

FINAL DRAINAGE REPORT FOR A-1 CHIPSEAL LOT 36 AND 37 CLAREMONT BUSINESS PARK FIL NO 2 7245 COLE VIEW COLORADO SPRINGS, COLORADO

MAY 2022

Prepared For:
A-1 CHIPSEAL
7245 Cole View
Colorado Springs, CO 80915
720.540.8264
Contact: Stephanie Wallis

Prepared By:

TERRA NOVA ENGINEERING, INC.

721 S. 23RD Street
Colorado Springs, CO 80904
719.635.6422
Contact: Dane Frank

TNE Job No. 2173.00 County Job No. ####



FINAL DRAINAGE REPORT FOR

A-1 CHIPSEAL

LOT 36 AND 37 CLAREMONT BUSINESS PARK FIL NO 2 7245 COLE VIEW COLORADO SPRINGS, COLORADO

TABLE OF CONTENTS

Engineer's Statement	Page 3
Purpose	Page 4
General Description	Page 4
Existing Drainage Conditions	Page 5
Proposed Drainage Conditions	Page 5
Hydrologic Calculations	Page 6
Hydraulic Calculations	Page 6
Floodplain Statement	Page 7
Water Quality	Page 7
Construction Cost Opinion	Page 7
Drainage Fees	Page 7
Maintenance	Page 7
Summary	Page 7
Bibliography	Page 8

APPENDICIES

VICINITY MAP
GENERAL LOCATION MAP
NRCS SOILS MAP
FEMA FIRM MAP
HYDROLOGIC CALCULATIONS
HYDRAULIC CALCULATIONS
DETENTION BASIN DESIGN CALCULATIONS
DRAINAGE MAPS

FINAL DRAINAGE REPORT FOR

A-1 CHIPSEAL LOT 36 AND 37 CLAREMONT BUSINESS PARK FIL NO 2 7245 COLE VIEW

COLORADO SPRINGS, COLORADO

DESIGN ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Dane Frank, P.E. 50207	Date
On behalf of Terra Nova Engineering, Inc.	
OWNER/DEVELOPER'S STATEMENT	` :
I, the owner/developer have read and will comply with a report and plan.	all of the requirements specified in this drainage
And wind Ginneton	Dete
Authorized Signature	Date
Printed Name, Title	
Business Name	
Address	
EL PASO COUNTY:	
Filed in accordance with the requirements of the Drain County Engineering Criteria Manual and Land Develope	
Jennifer Irvine, P.E.	Date
County Engineer / ECM Administrator	
Conditions: Please change to: Josh Palmer, P.E., Interim County Engineer	

FINAL DRAINAGE REPORT FOR

A-1 CHIPSEAL

LOT 36 AND 37 CLAREMONT BUSINESS PARK FIL NO 2 7245 COLE VIEW

COLORADO SPRINGS, COLORADO

PURPOSE

The purpose of this Final Drainage Report is to identify and analyze the proposed drainage patterns, determine proposed runoff quantities, size drainage structures for conveyance of developed runoff, and present solutions to drainage impacts on-site and off-site resulting from this development. The site has previously been platted and has previously been studied in:

"Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006,

prepared by Matrix Design Group.

include 7231 Cole View; both parcels combined approx. equal 1.36 acres.

GENERAL DESCRIPTION

This Final Drainage Report (FDR) is an analysis of approximately 1.36 acres of developed land located at 7245 Cole View. This site is currently in use as a paving business. The site is in the northeast quarter of Section 8, Township 14 South, Range 65 West of the 6th Principal Meridian within El Paso County. The parcels are bounded to the north and west by Sand Creek, to the southwest by LOT 35 CLAREMONT BUSINESS PARK FIL NO 2, to the southeast by LOTS 13 AND 14 CLAREMONT BUSINESS PARK FIL NO 2, and to the north by LOT 39 CLAREMONT BUSINESS PARK FIL NO 2. (see vicinity map).

The site lies within the Sand Creek Basin, with storm runoff surface draining west across the site, then into a gutter that flows off the site to the south, eventually entering a storm inlet, which drains into the adjacent East Fork Sand Creek. There are also storm inlets in Marksheffel Road that flow into a storm sewer that flows south along Markcheffel to an unknown outfall, which presumably also drains into Sand Creek.

Please discuss why the final drainage report was required.

Please include a discussion comparing the subdivision's approved final drainage report's amount of imperviousness for the lots versus the proposed imperviousness.

Soils for this project are delineated by the map in the appendix as Ellicott loamy coarse sand, 0 to 5 percent slopes (28). Soils in the study area are shown as mapped by NRCS in the "Soils Survey of El Paso County Area" and contains soils of Hydrologic Group D.

Soils map states Hydrologic Group A; please revise for consistency.

The site is developed with mostly pavement and roof surfaces, and a small amount of landscaping.

The site drains to the west, with an average slope of 3.7%.

Please add a Historical Conditions section. The section should describe the site's conditions before any work was completed.

EXISTING DRAINAGE CONDITIONS

The site is already developed with one building and outdoor parking and equipment storage. There are two drainage basins. See attached Existing Drainage Map (in appendix).

Basin EX-A is 1.01 acres that is mostly roof and parking area and drains to Design Point A and leaves the site in an existing carry curb. Basin EX-A has flows of $Q_5 = 4.4$ cfs and $Q_{100} = 8.6$ cfs.

Basin EX-B is 0.35 acres that is mostly landscaping and half a street and drains to Design Point B at the south corner of the site and flows offsite in the street gutter. Basin EX-B has flows of $Q_5 = 1.2$ cfs and $Q_{100} = 2.7$ cfs.

PROPOSED DRAINAGE CONDITIONS

The proposed drainage conditions are the same as the existing drainage conditions, with the addition of a full infiltration water quality sand filter in the west corner of the site. The County is requiring the addition of a water quality structure retroactively following paving of 0.38 acres on the south side of the site.

At the west corner of the site a full infiltration water quality sand filter will treat flow from Basin EX-A (Q_5 =4.4 cfs and Q_{100} =8.6 cfs). Runoff entering the sand filter will flow in from gutters on two sides, and sheet flow in from the asphalt area. Runoff entering the sand filter will flow down a riprap rundown to the filter sand. After flowing through the filter sand, the runoff infiltrates into the ground. Any flow above the WQCV will enter the sand filter and flow out the existing curb chase in the corner of the sand filter / site if the water ponds high enough. The 1.01 acres tributary to the sand filter are 95% impervious. Based upon this we need a WQCV of 0.013 ac-ft. No

Please include the total height of the sand filter walls. A building permit is required for retaining walls over 4 feet from the Pikes Peak Regional Building Department.

detention volume is included in the sand filter. The top of the filter sand is at an elevation of 6333.6 feet and the top of the WQCV is at 6334.52 feet.

In an effort to protect receiving water and as part of the "four-step process to minimize adverse impacts of urbanization" this site was analyzed in the following manner:

- 1. Reduce Runoff- The only development included in this FDR is the addition of a water quality structure. There is no runoff reduction associated with the installation of a water quality structure.
- 2. Stabilize Drainageways- There are no existing or proposed drainageways onsite. The adjacent East Fork Sand Creek has previously been stabilized and runoff from the site currently flows to a storm sewer system that discharges into East Fork Sand Creek.
- 3. Provide Water Quality Capture Volume (WQCV)- The proposed sand filter has been sized and designed to sufficiently capture the required WQCV and infiltrate the entire volume, thereby allowing solids and contaminants to settle out.
- 4. Consider Need for Industrial and Commercial BMPs- A water qualitructure doesn't require any Industrial and Commercial BMPs. As the site is currently used for a paving business, there are likely existing industrial BMPs in place at the site.

I dont believe there are. No Industrial or Commercial BMPs are required for the Site.

HYDROLOGIC CALCULATIONS

Hydrologic calculations were performed using the El Paso County Storm Drainage Design Criteria Manual - Volumes 1 & 2, latest editions. The Rational Method was used to estimate storm water runoff anticipated from design storms with 5-year and 100-year recurrence intervals. The Urban Drainage Criteria Manual was used to calculate the detention and water quality volume.

HYDRAULIC CALCULATIONS

Please add this manual to the bibliography.

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County Storm Drainage Design Criteria Manual – Volumes 1 & 2, latest editions. The pertinent data sheets are included in the appendix of this report.

FLOODPLAIN STATEMENT

No portion of this site is within a designated F.E.M.A. floodplain, as determined by Flood Insurance Rate Map No. 08041C0752 G, dated December 7, 2018 (see appendix).

WATER QUALITY

The proposed full infiltration water quality sand filter provides water quality treatment for all of the recently added 0.38 acres of asphalt, as well as most of the remainder of the site.

There is no water quality treatment for existing basin EX-B. This basin is already fully developed

and no changes to it are proposed.

CONSTRUCTION COST OPINION

Public Reimbursable

None

For sites where full infiltration for WQ is proposed, an on-site infiltration test using double-ring infiltrometer is required (or approved equal). Infiltration tests should be performed or supervised by a licensed professional engineer and conducted at a minimum depth equal to the bottom of the sand filter. Underdrains are required for sand filters and should be provided if infiltration tests show rates slower than 2 times that required to drain the WQCV over 12 hours.

Public Non-Reimbursable

None

Private Non-Reimbursable

1. Sand Filter 1 EA \$ 20,000 <u>\$ 20,000</u> Total \$ 20,000

DRAINAGE FEES

This drainage report is part of a site development application; therefore, no drainage fees are due.

MAINTENANCE

The sand filter is private and will be maintained by the property owner.

SUMMARY

Development of this site will not adversely affect the surrounding development. This report is in general conformance with the previous reports which included this site. Site runoff and storm

drain appurtenances from the A-1 Chipseal development will not adversely affect the downstream and surrounding developments and will be safely routed to the proposed sand filter to slowly treat the water quality capture volume. Runoff leaving the site is routed to the existing public storm sewer system.

PREPARED BY:

TERRA NOVA ENGINEERING, INC.

Dane Frank, P.E. Project Engineer

Jobs/2173.00/drainage/217300 FDR.doc

BIBLIOGRAPHY

El Paso County Drainage Criteria Manual-Volumes 1 & 2, latest edition

El Paso County Board Resolution No 15-042 (Adoption of Chapter 6 and Section 3.2.1 Chapter 13 of the City of Colorado Springs Drainage Criteria Manual dated May 2014, Hydrology and Full Spectrum Detention)

"Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, prepared by Matrix Design Group.



El Paso County - Community: Property Search

Schedule Number: 5408102040

A-1 Chipseal - Vicinity Map

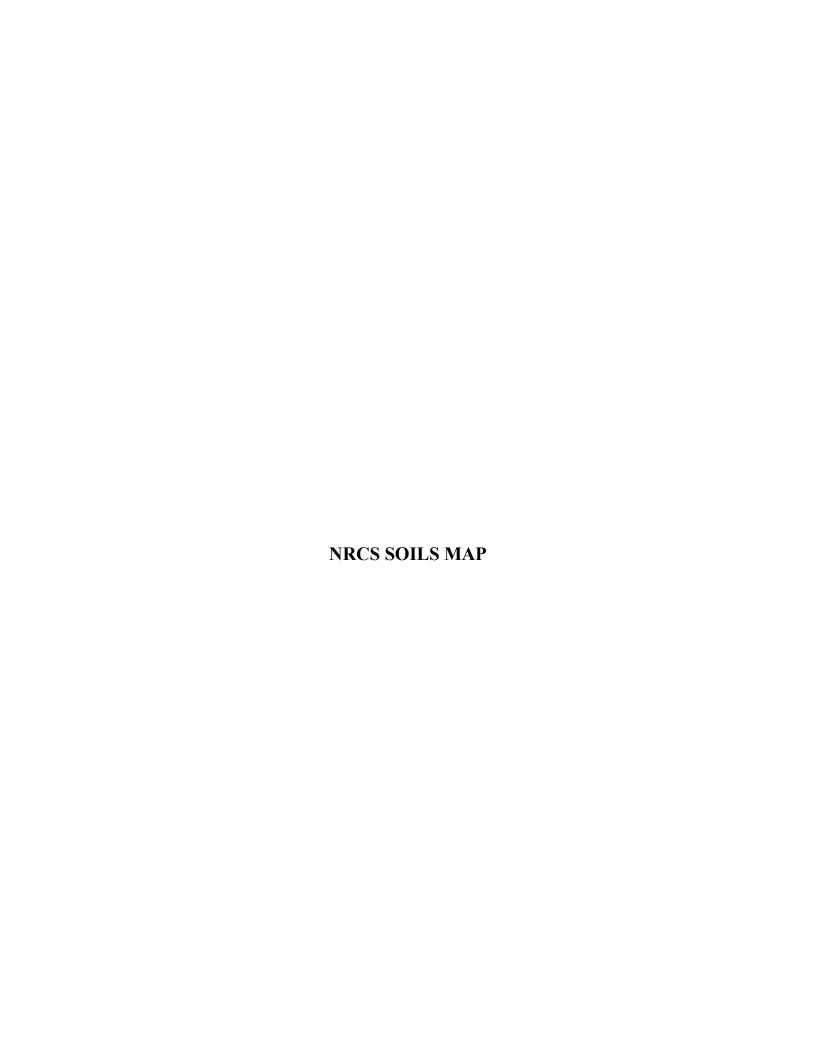


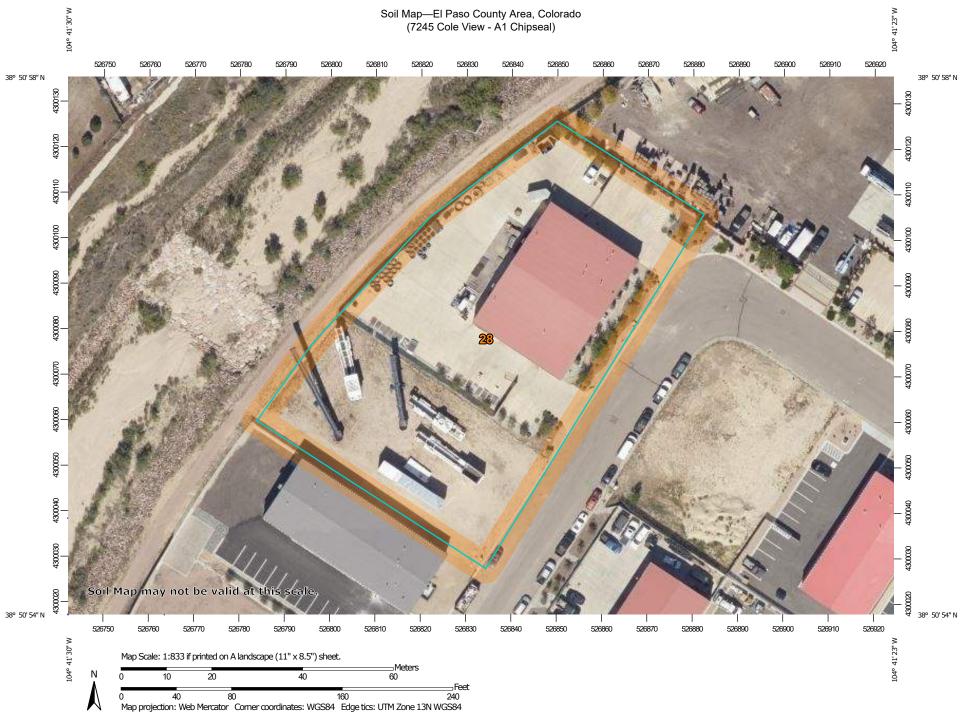
North is up ^

1 of 2 4/5/2022, 9:02 AM









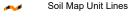
MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Points

Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Candfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

→ Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

00

Spoil Area

Stony Spot

Very Stony Spot

Wet Spot

△ Other

Special Line Features

Water Features

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 19, Aug 31, 2021

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Aug 19, 2018—Sep 23, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI		
28	Ellicott loamy coarse sand, 0 to 5 percent slopes	1.2	100.0%		
Totals for Area of Interest		1.2	100.0%		

El Paso County Area, Colorado

28—Ellicott loamy coarse sand, 0 to 5 percent slopes

Map Unit Setting

National map unit symbol: 3680 Elevation: 5,500 to 6,500 feet

Mean annual precipitation: 13 to 15 inches
Mean annual air temperature: 47 to 50 degrees F

Frost-free period: 125 to 145 days

Farmland classification: Not prime farmland

Map Unit Composition

Ellicott and similar soils: 97 percent Minor components: 3 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Ellicott

Setting

Landform: Flood plains, stream terraces Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy alluvium

Typical profile

A - 0 to 4 inches: loamy coarse sand

C - 4 to 60 inches: stratified coarse sand to sandy loam

Properties and qualities

Slope: 0 to 5 percent

Depth to restrictive feature: More than 80 inches Drainage class: Somewhat excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High to

very high (5.95 to 19.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: FrequentNone

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7w

Hydrologic Soil Group: A

Ecological site: R069XY031CO - Sandy Bottomland LRU's A and

В

Other vegetative classification: SANDY BOTTOMLAND

(069AY031CO) Hydric soil rating: No

Discussion states soil group D please revise for consistency.

Conservation Service

Minor Components

Fluvaquentic haplaquoll

Percent of map unit: 1 percent Landform: Swales

Hydric soil rating: Yes

Other soils

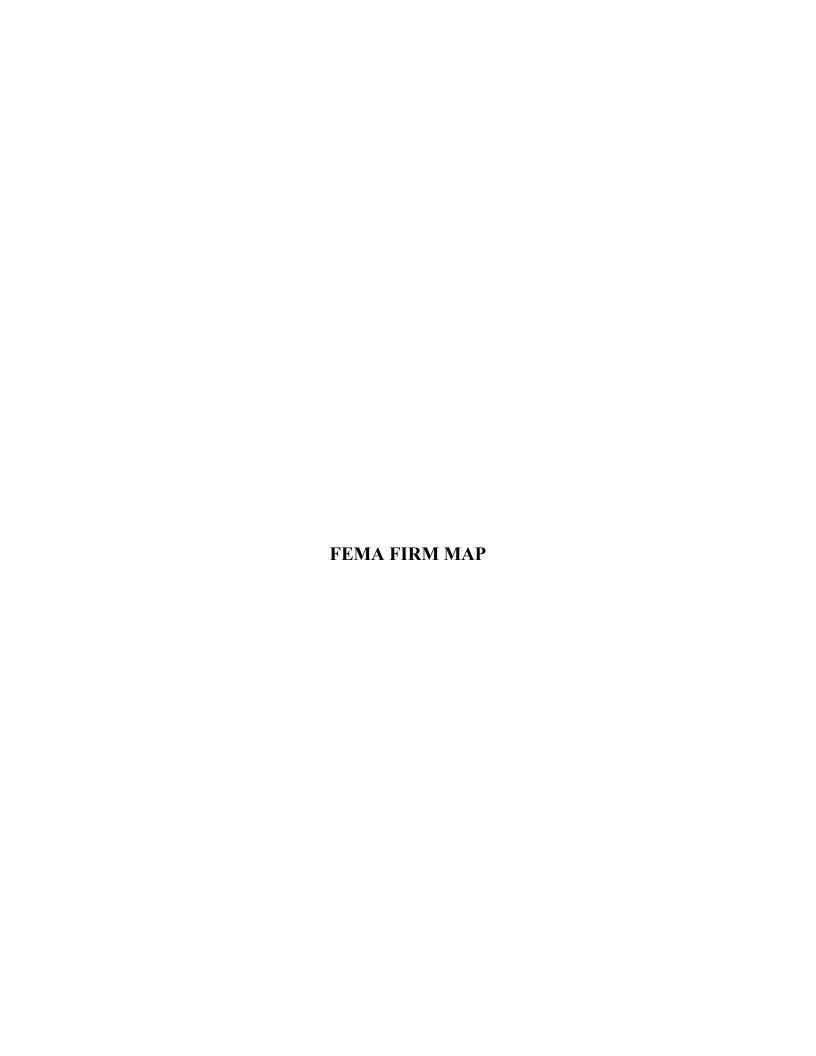
Percent of map unit: 1 percent Hydric soil rating: No

Pleasant

Percent of map unit: 1 percent Landform: Depressions Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 19, Aug 31, 2021



NOTES TO USERS

is map is for use in administering the National Flood Insurance Program. It does necessarily identify all areas subject to flooding, particularly from local drainage urces of small size. The community map repository should be consulted fo sable updated or additional flood hazard information.

To obtain more detabled information in sease where Baser Flood Elevations (EETs) another Gouleage likes been definement, users as mooranged to consult the Though profession and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Food Insurance Study (FIS) proof that accompanies the FIRM. Users alroad to assess that SFEs shown on the FIRM represent rounded whole-food and the FIRM state of the FIRM represent study of the should not be used as the side source of food developin information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purpose of construction and for floodighal management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0° North American Vertical Datum of 1986 (NAVD68). Users of this FIRM should be ware that coastal flood elevations are also provided in the Summary of Sillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Sillwater Elevations table should be used for construction ind/or floodplain management purposes when they are higher than the elevatio hown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway width and other pertinent floodway data are provided in the Flood insurance Study repor

Certain areas not in Special Flood Hazard Areas may be protected by **flood contro** structures. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAJ03, GRS90 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight postboral differences in map features across jurisdiction boundaries. These differences on not differences in map features acro-affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD89). These food elevations must be compared to structure and conversion between the National Geodesic Vertical Datum of 1923 and the North American Vertical Datum of 1988, visit the National Geodesic Curvey websites with publications of 1989 and 1989 are the National Geodesic Survey websites with publications again or contact the National Geodesic Survey are better the National Geodesic Survey and the Datum of 1989 and 1989 are the National Geodesic Survey and the National Geodesic Survey are the National Geodesic Survey and the National Geodesic Survey and the National Geodesic Survey are the National Geodesic Survey and the National Geodesic Survey are the National Geodesic Survey and the National Geodesic Survey are the National Geodesic Survey and National Survey Nationa

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench mark shown on this map, please contact the Information Services Branch of the Nation Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Cotorado Springs Utilities, City of Fountain, Bureau of Land Management, National Cocania and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map effects more detailed and up-to-date stream channel configurations and the map reference of the date of t and may appear outside of the floodplain.

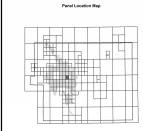
Corporate limits shown on this map are based on the best data available at the of publication. Because changes due to annexations or de-annexations may be occurred after this map was published, map users should contact appropriate ommunity officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood insurance Program dates on each community as well as a listing of the panels on which each community is

Contact ERAN May Sourice Center (INSC) is the FEMA May Information Oxforing, IRSM), 1-8477-38-2677 for information on waisable reviews assessment with FIRM. Available products may include newboardy issued Letter of May Change (FIRM) and the products may include newboardy issued Letter of May Change (Inschool Insurance Study Report, and/or digital versions of his map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.forma.gov/.

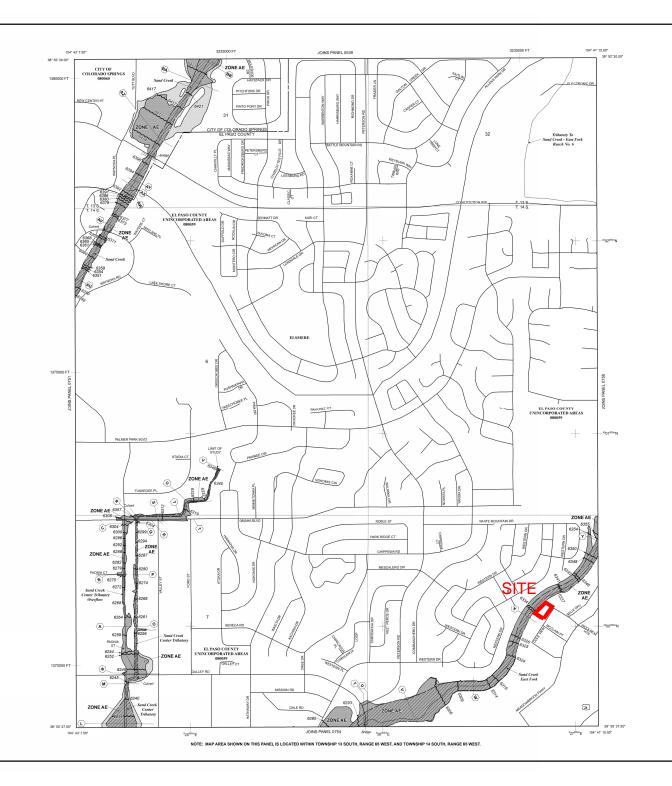
If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2827) or visit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table Flooding Source REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).







SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (1,00-year flood), also known as the base flood, is the flood that has a 1% chance of being equated or exceeded in any given year. The Special Flood Heard Ares is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Heard include zones A, AE, AM, AD, AR, A99, V, and VE. The Base Flood Delevation is the water-surface elevation of the 1% annual chance flood.

ZONE A No Base Flood Blevations determined.

ZONE AE Base Flood Blevations determined.

ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Blevations determined.

Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.

Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.

ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood Bevations determined. Coastal flood zone with velocity hazard (wave action); Base Flood Bevations determined

FLOODWAY AREAS IN ZONE AE

OTHER FLOOD AREAS

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas Floodplain boundary

____ Floodway boundary Zone D Boundary

..... CBRS and OPA boundary

-Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.

~~ 513 ~~ Base Flood Flevation line and value: elevation in feet*

Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

 $\begin{tabular}{c} \hline {\bf A} \\ \hline \end{tabular} \begin{tabular}{c} \hline {\bf A} \\ \hline \end{tabular} \begin{tabular}{c} \hline {\bf Cross section line} \\ \hline \end{tabular}$

23-----23

Geographic coordinates referenced to the North American Datum of 1983 (NAD 83) 97" 07" 30.00" 32" 22" 30.00"

4275000nN 1000-meter Universal Transverse Mercator grid ticks, zone 13

Bench mark (see explanation in Notes to Users section of this FIRM panel) DX5510

M1.5

NATIONAL F

MAP REPOSITORIES Refer to Map Repositories list on Map Index

MAP SCALE 1" = 500" 250 0 500 1000 HHH FEE1

PANEL 0752G

FIRM

FLOOD INSURANCE RATE MAP

EL PASO COUNTY. COLORADO AND INCORPORATED AREAS

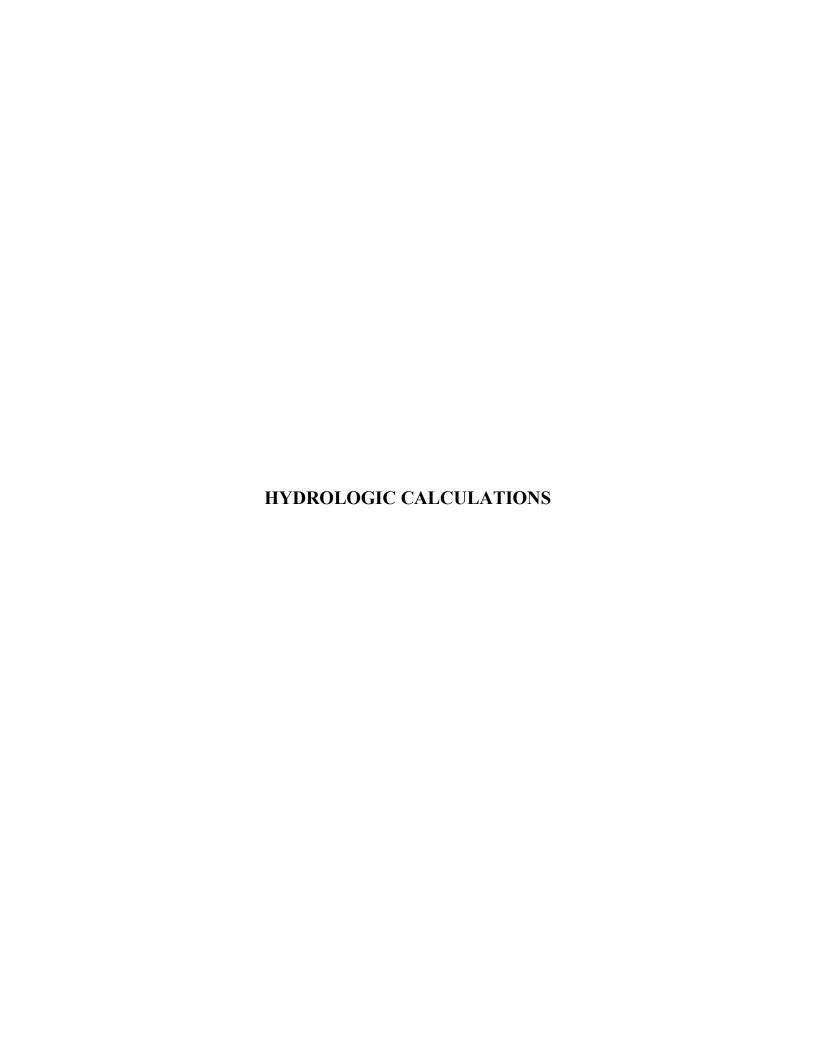
PANEL 752 OF 1300 (SEE MAP INDEX FOR FIRM PANEL LAYOUT

CONTAINS: COMMUNITY

MAP NUMBER 08041C0752G

MAP REVISED DECEMBER 7, 2018

Federal Emergency Management Agency



A-1 CHIPSEAL

(Area Runoff Coefficient Summary)

EXISTING CONDITIONS

		STREE	TS / DEVE	LOPED	OVERLA!	ND / UNDEV	ELOPED	WEIGHTED		
BASIN	TOTAL AREA	AREA	C_5	C ₁₀₀	AREA	C_5	C ₁₀₀	C_5	C ₁₀₀	
	(Acres)	(Acres)			(Acres)					
EX-A	1.01	0.96	0.90	0.96	0.05	0.16	0.51	0.86	0.94	
EX-B	0.35	0.26	0.90	0.96	0.09	0.16	0.51	0.71	0.84	

Calculated by: DLF

Date: 8/5/2020

Checked by: LD

DEVELOPED CONDITIONS

		STREE	TS / DEVE	LOPED	OVERLA!	ND / UNDEV	ELOPED	WEIGHTED		
BASIN	TOTAL AREA	AREA	C ₅	C ₁₀₀	AREA	C_5	C ₁₀₀	\mathbf{C}_{5}	C ₁₀₀	
	(Acres)	(Acres)			(Acres)					
EX-A	1.01	0.96	0.90	0.96	0.05	0.16	0.51	0.86	0.94	
EX-B	0.35	0.26	0.90	0.96	0.09	0.16	0.51	0.71	0.84	

Calculated by: DLF Date: 4/4/2022

Checked by:

A-1 CHIPSEAL AREA DRAINAGE SUMMARY

EXISTING CONDITIONS

	WEIGHTED OVERLAND				STRE	STREET / CHANNEL FLOW			T_t	INTEN	VSITY	TOTA	L FLOWS			
BASIN	AREA TOTAL	C ₅	C ₁₀₀	C ₅	Length	Slope	T_{C}	Length	Slope	Velocity	T_t	TOTAL	I_5	I ₁₀₀	Q_5	Q_{100}
	(Acres)	* For Calcs See	Runoff Summary		(ft)	(ft/ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
EX-A	1.01	0.86	0.94	0.86	100	0.03	3.0	300	3%	3.5	1.4	5.0	5.0	9.1	4.4	8.6
EX-B	0.35	0.71	0.84	0.71	30	0.03	2.7	150	3%	3.5	0.7	5.0	5.0	9.1	1.2	2.7

DEVELOPED CONDITIONS

		WEIGH	HTED		OVER	LAND		STRE	ET / CH	ANNEL F	LOW	T_t	INTEN	VSITY	TOTA	L FLOWS
BASIN	AREA TOTAL	C ₅	C ₁₀₀	C ₅	Length	Slope	T_{C}	Length	Slope	Velocity	T_t	TOTAL	I_5	I ₁₀₀	Q_5	Q ₁₀₀
	(Acres)	* For Calcs See	Runoff Summary		(ft)	(ft/ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
EX-A	1.01	0.86	0.94	0.86	100	0.03	3.0	300	3%	3.5	1.4	5.0	5.0	9.1	4.4	8.6
EX-B	0.35	0.71	0.84	0.71	30	0.03	2.7	150	3%	3.5	0.7	5.0	5.0	9.1	1.2	2.7

Calculated by: DLF

Date: 4/4/2022 Checked by: LD

A-1 CHIPSEAL PROPOSED SURFACE ROUTING SUMMARY

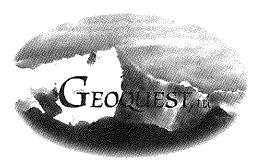
			Flow			
Design Point(s)	Contributing Basins	Area Ac	Q_5	Q 100		
A	EX-A	1.01	4.4	8.6		
В	EX-B	0.35	1.2	2.7		

Calculated by: DL

Date: 4/4/2022

Checked by: LD





6825 Silver Ponds Heights #101 Colorado Springs, CO 80908 (719) 481-4560

PERCOLATION TEST

FOR

HAMMERS CONSTRUCTION

This percolation test is for Lot 11. Lots 36 and 37 are discussed in this report. Please revise or remove report. Additionally, discuss in the narrative why the percolation test is included. JOB #16-0787

Lot 11, Filing 2,
Claremont Business Park Subdivision,
7176 Cole View,
El Paso County,
Colorado

Respectfully submitted,

Charles E. Milligan, P.E.

Civil Engineer



PERCOLATION TEST FINDINGS

Enclosed are the results of the percolation test for the retention pond to be installed at Lot 11, Filing 2, Claremont Business Park Subdivision, 7176 Cole View, El Paso County, Colorado. The locations of the percolation test borings were determined by Hammers Construction. The commercial structure will not be on a public water system. Due to the natural slope of the property, the entire system will feed to the west to northwest at 2% approximately 50 feet. All applicable regulations of the El Paso County Health Department ISDS Regulations must be complied with for the installation of the disposal system.

The percolation test was performed on October 6, 2016, in accordance with **E.P.C.P.H. OWS Regulations.** The field data and results of the percolation test are as follows:

PERC.	PERC HOLE #1	PERC HOLE #2	PERC HOLE #3
TEST	@ 34" DEPTH	@ 34" DEPTH	@ 34" DEPTH
0 TIME	DROP (IN	DROP (IN	DROP (IN
	INCHES)	INCHES)	INCHES)
12:24	1-3/4	4-1/4	2
12:34	3/4	1-7/8	5/8
12:44	3/4	1	5/8
12:54	11/16	13/16	9/16
1:04	5/8	5/8	9/16
1:14	5/8	5/8	1/2
7.7.7			
Rate/Hole	16.0	16.0	20.0

The average of the test holes is 17.3 minutes per inch.

Blow counts at the depth of 3 feet was 31/12.

The soil profile for the disposal system is as follows:

- 0 to 6" Sand- fine to coarse grain, high density, low moisture
 content, low cohesion, low plasticity, brown in color.
- 6" to 8' Sand- fine to coarse grain, moderate density, moderate moisture content, low clay content, low cohesion, low plasticity, brown in color.

No water was encountered during the drilling of all holes. Bedrock was not encountered during the drilling of the test borings. No known wells were observed within 100 feet of the proposed system. All setbacks shall conform to county regulations.

If during construction of the field itself, subsurface conditions change considerably or if the location of the proposed field changes, this office shall be notified to determine whether the conditions are adequate for the system as designed or whether a new system needs to be designed.

Weather conditions at the time of the test consisted of partly cloudy skies with cold temperatures.



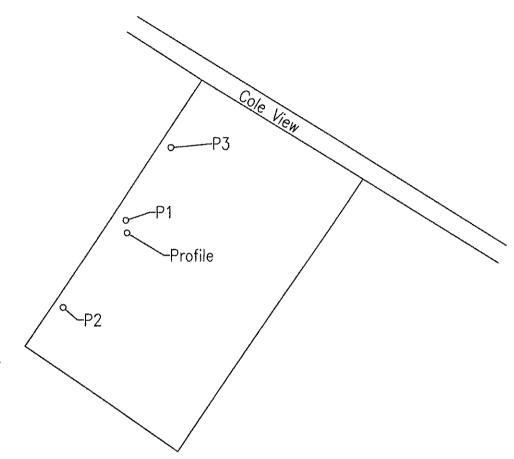
DRILL LOGS

JOB #: 16-0787 TEST BORING NO.: TH-1 DATE: 10/6/2016	DEPTH (in ft.)	SYMBOL	SAMPLES	BLOW COUNT	WATER %	SOIL TYPE	DEPTH (in ft.) SYMBOL SYMBOL SAMPLES BLOW COUNT WATER % SOIL TYPE
O"-6' Sand (SM) Fine-coarse grained Moderate-high density Moderate moisture content Low-moderate clay content Low plasticity Light Brown color 6'- 8' Sand (SC) Fine-coarse grained Moderate-high density Moderate-high moisture content Low-moderate clay content Low-moderate plasticity	1 2 3 4 5 6			<u>31</u> 12"	7.0	SM	
Greyisih Brown color	7 8 9 10			<u>Bag</u> 2"	11.2	sc	9

GEOQUESI LLC

SITE MAP

Lot 11, Filing 2
Claremont Business Park
7176 Cole View
El Paso County,
Colorado
Job #16-0787



Location from Southwest Lot Corner to Profile:

N. 41' E. - 78'

Location from Profile to:

P1: S. 78' E. - 3'

P2: S. 32° W. - 54'

P3: N. 35' E. - 45'

GPS coordinates:

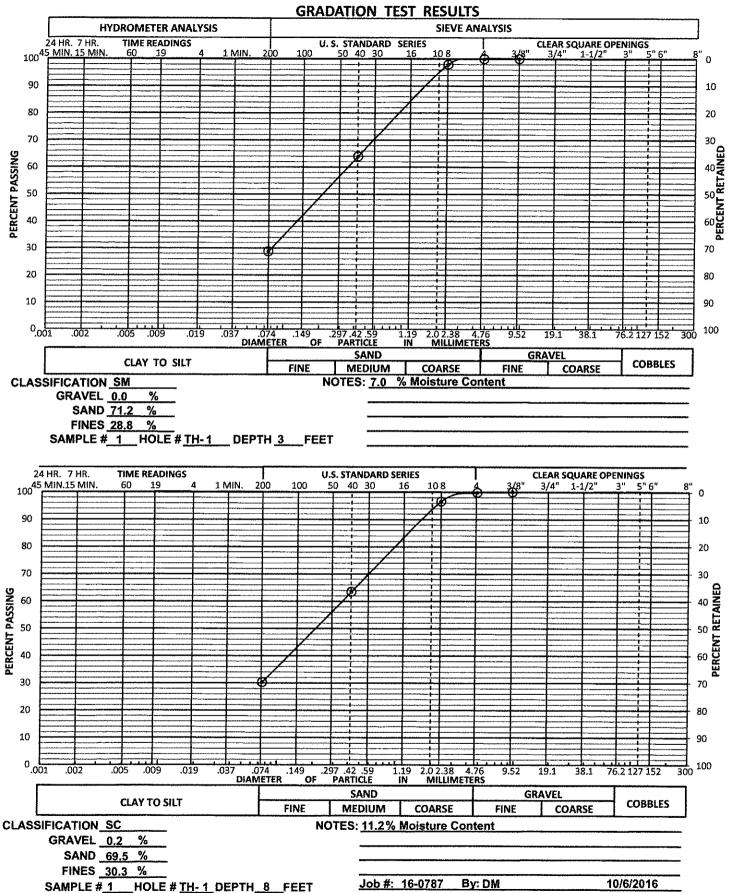
N. 38' 50' 55.05"

W. 104° 41' 22.66"



0 10 20 30 40 50 GRAPHIC SCALE IN FEET SCALE: 1" = 50'

GEOQUEST LLC





Stormwater Detention and Infiltration Design Data Sheet

Workhook Protected

Worksheet Protected

Stormwater Facility Name: A-1 Chipseal Sand Filter

Facility Location & Jurisdiction: 7245 Cole View, El Paso County

User Input: Watershed Characteristics

0.035	ft/ft
300	ft
1.01	acres
95.0%	percent
100.0%	percent
0.0%	percent
0.0%	percent
	300 1.01 95.0% 100.0% 0.0%

Location for 1-hr Rainfall Depths (use dropdown):

Denver - Capitol Building

▼

WQCV Treatment Method = Sand Filter

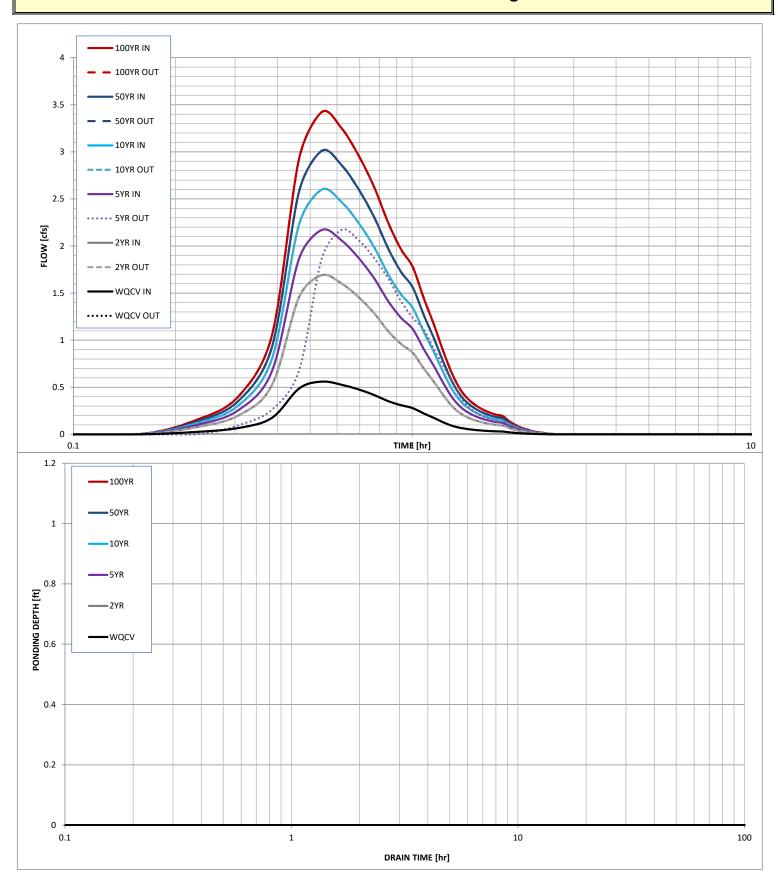
User Defined	User Defined	User Defined	User Defined
Stage [ft]	Area [ft^2]	Stage [ft]	Discharge [cfs]
0.00	1,431	0.00	7.50
0.25	1,431	0.25	7.50
0.50	1,431	0.50	7.50
0.75	1,431	0.75	7.50
1.00	1,431	1.00	7.50
1.25	1,431	1.25	7.50
1.50	1,431	1.50	7.50
1.75	1,431	1.75	7.50
2.00	1,431	2.00	7.50
2.25	1,431	2.25	7.50
2.50	1,431	2.50	7.50

After completing and printing this worksheet to a pdf, go to: https://maperture.digitaldataservices.com/gvh/?viewer=cswdif create a new stormwater facility, and attach the pdf of this worksheet to that record.

Routed Hydrograph Results

	Routeu Hyuro	grapii kesuits					_
Design Storm Return Period =	WQCV	2 Year	5 Year	10 Year	50 Year	100 Year	
One-Hour Rainfall Depth =	0.53	1.19	1.50	1.75	2.00	2.25	in
Calculated Runoff Volume =	0.030	0.092	0.119	0.143	0.165	0.189	acre-ft
OPTIONAL Override Runoff Volume =							acre-ft
Inflow Hydrograph Volume =	0.030	0.092	0.119	0.142	0.165	0.188	acre-ft
Time to Drain 97% of Inflow Volume =	0.5	0.5	0.5	0.5	0.5	0.5	hours
Time to Drain 99% of Inflow Volume =	0.5	0.5	0.5	0.5	0.5	0.5	hours
Maximum Ponding Depth =	0.00	0.00	0.00	0.00	0.00	0.00	ft
Maximum Ponded Area =	0.03	0.03	0.03	0.03	0.03	0.03	acres
Maximum Volume Stored =	0.000	0.000	0.000	0.000	0.000	0.000	acre-ft

Stormwater Detention and Infiltration Design Data Sheet



DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.05 (January 2022)

Project: 7245 Cole View - A1 Chipseal



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	SF	
Watershed Area =	1.01	acres
Watershed Length =	300	ft
Watershed Length to Centroid =	150	ft
Watershed Slope =	0.035	ft/ft
Watershed Imperviousness =	95.00%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	12.0	hours
Location for 1-hr Rainfall Depths =	Denver - Capit	ol Building

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

the embedded Colorado Urban Hydrograph Procedure.						
Water Quality Capture Volume (WQCV) =	0.030	acre-feet				
Excess Urban Runoff Volume (EURV) =	0.132	acre-feet				
2-yr Runoff Volume (P1 = 1.19 in.) =	0.086	acre-feet				
5-yr Runoff Volume (P1 = 1.5 in.) =	0.111	acre-feet				
10-yr Runoff Volume (P1 = 1.75 in.) =	0.131	acre-feet				
25-yr Runoff Volume (P1 = 2 in.) =	0.152	acre-feet				
50-yr Runoff Volume (P1 = 2.25 in.) =	0.172	acre-feet				
100-yr Runoff Volume (P1 = 2.52 in.) =	0.195	acre-feet				
500-yr Runoff Volume (P1 = 3 in.) =	0.234	acre-feet				
Approximate 2-yr Detention Volume =	0.088	acre-feet				
Approximate 5-yr Detention Volume =	0.113	acre-feet				
Approximate 10-yr Detention Volume =	0.134	acre-feet				
Approximate 25-yr Detention Volume =	0.158	acre-feet				
Approximate 50-yr Detention Volume =	0.171	acre-feet				
Approximate 100-yr Detention Volume =	0.183	acre-feet				

Optional User	Overrides
	acre-feet
	acre-feet
1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.030	acre-fee
Select Zone 2 Storage Volume (Optional) =		acre-fee
Select Zone 3 Storage Volume (Optional) =		acre-fee
Total Detention Basin Volume =	0.030	acre-fee
Initial Surcharge Volume (ISV) =	N/A	ft ³
Initial Surcharge Depth (ISD) =	N/A	ft
Total Available Detention Depth (H _{total}) =	user	ft
Depth of Trickle Channel (H _{TC}) =	N/A	ft
Slope of Trickle Channel (S _{TC}) =	N/A	ft/ft
Slopes of Main Basin Sides (S _{main}) =	user	H:V
Basin Length-to-Width Ratio (R _{L/W}) =	user	
		•

Initial Surcharge Area (A_{ISV}) = Surcharge Volume Length (L_{ISV}) = user Surcharge Volume Width (W_{ISV}) = Depth of Basin Floor (H_{FLOOR}) = user Length of Basin Floor (L_{FLOOR}) = Width of Basin Floor (W_{FLOOR}) = user user Area of Basin Floor (A_{FLOOR}) = user Volume of Basin Floor (V_{FLOOR}) = Depth of Main Basin (H_{MAIN}) = user Length of Main Basin (L_{MAIN}) = user Width of Main Basin (W_{MAIN}) = user Area of Main Basin (A_{MAIN}) = user Volume of Main Basin (V_{MAIN}) = Calculated Total Basin Volume (V_{total}) =

Please use UD-BMP calculation spreadsheet for sand filter basin design

Depth Increment =	0.10	ft		1		0.0		ı	I
Stage - Storage	Stage	Optional Override	Length	Width	Area	Optional Override	Area	Volume	Volume
Description	(ft)	Stage (ft)	(ft)	(ft)	(ft 2)	Area (ft ²)	(acre)	(ft 3)	(ac-ft)
Media Surface	-	0.00				1,431	0.033		
	-	0.25				1,431	0.033	358	0.008
	-	0.50				1,431	0.033	716	0.016
		0.75				1,431	0.033	1,073	0.025
6334.6		1.00				1,431	0.033	1,431	0.023
0334.0		1.25				1,431	0.033	1,789	0.033
		1.50				1,431	0.033		0.041
								2,147	
C22F C		1.75				1,431	0.033	2,504	0.057
6335.6	-	2.00				1,431	0.033	2,862	0.066
	-	2.25				1,431	0.033	3,220	0.074
	-	2.50	-			1,431	0.033	3,577	0.082
	-		-						
	-		-						
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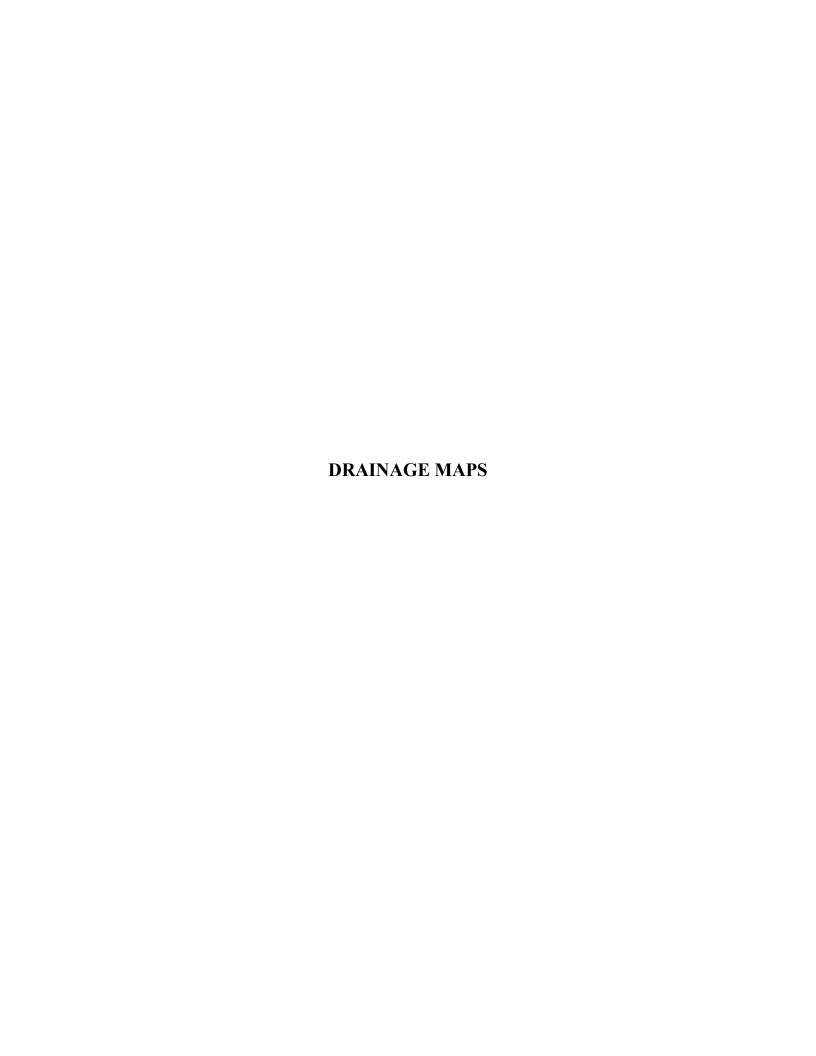
217300 UD-Det (SF), Basin 5/2/2022, 12:57 PM

INFILTRATION RATE TO VOLUME REQUIRED CONVERSION

2173.00 A-1 Chipseal Sand Filter - Full Infiltration Design Calcs Dane Frank, 2022/04/05

Avg Infiltration Rate = (per Geoquest Percolation Test)	16.0 <u>min</u> in	=	0.0625 <u>in</u> min		
Conversion to in/hr	0.0625 in	60 min		=	3.75 in
	min	1 hr			hr
Sand Filter Surface Area =	1441 sq ft				
Infiltration Rate of Sand Filter =	3.75 in	1 ft	1441 sq ft	=	450 cf
	hr	12 in			hr
Volume Afer 12 Hours of Flow =	450 cf	12 hr	1 ac	=	0.124 ac-ft
	hr		43560 sq ft		
Required WQCV = (per UD-Detention Spreadsheet)	0.033 ac-ft		Required volur	ne exceeded by 3	3.8 times

provide SFB riprap calculations

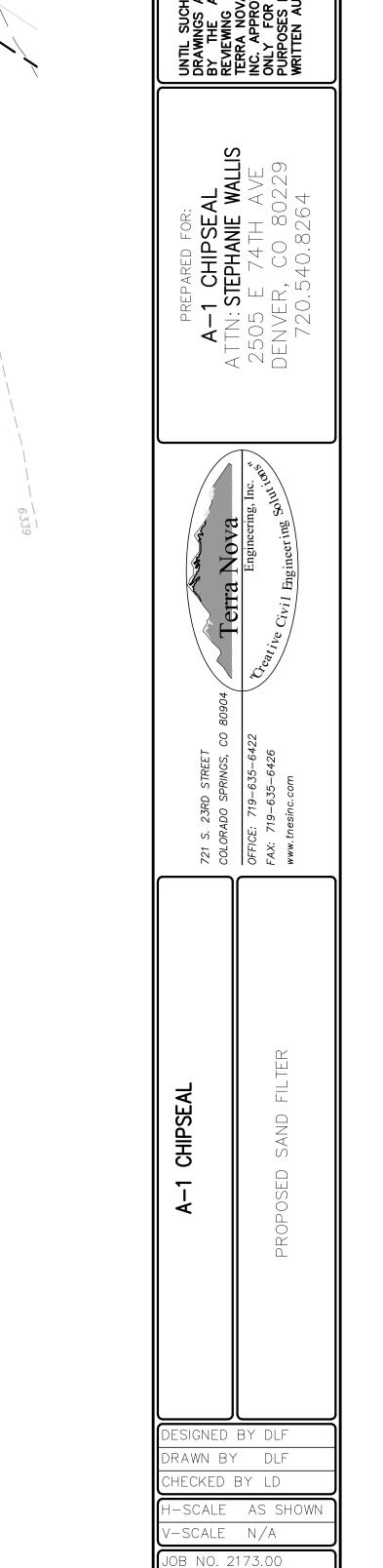


A-1 CHIPSEAL BASIN SUMMARY COLORADO SPRINGS DESIGN POINT PROPOSED DRAINAGE MAP 0.35 MAY 2022 ease show the time of concentration path and updat UNDERGROUND STORM SEWER LOCATION UNKNOWN. PIPE MAY NOT BE CONTAINED IN A UTILITY EASEMENT LOT 38 VACATED LOT LINE-R.N. -207009144 P-7 BASIN DESIGNATION AREA IN BASIN (AC) PERCENT IMPERVIOUS PARCEL B 7231 COLE VIEW COMMERCIAL BUILDING 7,575±s.f. FOOTPRINT --- EXISTING 10' CONTOUR GROUND SURFACE FLOW DIRECTION ROAD AND DITCH FLOW DIRECTION CHAIN-LINK FENCE LOT 37 - PR FULL INFILTRATION WQ SAND FILTER, 1,431 SF PR WHEEL STOPS (TYP) EX ASPHALT SURFACE (ONE LIFT) -EQUIPMENT STORAGE 0.38 AC RECENTLY ADDED ASPHALT EX SEWER MANHOLE EX SITE LOW POINT DRAINS -INTO EX CARRY CURB PARCEL A LOT 36 NO BUILDINGS NO POSTED ADDRESS ESIGNED BY DLF RAWN BY DLF HECKED BY LD -SCALE AS SHOWI -SCALE N/A SCALE: 1"=20' IOB NO. 2173.00 ATE ISSUED 05/02/

PER USDCM VOL 3 TABLE SF-1

BEDDING SAND: 6" DEEP CDOT-CLASS C FILTER MATERIAL PER USDCM VOL 3 TABLE SF-1 TOP OF NATIVE MATERIAL=6331.6

SAND FILTER DESIGN - PROFILE VIEW



ATE ISSUED 05/02/2 HEET NO. 3 OF 3