# A-1 CHIPSEAL LOT 36 AND 37 CLAREMONT BUSINESS PARK FIL NO 2 7245 COLE VIEW COLORADO SPRINGS, COLORADO

#### **MAY 2023**

Prepared For: **A-1 CHIPSEAL** 

7245 Cole View Colorado Springs, CO 80915 720.540.8264

Contact: Stephanie Wallis

#### Prepared By:

# TERRA NOVA ENGINEERING, INC.

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TNE Job No. 2173.00 County Job No. COM-22-014

# A-1 CHIPSEAL

# LOT 36 AND 37 CLAREMONT BUSINESS PARK FIL NO 2 7245 COLE VIEW COLORADO SPRINGS, COLORADO

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#### **APPENDICIES**

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NRCS SOILS MAP
FEMA FIRM MAP
HYDROLOGIC CALCULATIONS
HYDRAULIC CALCULATIONS
DRAINAGE MAPS

# A-1 CHIPSEAL LOT 36 AND 37 CLAREMONT BUSINESS PARK FIL NO 2 7245 COLE VIEW COLORADO SPRINGS, COLORADO

Conditions:

DESIGN ENGINEER'S STATEMENT:	
The attached drainage plan and report were prepared under my direction	n and supervision and are correct to
the best of my knowledge and belief. Said drainage report has been	
established by the County for drainage reports and said report is in cor	
plan of the drainage basin. I accept responsibility for any liability cau	
omissions on my part in preparing this report.	or of my megagem arm, enters or
RADO LICANO	
I time having	
Dave Frank 2023/05/04	
Vane trank \$ 2023/05/04 /2	2023/05/04
Dane Frank, P.E. 50207	Date
On behalf of Terra Nova Engineering, Inc.	
OWNER/DEVELOPER'S STATEMENT:	
I, the owner/developer have read and will comply with all of the requir	ements specified in this drainage
report and plan.	
· /// ~ ///	
	•
W V	2-4-33
Authorized Signature	5-4-33 Date
Printed Name, Title	
Printed Name, Title	
Trined Paine, Title	
A-1 Chiese of Co	
Business Name	
Dusiness Name	
Business Name  SUT E. 7th Are Denver (0	E1235
Address	6.00
Address	
EL PASO COUNTY:	
Filed in accordance with the requirements of the Drainage Criteria M	Manual, Volumes 1 and 2, El Paso
County Engineering Criteria Manual and Land Development Code as a	
	····
	,
Joshua Palmer, P.E.	Date
County Engineer/ECM Administrator	Date
County Engineer/ECW Administrator	

# A-1 CHIPSEAL LOT 36 AND 37 CLAREMONT BUSINESS PARK FIL NO 2

# 7245 COLE VIEW COLORADO SPRINGS, COLORADO

#### **PURPOSE**

The purpose of this Final Drainage Report is to identify and analyze the proposed drainage patterns, determine proposed runoff quantities, and present solutions to drainage impacts on-site and off-site resulting from this development. The site has previously been platted and has previously been studied in:

"Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, prepared by Matrix Design Group.

#### **GENERAL DESCRIPTION**

This Final Drainage Report (FDR) is an analysis of approximately 1.36 acres of developed land located at 7245 and 7231 Cole View. This site is currently in use as a paving business. This report and water quality are required because 0.38 acres of asphalt was recently added to the southern portion of the site. The site is in the northeast quarter of Section 8, Township 14 South, Range 65 West of the 6<sup>th</sup> Principal Meridian within El Paso County. The parcels are bounded to the north and west by Sand Creek, to the southwest by LOT 35 CLAREMONT BUSINESS PARK FIL NO 2, to the southeast by LOTS 13 AND 14 CLAREMONT BUSINESS PARK FIL NO 2, and to the north by LOT 39 CLAREMONT BUSINESS PARK FIL NO 2. (see vicinity map).

The site lies within the Sand Creek Basin, with storm runoff surface draining west across the site, then into a gutter that flows off the site to the south, eventually entering a storm inlet, which drains into the adjacent East Fork Sand Creek. There are also storm inlets in Marksheffel Road that flow into a storm sewer that flows south along Marksheffel to an unknown outfall, which presumably also drains into Sand Creek.

Soils for this project are delineated by the map in the appendix as Ellicott loamy coarse sand, 0 to 5 percent slopes (28). Soils in the study area are shown as mapped by NRCS in the "Soils Survey of El Paso County Area" and contains soils of Hydrologic Group A.

The site is developed with mostly pavement and roof surfaces, and a small amount of landscaping. The site drains to the west, with an average slope of 3.7%.

#### HISTORIC DRAINAGE CONDITIONS

The site was previously developed with one building and outdoor parking and equipment storage. Historically, the area that has now been given an asphalt surface was composed of dirt with little to no vegetation. There are two drainage basins. Historic Drainage Calculations have been added to the appendix with assumptions made about the site based on historic aerial photos.

Basin HS-A is 1.01 acres that is a combination of roof, parking area, and bare earth which drains to Design Point A and leaves the site in an existing carry curb. Basin HS-A has flows of  $Q_5 = 2.6$  cfs and  $Q_{100} = 6.0$  cfs.

Basin HS-B is 0.35 acres that is mostly landscaping and half a street and drains to Design Point B at the south corner of the site and flows offsite in the street gutter. Basin HS-B has flows of  $Q_5 = 1.2$  cfs and  $Q_{100} = 2.7$  cfs.

#### **EXISTING DRAINAGE CONDITIONS**

The site was previously developed with one building and outdoor parking and equipment storage. A 0.38 acre area which was historically composed of dirt with little to no vegetation has now been given an asphalt surface. There are two drainage basins. See attached Existing Drainage Map (in appendix).

Basin EX-A is 1.01 acres that is mostly roof and parking area and drains to Design Point A and leaves the site in an existing carry curb. Basin EX-A has flows of  $Q_5 = 4.4$  cfs and

Basin EX-B is 0.35 acres that is mostly landscaping and half a street and drains to Design Point B at the south corner of the site and flows offsite in the street gutter. Basin EX-B has flows of  $Q_5 = 1.2$  cfs and  $Q_{100} = 2.7$  cfs.

#### PROPOSED DRAINAGE CONDITIONS

The proposed drainage conditions are the same as the existing drainage conditions, with the exception being the removal of 3,300 square feet of asphalt and its replacement with 3,300 square feet of turf grass in the west corner of the site. The County is requiring the addition of a water quality feature retroactively following paving of 0.38 acres on the south side of the site. "The Final Drainage Report For Claremont Business Park Filing No. 2" assumed weighted coefficients of C(5)=0.80 and C(100)=0.90 for the proposed drainage basin which included this project site. In the appendix, this report has calculated weighted coefficients of C(5)=0.86 and C(100)=0.94 for Basin EX-A and C(5)=0.71 and C(100)=0.84 for Basin EX-B in the existing conditions and of C(5)=0.81 and C(100)=0.91 for Basin EX-A and C(5)=0.71 and C(100)=0.84 for Basin EX-B in the prosed conditions. Therefore, the imperviousness used in the existing and proposed reports are similar to the predictions made in the previous drainage report. Flows from Basin EX-B do not change from the historic conditions while flows from Basin EX-A increase by 1.5 cfs in the 5-year flow and 2.3 cfs in the 100-year flow. This additional runoff is directed towards and treated by the proposed turf grass acting as a water quality treatment area which is described below.

Water quality treatment for the recently added asphalt area in Basin EX-A is provided by the proposed turf grass area being added western corner of the site, downstream of the impervious asphalt area. The Runoff Reduction Spreadsheet shows the area being used (see appendix), and a visual representation of this area is shown on the Proposed Drainage Map. The proposed turf grass only provides water quality treatment for the recently added asphalt and has been oriented to accept as much sheet flow from this impervious area as possible.

In an effort to protect receiving water and as part of the "four-step process to minimize adverse impacts of urbanization" this site was analyzed in the following manner:

- 1. Reduce Runoff- The impervious asphalt area that was recently added to the site is directly upstream of the proposed turf grass which covers a 3,300 square foot area on the low end of the site. This will reduce the volume of runoff using ponding and infiltration. See the Runoff Reduction spreadsheet in the appendix for calculations showing how runoff from this impervious area is treated by the receiving pervious area.
- Stabilize Drainageways- There are no existing or proposed drainageways onsite.
   The adjacent East Fork Sand Creek has previously been stabilized and runoff from the site currently flows to a storm sewer system that discharges into East Fork Sand Creek.
- 3. Provide Water Quality Capture Volume (WQCV)- The proposed turf grass area will provide the water quality treatment for the recently added impervious area. See the Runoff Reduction Spreadsheet in the appendix.
- 4. Consider Need for Industrial and Commercial BMPs- As the site is currently used for a paving business, there are likely existing industrial BMPs in place at the site. However, no industrial or commercial BMPs are required for the site.

#### HYDROLOGIC CALCULATIONS

Hydrologic calculations were performed using the El Paso County Storm Drainage Design Criteria Manual - Volumes 1 & 2, latest editions. The Rational Method was used to estimate storm water runoff anticipated from design storms with 5-year and 100-year recurrence intervals.

#### HYDRAULIC CALCULATIONS

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County Storm Drainage Design Criteria Manual – Volumes 1 & 2, latest editions. The pertinent data sheets are included in the appendix of this report.

#### FLOODPLAIN STATEMENT

No portion of this site is within a designated F.E.M.A. floodplain, as determined by Flood Insurance Rate Map No. 08041C0752 G, dated December 7, 2018 (see appendix).

#### WATER QUALITY

Water quality treatment for the recently added asphalt in Basin EX-A is provided by the proposed turf grass area directly downstream in the western corner of the site. The Runoff Reduction Spreadsheet shows the area being used (see appendix), and a visual representation of this area is shown on the Proposed Drainage Map. The proposed turf grass only provides water quality treatment for the recently added asphalt. Detention is not necessary as the increase in runoff is not significant.

There is no water quality treatment for existing basin EX-B. This basin is already fully developed and no changes to it are proposed.

#### CONSTRUCTION COST OPINION

#### Public Reimbursable

None

#### **Public Non-Reimbursable**

None

#### **DRAINAGE FEES**

This drainage report is part of a site development application; therefore, no drainage fees are due.

#### **MAINTENANCE**

The proposed turf grass will be maintained by the property owner.

#### **SUMMARY**

Development of this site will not adversely affect the surrounding development. This report is in general conformance with the previous reports which included this site. Site

runoff and storm drain appurtenances from the A-1 Chipseal development will not adversely affect the downstream and surrounding developments and will be safely routed through the proposed grass buffer to treat the water quality capture volume. Runoff leaving the site is routed to the existing public storm sewer system.

PREPARED BY:

TERRA NOVA ENGINEERING, INC.

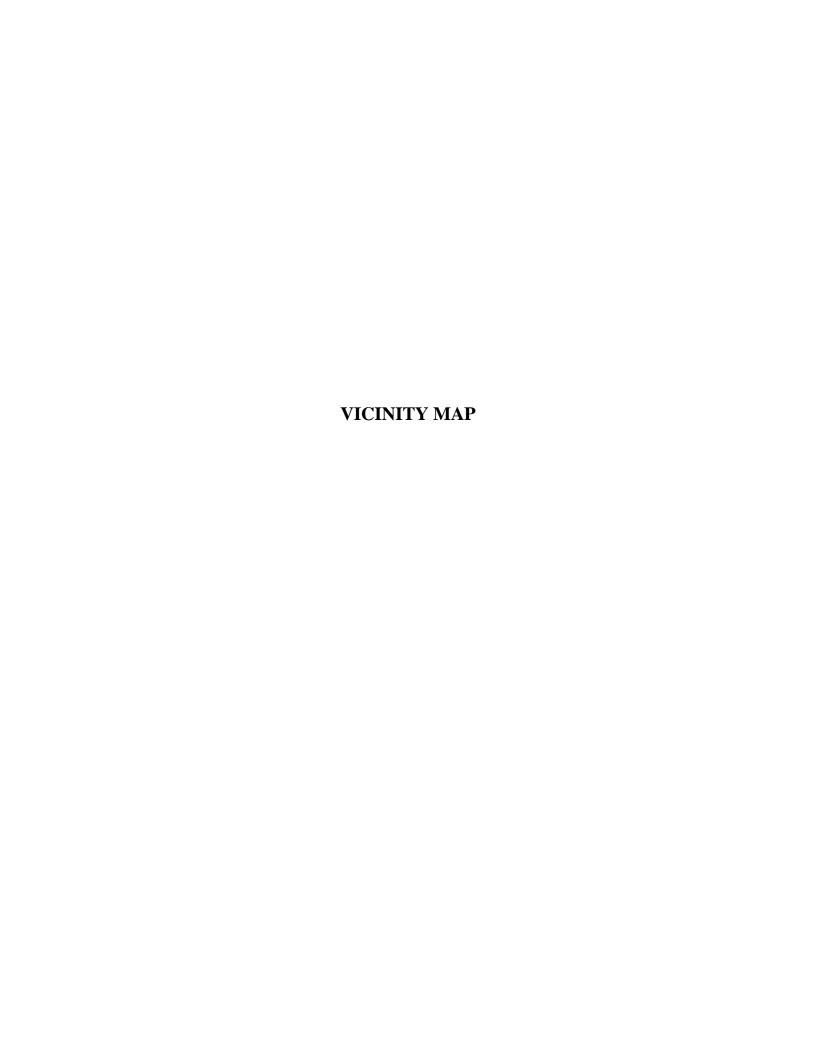
Dane Frank, P.E. Project Engineer

Jobs/2173.00/drainage/217300 FDR.doc

#### **BIBLIOGRAPHY**

El Paso County Drainage Criteria Manual-Volumes 1 & 2, latest edition

"Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, prepared by Matrix Design Group.



El Paso County - Community: Property Search

Schedule Number: 5408102040

A-1 Chipseal - Vicinity Map

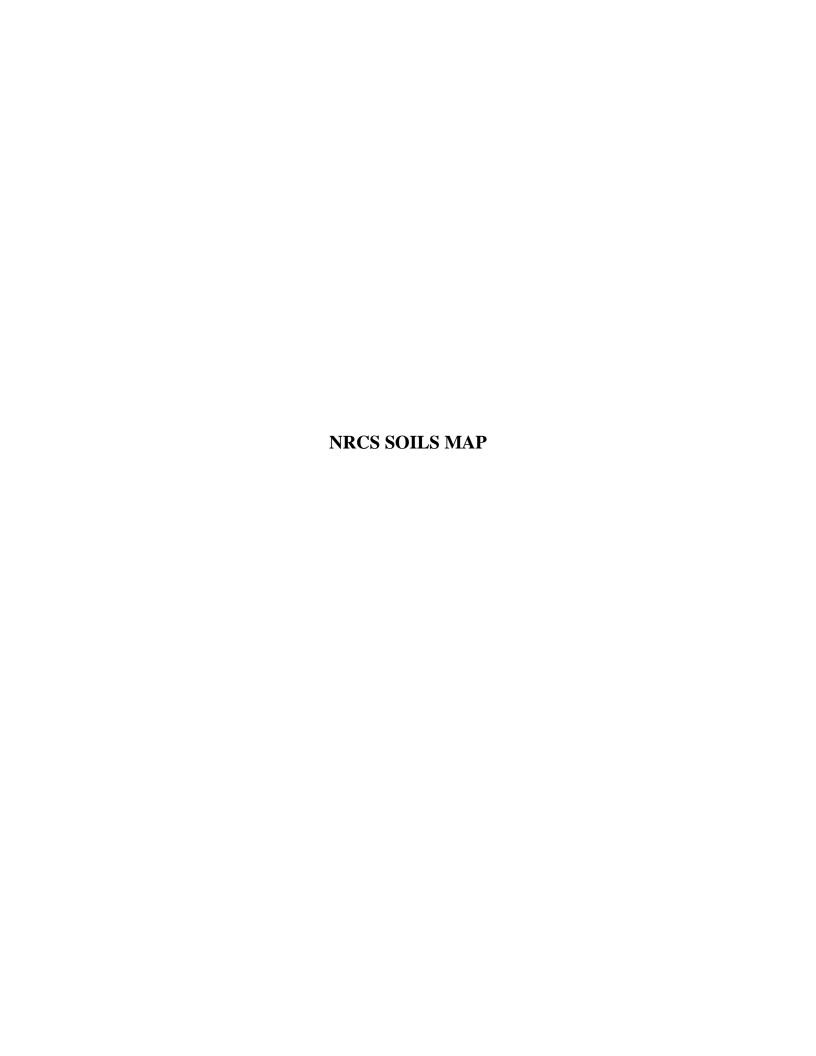


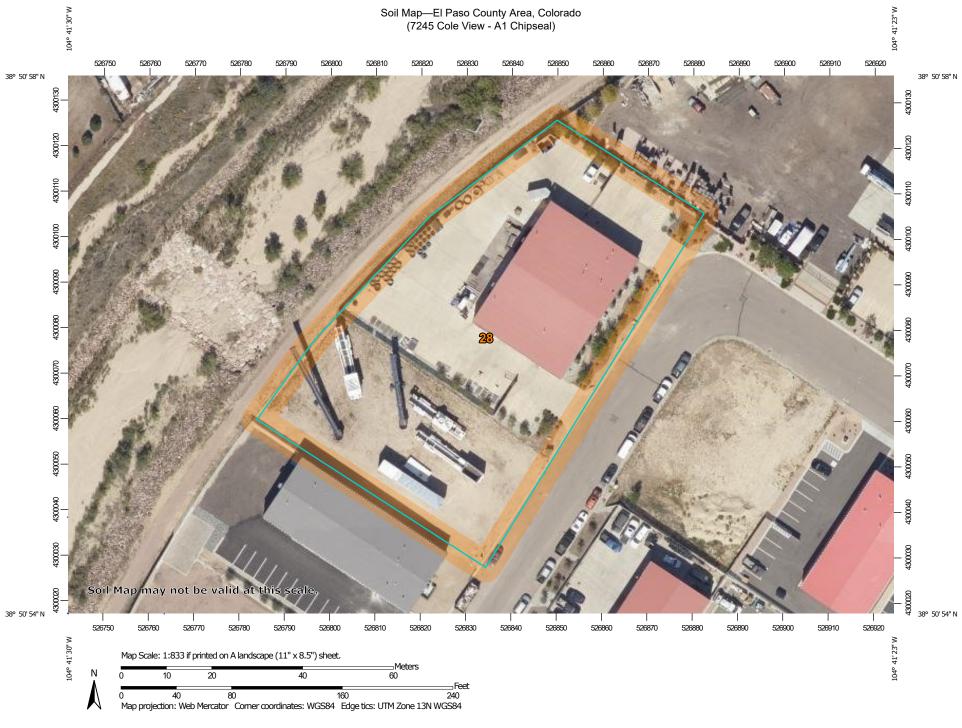
North is up ^

1 of 2 4/5/2022, 9:02 AM









#### MAP LEGEND

#### Area of Interest (AOI)

Area of Interest (AOI)

#### Soils

Soil Map Unit Polygons



Soil Map Unit Lines



Soil Map Unit Points

#### Special Point Features

Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



**Gravelly Spot** 



Landfill



Lava Flow

Marsh or swamp



Marsh of Swam



Mine or Quarry

Miscellaneous Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot

#### 8

Spoil Area



Stony Spot



Very Stony Spot



Wet Spot Other



Special Line Features

# Water Features

valer r

Streams and Canals

#### Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

#### Background



Aerial Photography

#### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 19, Aug 31, 2021

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Aug 19, 2018—Sep 23, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
28	Ellicott loamy coarse sand, 0 to 5 percent slopes	1.2	100.0%
Totals for Area of Interest		1.2	100.0%

### El Paso County Area, Colorado

#### 28—Ellicott loamy coarse sand, 0 to 5 percent slopes

#### **Map Unit Setting**

National map unit symbol: 3680 Elevation: 5,500 to 6,500 feet

Mean annual precipitation: 13 to 15 inches Mean annual air temperature: 47 to 50 degrees F

Frost-free period: 125 to 145 days

Farmland classification: Not prime farmland

#### **Map Unit Composition**

Ellicott and similar soils: 97 percent Minor components: 3 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

#### **Description of Ellicott**

#### Setting

Landform: Flood plains, stream terraces Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy alluvium

#### **Typical profile**

A - 0 to 4 inches: loamy coarse sand

C - 4 to 60 inches: stratified coarse sand to sandy loam

#### **Properties and qualities**

Slope: 0 to 5 percent

Depth to restrictive feature: More than 80 inches Drainage class: Somewhat excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High to

very high (5.95 to 19.98 in/hr)

Depth to water table: More than 80 inches Frequency of flooding: FrequentNone

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.1 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7w

Hydrologic Soil Group: A

Ecological site: R069XY031CO - Sandy Bottomland LRU's A and

В

Other vegetative classification: SANDY BOTTOMLAND

(069AY031CO) Hydric soil rating: No

#### **Minor Components**

#### Fluvaquentic haplaquoll

Percent of map unit: 1 percent Landform: Swales

Hydric soil rating: Yes

#### Other soils

Percent of map unit: 1 percent Hydric soil rating: No

#### **Pleasant**

Percent of map unit: 1 percent Landform: Depressions Hydric soil rating: Yes

#### **Data Source Information**

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 19, Aug 31, 2021



#### NOTES TO USERS

is map is for use in administering the National Flood Insurance Program. It does necessarily identify all areas subject to flooding, particularly from local drainage urces of small size. The community map repository should be consulted fo sable updated or additional flood hazard information.

To obtain more detabled information in sease where Baser Flood Elevations (EETs) another Gouleage likes been deficiently used as meaning to consult the Though professional professional

Coastal Base Flood Elevations shown on this map apply only landward of 0.0° North American Vertical Datum of 1986 (NAVD68). Users of this FIRM should be ware that coastal flood elevations are also provided in the Summary of Sillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Sillwater Elevations table should be used for construction ind/or floodplain management purposes when they are higher than the elevatio hown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway width and other pertinent floodway data are provided in the Flood insurance Study repor

Certain areas not in Special Flood Hazard Areas may be protected by **flood contro** structures. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAJ083, GRS90 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight postboral differences in map features across jurisdiction boundaries. These differences on not differences in map features acro-affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD89). These food elevations must be compared to structure and conversion between the National Geodesic Vertical Datum of 1923 and the North American Vertical Datum of 1988, visit the National Geodesic Curvey websites with publications of 1989 and 1989 are the National Geodesic Survey websites with publications again or contact the National Geodesic Survey are better the National Geodesic Survey and the Datum of 1989 and 1989 are the National Geodesic Survey and the National Geodesic Survey are the National Geodesic Survey and the National Geodesic Survey and the National Geodesic Survey are the National Geodesic Survey and the National Geodesic Survey are the National Geodesic Survey and the National Geodesic Survey are the National Geodesic Survey and National Survey Nationa

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench mark shown on this map, please contact the Information Services Branch of the Nation Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Cotorado Springs Utilities, City of Fountain, Bureau of Land Management, National Cocania and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map effects more detailed and up-to-date stream channel configurations and the map reference of the date of t paselines may deviate significantly from and may appear outside of the floodplain.

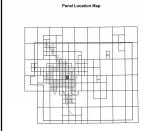
Corporate limits shown on this map are based on the best data available at the of publication. Because changes due to annexations or de-annexations may be occurred after this map was published, map users should contact appropriate ommunity officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood insurance Program dates on each community as well as a listing of the panels on which each community is

Contact ERAN May Sourice Center (INSC) is the FEMA May Information Oxforing, IRSM), 1-8477-38-2677 for information on waisable reviews assessment with FIRM. Available products may include newboardy issued Letter of May Change (FIRM) and the products may include newboardy issued Letter of May Change (Inschool Insurance Study Report, and/or digital versions of his map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.forma.gov/.

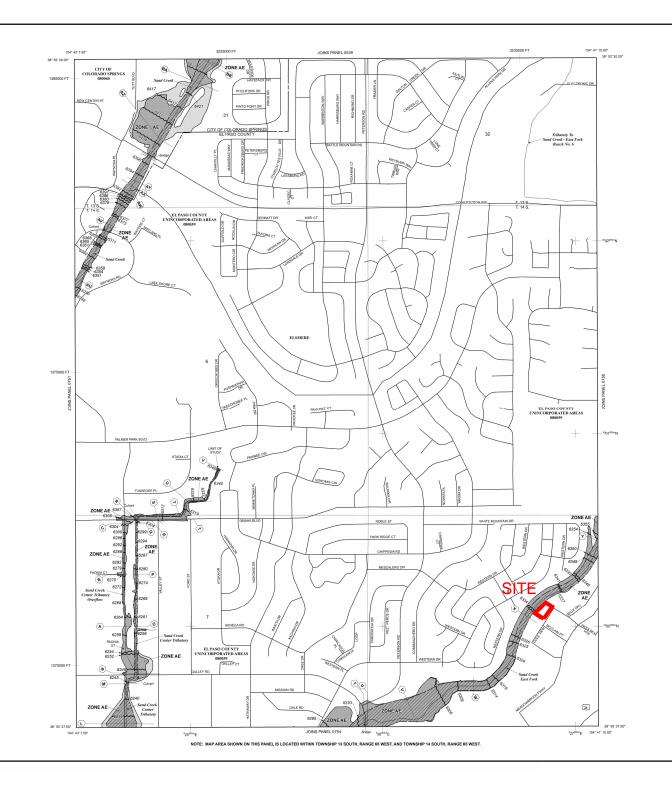
If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2827) or visit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table Flooding Source REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).







SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (1,00-year flood), also known as the base flood, is the flood that has a 1% chance of being equated or exceeded in any given year. The Special Flood Heard Ares is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Heard include zones A, AE, AM, AD, AR, A99, V, and VE. The Base Flood Deletolis is the water-surface elevation of the 1% annual chance flood.

ZONE A No Base Flood Blevations determined.

ZONE AE Base Flood Blevations determined.

ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Blevations determined.

Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.

Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.

ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood Bevations determined. Coastal flood zone with velocity hazard (wave action); Base Flood Bevations determined

FLOODWAY AREAS IN ZONE AE

OTHER FLOOD AREAS

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas Floodplain boundary

\_\_\_\_ Floodway boundary Zone D Boundary

..... CBRS and OPA boundary

-Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.

~~ 513 ~~ Base Flood Flevation line and value: elevation in feet\*

Base Flood Elevation value where uniform within zone; elevation in feet\*

\* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

23-----23

Geographic coordinates referenced to the North American Datum of 1983 (NAD 83) 97" 07" 30.00" 32" 22" 30.00"

4275000nN 1000-meter Universal Transverse Mercator grid ticks, zone 13

Bench mark (see explanation in Notes to Users section of this FIRM panel) DX5510

M1.5

NATIONAL F

MAP REPOSITORIES Refer to Map Repositories list on Map Index

MAP SCALE 1" = 500" 250 0 500 1000 H H H FEE1

PANEL 0752G

**FIRM** 

FLOOD INSURANCE RATE MAP EL PASO COUNTY.

COLORADO AND INCORPORATED AREAS

PANEL 752 OF 1300

(SEE MAP INDEX FOR FIRM PANEL LAYOUT CONTAINS:

COMMUNITY

MAP NUMBER 08041C0752G

MAP REVISED DECEMBER 7, 2018

Federal Emergency Management Agency



# A-1 CHIPSEAL

# (Area Runoff Coefficient Summary)

# **EXISTING CONDITIONS**

		STREE	TS / DEVE	LOPED	OVERLA!	ND / UNDEV	ELOPED	WEIGHTED		
BASIN	TOTAL AREA	AREA	$C_5$	$C_{100}$	AREA	C <sub>5</sub>	C <sub>100</sub>	$C_5$	C <sub>100</sub>	
	(Acres)	(Acres)			(Acres)					
EX-A	1.01	0.96	0.90	0.96	0.05	0.16	0.51	0.86	0.94	
EX-B	0.35	0.26	0.90	0.96	0.09	0.16	0.51	0.71	0.84	

Calculated by: JF

Date: 11/6/2020

Checked by: LD

# **DEVELOPED CONDITIONS**

		STREE	TS / DEVE	LOPED	<b>OVERLA</b> I	ND / UNDEV	WEIGHTED		
BASIN	TOTAL AREA	AREA	$C_5$	C <sub>100</sub>	AREA	C <sub>5</sub>	C <sub>100</sub>	C <sub>5</sub>	C <sub>100</sub>
	(Acres)	(Acres)			(Acres)				
EX-A	1.01	0.89	0.90	0.96	0.12	0.16	0.51	0.81	0.91
EX-B	0.35	0.26	0.90	0.96	0.09	0.16	0.51	0.71	0.84

Calculated by: JF
Date: 11/6/2022

Checked by: LD

# A-1 CHIPSEAL AREA DRAINAGE SUMMARY

# **EXISTING CONDITIONS**

	WEIGHTED			OVERLAND				STRE	STREET / CHANNEL FLOW				T <sub>t</sub> INTENSITY		TOTAL FLOWS	
BASIN	AREA TOTAL	$C_5$	C <sub>100</sub>	C <sub>5</sub>	Length	Slope	$T_{C}$	Length	Slope	Velocity	$T_t$	TOTAL	$I_5$	I <sub>100</sub>	$Q_5$	$Q_{100}$
	(Acres)	* For Calcs See	Runoff Summary		(ft)	(ft/ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
EX-A	1.01	0.86	0.94	0.86	100	0.03	3.0	300	3%	3.5	1.4	5.0	5.0	9.1	4.4	8.6
EX-B	0.35	0.71	0.84	0.71	30	0.03	2.7	150	3%	3.5	0.7	5.0	5.0	9.1	1.2	2.7

# **DEVELOPED CONDITIONS**

	WEIGHTED OVERLAND			STREET / CHANNEL FLOW				T <sub>t</sub> INTENSITY		TOTAL FLOWS						
BASIN	AREA TOTAL	C <sub>5</sub>	C <sub>100</sub>	$C_5$	Length	Slope	$T_{\rm C}$	Length	Slope	Velocity	$T_t$	TOTAL	$I_5$	I <sub>100</sub>	$Q_5$	Q <sub>100</sub>
	(Acres)	* For Calcs See	Runoff Summary		(ft)	(ft/ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
EX-A	1.01	0.81	0.91	0.81	100	0.03	3.6	300	3%	3.5	1.4	5.1	5.0	9.0	4.1	8.3
EX-B	0.35	0.71	0.84	0.71	30	0.03	2.7	150	3%	3.5	0.7	5.0	5.0	9.1	1.2	2.7

Calculated by: JF

Date: 11/6/2022 Checked by: LD

# A-1 CHIPSEAL PROPOSED SURFACE ROUTING SUMMARY

			Fle	ow
Design Point(s)	Contributing Basins	Area Ac	$Q_5$	Q 100
A	EX-A	1.01	4.1	8.3
В	EX-B	0.35	1.2	2.7

Calculated by: DL

Date: 4/4/2022

Checked by: LD

# A-1 CHIPSEAL (Area Runoff Coefficient Summary)

# **HISTORIC CONDITIONS**

		STREE	TS / DEVE	LOPED	OVERLA!	ND / UNDEV	ELOPED	WEIGHTED		
BASIN	TOTAL AREA	AREA	$C_5$	$C_{100}$	AREA	C <sub>5</sub>	C <sub>100</sub>	$C_5$	C <sub>100</sub>	
	(Acres)	(Acres)			(Acres)					
HS-A	1.01	0.58	0.90	0.96	0.43	0.16	0.51	0.58	0.77	
HS-B	0.35	0.26	0.90	0.96	0.09	0.16	0.51	0.71	0.84	

Calculated by: JF

Date: 10/6/2022

Checked by: LD

# A-1 CHIPSEAL AREA DRAINAGE SUMMARY

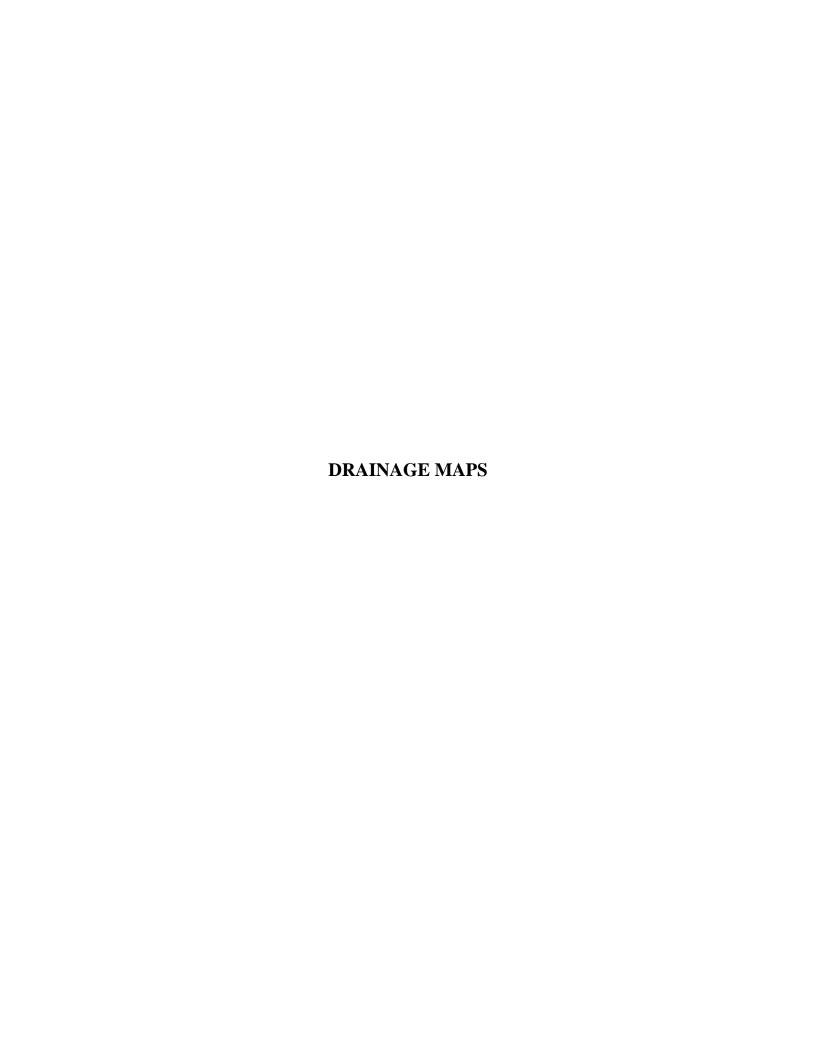
# **HISTORIC CONDITIONS**

		WEIGHTED OVERLAND			STRE	STREET / CHANNEL FLOW				T <sub>t</sub> INTENSITY		TOTAL FLOWS				
BASIN	AREA TOTAL	C <sub>5</sub>	C <sub>100</sub>	$C_5$	Length	Slope	$T_{C}$	Length	Slope	Velocity	$T_t$	TOTAL	$I_5$	I <sub>100</sub>	$Q_5$	$Q_{100}$
	(Acres)	* For Calcs See	Runoff Summary		(ft)	(ft/ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
HS-A	1.01	0.58	0.77	0.58	100	0.03	6.5	300	3%	3.5	1.4	7.9	4.4	7.8	2.6	6.0
HS-B	0.35	0.71	0.84	0.71	30	0.03	2.7	150	3%	3.5	0.7	5.0	5.0	9.1	1.2	2.7

Calculated by: JF

Date: 10/6/2022 Checked by: LD

			Desig	gn Procedu	re Form:	Runoff Rec	duction					
				UD-BMP (Ve	ersion 3.07, Ma	rch 2018)						Sheet 1 of 1
Designer:	Dane Frank Terra Nova E	naineerina									-	
Company: Date:	April 14, 2023										-	
Project:	A-1 Chipseal										-	
Location:		ew, Colorado S	prings								-	
200410111		,	F9-								•	
SITE INFORMATION (U	ser Input in B	lue Cells)										
(0.		Rainfall Depth	0.60	inches								
Depth of Average Ru	noff Producin	g Storm, d <sub>6</sub> =	0.43	inches (for V	Vatersheds C	outside of the l	Denver Regio	on, Figure 3-1	in USDCM V	ol. 3)		
Area Type	UIA:RPA											
Area ID												
Downstream Design Point ID												
Downstream BMP Type												
DCIA (ft²)												<del>                                     </del>
UIA (ft²) RPA (ft²												<del>                                     </del>
SPA (ft <sup>2</sup>												
HSG A (%)												† †
HSG B (%)												
HSG C/D (%)	0%											
Average Slope of RPA (ft/ft)												L
UIA:RPA Interface Width (ft)	100.00											
CALCULATED RUNOFF	RESULTS											
Area ID	###											
UIA:RPA Area (ft <sup>2</sup> )												
L/W Ratio												L
UIA / Area												<b>-</b>
Runoff (in) Runoff (ft <sup>3</sup>												<del>                                     </del>
Runoff Reduction (ft <sup>3</sup>												
				1	1				1	ı		
CALCULATED WQCV R												
Area ID												
WQCV (ft <sup>3</sup>												<b>.</b>
WQCV Reduction (ft <sup>3</sup> ) WQCV Reduction (%)												<b></b>
Untreated WQCV (ft <sup>3</sup> )												<del>                                     </del>
Character W QOV (it	, <u> </u>	L L		I.	l	1		l.		l		11
CALCULATED DESIGN	POINT RESU	JLTS (sums re	sults from	all columns v	with the sam	e Downstrea	ım Design Po	oint ID)				
Downstream Design Point ID												
DCIA (ft <sup>2</sup> )												
UIA (ft²												
RPA (ft²)												<del>                                     </del>
SPA (ft <sup>2</sup> ) Total Area (ft <sup>2</sup> )												<del>                                     </del>
Total Impervious Area (ft <sup>2</sup> )	1											<del>                                     </del>
WQCV (ft <sup>3</sup>												
WQCV Reduction (ft <sup>3</sup>												
WQCV Reduction (%)												
Untreated WQCV (ft <sup>3</sup> )	0											
CALCULATED SITE RE	SIII TS (eume	e reculte from	all column	e in workeho	ot)							
Total Area (ft <sup>2</sup>		S results iroin	an column	S III WOLKSHE	et)							
Total Impervious Area (ft <sup>2</sup> )		1										
WQCV (ft <sup>3</sup>												
WQCV Reduction (ft <sup>3</sup>	552											
WQCV Reduction (%)		4										
Untreated WQCV (ft <sup>3</sup> )	0	]										
I												



# **BASIN SUMMARY** POINT EX-A0.35 1.2 2.7

# A-1 CHIPSEAL COLORADO SPRINGS EXISTING DRAINAGE MAP JULY 2022

EAST FORK SAND CREEK

EX ASPHALT SURFACE

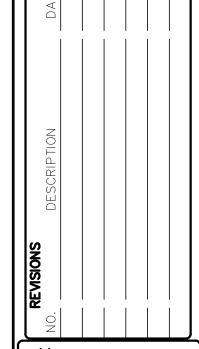
(ONE LIFT)

—EQUIPMENT

STORAGE

0.38 AC RECENTLY ADDED ASPHALT

NO BUILDINGS NO POSTED ADDRESS



UNDERGROUND STORM SEWER LOCATION UNKNOWN. PIPE MAY NOT BE CONTAINED IN A UTILITY EASEMENT

EX SEWER MANHOLE

PARCEL B

LOT 39

ESIGNED BY DLF HECKED BY LD

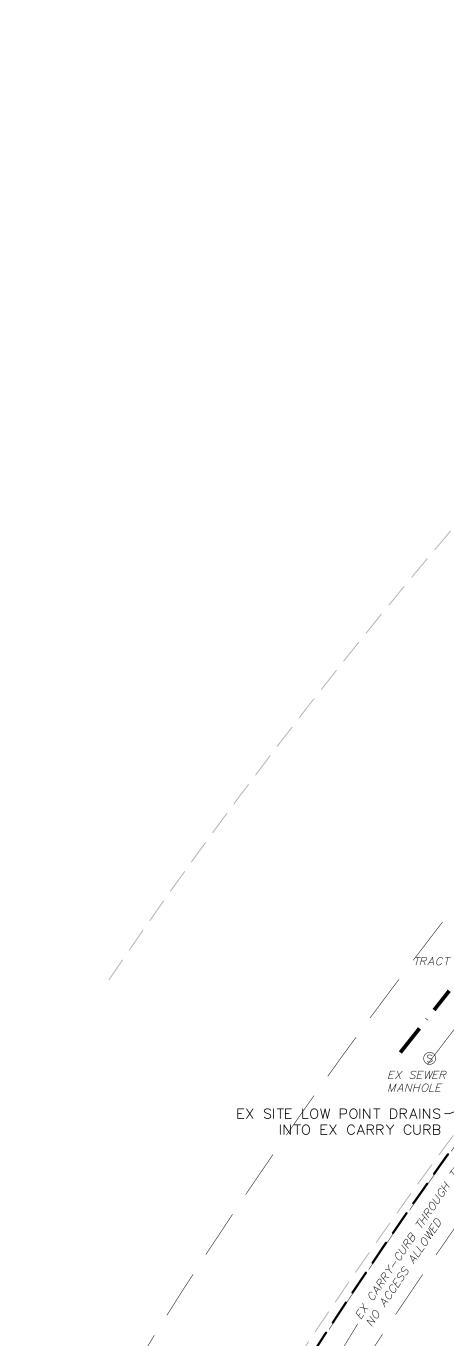
-SCALE N/A OB NO. 2173.00 ATE ISSUED 11/04/2 HEET NO. 1 OF 2

<u>LEGEND</u> BASIN DESIGNATIONAREA IN BASIN (AC)PERCENT IMPERVIOUS DESIGN POINT GROUND SURFACE FLOW DIRECTION ROAD AND DITCH FLOW DIRECTION → → TIME OF CONCENTRATION PATH

# 1. ALL FEATURE SHOWN ARE EXISTING.

<u>NOTES</u>

SCALE: 1"=20'



# BASIN SUMMARY

DESIGN		AREA	FLOW					
POINT	BASIN	(ACRES)	5 YR (cfs)	100 YR (cfs)				
			(015)	(015)				
Α	EX-A	1.01	4.1	8.3				
R	FX-B	0.35	1 2	2.7				

# A-1 CHIPSEAL COLORADO SPRINGS PROPOSED DRAINAGE MAP JULY 2022

EAST FORK SAND CREEK

EX SEWER MANHOLE

PR A(PER), 3,300 SF -/

LOT 35

PR 3(IMP), 13,250 SF

WATER QUALITY TREATMENT AREA,
TREATMENT FROM RUNOFF FLOW
OVER PROPOSED TURF GRASS,
BASED ON RUNOFF REDUCTION
SPREADSHEET VALUES

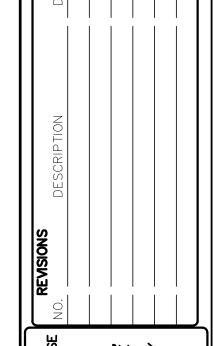
EX SITE LOW POINT DRAINS -INTO EX CARRY CURB

EX ASPHALT SURFACE (ONE LIFT) -EQUIPMENT

0.38 AC RECENTLY ADDED ASPHALT

PARCELA

NO POSTED ADDRES



UNDERGROUND STORM SEWER LOCATION UNKNOWN. PIPE MAY NOT BE CONTAINED IN A UTILITY EASEMENT

EX SEWER

PARCEL B

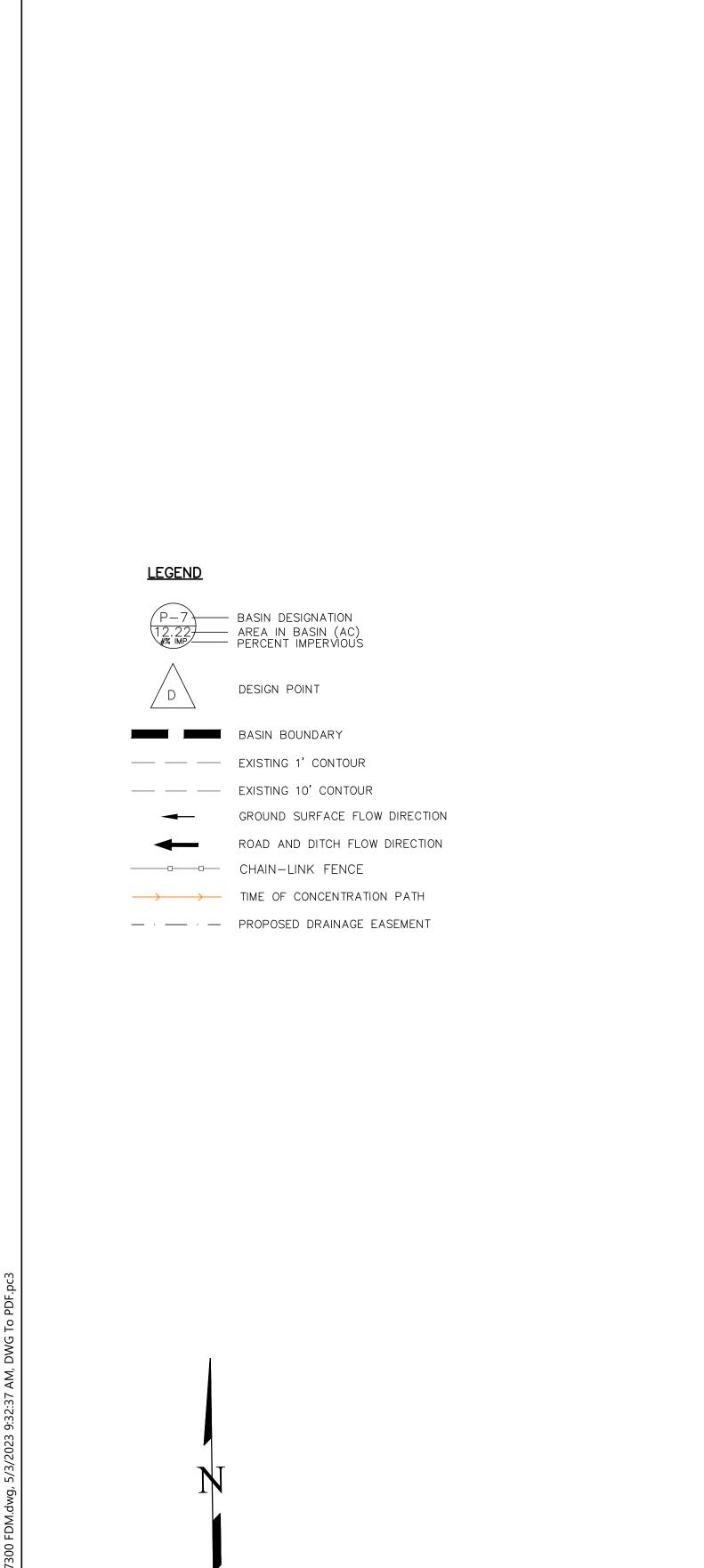
LOT 37

INDUSTRIAL BUILDING 7,575±s.f. FOOTPRINT

ESIGNED BY DLF RAWN BY DLF HECKED BY LD

-SCALE AS SHOWN -SCALE N/A OB NO. 2173.00

ATE ISSUED 11/04/2 HEET NO. 2 OF 2



SCALE: 1"=20'