

FINAL DRAINAGE REPORT FOR CLAREMONT BUSINESS PARK 2 FILING NO.1

EL PASO COUNTY, COLORADO

AUGUST 2020

Prepared for:

**Hammers Construction, Inc.
1411 Woosley Heights
Colorado Springs, CO 80906
(719) 570-1599
&
Lena Gail Case
c/o Randy Case II
102 E. Pikes Peak Ave, Suite 200
Colorado Springs, CO 80903**

Prepared by:



102 E. Pikes Peak, 5th Floor
Colorado Springs, CO 80903
(719) 955-5485

Project #44-037
PCD – SF- 20-014

**FINAL DRAINAGE REPORT FOR CLAREMONT
BUSINESS PARK FILING NO.1
EL PASO COUNTY COLORADO**

Revise the title to Claremont Business Park 2 Filing No. 1, typical through out the report. I tried to highlight all the titles that need changing. Please go through the report and change any that i missed.

DRAINAGE PLAN STATEMENTS

ENGINEERS STATEMENT

The attached drainage plan and report was prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Virgil A. Sanchez, P.E. #37160
For and on Behalf of M&S Civil Consultants, Inc

DEVELOPER'S STATEMENT

I, the developer(s) have read and will comply with all the requirements specified in this drainage report and plan.

BY: _____

BY: _____

TITLE: _____

TITLE: _____

DATE: _____

DATE: _____

ADDRESS: Lena Gail Case
2432 Parkview Lane
Colorado Springs, CO 80903

Hammers Construction, Inc.
1411 Woosley Heights
Colorado Springs, CO 80906

EL PASO COUNTY'S STATEMENT

Filed in accordance with the requirements of El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Manual, as amended.

BY: _____

DATE: _____

Jennifer Irvin, P.E.
County Engineer / ECM Administrator

CONDITIONS:

**FINAL DRAINAGE REPORT FOR CLAREMONT
BUSINESS PARK **FILING NO.1**
EL PASO COUNTY COLORADO**

TABLE OF CONTENTS

| | |
|--|----|
| PURPOSE | 4 |
| GENERAL LOCATION AND DESCRIPTION | 4 |
| SOILS | 5 |
| HYDROLOGIC CALCULATIONS | 5 |
| HYDRAULIC CALCULATIONS | 5 |
| FLOODPLAIN STATEMENT | 5 |
| DRAINAGE CRITERIA | 5 |
| FOUR STEP PROCESS | 5 |
| EXISTING DRAINAGE CONDITIONS | 6 |
| PROPOSED DRAINAGE CONDITIONS | 6 |
| WATER QUALITY PROVISIONS AND MAINTENANCE | 10 |
| DRAINAGE EASEMENTS | 11 |
| EROSION CONTROL | 11 |
| CONSTRUCTION COST OPINION | 11 |
| DRAINAGE & BRIDGE FEES | 12 |
| SUMMARY | 12 |
| REFERENCES | 13 |

APPENDIX

Vicinity Map
Soils Map
FEMA FIRM Panel
Hydrologic Calculations
Hydraulic Calculations / SFB WQCV Calculations
BOCC Resolution 16-426
Proposed Drainage Map
Existing Drainage Map / & Excerpt of Matrix Report

FINAL DRAINAGE REPORT FOR CLAREMONT BUSINESS PARK **FILING NO.1 EL PASO COUNTY COLORADO**

PURPOSE

This document is intended to serve as the FINAL DRAINAGE REPORT FOR CLAREMONT BUSINESS PARK **FILING NO.1** and will effectively supersede the previously approved Preliminary Drainage Report for Claremont Commercial Subdivision Fil No. 2, A Resubdivision of Tract C of Claremont Business Park Filing No. 2, El Paso County, Colorado previously approved in December of 2018. The purpose of this document is to identify and analyze the onsite drainage patterns and to ensure that post development runoff is routed through the site safely and in a manner that satisfies the requirements set forth by the El Paso County and City of Colorado Springs Drainage Criteria Manual. The proposed principal use for the site will be neighborhood commercial and light industrial. The parcel is zoned by El Paso County for commercial service as CS. This is a final drainage report; with no significant change from the Preliminary Drainage Report that was submitted previously with the Preliminary plan.

The 13.66 acres that encompasses Claremont Commercial Filing No.3 will be platted as one filing. For construction purposes the south portion (8.33 acres) will be developed and will treat and convey runoff to WQCV Pond 2. The north portion (5.33 acres) will be analyzed in two conditions, undeveloped and future development. In the undeveloped condition, the undeveloped runoff will be routed to a 24" flared end section at southwest corner of the site. In the future developed condition, runoff will be routed to a WQCV Pond 1 at southwest corner of the site. Upon construction of the north portion, a drainage letter will have to be submitted and approved to confirm the study and design were done in accordance with this report.

Please revise the title

GENERAL LOCATION AND DESCRIPTION

Claremont Commercial Subdivision Filing No. 3 is located in the Northeast $\frac{1}{4}$ of the Northeast $\frac{1}{4}$ of Section 8, and the Southeast $\frac{1}{4}$ of the Southeast $\frac{1}{4}$ of Section 5, Township 14 South, Range 65 West of the 6th P.M. in El Paso County, Colorado. The site is bordered to the southeast by U.S. Highway 24 and to the northeast by N. Marksheffel Road, to the north and west by Meadowbrook Parkway, and to the south by a vacant, undeveloped lot. The site lies within the Sand Creek Drainage Basin. Flows from this site are tributary to Sand Creek.

The site consists of 13.66 acres which is currently vacant land with a relatively new roadway infrastructure for Meadowbrook Parkway and associated utilities services directly adjacent to the site. Vegetation is sparse, consisting of native grasses and weeds. Existing site terrain generally slopes from north to southwest at grade rates that vary between 1.2% and 2%. A soil retention wall runs along the eastside of the proposed site, next to U.S. Highway 24 and N. Marksheffel Road, and borders a large portion of the back of the proposed lots. The Claremont Commercial site is currently zoned "CS" and the proposed principal use for the site will be neighborhood commercial and light industrial.

Two (2) sand filter basins will provide water quality treatment for the proposed (Pond 2) and future developments (Pond 1). The outlet structures of the proposed and future water quality ponds will tie into an existing storm sewer system near Meadowbrook Parkway, which routes the treated runoff southwest into Sand Creek. See Appendix for details.

SOILS

Soils for this project are delineated by the map in the appendix as Ellicott Loamy Course Sand (28), Blendon Sandy Loam (10) and Blakeland Loamy Sand (8) and have been characterized as Hydrologic Soil Types "A" & "B". Soils in the study area are shown as mapped by S.C.S. in the "Soils Survey of El Paso County Area". See Appendix for soils report.

HYDROLOGIC CALCULATIONS

Hydrologic calculations were performed using the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual and where applicable the Urban Storm Drainage Criteria Manual. The Rational Method was used to estimate stormwater runoff anticipated from design storms with 5-year and 100-year recurrence intervals.

HYDRAULIC CALCULATIONS

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual. The relevant data sheets are included in the Appendix of this report.

FLOODPLAIN STATEMENT

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 08041C0756G, revised December 7, 2018. No portion of this site is located within the 100 year floodplain. See Appendix.

DRAINAGE CRITERIA

This drainage analysis has been prepared in accordance with the current City of Colorado Springs/El Paso County Drainage Criteria Manual. Calculations were performed to determine runoff quantities for the 5-year and 100-year frequency storms for developed conditions using the Rational Method as required for basins having areas less than 100 acres. See Appendix for calculations.

FOUR STEP PROCESS

Step1 Employ Runoff Reduction Practices –Roof drains will be directed to property lines swales to minimize direct connection of impervious surfaces.

Step 2 Stabilize Drainageways – The site is upstream of an existing 42"/48" RCP storm sewer system that directly discharges to Sand Creek Channel via an outlet structure with wingwalls (privately owned and maintained by the Central Marksheffel Metropolitan District) . The "Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, by Matrix Design Group, Inc. (henceforth referred to as "Matrix FDR") has been designed to discharge developed flows via a 48" RCP storm sewer system directly into the East Fork Sand Creek. The Claremont Commercial Filing No. 2 site proposes a two (2) Sand Filter Water Quality Facilities before flows from both WQ facilities are discharged to the existing private 42"/48" RCP system east of Meadowbrook Parkway. The outlet underdrains are designed to drain the ponds in a peak event within 12 hours, therefore it's not anticipated to have negative effects on the downstream drainageways.

Step 3 Provide Water Quality Capture Volume – Two (2) Sand Filter Basin water quality facilities are proposed to provide WQCV.

Step 4 Consider Need for Industrial and Commercial BMP's – This submittal provides a Preliminary Grading and Erosion Control plan. A Final GEC plan with BMP's in place shall be required with a Final Plat and Site Development applications. The proposed project will use silt fence, a vehicle tracking control pad, a concrete washout area, mulching and reseeded to mitigate the potential for erosion across the site.

Revise title

EXISTING DRAINAGE CONDITIONS

Review 1 comment: Please provide an existing drainage plan showing the appropriate basins and design points (Q) of the historic/existing drainage flow patterns.
Review 2: Unresolved.

The Claremont Commercial Subdivision Filing No. 3 site consists of 13.66 acres and is situated east of the East Fork Reach of the Sand Creek Watershed. This area was previously studied in the "Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, by Matrix Design Group, Inc. (henceforth referred to as "Matrix FDR"). The Matrix FDR calculations indicate that, under the fully developed conditions, the total tributary area of Sub-basins B1, B2, and B3 (18.1 acres), with basin B3 including the eastern half of Meadowbrook Parkway, would produce a cumulative runoff of approximately Q5=42.6 cfs and Q100=86.6 cfs (Design Point 2). The Matrix FDR illustrates that the watershed would drain from east to the southwest towards Meadowbrook Parkway. Sub-Basin B2 identifies a future private 30" RCP to be installed and used to drain the sub-basin into the 42"/48" storm system (privately owned and maintained by the Central Marksheffel Metropolitan District). Field locates has confirmed the 30" RCP was installed. The existing 30" RCP will be removed and a 30" PP will replace it. Sub-Basin B1 identifies a future private 36" RCP to be installed along Meadowbrook Pkwy and stubbed to the sub-basin and used to drain also into the 42"/48" storm system. Field inspection of the existing manhole show the private 30" and 36" RCP were installed. The existing 36" RCP will be removed and a 24" PP will replace it. As stated in the Matrix FDR, overlot grading activities for the entire site have been completed. Per Resolution 16-426 of the BoCC, on-site WQCV is required but on-site stormwater detention is not required per the FDR for Claremont Business Park Fil. 2.

A 48" public storm sewer runs along Meadowbrook Parkway and is routed directly to the Sand Creek channel. Two 10' Type R at grade inlets exist at the intersection of Woolsey Heights and Meadowbrook Parkway, one on the northwest and the other on the northeast corner of the intersection. Runoff from the site and the two surrounding streets, Meadowbrook Parkway and Woolsey Heights, is intercepted by these inlets and conveyed to the Sand Creek channel via the existing 48" public storm sewer.

Refer to the Final Drainage Plan DR01, Matrix FDR, within the Appendix of this report for basin and design point information.

PROPOSED DRAINAGE CHARACTERISTICS

General Concept Drainage Discussion

The majority of the site will consist of neighborhood commercial and light industrial, asphalt, curb, two (2) storm water quality sand filter basins (proposed pond 2 and future pond 1), and landscaping. The flows generated by the site will typically sheet flow across asphalt and impermeable surfaces which direct runoff primarily to the south and southwest to proposed private pipe systems which direct runoff to one of two private ponds. The outlet structures of the proposed water quality ponds will release runoff to the existing private 42" RCP storm sewer located at the southwest corner of the site. A survey and inspection of the existing 42" RCP shall be made before use. The existing private 42" storm sewer ties into an existing public 48" storm sewer which will route the treated runoff to Sand Creek. For more information of drainage basins, existing and proposed structures refer to the Proposed Drainage Map located within the Appendix of this report.

The 13.66 acres that encompasses **Claremont Commercial Filing No.3** will be platted as one filing. For construction purposes the south portion (8.33 acres) will be developed and will treat and convey runoff to WQCV Pond 2. The north portion (5.33 acres) will be analyzed in two conditions, undeveloped and future development. In the undeveloped condition, the north portion will remain undisturbed, except for some minor grading around the perimeter which will route undeveloped flows to a 24" flared end section at the southwest corner of the site. In the future developed condition, final drainage patterns, calculations, treatment and conveyance to a WQCV Pond 1 will be addressed. Upon construction of the north portion, a drainage letter will have to be submitted and approved to confirm the drainage study and design were done in accordance with this report.

To assist in the Detailed Drainage Discussion and differentiate between the north portion undeveloped and north portion future developed, a ** before a drainage basin designation label, design point and pipe run will signify the undeveloped condition. A *** before a drainage basin designation label, design point and pipe run will signify the future developed condition.

Detailed Drainage Discussion

Basin **OS1, 0.19 acres, consists of steep slopes of 32% adjacent to portions of U.S Highway 24 and N. Marksheffel Rd. The roadway embankment within **Basin **OS1** slopes into a soil retention wall that runs along the south east boundary of the site. Runoff for **Basin **OS1** is limited has been calculated to reach peak flow rates of Q5=0.1 cfs and Q100=0.6 cfs. Flows produced within the basin will be conveyed westward into adjacent basin **Basin **OS3** as sheet flow.

Basin **OS2, 0.30 acres, consists of portion of steep slopes of up to 33% that lie adjacent to portions of U.S Highway 24. Similar to **Basin **OS1**, the roadway embankment within **Basin **OS2**, slopes into a soil retention wall that runs along the eastern boundary. Runoff for **Basin **OS2** has been calculated to reach peak flow rates of Q5=0.1 cfs and Q100=1.0 cfs. The limited runoff produced is conveyed westward into adjacent **Basin **OS3** as sheet flow.

BasinOS3**, 4.92 acres, consists of the north portion of the undeveloped site. Runoff produced within **Basin **OS3** is anticipated to reach peak runoff rates of Q5=1.6 cfs and Q100=10.7 cfs. Runoff from the **Basins **OS1, **OS2 and **OS3** shall be conveyed via historic drainage patterns and some minor grading around the perimeter, to a low point at the southwest corner of the site located at **Design Point **4** (Q5=1.8 cfs and Q100=11.8 cfs). A temporary sediment basin will outlet route treated runoff to a proposed private 24" polyethylene storm drain (**Pipe ***5, ***5.1, ***5.2**) and will route the flows south, adjacent to the Meadowbrook ROW to a proposed manhole and existing 42" RCP. The 24" polyethylene storm drain has been sized using the future condition to allow for sufficient capacity.

Basin **OS4, 0.23 acres, consists of a landscaping strip running alongside and adjacent to Meadowbrook Parkway. The landscaping strip consists primarily of trees, bushes/grasses, and decorative ground cover. Low runoff values produced by **Basin **OS4** of Q5=0.1 cfs and Q100=0.7 cfs will travel as sheet flow into Meadowbrook Parkway.

Basin *A**, 0.19 acres, consists of steep slopes of 32% adjacent to portions of U.S Highway 24 and N. Marksheffel Rd. The roadway embankment within **Basin ***A**, slopes into a soil retention wall that runs along the south east boundary of the site. Runoff for **Basin ***A** is limited has been calculated to reach peak flow rates of Q5=0.1 cfs and Q100=0.6 cfs. Flows produced within the basin will be conveyed westward into adjacent basin **Basin ***B** as sheet flow.

Basin *B**, 1.39 acres, consists of Lot 10 along the northeast corner of the proposed site. Runoff produced within **Basin ***B** is anticipated to reach peak runoff rates of Q5=5.5 cfs and Q100=10.1 cfs. The cumulative flows of **Basin ***A** and **Basin ***B** will be routed to the southend of **Basin ***B** to

Design Point *1**(Q5=5.6 cfs and Q100=10.6 cfs). A proposed private 24" polyethylene storm drain (**Pipe ***1**) will be extended to **Design Point ***1** to capture runoff from **Basins ***A** and *****B**.

Basin *C**, 0.30 acres, consists of portion of steep slopes of up to 33% that lie adjacent to portions of U.S Highway 24. Similar to **Basin ***A**, the roadway embankment within **Basin ***C**, slopes into a soil retention wall that runs along the eastern boundary. Runoff for **Basin ***C** has been calculated to reach peak flow rates of Q5=0.1 cfs and Q100=1.0 cfs. The limited runoff produced will be conveyed westward into adjacent **Basin ***D** as sheet flow.

Basin*D**, 1.53 acres, consists of Lot 8 along the eastern boundary of the proposed site. Runoff produced within **Basin ***D** is anticipated to reach peak runoff rates of Q5=6.3 cfs and Q100=11.5 cfs. The cumulative flows of **Basin ***C** and **Basin ***D** will be routed to the southwest corner of **Basin ***D** to **Design Point ***2** (Q5=6.4 cfs and Q100=12.4 cfs). A proposed private 24" polyethylene storm drain (**Pipe ***2**) will be extended to **Design Point ***2** to capture runoff from **Basins ***C** and *****D**. Runoff collected within **Pipes ***1** and *****2** will be routed to a proposed private sand filter water quality pond via a private 30" polyethylene storm drain (**Pipe ***3**) at peak flow rates of Q5=11.8 cfs and Q100=22.6 cfs. A small riprap pad will be required to reduce velocities prior to entering the pond. A swale/berm shall be constructed along the south line of Lot 8, to ensure flows are conveying westerly to a 24" pipe to the proposed storm system and then conveyed to the proposed Sand Filter WQ pond (**Pond 1**) for the retail center area.

Basin *E**, 1.55 acres, consists of Lot 9 and a portion of the planned private access entrance, which is located adjacent to a portion of Meadowbrook Parkway. Runoff produced within **Basin ***E** is anticipated to reach peak runoff rates of Q5=6.5 cfs and Q100=11.8 cfs. A proposed private 24" polyethylene storm drain (**Pipe ***4**) will be extended from the private pond to collect runoff reaching **Design Point ***3** (Q5=6.5 cfs and Q100=11.8 cfs). A small riprap pad will be required to reduce velocities prior to entering the pond.

Basin *F**, 0.36 acres, consists of a land (Tract B) which is dedicated to house a proposed private onsite Sand Filter Basin Water Quality Pond (**Pond 1**) adjacent to existing Meadowbrook Parkway. Runoff produced within **Basin ***F** (Q5=0.2 cfs and Q100=1.0 cfs) will ultimately combine with flows entering the pond via **Pipes ***3** and *****4** at **Design Point ***4**. The total flow anticipated to reach the pond at **Design Point ***4** is calculated by the rational method to be Q5=18.2 cfs and Q100=35.0 cfs. Using the UD-Detention worksheet, flows treated via the Sand Filter Basin are to be discharged through a 6.0' x 2.91' CDOT Modified Type D outlet structure and proposed private 24" polyethylene Storm Sewer (**Pipe Runs ***5, ***5.1, ***5.2**). The proposed pond shall be constructed with 4:1 SS and is anticipated to store 0.118, 0.157 and 0.200 ac-ft and discharge 0.1 cfs, 7.2 cfs, and 15.5 cfs in the water quality, 5 year and 100 year events respectively. The 8.5' wide emergency spillway shall be designed to discharge the peak inflow safely to Meadowbrook Parkway in the event that the inlet would become clogged. Runoff conveyed in **Pipe ***5.2** will combine with flows from a second onsite pond, prior to being discharged downstream via an existing 42" RCP storm sewer.

Basin *G**, 0.29 acres, consists of a landscaping strip running alongside and adjacent to Meadowbrook Parkway and a small section of pavement associated with site access. Excluding the small section of street the basin consists primarily of trees, bushes/grasses, and decorative ground cover. Low runoff values produced by **Basin ***G** of Q5=0.5 cfs and Q100=1.3 cfs will travel as sheet flow into Meadowbrook Parkway.

Basin H, 0.71 acres, consists of steep slopes of up to 33% adjacent to portions of U.S Highway 24. The roadway embankment within **Basin H** slopes into a soil retention wall that runs along the south east boundary of the site. Runoff for **Basin H** has been calculated to reach peak flow rates of Q5=0.3 cfs and Q100=2.0 cfs. Flows produced within the basin will be conveyed westward into adjacent basins (**Basin I**) as sheet flow.

Basin I, 2.75 acres, consists of Lots 5 and 6, portions of Lots 2-4 and 7 and section of proposed private street, which is generally located within the center of the proposed site. Runoff produced within **Basin I** is anticipated to reach peak runoff rates of Q5=9.8 cfs and Q100=17.9 cfs. Runoff from the **Basins H and I** shall be conveyed via side lot swales and curb and gutter to a proposed private street and a pair of proposed CDOT 15' Type R at grade inlets located at **Design Point 5** (Q5=10.1 cfs and Q100=19.8 cfs). Runoff intercepted by the inlets will be conveyed south to proposed water quality sand filter pond 2 via proposed private 24" polyethylene **Pipes 6 and 7** at 5 year flow rates of 6.4 cfs and 10.1 cfs and at 100 year flow rates of 9.1 and 18.3 cfs respectively. A small riprap pad will be required to reduce velocities prior to entering the WQ pond 2. Runoff by passing the inlets will continue west within the street to **Design Point 6**. Pipe 7 shall have gasketed, watertight joints and adhere to performance standards per ASTM D3212.

Basin J, 1.05 acres, consists of portions of Lots 1, 2 and 7 and a segment of the proposed street, which is located along the western edge of the proposed site. Runoff produced within **Basin J** is anticipated to reach peak runoff rates of Q5=4.4 cfs and Q100=8.0 cfs. Runoff from the **Basins J** and flow-by from **Design Point 5** shall be intercepted by a pair of proposed CDOT 15' Type R at grade inlets located at **Design Point 6** (Q5=4.4 cfs and Q100=9.8 cfs). Runoff intercepted by the proposed inlets will be conveyed south to proposed water quality pond 2 via proposed private 18" and 24" polyethylene **Pipes 8 and 9** at 5 year flow rates of 3.0 cfs and 4.5 cfs and at 100 year flow rates of 4.9 and 9.8 cfs respectively. No flowby is anticipated to bypass **Design Point 6** into Meadowbrook Parkway.

Basin K, 0.42 acres, consists of the rear halves of Lots 1 and 2, which is generally located along the southwest corner of the proposed site. Runoff produced within **Basin K** is anticipated to reach peak runoff rates of Q5=1.8 cfs and Q100=3.2 cfs. Runoff from the **Basins K** can be conveyed to a CDOT Type C grated inlet at the southwest corner of Lot 1, at **Design Point 7**. Runoff collected at the local depression would combine with flows in **Pipe 9** and continue to the proposed water quality sand filter pond 2 via pipes 10 and 10.1 at peak flow rates of 6.3 cfs and 13.0 cfs in the 5 and 100 year storm events. A small riprap pad will be required to reduce velocities prior to entering the pond.

Basin L, 1.32 acres, consists of steep slopes of 32% adjacent to portions of U.S Highway 24. The roadway embankment within **Basin L** slopes into a soil retention wall that runs along the south east boundary of the site. Runoff for **Basin L** has been calculated to reach peak flow rates of Q5=0.5 cfs and Q100=3.7 cfs. Flows produced within the basin will be conveyed westward into adjacent basins (**Basin M**) as sheet flow.

Basin M, 1.84 acres, consists of a portion of Lots 3 and 4, which is generally located along the south and southeast sides of the proposed site. Runoff produced within **Basin M** is anticipated to reach peak runoff rates of Q5=6.7 cfs and Q100=12.2 cfs. Runoff from the **Basins L and M** shall be conveyed to a proposed grassed lined swale which will outfall to proposed Sand Filter WQ Pond 2. Peak runoff reaching **Design Point 8** is anticipated to have peak flow rates of Q5=7.2 cfs and Q100=15.7 cfs. The proposed swale would need to be a minimum of 1.5' deep at 0.5% using a 2' bottom width and 3:1 side slopes. A riprap rundown and pad would need to be provided reduce velocities prior entering the pond.

Basin N, 0.47 acres, consists of a land (Tract A) which is dedicated to house a proposed private onsite Sand Filter Basin Water Quality Pond (**Pond 2**) adjacent to existing Meadowbrook Parkway. Runoff produced within **Basin N** (Q5=0.2 cfs and Q100=1.3) cfs will ultimately combine with flows entering the pond via **Pipes 7, 10.1** and from the Swale at **Design Point 8**. The total flow anticipated to reach the pond (**Design Point 9**) is calculated by the rational method to be Q5=23.1 cfs and Q100=46.9 cfs. Using the UD-Detention worksheet, flows treated via the Sand Filter Basin are to be discharged through a 7.0' x 2.91' CDOT Modified Type D outlet structure and proposed private 30" polyethylene Storm Sewer (**Pipe Run 11**). The proposed pond shall be constructed with 4:1 SS and is anticipated to store 0.143, 0.199 and 0.299 ac-ft and discharge 0.1 cfs, 13.3 cfs, and 23.8 cfs in the water quality, 5 year and 100 year events respectively. The 12.5' emergency spillway shall be designed to discharge the peak inflow safely to

Meadowbrook Parkway in the event that the inlet would become clogged. Runoff conveyed in **Pipe 11** will combine with flows within **Pipe 5.2** ($Q_5=20.4$ cfs and $Q_{100}=39.3$), prior to being discharged downstream via an existing 42" RCP storm sewer and into the backside of the existing 10' Type R at grade inlet along existing Woolsey Heights and then to the west via an existing 48" storm sewer.

The Matrix "Final Drainage Report for Claremont Business Park Filing No. 2" calculated that DP 1 combining Sub Basins B1 and B2 generated of ($Q_5=31.5$ cfs and $Q_{100}=63.6$). The proposed development will release $Q_5=20.4$ cfs and $Q_{100}=39.3$ and is less than that of the Matrix report. These flows will combine downstream in the existing 42" pipe with the flows from Lot 2-1A Claremont Business Park of ($Q_5=7.5$ cfs and $Q_{100}=15.4$). Therefore the proposed development shall not have a negative impact on the downstream storm system and is adequately sized to convey the proposed generated flows..

Basin O, 0.16 acres, consists of a landscaping strip running alongside and adjacent to Meadowbrook Parkway. The basin will most likely be composed of trees, bushes/grasses, and decorative ground cover. Low runoff values produced by **Basin O** of $Q_5=0.2$ cfs and $Q_{100}=0.6$ cfs will travel as sheet flow into Meadowbrook Parkway.

Basin P, 0.03 acres, consists of steep slopes of up to 33% adjacent to portions of U.S Highway 24. The roadway embankment within **Basin P** slopes into a soil retention wall that runs along the south east boundary of the site. Runoff for **Basin P** has been calculated to reach peak flow rates of $Q_5=0.0$ cfs and $Q_{100}=0.1$ cfs. Flows produced within the basin will be conveyed westward into adjacent basins (**Basin Q**) as sheet flow.

Basin Q, 0.11 acres, consists of a thin utility corridor alongside the south boundary of the site. The basin will most likely be composed native ground cover. Low runoff values produced by **Basin Q** of $Q_5=0.0$ cfs and $Q_{100}=0.3$ cfs will combine with flows from **Basin P** and will discharge to adjacent site to the south as sheet flow.

There are no planned or required improvements to the Sand Creek Drainage Channel with the development of the CLAREMONT BUSINESS PARK 2 site.

WATER QUALITY PROVISIONS AND MAINTENANCE

The subject site was previously analyzed within the Final Drainage Report for Claremont Business Park Filing No. 2 prepared by Matrix Design Group approved April 24, 2006. Per Resolution 16-426 of the BoCC, on-site WQCV is required but on-site stormwater detention is not required per the FDR for Claremont Business Park Fil. 2. The water quality volume required for the site has been determined using the UDFCD UD-Detention workbook per the guidelines set forth in the City of Colorado Springs/El Paso County Drainage Criteria Manual - Volume II.

As previously discussed water quality for the site is provided by two proposed Sand Filter Basins (SFB). Pond 1 is designed to treat runoff from approx 5.33 acres, by providing 0.118 acre-feet of water quality storage, while Pond 2 will treat runoff from approx 8.33 acres, by providing 0.143 acre-feet of water quality storage. Per ECM section 1.7.1.C.1, 20% of the project site (not to exceed 1.0 acre) may be excluded from the 100% WQ treatment requirement per El Paso County criteria. This report identifies that Basins G, O, P and Q are unable to reach one of the two proposed WQ ponds. Combined total acreage of the Basins are 0.59 AC, and doesn't exceed the 1.0 acre maximum allowance of acreage runoff.

Flows tributary to the two SFBs are released through outlet structures into an existing storm sewer system located along Meadowbrook Parkway. Water quality pond 1 will be private and shall be maintained by the property owners (equal shares determined by size of lots 8-10). Water quality pond 2 will be private and shall be maintained by the property owners (equal shares determined by size of lots 1-7). Access shall be granted to the owner and El Paso County for access and maintenance of the private WQCV facility. A

private maintenance agreement document shall accompany the final drainage report(s) submittal(s) which construct the two ponds.

DRAINAGE EASEMENTS

The Preliminary Plan for this site has submitted a Waiver of the El Paso County Land Development Code to request 2' foot side lot easements for drainage. Therefore, the drainage easement being 4' foot minimum in width (it is likely that some lots will have a building on each lot (4 feet apart) or building and parking lot(4' apart)). A calculation sheet is included in the appendix to show the maximum capacity of a concrete drainage channel with varying slopes. The drainage channel will be required between Lots 1 & 2, Lots 2 & 3, Lots 3 & 4 and Lots 5& 6. The channel is proposed to be a 6" inch tall x 3' foot wide trickle channel. This report will ensure that the maximum capacity is not exceeded.

EROSION CONTROL

As previously discussed and as indicated in the preliminary drainage report, please provide the slope, grading, and total amount of drainage to be conveyed in each of the drainage channels at these lots.

It is the policy of the El Paso County that we submit a grading and erosion control plan with the drainage report. Proposed silt fence, vehicle traffic control, and concrete washout area are proposed as erosion control measures.

CONSTRUCTION COST OPINION

Private Drainage Facilities (NON-Reimbursable) Sand Filter WQ Pond 1 (Future Construction):

| Item | Description | Quantity | Unit Cost | Cost |
|--------------|-----------------------|----------|--------------|--------------------|
| 1. | 24" PP | 344 LF | \$48 /LF | \$16,512.00 |
| 2. | 24" FES | 1 EA | \$520 /EA | \$520.00 |
| 3. | 30" PP | 88 LF | \$65 /LF | \$5,720.00 |
| 4. | 30" FES | 1 EA | \$597 /EA | \$597.00 |
| 5. | Type II Manhole | 1 EA | \$4,000 /EA | \$4,000.00 |
| 6. | WQCV Sand Filter Pond | 1 EA | \$19,000 /EA | \$19,000.00 |
| Total | | | | \$46,349.00 |

Private Drainage Facilities (NON-Reimbursable) (WQ Pond 1) Temporary Sediment Basin (TSB-N) (Interim Construction):

| Item | Description | Quantity | Unit Cost | Cost |
|--------------|----------------------------------|----------|-------------|--------------------|
| 1. | 24" PP | 476 LF | \$48 /LF | \$22,848.00 |
| 2. | 24" FES | 1 EA | \$520 /EA | \$520.00 |
| 3. | Type II Manholes | 2 EA | \$4,000 /EA | \$8,000.00 |
| 4. | Temporary Sediment Basin (TSB-N) | 1 EA | \$2500 /EA | \$2,500.00 |
| Total | | | | \$33,868.00 |

Private Drainage Facilities (NON-Reimbursable) Sand Filter WQ Pond 2:

| Item | Description | Quantity | Unit Cost | Cost |
|------|--------------------------------|----------|-------------|-------------|
| 1. | 18" PP | 47 LF | \$40 /LF | \$1,880.00 |
| 2. | 24" PP | 478 LF | \$48 /LF | \$22,944.00 |
| 3. | 24" FES | 2 EA | \$520 /EA | \$1,040.00 |
| 4. | 30" PP | 40 LF | \$65 /LF | \$2,600.00 |
| 5. | At Grade Inlets (Type R) L=15' | 4 EA | \$7,200 /EA | \$28,800.00 |
| 6. | CDOT Type C Grated Inlet | 1 EA | \$3500 /EA | \$3,500.00 |

| | | | | | | |
|--------------|-----------------------|----|----|----------|-----|--------------------|
| 7. | Type II Manhole | 1 | EA | \$4,000 | /EA | \$4,000.00 |
| 8. | Type I Manhole | 1 | EA | \$6,500 | /EA | \$6,500.00 |
| 9. | Type H Riprap | 9 | CY | \$80 | /CY | \$720.00 |
| 10. | Type L Riprap | 11 | CY | \$50 | /CY | \$550.00 |
| 11. | WQCV Sand Filter Pond | 1 | EA | \$19,000 | /EA | \$19,000.00 |
| Total | | | | | | \$91,534.00 |

M & S Civil Consultants, Inc. (M & S) cannot and does not guarantee the construction cost will not vary from these opinions of probable costs. These opinions represent our best judgment as design professionals familiar with the construction industry and this development in particular. The above is only an estimate of the facility cost in 2020.

Review 1: 10

Review 2: Unresolved. 10 lots are shown on the plat. revise

DRAINAGE & BRIDGE FEES

This site is in the Sand Creek Drainage Basin. The site is proposed to be subdivided into sixteen commercial lots. Drainage fees were paid at the time of the previous platting as Tract C of Claremont Business Park Filing No. 2 (Reception No. 207712506), therefore no additional Drainage Bridge and/or Pond fees are. The imperviousness from Basins B1 and B2 (77%) in the "Final Drainage Report for Claremont Business Park Filing No. 2", prepared by the Matrix Design Group is more than the imperviousness for the proposed site (70%). See Appendix of the "Final Drainage Report for Claremont Business Park Filing No. 2", Revised November 2006, by Matrix Design Group, Inc, for previously paid drainage and bridge fees.

SUMMARY

Development of Claremont Commercial Subdivision Filing No. 3 will not adversely affect the surrounding development. The proposed drainage facilities will adequately convey, detain and route runoff from the onsite & offsite flows to existing facilities. All drainage facilities described herein and shown on the included Proposed Drainage Map (See Appendix) are subject to change being dependent upon individual lot development but owners/developer of the lots shall comply with this final drainage report that will be submitted with the final plat application. Care will be taken to accommodate overland emergency flow routes on site and temporary drainage conditions.

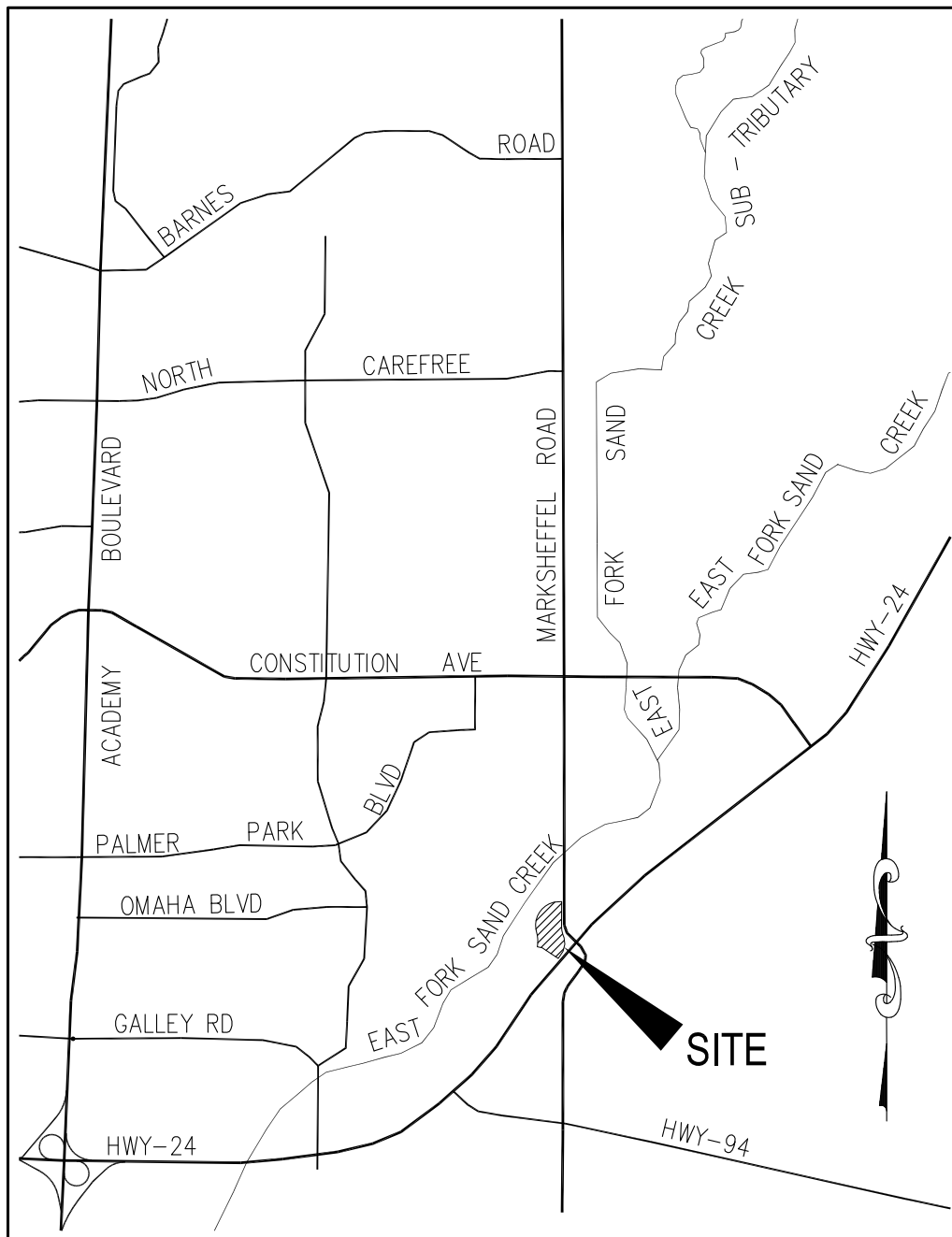
It appears that this may be the calculated impervious of basins B1 & B2. Please **also** state that the previously paid fees were based on 80% impervious as shown in the excerpt attached to your report and to match what is indicated in the letter of intent.

REFERENCES

- 1.) "El Paso County and City of Colorado Springs Drainage Criteria Manual".
- 2.) "Urban Storm Drainage Criteria Manual"
- 3.) SCS Soils Map for El Paso County.
- 4.) Flood Insurance Rate Map (FIRM), Federal Emergency Management Agency, Effective date December 7, 2018.
- 5.) "Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, by Matrix Design Group, Inc.

APPENDIX

VICINITY MAP



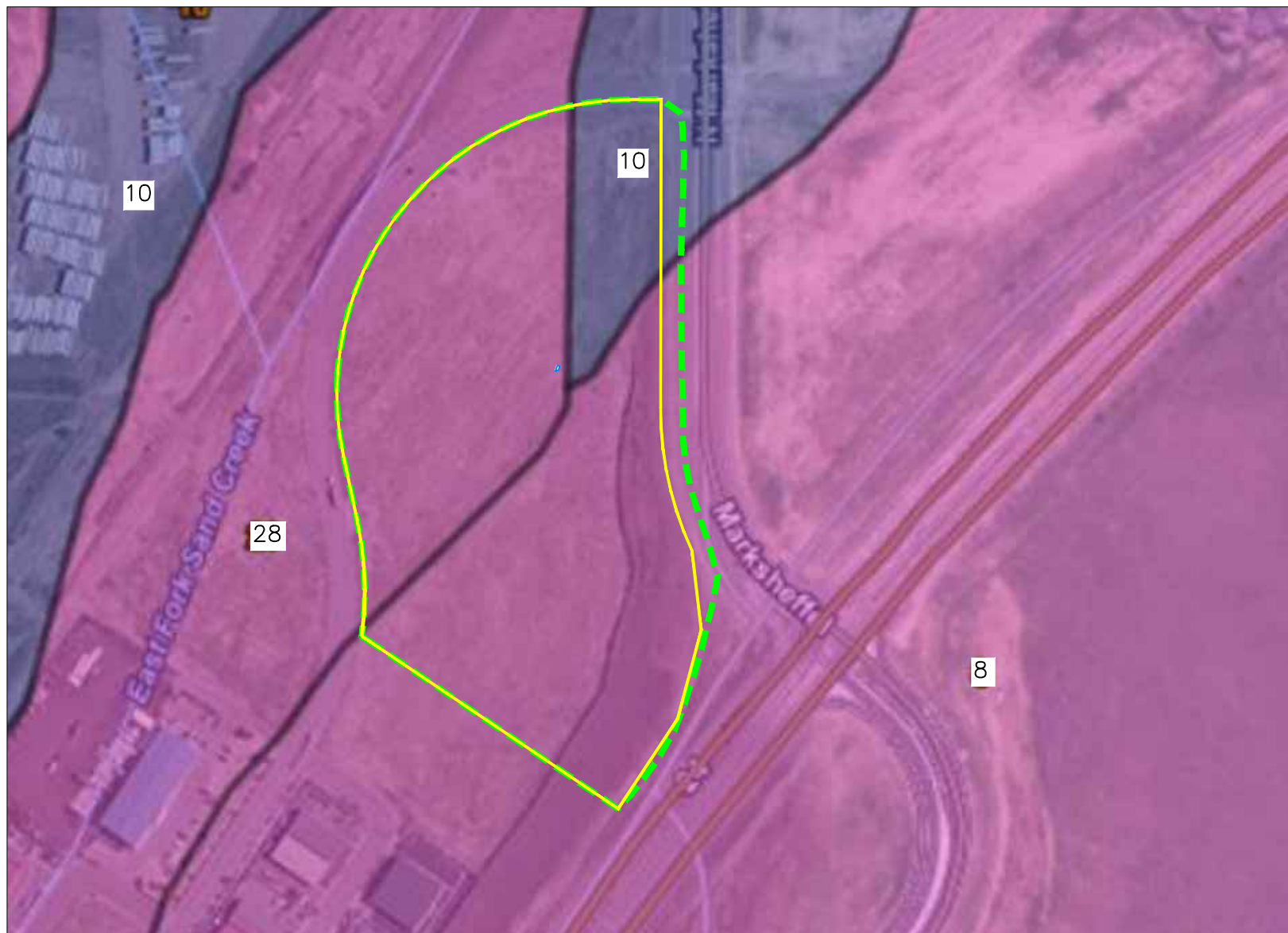
VICINITY MAP

N.T.S.



20 BOULDER CRESCENT, SUITE 110
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

SOILS MAP



Summary by Map Unit — El Paso County Area, Colorado (CO625)

| Map unit symbol | Map unit name | Rating |
|-----------------|---|--------|
| 8 | Blakeland loamy sand, 1 to 9 percent slopes | A |
| 10 | Blendon sandy loam, 0 to 3 percent slopes | B |
| 28 | Ellicott loamy coarse sand, 0 to 5 percent slopes | A |

CLAREMONT COMMERCIAL FILING NO. 2

HYDROLOGIC
TYPE A SOILS



HYDROLOGIC
TYPE B SOILS



SITE BOUNDARY



NOT TO SCALE

SOILS MAP



FEMA FIRM PANEL

National Flood Hazard Layer FIRMette



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

| | | |
|-----------------------------|--|---|
| SPECIAL FLOOD HAZARD AREAS | | Without Base Flood Elevation (BFE) Zone A, V, A99 |
| | | With BFE or Depth Zone AE, AO, AH, VE, AR |
| | | Regulatory Floodway |
| OTHER AREAS OF FLOOD HAZARD | | 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X |
| | | Future Conditions 1% Annual Chance Flood Hazard Zone X |
| | | Area with Reduced Flood Risk due to Levee. See Notes. Zone X |
| | | Area with Flood Risk due to Levee Zone D |
| OTHER AREAS | | Area of Minimal Flood Hazard Zone X |
| | | Effective LOMRs |
| GENERAL STRUCTURES | | Area of Undetermined Flood Hazard Zone D |
| | | Channel, Culvert, or Storm Sewer |
| | | Levee, Dike, or Floodwall |
| OTHER FEATURES | | 20.2 Cross Sections with 1% Annual Chance Water Surface Elevation |
| | | 17.5 Cross Sections with 1% Annual Chance Water Surface Elevation |
| | | Coastal Transect |
| | | Base Flood Elevation Line (BFE) |
| | | Limit of Study |
| | | Jurisdiction Boundary |
| MAP PANELS | | Coastal Transect Baseline |
| | | Profile Baseline |
| | | Hydrographic Feature |
| | | Digital Data Available |
| | | No Digital Data Available |
| | | Unmapped |



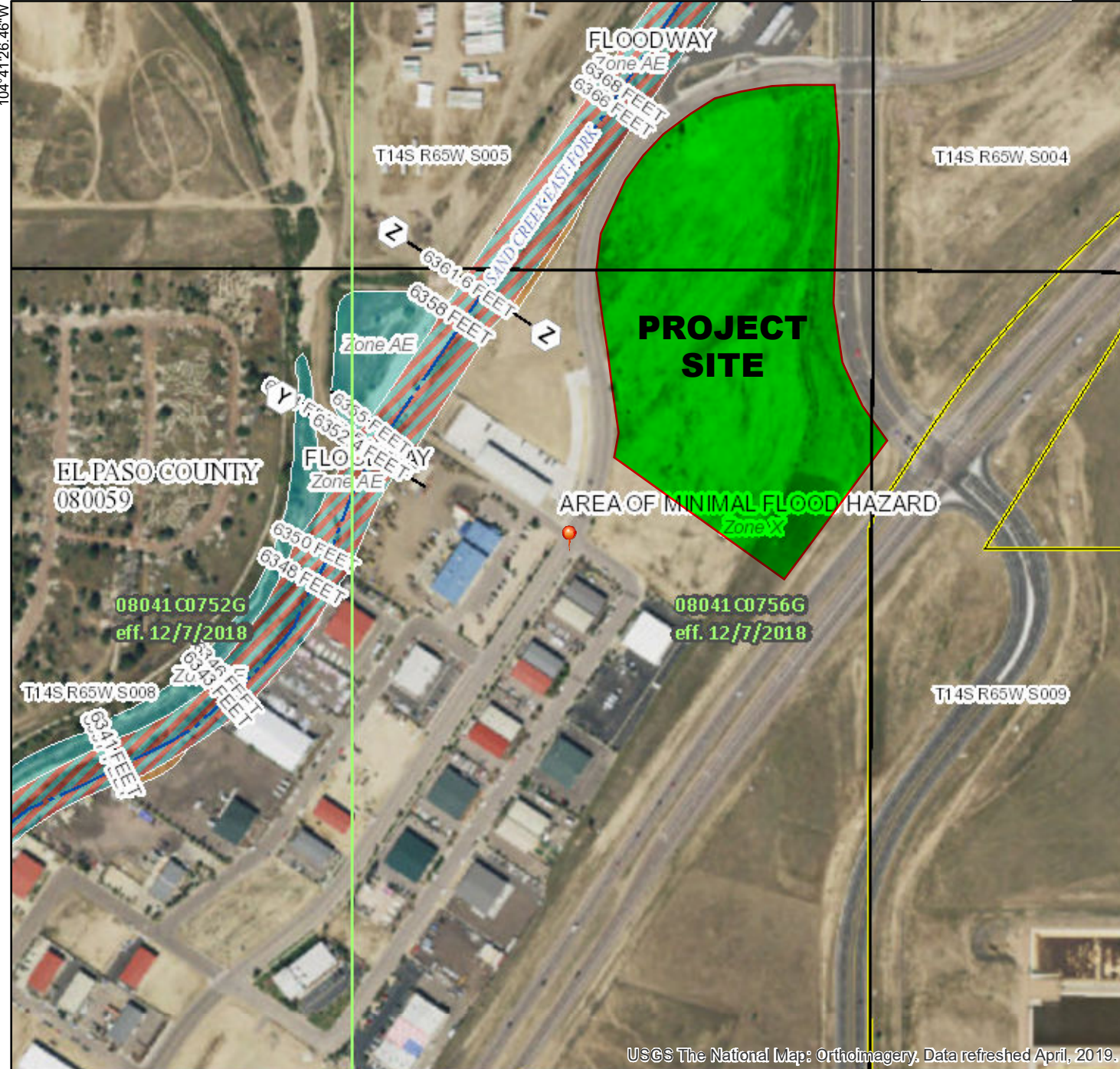
The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **2/13/2020 at 12:07:22 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

38°51'19.98"N



USGS The National Map: Orthoimagery. Data refreshed April, 2019.

0 250 500 1,000 1,500 2,000 Feet 1:6,000

38°50'51.96"N

104°40'49.00"W

HYDROLOGIC CALCULATIONS

FINAL DRAINAGE REPORT FOR CLAREMONT BUSINESS PARK 2 FILING NO.1
PROPOSED DRAINAGE CALCULATIONS
(Area Runoff Coefficient Summary)

| | | | ROOFS 0.73-0.81 COMMERCIAL AREAS 0.81-0.88 ASPHALT DRIVES 0.90-0.96 | | | LANDSCAPED AREAS 0.16-0.41 GRAVEL STORAGE YARD 0.30-0.50 LIGHT INDUST AREAS 0.59-0.70 | | | PARKS 0.12-0.39 GREENBELTS/AGRI. 0.09-0.36 | | | WEIGHTED | |
|--------------|-----------------|--------------------|---|----------------|------------------|---|----------------|------------------|---|----------------|------------------|----------------|------------------|
| BASIN | TOTAL AREA (SF) | TOTAL AREA (Acres) | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | C ₅ | C ₁₀₀ |
| **OS1 | 8359.6 | 0.19 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.19 | 0.09 | 0.36 | 0.09 | 0.36 |
| **OS2 | 13279.8 | 0.30 | 0.00 | 0.81 | 0.88 | 0.00 | 0.59 | 0.70 | 0.30 | 0.09 | 0.36 | 0.09 | 0.36 |
| **OS3 | 214320.8 | 4.92 | 0.00 | 0.81 | 0.88 | 0.00 | 0.59 | 0.70 | 4.92 | 0.09 | 0.36 | 0.09 | 0.36 |
| **OS4 | 9938.1 | 0.23 | 0.00 | 0.90 | 0.96 | 0.00 | 0.16 | 0.41 | 0.23 | 0.09 | 0.36 | 0.09 | 0.36 |
| ***A | 8359.6 | 0.19 | 0.00 | 0.90 | 0.96 | 0.00 | 0.16 | 0.41 | 0.19 | 0.09 | 0.36 | 0.09 | 0.36 |
| ***B | 60660.5 | 1.39 | 1.39 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 0.00 | 0.12 | 0.39 | 0.81 | 0.88 |
| ***C | 13279.8 | 0.30 | 0.00 | 0.90 | 0.96 | 0.00 | 0.16 | 0.41 | 0.30 | 0.09 | 0.36 | 0.09 | 0.36 |
| ***D | 66703.6 | 1.53 | 1.53 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 0.00 | 0.12 | 0.39 | 0.81 | 0.88 |
| ***E | 67533.9 | 1.55 | 1.55 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 0.00 | 0.12 | 0.39 | 0.81 | 0.88 |
| ***F | 15781.4 | 0.36 | 0.00 | 0.73 | 0.81 | 0.00 | 0.30 | 0.50 | 0.36 | 0.12 | 0.39 | 0.12 | 0.39 |
| ***G | 12722.3 | 0.29 | 0.06 | 0.90 | 0.96 | 0.23 | 0.16 | 0.41 | 0.00 | 0.12 | 0.39 | 0.32 | 0.53 |
| H | 31099.0 | 0.71 | 0.00 | 0.90 | 0.96 | 0.00 | 0.16 | 0.41 | 0.71 | 0.09 | 0.36 | 0.09 | 0.36 |
| I | 119584.6 | 2.75 | 2.75 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 0.00 | 0.12 | 0.39 | 0.81 | 0.88 |
| J | 45863.7 | 1.05 | 1.05 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 0.00 | 0.09 | 0.36 | 0.81 | 0.88 |
| K | 18476.1 | 0.42 | 0.42 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 0.00 | 0.09 | 0.36 | 0.81 | 0.88 |
| L | 57315.2 | 1.32 | 0.00 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 1.32 | 0.09 | 0.36 | 0.09 | 0.36 |
| M | 80126.1 | 1.84 | 1.84 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 0.00 | 0.09 | 0.36 | 0.81 | 0.88 |
| N | 20642.4 | 0.47 | 0.00 | 0.81 | 0.88 | 0.00 | 0.16 | 0.41 | 0.47 | 0.12 | 0.39 | 0.12 | 0.39 |
| O | 6997.2 | 0.16 | 0.02 | 0.90 | 0.96 | 0.00 | 0.30 | 0.50 | 0.14 | 0.12 | 0.41 | 0.22 | 0.48 |
| P | 1393.0 | 0.03 | 0.00 | 0.81 | 0.88 | 0.00 | 0.30 | 0.50 | 0.03 | 0.09 | 0.36 | 0.09 | 0.36 |
| Q | 4961.4 | 0.11 | 0.00 | 0.90 | 0.96 | 0.00 | 0.30 | 0.50 | 0.11 | 0.09 | 0.36 | 0.09 | 0.36 |

**Existing undeveloped

***Ultimate build out, developed. Used to size future pond 1 and storm sewer.

Calculated by: GT
Date: 7/24/2020
Checked by: VAS

FINAL DRAINAGE REPORT FOR CLAREMONT BUSINESS PARK 2 FILING NO.1

PROPOSED DRAINAGE CALCULATIONS

(Area Drainage Summary)

| From Area Runoff Coefficient Summary | | | | OVERLAND | | | | STREET / CHANNEL FLOW | | | | Time of Travel (T _t) | | INTENSITY * | | TOTAL FLOWS | |
|--------------------------------------|-----------------------|--------------------|------------------|----------------|----------------|----------------|-------------------------|-----------------------|--------------|-------------------|-------------------------|----------------------------------|----------------|---------------------------|-----------------------------|----------------------------|------------------------------|
| BASIN | AREA TOTAL (Acres) | C ₅ | C ₁₀₀ | C ₅ | Length (ft) | Height (ft) | T _C (min) | Length (ft) | Slope (%) | Velocity (fps) | T _t (min) | *TOTAL (min) | CHECK (min) | I ₅ (in/hr) | I ₁₀₀ (in/hr) | Q ₅ (c.f.s.) | Q ₁₀₀ (c.f.s.) |
| | | From DCM Table 5-1 | | | | | | | | | | | | | | | |
| **OS1 | 0.19 | 0.09 | 0.36 | 0.09 | 40 | 5.0 | 5.0 | 0 | 0.0% | 0.0 | 0.0 | 5.0 | 10.2 | 5.2 | 8.7 | 0.1 | 0.6 |
| **OS2 | 0.30 | 0.09 | 0.36 | 0.09 | 40 | 8.0 | 4.3 | 0 | 0.0% | 0.0 | 0.0 | 4.3 | 10.2 | 5.2 | 8.7 | 0.1 | 1.0 |
| **OS3 | 4.92 | 0.09 | 0.36 | 0.09 | 100 | 2.0 | 14.5 | 637 | 1.7% | 0.6 | 16.5 | 31.0 | 14.1 | 3.6 | 6.1 | 1.6 | 10.7 |
| **OS4 | 0.23 | 0.09 | 0.36 | 0.09 | 20 | 0.5 | 6.0 | 0 | 0.0% | 0.0 | 0.0 | 6.0 | 10.1 | 4.9 | 8.2 | 0.1 | 0.7 |
| ***A | 0.19 | 0.09 | 0.36 | 0.09 | 40 | 5.0 | 5.0 | 0 | 0.0% | 0.0 | 0.0 | 5.0 | 10.2 | 5.2 | 8.7 | 0.1 | 0.6 |
| ***B | 1.39 | 0.81 | 0.88 | 0.81 | 80 | 1.0 | 4.4 | 250 | 1.6% | 2.5 | 1.7 | 6.0 | 11.8 | 4.9 | 8.2 | 5.5 | 10.1 |
| ***C | 0.30 | 0.09 | 0.36 | 0.09 | 40 | 8.0 | 4.3 | 0 | 0.0% | 0.0 | 0.0 | 4.3 | 10.2 | 5.2 | 8.7 | 0.1 | 1.0 |
| ***D | 1.53 | 0.81 | 0.88 | 0.81 | 60 | 1.2 | 3.2 | 350 | 2.0% | 2.8 | 2.1 | 5.3 | 12.3 | 5.1 | 8.5 | 6.3 | 11.5 |
| ***E | 1.55 | 0.81 | 0.88 | 0.81 | 60 | 1.2 | 3.2 | 167 | 2.0% | 2.8 | 1.0 | 4.2 | 11.3 | 5.2 | 8.7 | 6.5 | 11.8 |
| ***F | 0.36 | 0.12 | 0.39 | 0.12 | 60 | 1.2 | 10.9 | 30 | 33.0% | 11.5 | 0.0 | 10.9 | 10.5 | 4.1 | 6.8 | 0.2 | 1.0 |
| ***G | 0.29 | 0.32 | 0.53 | 0.32 | 25 | 0.5 | 5.6 | 0 | 0.0% | 0.0 | 0.0 | 5.6 | 10.1 | 5.0 | 8.4 | 0.5 | 1.3 |
| H | 0.71 | 0.09 | 0.36 | 0.09 | 100 | 17.0 | 7.2 | 0 | 0.0% | 0.0 | 0.0 | 7.2 | 10.6 | 4.6 | 7.8 | 0.3 | 2.0 |
| I | 2.75 | 0.81 | 0.88 | 0.81 | 60 | 1.2 | 3.2 | 425 | 2.0% | 1.4 | 5.0 | 8.2 | 12.7 | 4.4 | 7.4 | 9.8 | 17.9 |
| J | 1.05 | 0.81 | 0.88 | 0.81 | 60 | 1.2 | 3.2 | 200 | 2.0% | 2.8 | 1.2 | 4.4 | 11.4 | 5.2 | 8.7 | 4.4 | 8.0 |
| K | 0.42 | 0.81 | 0.88 | 0.81 | 60 | 1.2 | 3.2 | 175 | 2.0% | 2.8 | 1.0 | 4.3 | 11.3 | 5.2 | 8.7 | 1.8 | 3.2 |
| L | 1.32 | 0.09 | 0.36 | 0.09 | 100 | 17.0 | 7.2 | 0 | 0.0% | 0.0 | 0.0 | 7.2 | 10.6 | 4.6 | 7.8 | 0.5 | 3.7 |
| M | 1.84 | 0.81 | 0.88 | 0.81 | 100 | 1.0 | 5.2 | 400 | 1.5% | 2.4 | 2.7 | 8.0 | 12.8 | 4.5 | 7.5 | 6.7 | 12.2 |
| N | 0.47 | 0.12 | 0.39 | 0.12 | 60 | 1.2 | 10.9 | 30 | 33.0% | 11.5 | 0.0 | 10.9 | 10.5 | 4.1 | 6.8 | 0.2 | 1.3 |
| O | 0.16 | 0.22 | 0.48 | 0.22 | 25 | 0.5 | 6.3 | 0 | 0.0% | 0.0 | 0.0 | 6.3 | 10.1 | 4.8 | 8.1 | 0.2 | 0.6 |
| P | 0.03 | 0.09 | 0.36 | 0.09 | 100 | 17.0 | 7.2 | 0 | 0.0% | 0.0 | 0.0 | 7.2 | 10.6 | 4.6 | 7.8 | 0.0 | 0.1 |
| Q | 0.11 | 0.09 | 0.36 | 0.09 | 25 | 0.5 | 7.1 | 0 | 0.0% | 0.0 | 0.0 | 7.1 | 10.1 | 4.6 | 7.8 | 0.0 | 0.3 |

* Intensity equations assume a minimum travel time of 5 minutes.

**Existing undeveloped

***Ultimate build out, developed. Used to size future pond 1 and storm sewer.

Calculated by: GT
Date: 7/24/2020
Checked by: VAS

FINAL DRAINAGE REPORT FOR CLAREMONT BUSINESS PARK 2 FILING NO.1

PROPOSED DRAINAGE CALCULATIONS

(Basin Routing Summary)

| From Area Runoff Coefficient Summary | | | | OVERLAND | | | | PIPE / CHANNEL FLOW | | | | Time of Travel (T _t) | INTENSITY * | | TOTAL FLOWS | | COMMENTS |
|--------------------------------------|--|-----------------|-------------------|----------------|------------------------------------|----------------|-------------------------|---------------------|--------------|-------------------|-------------------------|----------------------------------|---------------------------|-----------------------------|----------------------------|------------------------------|----------------------------------|
| DESIGN POINT | CONTRIBUTING BASINS DPS AND/OR PIPES | CA ₅ | CA ₁₀₀ | C ₅ | Length (ft) | Height (ft) | T _c (min) | Length (ft) | Slope (%) | Velocity (fps) | T _t (min) | *TOTAL (min) | I ₅ (in/hr) | I ₁₀₀ (in/hr) | Q ₅ (c.f.s.) | Q ₁₀₀ (c.f.s.) | |
| | | | | | | | | | | | | | | | | | |
| ***1 | ***A, ***B OFFSITE DEVELOPED | 1.15 | 1.29 | | TAKEN FROM BASIN B | | | | | | | 6.0 | 4.9 | 8.2 | 5.6 | 10.6 | Proposed PVT 24" Storm Sewer |
| | | | | | | | | | | | | | | | | | |
| ***2 | ***C, ***D OFFSITE DEVELOPED | 1.27 | 1.46 | | TAKEN FROM BASIN D | | | | | | | 5.3 | 5.1 | 8.5 | 6.4 | 12.4 | Proposed PVT 24" Storm Sewer |
| | | | | | | | | | | | | | | | | | |
| ***3 | ***E OFFSITE DEVELOPED | 1.26 | 1.36 | | TAKEN FROM BASIN E (ADJ MIN Tc) | | | | | | | 5.0 | 5.2 | 8.7 | 6.5 | 11.8 | Proposed PVT 24" Storm Sewer |
| | | | | | | | | | | | | | | | | | |
| ***4 | ***F, ***PR3 & ***PR4 OFFSITE DEVELOPED | 3.71 | 4.26 | | TAKEN FROM DESIGN POINT 1 | | | | | | | 6.0 | 4.9 | 8.2 | 18.2 | 35.0 | PVT Sand Filter Basin FSD Pond 1 |
| | | | | | | | | | | | | | | | | | |
| **4 | **OS1, **OS2, **OS3 OFFSITE UNDEVELOPED | 0.49 | 1.95 | | TAKEN FROM DESIGN POINT 4 | | | | | | | 14.1 | 3.6 | 6.1 | 1.8 | 11.8 | 24" PP & FES |
| | | | | | | | | | | | | | | | | | |
| 5 | H, I | 2.29 | 2.67 | | TAKEN FROM BASIN I | | | | | | | 8.2 | 4.4 | 7.4 | 10.1 | 19.8 | 2-15' Type R Inlets |
| | | | | | | | | | | | | | | | | 9.9 | |
| 6 | J, FB DP5 | 0.85 | 1.13 | | TAKEN FROM BASIN J (Adj to Min Tc) | | | | | | | 5.0 | 5.2 | 8.7 | 4.4 | 9.8 | 2-15' Type R Inlets |
| | | | | | | | | | | | | | | | | 4.9 | |
| 7 | K | 0.34 | 0.37 | | TAKEN FROM BASIN K (Adj to Min Tc) | | | | | | | 5.0 | 5.2 | 8.7 | 1.8 | 3.2 | Manhole w/ Grate |
| | | | | | | | | | | | | | | | | | |
| 8 | L, M | 1.61 | 2.09 | | TAKEN FROM BASIN M | | | | | | | 8.0 | 4.5 | 7.5 | 7.2 | 15.7 | PVT Swale or PVT 24" Storm Sewer |
| | | | | | | | | | | | | | | | | | |
| 9 | N, DP8, PR7 & PR10.1 | 5.16 | 6.24 | | TAKEN FROM DESIGN POINT 8 | | | | | | | 8.0 | 4.5 | 7.5 | 23.1 | 46.9 | PVT Sand Filter Basin FSD Pond 2 |
| | | | | | | | | | | | | | | | | | |

* Intensity equations assume a minimum travel time of 5 minutes.

**Existing undeveloped

***Ultimate build out, developed. Used to size future pond 1 and storm sewer.

Calculated by: GT

Date: 7/24/2020

Checked by: VAS

FINAL DRAINAGE REPORT FOR CLAREMONT BUSINESS PARK 2 FILING NO.1
PROPOSED DRAINAGE CALCULATIONS
(Storm Sewer Routing Summary)

| PIPE RUN | Contributing Pipes/Design Points | Equivalent CA_5 | Equivalent CA_{100} | Maximum T_c | Intensity* | | Flow | | Pipe Size | |
|----------|----------------------------------|-------------------|-------------------------------------|---------------|------------|-----------|-------|-----------|-------------|------------|
| | | | | | I_5 | I_{100} | Q_5 | Q_{100} | | |
| ***1 | ***DP1 | 1.15 | 1.29 | 6.0 | 4.9 | 8.2 | 5.6 | 10.6 | PROP 24" PP | |
| ***2 | ***DP2 | 1.27 | 1.46 | 5.3 | 5.1 | 8.5 | 6.4 | 12.4 | PROP 24" PP | |
| ***3 | ***PR1, ***PR2 | 2.41 | 2.75 | 6.0 | 4.9 | 8.2 | 11.8 | 22.6 | PROP 30" PP | |
| ***4 | ***DP3 | 1.26 | 1.36 | 5.0 | 5.2 | 8.7 | 6.5 | 11.8 | PROP 24" PP | |
| ***5 | POND 1 OUTFALL DEVELOPED | 1.48 | 1.89 | 6.0 | 4.9 | 8.2 | 7.2 | 15.5 | PROP 24" PP | |
| **5 | **DP4 OFFSITE UNDEVELOPED | 0.49 | 1.95 | 14.1 | 3.6 | 6.1 | 1.8 | 11.8 | PROP 24" PP | |
| ***5.1 | ***PR5 | 1.48 | 1.89 | 6.0 | 4.9 | 8.2 | 7.2 | 15.5 | PROP 24" PP | |
| ***5.2 | ***PR5.1 | 1.48 | 1.89 | 6.0 | 4.9 | 8.2 | 7.2 | 15.5 | PROP 24" PP | |
| 6 | INLET 1 | 1.45 | 1.23 | 8.2 | 4.4 | 7.4 | 6.4 | 9.1 | PROP 24" PP | |
| 7 | PR6, INLET 2 | 2.28 | 2.47 | 8.2 | 4.4 | 7.4 | 10.1 | 18.3 | PROP 24" PP | |
| 8 | INLET 3 | 0.58 | 0.56 | 5.0 | 5.2 | 8.7 | 3.0 | 4.9 | PROP 18" PP | |
| 9 | PR8, INLET 4 | 0.87 | 1.13 | 5.0 | 5.2 | 8.7 | 4.5 | 9.8 | PROP 24" PP | |
| 10 | PR9, DP7 | 1.21 | 1.50 | 5.0 | 5.2 | 8.7 | 6.3 | 13.0 | PROP 24" PP | |
| 10.1 | PR10 | 1.21 | 1.50 | 5.0 | 5.2 | 8.7 | 6.3 | 13.0 | PROP 24" PP | |
| 11 | POND 2 OUTFALL | 2.95 | 3.17 | 8.0 | 4.5 | 7.5 | 13.2 | 23.8 | PROP 30" PP | |
| 12 | ***PR5.2, PR11 | | FROM UD-DET SHEETS CUMMALATIVE FLOW | | | | | 20.4 | 39.3 | EX 42" RCP |

* Intensity equations assume a minimum travel time of 5 minutes.

DP - Design Point

PR - Pipe Run

FB- Flow By from Design Point

INT- Intercepted Flow from Design Point

Calculated by: GT

Date: 7/24/2020

Checked by: VAS

**Existing undeveloped

***Ultimate build out, developed. Used to size future pond 1 and storm sewer.

HYDRAULIC CALCULATIONS / SFB WQCV CALCULATIONS

Rating Table for 3' wide 6" deep Rectangular Channel

| Project Description | | | | | | |
|--------------------------|--------------------|--------------------|--------------------|--------------------------|-------------------|--|
| Friction Method | | Manning | | | | |
| Solve For | | Formula | | | | |
| | | Discharge | | | | |
| Input Data | | | | | | |
| Roughness Coefficient | | 0.013 | | | | |
| Channel Slope | | 0.003 ft/ft | | | | |
| Normal Depth | | 6.0 in | | | | |
| Bottom Width | | 3.00 ft | | | | |
| Channel Slope (ft/ft) | Discharge (cfs) | Velocity (ft/s) | Flow Area (ft²) | Wetted Perimeter (ft) | Top Width (ft) | |
| 0.003 | 4.46 | 2.97 | 1.5 | 4.0 | 3.00 | |
| 0.005 | 6.30 | 4.20 | 1.5 | 4.0 | 3.00 | |
| 0.008 | 7.72 | 5.15 | 1.5 | 4.0 | 3.00 | |
| 0.010 | 8.92 | 5.94 | 1.5 | 4.0 | 3.00 | |
| 0.013 | 9.97 | 6.65 | 1.5 | 4.0 | 3.00 | |
| 0.015 | 10.92 | 7.28 | 1.5 | 4.0 | 3.00 | |
| 0.018 | 11.79 | 7.86 | 1.5 | 4.0 | 3.00 | |
| 0.020 | 12.61 | 8.41 | 1.5 | 4.0 | 3.00 | |
| 0.023 | 13.37 | 8.92 | 1.5 | 4.0 | 3.00 | |
| 0.025 | 14.10 | 9.40 | 1.5 | 4.0 | 3.00 | |
| 0.028 | 14.79 | 9.86 | 1.5 | 4.0 | 3.00 | |
| 0.030 | 15.44 | 10.30 | 1.5 | 4.0 | 3.00 | |

Claremont Business Park 2 Filing No.1
DRAINAGE PLAN CALCULATIONS
(Pond Volume Calculation)

WQCV POND 1

| Elevation | SF | CF | Storage | |
|-----------|----------|----------|---------|------|
| | | | AF | Sum |
| 6373.00 | 3,690.00 | 0.00 | 0.00 | 0.00 |
| 6374.00 | 4,280.00 | 3,985.00 | 0.09 | 0.09 |
| 6375.00 | 6,051.00 | 5,165.50 | 0.12 | 0.21 |
| 6376.00 | 7,382.00 | 6,716.50 | 0.15 | 0.36 |
| 6376.50 | 8,085.00 | 3,866.75 | 0.09 | 0.45 |

Total = 19,734 CF
Total = 0.5 Ac-ft

Calculated by: DLM
Date: 11/20/2019
Checked by: _____

Claremont Business Park 2 Filing No.1

(PROPOSED CONDITIONS)

| Weighted Percent Imperviousness of Proposed WQ Sand Filter Pond 1 | | | | |
|--|---------------------|----------------------|-------------------------|--------------------|
| Contributing Basins | Area (Acres) | C_s | Impervious % (I) | (Acres)*(I) |
| A | 0.19 | 0.09 | 2 | 0.38 |
| B | 1.39 | 0.81 | 95 | 132.29 |
| C | 0.30 | 0.09 | 2 | 0.61 |
| D | 1.53 | 0.81 | 95 | 145.47 |
| E | 1.55 | 0.81 | 95 | 147.28 |
| F | 0.36 | 0.12 | 7 | 2.54 |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| Totals | 5.33 | | | 428.58 |
| Imperviousness % to FSD | 80.4 | | | |

1.77 A soils 33%
 3.57 B soils 67%
 5.33 total area

Claremont Business Park 2 Filing No.1

EMERGENCY SPILLWAY CALCULATIONS POND 1

| Horizontal Broad-Crested Weir (Eqn 12-20 UDFCD) | | | | |
|---|------|-----|-----------|--------|
| Variable | | | Solve For | |
| <i>C</i> | 3.00 | | L (ft) | H (ft) |
| <i>L</i> | 8.50 | ft | 0.0 | 0.0 |
| <i>H</i> | 1.00 | ft | | |
| <i>Q</i> | | cfs | | |

| | |
|----------------|-------|
| Total <i>Q</i> | 35.10 |
|----------------|-------|

Equation 12-20

$$Q = C_{BCW} L H^{1.5}$$

Where:

Q = discharge (cfs)

*C*_{BCW} = broad-crested weir coefficient (This ranges from 2.6 to 3.0. A value of 3.0 is often used in practice.) See Hydraulic Engineering Circular No. 22 for additional information.

L = broad-crested weir length (ft)

H = head above weir crest (ft)

| Sloping Broad-Crested Weir (Eqn 12-21 UDFCD) | | | | |
|--|------|-----|---------------|---------------|
| Variable | | | Solve For | |
| <i>C</i> | 3.00 | | <i>Z</i> (ft) | <i>H</i> (ft) |
| <i>Z</i> | 4.00 | ft | 0.0 | 0.0 |
| <i>H</i> | 1.00 | ft | | |
| <i>Q</i> | | cfs | | |

Equation 12-21

$$Q = \left(\frac{2}{5}\right) C_{BCW} Z H^{2.5}$$

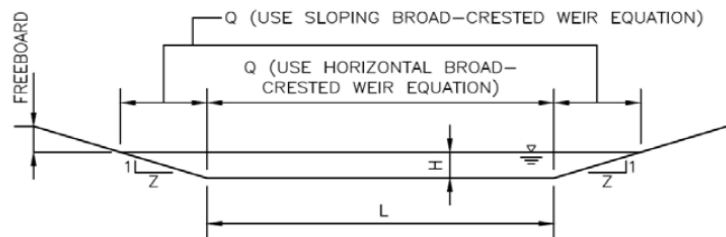
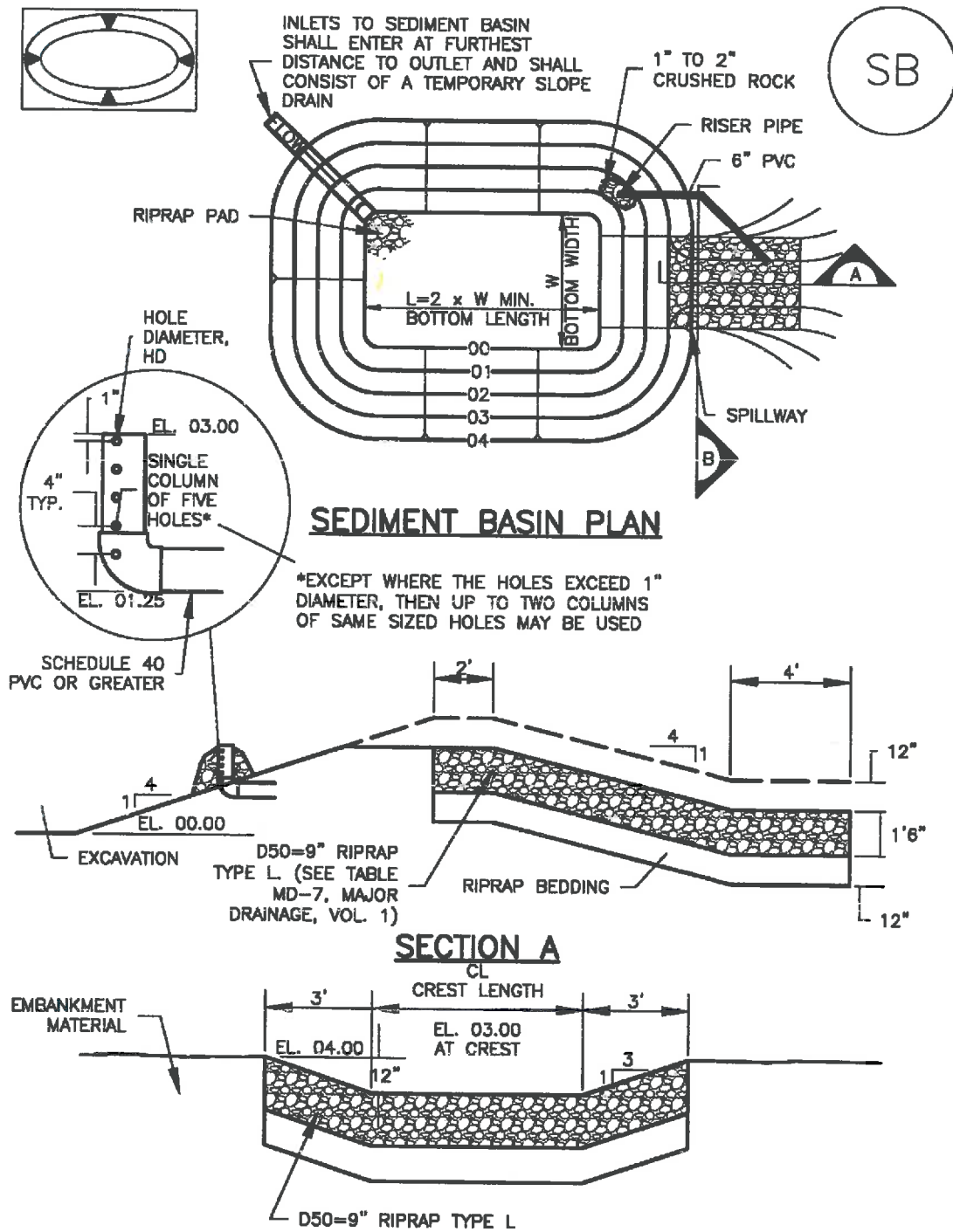


Figure 12-20. Sloping broad-crest weir



TEMPORARY SEDIMENT BASIN DP4

| TABLE SB-1. SIZING INFORMATION FOR STANDARD SEDIMENT BASIN | | | |
|--|------------------------------|----------------------------------|--------------------------|
| Upstream Drainage Area (rounded to nearest acre), (ac) | Basin Bottom Width (W), (ft) | Spillway Crest Length (CL), (ft) | Hole Diameter (HD), (in) |
| 1 | 12 1/2 | 2 | 9/32 |
| 2 | 21 | 3 | 1 3/16 |
| 3 | 28 | 5 | 1/2 |
| 4 | 33 1/2 | 6 | 9/16 |
| 5 | 38 1/2 | 8 | 2 1/32 |
| 6 | 43 | 9 | 2 1/32 |
| 7 | 47 1/4 | 11 | 2 5/32 |
| 8 | 51 | 12 | 2 7/32 |
| 9 | 55 | 13 | 7/8 |
| 10 | 58 1/4 | 15 | 1 5/16 |
| 11 | 61 | 16 | 3 1/32 |
| 12 | 64 | 18 | 1 |
| 13 | 67 1/2 | 19 | 1 1/16 |
| 14 | 70 1/2 | 21 | 1 1/8 |
| 15 | 73 1/4 | 22 | 1 3/16 |

SEDIMENT BASIN INSTALLATION NOTES

- SEE PLAN VIEW FOR:
 - LOCATION OF SEDIMENT BASIN.
 - TYPE OF BASIN (STANDARD BASIN OR NONSTANDARD BASIN).
 - FOR STANDARD BASIN, BOTTOM WIDTH W, CREST LENGTH CL, AND HOLE DIAMETER, HD.
 - FOR NONSTANDARD BASIN, SEE CONSTRUCTION DRAWINGS FOR DESIGN OF BASIN INCLUDING RISER HEIGHT H, NUMBER OF COLUMNS N, HOLE DIAMETER HD AND PIPE DIAMETER D.
- FOR STANDARD BASIN, BOTTOM DIMENSION MAY BE MODIFIED AS LONG AS BOTTOM AREA IS NOT REDUCED.
- SEDIMENT BASINS SHALL BE INSTALLED PRIOR TO ANY OTHER LAND-DISTURBING ACTIVITY THAT RELIES ON ON BASINS AS A STORMWATER CONTROL.
- EMBANKMENT MATERIAL SHALL CONSIST OF SOIL FREE OF DEBRIS, ORGANIC MATERIAL, AND ROCKS OR CONCRETE GREATER THAN 3 INCHES AND SHALL HAVE A MINIMUM OF 15 PERCENT BY WEIGHT PASSING THE NO. 200 SIEVE.
- EMBANKMENT MATERIAL SHALL BE COMPACTED TO AT LEAST 95 PERCENT OF MAXIMUM DENSITY IN ACCORDANCE WITH ASTM D698.
- PIPE SCH 40 OR GREATER SHALL BE USED.
- THE DETAILS SHOWN ON THESE SHEETS PERTAIN TO STANDARD SEDIMENT BASIN(S) FOR DRAINAGE AREAS LESS THAN 15 ACRES. SEE CONSTRUCTION DRAWINGS FOR EMBANKMENT, STORAGE VOLUME, SPILLWAY, OUTLET, AND OUTLET PROTECTION DETAILS FOR ANY SEDIMENT BASIN(S) THAT HAVE BEEN INDIVIDUALLY DESIGNED FOR DRAINAGE AREAS LARGER THAN 15 ACRES.

TEMPORARY SEDIMENT BASIN DP4

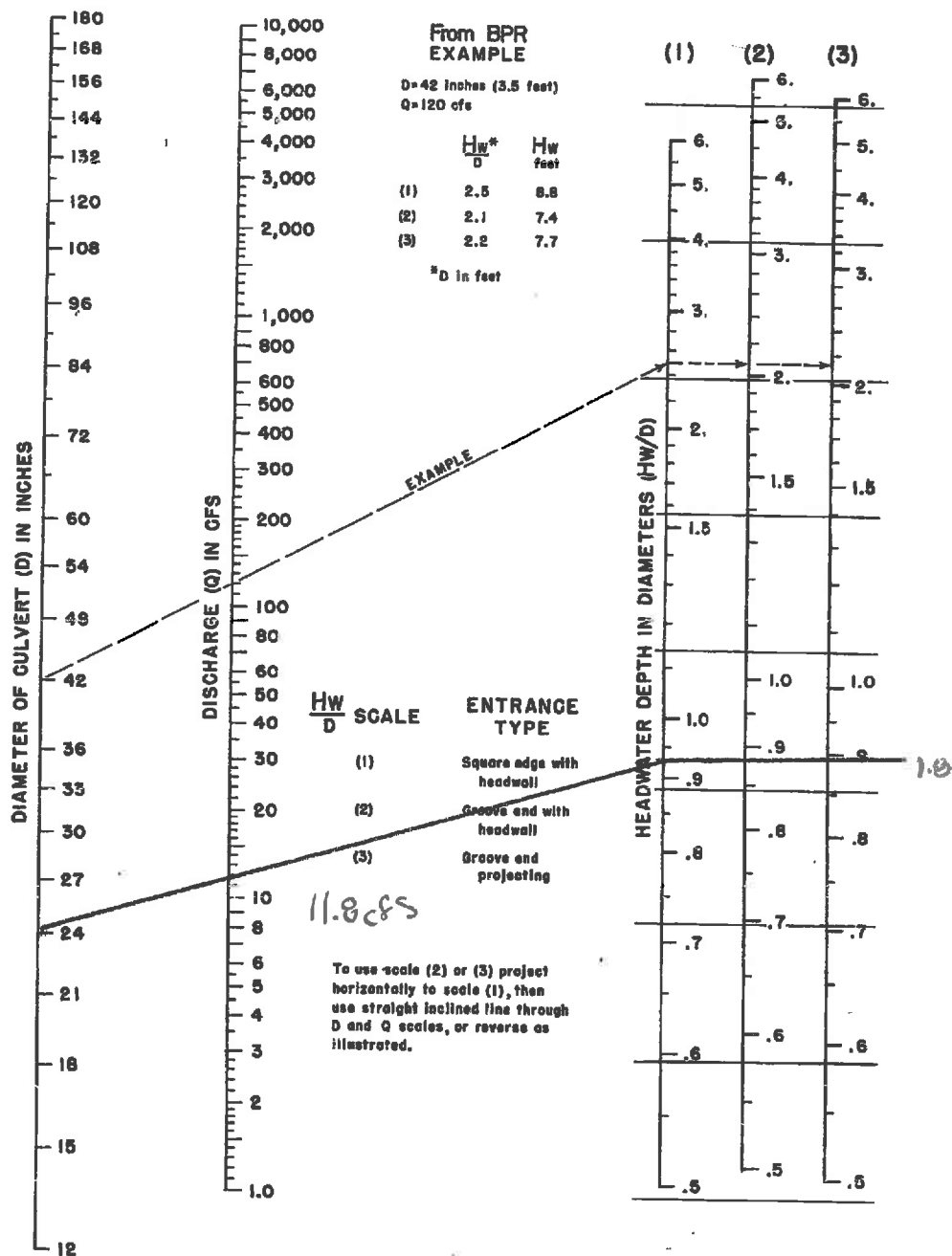


Figure 11-9. Inlet control nomograph—example

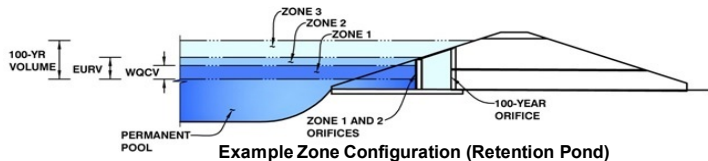
DP4 INLET CONTROL NOMOGRAPH FOR 24" FES

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.02 (February 2020)

Project: Claremont Business Park 2 Filing No.1

Basin ID: WOCV POND 1



Watershed Information

| | | |
|--|-----------|---------|
| Selected BMP Type = | SF | |
| Watershed Area = | 5.33 | acres |
| Watershed Length = | 735 | ft |
| Watershed Length to Centroid = | 325 | ft |
| Watershed Slope = | 0.016 | ft/ft |
| Watershed Imperviousness = | 80.40% | percent |
| Percentage Hydrologic Soil Group A = | 33.0% | percent |
| Percentage Hydrologic Soil Group B = | 67.0% | percent |
| Percentage Hydrologic Soil Groups C/D = | 0.0% | percent |
| Target WQCV Drain Time = | 12.0 | hours |
| Location for 1-hr Rainfall Depths = User Input | | |

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

| | | |
|--|-------|-----------|
| Water Quality Capture Volume (WQCV) = | 0.118 | acre-feet |
| Excess Urban Runoff Volume (EURV) = | 0.505 | acre-feet |
| 2-yr Runoff Volume (P1 = 1.19 in.) = | 0.399 | acre-feet |
| 5-yr Runoff Volume (P1 = 1.5 in.) = | 0.525 | acre-feet |
| 10-yr Runoff Volume (P1 = 1.75 in.) = | 0.629 | acre-feet |
| 25-yr Runoff Volume (P1 = 2 in.) = | 0.749 | acre-feet |
| 50-yr Runoff Volume (P1 = 2.25 in.) = | 0.860 | acre-feet |
| 100-yr Runoff Volume (P1 = 2.52 in.) = | 0.989 | acre-feet |
| 500-yr Runoff Volume (P1 = 2.53 in.) = | 0.994 | acre-feet |
| Approximate 2-yr Detention Volume = | 0.375 | acre-feet |
| Approximate 5-yr Detention Volume = | 0.492 | acre-feet |
| Approximate 10-yr Detention Volume = | 0.603 | acre-feet |
| Approximate 25-yr Detention Volume = | 0.668 | acre-feet |
| Approximate 50-yr Detention Volume = | 0.706 | acre-feet |
| Approximate 100-yr Detention Volume = | 0.749 | acre-feet |

Optional User Overrides

| | |
|-------|-----------|
| 0.118 | acre-feet |
| 0.505 | acre-feet |
| 1.19 | inches |
| 1.50 | inches |
| 1.75 | inches |
| 2.00 | inches |
| 2.25 | inches |
| 2.52 | inches |
| 2.53 | inches |

Define Zones and Basin Geometry

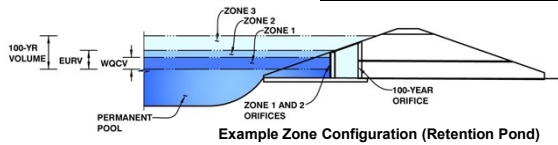
| | | |
|---|-------|-----------------|
| Zone 1 Volume (WQCV) = | 0.118 | acre-feet |
| Zone 2 Volume (100-year - Zone 1) = | 0.631 | acre-feet |
| Select Zone 3 Storage Volume (Optional) = | | acre-feet |
| Total Detention Basin Volume = | 0.749 | acre-feet |
| Initial Surge Volume (ISV) = | N/A | ft ³ |
| Initial Surge Depth (ISD) = | N/A | ft |
| Total Available Detention Depth (H_{total}) = | user | ft |
| Depth of Trickle Channel (H_{TC}) = | N/A | ft |
| Slope of Trickle Channel (S_{TC}) = | N/A | ft/ft |
| Slopes of Main Basin Sides (S_{main}) = | user | H:V |
| Basin Length-to-Width Ratio ($R_{L/W}$) = | user | |

[illegible]

DETENTION BASIN OUTLET STRUCTURE DESIGN

Project: **Claremont Business Park 2 Filing No.1** Version 4.02 (February 2020)

Basin ID: **WQCV POND 1**



Example Zone Configuration (Retention Pond)

| | Estimated Stage (ft) | Estimated Volume (ac-ft) | Outlet Type |
|-------------------|----------------------|--------------------------|----------------------|
| Zone 1 (WQCV) | 1.26 | 0.118 | Filtration Media |
| Zone 2 (100-year) | #VALUE! | 0.631 | Weir&Pipe (Restrict) |
| Zone 3 | | | |
| Total (all zones) | | 0.749 | |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = 2.50 ft (distance below the filtration media surface)
Underdrain Orifice Diameter = 1.63 inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = 0.0 ft²
Underdrain Orifice Centroid = 0.07 feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = N/A ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = N/A inches
Orifice Plate: Orifice Area per Row = N/A inches

Calculated Parameters for Plate
WQ Orifice Area per Row = N/A ft²
Elliptical Half-Width = N/A feet
Elliptical Slot Centroid = N/A feet
Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (optional) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|--------------------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|
| Stage of Orifice Centroid (ft) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Orifice Area (sq. inches) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|--------------------------------|------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Stage of Orifice Centroid (ft) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Orifice Area (sq. inches) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = Not Selected ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice = Not Selected ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter = Not Selected inches

Calculated Parameters for Vertical Orifice
Vertical Orifice Area = Not Selected ft²
Vertical Orifice Centroid = Not Selected feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

Overflow Weir Front Edge Height, H_o = 1.27 ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length = 6.00 feet
Overflow Weir Grate Slope = 0.00 H:V
Horiz. Length of Weir Sides = 2.91 feet
Overflow Grate Open Area % = 70%
Debris Clogging % = 50%

Calculated Parameters for Overflow Weir
Height of Grate Upper Edge, H_u = 1.27 feet
Overflow Weir Slope Length = 2.91 feet
Grate Open Area / 100-yr Orifice Area = 7.70
Overflow Grate Open Area w/o Debris = 12.22 ft²
Overflow Grate Open Area w/ Debris = 6.11 ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = 2.75 ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter = 24.00 inches
Restrictor Plate Height Above Pipe Invert = 12.10 inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate
Outlet Orifice Area = 1.59 ft²
Outlet Orifice Centroid = 0.58 feet
Half-Central Angle of Restrictor Plate on Pipe = 1.58 radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 2.00 ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = 8.50 feet
Spillway End Slopes = 4:00 H:V
Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway
Spillway Design Flow Depth = 0.67 feet
Stage at Top of Freeboard = 3.67 feet
Basin Area at Top of Freeboard = 0.19 acres
Basin Volume at Top of Freeboard = 0.45 acre-ft

Routed Hydrograph Results

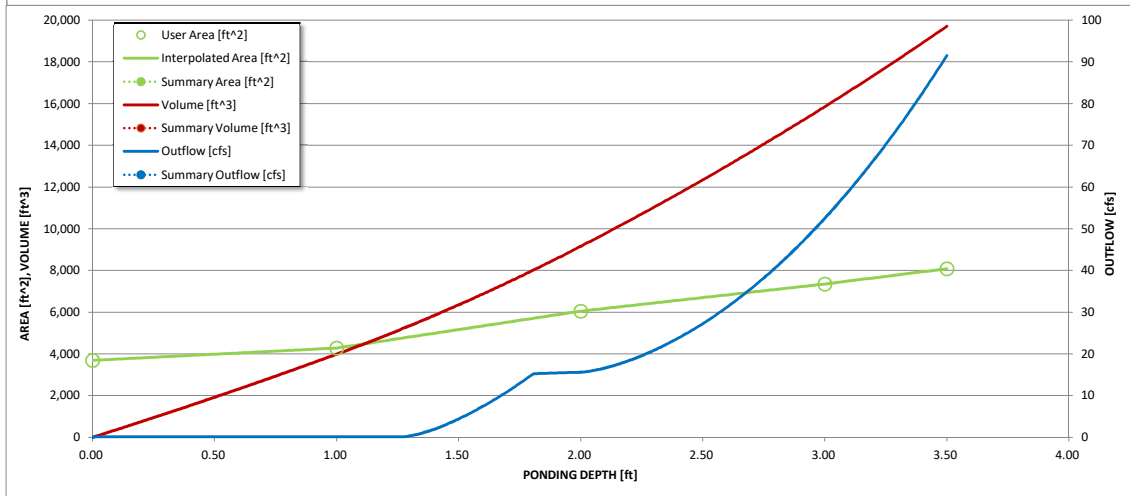
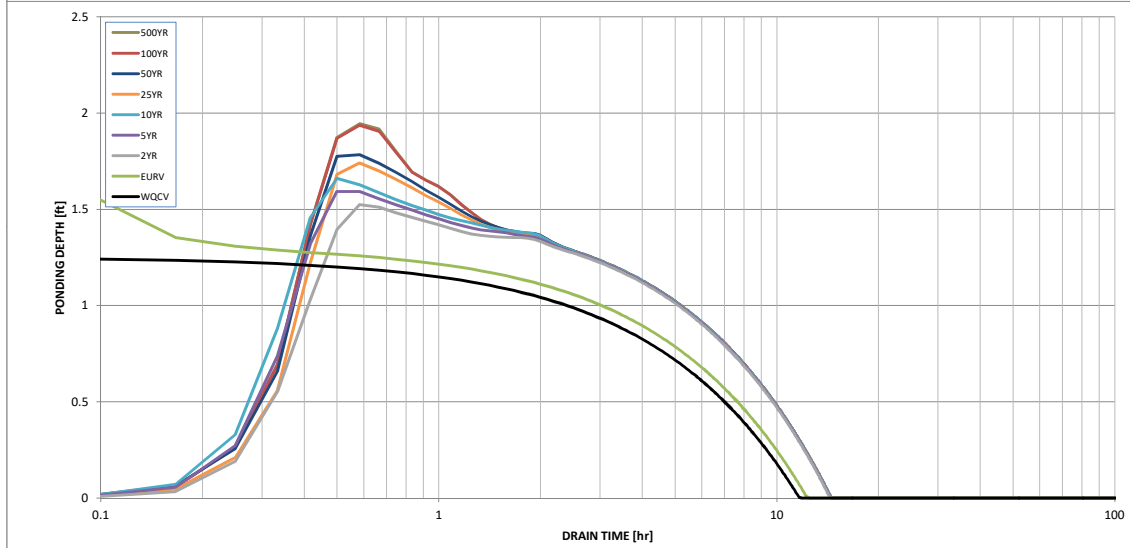
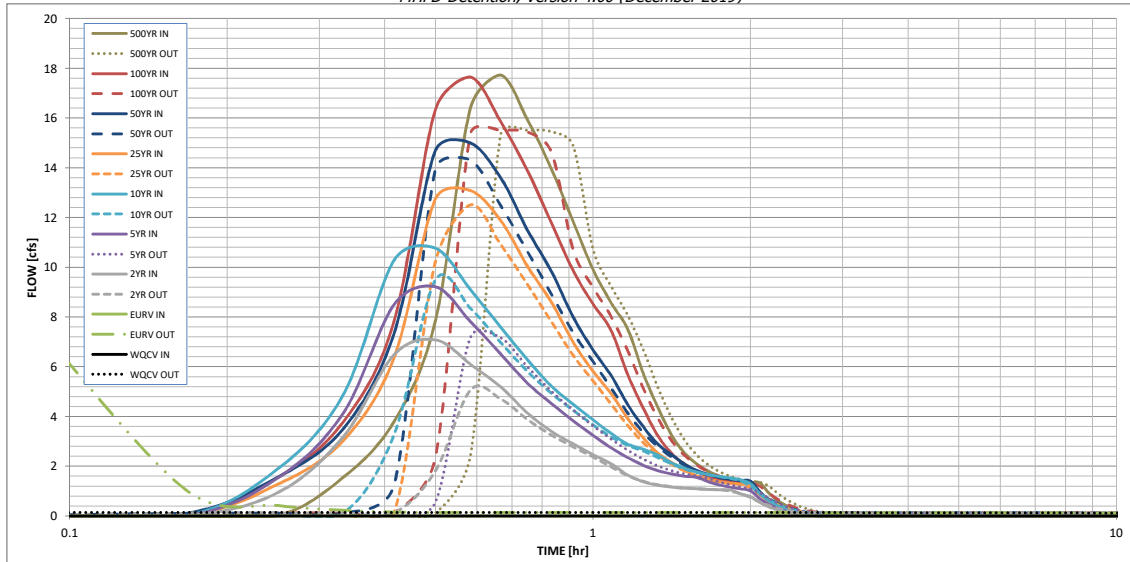
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

| | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year | 500 Year |
|---|------------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|----------------|----------------|
| Design Storm Return Period | N/A | N/A | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 | 2.53 |
| One-Hour Rainfall Depth (in) | N/A | N/A | 0.399 | 0.525 | 0.629 | 0.749 | 0.860 | 0.989 | 0.994 |
| CUHP Runoff Volume (acre-ft) | N/A | N/A | 0.399 | 0.525 | 0.629 | 0.749 | 0.860 | 0.989 | 0.994 |
| Inflow Hydrograph Volume (acre-ft) | N/A | N/A | 0.1 | 0.9 | 1.6 | 3.4 | 4.5 | 5.9 | 5.9 |
| CUHP Predevelopment Peak Q (cfs) | N/A | N/A | 0.02 | 0.16 | 0.31 | 0.64 | 0.84 | 1.11 | 1.12 |
| OPTIONAL Override Predevelopment Peak Q (cfs) | N/A | N/A | 7.1 | 9.2 | 10.8 | 13.1 | 15.0 | 17.6 | 17.7 |
| Predevelopment Unit Peak Flow, q (cfs/acre) | N/A | N/A | 5.1 | 7.2 | 9.5 | 12.5 | 14.3 | 15.5 | 15.5 |
| Peak Inflow Q (cfs) | N/A | N/A | 8.2 | 5.8 | 3.7 | 3.2 | 2.6 | 2.6 | |
| Peak Outflow Q (cfs) | N/A | N/A | | | | | | | |
| Ratio Peak Outflow to Predevelopment Q | N/A | N/A | | | | | | | |
| Structure Controlling Flow | Filtration Media | Overflow Weir 1 | Overflow Weir 1 | Overflow Weir 1 | Overflow Weir 1 | Overflow Weir 1 | Overflow Weir 1 | Outlet Plate 1 | Outlet Plate 1 |
| Max Velocity through Grate 1 (fps) | N/A | 0.65 | 0.39 | 0.6 | 0.8 | 1.0 | 1.1 | 1.3 | 1.3 |
| Max Velocity through Grate 2 (fps) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Time to Drain 97% of Inflow Volume (hours) | 11 | 11 | 13 | 13 | 13 | 12 | 12 | 11 | 11 |
| Time to Drain 99% of Inflow Volume (hours) | 12 | 12 | 14 | 14 | 14 | 14 | 14 | 13 | 13 |
| Maximum Ponding Depth (ft) | 1.26 | 1.62 | 1.52 | 1.59 | 1.66 | 1.74 | 1.78 | 1.94 | 1.94 |
| Area at Maximum Ponding Depth (acres) | 0.11 | 0.12 | 0.12 | 0.12 | 0.12 | 0.13 | 0.13 | 0.14 | 0.14 |
| Maximum Volume Stored (acre-ft) | 0.118 | 0.159 | 0.148 | 0.157 | 0.164 | 0.174 | 0.180 | 0.200 | 0.202 |

Per resolution 16-426 of the BoCC, on-site WQCV is required but on-site stormwater detention is not required per the FDR for Claremont Business Park Filing No. 2.

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.00 (December 2019)



| S-A-V-D Chart Axis Override | X-axis | Left Y-axis | Right Y-axis |
|-----------------------------|--------|-------------|--------------|
| minimum bound | | | |
| maximum bound | | | |

Claremont Business Park 2 Filing No.1
DRAINAGE REPORT DRAINAGE CALCULATIONS
(Pond Volume Calculation)

WQCV POND 2

| Elevation | SF | CF | Storage | |
|-----------|----------|----------|---------|------|
| | | | AF | Sum |
| 6364.45 | 2,957.00 | 0.00 | 0.00 | 0.00 |
| 6365.00 | 3,660.00 | 1,819.68 | 0.04 | 0.04 |
| 6366.00 | 4,942.00 | 4,301.00 | 0.10 | 0.14 |
| 6367.00 | 6,327.00 | 5,634.50 | 0.13 | 0.27 |
| 6368.00 | 7,808.00 | 7,067.50 | 0.16 | 0.43 |

Total = 18,823 CF

Total = 0.4 Ac-ft

Calculated by: GT

Date: 4/27/2020

Checked by: _____

Claremont Business Park 2 Filing No.1

(PROPOSED CONDITIONS)

| <i>Weighted Percent Imperviousness of Proposed WQ Sand Filter Pond 2</i> | | | | |
|--|---------------------|----------------------|-------------------------|--------------------|
| <i>Contributing Basins</i> | <i>Area (Acres)</i> | <i>C_s</i> | <i>Impervious % (I)</i> | <i>(Acres)*(I)</i> |
| <i>H</i> | 0.71 | 0.09 | 2 | 1.43 |
| <i>I</i> | 2.75 | 0.81 | 95 | 260.80 |
| <i>J</i> | 1.05 | 0.81 | 95 | 100.02 |
| <i>K</i> | 0.42 | 0.81 | 2 | 0.85 |
| <i>L</i> | 1.32 | 0.09 | 2 | 2.63 |
| <i>M</i> | 1.84 | 0.81 | 95 | 174.75 |
| <i>N</i> | 0.47 | 0.12 | 7 | 3.32 |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| <i>Totals</i> | <i>8.57</i> | | | <i>543.80</i> |
| <i>Imperviousness of WQ Pond 2</i> | 63.5 | | | |

8.57 B soils
8.57 total area

Claremont Business Park 2 Filing No.1

EMERGENCY SPILLWAY CALCULATIONS POND 2

| Horizontal Broad-Crested Weir (Eqn 12-20 UDFCD) | | | | |
|---|-------|-----|-----------|--------|
| Variable | | | Solve For | |
| C | 3.00 | | L (ft) | H (ft) |
| L | 12.50 | ft | 0.0 | 0.0 |
| H | 1.00 | ft | | |
| Q | | cfs | | |

| | |
|---------|-------|
| Total Q | 47.10 |
|---------|-------|

Equation 12-20

$$Q = C_{BCW} L H^{1.5}$$

Where:

Q = discharge (cfs)

C_{BCW} = broad-crested weir coefficient (This ranges from 2.6 to 3.0. A value of 3.0 is often used in practice.) See Hydraulic Engineering Circular No. 22 for additional information.

L = broad-crested weir length (ft)

H = head above weir crest (ft)

| Sloping Broad-Crested Weir (Eqn 12-21 UDFCD) | | | | |
|--|------|-----|-----------|--------|
| Variable | | | Solve For | |
| C | 3.00 | | Z (ft) | H (ft) |
| Z | 4.00 | ft | 0.0 | 0.0 |
| H | 1.00 | ft | | |
| Q | | cfs | | |

Equation 12-21

$$Q = \left(\frac{2}{5}\right) C_{BCW} Z H^{2.5}$$

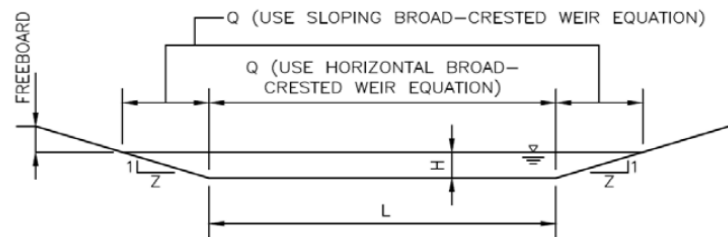
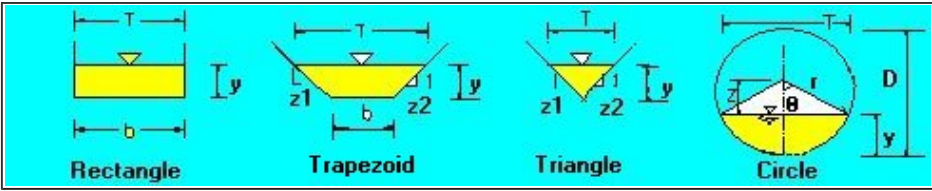


Figure 12-20. Sloping broad-crest weir

| The open channel flow calculator | | | |
|---|---|--|---|
| <p style="color: red;">Select Channel Type:</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">Trapezoid ▾</div> |  <div style="display: flex; justify-content: space-around; font-size: small;"> Rectangle Trapezoid Triangle Circle </div> | | |
| <div style="border: 1px solid black; padding: 2px; display: inline-block;">Velocity(V)&Discharge(Q) ▾</div> | <p style="color: red;">Select unit system:</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">Feet(ft) ▾</div> | | |
| <p>Channel slope: <input type="text" value=".023"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft/ft</div> | <p>Water depth(y): <input type="text" value="1"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft</div> | <p>Bottom width(b) <input type="text" value="12.5"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft</div> | |
| <p>Flow velocity 7.9518</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft/s</div> | <p>LeftSlope (Z1): <input type="text" value="4"/> to 1 (H:V)</p> | | <p>RightSlope (Z2): <input type="text" value="10"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">to 1 (H:V)</div> |
| <p>Flow discharge 155.0594</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft^3/s</div> | <p>Input n value <input type="text" value="0.023"/> or select n</p> | | |
| <div style="border: 1px solid black; padding: 2px; display: inline-block;">Calculate!</div> | <p>Status: Calculation finished</p> | <div style="border: 1px solid black; padding: 2px; display: inline-block;">Reset</div> | |
| <p>Wetted perimeter <input type="text" value="26.67"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft</div> | <p>Flow area <input type="text" value="19.5"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft^2</div> | <p>Top width(T) <input type="text" value="26.5"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft</div> | |
| <p>Specific energy <input type="text" value="1.98"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft</div> | <p>Froude number <input type="text" value="1.63"/></p> | | <p>Flow status <input type="text" value="Supercritical flow"/></p> |
| <p>Critical depth <input type="text" value="1.31"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft</div> | <p>Critical slope <input type="text" value="0.008"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft/ft</div> | <p>Velocity head <input type="text" value="0.98"/></p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">ft</div> | |

Copyright 2000 Dr. Xing Fang, Department of Civil Engineering, Lamar University.

?

POND 2 EMERGENCY SPILLWAY



Material and Performance Specification Sheet

North American Green
14649 Highway 41 North
Evansville, IN 47725
800-772-2040
FAX: 812-867-0247
www.nagreen.com

A **tensar** Company

SC150 Erosion Control Blanket

The extended-term double net erosion control blanket shall be a machine-produced mat of 70% agricultural straw and 30% coconut fiber with a functional longevity of up to 24 months. (NOTE: functional longevity may vary depending upon climatic conditions, soil, geographical location, and elevation). The blanket shall be of consistent thickness with the straw and coconut evenly distributed over the entire area of the mat. The blanket shall be covered on the top side with a heavyweight photodegradable polypropylene netting having ultraviolet additives to delay breakdown and an approximate 0.63 x 0.63 (1.59 x 1.59 cm) mesh, and on the bottom side with a lightweight photodegradable polypropylene netting with an approximate 0.50 x 0.50 in (1.27 x 1.27 cm) mesh. The blanket shall be sewn together on 1.50 inch (3.81 cm) centers with degradable thread.

The SC150 shall meet requirements established by the Erosion Control Technology Council (ECTC) Specification and the US Department of Transportation, Federal Highway Administration's (FHWA) *Standard Specifications for Construction of Roads and Bridges on Federal Highway Projects, FP-03 Section 713.17 as a type 3.B Extended-term Erosion Control Blanket*.

The SC150 is also available with the DOT System™, which consists of installation staple patterns clearly marked on the erosion control blanket with environmentally safe paint. The blanket shall be manufactured with a colored thread stitched along both outer edges (approximately 2-5 inches [5-12.5 cm] from the edge) as an overlap guide for adjacent mats.

| Material Content | | |
|------------------|---|---|
| Matrix | 70% Straw Fiber | 0.35 lbs/yd ² (0.19 kg/m ²) |
| | 30% Coconut Fiber | 0.15 lbs/yd ² (0.08 kg/m ²) |
| Nettings | Top - Heavyweight photodegradable with UV additives | 3.0 lb/1000 ft ² (1.47 kg/100 m ²) |
| | Bottom - Lightweight Photodegradable | 1.5 lb/1000 ft ² (0.73 kg/100 m ²) |
| Thread | Degradable | |

SC150 is available in the following standard roll sizes:

| | | |
|--------------|---|---|
| Width | 6.67 ft (2.03 m) | 16 ft (4.87 m) |
| Length | 108 ft (32.92 m) | 108 ft (32.92 m) |
| Weight ± 10% | 44 lbs (19.95 kg) | 105.6 lbs (47.9 kg) |
| Area | 80.0 yd ² (66.9 m ²) | 192 yd ² (165.5 m ²) |

Index Value Properties:

| Property | Test Method | Typical |
|-----------------------|-----------------|--|
| Thickness | ASTM D6525 | 0.39 in (9.91 mm) |
| Resiliency | ECTC Guidelines | 75% |
| Water Absorbency | ASTM D1117 | 285% |
| Mass/Unit Area | ASTM 6475 | 11.44 oz/yd ² (388 g/m ²) |
| Swell | ECTC Guidelines | 30% |
| Smolder Resistance | ECTC Guidelines | Yes |
| Stiffness | ASTM D1388 | 1.11 oz-in |
| Light Penetration | ECTC Guidelines | 8.7% |
| Tensile Strength - MD | ASTM D6818 | 146.6 lbs/ft (2.17 kN/m) |
| Elongation - MD | ASTM D6818 | 26.9% |
| Tensile Strength - TD | ASTM D6818 | 147.6 lbs/ft (2.19 kN/m) |
| Elongation - TD | ASTM D6818 | 25.2% |

Bench Scale Testing* (NTPEP):

| Test Method | Parameters | Results |
|-----------------------------------|-------------------------------------|-----------------------------|
| ECTC Method 2 Rainfall | 50 mm (2 in)/hr for 30 min | SLR** = 5.47 |
| | 100mm (4 in)/hr for 30 min | SLR** = 5.67 |
| | 150 mm (6 in)/hr for 30 min | SLR** = 5.88 |
| ECTC Method 3 Shear Resistance | Shear at 0.50 inch soil loss | 2.72 lbs/ft ² |
| ECTC Method 4 Germination | Top Soil, Fescue, 21 day incubation | 538% improvement of biomass |

* Bench Scale tests should not be used for design purposes
** Soil Loss Ratio = Soil loss with Bare Soil/Soil Loss with RECP (soil loss is based on regression analysis)

Performance Design Values:

| Maximum Permissible Shear Stress | |
|----------------------------------|----------------------------------|
| Unvegetated Shear Stress | 2.00 lbs/ft ² (96 Pa) |
| Unvegetated Velocity | 8.00 ft/s (2.44 m/s) |

| Slope Design Data: C Factors | | | |
|------------------------------|---------------------|-----------|-------|
| | Slope Gradients (S) | | |
| Slope Length (L) | ≤ 3:1 | 3:1 - 2:1 | ≥ 2:1 |
| ≤ 20 ft (6 m) | 0.001 | 0.048 | 0.100 |
| 20-50 ft | 0.051 | 0.079 | 0.145 |
| ≥ 50 ft (15.2 m) | 0.10 | 0.110 | 0.190 |

| Roughness Coefficients- Unveg. | |
|--------------------------------|---------------|
| Flow Depth | Manning's n |
| ≤ 0.50 ft (0.15 m) | 0.050 |
| 0.50 - 2.0 ft | 0.050 - 0.018 |
| ≥ 2.0 ft (0.60 m) | 0.018 |



Product Participant of:



PROJECT: CLAREMONT BUSINESS PARK 2 FIL. 1

DATE: 4-21-20

STORM 10 OUTFALL LOW TALL WATER BASIN

RIPPRAP SIZING FIG. 9-38 (UDFCD)

$$Q_{100} = 14.3 \text{ cfs} \quad 24" \text{ ADS}$$

$$Q/D^{2.5} \leq 6.0 \quad 14.3/2^{2.5} = 2.53 \leq 6.0 \quad \text{OK}$$

$$Q/D^{1.5} = 14.3/2^{1.5} = 5.06 \quad \text{Assume } 0.4 = 4\epsilon/D$$

FROM FIG 9-38 USE TYPE L RIPRAP $T = 2D_{50} = 2(9") = 18"$

STORM 7 OUTFALL LOW TALL WATER BASIN

RIPPRAP SIZING FIG 9-38 (UDFCD)

$$Q_{100} = 16.6 \text{ cfs} \quad 24" \text{ ADS}$$

$$Q/D^{2.5} \leq 6.0 \quad 16.6/2^{2.5} = 2.93 \leq 6.0 \quad \text{OK}$$

$$Q/D^{1.5} = 16.6/2^{1.5} = 5.87 \quad \text{Assume } 0.4 = 4\epsilon/D$$

FROM FIG 9-38 USE TYPE L RIPRAP $T = 2D_{50} = 2(9") = 18"$

DP 8 4% RUNDOWN OUTFALL

RIPPRAP SIZING FIG 12-21 EMBANKMENT PROTECTION

$$Q_{100} = 16.7 \text{ cfs} \quad 3' \text{ WIDTH}$$

$$\text{UNIT DISCHARGE (cfs/ft)} = 15.7/3 = 5.23 \quad 4:1 = 25\% \text{ LONGITUDINAL SLOPE}$$

FROM FIG 12-21 USE TYPE H RIPRAP $D_{50} = 18"$

$$H_a = \frac{(H + Y_n)}{2}$$

Equation 9-19

Where the maximum value of H_a shall not exceed H , and:

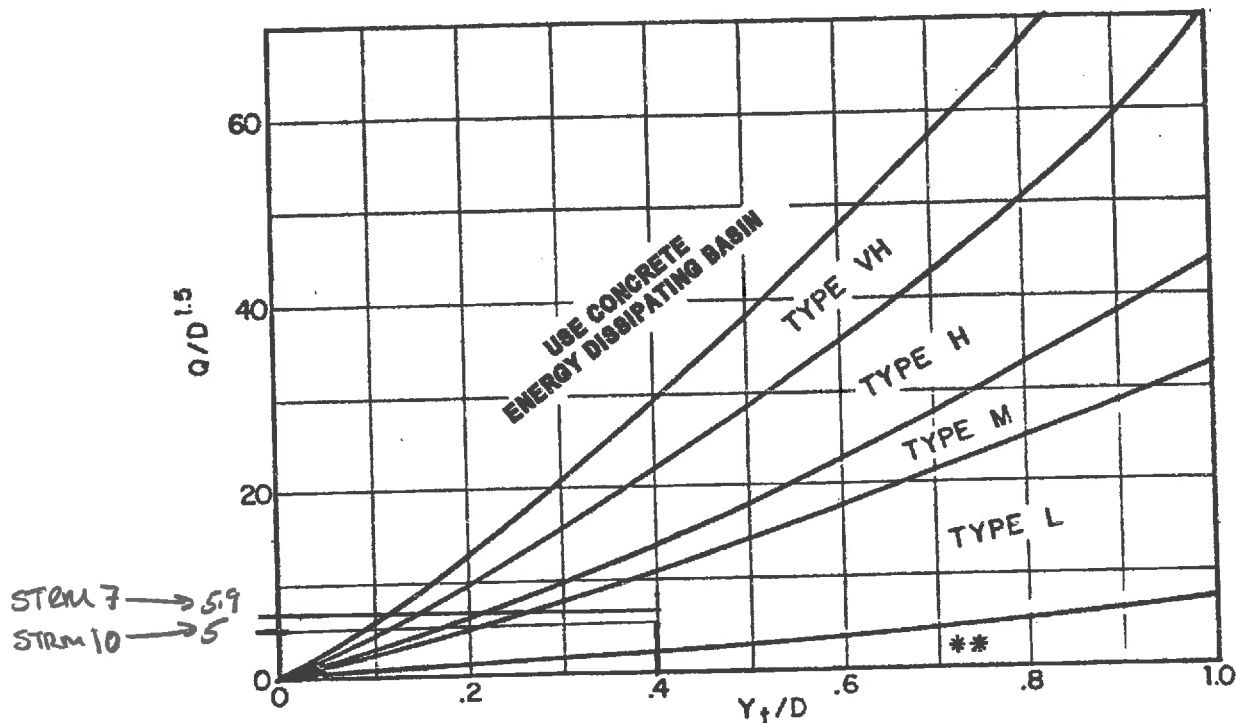
D_a = parameter to use in place of D in Figure 9-38 when flow is supercritical (ft)

D_c = diameter of circular culvert (ft)

H_a = parameter to use in place of H in Figure 9-39 when flow is supercritical (ft)

H = height of rectangular culvert (ft)

Y_n = normal depth of supercritical flow in the culvert (ft)



Use D_a instead of D whenever flow is supercritical in the barrel.
 ** Use Type L for a distance of $3D$ downstream.

RIPRAP SIZING FOR LOW TAILWATER BASINS POND 2

Figure 9-38. Riprap erosion protection at circular conduit outlet (valid for $Q/D^{2.5} \leq 6.0$)

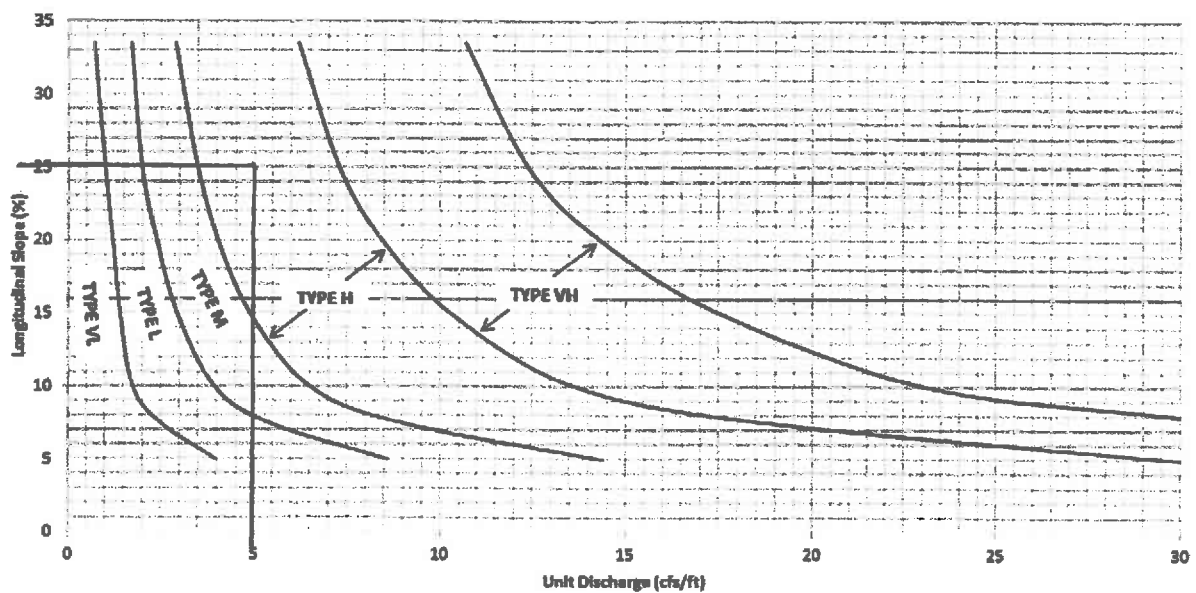
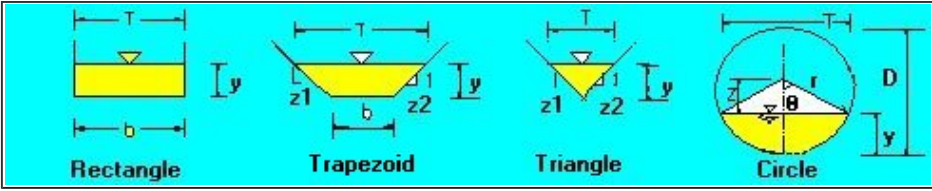


Figure 12-21. Embankment protection details and rock sizing chart (adapted from Arapahoe County)

RIPRAP SIZING FOR RUNDOWN DP8 POND 2

| The open channel flow calculator | | | |
|--|--|--|---|
| <p style="color: red;">Select Channel Type:</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">Triangle ▾</div> |  <div style="display: flex; justify-content: space-around; font-weight: bold; font-size: small;"> Rectangle Trapezoid Triangle Circle </div> | | |
| <div style="border: 1px solid black; padding: 2px; display: inline-block;">Velocity(V)&Discharge(Q) ▾</div> | <p style="color: red;">Select unit system:</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">Feet(ft) ▾</div> | | |
| <p>Channel slope: <input type="text" value="0.12"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/ft</div> | <p>Water depth(y): <input type="text" value="0.4"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Bottom W(b) <input type="text" value="0"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | |
| <p>Flow velocity 6.7989</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/s</div> | <p>LeftSlope (Z1): <input type="text" value="3"/> to 1 (H:V)</p> | | <p>RightSlope (Z2): <input type="text" value="3"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">to 1 (H:V)</div> |
| <p>Flow discharge 3.2635</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft^3/s</div> | <p>Input n value <input type="text" value="0.025"/> or select n</p> | | |
| <div style="border: 1px solid black; padding: 2px; width: 100px;">Calculate!</div> | <p>Status: Calculation finished</p> | <div style="border: 1px solid black; padding: 2px; width: 100px;">Reset</div> | |
| <p>Wetted perimeter 2.53</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Flow area 0.48</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft^2</div> | | <p>Top width(T) 2.4</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> |
| <p>Specific energy 1.12</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Froude number 2.68</p> | | <p>Flow status Supercritical flow</p> |
| <p>Critical depth 0.59</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Critical slope 0.0146</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/ft</div> | | <p>Velocity head 0.72</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> |

Copyright 2000 Dr. Xing Fang, Department of Civil Engineering, Lamar University.

DP 7 SWALE TO CDOT TYPE C INLET

| The open channel flow calculator | | | |
|---|---|--|--|
| <p style="color: red;">Select Channel Type:</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">Trapezoid ▾</div> | <div style="display: flex; justify-content: space-around; font-weight: bold; font-size: small;"> Rectangle Trapezoid Triangle Circle </div> | | |
| <div style="border: 1px solid black; padding: 2px; display: inline-block;">Velocity(V)&Discharge(Q) ▾</div> | <p style="color: red;">Select unit system:</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">Feet(ft) ▾</div> | | |
| <p>Channel slope: <input style="width: 50px;" type="text" value="0.005"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/ft</div> | <p>Water depth(y): <input style="width: 50px;" type="text" value="1.02"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Bottom width(b) <input style="width: 50px;" type="text" value="2"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | |
| <p>Flow velocity 3.0255</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/s</div> | <p>LeftSlope (Z1): <input style="width: 50px;" type="text" value="3"/> to 1 (H:V)</p> | | <p>RightSlope (Z2): <input style="width: 50px;" type="text" value="3"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">to 1 (H:V)</div> |
| <p>Flow discharge 15.615</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft^3/s</div> | <p>Input n value <input style="width: 50px;" type="text" value="0.025"/> or select n</p> | | |
| <div style="border: 1px solid black; padding: 2px; display: inline-block;">Calculate!</div> | <p>Status: Calculation finished</p> | <div style="border: 1px solid black; padding: 2px; display: inline-block;">Reset</div> | |
| <p>Wetted perimeter <input style="width: 50px;" type="text" value="8.45"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Flow area <input style="width: 50px;" type="text" value="5.16"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft^2</div> | <p>Top width(T) <input style="width: 50px;" type="text" value="8.12"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | |
| <p>Specific energy <input style="width: 50px;" type="text" value="1.16"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Froude number <input style="width: 50px;" type="text" value="0.67"/></p> | | <p>Flow status <input style="width: 50px;" type="text" value="Subcritical flow"/></p> |
| <p>Critical depth <input style="width: 50px;" type="text" value="0.84"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Critical slope <input style="width: 50px;" type="text" value="0.0116"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/ft</div> | <p>Velocity head <input style="width: 50px;" type="text" value="0.14"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | |

Copyright 2000 Dr. Xing Fang, Department of Civil Engineering, Lamar University.

DP8 SWALE
 Q₁₀₀=15.7 cfs

| The open channel flow calculator | | | |
|--|---|---|--|
| <p style="color: red;">Select Channel Type:</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">Trapezoid ▾</div> | <div style="display: flex; justify-content: space-around; font-weight: bold; font-size: small;"> Rectangle Trapezoid Triangle Circle </div> | | |
| <div style="border: 1px solid black; padding: 2px; display: inline-block;">Velocity(V)&Discharge(Q) ▾</div> | <p style="color: red;">Select unit system:</p> <div style="border: 1px solid black; padding: 2px; display: inline-block;">Feet(ft) ▾</div> | | |
| <p>Channel slope: <input style="width: 50px;" type="text" value="0.25"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/ft</div> | <p>Water depth(y): <input style="width: 50px;" type="text" value="0.41"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Bottom width(b) <input style="width: 50px;" type="text" value="3"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | |
| <p>Flow velocity 10.0147</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/s</div> | <p>LeftSlope (Z1): <input style="width: 50px;" type="text" value="2"/> to 1 (H:V)</p> | | <p>RightSlope (Z2): <input style="width: 50px;" type="text" value="2"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">to 1 (H:V)</div> |
| <p>Flow discharge 15.6851</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft³/s</div> | <p>Input n value <input style="width: 50px;" type="text" value="0.035"/> or select n</p> | | |
| <div style="border: 1px solid black; padding: 2px; width: 100px;">Calculate!</div> | <p>Status: Calculation finished</p> | <div style="border: 1px solid black; padding: 2px; width: 100px;">Reset</div> | |
| <p>Wetted perimeter <input style="width: 50px;" type="text" value="4.83"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Flow area <input style="width: 50px;" type="text" value="1.57"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft²</div> | | <p>Top width(T) <input style="width: 50px;" type="text" value="4.64"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> |
| <p>Specific energy <input style="width: 50px;" type="text" value="1.97"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Froude number <input style="width: 50px;" type="text" value="3.04"/></p> | | <p>Flow status Supercritical flow</p> |
| <p>Critical depth <input style="width: 50px;" type="text" value="0.79"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> | <p>Critical slope <input style="width: 50px;" type="text" value="0.023"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft/ft</div> | | <p>Velocity head <input style="width: 50px;" type="text" value="1.56"/></p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;">ft</div> |

Copyright 2000 Dr. Xing Fang, Department of Civil Engineering, Lamar University.

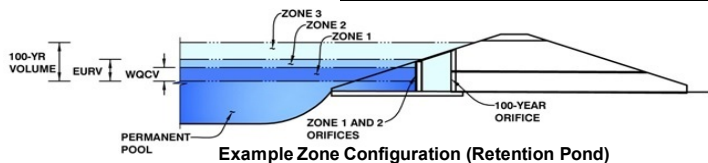
DP8 RUNDOWN POND 2

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.02 (February 2020)

Project: Claremont Business Park 2 Filing No.1

Basin ID: WOCV POND 2



Example Zone Configuration (Retention Pond)

Watershed Information

| | | |
|--|-----------|---------|
| Selected BMP Type = | SF | |
| Watershed Area = | 8.57 | acres |
| Watershed Length = | 665 | ft |
| Watershed Length to Centroid = | 325 | ft |
| Watershed Slope = | 0.018 | ft/ft |
| Watershed Imperviousness = | 63.50% | percent |
| Percentage Hydrologic Soil Group A = | 0.0% | percent |
| Percentage Hydrologic Soil Group B = | 100.0% | percent |
| Percentage Hydrologic Soil Groups C/D = | 0.0% | percent |
| Target WQCV Drain Time = | 12.0 | hours |
| Location for 1-hr Rainfall Depths = User Input | | |

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

| | | |
|--|-------|-----------|
| Water Quality Capture Volume (WQCV) = | 0.142 | acre-feet |
| Excess Urban Runoff Volume (EURV) = | 0.593 | acre-feet |
| 2-yr Runoff Volume (P1 = 1.19 in.) = | 0.521 | acre-feet |
| 5-yr Runoff Volume (P1 = 1.5 in.) = | 0.713 | acre-feet |
| 10-yr Runoff Volume (P1 = 1.75 in.) = | 0.877 | acre-feet |
| 25-yr Runoff Volume (P1 = 2 in.) = | 1.080 | acre-feet |
| 50-yr Runoff Volume (P1 = 2.25 in.) = | 1.254 | acre-feet |
| 100-yr Runoff Volume (P1 = 2.52 in.) = | 1.466 | acre-feet |
| 500-yr Runoff Volume (P1 = 2.53 in.) = | 1.473 | acre-feet |
| Approximate 2-yr Detention Volume = | 0.459 | acre-feet |
| Approximate 5-yr Detention Volume = | 0.616 | acre-feet |
| Approximate 10-yr Detention Volume = | 0.788 | acre-feet |
| Approximate 25-yr Detention Volume = | 0.850 | acre-feet |
| Approximate 50-yr Detention Volume = | 0.885 | acre-feet |
| Approximate 100-yr Detention Volume = | 0.960 | acre-feet |

Optional User Overrides

| | |
|-------|-----------|
| 0.142 | acre-feet |
| 0.593 | acre-feet |
| 1.19 | inches |
| 1.50 | inches |
| 1.75 | inches |
| 2.00 | inches |
| 2.25 | inches |
| 2.52 | inches |
| 2.53 | inches |

Define Zones and Basin Geometry

| | | |
|---|-------|-----------------|
| Zone 1 Volume (WQCV) = | 0.142 | acre-feet |
| Zone 2 Volume (100-year - Zone 1) = | 0.818 | acre-feet |
| Select Zone 3 Storage Volume (Optional) = | | acre-feet |
| Total Detention Basin Volume = | 0.960 | acre-feet |
| Initial Surcharge Volume (ISV) = | N/A | ft ³ |
| Initial Surcharge Depth (ISD) = | N/A | ft |
| Total Available Detention Depth (H_{total}) = | user | ft |
| Depth of Trickle Channel (H_{TC}) = | N/A | ft |
| Slope of Trickle Channel (S_{TC}) = | N/A | ft/ft |
| Slopes of Main Basin Sides (S_{main}) = | user | H:V |
| Basin Length-to-Width Ratio (L/W) = | user | |

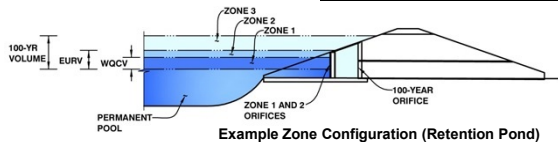
[illegible]

DETENTION BASIN OUTLET STRUCTURE DESIGN

Version 4.02 (February 2020)

Project: **Claremont Business Park 2 Filing No.1**

Basin ID: **WQCV POND 2**



Example Zone Configuration (Retention Pond)

| | Estimated Stage (ft) | Estimated Volume (ac-ft) | Outlet Type |
|-------------------|----------------------|--------------------------|----------------------|
| Zone 1 (WQCV) | 1.72 | 0.142 | Filtration Media |
| Zone 2 (100-year) | #VALUE! | 0.818 | Weir&Pipe (Restrict) |
| Zone 3 | | | |
| Total (all zones) | | 0.960 | |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = 2.50 ft (distance below the filtration media surface)
Underdrain Orifice Diameter = 1.66 inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = 0.0 ft²
Underdrain Orifice Centroid = 0.07 feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = N/A ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = N/A inches
Orifice Plate: Orifice Area per Row = N/A inches

Calculated Parameters for Plate
WQ Orifice Area per Row = N/A ft²
Elliptical Half-Width = N/A feet
Elliptical Slot Centroid = N/A feet
Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (optional) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|--------------------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|
| Stage of Orifice Centroid (ft) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Orifice Area (sq. inches) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|--------------------------------|------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Stage of Orifice Centroid (ft) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Orifice Area (sq. inches) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = Not Selected ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice = Not Selected ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter = Not Selected inches

Calculated Parameters for Vertical Orifice
Vertical Orifice Area = Not Selected ft²
Vertical Orifice Centroid = Not Selected feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

Overflow Weir Front Edge Height, H_o = 1.75 ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length = 7.00 feet
Overflow Weir Grate Slope = 0.00 H:V
Horiz. Length of Weir Sides = 2.91 feet
Overflow Grate Open Area % = 70% %, grate open area/total area
Debris Clogging % = 50% %

Calculated Parameters for Overflow Weir
Height of Grate Upper Edge, H_u = 1.75 feet
Overflow Weir Slope Length = 2.91 feet
Grate Open Area / 100-yr Orifice Area = 6.47
Overflow Grate Open Area w/o Debris = 14.26 ft²
Overflow Grate Open Area w/ Debris = 7.13 ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = 2.75 ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter = 30.00 inches
Restrictor Plate Height Above Pipe Invert = 13.80 inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate
Outlet Orifice Area = 2.20 ft²
Outlet Orifice Centroid = 0.67 feet
Half-Central Angle of Restrictor Plate on Pipe = 1.49 radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 3.00 ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = 12.50 feet
Spillway End Slopes = 4.00 H:V
Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway
Spillway Design Flow Depth = 0.75 feet
Stage at Top of Freeboard = 4.75 feet
Basin Area at Top of Freeboard = 0.18 acres
Basin Volume at Top of Freeboard = 0.40 acre-ft

Routed Hydrograph Results

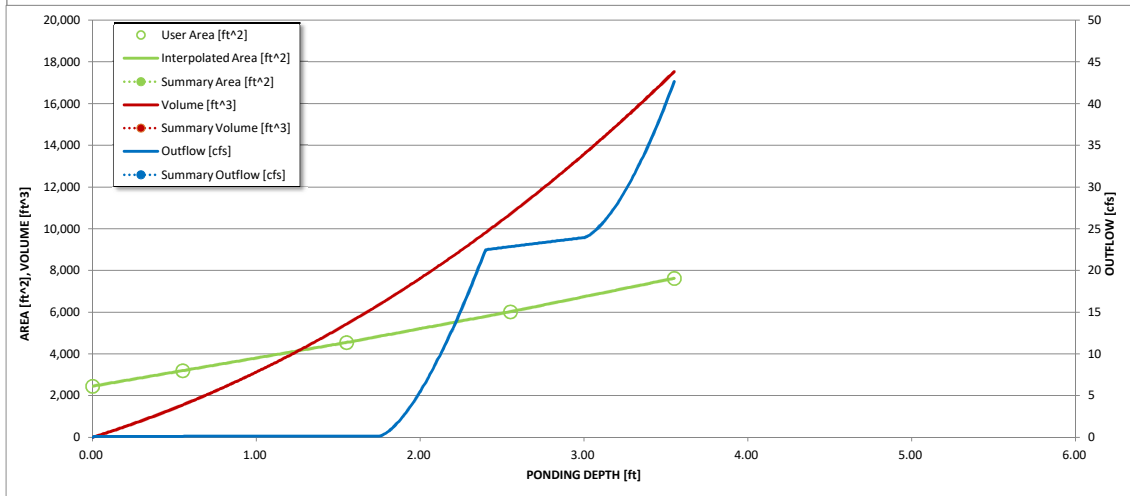
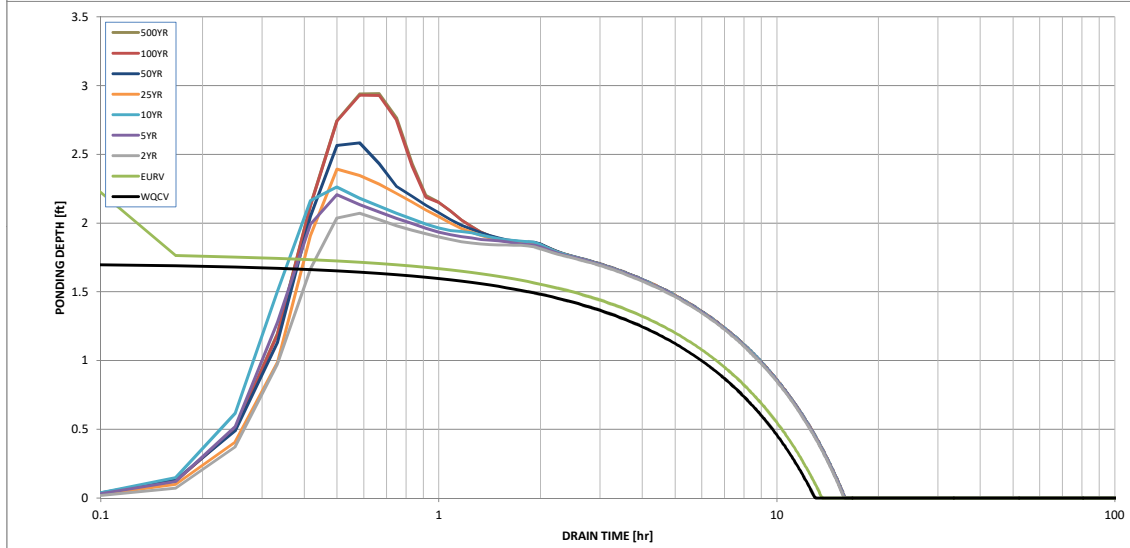
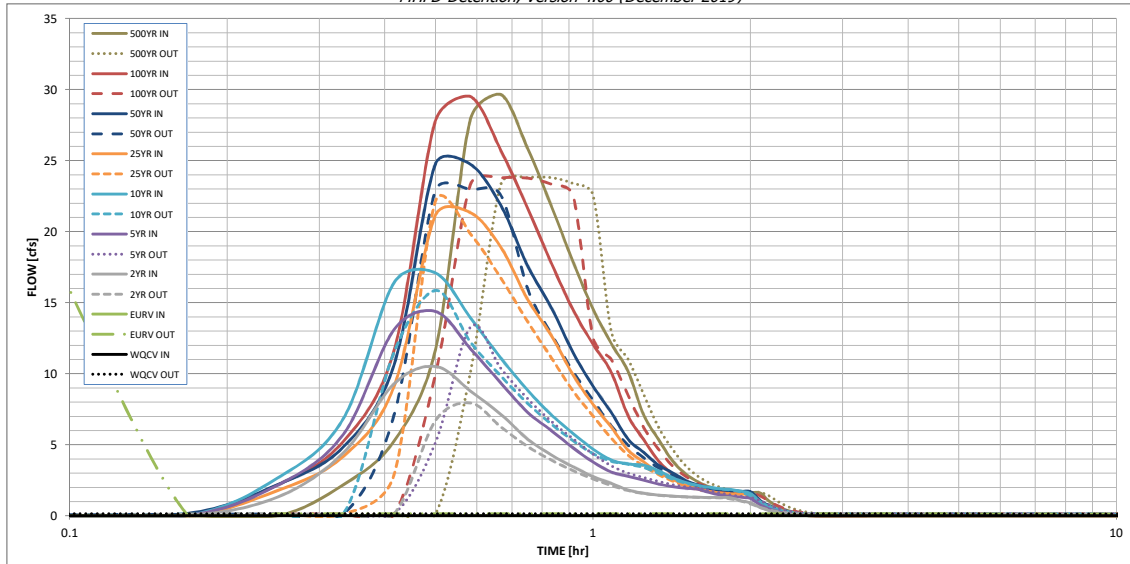
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

| | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year | 500 Year |
|---|------------------|----------------|-----------------|-----------------|-----------------|----------------|----------------|----------------|----------------|
| Design Storm Return Period | N/A | N/A | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 | 2.53 |
| One-Hour Rainfall Depth (in) | N/A | N/A | 0.521 | 0.713 | 0.877 | 1.080 | 1.254 | 1.466 | 1.473 |
| CUHP Runoff Volume (acre-ft) | N/A | N/A | 0.521 | 0.713 | 0.877 | 1.080 | 1.254 | 1.466 | 1.473 |
| Inflow Hydrograph Volume (acre-ft) | N/A | N/A | 1.2 | 3.2 | 4.8 | 8.5 | 10.7 | 13.3 | 13.4 |
| CUHP Predevelopment Peak Q (cfs) | N/A | N/A | 0.14 | 0.38 | 0.56 | 0.99 | 1.24 | 1.56 | 1.57 |
| OPTIONAL Override Predevelopment Peak Q (cfs) | N/A | N/A | 0.14 | 0.38 | 0.56 | 0.99 | 1.24 | 1.56 | 1.57 |
| Predevelopment Unit Peak Flow, q (cfs/acre) | N/A | N/A | 10.5 | 14.4 | 17.1 | 21.3 | 24.8 | 29.5 | 29.6 |
| Peak Inflow Q (cfs) | N/A | N/A | 8.0 | 13.3 | 15.9 | 22.1 | 22.9 | 23.8 | 23.8 |
| Peak Outflow Q (cfs) | N/A | N/A | 4.1 | 3.3 | 2.6 | 2.2 | 1.8 | 1.8 | 1.8 |
| Ratio Peak Outflow to Predevelopment Q | N/A | N/A | 4.1 | 3.3 | 2.6 | 2.2 | 1.8 | 1.8 | 1.8 |
| Structure Controlling Flow | Filtration Media | Outlet Plate 1 | Overflow Weir 1 | Overflow Weir 1 | Overflow Weir 1 | Outlet Plate 1 | Outlet Plate 1 | Outlet Plate 1 | Outlet Plate 1 |
| Max Velocity through Grate 1 (fps) | N/A | 1.53 | 0.54 | 0.9 | 1.1 | 1.5 | 1.6 | 1.7 | 1.7 |
| Max Velocity through Grate 2 (fps) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Time to Drain 97% of Inflow Volume (hours) | 13 | 12 | 14 | 14 | 13 | 13 | 12 | 12 | 12 |
| Time to Drain 99% of Inflow Volume (hours) | 13 | 13 | 15 | 15 | 15 | 15 | 14 | 14 | 14 |
| Maximum Ponding Depth (ft) | 1.72 | 2.39 | 2.07 | 2.21 | 2.26 | 2.39 | 2.58 | 2.93 | 2.94 |
| Area at Maximum Ponding Depth (acres) | 0.11 | 0.13 | 0.12 | 0.13 | 0.13 | 0.13 | 0.14 | 0.15 | 0.15 |
| Maximum Volume Stored (acre-ft) | 0.143 | 0.223 | 0.183 | 0.199 | 0.207 | 0.224 | 0.250 | 0.299 | 0.302 |

Per resolution 16-426 of the BoCC, on-site WQCV is required but on-site stormwater detention is not required per the FDR for Claremont Business Park Filing No. 2.

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.00 (December 2019)



| S-A-V-D Chart Axis Override | X-axis | Left Y-Axis | Right Y-Axis |
|-----------------------------|--------|-------------|--------------|
| minimum bound | | | |
| maximum bound | | | |



20 BOULDER CRESCENT, STE 110
COLORADO SPRINGS, CO 80903
(719) 955-5485

PROJECT: CLAREMONT BUSINESS PARK 2 FIL.1

DATE: 4-17-2020

CONTROL ORIFICE FOR WQEFV

EQU. SF-3 UDFCD V2.3 SAND FILTER

$$D_{12 \text{ HOUR DRAIN TIME}} = \sqrt{\frac{V}{1414 y^{0.41}}}$$

$$y = 2.5' - \left(\frac{4' - 2'}{12' \text{ in } / \text{ ft}} \right) = \underline{2.33'} \quad V = 0.143 \text{ AC-H} \times \frac{43560 \text{ ft}^2}{1 \text{ AC-H}} = \underline{6229 \text{ ft}^3}$$

FROM MHFD-DETENTION SLOOT

$$D = \sqrt{\frac{6229 \text{ ft}^3}{1414 (2.33^{0.41})}} = 1.76 \text{ in DIA} \quad K^2 = A = K (0.88)^2 = 244 \text{ in}^2$$
$$2.44 \text{ m}^2 \approx 0.017 \text{ ft}^2$$

[Fluid Flow Table of Contents](#)
[Hydraulic and Pneumatic Knowledge](#)
[Fluid Power Equipment](#)

This engineering calculator determines the Flow within a partially full pipe (&e1/2 full) using the Manning equation. This calculator can also be used for uniform flow in a pipe, but the Manning roughness coefficient needs to be considered to be variable, dependent upon the depth of flow.

Partially Full Pipe Flow Calculations - U.S. Units

II. Calculation of Discharge, Q, and average velocity, V
for pipes more than half full

Instructions: Enter values in blue boxes. Calculations in yellow

Inputs

Pipe Diameter, **D** = in
Depth of flow, **y** = in
(must have $y \geq D/2$)

Full Pipe Manning
roughness, **n_{full}** =
Channel bottom
slope, **S** = ft/ft

Calculations
n/n_{full} =
Partially Full Manning
roughness, **n** =

Calculations

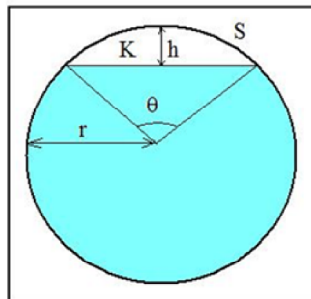
Pipe Diameter, **D** = ft
Pipe Radius, **r** = ft

Circ. Segment Height, **h** = ft

Central Angle, **q** = radians
Cross-Sept. Area, **A** = ft²

Wetted Perimeter, **P** = ft
Hydraulic Radius, **R** = ft
Discharge, **Q** = cfs
Ave. Velocity, **V** = ft/sec

pipe % full $[(A/A_{full}) * 100\%]$ =



Partially Full Pipe Flow Parameters
(More Than Half Full)

$$r = D/2$$

$$h = 2r - y$$

(hydraulic radius)

$$R = A/P$$

(Manning Equation)

$$Q = (1.49/n)(A)(R^{2/3})(S^{1/2})$$

$$V = Q/A$$

$$\theta = 2 \arccos \left(\frac{r-h}{r} \right)$$

$$A = \pi r^2 - \frac{r^2(\theta - \sin \theta)}{2}$$

$$P = 2\pi r - r * \theta$$

Equation used for n/n_{full} : $n/n_{full} = 1.25 - (y/D - 0.5) * 0.5$ (for $0.5 \leq y/D \leq 1$)

DEPTH OF FLOW FOR 12 HR DRAIN TIME 4" PVC

[Membership](#)
[Register](#) | [Login](#)

Search

Main Categories

- » [Home](#)
- » [Engineering Book Store](#)
- » [Engineering Forum](#)
- » [Excel App. Downloads](#)
- » [Online Books & Manuals](#)
- » [Engineering News](#)
- » [Engineering Videos](#)
- » [Engineering Calculators](#)
- » [Engineering Toolbox](#)
- » [GD&T Training](#)
- » [Geometric Dimensioning](#)
- » [Tolerancing](#)
- » [DFM DFA Training](#)
- » [Training Online](#)
- » [Engineering](#)
- » [Advertising Center](#)

Follow @engineersedge

Print Webpage

» [Copyright Notice](#)

**Submit an
Article**



Become an
Engineers Edge
Contributor

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

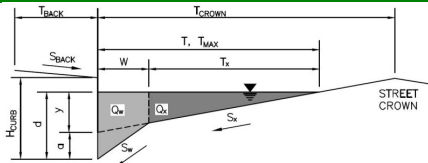
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

CLAREMONT BUSINESS PARK 2 FILING NO. 1

Inlet ID:

Inlet 1 DP 5 NORTH

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

 $T_{BACK} = 7.5$ ft

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

 $S_{BACK} = 0.020$ ft/ft

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line

 $H_{CURB} = 6.00$ inches

Distance from Curb Face to Street Crown

 $T_{CROWN} = 17.0$ ft

Gutter Width

 $W = 2.00$ ft

Street Transverse Slope

 $S_X = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

 $S_W = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

 $S_O = 0.012$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $n_{STREET} = 0.015$

Max. Allowable Spread for Minor & Major Storm

| | Minor Storm | Major Storm | |
|-------------|-------------|-------------|--------|
| $T_{MAX} =$ | 15.8 | 17.0 | ft |
| $d_{MAX} =$ | 4.6 | 7.8 | inches |

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Allow Flow Depth at Street Crown (leave blank for no)

☐ ☐ check = yes
MINOR STORM Allowable Capacity is based on Depth Criterion**MAJOR STORM Allowable Capacity is based on Spread Criterion**

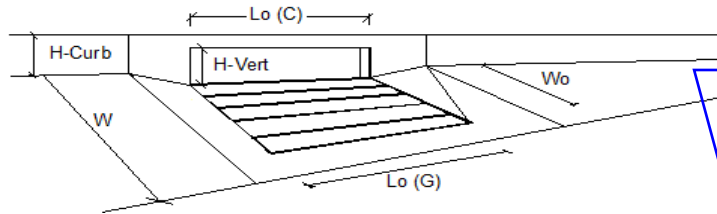
| | Minor Storm | Major Storm | |
|---------------|-------------|-------------|-----|
| $Q_{allow} =$ | 6.5 | 12.7 | cfs |

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Review #1: Revise so that it matches the CD's and drainage plan. Inlet 1 is indicated as 15' on the CD's.

Review 2: unresolved.

| Design Information (Input) | | MINOR | | MAJOR | |
|---|--------------------------|----------------------|--------------------------|-------|--------|
| Type of Inlet | CDOT Type R Curb Opening | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a') | | a _{LOCAL} = | 3.0 | 3.0 | inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | | No = | 3 | 3 | |
| Length of a Single Unit Inlet (Grate or Curb Opening) | | L _o = | 5.00 | 5.00 | ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | | W _o = | N/A | N/A | ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | | C _{r-G} = | N/A | N/A | |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | | C _{r-C} = | 0.10 | 0.10 | |
| Street Hydraulics: OK - Q < Allowable Street Capacity | | MINOR | | MAJOR | |
| Total Inlet Interception Capacity | | Q = | 6.4 | 9.1 | cfs |
| Total Inlet Carry-Over Flow (flow bypassing inlet) | | Q _b = | 0.0 | 0.8 | cfs |
| Capture Percentage = Q _i /Q _c = | | C% = | 100 | 92 | % |

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

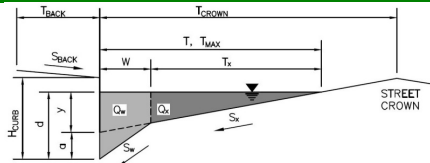
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

CLAREMONT BUSINESS PARK 2 FILING NO. 1

Inlet ID:

Inlet 2 DP 5 SOUTH

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

 $T_{BACK} = 7.5$ ft

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

 $S_{BACK} = 0.020$ ft/ft

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line

 $H_{CURB} = 6.00$ inches

Distance from Curb Face to Street Crown

 $T_{CROWN} = 17.0$ ft

Gutter Width

 $W = 2.00$ ft

Street Transverse Slope

 $S_X = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

 $S_W = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

 $S_O = 0.011$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm

| | Minor Storm | Major Storm | |
|-------------|-------------|-------------|--------|
| $T_{MAX} =$ | 15.8 | 17.0 | ft |
| $d_{MAX} =$ | 4.6 | 7.8 | inches |

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Allow Flow Depth at Street Crown (leave blank for no)

☐ ☐ check = yes
MINOR STORM Allowable Capacity is based on Depth Criterion

| | Minor Storm | Major Storm | |
|---------------|-------------|-------------|-----|
| $Q_{allow} =$ | 5.8 | 11.3 | cfs |

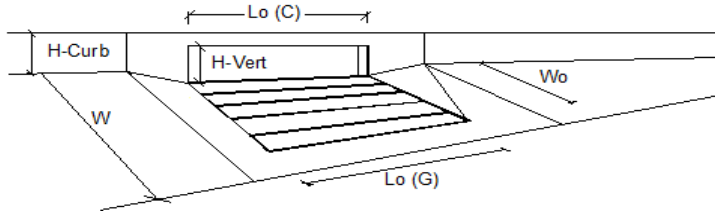
MAJOR STORM Allowable Capacity is based on Spread Criterion

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Review 1 comment: Revise so that it matches the CD's and drainage plan. Inlet 2 is indicated as 10' on the CD's.
Review 2: Unresolved. Revise to match the CD's and drainage plan which now indicate a 15' inlet.

| Design Information (Input) | | MINOR | | MAJOR | | |
|---|--------------------------|----------------------|--------------------------|-------|--------|--|
| Type of Inlet | CDOT Type R Curb Opening | Type = | CDOT Type R Curb Opening | | | |
| Local Depression (additional to continuous gutter depression 'a') | | a _{LOCAL} = | 3.0 | 3.0 | inches | |
| Total Number of Units in the Inlet (Grate or Curb Opening) | | No = | 3 | 3 | | |
| Length of a Single Unit Inlet (Grate or Curb Opening) | | L _o = | 5.00 | 5.00 | ft | |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | | W _o = | N/A | N/A | ft | |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | | C _r -G = | N/A | N/A | | |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | | C _r -C = | 0.10 | 0.10 | | |
| Street Hydraulics: OK - Q < Allowable Street Capacity | | MINOR | | MAJOR | | |
| Total Inlet Interception Capacity | | Q = | 3.7 | 9.2 | cfs | |
| Total Inlet Carry-Over Flow (flow bypassing inlet) | | Q _b = | 0.0 | 0.7 | cfs | |
| Capture Percentage = Q _i /Q _c = | | C% = | 100 | 93 | % | |

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

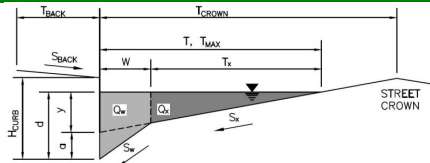
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

CLAREMONT BUSINESS PARK 2 FILING NO. 1

Inlet ID:

Inlet 3 DP 6 NORTH

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

 $T_{BACK} = 7.5$ ft

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

 $S_{BACK} = 0.020$ ft/ft

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line

 $H_{CURB} = 6.00$ inches

Distance from Curb Face to Street Crown

 $T_{CROWN} = 17.0$ ft

Gutter Width

 $W = 2.00$ ft

Street Transverse Slope

 $S_X = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

 $S_W = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

 $S_O = 0.010$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $n_{STREET} = 0.015$

Max. Allowable Spread for Minor & Major Storm

| | Minor Storm | Major Storm | |
|-------------|-------------|-------------|--------|
| $T_{MAX} =$ | 15.8 | 17.0 | ft |
| $d_{MAX} =$ | 4.6 | 7.8 | inches |

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Allow Flow Depth at Street Crown (leave blank for no)

☐☐

check = yes

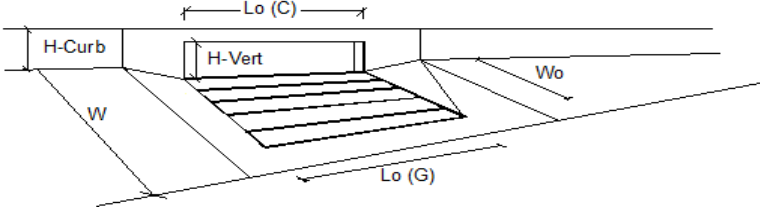
MINOR STORM Allowable Capacity is based on Depth Criterion**MAJOR STORM Allowable Capacity is based on Spread Criterion**

| | Minor Storm | Major Storm | |
|---------------|-------------|-------------|-----|
| $Q_{allow} =$ | 5.9 | 11.6 | cfs |

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Review 1: Revise so that it matches the CD's and drainage plan. Inlet 3 is indicated as 15' on the CD's.
Review 2: Unresolved.

| INLET ON A CONTINUOUS GRADE | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---|---|-------|--------|--|-------|-------|--|-----------------------------------|--------------------------|-----|-----|--|-----|-----|--------|----------------------------------|-----|-----|---|---------|------|------|----|---------|-----|-----|----|-----------|-----|-----|--|-----------|------|------|--|
| Version 4.05 Released March 2017 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|  | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Design Information (Input) Type of Inlet: CDOT Type R Curb Opening Local Depression (additional to continuous gutter depression 'a') Total Number of Units in the Inlet (Grate or Curb Opening) Length of a Single Unit Inlet (Grate or Curb Opening) Width of a Unit Grate (cannot be greater than W, Gutter Width) Clogging Factor for a Single Unit Grate (typical min. value = 0.5) Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | <table border="1" style="width: 100%; border-collapse: collapse; text-align: center;"> <thead> <tr> <th></th> <th style="width: 30%;">MINOR</th> <th style="width: 30%;">MAJOR</th> <th></th> </tr> </thead> <tbody> <tr> <td>Type =</td> <td colspan="2" style="text-align: left;">CDOT Type R Curb Opening</td> <td></td> </tr> <tr> <td>a_{LOCAL} =</td> <td>3.0</td> <td>3.0</td> <td>inches</td> </tr> <tr> <td>N_o =</td> <td>3</td> <td>3</td> <td></td> </tr> <tr> <td>L_o =</td> <td>5.00</td> <td>5.00</td> <td>ft</td> </tr> <tr> <td>W_o =</td> <td>N/A</td> <td>N/A</td> <td>ft</td> </tr> <tr> <td>C_r-G =</td> <td>N/A</td> <td>N/A</td> <td></td> </tr> <tr> <td>C_r-C =</td> <td>0.10</td> <td>0.10</td> <td></td> </tr> </tbody> </table> | | | | MINOR | MAJOR | | Type = | CDOT Type R Curb Opening | | | a_{LOCAL} = | 3.0 | 3.0 | inches | N_o = | 3 | 3 | | L_o = | 5.00 | 5.00 | ft | W_o = | N/A | N/A | ft | C_r-G = | N/A | N/A | | C_r-C = | 0.10 | 0.10 | |
| | MINOR | MAJOR | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Type = | CDOT Type R Curb Opening | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| a_{LOCAL} = | 3.0 | 3.0 | inches | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| N_o = | 3 | 3 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| L_o = | 5.00 | 5.00 | ft | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| W_o = | N/A | N/A | ft | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C_r-G = | N/A | N/A | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C_r-C = | 0.10 | 0.10 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Street Hydraulics: OK - Q < Allowable Street Capacity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| <table border="1" style="width: 100%; border-collapse: collapse; text-align: center;"> <thead> <tr> <th></th> <th style="width: 30%;">MINOR</th> <th style="width: 30%;">MAJOR</th> <th></th> </tr> </thead> <tbody> <tr> <td>Total Inlet Interception Capacity</td> <td>3.0</td> <td>5.7</td> <td>cfs</td> </tr> <tr> <td>Total Inlet Carry-Over Flow (flow bypassing inlet)</td> <td>0.0</td> <td>0.0</td> <td>cfs</td> </tr> <tr> <td>Capture Percentage = Q_i/Q_o =</td> <td>100</td> <td>100</td> <td>%</td> </tr> </tbody> </table> | | | | | MINOR | MAJOR | | Total Inlet Interception Capacity | 3.0 | 5.7 | cfs | Total Inlet Carry-Over Flow (flow bypassing inlet) | 0.0 | 0.0 | cfs | Capture Percentage = Q_i/Q_o = | 100 | 100 | % | | | | | | | | | | | | | | | | |
| | MINOR | MAJOR | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Inlet Interception Capacity | 3.0 | 5.7 | cfs | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Inlet Carry-Over Flow (flow bypassing inlet) | 0.0 | 0.0 | cfs | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Capture Percentage = Q_i/Q_o = | 100 | 100 | % | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

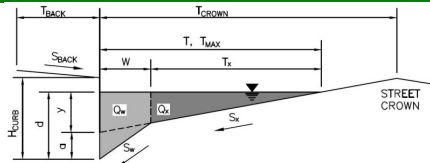
(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

CLAREMONT BUSINESS PARK 2 FILING NO. 1

Inlet ID:

Inlet 4 DP 6 SOUTH

**Gutter Geometry (Enter data in the blue cells)**

Maximum Allowable Width for Spread Behind Curb

 $T_{BACK} = 7.5$ ft

Side Slope Behind Curb (leave blank for no conveyance credit behind curb)

 $S_{BACK} = 0.020$ ft/ft

Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line

 $H_{CURB} = 6.00$ inches

Distance from Curb Face to Street Crown

 $T_{CROWN} = 17.0$ ft

Gutter Width

 $W = 2.00$ ft

Street Transverse Slope

 $S_x = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

 $S_w = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

 $S_o = 0.010$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

 $n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm

| | Minor Storm | Major Storm | |
|-------------|-------------|-------------|--------|
| $T_{MAX} =$ | 15.8 | 17.0 | ft |
| $d_{MAX} =$ | 4.6 | 7.8 | inches |

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Allow Flow Depth at Street Crown (leave blank for no)

☐ ☐ check = yes

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

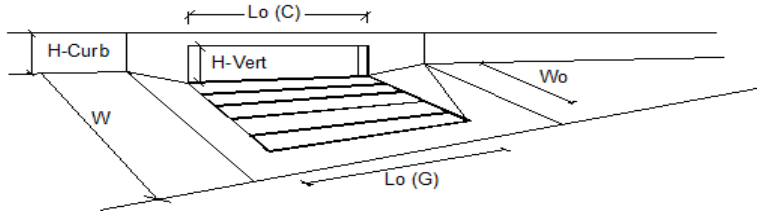
| | Minor Storm | Major Storm | |
|---------------|-------------|-------------|-----|
| $Q_{allow} =$ | 5.5 | 10.9 | cfs |

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



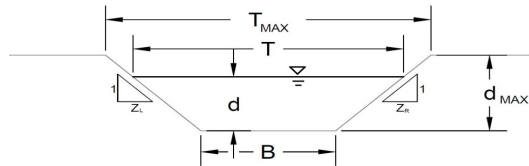
Review 1: Revise so that it matches the CD's and drainage plan. Inlet 4 is indicated as 10' on the CD's.
Review 2: Unresolved. Please revise to match the CD's and drainage plan which now indicate a 15' inlet.

| Design Information (Input) | MINOR | MAJOR | |
|---|--------------------------|-------|--------|
| Type of Inlet | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a') | 3.0 | 3.0 | inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | 3 | 3 | |
| Length of a Single Unit Inlet (Grate or Curb Opening) | 5.00 | 5.00 | ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | N/A | N/A | ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | N/A | N/A | |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | 0.10 | 0.10 | |
| Street Hydraulics: OK - Q < Allowable Street Capacity | | | |
| Total Inlet Interception Capacity | 1.5 | 5.6 | cfs |
| Total Inlet Carry-Over Flow (flow bypassing inlet) | 0.0 | 0.0 | cfs |
| Capture Percentage = Q_i/Q_o = | 100 | 100 | % |

AREA INLET IN A SWALE

CLAREMONT BUSINESS PARK 2 FILING NO. 1

INLET 6 DP 7



This worksheet uses the NRCS
vegetal retardance method to
determine Manning's n.

For more information see
Section 7.2.3 of the USDCM.

Analysis of Trapezoidal Grass-Lined Channel Using SCS Method

NRCS Vegetal Retardance (A, B, C, D, or E)

Manning's n (Leave cell D16 blank to manually enter an n value)

Channel Invert Slope

Bottom Width

Left Side Slope

Right Side Slope

Check one of the following soil types:

| Soil Type: | Max. Velocity (V_{MAX}) | Max Froude No. (F_{MAX}) |
|--------------|-----------------------------|------------------------------|
| Non-Cohesive | 5.0 fps | 0.60 |
| Cohesive | 7.0 fps | 0.80 |
| Paved | N/A | N/A |

Max. Allowable Top Width of Channel for Minor & Major Storm

Max. Allowable Water Depth in Channel for Minor & Major Storm

A, B, C, D or E

n = 0.025

 S_0 = 0.1200 ft/ft

B = 0.00 ft

Z1 = 3.00 ft/ft

Z2 = 3.00 ft/ft

Choose One:

☒ Non-Cohesive☐ Cohesive☐ Paved

| | Minor Storm | Major Storm | |
|-------------|-------------|-------------|------|
| T_{MAX} = | 1.92 | 2.40 | feet |
| d_{MAX} = | 0.32 | 0.40 | feet |

Allowable Channel Capacity Based On Channel Geometry

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Top Width Criterion

| | Minor Storm | Major Storm | |
|---------------|-------------|-------------|-----|
| Q_{allow} = | 1.8 | 3.3 | cfs |
| d_{allow} = | 0.32 | 0.40 | ft |

Water Depth in Channel Based On Design Peak Flow

Design Peak Flow

Water Depth

| | | | |
|---------|------|------|------|
| Q_c = | 1.8 | 3.2 | cfs |
| d = | 0.32 | 0.40 | feet |

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

AREA INLET IN A SWALE

CLAREMONT BUSINESS PARK 2 FILING NO. 1

INLET 6 DP 7

Inlet Design Information (Input)

Type of Inlet: Inlet Type =

Angle of Inclined Grate (must be <= 30 degrees): degrees

Width of Grate: feet

Length of Grate: feet

Open Area Ratio:

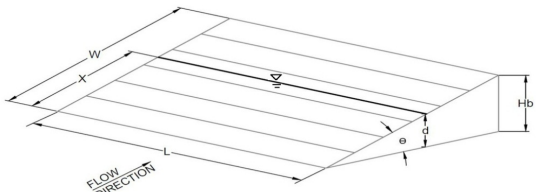
Height of Inclined Grate: feet

Clogging Factor:

Grate Discharge Coefficient:

Orifice Coefficient:

Weir Coefficient:



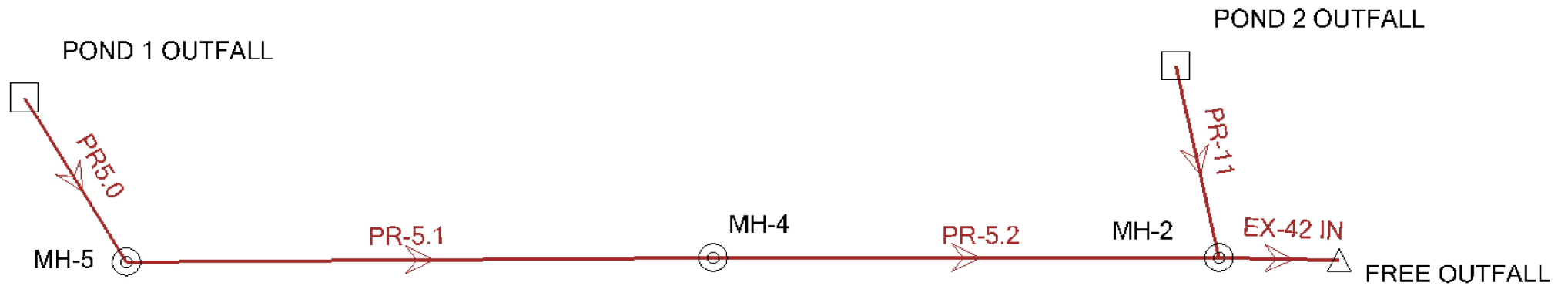
Water Depth at Inlet (for depressed inlets, 1 foot is added for depression): feet

Total Inlet Interception Capacity (assumes clogged condition)

| | MINOR | MAJOR | |
|--------------------------------------|-------|-------|-----|
| d = | 1.32 | 1.40 | |
| $Q_a =$ | 16.3 | 16.8 | cfs |
| Bypassed Flow, $Q_b =$ | 0.0 | 0.0 | cfs |
| Capture Percentage = $Q_a/Q_o = C\%$ | 100 | 100 | % |

Warning 04: Froude No. exceeds USDCM Volume I recommendation.

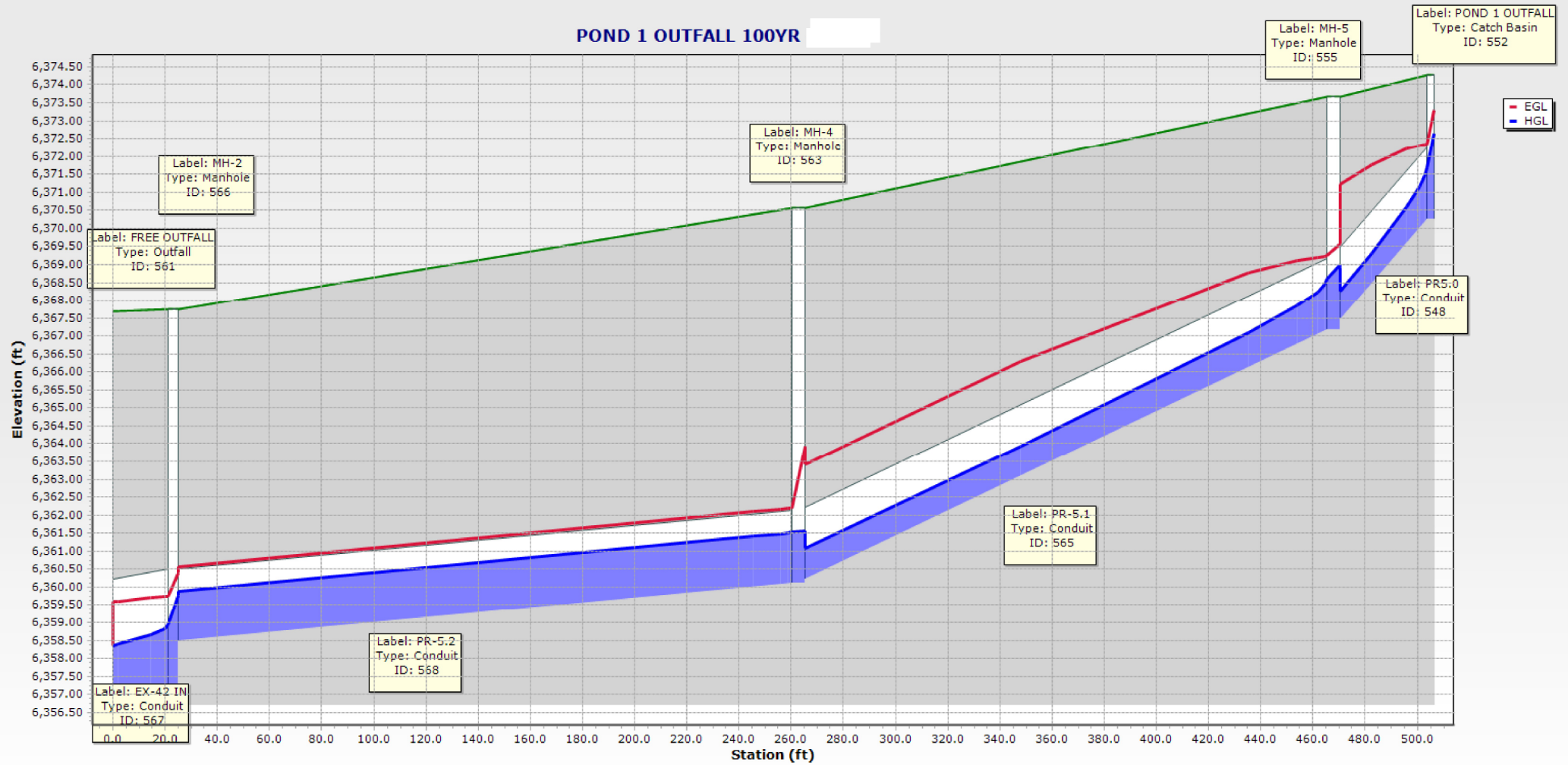
POND 1 & 2 INDEX MAP

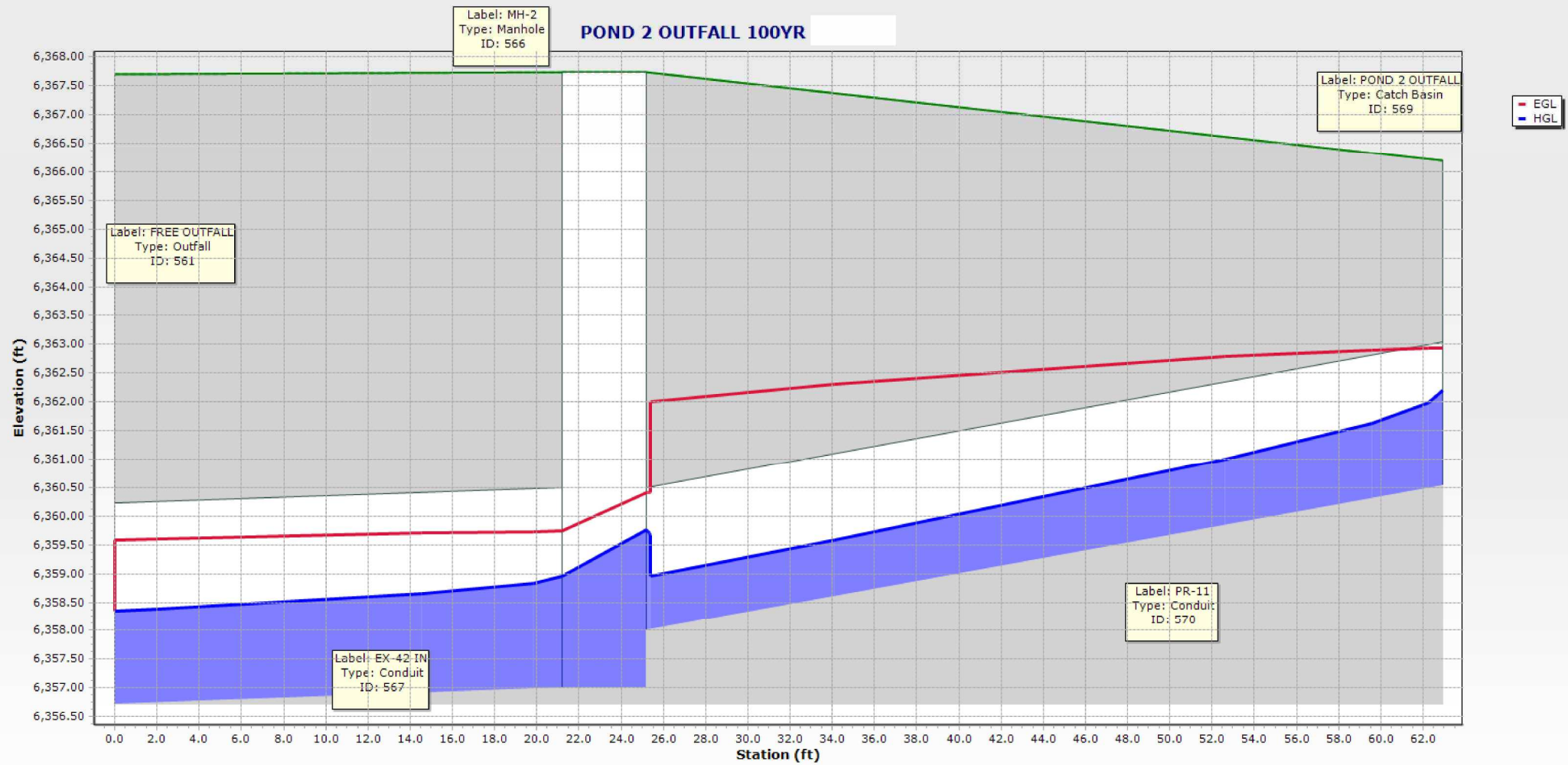


Conduit FlexTable: POND 1 & 2 OUTFALL

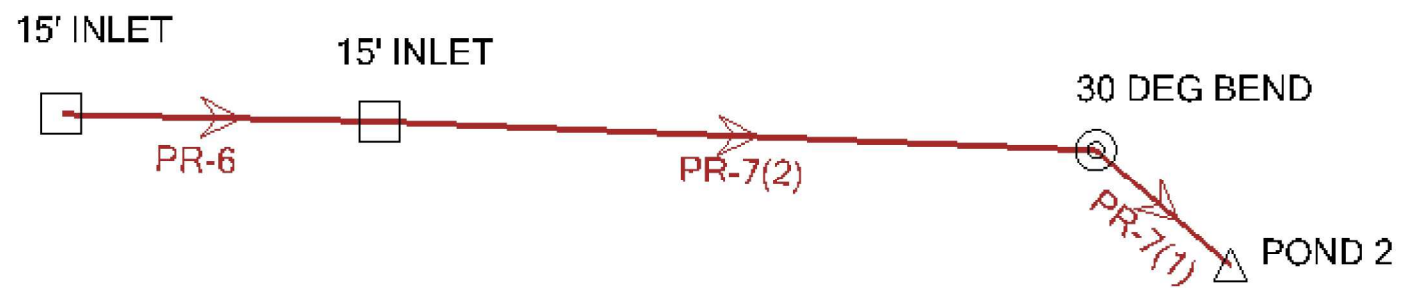
| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) |
|---|---|---|----------------------------------|-------------------------------|------------------------------|---------------------|------------------------|---------------------|-----------------------|-----------------------------|------------------------------|--------------------------------|---------------------------------|---------------|
| PR5.0 | 548 | POND 1 OUTFALL | 15.50 | 25.0 | 37.0 | 16.40 | 4.096 | 0.68 | 1.42 | 6,372.33 | 6,369.56 | 6,371.67 | 6,368.99 | 2.68 |
| PR-5.1 | 565 | MH-5 | 15.50 | 37.2 | 205.1 | 12.28 | 2.709 | 0.85 | 1.42 | 6,369.25 | 6,363.41 | 6,368.59 | 6,361.06 | 7.52 |
| EX-42 IN | 567 | MH-2 | 39.30 | 35.6 | 23.2 | 10.51 | 1.780 | 1.44 | 1.95 | 6,359.74 | 6,359.59 | 6,358.95 | 6,358.35 | 0.59 |
| PR-5.2 | 568 | MH-4 | 15.50 | 83.3 | 239.6 | 6.63 | 1.035 | 1.39 | 1.42 | 6,362.20 | 6,360.58 | 6,361.54 | 6,359.90 | 1.64 |
| PR-11 | 570 | POND 2 OUTFALL | 23.80 | 23.0 | 39.7 | 17.12 | 3.918 | 0.82 | 1.66 | 6,362.92 | 6,360.40 | 6,362.19 | 6,359.76 | 2.43 |
| Upstream Structure Hydraulic Grade Line (In) (ft) | Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description | | | | | | |
| 6,372.65 | 6.50 | 1.500 | 0.98 | 6,373.68 | 6,374.27 | 6,367.47 | 6,370.25 | Circle - 24.0 in | | | | | | |
| 6,368.99 | 6.05 | 1.000 | 0.40 | 6,370.58 | 6,373.68 | 6,360.22 | 6,367.17 | Circle - 24.0 in | | | | | | |
| 6,359.76 | 6.46 | 1.020 | 0.81 | 6,367.70 | 6,367.74 | 6,356.72 | 6,357.00 | Circle - 42.0 in | | | | | | |
| 6,361.57 | 12.28 | 0.050 | 0.03 | 6,367.74 | 6,370.58 | 6,358.50 | 6,360.12 | Circle - 24.0 in | | | | | | |
| 6,363.29 | 6.87 | 1.500 | 1.10 | 6,366.20 | 6,367.74 | 6,360.53 | 6,358.00 | Circle - 30.0 in | | | | | | |

POND 1 OUTFALL 100YR





STORM 6, 7 INDEX MAP

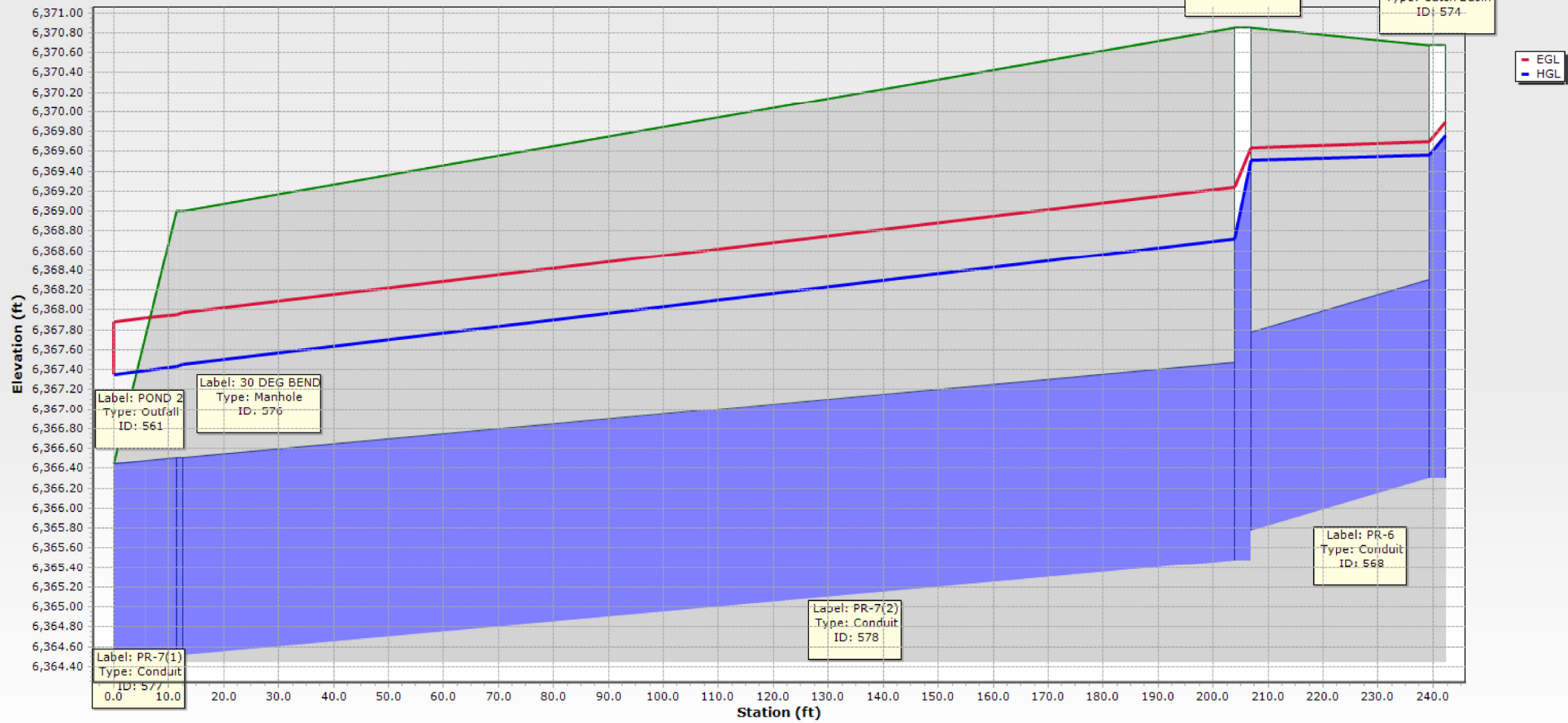


Review 1: The flow is greater than the capacity. Revise accordingly.
Review 2: Unresolved

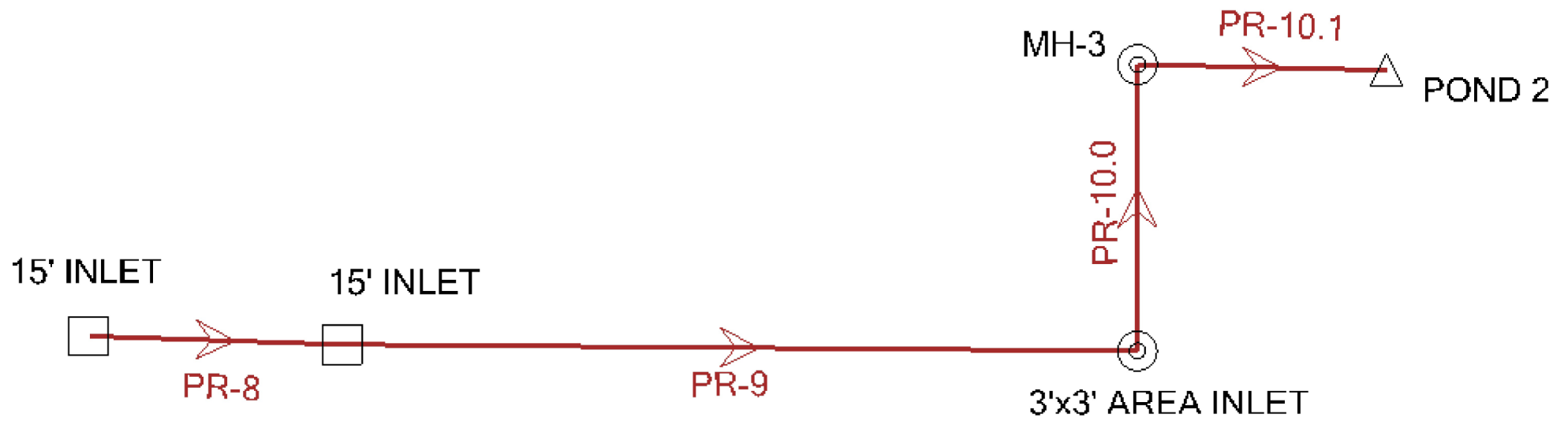
Conduit FlexTable: STRM 7

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) |
|---|---|---|----------------------------------|-------------------------------|------------------------------|---------------------|------------------------|---------------------|-----------------------|-----------------------------|------------------------------|--------------------------------|---------------------------------|---------------|
| PR-6 | 568 | 15' INLET | 9.10 | 32.8 | 35.3 | 2.90 | 1.815 | 0.79 | 1.08 | 6,369.72 | 6,369.67 | 6,369.59 | 6,369.54 | 0.06 |
| PR-7(1) | 577 | 30 DEG BEND | 18.30 | 114.4 | 12.0 | 5.83 | 0.726 | (N/A) | 1.54 | 6,367.99 | 6,367.91 | 6,367.46 | 6,367.38 | 0.08 |
| PR-7(2) | 578 | 15' INLET | 18.30 | 114.8 | 193.5 | 5.83 | 0.726 | (N/A) | 1.54 | 6,369.27 | 6,368.01 | 6,368.75 | 6,367.48 | 1.27 |
| Upstream Structure Hydraulic Grade Line (In) (ft) | Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description | | | | | | |
| 6,369.79 | 2.90 | 1.500 | 0.20 | 6,370.85 | 6,370.67 | 6,365.77 | 6,366.30 | Circle - 24.0 in | | | | | | |
| 6,367.48 | 5.83 | 0.040 | 0.02 | 6,366.45 | 6,369.00 | 6,364.45 | 6,364.51 | Circle - 24.0 in | | | | | | |
| 6,369.54 | 2.90 | 1.500 | 0.79 | 6,369.00 | 6,370.85 | 6,364.51 | 6,365.47 | Circle - 24.0 in | | | | | | |

STRM 7 PROFILE - 100 yr



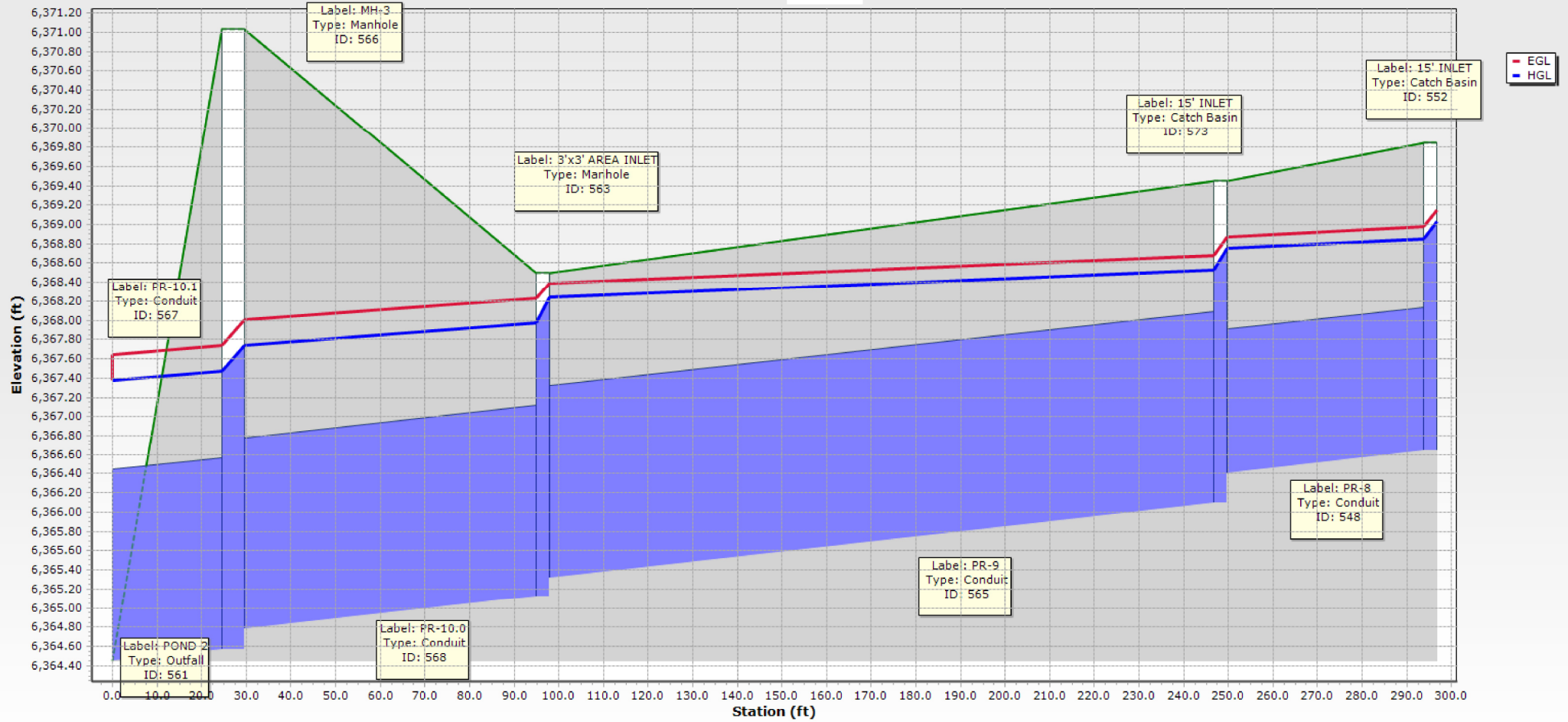
STORM 8, 9, 10 INDEX MAP



Conduit FlexTable: STRM 8,9,10

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) |
|---|---|---|----------------------------------|-------------------------------|------------------------------|---------------------|------------------------|---------------------|-----------------------|-----------------------------|------------------------------|--------------------------------|---------------------------------|---------------|
| PR-8 | 548 | 15' INLET | 4.90 | 66.7 | 47.0 | 2.77 | 0.908 | 0.90 | 0.85 | 6,368.97 | 6,368.87 | 6,368.85 | 6,368.75 | 0.10 |
| PR-9 | 565 | 15' INLET | 9.80 | 60.8 | 151.9 | 3.12 | 0.989 | 1.13 | 1.12 | 6,368.68 | 6,368.39 | 6,368.53 | 6,368.24 | 0.29 |
| PR-10.1 | 567 | MH-3 | 13.00 | 86.4 | 27.1 | 4.14 | 0.821 | 1.43 | 1.30 | 6,367.74 | 6,367.65 | 6,367.47 | 6,367.38 | 0.09 |
| PR-10.0 | 568 | 3'x3' AREA INLET | 13.00 | 82.0 | 69.2 | 4.14 | 0.889 | 1.38 | 1.30 | 6,368.24 | 6,368.01 | 6,367.97 | 6,367.74 | 0.23 |
| Upstream Structure Hydraulic Grade Line (In) (ft) | Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description | | | | | | |
| 6,369.03 | 2.77 | 1.500 | 0.18 | 6,369.45 | 6,369.84 | 6,366.41 | 6,366.64 | Circle - 18.0 in | | | | | | |
| 6,368.75 | 2.77 | 1.500 | 0.23 | 6,368.50 | 6,369.45 | 6,365.32 | 6,366.09 | Circle - 24.0 in | | | | | | |
| 6,367.74 | 4.14 | 1.020 | 0.27 | 6,364.45 | 6,371.03 | 6,364.45 | 6,364.57 | Circle - 24.0 in | | | | | | |
| 6,368.24 | 3.12 | 1.020 | 0.27 | 6,371.03 | 6,368.50 | 6,364.78 | 6,365.12 | Circle - 24.0 in | | | | | | |

STRM 8,9,10 100 YR



BOCC RESOLUTION 16-426

**RESOLUTION NO. 16- 426****BOARD OF COUNTY COMMISSIONERS
COUNTY OF EL PASO, STATE OF COLORADO**

Resolution Denying an Appeal by Hammers Construction LLC (APP-16-002) of the Administrative Determination made by the Planning and Community Development Department Executive Director regarding the requirement for permanent/post construction Water Quality (permanent stormwater quality best management practices or BMP's).

WHEREAS, pursuant to §§30-11-101(1)(e) and 30-11-107(1)(e), C.R.S., the Board of County Commissioners of El Paso County, Colorado (hereinafter "Board") has the legislative authority to manage the concerns of El Paso County when deemed by the Board to be in the best interests of the County and its inhabitants; and

WHEREAS, after consultation with the County Attorney's Office, the Executive Director of Planning and Community Development on August 4, 2016 issued an administrative determination finding made an administrative determination that all undeveloped lots within the Claremont Business Park are subject to installation of permanent stormwater management best management practices (BMP's) associated with development, and that the terms of a 2008 approved deviation relieving the developer of the requirements have not been met.; and

WHEREAS, an appeal of the administrative determination was filed by Hammers Construction on August 10, 2016, and a hearing date was set for September 27, 2016 to hear the appeal; and

WHEREAS, the hearing was continued to a date certain of November 22, 2016; and

WHEREAS, at the Applicant's appeal hearing on November 22, 2016, testimony from the Applicant and the Applicant's representatives was heard by the Board in favor of the appeal, testimony from representatives of Planning and Community Development Department and was presented, and such testimony and associated evidence was weighed by the Board; and

WHEREAS, the Board, having reviewed the testimony and evidence, hereby finds and determines that the requested appeal of the administrative determination by the Planning and Community Development Executive Director by the Applicant did not satisfy the criteria of approval to overturn the administrative determination.

NOW, THEREFORE, BE IT RESOLVED that the Board of County Commissioners of El Paso County, Colorado, hereby denies the appeal of the administrative determination by Hammers Construction and determines that permanent stormwater management best management practices (BMP's) are required with new development within the Claremont Business Park: and

BE IT FURTHER RESOLVED that Sallie Clark, duly elected, qualified member and Chair of the Board of County Commissioners, or Darryl Glenn, duly elected, qualified member and Vice Chair of the Board of County Commissioners, be and is hereby authorized on behalf of the Board to execute any and all documents necessary to carry out the intent of the Board as described herein.

DONE THIS 22nd day of November, 2016, at Colorado Springs Colorado.

**BOARD OF COUNTY COMMISSIONERS
EL PASO COUNTY, COLORADO**


ATTEST: Cheryl D. Broerman
County Clerk & Recorder

By: Sallie Clark
Chair of the Board

**EXCERPT OF “FINAL DRAINAGE REPORT FOR CLAREMONT
BUSINESS PARK FIL NO. 2 “, BY MATRIX DESIGN DATED
NOVEMBER 2006
&
EXISTING DRAINAGE MAP**



FINAL DRAINAGE REPORT

For

“Claremont Business Park Filing No. 2”

Prepared for:
El Paso County
Department of Public Works
Engineering Division

On Behalf of:
Claremont Development, Inc.

Prepared by:



2435 Research Parkway, Suite 300
Colorado Springs, CO 80920
(719) 575-0100
fax (719) 572-0208

Revised November 2006

Engineer's Statement:

The *revisions* (changes made to the base Final Drainage Report since July, 2006) to the attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. The revisions encompassed adding additional right of way to the study area at the County's request, the handling of offsite drainage due to the additional right of way, a breakdown of private drainage within lot numbers 10 through 25 of Filing No. 2 due to cross-lot drainage (contrary to note # 25 on the recorded plat), profiling additional inlets along the channel edge, and rip-rap sizing for outlet structures along the channel. The Final Drainage Report dated July, 2006 was prepared under the direct supervision of Richard G. Gallegos, Jr. in July, 2006 and stamped (see next sheet).

The Final Drainage Report was prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing the *revisions* to this report.

Brady A. Shyrock
Registered Professional Engineer
State of Colorado
No. 38164

SEAL

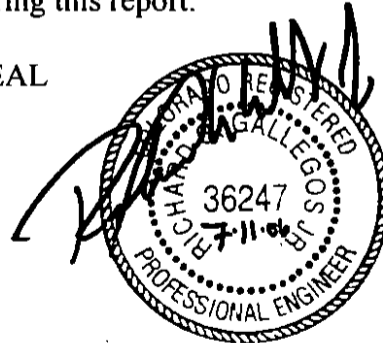


Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

SEAL

Richard G. Gallegos, Jr.
Registered Professional Engineer
State of Colorado
No. 36247

**Developer's Statement:**

I, the developer have read and will comply with all of the requirements specified in this drainage report and plan.

Claremont Development, Inc.

Business Name

By: _____

Title: _____

Address: 3460 Capital Drive
Colorado Springs, CO 80915

El Paso County:

Filed in accordance with Section 51.1 of the El Paso Land Development Code, as amended.

For Mr. John McCarthy, County Engineer/Director

Date 4/23/07

Conditions:

D. Drainage and Bridge Fees

Claremont Business Park Filing No. 2 has not been previously platted. The drainage basin and bridge fees have been determined per the El Paso County Drainage Basin Fees Sheet, dated February 3, 2006, Resolution No. 06-31. The site is located entirely within the Sand Creek Drainage Basin. The fees are based upon the percent impervious of the development, which have been included within the appendix of this report. The fees due have been calculated as follows.

Claremont Business Park Filing No. 2

Final Drainage Report
Drainage and Bridge Fees

| | Area (ac.) | Fee/Imp. Acre | % Imp. | Fee Due | Reimbursable Const. Costs | Fee Credit | Fee Due at Platting | Fee Credit Remaining |
|----------------------------------|---------------|------------------|--------|--------------|------------------------------|----------------|------------------------|-------------------------|
| Drainage Fee | 62.967 | \$15,000.00 | 80% | \$755,604.00 | \$0.00 | \$1,225,355.45 | \$0.00 | \$469,751.45 |
| Bridge Fee | 62.967 | \$1,503.00 | 80% | \$75,711.52 | \$75,711.52 | \$0.00 | \$0.00 | \$0.00 |
| Total Fee Due at Platting | | | | | | | \$0.00 | |

The developer of Claremont Business Park is completing the construction of the channel improvements on behalf of the Central Marksheffel Metropolitan District. The construction costs for both Filing 1 and Filing 2 combined exceed the drainage fees due for the site. No drainage fees will be required at the time of platting.

It should be noted that the Central Marksheffel Business District is reimbursing the developer of Claremont Business Park Filing 2 for the construction costs of the channel minus the drainage fees due for the site. The District has \$1,225,355.45 of drainage credits available within the Sand Creek Basin. This credit amount is based upon the construction cost estimate for the channel minus the drainage fees assessed for Claremont Business Park Filing No. 1. The District will use an additional \$755,604.00 of the drainage credits for the platting of Claremont Business Park Filing No. 2. The District will have \$469,751.45 of drainage credits left within the Sand Creek Fee basin.

The Central Marksheffel Business District has also constructed the Marksheffel Road Bridge over East Fork Sand Creek. This structure has been identified as a needed public improvement within the Drainage Basin Planning Study for Sand Creek and is eligible for reimbursement. Since the construction of the Marksheffel Bridge is in excess of the \$75,711.52 in bridge fees due for this site, no bridge fees are required at the time of platting. The fee will be deducted from the eligible reimbursable construction costs of the bridge and the remaining credits will be utilized or reimbursement applied for by the Central Marksheffel Business District.

OWNER ≠ ADVANCED RV & SELF STORAGE LP
(69.8 i.c.)

LAND USE = VACANT LAND

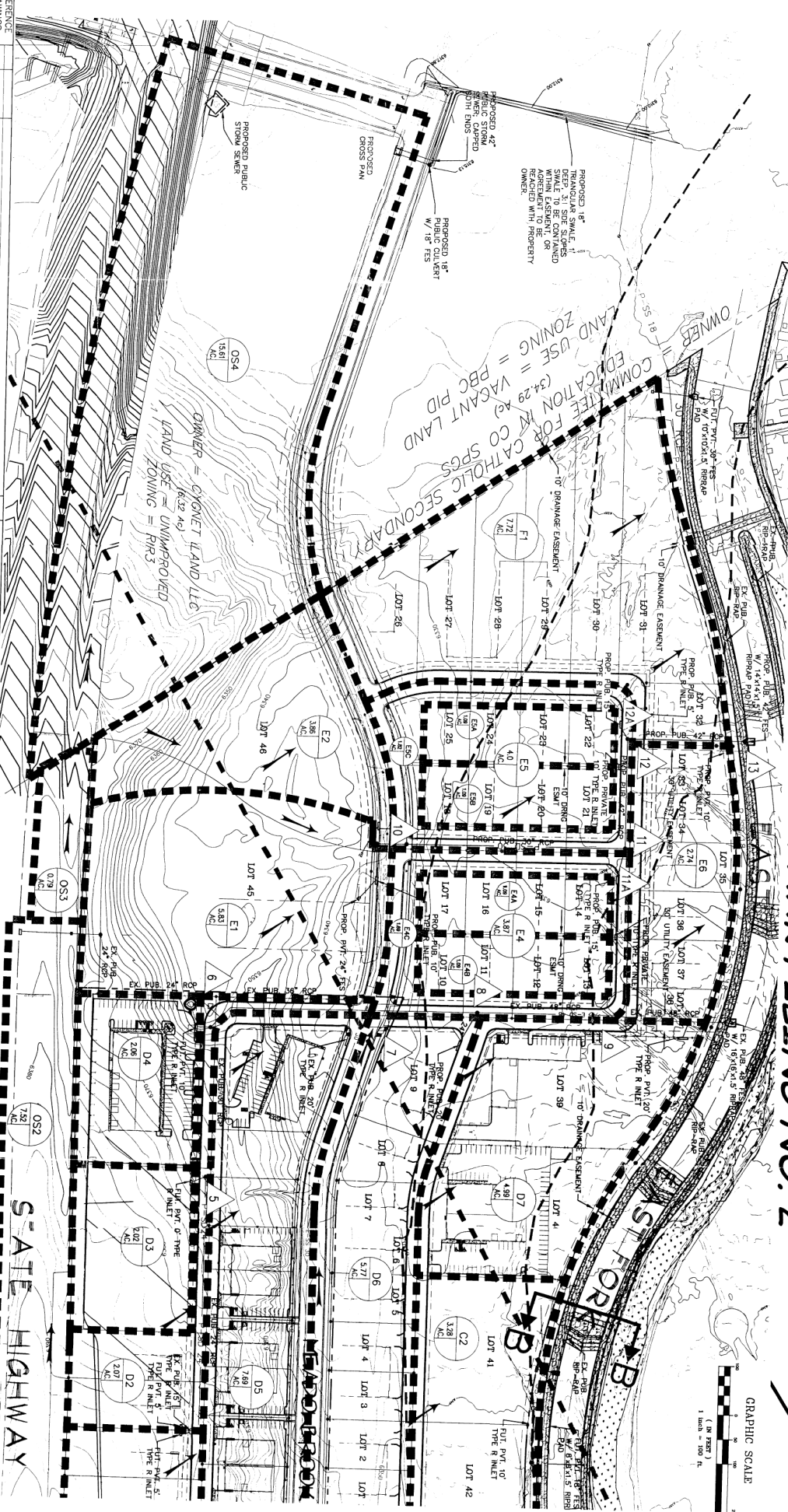
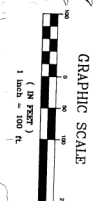
35
ZONING = PHD-7D-7
6-2-10

N.T.S.
FOR INFORMATION ONLY

1

| DESIGN POINT SUMMARY | | | |
|----------------------|-----------|----------|----|
| POINT | ELEVATION | DATE | BY |
| 1 | 7200 | 01/15/07 | MM |
| 2 | 7200 | 01/15/07 | MM |
| 3 | 7200 | 01/15/07 | MM |
| 4 | 7200 | 01/15/07 | MM |
| 5 | 7200 | 01/15/07 | MM |
| 6 | 7200 | 01/15/07 | MM |
| 7 | 7200 | 01/15/07 | MM |
| 8 | 7200 | 01/15/07 | MM |
| 9 | 7200 | 01/15/07 | MM |
| 10 | 7200 | 01/15/07 | MM |

DRAINAGE PLAN CLAREMONT BUSINESS PARK FILING NO. 2



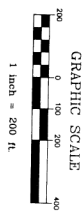
| REFERENCE DRAWINGS | | SUBDIVIDER | | REVISIONS | | BENCHMARK DATA/ELEV. | | DESCRIPTION / LOCATION | |
|--------------------|----------|------------|----------|-----------|----------|----------------------|----------|------------------------|----------|
| NO. | DATE | NO. | DATE | NO. | DATE | NO. | DATE | NO. | DATE |
| 1 | 01/15/07 | 1 | 01/15/07 | 1 | 01/15/07 | 1 | 01/15/07 | 1 | 01/15/07 |
| 2 | 01/15/07 | 2 | 01/15/07 | 2 | 01/15/07 | 2 | 01/15/07 | 2 | 01/15/07 |
| 3 | 01/15/07 | 3 | 01/15/07 | 3 | 01/15/07 | 3 | 01/15/07 | 3 | 01/15/07 |
| 4 | 01/15/07 | 4 | 01/15/07 | 4 | 01/15/07 | 4 | 01/15/07 | 4 | 01/15/07 |
| 5 | 01/15/07 | 5 | 01/15/07 | 5 | 01/15/07 | 5 | 01/15/07 | 5 | 01/15/07 |
| 6 | 01/15/07 | 6 | 01/15/07 | 6 | 01/15/07 | 6 | 01/15/07 | 6 | 01/15/07 |
| 7 | 01/15/07 | 7 | 01/15/07 | 7 | 01/15/07 | 7 | 01/15/07 | 7 | 01/15/07 |
| 8 | 01/15/07 | 8 | 01/15/07 | 8 | 01/15/07 | 8 | 01/15/07 | 8 | 01/15/07 |
| 9 | 01/15/07 | 9 | 01/15/07 | 9 | 01/15/07 | 9 | 01/15/07 | 9 | 01/15/07 |
| 10 | 01/15/07 | 10 | 01/15/07 | 10 | 01/15/07 | 10 | 01/15/07 | 10 | 01/15/07 |

Matrix Design Group, Inc.
Integrated Design Solutions 2435 Research Parkway, Suite 300
Colorado Springs, CO 80920
Phone 719-575-4100
Fax 719-575-0208

CLAREMONT BUSINESS PARK
REVISED FINAL DRAINAGE PLAN
MASTER DEVELOPMENT DRAINAGE PLAN
REVISED FINAL DRAINAGE PLAN
FILING NO. 2

DR02

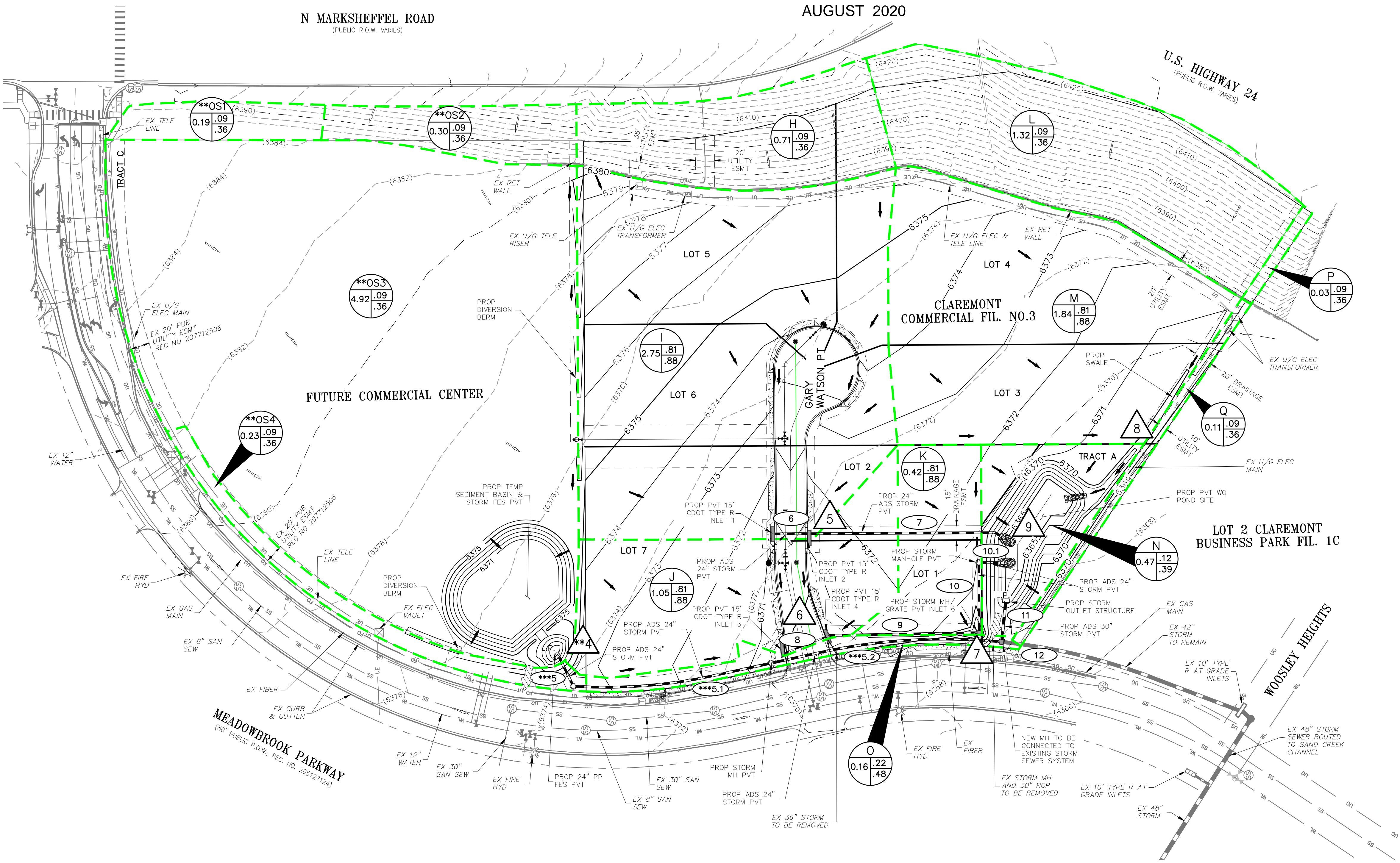
1

EXH01

PROPOSED DRAINAGE MAP

FINAL DRAINAGE REPORT FOR
CLAREMONT BUSINESS PARK 2 FILING NO. 1
COUNTY OF EL PASO, STATE OF COLORADO
PROPOSED DRAINAGE MAP

AUGUST 2020



LEGEND

- BASIN DESIGNATION
ACRES
PIPE RUN REFERENCE LABEL
SURFACE DESIGN POINT
BASIN BOUNDARY
EXISTING CONTOUR
PROP CONTOUR
UNDERGROUND ELECTRICAL
EXISTING GAS LINE
STORM SEWER PIPE
EXISTING STORM SEWER PIPE
CROSSSPAN
INLET
EXISTING FLOW DIRECTION
EMERGENCY OVERFLOW DIRECTION
FLOW DIRECTION
FLARED END SECTION
HIGH POINT
LOW POINT

| BASIN SUMMARY | | | | |
|---------------|--------------|----------------|------------------|--|
| BASIN | AREA (ACRES) | Q ₅ | Q ₁₀₀ | |
| **OS1 | 0.19 | 0.1 | 0.6 | |
| **OS2 | 0.3 | 0.1 | 1.0 | |
| **OS3 | 4.92 | 1.6 | 10.7 | |
| **OS4 | 0.23 | 0.1 | 0.7 | |
| H | 0.71 | 0.3 | 2.0 | |
| I | 2.75 | 9.8 | 17.9 | |
| J | 1.05 | 4.4 | 8.0 | |
| K | 0.42 | 1.8 | 3.2 | |
| L | 1.32 | 0.5 | 3.7 | |
| M | 1.84 | 6.7 | 12.2 | |
| N | 0.47 | 0.2 | 1.3 | |
| O | 0.16 | 0.2 | 0.6 | |
| P | 0.03 | 0.0 | 0.1 | |
| Q | 0.11 | 0.0 | 0.3 | |

| DESIGN POINT SUMMARY | | | | |
|----------------------|----------------|------------------|-------------------|---------------------------|
| DESIGN POINT | Q ₅ | Q ₁₀₀ | BASIN | STRUCTURE |
| **4 | 1.8 | 11.8 | **OS1,**OS2,**OS3 | 24" PP & FES |
| 5 | 10.1 | 19.8 | H, I | 2-15' INLETS |
| 6 | 4.4 | 9.8 | J, FBOP5 | 2-15' INLETS |
| 7 | 1.8 | 3.2 | K | CDOT TYPE C INLET W/GRATE |
| 8 | 7.2 | 15.7 | L, M | 24" PP OR SWALE |
| 9 | 23.1 | 46.9 | DP8, 7,10,1, N | POND 2 |

| STORM SEWER SUMMARY | | | | |
|---------------------|----------------|------------------|-----------|-----------------------------|
| PIPE RUN | Q ₅ | Q ₁₀₀ | PIPE SIZE | CONTRIBUTING DP/BASIN/PIPES |
| ***5 | 7.2 | 15.5 | 24" | **DP4 |
| ***5.1 | 7.2 | 15.5 | 24" | ***PR5 |
| ***5.2 | 7.2 | 15.5 | 24" | ***PR5.1 |
| 6 | 6.4 | 9.1 | 24" | INLET 1 |
| 7 | 10.1 | 18.3 | 24" | PR6, INLET 2 |
| 8 | 3.0 | 4.9 | 18" | INLET 3 |
| 9 | 4.5 | 9.8 | 24" | PR8, INLET 4 |
| 10 | 6.3 | 13.0 | 24" | PR9, DP7 |
| 10.1 | 6.3 | 13.0 | 24" | PR10 |
| 11 | 13.2 | 23.8 | 30" | POND 2 OUTFALL |
| 12 | 20.4 | 39.3 | EX42" | ***PR5.2, PR11 |

**EXISTING UNDEVELOPED
***ULTIMATE BUILD OUT, DEVELOPED. USED TO SIZE FUTURE POND 1 AND STORM SEWER.

| SF WQCV POND 2 SUMMARY | | |
|--|----------|--|
| EPC/URBAN DRAINAGE SAND FILTER BASIN-SEE STD. DET. | | |
| AREA REQUIRED | 2,962 SF | |
| AREA PROVIDED | 2,450 SF | |

SF ELEV = 6364.45
WQCV WSE = 6366.17
100 YR SPILLWAY ELEV = 6367.74
100 YR WSE = 6367.38

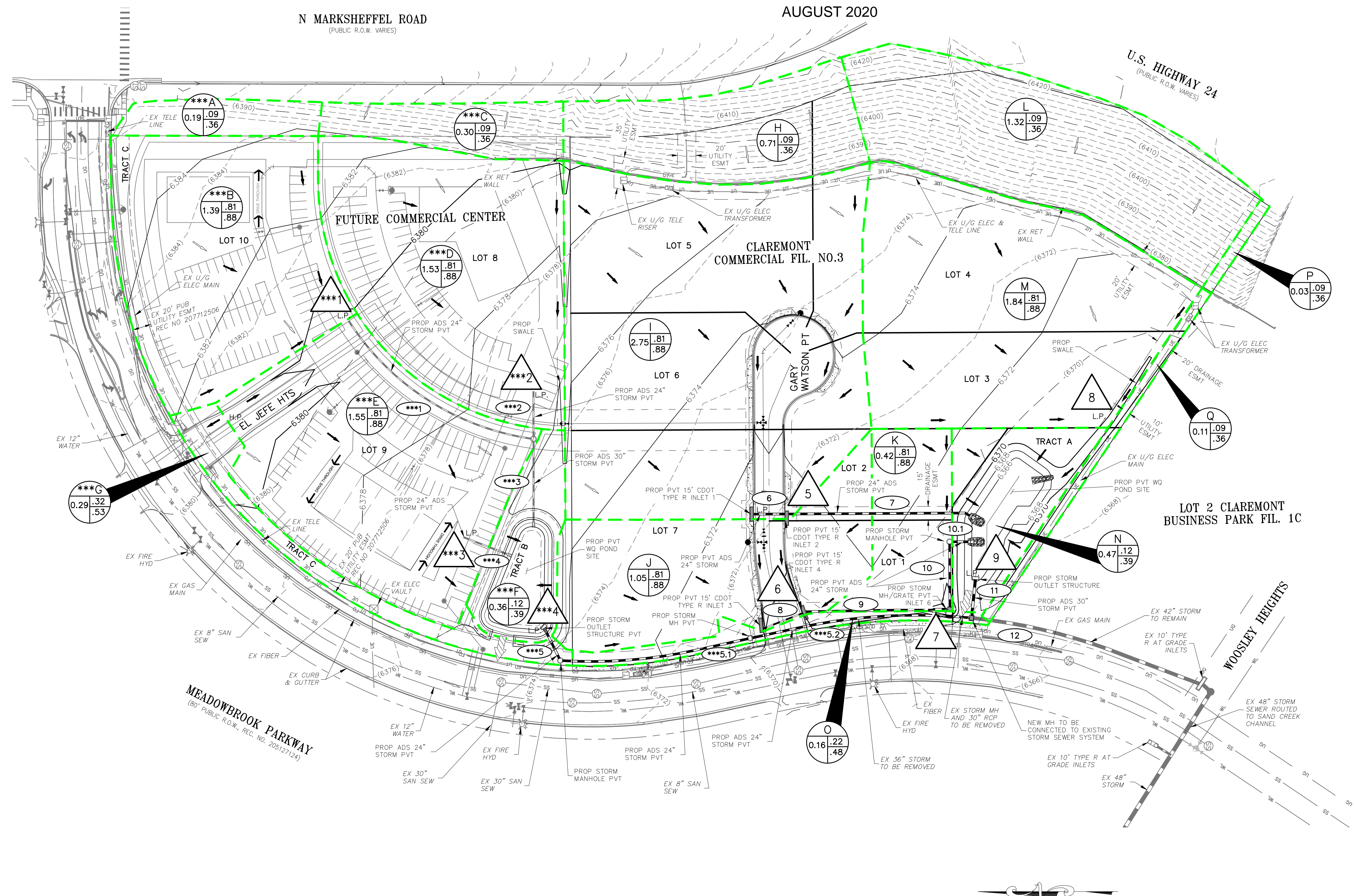
FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES
FOR BURIED UTILITY INFORMATION 48 HRS BEFORE YOU DIG CALL 1-800-922-1987

M&S
CIVIL CONSULTANTS, INC.
102 E. PIKES PEAK AVE., 5TH FLOOR
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

| CLAREMONT BUSINESS PARK 2 FIL. NO.1 | | | |
|--------------------------------------|--|------------------|-------|
| PROP. DRAINAGE W/OFFSITE UNDEVELOPED | | | |
| PROJECT NO. 44-037A | FILE: \dwg\Eng Exhibits\44037-FDRM-1.dwg | DATE: 08-04-2020 | |
| DESIGNED BY: ET | SCALE: HORIZ: 1"=60' | | |
| DRAWN BY: CLP | VERT: N/A | | |
| CHECKED BY: VAS | | SHEET 1 OF 2 | FDM01 |

FINAL DRAINAGE REPORT FOR
CLAREMONT BUSINESS PARK 2 FILING NO.1
COUNTY OF EL PASO, STATE OF COLORADO
PROPOSED DRAINAGE MAP

AUGUST 2020



LEGEND

- BASIN DESIGNATION: Z, 25, .25, .35, C5, C100
- ACRES: 4, 6
- PIPE RUN REFERENCE LABEL: 4, 6
- SURFACE DESIGN POINT: 6
- BASIN BOUNDARY: --- (6920) ---
- EXISTING CONTOUR: --- (6920) ---
- PROP CONTOUR: --- 6920 ---
- UNDERGROUND ELECTRICAL: --- UGE ---
- EXISTING GAS LINE: --- G ---
- STORM SEWER PIPE: --- S ---
- EXISTING STORM SEWER PIPE: --- S ---
- CROSSSPAN: --- C ---
- INLET: --- I ---
- EXISTING FLOW DIRECTION: --- F ---
- EMERGENCY OVERFLOW DIRECTION: --- E ---
- FLOW DIRECTION: --- F ---
- FLARED END SECTION: --- F ---
- HIGH POINT: H.P. X
- LOW POINT: L.P. X

| BASIN SUMMARY | | | | |
|---------------|-------|--------------|----------------|------------------|
| 1 | A | B | C | D |
| 2 | BASIN | AREA (ACRES) | Q ₅ | Q ₁₀₀ |
| 3 | ***A | 0.19 | 0.1 | 0.6 |
| 4 | ***B | 1.39 | 5.5 | 10.1 |
| 5 | ***C | 0.3 | 0.1 | 1.0 |
| 6 | ***D | 1.53 | 6.3 | 11.5 |
| 7 | ***E | 1.55 | 6.5 | 11.8 |
| 8 | ***F | 0.36 | 0.2 | 1.0 |
| 9 | ***G | 0.29 | 0.5 | 1.3 |
| 10 | H | 0.71 | 0.3 | 2.0 |
| 11 | I | 2.75 | 9.8 | 17.9 |
| 12 | J | 1.05 | 4.4 | 8.0 |
| 13 | K | 0.42 | 1.8 | 3.2 |
| 14 | L | 1.32 | 0.5 | 3.7 |
| 15 | M | 1.84 | 6.7 | 12.2 |
| 16 | N | 0.47 | 0.2 | 1.3 |
| 17 | O | 0.16 | 0.2 | 0.6 |
| 18 | P | 0.03 | 0.0 | 0.1 |
| 19 | Q | 0.11 | 0.0 | 0.3 |

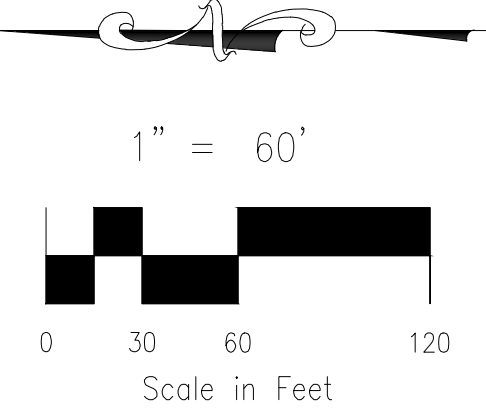
| SF WQCV POND 1 SUMMARY | | | | |
|--|-------|----|--|--|
| EPC/URBAN DRAINAGE SAND FILTER BASIN-SEE STD. DET. | | | | |
| AREA REQUIRED | 2,335 | SF | | |
| AREA PROVIDED | 3,690 | SF | | |
| SF ELEV = 6373.00 | | | | |
| WQCV WSE = 6374.26 | | | | |
| 100 YR SPILLWAY ELEV = 6375.00 | | | | |
| 100 YR WSE = 6375.00 | | | | |

| SF WQCV POND 2 SUMMARY | | | | |
|--|-------|----|--|--|
| EPC/URBAN DRAINAGE SAND FILTER BASIN-SEE STD. DET. | | | | |
| AREA REQUIRED | 2,962 | SF | | |
| AREA PROVIDED | 2,450 | SF | | |
| SF ELEV = 6364.45 | | | | |
| WQCV WSE = 6366.17 | | | | |
| 100 YR SPILLWAY ELEV = 6367.74 | | | | |
| 100 YR WSE = 6367.38 | | | | |

| DESIGN POINT SUMMARY | | | | |
|----------------------|----------------|------------------|------------------|-----------------|
| DESIGN POINT | Q ₅ | Q ₁₀₀ | BASIN | STRUCTURE |
| ***1 | 5.6 | 10.6 | ***A, ***B | 24" PP |
| ***2 | 6.4 | 12.4 | ***C, ***D | 24" PP |
| ***3 | 6.5 | 11.8 | ***E | 24" PP |
| ***4 | 18.2 | 35.0 | ***F, ***G, ***H | POND 1 |
| 5 | 10.1 | 19.8 | H, I | 2-15' INLETS |
| 6 | 4.4 | 9.8 | J, FBPD5 | 2-15' INLETS |
| 7 | 1.8 | 3.2 | K | MH W/GRATE |
| 8 | 7.2 | 15.7 | L, M | 24" PP OR SWALE |
| 9 | 23.1 | 46.9 | DP8, 7, 10, 1, N | POND 2 |

| STORM SEWER SUMMARY | | | | |
|---------------------|----------------|------------------|-----------|-----------------------------|
| PIPE RUN | Q ₅ | Q ₁₀₀ | PIPE SIZE | CONTRIBUTING DP/BASIN/PIPES |
| ***1 | 5.6 | 10.6 | 24" | ***DP1 |
| ***2 | 6.4 | 12.4 | 24" | ***DP2 |
| ***3 | 11.8 | 22.6 | 30" | ***PR1, ***PR2 |
| ***4 | 6.5 | 11.8 | 24" | ***DP3 |
| ***5 | 7.2 | 15.5 | 24" | POND 1 OUTFALL |
| ***5.1 | 7.2 | 15.5 | 24" | ***PR5 |
| ***5.2 | 7.2 | 15.5 | 24" | ***PR5.1 |
| 6 | 6.4 | 9.1 | 24" | INLET 1 |
| 7 | 10.1 | 18.3 | 30" | PR6, INLET 2 |
| 8 | 3.0 | 4.9 | 18" | INLET 3 |
| 9 | 4.5 | 9.8 | 24" | PR8, INLET 4 |
| 10 | 6.3 | 13.0 | 24" | PR9, DP7 |
| 10.1 | 6.3 | 13.0 | 24" | PIPE 10 |
| 11 | 13.2 | 23.8 | 30" | POND 2 OUTFALL |
| 12 | 20.4 | 39.3 | EX42" | ***PR5.2, PR11 |

***ULTIMATE BUILD OUT, DEVELOPED. DESIGNED TO SIZE FUTURE POND 1 AND STORM SEWER.



FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES
FOR BURIED UTILITY INFORMATION
48 HRS BEFORE YOU DIG
CALL 1-800-922-1987

M&S
CIVIL CONSULTANTS, INC.
102 E. PIKES PEAK AVE., 5TH FLOOR
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

| CLAREMONT BUSINESS PARK 2 FIL. NO.1 | | | | |
|-------------------------------------|---|------------------|--------------|-------|
| PROP. DRAINAGE OFFSITE DEVELOPED | | | | |
| PROJECT NO. 44-037A | FILE: \\dwg\Eng Exhibits\44037-FDRM-2.dwg | DATE: 08-04-2020 | | |
| DESIGNED BY: ET | SCALE: HORIZ: 1"=60' | VERT: N/A | | |
| DRAWN BY: CLP | | | SHEET 2 OF 2 | FDM02 |
| CHECKED BY: VAS | | | | |