir Front Edge Height, H_o = ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length = feet
Overflow Weir Slope = H:V (enter zero for flat grate)
Horiz. Length of Weir Sides = feet
Overflow Grate Open Area % = %
Debris Clogging % = %

Calculated Parameters for Overflow Weir

Height of Grate Upper Edge, H_t = feet
Over Flow Weir Slope Length = feet
Grate Open Area / 100-yr Orifice Area = should be ≥ 4
Overflow Grate Open Area w/o Debris = ft²
Overflow Grate Open Area w/ Debris = ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter = inches
Restrictor Plate Height Above Pipe Invert = inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

Outlet Orifice Area = ft²
Outlet Orifice Centroid = feet
Half-Central Angle of Restrictor Plate on Pipe = radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = feet
Spillway End Slopes = H:V
Freeboard above Max Water Surface = feet

Calculated Parameters for Spillway

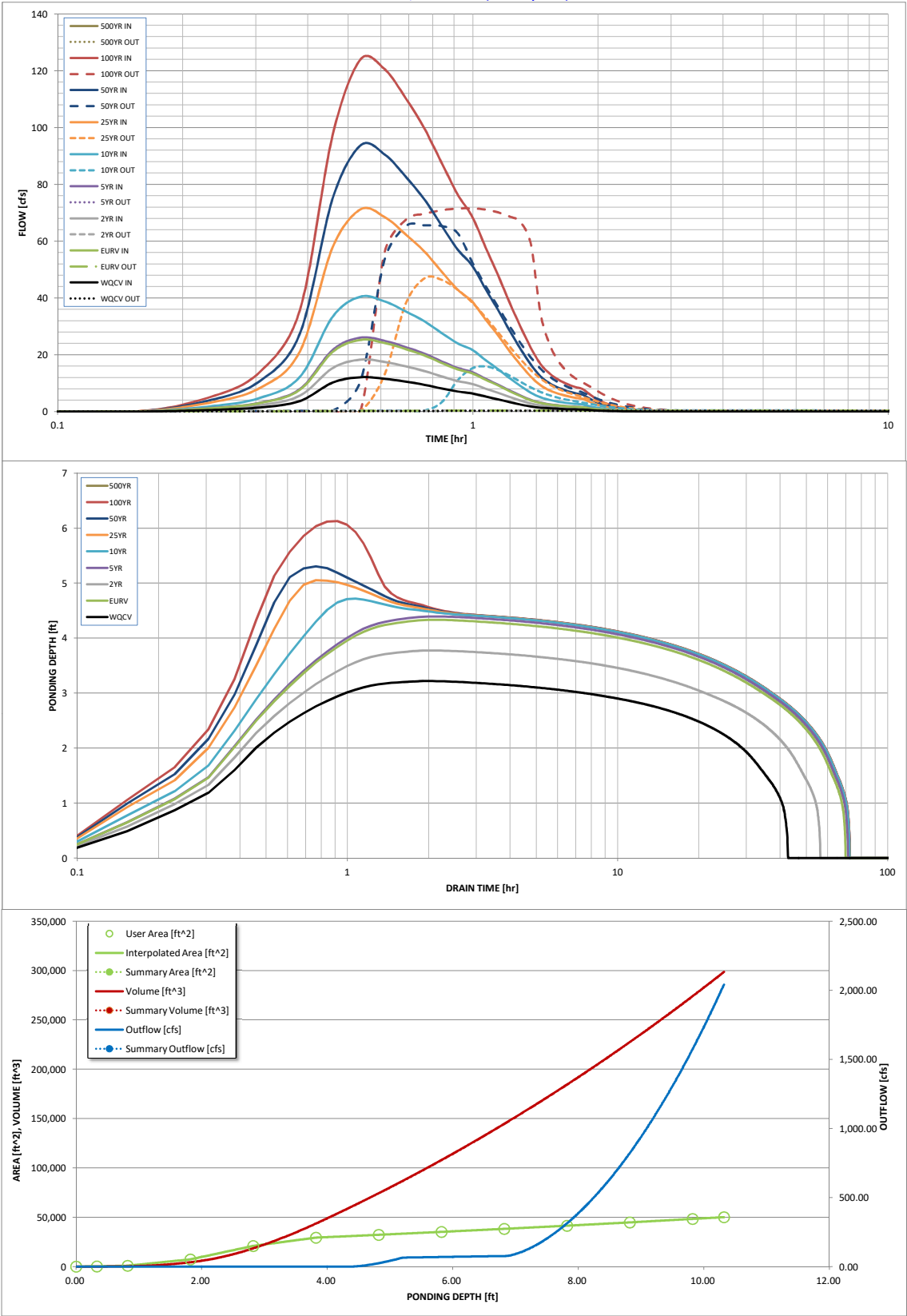
Spillway Design Flow Depth = feet
Stage at Top of Freeboard = feet
Basin Area at Top of Freeboard = acres

Routed Hydrograph Results

| | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year | 500 Year |
|---|-------|-------|--------|--------|------------------|------------------|----------------|----------------|----------|
| Design Storm Return Period = | | | | | | | | | |
| One-Hour Rainfall Depth (in) = | 0.53 | 1.07 | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 | 0.00 |
| Calculated Runoff Volume (acre-ft) = | 0.668 | 1.406 | 1.016 | 1.450 | 2.269 | 4.027 | 5.340 | 7.102 | 0.000 |
| OPTIONAL Override Runoff Volume (acre-ft) = | | | | | | | | | |
| Inflow Hydrograph Volume (acre-ft) = | 0.668 | 1.406 | 1.016 | 1.451 | 2.270 | 4.030 | 5.336 | 7.108 | #N/A |
| Predevelopment Unit Peak Flow, q (cfs/acre) = | 0.00 | 0.00 | 0.01 | 0.02 | 0.16 | 0.53 | 0.78 | 1.12 | 0.00 |
| Predevelopment Peak Q (cfs) = | 0.0 | 0.0 | 0.6 | 1.2 | 10.5 | 33.6 | 49.9 | 71.5 | 0.0 |
| Peak Inflow Q (cfs) = | 12.1 | 25.2 | 18.3 | 26.0 | 40.4 | 71.2 | 93.7 | 123.9 | #N/A |
| Peak Outflow Q (cfs) = | 0.3 | 0.4 | 0.3 | 0.4 | 15.9 | 47.0 | 65.5 | 71.5 | #N/A |
| Ratio Peak Outflow to Predevelopment Q = | N/A | N/A | N/A | 0.3 | 1.5 | 1.4 | 1.3 | 1.0 | #N/A |
| Structure Controlling Flow = | Plate | Plate | Plate | Plate | Overflow Grate 1 | Overflow Grate 1 | Outlet Plate 1 | Outlet Plate 1 | #N/A |
| Max Velocity through Grate 1 (fps) = | N/A | N/A | N/A | N/A | 0.3 | 1.0 | 1.4 | 1.5 | #N/A |
| Max Velocity through Grate 2 (fps) = | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | #N/A |
| Time to Drain 97% of Inflow Volume (hours) = | 40 | 64 | 52 | 65 | 64 | 60 | 57 | 54 | #N/A |
| Time to Drain 99% of Inflow Volume (hours) = | 42 | 68 | 55 | 69 | 69 | 67 | 65 | 64 | #N/A |
| Maximum Ponding Depth (ft) = | 3.22 | 4.33 | 3.77 | 4.39 | 4.72 | 5.05 | 5.31 | 6.13 | #N/A |
| Area at Maximum Ponding Depth (acres) = | 0.55 | 0.71 | 0.66 | 0.71 | 0.73 | 0.75 | 0.77 | 0.83 | #N/A |
| Maximum Volume Stored (acre-ft) = | 0.623 | 1.350 | 0.965 | 1.392 | 1.623 | 1.875 | 2.066 | 2.721 | #N/A |

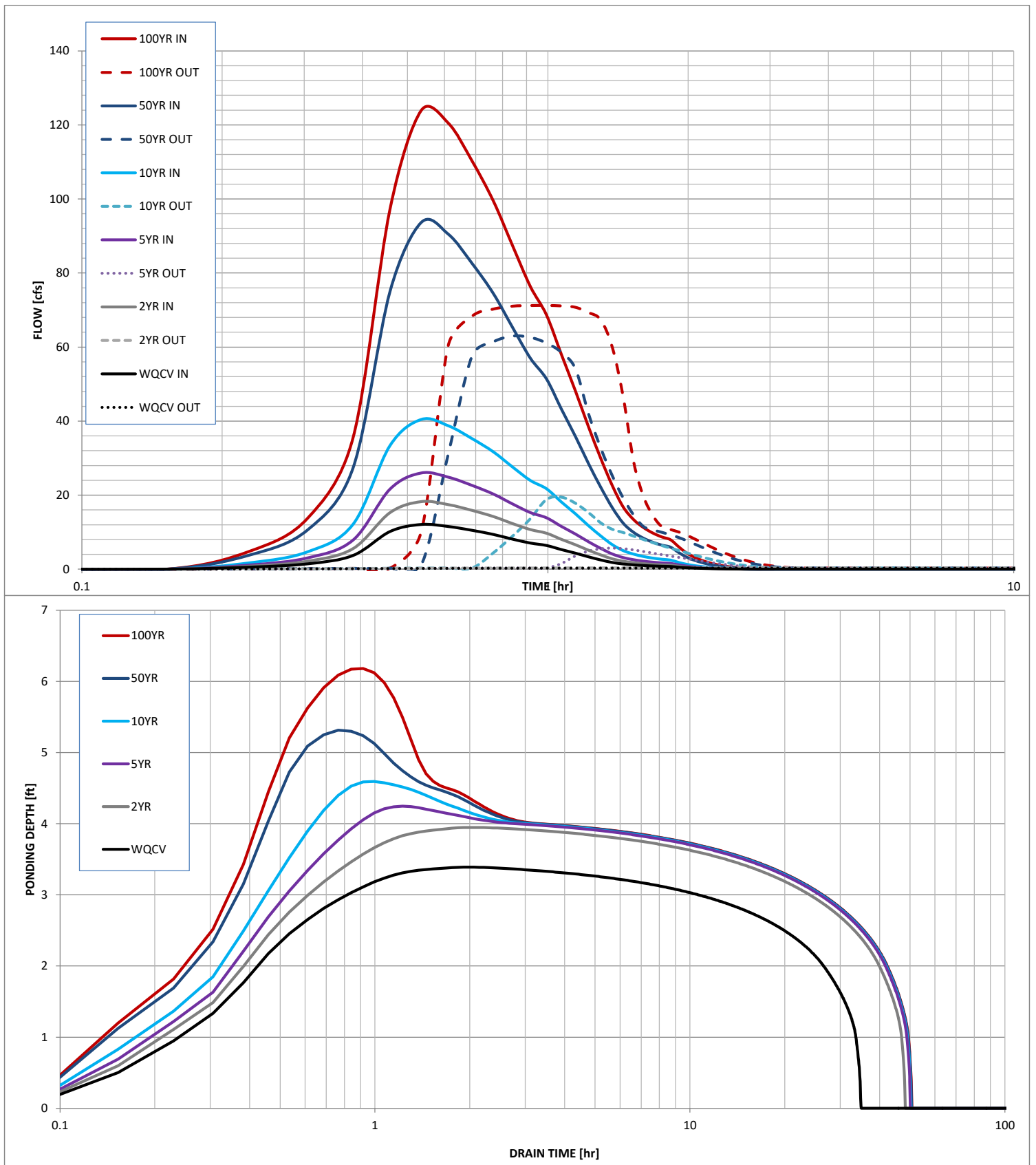
Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)



| S-A-V-D Chart Axis Override | X-axis | Left Y-Axis | Right Y-Axis |
|-----------------------------|--------|-------------|--------------|
| minimum bound | | | |
| maximum bound | | | |

Stormwater Detention and Infiltration Design Data Sheet



DWIRE YARD

EMERGENCY SPILLWAY CALCULATIONS FSD POND

| Horizontal Broad-Crested Weir (Eqn 12-20 UDFCD) | | | | |
|---|-------|-----|-----------|--------|
| Variable | | | Solve For | |
| C | 3.00 | | L (ft) | H (ft) |
| L | 25.00 | ft | 0.0 | 0.0 |
| H | 0.87 | ft | | |
| Q | | cfs | | |

| | |
|---------|-------|
| Total Q | 67.64 |
|---------|-------|

Equation 12-20

$$Q = C_{BCW} L H^{1.5}$$

Where:

Q = discharge (cfs)

C_{BCW} = broad-crested weir coefficient (This ranges from 2.6 to 3.0. A value of 3.0 is often used in practice.) See Hydraulic Engineering Circular No. 22 for additional information.

L = broad-crested weir length (ft)

H = head above weir crest (ft)

| Sloping Broad-Crested Weir (Eqn 12-21 UDFCD) | | | | |
|--|------|-----|-----------|--------|
| Variable | | | Solve For | |
| C | 3.00 | | Z (ft) | H (ft) |
| Z | 4.00 | ft | 0.0 | 0.0 |
| H | 0.87 | ft | | |
| Q | | cfs | | |

Equation 12-21

$$Q = \left(\frac{2}{5}\right) C_{BCW} Z H^{2.5}$$

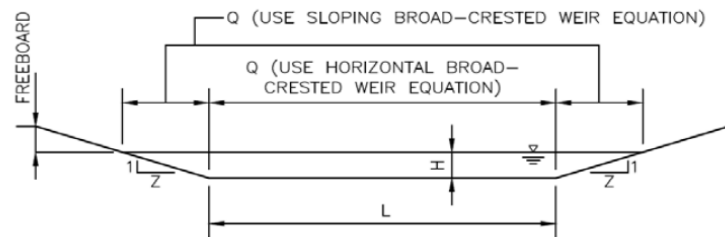


Figure 12-20. Sloping broad-crest weir

PROJECT: Dwire Storage Yard

DATE: _____

Riprap Apron

$V_{\text{allowed}} = 5 \text{ ft/s}$ non-cohesive soils

$$Q = 22.2 \text{ cfs}$$

$$W = 2 \text{ ft}$$

$$Y_t = 1.5 \text{ ft, } 65\% \text{ full at } Q = 22.2 \text{ cfs}$$

$$A_t = \frac{Q}{V} \quad (\text{Eq 9-12})$$

$$\frac{Y_t}{D} = 0.75$$

$$A_t = \frac{22.2 \text{ cfs}}{11.3 \text{ ft/s}} = 1.99 \approx 2.0$$

$$\frac{Q}{D^{2.5}} = \frac{22.2 \text{ cfs}}{(2 \text{ ft})^{2.5}} = 3.92 \approx 4.0 < 6.0 \text{ spread}$$

$$\theta = \tan^{-1} \left(\frac{1}{2 (\text{Expansion Factor})} \right) = \tan^{-1} \left(\frac{1}{2 (6.6)} \right) = 4.33 \text{ degrees}$$

$$L_p = \left(\frac{1}{2 \tan(4.33)} \right) \left(\frac{2.0}{0.75} - 2 \right) = 4.40 < 3(D)$$

$$3(D) = 3(2.0 \text{ ft}) = 6 \text{ ft}$$

Use 6 ft minimum

Figure 9-38

Riprap is Type L $D_{50} = 9"$

$$\frac{Q}{D^{1.5}} = \frac{22.2}{2^{1.5}} = 7.84$$

Upgrade to Type M $D_{50} = 12"$

Thickness is $2 D_{50} = 2(12") = 24"$ Thick

$$L_p = 6 \text{ ft}$$

$$T = 2 (6 \tan(4.33)) + 2$$

$$T = 2.90$$

PROJECT: Dwire Storage Yard

DATE: _____

Forebay Volumes FSD Pond

Size of contributing area = 17.90 Acres

Min Forebay Volume = 3% WQCV (UDFCO TS EDB)

WQCV = 0.260 Ac-ft

$$\begin{aligned} \text{Total Volume Required} &= 0.03 (0.260 \text{ Ac-ft}) \left(\frac{43560 \text{ ft}^2}{1 \text{ Ac-ft}} \right) \\ &= 339.79 \text{ ft}^3 \approx 340 \text{ CF} \end{aligned}$$

Divide Volume based on contributing flow to forebay.

$$\begin{aligned} Q_{100} &= \text{Pipe Run 1} = 12.2 \text{ cfs} \\ Q_{\text{total}} &= 49.8 \text{ cfs} \end{aligned} \quad \left\} \frac{12.2 \text{ cfs}}{49.8 \text{ cfs}} = 24\% \quad \text{East Forebay}$$

$$\begin{aligned} Q_{100} &= \text{Design Point 3} = 10.3 \text{ cfs} \\ Q_{\text{total}} &= 49.8 \text{ cfs} \end{aligned} \quad \left\} \frac{10.3 \text{ cfs}}{49.8 \text{ cfs}} = 20\% \quad \text{North Forebay}$$

$$\begin{aligned} Q_{100} &= \text{Design Point 4} = 28.4 \text{ cfs} \\ Q_{\text{total}} &= 49.8 \text{ cfs} \end{aligned} \quad \left\} \frac{28.4 \text{ cfs}}{49.8 \text{ cfs}} = 56\% \quad \text{West Forebay}$$

Required Volume

$$(340 \text{ CF})(0.24) = 81 \text{ CF}$$

$$(340 \text{ CF})(0.20) = 68 \text{ CF}$$

$$(340 \text{ CF})(0.56) = 190.4 \approx 191 \text{ CF}$$

Provided Volume

$$(125 \text{ SF})(1.25 \text{ ft}) = 156 \text{ CF} \quad \text{East}$$

$$(97 \text{ SF})(1.00 \text{ ft}) = 97 \text{ CF} \quad \text{North}$$

$$(167 \text{ SF})(1.25 \text{ ft}) \approx 209 \text{ CF} \quad \text{West}$$

PROJECT: Dwire Storage Yard

DATE: _____

Size Notches for Forebays

2% of Contributing 100 yr flow

East $Q_{100} = 12.2 \text{ cfs} \rightarrow 0.02(12.2) = 0.244 \text{ cfs}$

North $Q_{100} = 9.2 \text{ cfs} \rightarrow 0.02(9.2) = 0.184 \text{ cfs}$

West $Q_{100} = 28.4 \text{ cfs} \rightarrow 0.02(28.4) = 0.568 \text{ cfs}$

Size Notches Using Weir Eqn.

$Q = CLH^{1.5}$
solve for L

$L = \frac{Q_{notch}}{CH^{1.5}}$

East Notch

$H = 1.25 \text{ ft}$

$L = \frac{0.244}{3.0(1.25)^{1.5}}$

$= (0.0582) \left(\frac{12 \text{ inches}}{1 \text{ ft}} \right) = (0.06134) \left(\frac{12 \text{ inches}}{1 \text{ ft}} \right)$

$L = 0.698''$

Use 1" wide
or it may clog

North Notch

$H = 1.00 \text{ ft}$

$L = \frac{0.184}{3.0(1.00)^{1.5}}$

$L = 0.736''$

Use 1" wide
or it may clog

West Notch

$H = 1.25 \text{ ft}$

$L = \frac{0.568}{3.0(1.25)^{1.5}}$

$= (0.1355) \left(\frac{12 \text{ inches}}{1 \text{ ft}} \right)$

$L = 1.626''$

Use 1.75" wide

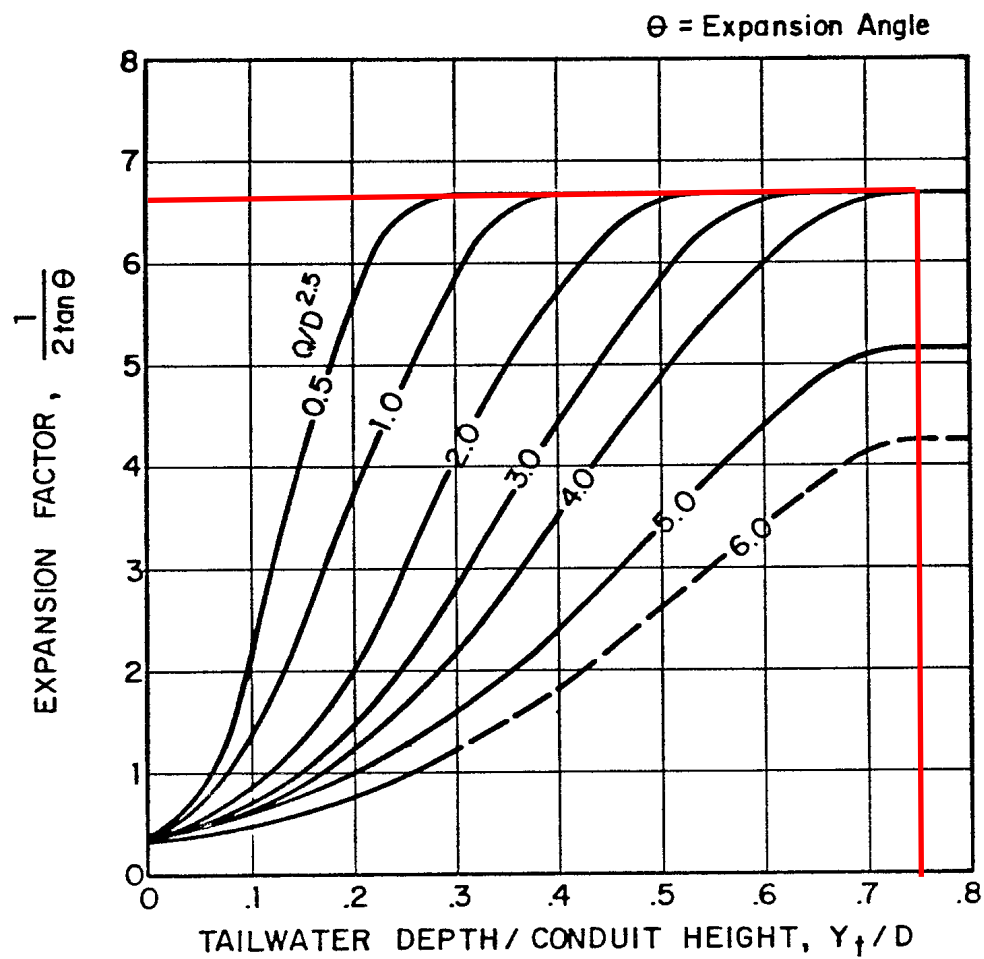


Figure 9-35. Expansion factor for circular conduits

$$H_a = \frac{(H + Y_n)}{2}$$

Equation 9-19

Where the maximum value of H_a shall not exceed H , and:

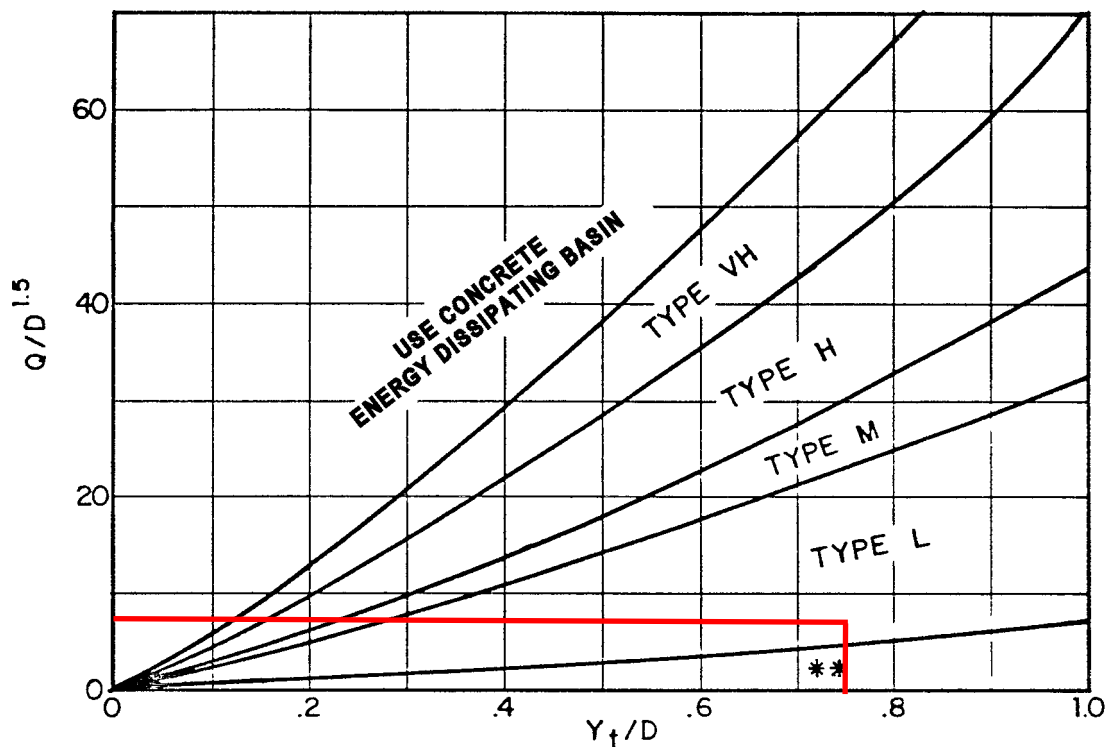
D_a = parameter to use in place of D in Figure 9-38 when flow is supercritical (ft)

D_c = diameter of circular culvert (ft)

H_a = parameter to use in place of H in Figure 9-39 when flow is supercritical (ft)

H = height of rectangular culvert (ft)

Y_n = normal depth of supercritical flow in the culvert (ft)



Use D_a instead of D whenever flow is supercritical in the barrel.

** Use Type L for a distance of $3D$ downstream.

Figure 9-38. Riprap erosion protection at circular conduit outlet (valid for $Q/D^{2.5} \leq 6.0$)



Specification Sheet – VMax® SC250® Turf Reinforcement Mat

DESCRIPTION

The composite turf reinforcement mat (C-TRM) shall be a machine-produced mat of 70% straw and 30% coconut fiber matrix incorporated into permanent three-dimensional turf reinforcement matting. The matrix shall be evenly distributed across the entire width of the matting and stitch bonded between a heavy duty UV stabilized nettings with 0.50 x 0.50 inch (1.27 x 1.27 cm) openings, an ultra heavy UV stabilized, dramatically corrugated (crimped) intermediate netting with 0.5 x 0.5 inch (1.27 x 1.27 cm) openings, and covered by an heavy duty UV stabilized nettings with 0.50 x 0.50 inch (1.27 x 1.27 cm) openings. The middle corrugated netting shall form prominent closely spaced ridges across the entire width of the mat. The three nettings shall be stitched together on 1.50 inch (3.81cm) centers with UV stabilized polypropylene thread to form permanent three-dimensional turf reinforcement matting. All mats shall be manufactured with a colored thread stitched along both outer edges as an overlap guide for adjacent mats.

The SC250 shall meet Type 5A, 5B, and 5C specification requirements established by the Erosion Control Technology Council (ECTC) and Federal Highway Administration's (FHWA) FP-03 Section 713.18

Material Content

| | | |
|---------|--|-------------------------------------|
| Matrix | 70% Straw Fiber | 0.35 lb/sq yd (0.19 kg/sm) |
| | 30% Coconut Fiber | 0.15 lbs/sq yd (0.08 kg/sm) |
| Netting | Top and Bottom, UV-Stabilized Polypropylene | 5 lb/1000 sq ft (2.44 kg/100 sm) |
| | Middle, Corrugated UV-Stabilized Polypropylene | 24 lb/1000 sf (11.7 kg/100 sm) |
| Thread | Polypropylene, UV Stable | |

Standard Roll Sizes

| | | |
|--------------|--------------------|----------------------|
| Width | 6.5 ft (2.0 m) | 8 ft (2.44m) |
| Length | 55.5 ft (16.9 m) | 90 ft (27.4 m) |
| Weight ± 10% | 34 lbs (15.42 kg) | 70 lbs (31.8 kg) |
| Area | 40 sq yd (33.4 sm) | 80 sq. yd. (66.8 sm) |

| Index Property | Test Method | Typical |
|-----------------------|------------------------|----------------------------|
| Thickness | ASTM D6525 | 0.62 in. (15.75 mm) |
| Resiliency | ASTM 6524 | 95.2% |
| Density | ASTM D792 | 0.891 g/cm ³ |
| Mass/Unit Area | ASTM 6566 | 16.13 oz/sy (548 g/sm) |
| UV Stability | ASTM D4355/ 1000 HR | 100% |
| Porosity | ECTC Guidelines | 99% |
| Stiffness | ASTM D1388 | 222.65 oz-in. |
| Light Penetration | ASTM D6567 | 4.1% |
| Tensile Strength – MD | ASTM D6818 | 709 lbs/ft (10.51 kN/m) |
| Elongation – MD | ASTM D6818 | 23.9% |
| Tensile Strength – TD | ASTM D6818 | 712 lbs/ft (10.56 kN/m) |
| Elongation – TD | ASTM D6818 | 36.9% |
| Biomass Improvement | ASTM D7322 | 441% |

Design Permissible Shear Stress

| | Short Duration | Long Duration |
|-------------------------|-------------------|------------------|
| Phase 1: Unvegetated | 3.0 psf (144 Pa) | 2.5 psf (120 Pa) |
| Phase 2: Partially Veg. | 8.0 psf (383 Pa) | 8.0 psf (383 Pa) |
| Phase 3: Fully Veg. | 10.0 psf (480 Pa) | 8.0 psf (383 Pa) |
| Unvegetated Velocity | 9.5 fps (2.9 m/s) | |
| Vegetated Velocity | 15 fps (4.6 m/s) | |

Slope Design Data: C Factors

| | Slope Gradients (S) | | |
|------------------|---------------------|-----------|--------|
| Slope Length (L) | ≤ 3:1 | 3:1 – 2:1 | ≥ 2:1 |
| ≤ 20 ft (6 m) | 0.0010 | 0.0209 | 0.0507 |
| 20-50 ft | 0.0081 | 0.0266 | 0.0574 |
| ≥ 50 ft (15.2 m) | 0.0455 | 0.0555 | 0.081 |

Roughness Coefficients – Unveg.

| Flow Depth | Manning's n |
|--------------------|-------------|
| ≤ 0.50 ft (0.15 m) | 0.040 |
| 0.50 – 2.0 ft | 0.040-0.012 |
| ≥ 2.0 ft (0.60 m) | 0.011 |



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Poseyville, Indiana 47633

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REFERENCED REPORTS

TIMBERLINE STORAGE YARD

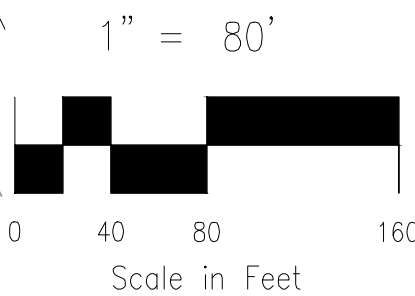
COUNTY OF EL PASO, STATE OF COLORADO

PROPOSED DRAINAGE MAP

DECEMBER 2017

LEGEND

- BASIN DESIGNATION
- ACRES
- PIPE RUN (PR) REFERENCE LABEL
- SURFACE DESIGN POINT (DP)
- BASIN BOUNDARY
- EXISTING CONTOUR
- PROP. CONTOUR
- PARCEL BOUNDARY
- TIMBERLINE STORAGE YARD SITE BOUNDARY
- STORM SEWER PIPE
- FUTURE STORM SEWER PIPE
- INLET
- EXISTING FLOW DIRECTION ARROW
- EMERGENCY OVERFLOW DIRECTION
- FLOW DIRECTION
- FLARED END SECTION
- H.P. X
- L.P. X
- HIGH POINT
- LOW POINT

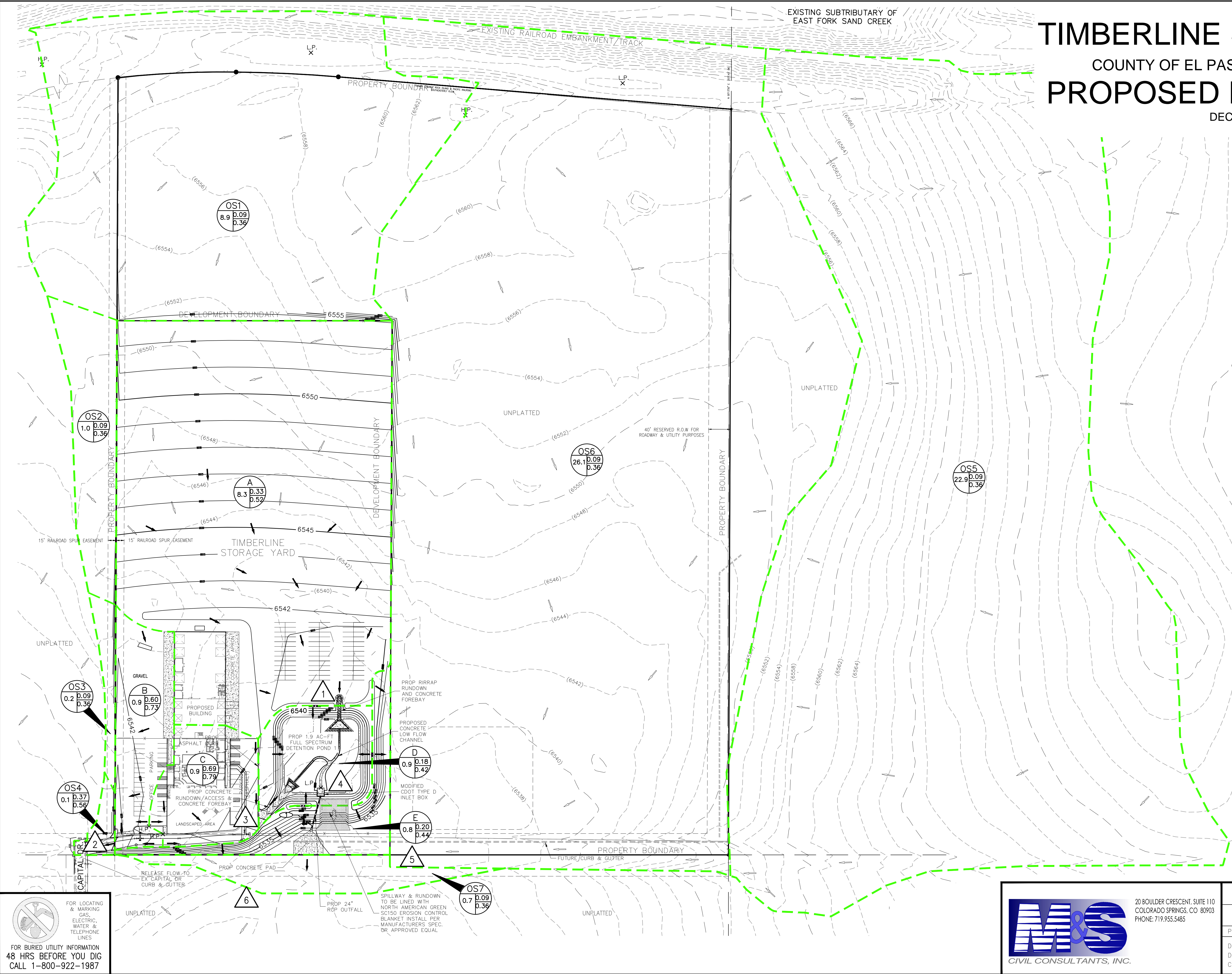


| BASIN SUMMARY | | | | |
|---------------|--------------|----------------|------------------|--|
| BASIN | AREA (ACRES) | Q ₅ | Q ₁₀₀ | |
| A | 8.27 | 9.4 | 25.1 | |
| B | 0.94 | 2.2 | 4.4 | |
| C | 0.90 | 3.2 | 6.1 | |
| D | 0.90 | 0.8 | 3.1 | |
| E | 0.82 | 0.9 | 3.1 | |
| OS1 | 8.93 | 2.5 | 16.6 | |
| OS2 | 1.03 | 0.3 | 2.2 | |
| OS3 | 0.21 | 0.1 | 0.5 | |
| OS4 | 0.13 | 0.3 | 0.7 | |
| OS5 | 22.93 | 7.6 | 50.9 | |
| OS6 | 26.07 | 4.9 | 33.2 | |
| OS7 | 0.68 | 0.2 | 1.4 | |

| DESIGN POINT SUMMARY | | | | |
|----------------------|----------------|------------------|------------------|--|
| DESIGN POINT | Q ₅ | Q ₁₀₀ | BASIN | STRUCTURE |
| 1 | 8.1 | 29.6 | OS1, OS2, A | RIP RAP RUNDOWN INTO FULL-SPECTRUM DETENTION POND |
| 2 | 2.4 | 5.4 | B, OS3, OS4 | OUTFALL TO CAPITAL DR. C&G |
| 3 | 3.2 | 6.1 | C | CONCRETE RUNDOWN INTO FULL-SPECTRUM DETENTION POND |
| 4 | 9.8 | 33.7 | DP1, DP3, D | PROPOSED FULL-SPECTRUM DETENTION POND |
| 5 | 11.3 | 76.2 | OS5, OS6 | HISTORIC DRAINAGE PATTERNS |
| 6 | 12.4 | 85.8 | DP5, PIPE 1, OS7 | HISTORIC DRAINAGE PATTERNS TOTAL DISCHARGE |

| STORM SEWER SUMMARY | | | |
|---------------------|----------------|------------------|-------------------------|
| PIPE RUN | Q ₅ | Q ₁₀₀ | CONTRIBUTING STRUCTURES |
| 1 | 0.8 | 8.3 | 24" RCP POND 1 OUTFALL |

| POND 1 FULL SPECTRUM DETENTION BASIN DATA | |
|---|--|
| WQ WATER SURFACE EL=7035.72 | |
| WQ VOLUME=0.216 AC-FT | |
| EURV WATER SURFACE EL=6536.40 | |
| EURV VOLUME=0.516 AC-FT | |
| 100-YR WATER SURFACE EL=6538.93 | |
| SPILLWAY CREST EL=6539.05 | |
| TOP OF EMBANKMENT EL=6541.0 | |
| 100-YR VOLUME=1.876 AC-FT | |
| 100-YR INFLOW=33.7 CFS | |
| 100-YR RELEASE=8.3 CFS | |



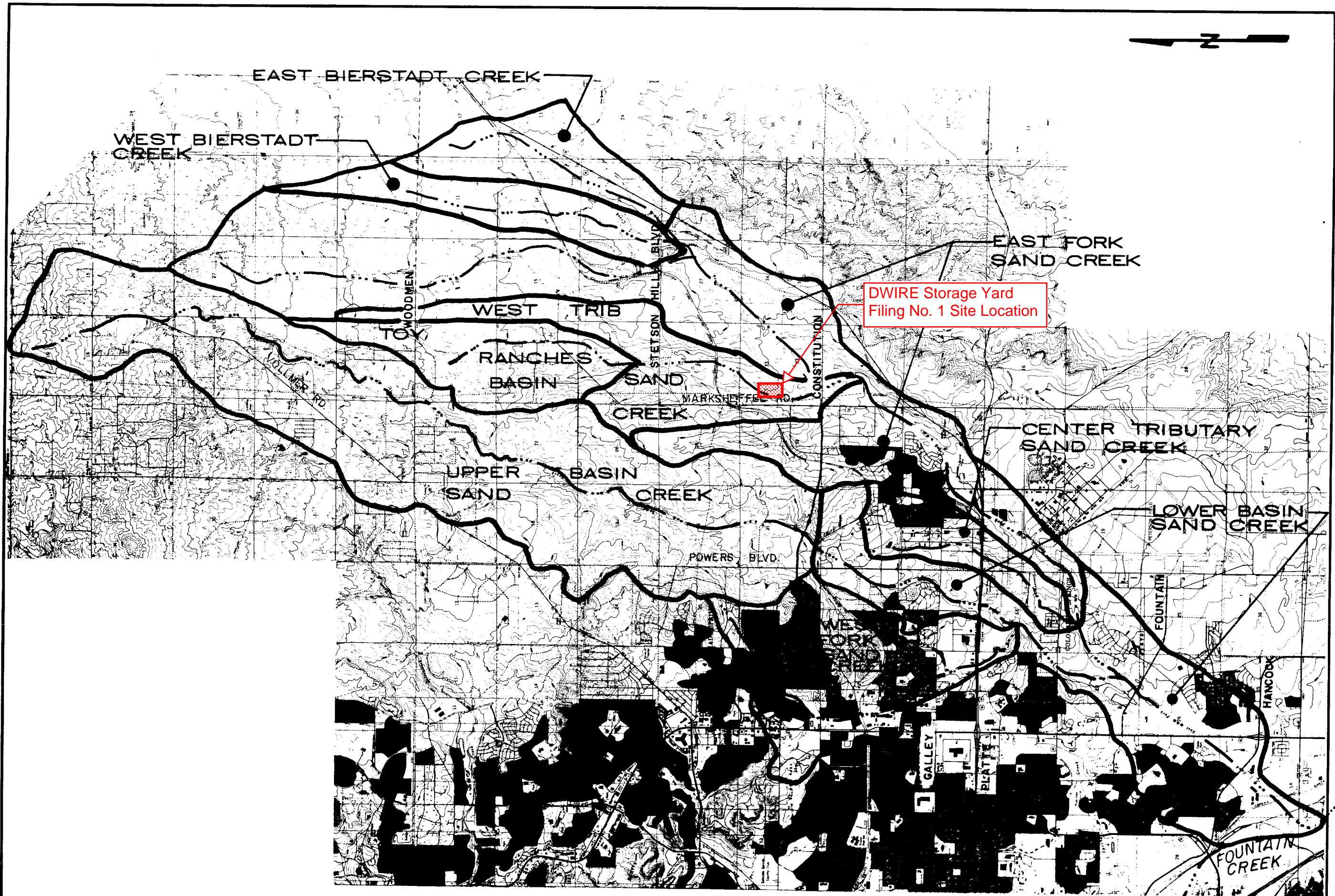
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COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

| TIMBERLINE STORAGE YARD | | | |
|-------------------------|---|------------------|-----|
| PROPOSED DRAINAGE MAP | | | |
| PROJECT NO. 43-095 | SCALE: HORIZONTAL: 1"=80' VERTICAL: N/A | DATE: 12/01/2017 | |
| DESIGNED BY: CMN | DRAWN BY: CMN | SHEET 1 OF 1 | PDM |
| CHECKED BY: VAS | | | |

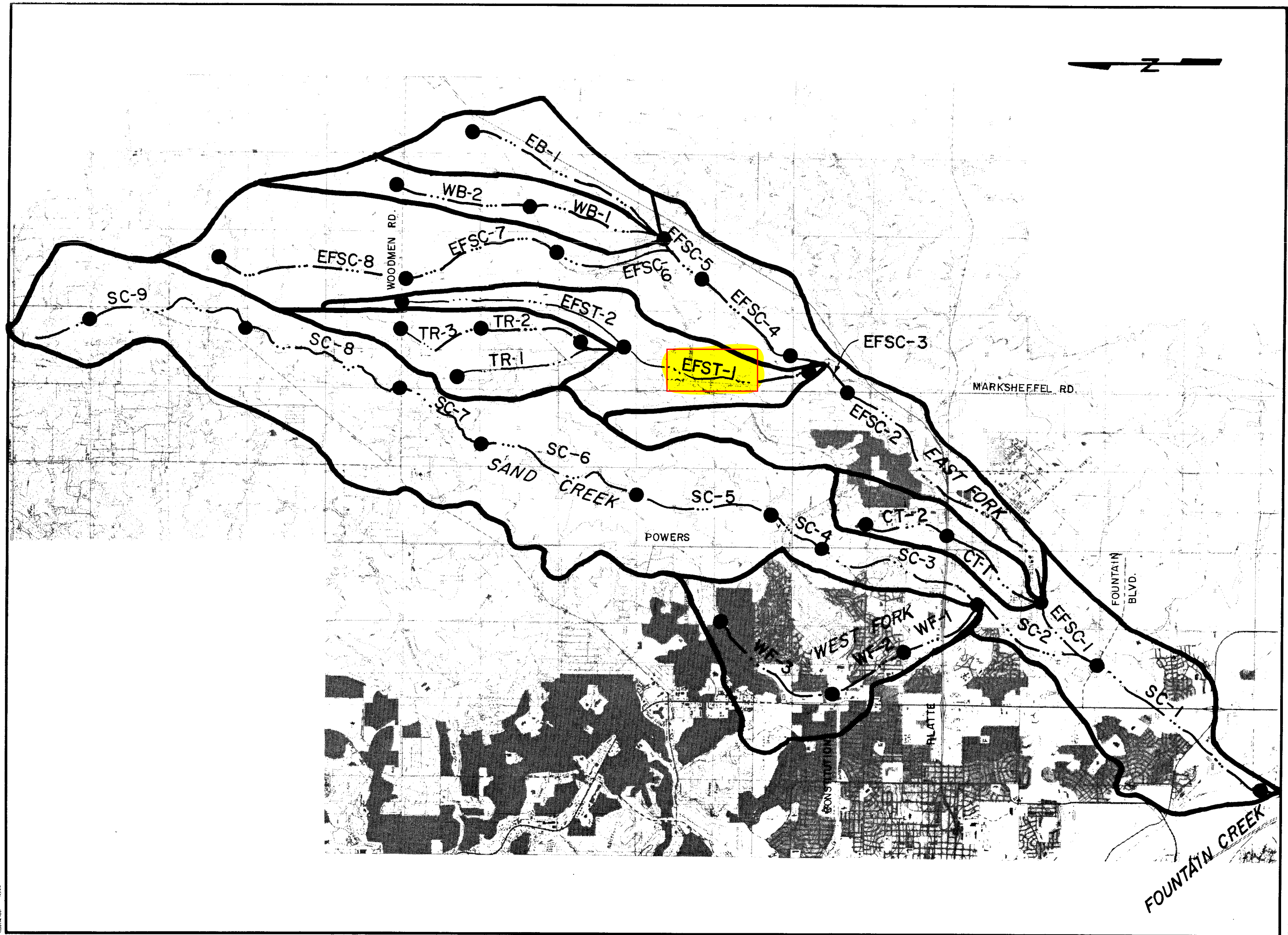


Kiowa Engineering Corporation

419 W. Bijou Street
Colorado Springs, Colorado
80905-1308

SAND CREEK DRAINAGE
BASIN PLANNING STUDY
REGIONAL SUB-BASINS

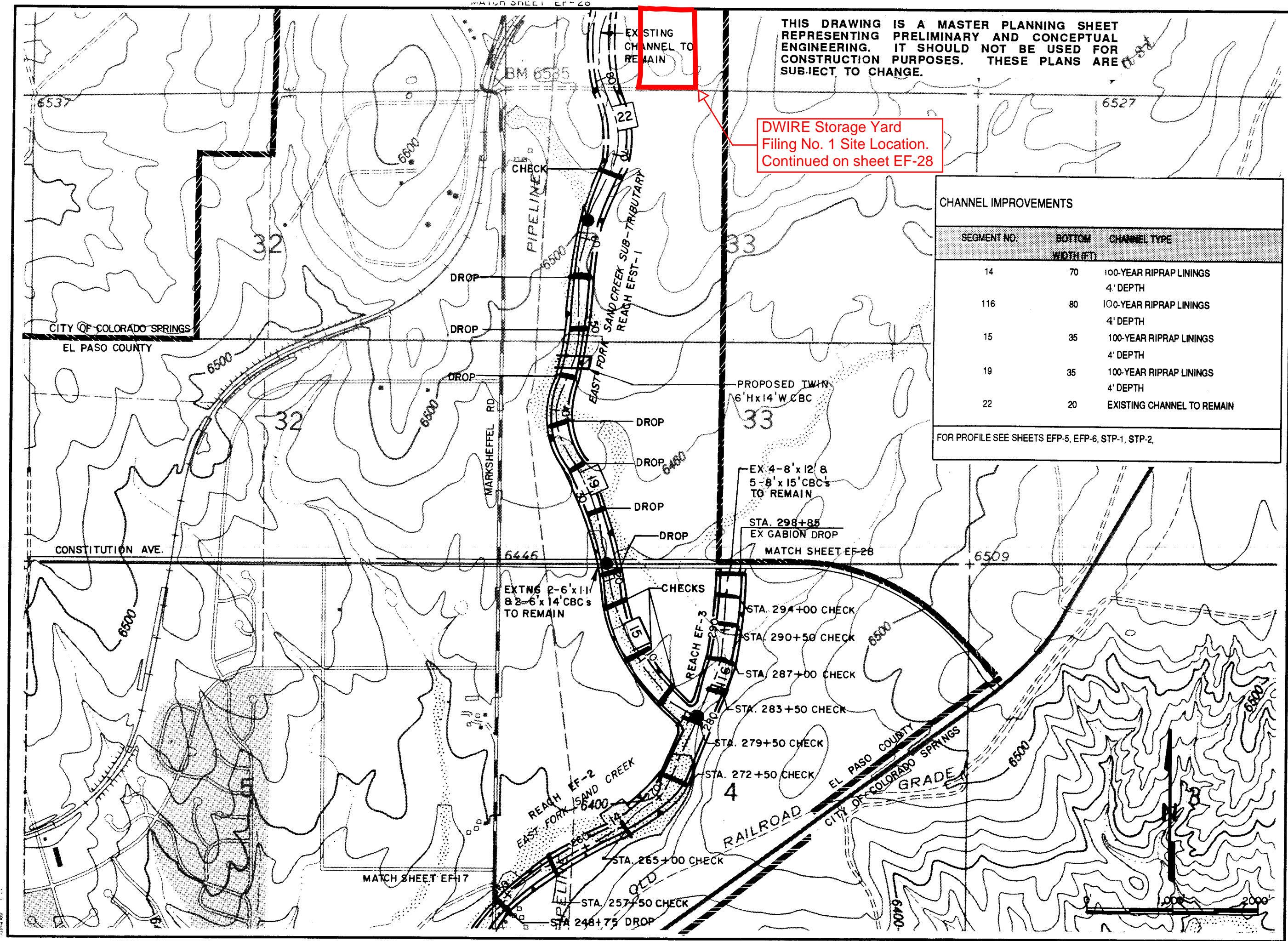
Project No 90-04-09
Date: 11/90
Design:
Drawn: EAK
Check:
Revisions:



Kiowa Engineering Corporation
 419 W. Bijou Street
 Colorado Springs, Colorado
 80905-1308

SAND CREEK DRAINAGE
 BASIN PLANNING STUDY
 REACH DELINEATIONS

| | |
|-------------|----------|
| Project No. | 90-04-09 |
| Date: | 1-91 |
| Design: | RNW |
| Drawn: | EAK |
| Check: | |
| Revisions: | |



THIS DRAWING IS A MASTER PLANNING SHEET REPRESENTING PRELIMINARY AND CONCEPTUAL ENGINEERING. IT SHOULD NOT BE USED FOR CONSTRUCTION PURPOSES. THESE PLANS ARE SUBJECT TO CHANGE.

DWIRE Storage Yard
Filing No. 1 Site Location.
Continued on sheet EF-28

CHANNEL IMPROVEMENTS

| SEGMENT NO. | BOTTOM WIDTH (FT) | CHANNEL TYPE |
|-------------|-------------------|-------------------------------------|
| 14 | 70 | 100-YEAR RIPRAP LININGS 4' DEPTH |
| 116 | 80 | 100-YEAR RIPRAP LININGS 4' DEPTH |
| 15 | 35 | 100-YEAR RIPRAP LININGS 4' DEPTH |
| 19 | 35 | 100-YEAR RIPRAP LININGS 4' DEPTH |
| 22 | 20 | EXISTING CHANNEL TO REMAIN |

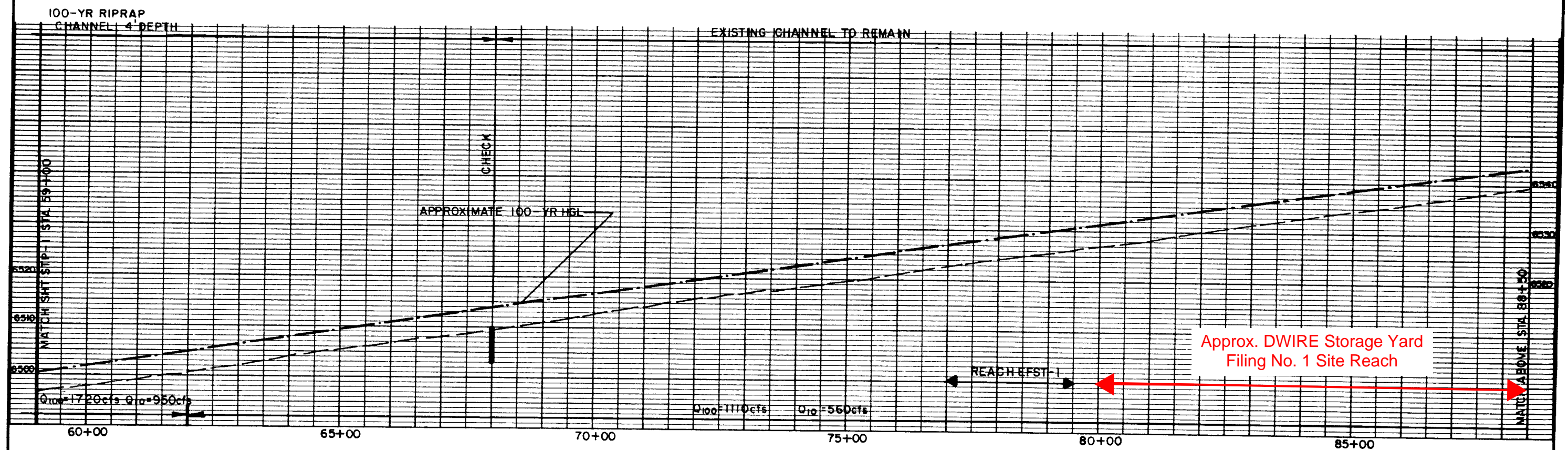
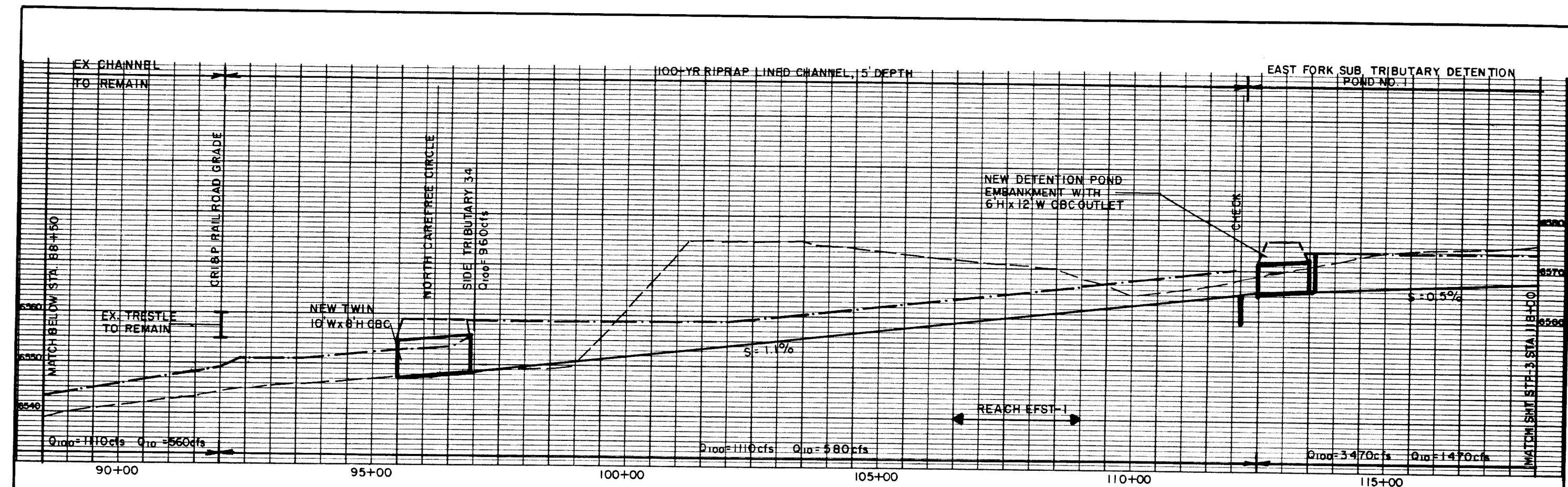
FOR PROFILE SEE SHEETS EFP-5, EFP-6, STP-1, STP-2.

Kiowa Engineering Corporation
419 W. Bijou Street
Colorado Springs, Colorado
80905-1308

SAND CREEK DRAINAGE
BASIN PLANNING STUDY
PRELIMINARY DESIGN PLANS

Project No.
Date:
Design:
Drawn:
Check:
Revisions:

EF-27



Kiowa Engineering Corporation

DESIGNED RNW DATE _____
 CHECKED _____ DATE _____
 DRAWN EAK DATE _____
 REVISED _____ DATE _____

SAND CREEK DRAINAGE BASIN PLANNING STUDY
 PRELIMINARY DESIGN PROFILES

CITY OF COLORADO SPRINGS
 EL PASO COUNTY, COLORADO

EAST FORK SUB-TRIBUTARY
 Station 59+00 to 118+00

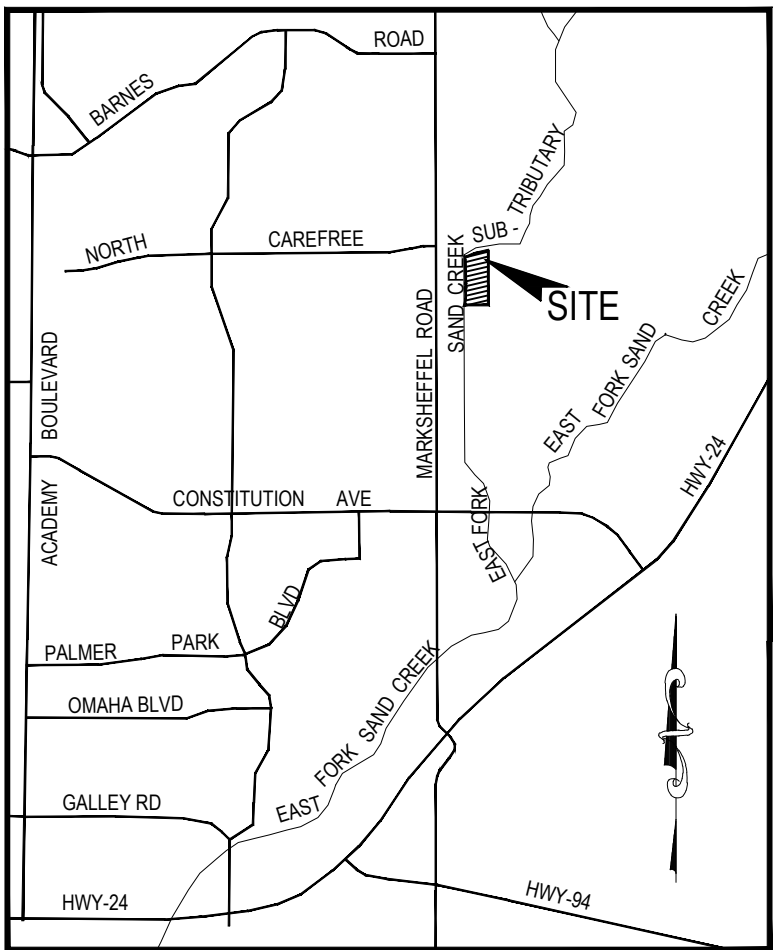
STP-2

GRADING AND EROSION CONTROL PLAN

DWIRE YARD - GRADING AND EROSION CONTROL PLAN

EL PASO COUNTY, STATE OF COLORADO

A PORTION OF THE SOUTHWEST QUARTER OF SECTION 28, TOWNSHIP 13 SOUTH, RANGE 65 WEST OF THE 6TH PRINCIPAL MERIDIAN,



VICINITY MAP
N.T.S.

DESIGN ENGINEER'S STATEMENT

THIS GRADING AND EROSION CONTROL PLAN WAS PREPARED UNDER MY DIRECTION AND SUPERVISION AND IS CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. SAID PLAN HAS BEEN PREPARED ACCORDING TO THE CRITERIA ESTABLISHED BY THE COUNTY FOR GRADING AND EROSION CONTROL PLANS. I ACCEPT RESPONSIBILITY FOR ANY LIABILITY CAUSED BY NEGLIGENT ACTS, ERRORS OR OMISSIONS ON MY PART IN PREPARING THIS PLAN.

VIRGIL A. SANCHEZ, COLORADO P.E. #37160
FOR AND ON BEHALF OF M & S CIVIL CONSULTANTS, INC. DATE

OWNER/DEVELOPER'S STATEMENT:

I, THE OWNER/DEVELOPER HAVE READ AND WILL COMPLY WITH ALL OF THE REQUIREMENTS SPECIFIED IN THESE DETAILED PLANS AND SPECIFICATIONS.

NAME: DATE

BUSINESS NAME:

ADDRESS:

EL PASO COUNTY:

COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

FILED IN ACCORDANCE WITH THE REQUIREMENTS OF THE EL PASO COUNTY LAND DEVELOPMENT CODE, DRAINAGE CRITERIA, AND ENGINEERING CRITERIA MANUAL AS AMENDED.

IN ACCORDANCE WITH EGM SECTION 1.12, THESE CONSTRUCTION DOCUMENTS WILL BE VALID FOR CONSTRUCTION FOR A PERIOD OF 2 YEARS FROM THE DATE SIGNED BY THE EL PASO COUNTY ENGINEER. IF CONSTRUCTION HAS NOT STARTED WITHIN THOSE 2 YEARS, THE PLANS WILL NEED TO BE RESUBMITTED FOR APPROVAL, INCLUDING PAYMENT OF REVIEW FEES AT THE PLANNING AND COMMUNITY DEVELOPMENT DIRECTOR'S DISCRETION.

JENNIFER IRVINE, P.E.
COUNTY ENGINEER / ECM ADMINISTRATOR DATE

LEGAL DESCRIPTION:

A PARCEL OF LAND IN THE SOUTHWEST QUARTER (SW 1/4) OF SECTION 28, TOWNSHIP 13 SOUTH, RANGE 65 WEST OF THE 6TH PRINCIPAL MERIDIAN, EL PASO COUNTY, COLORADO MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BASIS OF BEARINGS: A PORTION OF THE SOUTH LINE OF THE SOUTHWEST QUARTER (SW1/4) OF SECTION 28, TOWNSHIP 13 SOUTH, RANGE 65 WEST OF THE 6TH PRINCIPAL MERIDIAN, EL PASO COUNTY, COLORADO. THE SECTION CORNER COMMON TO SECTIONS 28, 29, 32, AND 33 BEING MONUMENTED WITH A 3" ALUMINUM CAP STAMPED "PLS 38256", FROM WHICH A YELLOW PLASTIC CAP STAMPED "PLS 37909", BEARS N89°58'59"E, A DISTANCE OF 861.03 FEET.

COMMENCING AT THE AFORESAID YELLOW PLASTIC CAP;
THENCE N09°06'45"E A DISTANCE OF 145.85 FEET;
THENCE N00°59'19"E A DISTANCE OF 325.07 FEET;
THENCE N01°07'15"W A DISTANCE OF 804.53 FEET TO THE SOUTHERLY LINE OF THE FORMER CHICAGO, ROCK ISLAND, AND PACIFIC RAILROAD AS RECORDED UNDER RECEPTION NO. 215057836 IN THE RECORDS OF EL PASO COUNTY, COLORADO;
THENCE 649.44 FEET ON THE ARC OF A NON-TANGENT CURVE TO THE RIGHT ON SAID SOUTHERLY LINE, SAID CURVE HAVING A RADIUS OF 1795.32 FEET, AND A CENTRAL ANGLE OF 20°43'34" (THE CHORD OF WHICH BEARS N75°36'49"E, 645.91 FEET);
THENCE S00°13'52"W A DISTANCE OF 1455.11 FEET TO THE AFORESAID SOUTH LINE OF SECTION 28;
THENCE N89°57'07"W ALONG SAID SOUTH LINE A DISTANCE OF 626.77 FEET TO THE POINT OF BEGINNING;

SAID PARCEL CONTAINS A CALCULATED AREA OF 843,414 S.F. (19.362 ACRES MORE OR LESS).

EL PASO COUNTY FILE NO. MS 20-002

DWIRE YARD

GRADING AND EROSION CONTROL PLAN

DATE: 05/21/2020

SCALE: N/A

PROJECT NO. 43-117

DESIGNED BY: DLM

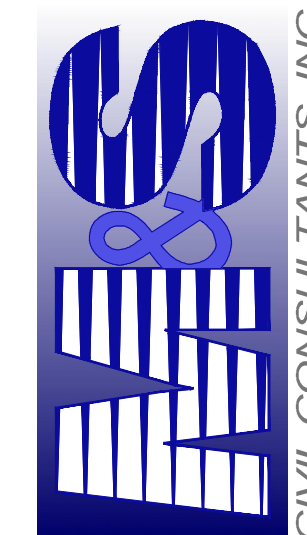
DRAWN BY: JP

CHECKED BY: VAS

GR01

SHEET 1 OF 4

102 E. PIKES PEAK AVE. SUITE 300
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5465



FOR AND ON BEHALF OF
M&S CIVIL CONSULTANTS,
INC.

VIRGIL A. SANCHEZ, COLORADO P.E. NO. 37160

DATE:

DATE:

DATE:

DATE:

DATE:

DATE:

DATE:

DATE:

DATE:

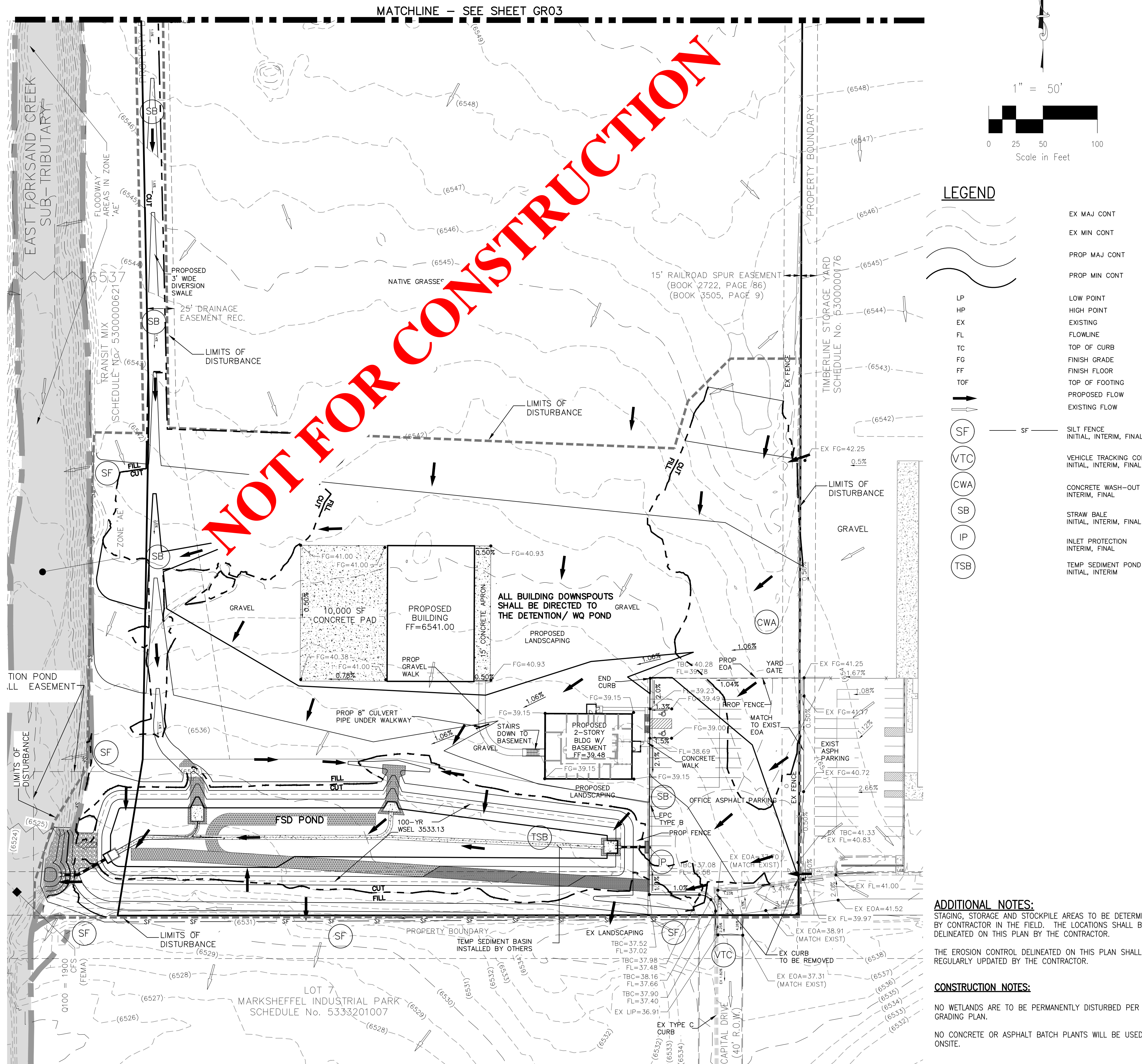
DATE:

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CAUTION

1. STORMWATER DISCHARGES FROM CONSTRUCTION SITES SHALL NOT CAUSE OR THREATEN TO CAUSE POLLUTION, CONTAMINATION, OR DEGRADATION OF STATE WATERS. ALL WORK AND EARTH DISTURBANCE SHALL BE DONE IN A MANNER THAT MINIMIZES POLLUTION OF ANY ON-SITE OR OFF-SITE WATERS, INCLUDING WETLANDS.
2. NOTWITHSTANDING ANYTHING DEPICTED IN THESE PLANS IN WORDS OR GRAPHIC REPRESENTATION, ALL DESIGN AND CONSTRUCTION RELATED TO ROADS, STORM DRAINAGE AND EROSION CONTROL SHALL CONFORM TO THE STANDARDS AND REQUIREMENTS OF THE MOST RECENT VERSION OF THE RELEVANT ADOPTED EL PASO COUNTY STANDARDS, INCLUDING THE LAND DEVELOPMENT CODE, THE ENGINEERING CRITERIA MANUAL, THE DRAINAGE CRITERIA MANUAL, AND THE DRAINAGE CRITERIA MANUAL VOLUME 2. ANY DEVIATIONS FROM REGULATIONS AND STANDARDS MUST BE REQUESTED, AND APPROVED, IN WRITING.
3. A SEPARATE STORMWATER MANAGEMENT PLAN (SMWP) FOR THIS PROJECT SHALL BE COMPLETED AND AN EROSION AND STORMWATER QUALITY CONTROL PERMIT (ESQCP) ISSUED PRIOR TO COMMENCING CONSTRUCTION. MANAGEMENT OF THE SWMP DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE DESIGNATED QUALIFIED STORMWATER MANAGER OR CERTIFIED EROSION CONTROL INSPECTOR. THE SWMP SHALL BE LOCATED ON SITE AT ALL TIMES DURING CONSTRUCTION AND SHALL BE KEPT UP TO DATE WITH WORK PROGRESS AND CHANGES IN THE FIELD.
4. ONCE THE ESQCP IS APPROVED AND A "NOTICE TO PROCEED" HAS BEEN ISSUED, THE CONTRACTOR MAY INSTALL THE INITIAL STAGE EROSION AND SEDIMENT CONTROL MEASURES AS INDICATED ON THE APPROVED GEC. A PRECONSTRUCTION MEETING BETWEEN THE CONTRACTOR, ENGINEER, AND EL PASO COUNTY WILL BE HELD PRIOR TO ANY CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE APPLICANT TO COORDINATE THE MEETING TIME AND PLACE WITH COUNTY STAFF.
5. CONTROL MEASURES MUST BE INSTALLED PRIOR TO COMMENCEMENT OF ACTIVITIES THAT COULD CONTRIBUTE POLLUTANTS TO STORMWATER. CONTROL MEASURES FOR ALL SLOPES, CHANNELS, DITCHES, AND DISTURBED LAND AREAS SHALL BE INSTALLED IMMEDIATELY UPON COMPLETION OF THE DISTURBANCE.
6. ALL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES SHALL BE MAINTAINED AND REMAIN IN EFFECTIVE OPERATING CONDITION UNTIL PERMANENT SOIL EROSION CONTROL MEASURES ARE IMPLEMENTED AND FINAL STABILIZATION IS ESTABLISHED. ALL PERSONS ENGAGED IN LAND DISTURBANCE ACTIVITIES SHALL ASSESS THE ADEQUACY OF CONTROL MEASURES AT THE SITE AND IDENTIFY IF CHANGES TO THOSE CONTROL MEASURES ARE NEEDED TO ENSURE THE CONTINUED EFFECTIVE PERFORMANCE OF THE CONTROL MEASURES. ALL CHANGES TO TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES MUST BE INCORPORATED INTO THE STORMWATER MANAGEMENT PLAN.
7. TEMPORARY STABILIZATION SHALL BE IMPLEMENTED ON DISTURBED AREAS AND STOCKPILES WHERE GROUND DISTURBING CONSTRUCTION ACTIVITY HAS PERMANENTLY CEASED OR TEMPORARILY CEASED FOR LONGER THAN 14 DAYS.
8. FINAL STABILIZATION MUST BE IMPLEMENTED AT ALL APPLICABLE CONSTRUCTION SITES. FINAL STABILIZATION IS ACHIEVED WHEN ALL GROUND DISTURBING ACTIVITIES ARE COMPLETE AND ALL DISTURBED AREAS EITHER HAVE A UNIFORM VEGETATIVE COVER WITH INDIVIDUAL PLANT DENSITY OF 70 PERCENT OF PRE-DISTURBANCE LEVELS ESTABLISHED OR EQUIVALENT PERMANENT ALTERNATIVE STABILIZATION METHOD IS IMPLEMENTED. ALL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES SHALL BE REMOVED UPON FINAL STABILIZATION AND BEFORE PERMIT CLOSURE.
9. ALL PERMANENT STORMWATER MANAGEMENT FACILITIES SHALL BE INSTALLED AS DESIGNED IN THE APPROVED PLANS. ANY PROPOSED CHANGES THAT EFFECT THE DESIGN OR FUNCTION OF PERMANENT STORMWATER MANAGEMENT STRUCTURES MUST BE APPROVED BY THE ECM ADMINISTRATOR PRIOR TO IMPLEMENTATION.
10. EARTH DISTURBANCES SHALL BE CONDUCTED IN SUCH A MANNER SO AS TO EFFECTIVELY MINIMIZE ACCELERATED SOIL EROSION AND RESULTING SEDIMENTATION. ALL DISTURBANCES SHALL BE DESIGNED, CONSTRUCTED, AND COMPLETED SO THAT THE EXPOSED AREA OF ANY DISTURBED LAND SHALL BE LIMITED TO THE SHORTEST PRACTICAL PERIOD OF TIME. PRE-EXISTING VEGETATION SHALL BE PROTECTED AND MAINTAINED WITHIN 50 HORIZONTAL FEET OF A WATERS OF THE STATE UNLESS SHOWN TO BE INFEASIBLE AND SPECIFICALLY REQUESTED AND APPROVED.
11. COMPACTION OF SOIL MUST BE PREVENTED IN AREAS DESIGNATED FOR INFILTRATION CONTROL MEASURES OR WHERE FINAL STABILIZATION WILL BE ACHIEVED BY VEGETATIVE COVER. AREAS DESIGNATED FOR INFILTRATION CONTROL MEASURES SHALL ALSO BE PROTECTED FROM SEDIMENTATION DURING CONSTRUCTION UNTIL FINAL STABILIZATION IS ACHIEVED. IF COMPACTION PREVENTION IS NOT FEASIBLE DUE TO SITE CONSTRAINTS, ALL AREAS DESIGNATED FOR INFILTRATION AND VEGETATION CONTROL MEASURES MUST BE LOOSENEED PRIOR TO INSTALLATION OF THE CONTROL MEASURE(S).
12. ANY TEMPORARY OR PERMANENT FACILITY DESIGNED AND CONSTRUCTED FOR THE CONVEYANCE OF STORMWATER AROUND, THROUGH, OR FROM THE EARTH DISTURBANCE AREA SHALL BE A STABILIZED CONVEYANCE DESIGNED TO MINIMIZE EROSION AND THE DISCHARGE OF SEDIMENT OFF SITE.
13. CONCRETE WASH WATER SHALL BE CONTAINED AND DISPOSED OF IN ACCORDANCE WITH THE SWMP. NO WASH WATER SHALL BE DISCHARGED TO OR ALLOWED TO ENTER STATE WATERS, INCLUDING ANY SURFACE OR SUBSURFACE STORM DRAINAGE SYSTEM OR FACILITIES. CONCRETE WASHOUTS SHALL NOT BE LOCATED IN AN AREA WHERE SHALLOW GROUNDWATER MAY BE PRESENT, OR WITHIN 50 FEET OF A SURFACE WATER BODY, CREEK OR STREAM.
14. DURING DEWATERING OPERATIONS OF UNCONTAMINATED GROUND WATER MAY BE DISCHARGED ON SITE, BUT SHALL NOT LEAVE THE SITE IN THE FORM OF SURFACE RUNOFF UNLESS AN APPROVED STATE DEWATERING PERMIT IS IN PLACE.
15. EROSION CONTROL BLANKETING OR OTHER PROTECTIVE COVERING SHALL BE USED ON SLOPES STEEPER THAN 3:1.
16. CONTRACTOR SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL WASTES FROM THE CONSTRUCTION SITE FOR DISPOSAL IN ACCORDANCE WITH LOCAL AND STATE REGULATORY REQUIREMENTS. NO CONSTRUCTION DEBRIS, TREE SLASH, BUILDING MATERIAL WASTES OR UNUSED BUILDING MATERIALS SHALL BE BURIED, DUMPED, OR DISCHARGED AT THE SITE.
17. WASTE MATERIALS SHALL NOT BE TEMPORARILY PLACED OR STORED IN THE STREET, ALLEY, OR OTHER PUBLIC WAY, UNLESS IN ACCORDANCE WITH AN APPROVED TRAFFIC CONTROL PLAN. CONTROL MEASURES MAY BE REQUIRED BY EL PASO COUNTY ENGINEERING IF DEEMED NECESSARY, BASED ON SPECIFIC CONDITIONS AND CIRCUMSTANCES.
18. TRACKING OF SOILS AND CONSTRUCTION DEBRIS OFF-SITE SHALL BE MINIMIZED. MATERIALS TRACKED OFF-SITE SHALL BE CLEANED UP AND PROPERLY DISPOSED OF IMMEDIATELY.
19. THE OWNER/DEVELOPER SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL CONSTRUCTION DEBRIS, DIRT, TRASH, ROCK, SEDIMENT, SOIL, AND SAND THAT MAY ACCUMULATE IN ROADS, STORM DRAINS AND OTHER DRAINAGE CONVEYANCE SYSTEMS AND STORMWATER APPURTENANCES AS A RESULT OF SITE DEVELOPMENT.
20. THE QUANTITY OF MATERIALS STORED ON THE PROJECT SITE SHALL BE LIMITED, AS MUCH AS PRACTICAL, TO THAT QUANTITY REQUIRED TO PERFORM THE WORK IN AN ORDERLY SEQUENCE. ALL MATERIALS STORED ON-SITE SHALL BE STORED IN A NEAT, ORDERLY MANNER, IN THEIR ORIGINAL CONTAINERS, WITH ORIGINAL MANUFACTURER'S LABELS.
21. NO CHEMICAL(S) HAVING THE POTENTIAL TO BE RELEASED IN STORMWATER ARE TO BE STORED OR USED ONSITE UNLESS PERMISSION FOR THE USE OF SUCH CHEMICAL(S) IS GRANTED IN WRITING BY THE ECM ADMINISTRATOR. IN GRANTING APPROVAL FOR THE USE OF SUCH CHEMICAL(S), SPECIAL CONDITIONS AND MONITORING MAY BE REQUIRED.
22. BULK STORAGE OF ALLOWED PETROLEUM PRODUCTS OR OTHER ALLOWED LIQUID CHEMICALS IN EXCESS OF 55 GALLONS SHALL REQUIRE ADEQUATE SECONDARY CONTAINMENT PROTECTION TO CONTAIN ALL SPILLS ONSITE AND TO PREVENT ANY SPILLED MATERIALS FROM ENTERING STATE WATERS, ANY SURFACE OR SUBSURFACE STORM DRAINAGE SYSTEM OR OTHER FACILITIES.
23. NO PERSON SHALL CAUSE THE IMPEDIMENT OF STORMWATER FLOW IN THE CURB AND GUTTER OR DITCH EXCEPT WITH APPROVED SEDIMENT CONTROL MEASURES.
24. OWNER/DEVELOPER AND THEIR AGENTS SHALL COMPLY WITH THE "COLORADO WATER QUALITY CONTROL ACT" (TITLE 25, ARTICLE 8, CRS), AND THE "CLEAN WATER ACT" (33 USC 1344), IN ADDITION TO THE REQUIREMENTS OF THE LAND DEVELOPMENT CODE, DCM VOLUME II AND THE ECM APPENDIX I. ALL APPROPRIATE PERMITS MUST BE OBTAINED BY THE CONTRACTOR PRIOR TO CONSTRUCTION (1041, NPDES, FLOODPLAIN, 404, FUGITIVE DUST, ETC.). IN THE EVENT OF CONFLICTS BETWEEN THESE REQUIREMENTS AND OTHER LAWS, RULES, OR REGULATIONS OF OTHER FEDERAL, STATE, LOCAL, OR COUNTY AGENCIES, THE MOST RESTRICTIVE LAWS, RULES, OR REGULATIONS SHALL APPLY.
25. ALL CONSTRUCTION TRAFFIC MUST ENTER/EXIT THE SITE ONLY AT APPROVED CONSTRUCTION ACCESS POINTS.
26. PRIOR TO CONSTRUCTION THE PERMITTEE SHALL VERIFY THE LOCATION OF EXISTING UTILITIES.
27. A WATER SOURCE SHALL BE AVAILABLE ON SITE DURING EARTHWORK OPERATIONS AND SHALL BE UTILIZED AS REQUIRED TO MINIMIZE DUST FROM EARTHWORK EQUIPMENT AND WIND.
28. THE SOILS REPORT FOR THIS SITE HAS BEEN PREPARED BY RMG ENGINEERS, DATED MAY 3RD, 2019 AND SHALL BE CONSIDERED A PART OF THESE PLANS.
29. AT LEAST TEN (10) DAYS PRIOR TO THE ANTICIPATED START OF CONSTRUCTION, FOR PROJECTS THAT WILL DISTURB ONE (1) ACRE OR MORE, THE OWNER OR OPERATOR OF CONSTRUCTION ACTIVITY SHALL SUBMIT A PERMIT APPLICATION FOR STORMWATER DISCHARGE TO THE COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT, WATER QUALITY DIVISION. THE APPLICATION CONTAINS CERTIFICATION OF COMPLETION OF A STORMWATER MANAGEMENT PLAN (SWMP), OF WHICH THIS GRADING AND EROSION CONTROL PLAN MAY BE A PART. FOR INFORMATION OR APPLICATION MATERIALS CONTACT:

GRADING AND EROSION CONTROL PLAN



ADDITIONAL NOTES:
STAGING, STORAGE AND STOCKPILE AREAS TO BE DETERMINED
BY CONTRACTOR IN THE FIELD. THE LOCATIONS SHALL BE
DELINEATED ON THIS PLAN BY THE CONTRACTOR.

THE EROSION CONTROL DELINEATED ON THIS PLAN SHALL BE
REGULARLY UPDATED BY THE CONTRACTOR.

CONSTRUCTION NOTES:

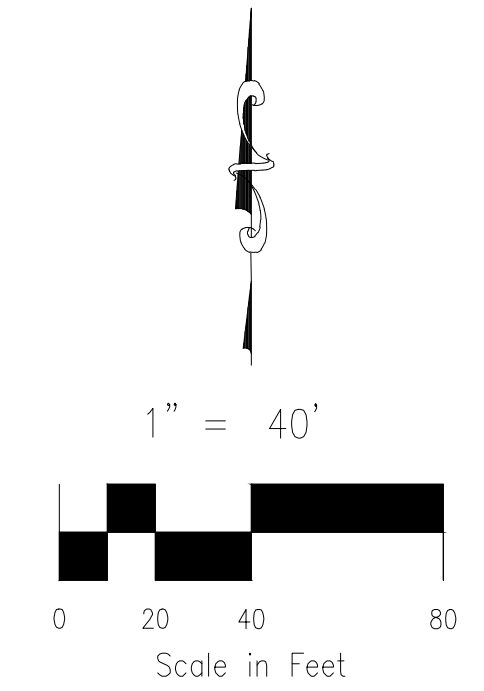
NO WETLANDS ARE TO BE PERMANENTLY DISTURBED PER THIS GRADING PLAN.








NO CONCRETE OR ASPHALT BATCH PLANTS WILL BE USED
ON-SITE.

EL PASO COUNTY FILE NO. MS 20-002

[illegible]

GRADING AND EROSION CONTROL PLAN



- | | |
|--|---|
| LP | LOW POINT |
| HP | HIGH POINT |
| EX | EXISTING |
| FL | FLOWLINE |
| TC | TOP OF CURB |
| FG | FINISH GRADE |
| FF | FINISH FLOOR |
| TOF | TOP OF FOOTING |
|  | PROPOSED FLOW |
|  | EXISTING FLOW |
|  — SF — | SILT FENCE INITIAL, INTERIM, FINAL |
|  | VEHICLE TRACKING CONTROL INITIAL, INTERIM, FINAL |
|  | CONCRETE WASH-OUT BASIN INTERIM, FINAL |
|  | STRAW BALE INITIAL, INTERIM, FINAL |
|  | INLET PROTECTION INTERIM, FINAL |

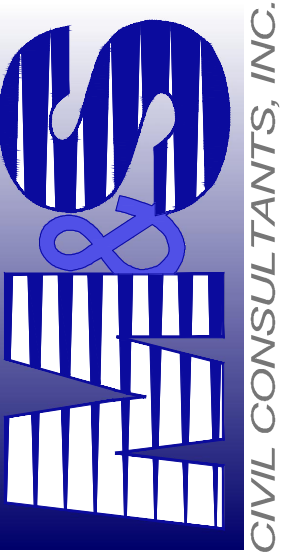
NO BUILD/NO DISTURBANCE
AREAS TO REMAIN UNDEVELOPED

NOT FOR CONSTRUCTION

NATIVE GRASSES

| | | | | | |
|----------------------------------|-----|-------------|--|------------------|--|
| PROJECT NO. 43-117 | | SCALE: | | DATE: 05/21/2020 | |
| DESIGNED BY: | DLM | HORIZONTAL: | | | |
| DRAWN BY: | JP | VERTICAL: | | SHEET 3 OF 4 | |
| CHECKED BY: | VAS | N/A | | | |
| GRADING AND EROSION CONTROL PLAN | | | | | |
| DWIRE STORAGE YARD | | | | | |
| GR03 | | | | | |

102 E. FIRST ST. STE. 100, COLO SPRINGS, CO 80903
PHONE: 719.555.5485



FOR AND ON
BEHALF OF
M&S CIVIL
CONSULTANTS,
INC.

| | |
|--|--|
| | |
|--|--|

[illegible]

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE FOR UNLAME PLAN, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CONCLUSION

EL PASO COUNTY FILE NO. MS 20-002

File: C:\4317A-Dwire Yard\dwire Yard\dwg\Dev Plan\PGR02-04.dwg Plotstamp: 5/28/2020 10:30 AM

GRADING AND EROSION CONTROL PLAN

MM-1

- ### CWA INSTALLATION NOTES
1. SEE PLAN VIEW FOR CWA INSTALLATION LOCATION.
 2. DO NOT LOCATE AN UNLINED CWA WITHIN 4'00" OF ANY NATURAL DRAINAGE PATHWAY OR WATERBODY; DO NOT LOCATE WITHIN 1'00" OF ANY WELLS OR DRINKING WATER SOURCES. IF THE CWA IS LOCATED WITHIN 1'00" OF ANY OF THESE FEATURES, THE CWA MUST BE INSTALLED WITH AN IMPERMEABLE LINER (15 MIL THICKNESS) OR ANOTHER MEANS OF PREVENTING LEAKAGE. IF THE CWA IS LOCATED WITHIN 1'00" OF A LINED ABOVE-GROUND STRUCTURE, BE SHOULD BE USED.
 3. THE CWA SHALL BE INSTALLED PRIOR TO CONCRETE PLACEMENT ON SITE.
 4. CWA SHALL INCLUDE A FLAT SUBSURFACE PIT THAT IS AT LEAST 8" BY 8" SLOPES LEADING OUT OF THE SUBSURFACE PIT SHALL BE 3:1 OR FLATTER. THE PIT SHALL BE AT LEAST 1' DEEP.
 5. BERM SURROUNDING SLOPS AND BACK OF THE CWA SHALL HAVE MINIMUM HEIGHT OF 1'.
 6. VEHICLE TRACKING PAD SHALL BE SLOPED 2% TOWARDS THE CWA.
 7. SIGNS SHALL BE PLACED AT THE CONSTRUCTION ENTRANCE, AT THE CWA AND ELSEWHERE AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CWA TO OPERATORS OF ALL CONCRETE MIXERS AND PUMP TRUCKS.
 8. USE EXCAVATED MATERIAL FOR PERMANENT BERM CONSTRUCTION.

November 2010 Urban Drainage and Flood Control District CWA-3
 Urban Storm Drainage Criteria Manual Volume 3

SM-4



November 2010 Urban Drainage and Flood Control District VTC-3
Urban Storm Drainage Criteria Manual Volume 3

SC-1



September 2010 Urban Drainage and Flood Control District SF-3
Urban Storm Drainage Criteria Manual Volume 3

Table TS/PS-2. Minimum Drill Seeding Rates for Perennial Grasses

| Common Name | Botanical Name | Growth Season | Growth Form | Seeds/Pond | Pounds of PL/Acre |
|---|--------------------------------------|---------------|-------------|------------|-------------------|
| Alkali Soil Seed Mix | | | | | |
| Alkali sedge | <i>Sporobolus airoides</i> | Cool | Bunch | 1,750,000 | 0.25 |
| Basin wildrye | <i>Elymus cinereus</i> | Cool | Bunch | 165,000 | 2.5 |
| Sooty streambank whetgrass | <i>Agropyron repensum 'Soda'</i> | Cool | Sod | 79,000 | 2.5 |
| Soot tail whetgrass | <i>Agropyron elongatum 'Alkali'</i> | Cool | Bunch | 79,000 | 7.0 |
| Archa western whetgrass | <i>Agropyron ambly 'Verba'</i> | Cool | Sod | 110,000 | 5.5 |
| Total | | | | | 17.75 |
| Fertile Loamy Soil Seed Mix | | | | | |
| Epilobium crested whetgrass | <i>Agropyron cristatum 'Epilob'</i> | Cool | Sod | 175,000 | 2.0 |
| Daral hard fescue | <i>Festuca ovina 'haruecula'</i> | Cool | Bunch | 565,000 | 1.0 |
| Lincoln smooth brome | <i>Bromus inermis leys 'Lincoln'</i> | Cool | Sod | 130,000 | 3.0 |
| Sooty streambank whetgrass | <i>Agropyron repensum 'Soda'</i> | Cool | Sod | 170,000 | 2.5 |
| Archa western whetgrass | <i>Agropyron ambly 'Verba'</i> | Cool | Sod | 110,000 | 7.0 |
| Total | | | | | 15.5 |
| High Water Table Soil Seed Mix | | | | | |
| Meadow foxtail | <i>Alopecurus pratensis</i> | Cool | Sod | 900,000 | 0.25 |
| Redtop | <i>Agrostis alba</i> | Warm | Open sod | 5,000,000 | 0.25 |
| Reed canarygrass | <i>Phalaris arundacea</i> | Cool | Sod | 68,000 | 0.5 |
| Lincoln smooth brome | <i>Bromus inermis leys 'Lincoln'</i> | Cool | Sod | 130,000 | 3.0 |
| Pathfinder switchgrass | <i>Panicum virgatum 'Pathfinder'</i> | Warm | Sod | 389,000 | 1.0 |
| Albar tail whetgrass | <i>Agropyron elongatum 'Albar'</i> | Cool | Bunch | 79,000 | 5.5 |
| Total | | | | | 10.75 |
| Transition Turf Seed Mix¹ | | | | | |
| Bransley Canadian bluegrass | <i>Poa compressa 'Bransley'</i> | Cool | Sod | 2,500,000 | 0.5 |
| Daral hard fescue | <i>Festuca ovina 'haruecula'</i> | Cool | Bunch | 565,000 | 1.0 |
| Citation perennial ryegrass | <i>Lolium perenne 'Citation'</i> | Cool | Sod | 247,000 | 3.0 |
| Lincoln smooth brome | <i>Bromus inermis leys 'Lincoln'</i> | Cool | Sod | 130,000 | 3.0 |
| Total | | | | | 7.5 |

| | | |
|---------|--|-----------|
| TS/PS-4 | Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual Volume 3 | June 2012 |
|---------|--|-----------|

EC-2

Table TS/PS-2. Minimum Drill Seeding Rates for Perennial Grasses (cont.)

| Common Name | Botanical Name | Growth Season ¹ | Growth Form | Seeds/ Pound | |
|--|---|----------------------------|------------------------|-----------------|--------------|
| Sandy Soil Seed Mix | | | | | |
| Blue grama | <i>Bouteloua gracilis</i> | Warm | Scd-forming bunchgrass | | |
| Camp little bluestem | <i>Shizachyrium scoparium</i> "Camper" | Warm | Bunch | | |
| Prairie sandreed | <i>Cidamodella longifolia</i> | Warm | Clp ² | 0.7 | |
| Sand dropseed | <i>Sporobolus cryptandrus</i> | Cool ³ | | 0.25 | |
| Vaughn sidcoats grama | <i>Bouteloua curtipendula</i> "Tough" | Warm | | 2.0 | |
| Arriba western wheatgrass | <i>Agropyron smithii</i> "Arriba" | C ⁴ | | 5.5 | |
| Total | | | | | 10.25 |
| Heavy Clay, Rocky Foothill Seed Mix | | | | | |
| Ephraim crested wheatgrass ⁵ | <i>Agropyron cristatum</i> "Ephraim" | | | 175,000 | 1.5 |
| Oahe Intermediate wheatgrass | <i>Agropyron inter-</i> "Oahe" | | scd | 115,000 | 5.5 |
| Vaughn sidcoats grama | <i>Bouteloua</i> "Tough" | | Scd | 191,000 | 2.0 |
| Limbos smooth horene | <i>Br</i> | scd | Scd | 130,000 | 3.0 |
| Arriba western wheatgrass | | Cool | Scd | 110,000 | 5.5 |
| Total | | | | | 17.5 |

¹ All of the above seedlings sowing followed by crimped straw mulch. These rates should be percent if the seeding is done using a Brillion Drill or is applied substituted for drilling only where slopes are steeper than 3:1. If sowing is done as a separate operation.

² See Table TSPS-3 for.

³ If site is to be irrigated, .cool rates should be doubled.

⁴ Crested wheatgrasses should not , on slopes steeper than 6:1 to 1V.

⁵ Can substitute 0.5 the PLS of blue grama for the 2.0 the PLS of Vaughn sidcoats grama.

| | | |
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| June 2012 | Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual Volume 3 | TS/PS-5 |
|-----------|--|---------|

Inlet Protection (IP)



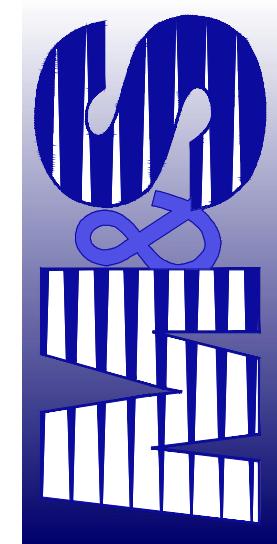
- CURB ROCK SOCK INLET PROTECTION INSTALLATION NOTES
1. SEE ROCK SOCK DESIGN DETAIL INSTALLATION REQUIREMENTS.
 2. PLACEMENT OF THE SOCK SHALL BE APPROXIMATELY 30 DEGREES FROM PERPENDICULAR IN THE OPPOSITE DIRECTION OF FLOW.
 3. SOCKS ARE TO BE FLUSH WITH THE CURB AND SPACED A MINIMUM OF 5 FEET APART.
 4. AT LEAST TWO CURB SOCKS IN SERIES ARE REQUIRED UPSTREAM OF ON-GRADE INLETS.

IP-4
Urban Drainage and Flood Control District
Urban Storm Drainage Criteria Manual Volume 3
August 2013

Straw Bale Barrier (SB)



SBB-2
Urban Drainage and Flood Control District
Urban Storm Drainage Criteria Manual Volume 3
November 2010



160 FOR AND ON
BEHALF OF
M&S CIVIL
CONSULTANTS,

VIRGIL A. SANCHEZ. COLORADO P.F. NO. 37160

[illegible]

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER

CAUTION

DWIRE STORAGE YARD

GRADING AND EROSION CONTROL DETAILS

DATE: 05/21/2020

GR04

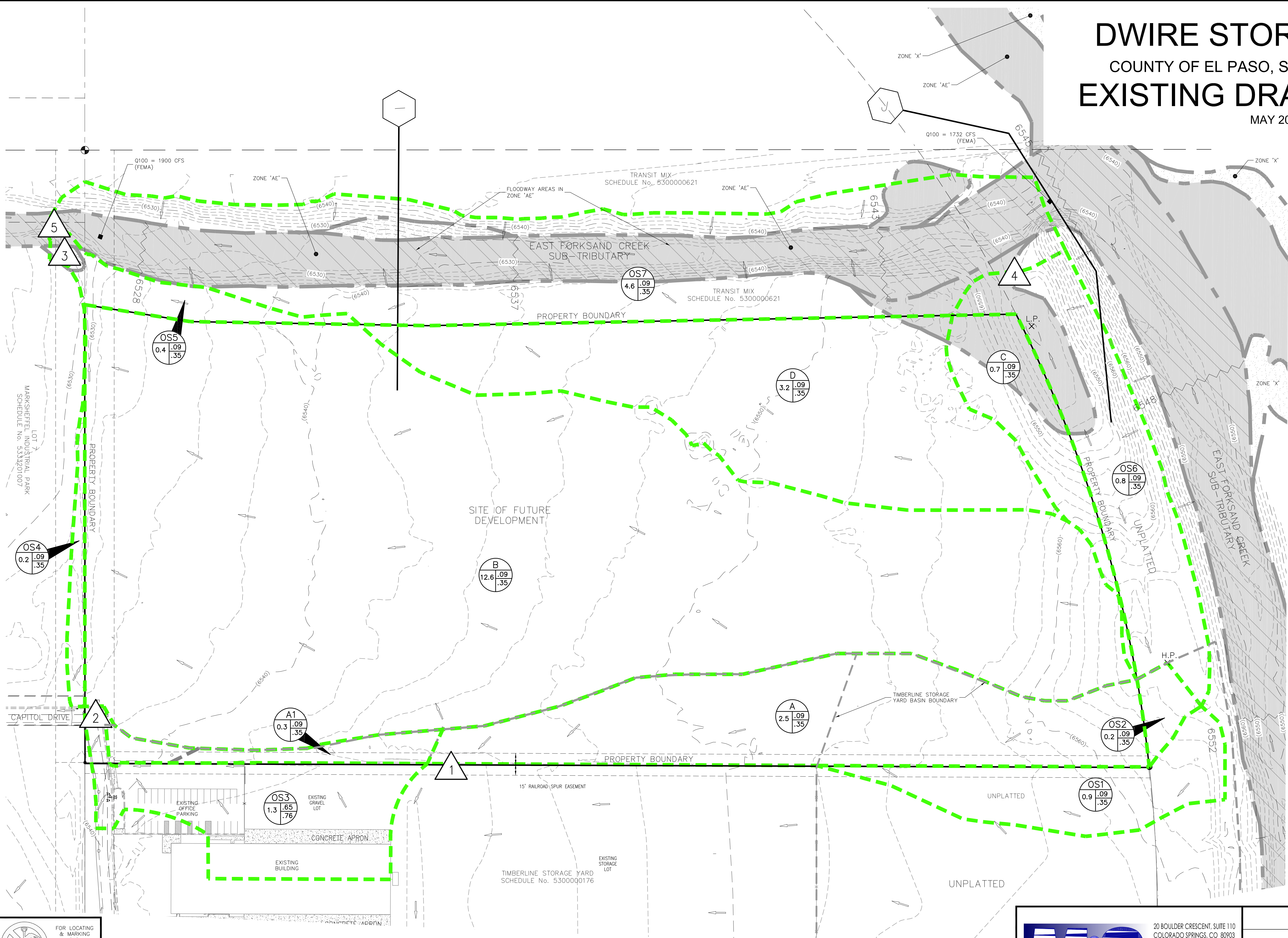
EXISTING AND PROPOSED DRAINAGE MAPS

DWIRE STORAGE YARD

COUNTY OF EL PASO, STATE OF COLORADO

EXISTING DRAINAGE MAP

MAY 2020



LEGEND

BASIN DESIGNATION: Z, C5, C100

ACRES: 25, .25, .35

6: SURFACE DESIGN POINT (DP)

---: BASIN BOUNDARY

---: TIMBERLINE STORAGE YARD BASIN BOUNDARY

---: EXISTING INDEX CONTOUR (10')

---: EXISTING NOMINAL CONTOUR (2')

---: FEMA BASE FLOOD EL. (NGVD29)

---: DWIRE STORAGE YARD SITE BOUNDARY

---: EXISTING FLOW DIRECTION ARROW

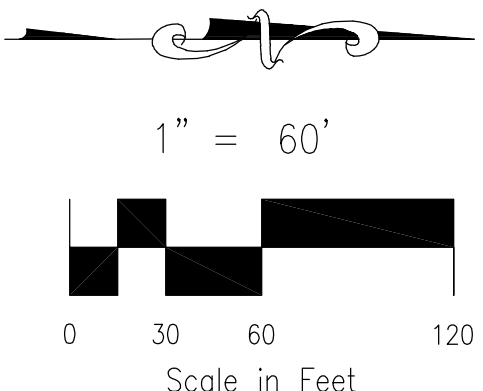
H.P. X: HIGH POINT

L.P. X: LOW POINT

---: FEMA CROSS SECTION ID

| BASIN SUMMARY | | | | |
|---------------|--------------|----------------|------------------|--|
| BASIN | AREA (ACRES) | Q _s | Q ₁₀₀ | |
| OS1 | 0.90 | 0.2 | 1.6 | |
| OS2 | 0.15 | 0.0 | 0.3 | |
| OS3 | 1.26 | 3.4 | 6.8 | |
| OS4 | 0.16 | 0.0 | 0.3 | |
| OS5 | 0.36 | 0.1 | 0.6 | |
| OS6 | 0.78 | 0.2 | 1.3 | |
| OS7 | 4.55 | 0.9 | 6.0 | |
| A | 2.47 | 0.6 | 3.6 | |
| A1 | 0.30 | 0.1 | 0.7 | |
| B | 12.64 | 2.5 | 16.6 | |
| C | 0.66 | 0.2 | 1.5 | |
| D | 3.19 | 0.7 | 4.3 | |

| DESIGN POINT SUMMARY | | | | |
|----------------------|----------------|------------------|------------------|--|
| DESIGN POINT | Q _s | Q ₁₀₀ | BASIN & DES. PTS | |
| 1 | 0.8 | 5.1 | OS1, OS2, A | |
| 2 | 3.6 | 7.5 | A1, OS3 | |
| 3 | 2.6 | 17.2 | OS4, OS5, B | |
| 4 | 0.4 | 2.4 | OS6, C | |
| 5 | 4.5 | 29.3 | DP3, DP4, OS7, D | |



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| | | | |
|-----------------------|---|------------------|--------------|
| DWIRE STORAGE YARD | | | |
| EXISTING DRAINAGE MAP | | | |
| PROJECT NO. 43-117 | SCALE: HORIZONTAL: 1"=60' VERTICAL: N/A | DATE: 05/21/2020 | |
| DESIGNED BY: DLM | DRAWN BY: DLM | CHECKED BY: VAS | SHEET 1 OF 1 |
| | | | EDM |

DWIRE STORAGE YARD

COUNTY OF EL PASO, STATE OF COLORADO

PROPOSED DRAINAGE MAP

MAY 2020

LEGEND

- BASIN DESIGNATION
- ACRES
- PIPE RUN REFERENCE LABEL
- SURFACE DESIGN POINT (DP)
- PROPOSED BASIN BOUNDARY
- TIMBERLINE STORAGE YARD BASIN BOUNDARY
- EXISTING CONTOUR
- PROPOSED CONTOUR
- PROPOSED FENCE
- PROPOSED STORM SEWER PIPE
- DWIRE STORAGE YARD SITE BOUNDARY
- FEMA BASE FLOOD EL. (NGVD29)
- FLARED END SECTION
- INLET/OUTLET STRUCTURE
- PROPOSED RIPRAP
- PERMANENT EROSION CONTROL BLANKET
- EXISTING FLOW DIRECTION ARROW
- PROPOSED FLOW DIRECTION
- EMERGENCY SPILLWAY OVERFLOW DIRECTION
- H.P. = HIGH POINT
- L.P. = LOW POINT
- FEMA CROSS SECTION ID

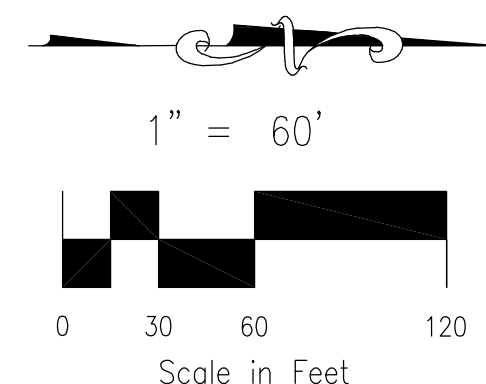
| BASIN SUMMARY | | | | |
|---------------|--------------|----------------|------------------|--|
| BASIN | AREA (ACRES) | Q _s | Q ₁₀₀ | |
| OS1 | 0.15 | 0.1 | 0.4 | |
| OS2 | 1.50 | 4.9 | 9.8 | |
| OS3 | 0.46 | 0.2 | 1.1 | |
| OS4 | 0.12 | 0.0 | 0.3 | |
| OS5 | 0.78 | 0.3 | 1.7 | |
| OS6 | 4.64 | 1.4 | 9.3 | |
| A | 2.44 | 4.1 | 9.0 | |
| B | 1.39 | 3.4 | 6.7 | |
| B1 | 2.74 | 5.9 | 11.7 | |
| B2 | 7.48 | 14.1 | 28.2 | |
| B3 | 1.22 | 0.8 | 3.6 | |
| C | 0.66 | 0.3 | 1.7 | |
| D | 3.19 | 6.5 | 13.0 | |

| DESIGN POINT SUMMARY | | | | |
|----------------------|----------------|------------------|------------------------|--|
| DESIGN POINT | Q _s | Q ₁₀₀ | BASIN & DES. PTS | |
| 1 | 4.1 | 9.3 | OS1, A | |
| 2 | 7.2 | 14.4 | OS2, B | |
| 3 | 5.9 | 11.7 | B1 | |
| 4 | 20.2 | 40.2 | B2, D | |
| 5 | 31.7 | 64.7 | DP2, DP3, DP4, OS4, B3 | |
| 6 | 0.5 | 3.2 | OS5, C | |
| 7 | 2.3 | 29.7 | DP6, OS6, PR2 | |

| PIPE RUN SUMMARY | | | |
|------------------|----------------|------------------|-----------------------------------|
| PIPE RUN | Q _s | Q ₁₀₀ | CONTRIBUTING DES. PTS, STRUCTURES |
| 1 | 7.2 | 14.4 | DP2 |
| 2 | 0.5 | 17.4 | FSD OUTLET STRUCTURE |

FULL SPECTRUM DETENTION POND DATA

WQ WATER SURFACE EL=6530.15
WQ VOLUME=0.453 AC-FT.
EURV WATER SURFACE EL=6532.01
EURV VOLUME=1.519 AC-FT
100-YR WATER SURFACE EL=6533.08
TOP OF EMBANKMENT EL=6535.00
100-YR VOLUME=2.296 AC-FT
100-YR INFLOW=57.8 CFS
100-YR RELEASE=17.4 CFS

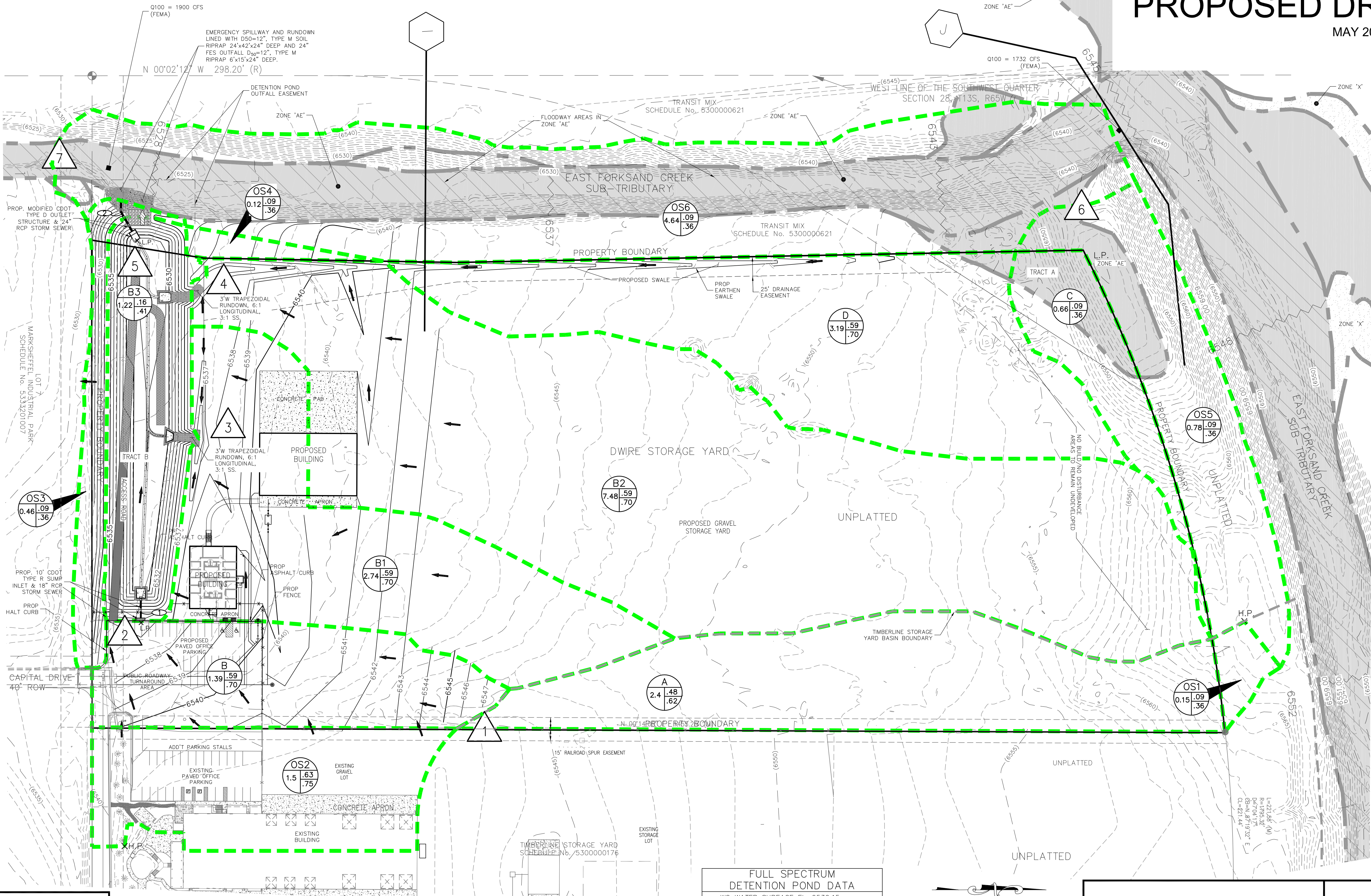


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DWIRE STORAGE YARD

PROPOSED DRAINAGE MAP

| | | | | | | | |
|--------------------|-----|---|--|------------------|--|-----|--|
| PROJECT NO. 43-117 | | SCALE: HORIZONTAL: 1"=60' VERTICAL: N/A | | DATE: 05/21/2020 | | | |
| DESIGNED BY: | DLM | | | SHEET 1 OF 1 | | PDM | |
| DRAWN BY: | DLM | | | | | | |
| CHECKED BY: | VAS | | | | | | |



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