



INNOVATIVE DESIGN. **CLASSIC RESULTS.**

**FINAL DRAINAGE REPORT
ADDENDUM NO. 1
“PRELIMINARY DRAINAGE REPORT FOR FLYING HORSE
NORTH PRELIMINARY PLAN AND FINAL DRAINAGE REPORT
FOR FLYING HORSE NORTH FILING NO. 1”
(PCD file No. SP-17-012 and SF-18-001)**

**TRACT K, FLYING HORSE NORTH FILING NO. 1
(GOLF COURSE TEMPORARY CLUBHOUSE / EVENT LAWN)**

JANUARY 2021

Prepared for:
FLYING HORSE COUNTRY CLUB, LLC
2138 FLYING HORSE CLUB DRIVE
COLORADO SPRINGS, CO 80921
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Job no. 1096.15
PCD File No. PPR-20-043

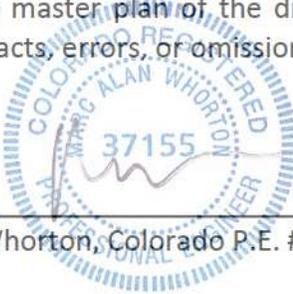


**FINAL DRAINAGE REPORT ADDENDUM NO. 1 - FLYING HORSE NORTH FILING NO. 1
FOR FLYING HORSE NORTH GOLF COURSE TEMPORARY CLUBHOUSE / EVENT LAWN**

DRAINAGE REPORT STATEMENT

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.



Marc A. Whorton, Colorado P.E. #37155

3/11/2021
Date

OWNER'S/DEVELOPER'S STATEMENT:

I, the owner/developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: Flying Horse Country Club, LLC


Title: Vice President

Address: 2138 Flying Horse Club Drive

Colorado Springs, CO 80921

EL PASO COUNTY:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Jennifer Irvine, P.E.
County Engineer, / ECM Administrator

Date

Conditions:



**FINAL DRAINAGE REPORT ADDENDUM NO. 1 - FLYING HORSE NORTH FILING NO. 1
FOR FLYING HORSE NORTH GOLF COURSE TEMPORARY CLUBHOUSE / EVENT LAWN**

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FINAL DRAINAGE REPORT ADDENDUM NO. 1 - FLYING HORSE NORTH FILING NO. 1 FOR FLYING HORSE NORTH GOLF COURSE TEMPORARY CLUBHOUSE / EVENT LAWN

PURPOSE

This portion of the Flying Horse North development was previously platted as Tract K within Flying Horse North Filing No. 1. It was also previously analyzed from a drainage standpoint and included in the Flying Horse North Filing No. 1 Final Drainage Report, approved August 2018. As found in that report, the previous drainage basin CC-4C encompasses the proposed development within Tract K. The purpose of this report is to allow for the submittal of a Site Development for the golf course temporary clubhouse and event lawn and better define the temporary drainage patterns for this development.

GENERAL DESCRIPTION

Tract K, Flying Horse North Filing No. 1 contains a total acreage of 65.023 ac., located in the county of El Paso within Section 36, Township 11 South, Range 66 West of the Sixth Principal Meridian, El Paso County, Colorado. The portion of the site proposed for development is bounded on the north by rural residential lots 32 and 33, Flying Horse North Filing No. 1, on the east by Allen Ranch Road, on the west and south by Flying Horse North Golf Course within the remaining portion of Tract K, Flying Horse North Filing No. 1.

The average soil condition reflects Hydrologic Group "B" (Peyton sandy loam), as determined by the "Soil Survey of El Paso County Area," prepared by the Soil Conservation Service. (See Appendix) For the purposes of the hydrologic calculations within this report, the soil type B was utilized.

EXISTING/DEVELOPED DRAINAGE CONDITIONS

The entire proposed development area was previously overlotted graded along with the golf course and Filing 1. The revegetation consists of native grasses with slopes of 2%-4%. The entire property sheet flows in an easterly direction towards the side road ditch along the west side of Allen Ranch Road. The pre-developed flows for this area were described in the approved drainage report as Basin CC-4C ($Q_5 = 1.8$ cfs and $Q_{100} = 11.2$ cfs). The designed outfall



Revise to
"west"

for these flows are into the side road ditch along the west side of Allen Ranch Road and then east within the side road ditch along the south side of Stagecoach Road. The flows will then cross Stagecoach Road at a low point via a 36" RCP culvert and then head northeast within a natural channel towards the existing constructed Detention Pond 12. (Reference previous drainage map in Appendix along with ditch and culvert capacity calculations)

The proposed development plans to construct a temporary clubhouse (three trailers), an event lawn, paved parking lot and a golf cart barn. All of these facilities are temporary in nature as a permanent clubhouse development is ultimately planned for this area. Timing of this ultimate development is unknown at this point but will require a separate formal site plan submittal. Vehicular access to the parking lot will be taken directly from Allen Ranch Road. The event lawn area will sheet flow towards the private golf course while the remaining portion of the site (Basin B) will continue to drain in an easterly direction towards a temporary sediment basin adjacent to Allen Ranch Road. These developed flows will sheet flow across the remaining native areas and the proposed parking lot with portions being routed via asphalt curbing adjacent to the parking lot and ditches that will be created to help route all flows towards the sediment basin. This facility will help control the sediment runoff from this temporary development and remain in place until a permanent facility is constructed along with the ultimate development. Stormwater quality for this development will continue to be handled in the off-site detention pond 12 as described and approved in the previously mentioned Final Drainage Report for Flying Horse North Filing No. 1.

Basin A ($Q_5 = 0.2$ cfs and $Q_{100} = 1.0$ cfs) represents developed flows from Basin A. This basin continues to sheet flow towards the private Golf Course. The Golf Course will maintain the existing natural slope to prevent any erosion. No further improvements are proposed for this basin.

Basin B ($Q_5 = 5.5$ cfs and $Q_{100} = 11.8$ cfs) represents developed flows from Basin B. This basin is the major portion of the temporary development and includes the trailers, paved parking lot

and cart barn. The cart barn downspouts will be routed to the east towards the ditch created to route the flows towards the sediment basin. This compares to the pre-developed flows of ($Q_5 = 1.8$ cfs and $Q_{100} = 11.2$ cfs) as presented in the original final drainage report. The downstream side road ditches can adequately handle these flows. (See Appendix for ditch calculation) This minor increase in flows is handled by the proposed sediment basin along with two planned permanent rock check dams downstream within the side road ditch along Allen Ranch Road. These two structures will help control erosion from the release of the sediment basin.

Basin C ($Q_5 = 0.5$ cfs and $Q_{100} = 2.5$ cfs) represents developed flows from Basin C. This basin will remain undisturbed other than the construction of the proposed concrete golf cart path. The minimal developed flows from this basin will sheet flow across the native ground towards the side road ditch along the west side of Allen Ranch Road. Given the existing highpoint in the roadway, these flows will travel in a southerly direction where a proposed 18" culvert will be installed at the golf cart crossing. These ditch flows eventually sheet flow into the golf course area just west of the existing temporary roadway turn-around.

Given the temporary nature of this development and as mentioned previously, the off-site detention/stormwater quality pond 12 is currently handling the stormwater quality for the site, a temporary sediment basin is planned for this development. This facility will handle sediment control for all the developed portion of the site prior to release. Upon the ultimate clubhouse site being planned and constructed, a separate site specific Final Drainage Report with associated detention and stormwater quality will be required.

The following are the parameters for the temporary facility:

Temporary Sediment Basin

Design criteria per Urban Storm Drainage Criteria Manual Vol. 3 (SC-7)

| | | | |
|--------------------------------|--|---|-----------------------------|
| Undisturbed area (0% imp.) | 1.4 ac. | | |
| Disturbed area (100% imp.) | 1.8 ac. | | |
| Basin Storage Volume Required: | 1.4 ac. X 500 cu. ft. | = | 700 cu. ft. |
| | 1.8 ac. X 3,600 cu. ft. | = | 6,480 cu. ft. |
| | | = | <u>7,180 cu. ft.</u> |
| Spillway Crest Length: | 5.0' wide rip-rap outfall | | |
| Hole Diameter: | 1/2 in. (single row of 5 holes – total area = 0.98 sq-in.) | | |
| Basin Outlet: | 6" PVC Riser pipe and outfall | | |

This facility will be constructed on-site within a drainage easement with ownership and maintenance by the property owner.

HYDROLOGIC CALCULATIONS

Hydrologic calculations were performed using the City of Colorado Springs/El Paso County Drainage Criteria Manual, as revised in November 1991 and 1994 with County adopted Chapter 6 and Section 3.2.1 of Chapter 13 of the City of Colorado Springs/El Paso County Drainage Criteria Manual as revised in May 2014. Individual on-site developed basin design was calculated using the Rational Method. BMP design was calculated using the Urban Storm Drainage Criteria Manual for a Sediment Basin (SC-7).

The City of Colorado Springs/El Paso County DCM requires the Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainage ways, and implementing long-term source controls. The Four Step Process pertains to management of smaller, frequently occurring storm events, as opposed to larger storms for which drainage and flood control infrastructure are sized.



Implementation of these four steps helps to achieve storm water permit requirements. This site adheres to this **Four Step Process** as follows:

1. **Employ Runoff Reduction Practices:** Development of this site is proposing two temporary buildings, paved parking lot and an event lawn area. Proposed impervious areas (roof tops, sidewalks and parking lot) will sheet flow across revegetated ground within the property to slow runoff and increase time of concentration prior to entering the on-site sediment pond and then being conveyed to the sideroad ditch along the public road. This will minimize directly connected impervious areas within the project site.
2. **Stabilize Drainageways:** This site will utilize an on-site sediment basin to control sediment and erosion and then install permanent rock check dams to help stabilize the downstream drainageway. With the use of these multiple BMPs, there will be no significant change from the pre-developed flows and thus no impact to downstream drainageways is anticipated.
3. **Provide Water Quality Capture Volume (WQCV):** Per the approved Final Drainage Report for Flying Horse North Filing No. 1, the downstream detention/stormwater quality pond 12, currently constructed, provides the required WQCV for this development. Upon the future ultimate clubhouse development, a separate permanent on-site facility will be required.
4. **Consider need for Industrial and Commercial BMPs:** No industrial or commercial uses are proposed within this development. However, a site specific storm water quality and erosion control plan addendum is being submitted concurrently with this report and development. Details such as site specific construction BMP's as well as permanent sediment control BMP's are detailed in this plane to protect receiving waters. Roadside ditch stabilization, in the form of permanent rock check dams (as specified on the plans)

are also proposed. The described BMP's will be constructed by the developer upon approval by El Paso County Staff.

FLOODPLAIN STATEMENT

No portion of this site is located within a FEMA floodplain as determined by the Flood Insurance Rate Map (F.I.R.M.) Map Number 08041C0315G, with effective date of December, 7 2018. (See Appendix)

DRAINAGE AND BRIDGE FEES

This site lies entirely within the East Cherry Creek Drainage Basin boundaries and was platted as Tract K within the Flying Horse North Filing No. 1 Final Plat. This basin currently does not have any basin fees and thus, no fees are due at this time.

SUMMARY

The proposed temporary development, grading plan, drainage patterns and quantities remain consistent with the previously approved final drainage report for Flying Horse North Filing No. 1. The proposed development will not adversely impact surrounding properties.

PREPARED BY:

Classic Consulting Engineers & Surveyors, LLC



Marc A. Whorton, P.E.
Project Manager

mw/109615/Reports/109615FDR Addendum.doc



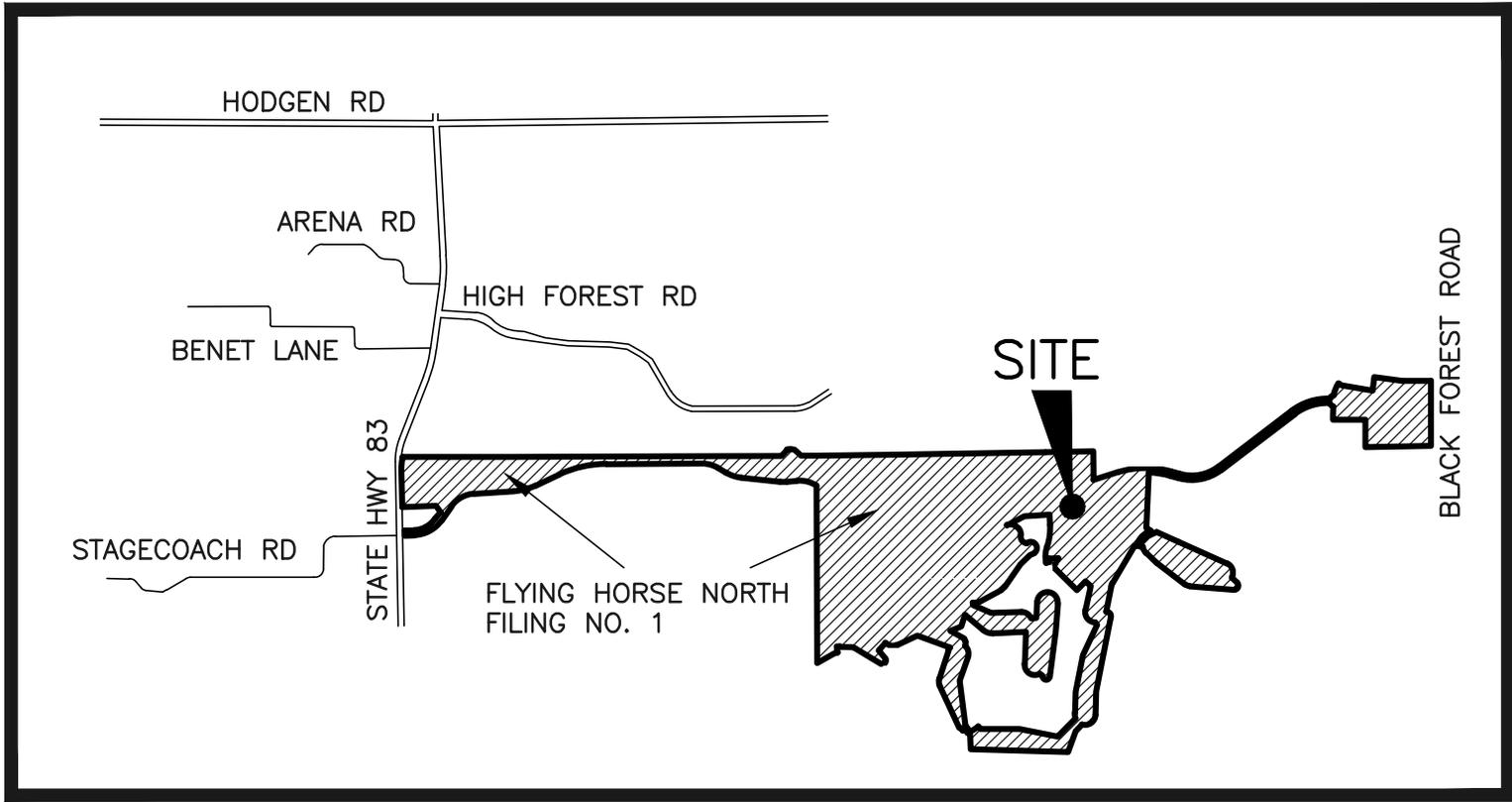
REFERENCES

1. City of Colorado Springs/County of El Paso Drainage Criteria Manual, as revised in November 1991 and 1994 with County adopted Chapter 6 and Section 3.2.1 of Chapter 13 of the City of Colorado Springs/El Paso County Drainage Criteria Manual as revised in May 2014.
2. "Master Development Drainage Plan for Flying Horse North", Classic Consulting, dated September 2016.
3. "Preliminary Drainage Report for Flying Horse North (Golf Course Grading and Private Access Roads)", Classic Consulting, dated September 2016.
4. "Final Drainage Report Flying Horse North Filing No. 1" Classic Consulting, dated September 2018.
5. "Urban Storm Drainage Criteria Manual Volume 1, 2 & 3" Urban Drainage and Flood Control District, dated January 2016.



APPENDIX

VICINITY MAP

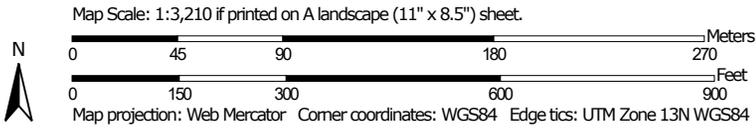


SOILS MAP (S.C.S. SURVEY)

Soil Map—El Paso County Area, Colorado



Soil Map may not be valid at this scale.



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 18, Jun 5, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 8, 2018—May 26, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
|------------------------------------|---|--------------|----------------|
| 26 | Elbeth sandy loam, 8 to 15 percent slopes | 6.6 | 14.5% |
| 66 | Peyton sandy loam, 1 to 5 percent slopes | 17.0 | 37.5% |
| 67 | Peyton sandy loam, 5 to 9 percent slopes | 21.7 | 48.0% |
| Totals for Area of Interest | | 45.2 | 100.0% |

El Paso County Area, Colorado

66—Peyton sandy loam, 1 to 5 percent slopes

Map Unit Setting

National map unit symbol: 369c

Elevation: 6,800 to 7,600 feet

Farmland classification: Prime farmland if irrigated and the product of
I (soil erodibility) x C (climate factor) does not exceed 60

Map Unit Composition

Peyton and similar soils: 85 percent

*Estimates are based on observations, descriptions, and transects of
the mapunit.*

Description of Peyton

Setting

Landform: Flats, hills

Landform position (three-dimensional): Side slope, talf

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock
and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam

Bt - 12 to 25 inches: sandy clay loam

BC - 25 to 35 inches: sandy loam

C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 1 to 5 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water

(Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water capacity: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4c

Hydrologic Soil Group: B

Ecological site: R049XB216CO - Sandy Divide

Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:

Hydric soil rating: No

Pleasant

Percent of map unit:

Landform: Depressions

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado

Survey Area Data: Version 18, Jun 5, 2020

F.E.M.A MAP

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, NUNCS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at <http://www.ngs.noaa.gov/>.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, City of Fountain, Bureau of Land Management, National Oceanic and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

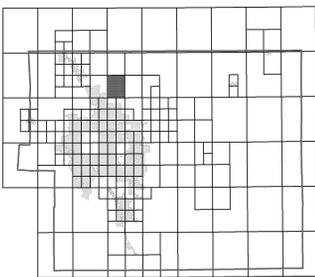
Contact **FEMA Map Service Center (MSC)** via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at <http://www.msc.fema.gov/>.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at <http://www.fema.gov/business/nfp/>.

El Paso County Vertical Datum Offset Table

| Flooding Source | Vertical Datum Offset (ft) |
|---|----------------------------|
| REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION | |

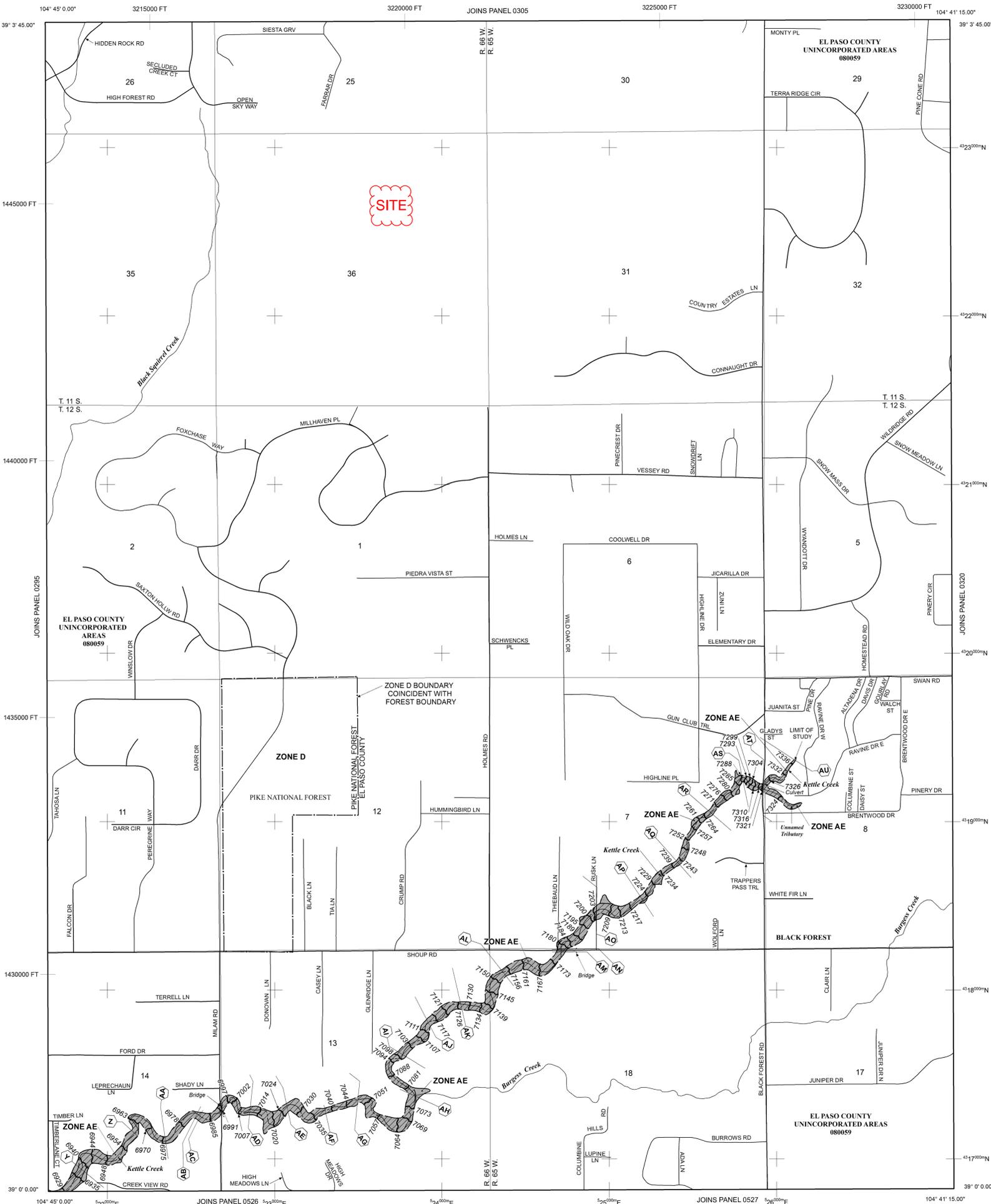
Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCW) and the Federal Emergency Management Agency (FEMA).



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equalled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
- ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
- ZONE AR** Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot, or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.

ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- Floodplain boundary
- Floodway boundary
- Zone D Boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.

513 Base Flood Elevation line and value; elevation in feet* (EL 987)

513 Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

A-A Cross section line

23-23 Transsect line

97° 07' 30.00" 32° 22' 30.00" Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)

4750000N 1000-meter Universal Transverse Mercator grid ticks, zone 13

6000000 FT 5000-foot grid ticks; Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection

DX5510 Bench mark (see explanation in Notes to Users section of this FIRM map)

M1.5 River Mile

MAP REPOSITORIES Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP

MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

MAP SCALE 1" = 1000'

500 0 1000 2000 FEET

300 0 300 600 METERS

NFIP **PANEL 0315G**

FIRM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY,
COLORADO
AND INCORPORATED AREAS

PANEL 315 OF 1300
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:
COMMUNITY NUMBER PANEL SUFFIX
EL PASO COUNTY 080059 0315 0

Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.

MAP NUMBER
08041C0315G

MAP REVISED
DECEMBER 7, 2018

Federal Emergency Management Agency

CALCULATIONS

ALL LAND ASSUMED 2 ACRE RESIDENTIAL LOTS, UNDEVELOPED WOODS OR
GOOD CONDITION OPEN SPACE (LAWNS, PARKS GOLF COURSES, CEMETARIES ETC.)

C_N VALUES - DEVELOPED CONDITIONS

| BASIN (label) | BASIN AREA (Ac) | GOLF COURSE / WOODS (B) | | 2 AC. RESIDENTIAL (B) | | COMPOSITE C _N |
|------------------|-----------------------|-------------------------|---------------|-----------------------|---------------|-----------------------------|
| | | CN | AREA (Ac.) | CN | AREA (Ac.) | |
| CC-1A | 9.8 | 61 | 0.0 | 65 | 9.8 | 65.0 |
| CC-1B | 12.6 | 61 | 0.5 | 65 | 12.1 | 64.8 |
| CC-2A | 11.0 | 61 | 0.0 | 65 | 11.0 | 65.0 |
| CC-2B | 20.8 | 61 | 0.0 | 65 | 20.8 | 65.0 |
| CC-2C | 6.4 | 61 | 0.0 | 65 | 6.4 | 65.0 |
| CC-3 | 52.5 | 61 | 25.0 | 65 | 27.5 | 63.1 |
| CC-4A | 108.7 | 61 | 65.0 | 65 | 43.7 | 62.6 |
| CC-4B | 8.1 | 85 | 4.5 | 65 | 3.6 | 76.1 |
| CC-4C (Pre-Dev.) | 7.4 | 61 | 7.4 | 65 | 0.0 | 61.0 |
| CC-5 | 22.4 | 61 | 0.0 | 65 | 22.4 | 65.0 |
| CC-6 | 27.8 | 61 | 0.0 | 65 | 27.8 | 65.0 |
| CC-7 | 18.4 | 61 | 0.0 | 65 | 18.4 | 65.0 |
| CC-8 | 7.7 | 61 | 0.0 | 65 | 7.7 | 65.0 |
| CC-9 | 5.6 | 61 | 0.0 | 65 | 5.6 | 65.0 |
| CC-10 | 85.6 | 61 | 51.0 | 65 | 34.6 | 62.6 |
| CC-11 | 18.6 | 61 | 9.0 | 65 | 9.6 | 63.1 |
| CC-12 | 12.2 | 61 | 0.0 | 65 | 12.2 | 65.0 |
| CC-13A | 19.3 | 61 | 0.0 | 65 | 19.3 | 65.0 |
| CC-13B | 25.5 | 61 | 0.0 | 65 | 25.5 | 65.0 |
| CC-13C | 9.9 | 61 | 0.0 | 65 | 9.9 | 65.0 |
| CC-13D | 18.8 | 61 | 0.0 | 65 | 18.8 | 65.0 |
| CC-14 | 4.6 | 61 | 0.0 | 65 | 4.6 | 65.0 |
| CC-15 | 12.8 | 61 | 0.0 | 65 | 12.8 | 65.0 |
| CC-16 | 16.3 | 61 | 0.0 | 65 | 16.3 | 65.0 |
| CC-17 | 25.0 | 61 | 0.0 | 65 | 25.0 | 65.0 |
| CC-18 | 6.2 | 65 | 5.8 | 89 | 0.4 | 66.5 |
| CC-19 | 3.7 | 61 | 0.0 | 65 | 3.7 | 65.0 |
| CC-20 | 39.3 | 61 | 0.0 | 65 | 39.3 | 65.0 |
| CC-21 | 6.2 | 61 | 6.2 | 65 | 0.0 | 61.0 |
| CC-22 | 13.8 | 61 | 0.0 | 65 | 13.8 | 65.0 |
| CC-23 | 5.7 | 61 | 0.4 | 65 | 5.3 | 64.7 |
| CC-24 | 39.6 | 61 | 0.0 | 65 | 39.6 | 65.0 |
| CC-25 | 3.5 | 61 | 0.0 | 65 | 3.5 | 65.0 |
| CC-26 | 16.7 | 61 | 0.0 | 65 | 16.7 | 65.0 |
| CC-27 | 18.9 | 61 | 3.0 | 65 | 15.9 | 64.4 |
| CC-28 | 154.8 | 61 | 23.0 | 65 | 131.8 | 64.4 |

TIME OF CONCENTRATION - DEVELOPED

| BASIN | COMPOSITE Cn | C(5) | Length (ft) | OVERLAND Height (ft) | Tc (min) | STREET / CHANNEL FLOW (DCM Vol. 1 Fig. 6-25) | | | | Tc TOTAL (min) | Tc LAG (0.6tc) (min) | Tc LAG (0.6tc) (hr) |
|------------------|-----------------|------|----------------|----------------------------|-------------|--|--------------|-------------------|-------------|----------------------|----------------------------|---------------------------|
| | | | | | | Length (ft) | Slope (%) | Velocity (fps) | Tc (min) | | | |
| CC-1A | 65.0 | 0.08 | 300 | 16 | 18.4 | 500 | 5.0% | 1.7 | 4.9 | 23.3 | 14.0 | 0.23 |
| CC-1B | 64.8 | 0.08 | 300 | 14 | 19.2 | 700 | 4.0% | 2.0 | 5.8 | 25.0 | 15.0 | 0.25 |
| CC-2A | 65.0 | 0.08 | 300 | 14 | 19.2 | 250 | 3.0% | 1.5 | 2.8 | 22.0 | 13.2 | 0.22 |
| CC-2B | 65.0 | 0.08 | 300 | 14 | 19.2 | 280 | 3.0% | 1.5 | 3.1 | 22.3 | 13.4 | 0.22 |
| CC-2C | 65.0 | 0.08 | 300 | 18 | 17.7 | | | | | 17.7 | 10.6 | 0.18 |
| CC-3 | 63.1 | 0.08 | 300 | 18 | 17.7 | 2300 | 3.0% | 1.5 | 25.6 | 43.2 | 25.9 | 0.43 |
| CC-4A | 62.6 | 0.08 | 300 | 14 | 19.2 | 2700 | 2.0% | 1.8 | 25.0 | 44.2 | 26.5 | 0.44 |
| CC-4B | 76.1 | 0.08 | 300 | 12 | 20.2 | 600 | 3.0% | 1.6 | 6.3 | 26.4 | 15.9 | 0.26 |
| CC-4C (Pre-Dev.) | 61.0 | 0.08 | 40 | 0.8 | 9.3 | 350 | 3.0% | 1.5 | 3.9 | 13.2 | 7.9 | 0.13 |
| CC-5 | 65.0 | 0.08 | 300 | 18 | 17.7 | 1000 | 4.0% | 2.0 | 8.3 | 26.0 | 15.6 | 0.26 |
| CC-6 | 65.0 | 0.08 | 300 | 14 | 19.2 | 550 | 2.5% | 1.6 | 5.7 | 24.9 | 14.9 | 0.25 |
| CC-7 | 65.0 | 0.08 | 300 | 16 | 18.4 | 1000 | 3.0% | 1.6 | 10.4 | 28.8 | 17.3 | 0.29 |
| CC-8 | 65.0 | 0.08 | 300 | 10 | 21.4 | 250 | 2.0% | 1.2 | 3.5 | 24.9 | 14.9 | 0.25 |
| CC-9 | 65.0 | 0.08 | 300 | 18 | 17.7 | 100 | 2.0% | 1.2 | 1.4 | 19.0 | 11.4 | 0.19 |
| CC-10 | 62.6 | 0.08 | 300 | 22 | 16.5 | 2400 | 3.0% | 1.8 | 22.2 | 38.7 | 23.2 | 0.39 |
| CC-11 | 63.1 | 0.08 | 300 | 18 | 17.7 | 450 | 5.0% | 2.1 | 3.6 | 21.2 | 12.7 | 0.21 |
| CC-12 | 65.0 | 0.08 | 300 | 11 | 20.8 | 650 | 4.0% | 2.0 | 5.4 | 26.2 | 15.7 | 0.26 |
| CC-13A | 65.0 | 0.08 | 300 | 14 | 19.2 | 1400 | 4.0% | 2.0 | 11.7 | 30.9 | 18.5 | 0.31 |
| CC-13B | 65.0 | 0.08 | 300 | 18 | 17.7 | 1300 | 3.0% | 1.6 | 13.5 | 31.2 | 18.7 | 0.31 |
| CC-13C | 65.0 | 0.08 | 300 | 14 | 19.2 | 350 | 4.0% | 2.0 | 2.9 | 22.1 | 13.3 | 0.22 |
| CC-13D | 65.0 | 0.08 | 300 | 20 | 17.1 | 900 | 4.0% | 2.0 | 7.5 | 24.6 | 14.7 | 0.25 |
| CC-14 | 65.0 | 0.08 | 300 | 10 | 21.4 | | | | | 21.4 | 12.9 | 0.21 |
| CC-15 | 65.0 | 0.08 | 300 | 14 | 19.2 | 550 | 3.0% | 1.8 | 5.1 | 24.3 | 14.6 | 0.24 |
| CC-16 | 65.0 | 0.08 | 300 | 10 | 21.4 | 650 | 2.5% | 1.3 | 8.3 | 29.8 | 17.9 | 0.30 |
| CC-17 | 65.0 | 0.08 | 300 | 9 | 22.2 | 950 | 2.0% | 1.2 | 13.2 | 35.4 | 21.2 | 0.35 |
| CC-18 | 66.5 | 0.08 | 300 | 7 | 24.1 | 400 | 2.0% | 1.2 | 5.6 | 29.7 | 17.8 | 0.30 |
| CC-19 | 65.0 | 0.08 | 300 | 8 | 23.1 | 100 | 2.0% | 1.0 | 1.7 | 24.7 | 14.8 | 0.25 |
| CC-20 | 65.0 | 0.08 | 300 | 9 | 22.2 | 350 | 6.0% | 2.2 | 2.7 | 24.8 | 14.9 | 0.25 |
| CC-21 | 61.0 | 0.08 | 300 | 18 | 17.7 | 200 | 3.0% | 1.8 | 1.9 | 19.5 | 11.7 | 0.20 |
| CC-22 | 65.0 | 0.08 | 300 | 14 | 19.2 | 700 | 4.0% | 2.0 | 5.8 | 25.0 | 15.0 | 0.25 |
| CC-23 | 64.7 | 0.08 | 300 | 10 | 21.4 | 850 | 2.0% | 1.2 | 11.8 | 33.2 | 19.9 | 0.33 |
| CC-24 | 65.0 | 0.08 | 300 | 20 | 17.1 | 900 | 4.0% | 1.9 | 7.9 | 25.0 | 15.0 | 0.25 |
| CC-25 | 65.0 | 0.08 | 300 | 16 | 18.4 | 500 | 3.0% | 1.8 | 4.6 | 23.0 | 13.8 | 0.23 |
| CC-26 | 65.0 | 0.08 | 300 | 14 | 19.2 | 900 | 5.0% | 2.1 | 7.1 | 26.3 | 15.8 | 0.26 |
| CC-27 | 64.4 | 0.08 | 300 | 14 | 19.2 | 1300 | 3.0% | 1.8 | 12.0 | 31.2 | 18.7 | 0.31 |
| CC-28 | 64.4 | 0.08 | 300 | 14 | 19.2 | 4700 | 3.0% | 1.8 | 43.5 | 62.7 | 37.6 | 0.63 |

BASIN SUMMARY - DEVELOPED CONDITIONS

| BASIN (label) | AREA (acres) | COMPOSITE CN | TOTAL LAG TIME (hours) | Q 2 Yr. (cfs) | Q 5 Yr. (cfs) | Q 100 Yr. (cfs) |
|------------------|-----------------|-----------------|------------------------------|---------------------|---------------------|-----------------------|
| CC-1A | 9.80 | 65.0 | 0.23 | 0.8 | 3.3 | 16.0 |
| CC-1B | 12.60 | 64.8 | 0.25 | 1.0 | 4.0 | 19.4 |
| CC-2A | 11.00 | 65.0 | 0.22 | 1.0 | 3.8 | 18.3 |
| CC-2B | 20.80 | 65.0 | 0.22 | 1.9 | 7.1 | 34.6 |
| CC-2C | 6.40 | 65.0 | 0.18 | 0.7 | 2.5 | 11.5 |
| CC-3 | 52.50 | 63.1 | 0.43 | 1.8 | 8.8 | 54.5 |
| CC-4A | 108.70 | 62.6 | 0.44 | 15.4 | 39.0 | 156.0 |
| CC-4B | 8.10 | 76.1 | 0.26 | 4.0 | 7.3 | 20.6 |
| CC-4C (Pre-Dev.) | 7.40 | 61.0 | 0.13 | 0.2 | 1.8 | 11.2 |
| CC-5 | 22.40 | 65.0 | 0.26 | 1.8 | 7.1 | 34.3 |
| CC-6 | 27.80 | 65.0 | 0.25 | 2.3 | 9.1 | 43.2 |
| CC-7 | 18.40 | 65.0 | 0.29 | 1.4 | 5.4 | 27.0 |
| CC-8 | 7.70 | 65.0 | 0.25 | 0.6 | 2.5 | 12.0 |
| CC-9 | 5.60 | 65.0 | 0.19 | 0.6 | 2.1 | 9.8 |
| CC-10 | 85.60 | 62.6 | 0.39 | 2.6 | 14.1 | 91.9 |
| CC-11 | 18.60 | 63.1 | 0.21 | 0.9 | 5.0 | 28.1 |
| CC-12 | 12.20 | 65.0 | 0.26 | 1.0 | 3.9 | 18.7 |
| CC-13A | 19.30 | 65.0 | 0.31 | 1.4 | 5.4 | 27.3 |
| CC-13B | 25.50 | 65.0 | 0.31 | 1.8 | 7.2 | 36.1 |
| CC-13C | 9.90 | 65.0 | 0.22 | 0.9 | 3.4 | 16.5 |
| CC-13D | 18.80 | 65.0 | 0.25 | 1.5 | 6.2 | 29.2 |
| CC-14 | 4.60 | 65.0 | 0.21 | 0.4 | 1.6 | 7.8 |
| CC-15 | 12.80 | 65.0 | 0.24 | 1.1 | 4.3 | 20.4 |
| CC-16 | 16.30 | 65.0 | 0.30 | 1.2 | 4.6 | 23.6 |
| CC-17 | 25.00 | 65.0 | 0.35 | 1.7 | 6.5 | 32.8 |
| CC-18 | 6.20 | 66.5 | 0.30 | 0.7 | 2.2 | 9.7 |
| CC-19 | 3.70 | 65.0 | 0.25 | 0.3 | 1.2 | 5.8 |
| CC-20 | 39.30 | 65.0 | 0.25 | 3.2 | 12.9 | 61.0 |
| CC-21 | 6.20 | 61.0 | 0.20 | 0.1 | 1.2 | 8.5 |
| CC-22 | 13.80 | 65.0 | 0.25 | 1.1 | 4.5 | 21.4 |
| CC-23 | 5.70 | 64.7 | 0.33 | 0.4 | 1.5 | 7.7 |
| CC-24 | 39.60 | 65.0 | 0.25 | 3.3 | 13.0 | 61.5 |
| CC-25 | 3.50 | 65.0 | 0.23 | 0.3 | 1.2 | 5.7 |
| CC-26 | 16.70 | 65.0 | 0.26 | 1.4 | 5.3 | 25.6 |
| CC-27 | 18.90 | 64.4 | 0.31 | 1.2 | 4.9 | 25.8 |
| CC-28 | 154.80 | 64.4 | 0.63 | 6.5 | 24.7 | 136.3 |
| | | | | | | |

JOB NAME: FLYING HORSE NORTH EVENT LAWN
 JOB NUMBER: 1096.15
 DATE: 03/10/21
 CALCULATED BY: MAW

FINAL DRAINAGE REPORT ~ BASIN RUNOFF COEFFICIENT SUMMARY

| BASIN | TOTAL AREA (AC) | IMPERVIOUS AREA / DRIVEWAYS | | | | LANDSCAPE/UNDEVELOPED AREAS | | | | WEIGHTED | | | WEIGHTED CA | | |
|-------|--------------------|-----------------------------|------|------|--------|-----------------------------|------|------|--------|----------|------|--------|-------------|-------|---------|
| | | AREA (AC) | C(2) | C(5) | C(100) | AREA (AC) | C(2) | C(5) | C(100) | C(2) | C(5) | C(100) | CA(2) | CA(5) | CA(100) |
| A | 0.4 | 0.04 | 0.89 | 0.90 | 0.96 | 0.36 | 0.02 | 0.08 | 0.35 | 0.11 | 0.16 | 0.41 | 0.04 | 0.06 | 0.16 |
| B | 3.2 | 1.80 | 0.89 | 0.90 | 0.96 | 1.40 | 0.02 | 0.08 | 0.35 | 0.51 | 0.54 | 0.69 | 1.63 | 1.73 | 2.22 |
| C | 1.1 | 0.08 | 0.89 | 0.90 | 0.96 | 1.02 | 0.02 | 0.08 | 0.35 | 0.08 | 0.14 | 0.39 | 0.09 | 0.15 | 0.43 |

JOB NAME: FLYING HORSE NORTH EVENT LAWN
 JOB NUMBER: 1096.15
 DATE: 03/10/21
 CALC'D BY: MAW

Table 6-7. Conveyance Coefficient, C_v

| Type of Land Surface | C_v |
|---|-------|
| Heavy meadow | 2.5 |
| Tillage/field | 5 |
| Riprap (not buried)* $t_c = \frac{L}{180} + 10$ | 6.5 |
| Short pasture and lawns | 7 |
| Nearly bare ground | 10 |
| Grassed waterway | 15 |
| Paved areas and shallow paved swales | 20 |

*For buried riprap, select C_v value based on type of vegetative cover.

$$t_i = \frac{0.395(1.1 - C_s)\sqrt{L}}{S^{0.33}} \quad V = C_v S_w^{0.5} \quad T_c = L/V$$

FINAL DRAINAGE REPORT ~ BASIN RUNOFF SUMMARY

| BASIN | TOTAL AREA (AC) | WEIGHTED | | | OVERLAND | | | STREET / CHANNEL FLOW | | | | Tc TOTAL (min) | INTENSITY | | | TOTAL FLOWS | | | |
|-------|-----------------|----------|-------|---------|----------|-------------|-------------|-----------------------|-------------|-----------|----------------|----------------|-----------|--------------|--------------|----------------|------------|------------|--------------|
| | | CA(2) | CA(5) | CA(100) | C(5) | Length (ft) | Height (ft) | Tc (min) | Length (ft) | Slope (%) | Velocity (fps) | | Tc (min) | I(2) (in/hr) | I(5) (in/hr) | I(100) (in/hr) | Q(2) (cfs) | Q(5) (cfs) | Q(100) (cfs) |
| A | 0.4 | 0.04 | 0.06 | 0.16 | 0.08 | 120 | 3 | 14.9 | | | | | 14.9 | 2.82 | 3.53 | 5.93 | 0.1 | 0.2 | 1.0 |
| B | 3.2 | 1.63 | 1.73 | 2.22 | 0.08 | 100 | 2 | 14.7 | 250 | 4.0% | 1.0 | 4.2 | 18.8 | 2.54 | 3.18 | 5.34 | 4.1 | 5.5 | 11.8 |
| C | 1.1 | 0.09 | 0.15 | 0.43 | 0.08 | 230 | 12 | 16.2 | | | | | 16.2 | 2.72 | 3.41 | 5.72 | 0.2 | 0.5 | 2.5 |

Side road Ditch along Allen Ranch Rd.

| Project Description | |
|-----------------------|---------------------|
| Friction Method | Manning Formula |
| Solve For | Normal Depth |
| Input Data | |
| Roughness Coefficient | 0.035 |
| Channel Slope | 0.040 ft/ft |
| Left Side Slope | 5.000 H:V |
| Right Side Slope | 3.000 H:V |
| Discharge | 12.00 cfs |
| Results | |
| Normal Depth | 9.7 in |
| Flow Area | 2.6 ft ² |
| Wetted Perimeter | 6.7 ft |
| Hydraulic Radius | 4.7 in |
| Top Width | 6.49 ft |
| Critical Depth | 10.7 in |
| Critical Slope | 0.024 ft/ft |
| Velocity | 4.55 ft/s |
| Velocity Head | 0.32 ft |
| Specific Energy | 1.13 ft |
| Froude Number | 1.260 |
| Flow Type | Supercritical |
| GVF Input Data | |
| Downstream Depth | 0.0 in |
| Length | 0.0 ft |
| Number Of Steps | 0 |
| GVF Output Data | |
| Upstream Depth | 0.0 in |
| Profile Description | N/A |
| Profile Headloss | 0.00 ft |
| Downstream Velocity | Infinity ft/s |
| Upstream Velocity | Infinity ft/s |
| Normal Depth | 9.7 in |
| Critical Depth | 10.7 in |
| Channel Slope | 0.040 ft/ft |
| Critical Slope | 0.024 ft/ft |

Culvert Report

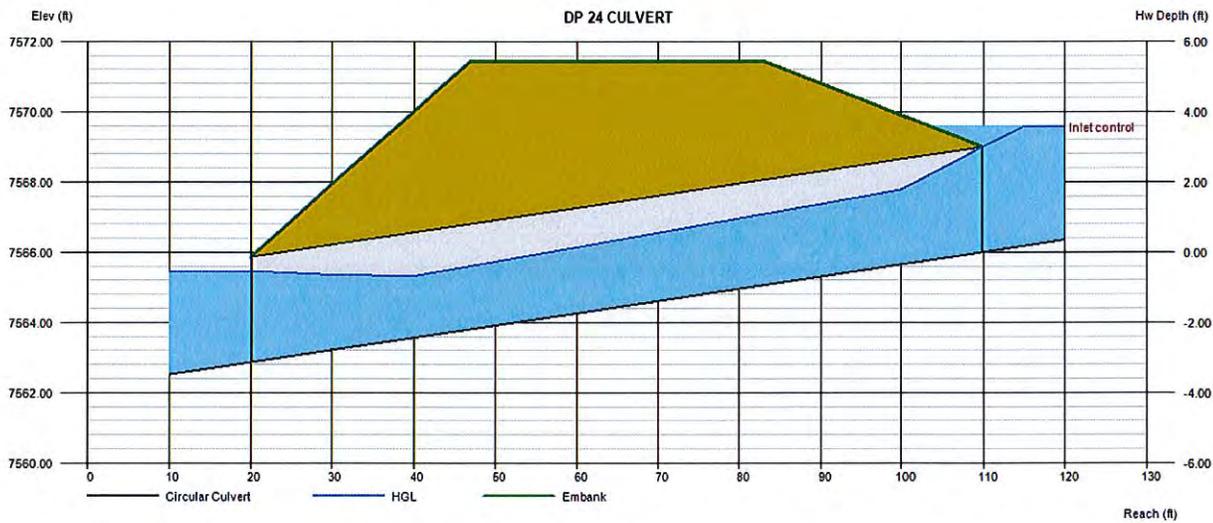
DP 24 CULVERT

Invert Elev Dn (ft) = 7562.87
 Pipe Length (ft) = 89.81
 Slope (%) = 3.49
 Invert Elev Up (ft) = 7566.00
 Rise (in) = 36.0
 Shape = Circular
 Span (in) = 36.0
 No. Barrels = 1
 n-Value = 0.012
 Culvert Type = Circular Concrete
 Culvert Entrance = Square edge w/headwall (C)
 Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment
 Top Elevation (ft) = 7571.44
 Top Width (ft) = 36.00
 Crest Width (ft) = 40.00

Calculations
 Qmin (cfs) = 0.00
 Qmax (cfs) = 45.00
 Tailwater Elev (ft) = (dc+D)/2

Highlighted
 Qtotal (cfs) = 45.00
 Qpipe (cfs) = 45.00
 Qovertop (cfs) = 0.00
 Veloc Dn (ft/s) = 6.93
 Veloc Up (ft/s) = 8.16
 HGL Dn (ft) = 7565.46
 HGL Up (ft) = 7568.18
 Hw Elev (ft) = 7569.57
 Hw/D (ft) = 1.19
 Flow Regime = Inlet Control



Description

A sediment basin is a temporary pond built on a construction site to capture eroded or disturbed soil transported in storm runoff prior to discharge from the site. Sediment basins are designed to capture site runoff and slowly release it to allow time for settling of sediment prior to discharge. Sediment basins are often constructed in locations that will later be modified to serve as post-construction stormwater basins.



Photograph SB-1. Sediment basin at the toe of a slope. Photo courtesy of WWE.

Appropriate Uses

Most large construction sites (typically greater than 2 acres) will require one or more sediment basins for effective management of construction site runoff. On linear construction projects, sediment basins may be impractical; instead, sediment traps or other combinations of BMPs may be more appropriate.

Sediment basins should not be used as stand-alone sediment controls. Erosion and other sediment controls should also be implemented upstream.

When feasible, the sediment basin should be installed in the same location where a permanent post-construction detention pond will be located.

Design and Installation

The design procedure for a sediment basin includes these steps:

- **Basin Storage Volume:** Provide a storage volume of at least 3,600 cubic feet per acre of drainage area. To the extent practical, undisturbed and/or off-site areas should be diverted around sediment basins to prevent “clean” runoff from mixing with runoff from disturbed areas. For undisturbed areas (both on-site and off-site) that cannot be diverted around the sediment basin, provide a minimum of 500 ft³/acre of storage for undeveloped (but stable) off-site areas in addition to the 3,600 ft³/acre for disturbed areas. For stable, developed areas that cannot be diverted around the sediment basin, storage volume requirements are summarized in Table SB-1.
- **Basin Geometry:** Design basin with a minimum length-to-width ratio of 2:1 (L:W). If this cannot be achieved because of site space constraints, baffling may be required to extend the effective distance between the inflow point(s) and the outlet to minimize short-circuiting.
- **Dam Embankment:** It is recommended that embankment slopes be 4:1 (H:V) or flatter and no steeper than 3:1 (H:V) in any location.

| Sediment Basins | |
|--------------------------|-----|
| Functions | |
| Erosion Control | No |
| Sediment Control | Yes |
| Site/Material Management | No |

- **Inflow Structure:** For concentrated flow entering the basin, provide energy dissipation at the point of inflow.

Table SB-1. Additional Volume Requirements for Undisturbed and Developed Tributary Areas Draining through Sediment Basins

| Imperviousness (%) | Additional Storage Volume (ft ³) Per Acre of Tributary Area |
|--------------------|--|
| Undeveloped | 500 |
| 10 | 800 |
| 20 | 1230 |
| 30 | 1600 |
| 40 | 2030 |
| 50 | 2470 |
| 60 | 2980 |
| 70 | 3560 |
| 80 | 4360 |
| 90 | 5300 |
| 100 | 6460 |

- **Outlet Works:** The outlet pipe shall extend through the embankment at a minimum slope of 0.5 percent. Outlet works can be designed using one of the following approaches:
 - **Riser Pipe (Simplified Detail):** Detail SB-1 provides a simplified design for basins treating no more than 15 acres.
 - **Orifice Plate or Riser Pipe:** Follow the design criteria for Full Spectrum Detention outlets in the EDB Fact Sheet provided in Chapter 4 of this manual for sizing of outlet perforations with an emptying time of approximately 72 hours. In lieu of the trash rack, pack uniformly sized 1½ - to 2-inch gravel in front of the plate or surrounding the riser pipe. This gravel will need to be cleaned out frequently during the construction period as sediment accumulates within it. The gravel pack will need to be removed and disposed of following construction to reclaim the basin for use as a permanent detention facility. If the basin will be used as a permanent extended detention basin for the site, a trash rack will need to be installed once contributing drainage areas have been stabilized and the gravel pack and accumulated sediment have been removed.
 - **Floating Skimmer:** If a floating skimmer is used, install it using manufacturer’s recommendations. Illustration SB-1 provides an illustration of a Faircloth Skimmer Floating Outlet™, one of the more commonly used floating skimmer outlets. A skimmer should be designed to release the design volume in no less than 48 hours. The use of a floating skimmer outlet can increase the sediment capture efficiency of a basin significantly. A floating outlet continually decants cleanest water off the surface of the pond and releases cleaner water than would discharge from a perforated riser pipe or plate.

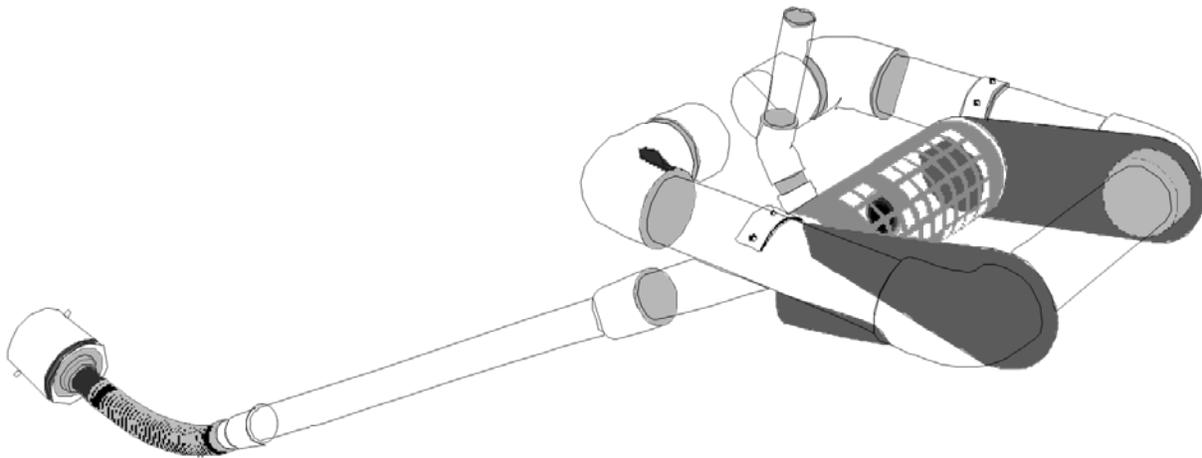


Illustration SB-1. Outlet structure for a temporary sediment basin - Faircloth Skimmer Floating Outlet. Illustration courtesy of J. W. Faircloth & Sons, Inc., FairclothSkimmer.com.

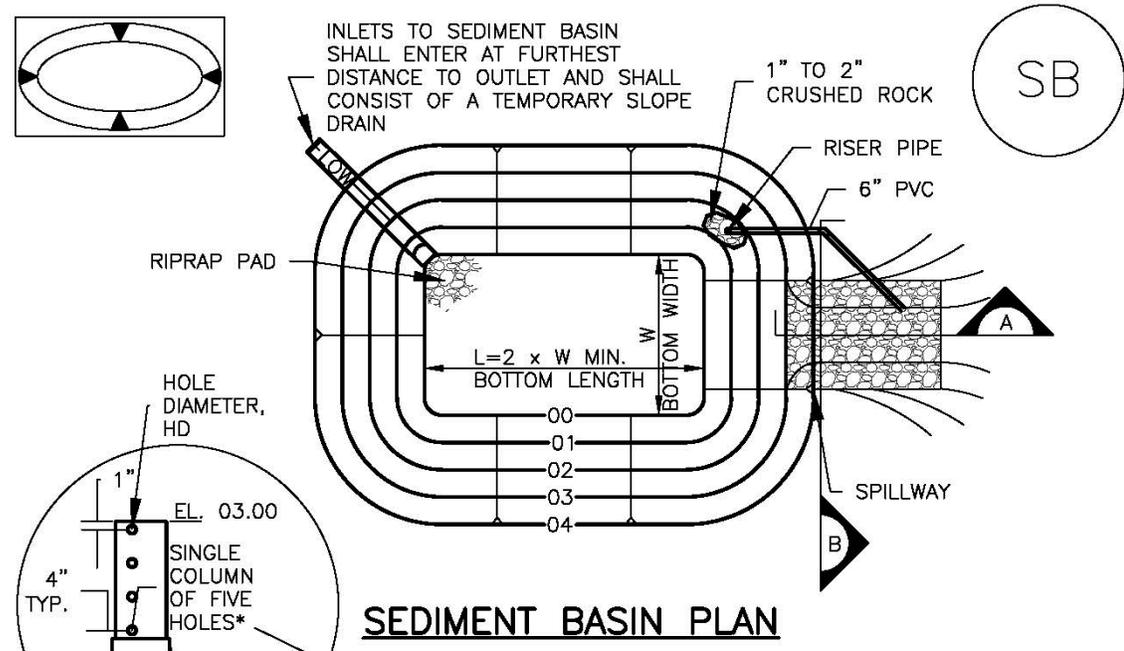
- **Outlet Protection and Spillway:** Consider all flow paths for runoff leaving the basin, including protection at the typical point of discharge as well as overtopping.
 - **Outlet Protection:** Outlet protection should be provided where the velocity of flow will exceed the maximum permissible velocity of the material of the waterway into which discharge occurs. This may require the use of a riprap apron at the outlet location and/or other measures to keep the waterway from eroding.
 - **Emergency Spillway:** Provide a stabilized emergency overflow spillway for rainstorms that exceed the capacity of the sediment basin volume and its outlet. Protect basin embankments from erosion and overtopping. If the sediment basin will be converted to a permanent detention basin, design and construct the emergency spillway(s) as required for the permanent facility. If the sediment basin will not become a permanent detention basin, it may be possible to substitute a heavy polyvinyl membrane or properly bedded rock cover to line the spillway and downstream embankment, depending on the height, slope, and width of the embankments.

Maintenance and Removal

Maintenance activities include the following:

- Dredge sediment from the basin, as needed to maintain BMP effectiveness, typically when the design storage volume is no more than one-third filled with sediment.
- Inspect the sediment basin embankments for stability and seepage.
- Inspect the inlet and outlet of the basin, repair damage, and remove debris. Remove, clean and replace the gravel around the outlet on a regular basis to remove the accumulated sediment within it and keep the outlet functioning.
- Be aware that removal of a sediment basin may require dewatering and associated permit requirements.
- Do not remove a sediment basin until the upstream area has been stabilized with vegetation.

Final disposition of the sediment basin depends on whether the basin will be converted to a permanent post-construction stormwater basin or whether the basin area will be returned to grade. For basins being converted to permanent detention basins, remove accumulated sediment and reconfigure the basin and outlet to meet the requirements of the final design for the detention facility. If the sediment basin is not to be used as a permanent detention facility, fill the excavated area with soil and stabilize with vegetation.



*EXCEPT WHERE THE HOLES EXCEED 1" DIAMETER, THEN UP TO TWO COLUMNS OF SAME SIZED HOLES MAY BE USED

| TABLE SB-1. SIZING INFORMATION FOR STANDARD SEDIMENT BASIN | | | |
|--|------------------------------|----------------------------------|--------------------------|
| Upstream Drainage Area (rounded to nearest acre), (ac) | Basin Bottom Width (W), (ft) | Spillway Crest Length (CL), (ft) | Hole Diameter (HD), (in) |
| 1 | 12 1/2 | 2 | 9/32 |
| 2 | 21 | 3 | 13/16 |
| 3 | 28 | 5 | 1/2 |
| 4 | 33 1/2 | 6 | 9/16 |
| 5 | 38 1/2 | 8 | 2 1/32 |
| 6 | 43 | 9 | 2 1/32 |
| 7 | 47 1/4 | 11 | 2 5/32 |
| 8 | 51 | 12 | 2 7/32 |
| 9 | 55 | 13 | 7/8 |
| 10 | 58 1/4 | 15 | 1 5/16 |
| 11 | 61 | 16 | 3 1/32 |
| 12 | 64 | 18 | 1 |
| 13 | 67 1/2 | 19 | 1 1/16 |
| 14 | 70 1/2 | 21 | 1 1/8 |
| 15 | 73 1/4 | 22 | 1 3/16 |

SEDIMENT BASIN INSTALLATION NOTES

1. SEE PLAN VIEW FOR:
 - LOCATION OF SEDIMENT BASIN.
 - TYPE OF BASIN (STANDARD BASIN OR NONSTANDARD BASIN).
 - FOR STANDARD BASIN, BOTTOM WIDTH W, CREST LENGTH CL, AND HOLE DIAMETER, HD.
 - FOR NONSTANDARD BASIN, SEE CONSTRUCTION DRAWINGS FOR DESIGN OF BASIN INCLUDING RISER HEIGHT H, NUMBER OF COLUMNS N, HOLE DIAMETER HD AND PIPE DIAMETER D.
2. FOR STANDARD BASIN, BOTTOM DIMENSION MAY BE MODIFIED AS LONG AS BOTTOM AREA IS NOT REDUCED.
3. SEDIMENT BASINS SHALL BE INSTALLED PRIOR TO ANY OTHER LAND-DISTURBING ACTIVITY THAT RELIES ON ON BASINS AS AS A STORMWATER CONTROL.
4. EMBANKMENT MATERIAL SHALL CONSIST OF SOIL FREE OF DEBRIS, ORGANIC MATERIAL, AND ROCKS OR CONCRETE GREATER THAN 3 INCHES AND SHALL HAVE A MINIMUM OF 15 PERCENT BY WEIGHT PASSING THE NO. 200 SIEVE.
5. EMBANKMENT MATERIAL SHALL BE COMPACTED TO AT LEAST 95 PERCENT OF MAXIMUM DENSITY IN ACCORDANCE WITH ASTM D698.
6. PIPE SCH 40 OR GREATER SHALL BE USED.
7. THE DETAILS SHOWN ON THESE SHEETS PERTAIN TO STANDARD SEDIMENT BASIN(S) FOR DRAINAGE AREAS LESS THAN 15 ACRES. SEE CONSTRUCTION DRAWINGS FOR EMBANKMENT, STORAGE VOLUME, SPILLWAY, OUTLET, AND OUTLET PROTECTION DETAILS FOR ANY SEDIMENT BASIN(S) THAT HAVE BEEN INDIVIDUALLY DESIGNED FOR DRAINAGE AREAS LARGER THAN 15 ACRES.

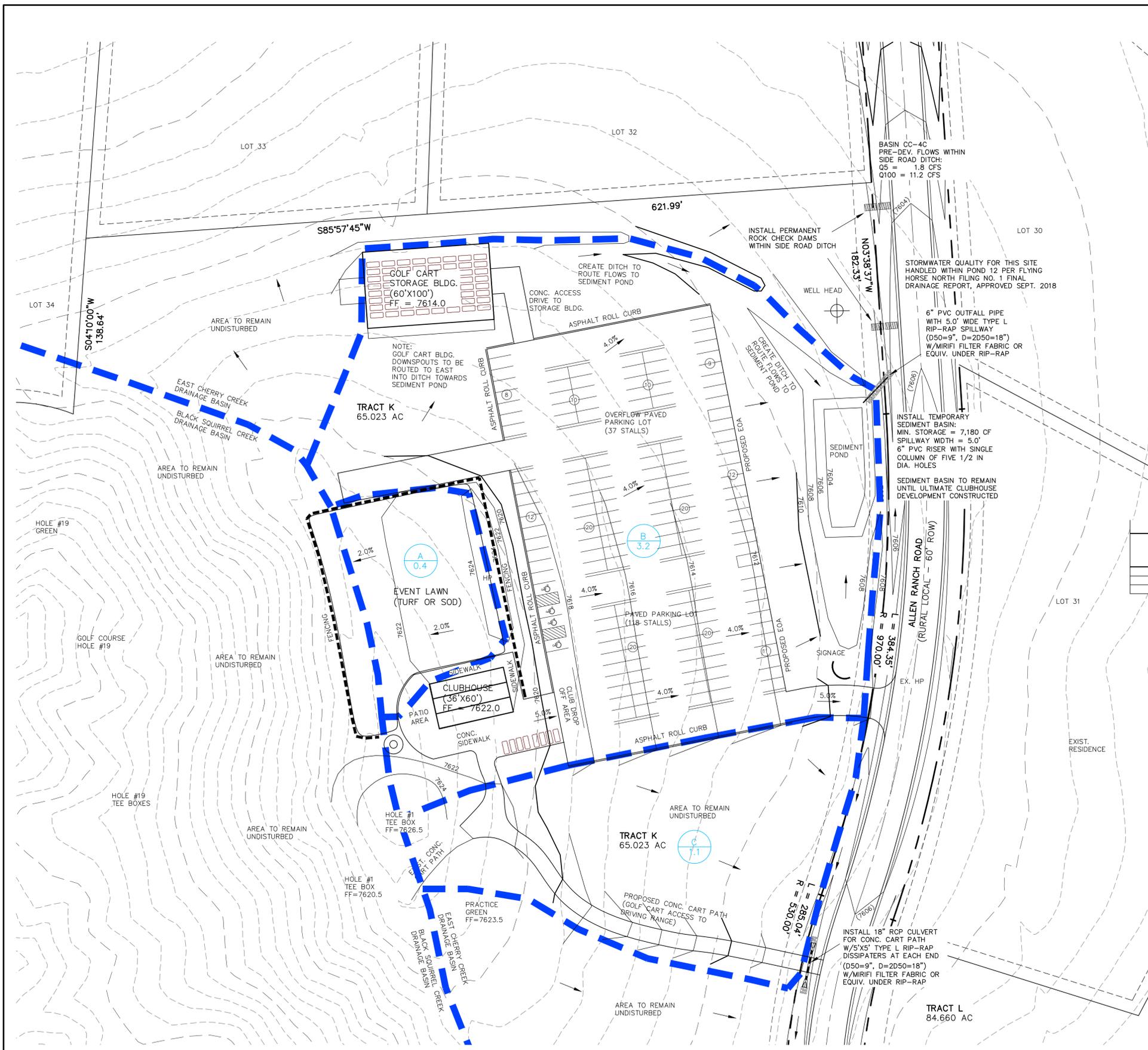
SEDIMENT BASIN MAINTENANCE NOTES

1. INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
4. SEDIMENT ACCUMULATED IN BASIN SHALL BE REMOVED AS NEEDED TO MAINTAIN BMP EFFECTIVENESS, TYPICALLY WHEN SEDIMENT DEPTH REACHES ONE FOOT (I.E., TWO FEET BELOW THE SPILLWAY CREST).
5. SEDIMENT BASINS ARE TO REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS STABILIZED AND GRASS COVER IS ACCEPTED BY THE LOCAL JURISDICTION.
6. WHEN SEDIMENT BASINS ARE REMOVED, ALL DISTURBED AREAS SHALL BE COVERED WITH TOPSOIL, SEEDED AND MULCHED OR OTHERWISE STABILIZED AS APPROVED BY LOCAL JURISDICTION.

(DETAILS ADAPTED FROM DOUGLAS COUNTY, COLORADO)

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

DRAINAGE MAP



TIME OF CONCENTRATION - DEVELOPED

| BASIN | COMPOSITE Cn | C(5) | Length (ft) | OVERLAND Height (ft) | Tc (min) | STREET / CHANNEL FLOW (DCM Vol 1 Fig 6-25) Length (ft) | Slope (%) | Velocity (fps) | Tc (min) | Tc TOTAL (min) | Tc LAG (0.6hr) (min) | Tc LAG (0.6hr) (hr) |
|------------------|--------------|------|-------------|----------------------|----------|--|-----------|----------------|----------|----------------|----------------------|---------------------|
| CC-4C (Pre-Dev.) | 61.0 | 0.08 | 40 | 0.8 | 9.3 | 350 | 3.0% | 1.5 | 3.9 | 13.2 | 7.9 | 0.13 |

BASIN SUMMARY - DEVELOPED CONDITIONS

| BASIN (label) | AREA (acres) | COMPOSITE CN | TOTAL LAG TIME (hours) | Q 2 Yr. (cfs) | Q 5 Yr. (cfs) | Q 100 Yr. (cfs) |
|------------------|--------------|--------------|------------------------|---------------|---------------|-----------------|
| CC-4C (Pre-Dev.) | 7.40 | 61.0 | 0.13 | 0.2 | 1.8 | 11.2 |

REFERENCE TAKEN FROM PRELIMINARY DRAINAGE REPORT FOR FLYING HORSE NORTH PRELIMINARY PLAN AND FINAL DRAINAGE REPORT FOR FLYING HORSE NORTH FILING NO. 1, APPROVED 9/25/18.
SEE APPENDIX FOR PREVIOUS DRAINAGE MAP FOR BASIN CC-4C

FINAL DRAINAGE REPORT - BASIN RUNOFF COEFFICIENT SUMMARY

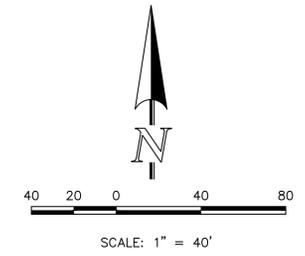
| BASIN | TOTAL AREA (AC) | IMPERVIOUS AREA / DRIVEWAYS | | | | LANDSCAPE/UNDEVELOPED AREAS | | | | WEIGHTED | | | WEIGHTED CA | | |
|-------|-----------------|-----------------------------|------|------|--------|-----------------------------|------|------|--------|----------|------|--------|-------------|-------|---------|
| | | AREA (AC) | C(2) | C(5) | C(100) | AREA (AC) | C(2) | C(5) | C(100) | C(2) | C(5) | C(100) | CA(2) | CA(5) | CA(100) |
| A | 0.4 | 0.04 | 0.89 | 0.90 | 0.96 | 0.36 | 0.02 | 0.08 | 0.35 | 0.11 | 0.16 | 0.41 | 0.04 | 0.06 | 0.16 |
| B | 3.2 | 1.80 | 0.89 | 0.90 | 0.96 | 1.40 | 0.02 | 0.08 | 0.35 | 0.51 | 0.54 | 0.69 | 1.63 | 1.73 | 2.22 |
| C | 1.1 | 0.08 | 0.89 | 0.90 | 0.96 | 1.02 | 0.02 | 0.08 | 0.35 | 0.08 | 0.14 | 0.39 | 0.09 | 0.15 | 0.43 |

FINAL DRAINAGE REPORT - BASIN RUNOFF SUMMARY

| BASIN | TOTAL AREA (AC) | WEIGHTED | | | | OVERLAND | | | STREET / CHANNEL FLOW | | | Tc (min) | INTENSITY (in/hr) | TOTAL FLOWS | | | | | |
|-------|-----------------|----------|-------|---------|------|-------------|-------------|----------|-----------------------|-----------|----------------|----------|-------------------|-------------|------------|------------|--------------|-----|------|
| | | CA(2) | CA(5) | CA(100) | C(5) | Length (ft) | Height (ft) | Tc (min) | Length (ft) | Slope (%) | Velocity (fps) | | | Tc (min) | Q(2) (cfs) | Q(5) (cfs) | Q(100) (cfs) | | |
| A | 0.4 | 0.04 | 0.06 | 0.16 | 0.08 | 120 | 3 | 14.9 | | | | 14.9 | 2.82 | 3.53 | 5.93 | 0.1 | 0.2 | 1.0 | |
| B | 3.2 | 1.63 | 1.73 | 2.22 | 0.08 | 100 | 2 | 14.7 | 250 | 4.0% | 1.0 | 4.2 | 18.8 | 2.54 | 3.18 | 5.34 | 4.1 | 5.5 | 11.8 |
| C | 1.1 | 0.09 | 0.15 | 0.43 | 0.08 | 230 | 12 | 16.2 | | | | 16.2 | 2.72 | 3.41 | 5.72 | 0.2 | 0.5 | 2.5 | |

LEGEND

| DESCRIPTION | SYMBOL |
|-------------------------|--------|
| EXISTING GROUND CONTOUR | 6910 |
| BASIN BOUNDARY | |
| PROPERTY BOUNDARY | |
| PROPOSED LOT LINE | |
| BASIN IDENTIFIER | |
| AREA IN ACRES | |



48 HOURS BEFORE YOU DIG,
CALL UTILITY LOCATORS
811
UTILITY NOTIFICATION CENTER OF COLORADO
IT'S THE LAW

THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE CAUSED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

| NO. | REVISION | DATE |
|-----|----------|------|
| | | |
| | | |
| | | |

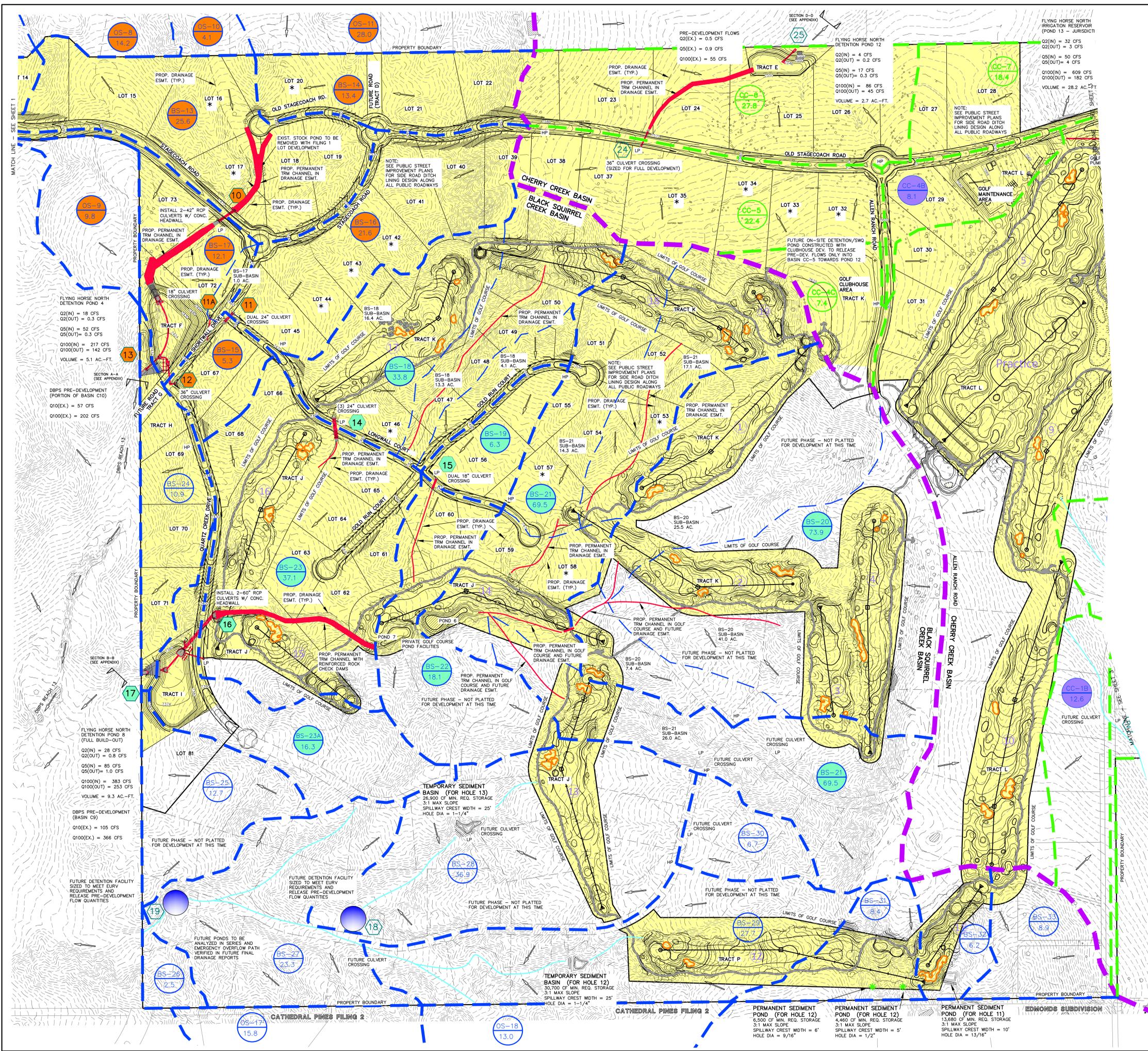
REVIEW:
PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF CLASSIC CONSULTING ENGINEERS AND SURVEYORS, LLC

MARC A. WHORTON, COLORADO P.E. #37155 DATE

619 N. Cascade Avenue, Suite 200
Colorado Springs, Colorado 80903
(719) 785-0790
(719) 785-0799 (Fax)

FLYING HORSE NORTH GOLF COURSE
TEMPORARY CLUBHOUSE / EVENT LAWN
DRAINAGE MAP

| | | | | |
|-------------|--------------|--------------|---------|---------|
| DESIGNED BY | MAW | SCALE | DATE | 1/15/21 |
| DRAWN BY | MAW | (H) 1" = 40' | SHEET | 1 OF 3 |
| CHECKED BY | (V) 1" = N/A | JOB NO. | 1096.15 | |



BASIN SUMMARY - DEVELOPED CONDITIONS

| BASIN (label) | AREA (acres) | COMPOSITE CN | LAG TIME (hours) | Q 2 Yr. (cfs) | Q 5 Yr. (cfs) | Q 100 Yr. (cfs) |
|-----------------|--------------|--------------|------------------|---------------|---------------|-----------------|
| OS-8 | 14.20 | 65.0 | 0.27 | 2.1 | 6.2 | 24.7 |
| OS-9 | 9.80 | 60.0 | 0.37 | 0.1 | 1.0 | 9.1 |
| OS-10 | 4.10 | 65.0 | 0.17 | 0.7 | 2.1 | 8.2 |
| OS-11 | 28.00 | 65.0 | 0.35 | 2.4 | 8.2 | 38.7 |
| OS-12 | 68.10 | 62.7 | 0.37 | 2.2 | 11.9 | 78.8 |
| OS-13 | 38.90 | 65.0 | 0.33 | 1.4 | 7.4 | 45.0 |
| OS-14 | 28.40 | 62.0 | 0.31 | 0.7 | 4.6 | 31.0 |
| OS-15 | 70.80 | 63.9 | 0.38 | 3.3 | 14.8 | 84.2 |
| OS-16 | 4.50 | 65.0 | 0.24 | 0.4 | 1.5 | 7.2 |
| OS-17 | 15.80 | 65.0 | 0.19 | 1.6 | 5.9 | 27.7 |
| OS-18 | 13.00 | 65.0 | 0.20 | 1.3 | 4.7 | 22.6 |
| BS-13 | 25.60 | 65.0 | 0.23 | 3.7 | 10.2 | 40.7 |
| BS-14 | 13.40 | 65.0 | 0.23 | 2.6 | 6.8 | 26.5 |
| BS-15 | 5.30 | 65.0 | 0.18 | 1.6 | 3.7 | 12.2 |
| BS-16 | 21.50 | 65.0 | 0.34 | 4.6 | 11.8 | 44.1 |
| BS-17 | 12.10 | 65.0 | 0.21 | 3.1 | 7.7 | 28.7 |
| BS-18 | 33.80 | 63.6 | 0.41 | 3.5 | 12.4 | 56.0 |
| BS-19 | 6.30 | 65.0 | 0.18 | 2.1 | 4.6 | 15.0 |
| BS-20 | 73.90 | 63.4 | 0.31 | 7.4 | 24.6 | 112.4 |
| BS-21 | 69.50 | 64.3 | 0.35 | 7.8 | 23.9 | 103.0 |
| BS-22 | 18.10 | 64.4 | 0.22 | 3.7 | 9.6 | 36.5 |
| BS-23 | 37.10 | 63.3 | 0.33 | 4.5 | 13.6 | 58.2 |
| BS-24 | 18.30 | 64.4 | 0.29 | 5.5 | 12.0 | 38.3 |
| BS-25 | 19.90 | 63.0 | 0.17 | 0.6 | 3.3 | 17.6 |
| EX-24 (Pre-Dev) | 13.20 | 60.0 | 0.17 | 0.2 | 2.2 | 17.8 |
| BS-25 | 12.70 | 63.0 | 0.23 | 0.4 | 2.7 | 17.3 |
| BS-26 | 2.90 | 65.0 | 0.18 | 0.0 | 0.4 | 3.4 |
| BS-27 | 23.30 | 65.0 | 0.22 | 2.1 | 8.0 | 38.8 |
| BS-28 | 38.90 | 64.4 | 0.32 | 2.2 | 9.3 | 49.4 |
| BS-29 | 27.70 | 64.0 | 0.33 | 1.4 | 6.5 | 36.9 |
| BS-30 | 6.70 | 65.0 | 0.20 | 0.7 | 2.4 | 11.7 |
| BS-31 | 8.40 | 63.1 | 0.43 | 1.8 | 1.9 | 11.8 |
| BS-32 | 6.20 | 62.6 | 0.20 | 0.3 | 1.6 | 9.4 |
| BS-33 | 8.90 | 64.7 | 0.19 | 0.8 | 3.2 | 15.3 |
| CC-1A | 9.80 | 65.0 | 0.23 | 0.8 | 3.3 | 16.0 |
| CC-1B | 12.80 | 64.8 | 0.28 | 1.0 | 4.0 | 19.4 |
| CC-2A | 11.00 | 65.0 | 0.22 | 1.0 | 3.8 | 18.3 |
| CC-2B | 20.80 | 65.0 | 0.22 | 1.9 | 7.1 | 34.6 |
| CC-2C | 6.40 | 65.0 | 0.18 | 0.7 | 2.5 | 11.5 |
| CC-3 | 52.50 | 63.1 | 0.43 | 1.8 | 8.8 | 54.5 |
| CC-4A | 108.70 | 62.6 | 0.44 | 15.4 | 39.0 | 156.0 |
| CC-4B | 8.10 | 76.1 | 0.26 | 4.0 | 7.3 | 20.6 |
| CC-4C (Pre-Dev) | 7.40 | 61.0 | 0.13 | 0.2 | 1.8 | 11.2 |
| CC-5 | 22.40 | 65.0 | 0.26 | 1.8 | 7.1 | 34.3 |
| CC-6 | 72.80 | 65.0 | 0.25 | 2.3 | 9.1 | 43.2 |
| CC-7 | 18.40 | 65.0 | 0.29 | 1.4 | 5.4 | 27.0 |

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

| Design Point (label) | Contributing Basins | Q 2 Yr. (cfs) | Q 5 Yr. (cfs) | Q 100 Yr. (cfs) |
|--|---|---------------|---------------|-----------------|
| DP-10 DEV | OS-8, OS-10, OS-11, BS-13, BS-14 | 10.7 | 32.0 | 143 |
| DP-11 DEV | BS-16 | 4.6 | 11.8 | 36 |
| DP-12 DEV | DP-11, 1.0 Ac. Portion of BS-17 and BS-15 | 4.2 | 11.8 | 46 |
| TOTAL INFLOW TO POND 4 (UD Detention hydrograph) | DP-10, DP-12, BS-17, OS-9 | 10 | 16 | 217 |
| DP-13 DEV | Release from FHN Pond 4 | 0.3 | 0.3 | 142 |
| DP-14 DEV | BS-18 | 3.5 | 12.4 | 56 |
| DP-15 DEV | BS-19 | 2.1 | 4.6 | 15 |
| DP-16 DEV | DP-14, DP-15, BS-20, BS-21, BS-22, BS-23 | 25.0 | 78.0 | 362 |
| TOTAL INFLOW TO FHN POND 8 (Full Build-out) (UD Detention hydrograph) | DP-10, DP-12, BS-17, OS-9 | 24 | 37 | 390 |
| DP-17 DEV (Full Build-out) | Release from FHN Pond 8 | 0.8 | 1.0 | 253 |
| TOTAL INFLOW TO FHN POND 8 (Filling 1 Only) (UD Detention hydrograph) | DP-10, DP-12, BS-17, OS-9 | 9 | 14 | 301 |
| DP-17 DEV (Filling 1 Only) | Release from FHN Pond 8 | 0.4 | 0.5 | 219 |
| DP-18 DEV | BS-28, BS-29, BS-30, OS-18 | 5.0 | 21.6 | 115 |
| DP-19 DEV | BS-27, OS-17, Release from DP-18 | 3.8 | 16.8 | 126 |
| DP-20 DEV | CC-1A, OS-12 | 3.2 | 14.3 | 88 |
| DP-21 DEV | CC-2A, OS-13 | 2.1 | 10.5 | 62 |
| DP-22 DEV | CC-2B, Release from DP-21 | 3.7 | 16.6 | 92 |
| DP-23 DEV | CC-3, OS-14 | 2.5 | 13.0 | 84 |
| DP-24 DEV | CC-4C (Pre-Dev), CC-5 | 1.9 | 8.4 | 45 |
| TOTAL INFLOW TO POND 12 (UD Detention hydrograph) | CC-4C, CC-5, CC-6 | 6 | 9 | 85 |
| DP-25 DEV | Release from FHN Pond 12 | 0.2 | 0.3 | 45 |

LEGEND

| DESCRIPTION | SYMBOL |
|-------------------------------------|---------|
| EXISTING GROUND CONTOUR | 6910 |
| PROPOSED FINISHED CONTOUR | 6910 |
| BASIN BOUNDARY EAST CHERRY CREEK | --- |
| MAJOR BASIN BOUNDARY | --- |
| BASIN BOUNDARY BLACK SQUIRREL | --- |
| DESIGN POINT | 3 |
| LOTS WITH NON-STANDARD CULVERT SIZE | * |
| BASIN IDENTIFIER | BB 10.0 |
| EXISTING DIRECTION OF FLOW | → |
| PROPOSED DIRECTION OF FLOW | → |
| STORM SEWER | --- |
| FILING NO. 1 PLAT AREA | --- |

SCALE: 1" = 200'

CLASSIC CONSULTING ENGINEERS & SURVEYORS

FLYING HORSE NORTH PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 DRAINAGE MAP

DESIGNED BY: MAW SCALE: DATE: 10-25-17
 DRAIN BY: MAW (H) 1" = 200' SHEET 2 OF 4
 CHECKED BY: (V) 1" = N/A JOB NO. 1096.11

619 N. Cascade Avenue, Suite 200 (719)785-0790
 Colorado Springs, Colorado 80903 (719)785-0799 (Fax)