

DRAINAGE LETTER REPORT

for

INTELIFAB

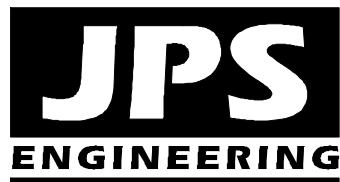
LOT 1, MAYBERRY, COLORADO SPRINGS FILING NO. 2

Prepared for:

Hammers Construction Inc.
1411 Woolsey Heights
Colorado Springs, CO 80915

March 30, 2020

Prepared by:



**19 E. Willamette Ave.
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**JPS Project No. 030502
PCD File No. PPR-20__**

INTELIFAB
LOT 1, MAYBERRY, COLORADO SPRINGS FILING NO. 2
DRAINAGE REPORT STATEMENTS

1. Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan for the drainage basin. I accept responsibility for liability caused by negligent acts, errors or omissions on my part in preparing this report:

John P. Schwab Colorado P.E. No. 29891

2. Developer's Statement:

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

By:

Printed Name:
Title:

3. El Paso County Statement:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2, and Engineering Criteria Manual as amended.

Jennifer Irvine, P.E.
County Engineer / ECM Administrator

Conditions:

I. INTRODUCTION

A. Property Location and Description

The Intelifab manufacturing facility is a proposed light industrial building to be constructed in the Mayberry, Colorado Springs (formerly known as “Ellicott Town Center”) Master Plan area of eastern El Paso County, Colorado. The 1.5-acre property is described as Lot 1, Mayberry, Colorado Springs Filing No. 2. Intelifab manufactures framing components used in the building industry. Lot 1 will be created upon recording of the subdivision plat for Mayberry, Colorado Springs Filing No. 2, which is a replat of tracts within Mayberry, Colorado Springs Filing No. 1.

The project site is located at the northwest corner of Springs Road and Cattlemen Run. A re-zoning is currently in process to establish the zoning of this property and adjoining property to the east as CS (Commercial Service).

State Highway 94 (SH94) adjoins the north boundary of this property, and the west boundary of the site adjoins an existing water storage tank parcel owned by Cherokee Metropolitan District. The south and east boundaries of the property adjoin undeveloped parts of the Mayberry, Colorado Springs (fka “Ellicott Town Center”) Master Plan area. Upon recording of Mayberry, Colorado Springs Filing No. 1, this property will be located within Tract L of Filing No. 1, and the Filing No. 2 replat is currently in process, which will establish the subject Lot 1 as a buildable lot.

The Intelifab project consists of a proposed 23,284 square-foot manufacturing building with associated parking and site improvements. Access will be provided by a driveway entrance onto Springs Road at the east boundary of the site, and an additional driveway entrance onto Cattlemen Run along the south boundary of the site.

B. Scope

In support of the El Paso County Site Development Plan submittal for this project, this report is intended to meet the requirements of a site-specific “Letter Type” drainage report in accordance with El Paso County drainage criteria. This report will provide a summary of site drainage issues impacting the proposed development. The report will analyze impacts from upstream drainage patterns, site-specific developed drainage patterns, and impacts on downstream facilities. This report is based on the guidelines and criteria presented in the City of Colorado Springs and El Paso County “Drainage Criteria Manual.”

C. References

City of Colorado Springs & El Paso County “Drainage Criteria Manual,” revised October 12, 1994, Volumes 1 and 2.

JPS Engineering, Inc., "Final Drainage Report for Mayberry, Colorado Springs (fka "Ellicott Town Center") Filing No. 2," revised March 30, 2020.

JPS Engineering, Inc., "Final Drainage Report for Mayberry, Colorado Springs (fka "Ellicott Town Center") Filing No. 1," revised March 30, 2020.

II. EXISTING / PROPOSED DRAINAGE CONDITIONS

Drainage planning for this lot has been addressed in the "Final Drainage Report for Mayberry, Colorado Springs Filing No. 1" and the "Final Drainage Report for Mayberry, Colorado Springs Filing No. 2" by JPS Engineering. As noted in the subdivision drainage reports, on-site soils are comprised of Truckton loamy sand soils. These soils are classified as hydrologic soils group A (rapid permeability). The existing topography within this site slopes downward to the southeast with a grade of approximately 1-3 percent.

The subject property, Lot 1, has identified as Basin C2.7 in the subdivision drainage reports, and drainage from this lot has been planned to sheet flow in a southeasterly direction to the curb and gutter along the west side of Springs Road. The subdivision drainage report identifies peak flows of $Q_5 = 5.2 \text{ cfs}$ and $Q_{100} = 10.3 \text{ cfs}$ for Basin C2.7. Developed flows in the curb and gutter along the west side of Springs Road flow south to Design Point #C2.8A at the corner of Springs Road and Village Main Street. Developed peak flows at Design Point #C2.8A are calculated as $Q_5 = 13.4 \text{ cfs}$ and $Q_{100} = 28.9 \text{ cfs}$.

Surface runoff from the developed site will continue to follow historic drainage patterns towards the south property boundary. The proposed building pad will be graded with protective slopes to provide positive drainage away from the face of the building. A high point will be graded in the northwest corner of the site, and drainage swales will be graded along the north and west faces of the building to convey developed flows around the building. Curb and gutter will be installed along the outer perimeter of the new parking areas to convey surface drainage to curb chases in the adjoining public streets, ultimately directing developed flows from this site to the curb and gutter along the west side of Springs Road.

Stormwater quality mitigation and detention will be provided by routing developed flows through the subdivision detention pond south of the property. Temporary Detention Pond C2.8 will be constructed at the northwest corner of Springs Road and Village Main Street, and this pond has been sized to mitigate developed drainage impacts from Basins C2.6, C2.7, and C2.8 during the initial phase of development. In conjunction with future development of Filing No. 4 southeast of this site, the larger sub-regional Detention Pond D will be constructed further to the southeast, after which temporary Detention Pond C2.8 will be removed.

The subdivision drainage report assumed full commercial / light industrial development of this site, and the proposed site development plan is entirely consistent with the approved subdivision drainage plan. The calculated impervious area of the Lot 1 site development is 80 percent, which matches the assumptions in the subdivision drainage reports (revised March, 2020). The proposed Site Development Plan for the Intelifab building project includes landscaped areas around the perimeter of the site, as well as a 40-foot right-of-way dedication along SH94, so the total impervious area remains below the maximum impervious area anticipated in the subdivision drainage report.

Hydrologic calculations are detailed in the attached spreadsheet (Appendix A), and peak flows are identified on Figure D1.12. The contractor will need to implement standard best management practices for erosion control during construction, as depicted on the Site Grading and Erosion Control Plans.

III. DRAINAGE PLANNING FOUR STEP PROCESS

El Paso County Drainage Criteria require drainage planning to include a Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long-term source controls.

As stated in DCM Volume 2, the Four Step Process is applicable to all new and re-development projects with construction activities that disturb 1 acre or greater or that disturb less than 1 acre but are part of a larger common plan of development. The Four Step Process has been implemented as follows in the planning of this project:

Step 1: Employ Runoff Reduction Practices

- Minimize Directly Connected Impervious Areas: Roof drain downspouts will be directed to flow over grass-lined drainage swales where possible. Drainage from the north side of the building will flow through a grass-lined drainage swale prior to reaching the downstream public street.

Step 2: Stabilize Drainageways

- There are no drainageways directly adjacent to this project site.

Step 3: Provide Water Quality Capture Volume (WQCV)

- EDB: The developed site will drain through an off-site Extended Detention Basin (EDB) south of the property. Detention Pond C2.8. Has been designed to provide stormwater detention and water quality for this site. The extended detention basin will capture and slowly discharge the WQCV over an extended release period.

Step 4: Consider Need for Industrial and Commercial BMPs

- The property owner will implement a Stormwater Management Plan (SWMP) which includes proper housekeeping and spill containment procedures.

- Site drainage will be routed through the downstream Extended Detention Basin (EDB) to minimize discharge of contaminants to the downstream drainage system.

IV. FLOODPLAIN IMPACTS

This site is located beyond the limits of any FEMA 100-year floodplain boundaries as shown in the FEMA floodplain map for this area, FIRM Panel No. 08041C0810G, dated December 7, 2018.

V. DRAINAGE BASIN FEES

This site is located within the Ellicott Consolidated Drainage Basin. No public drainage improvements are required for development of this site. The Ellicott Consolidated Drainage Basin does not have a drainage or bridge fee requirement, and any required drainage fees would have been addressed at the subdivision stage, so there are no applicable drainage fees required with the Site Development Plan.

VI. SUMMARY

The developed drainage patterns associated with the proposed Intelifab manufacturing facility will remain consistent with historic conditions and the overall drainage plan for this commercial subdivision. Developed flows from the site will continue to follow historic drainage patterns, flowing to the subdivision detention pond southeast of this site.

The subdivision detention pond will mitigate developed drainage impacts and meet the County's stormwater quality requirements for this site. Proper maintenance of the subdivision detention pond, in conjunction with proper erosion control practices, will ensure that this developed site has no significant adverse impact on downstream or surrounding areas.

**ELICOTT TOWN CENTER
RATIONAL METHOD - HYDROLOGIC CALCULATIONS**

DEVELOPED FLOWS

FILING NO. 1	Overland Flow						Channel flow					
	BASIN	DESIGN AREA (AC)	5-YEAR	100-YEAR	C	CHANNEL CONVEYANCE	SCS (2)	SLOPE VELOCITY (FT/SEC)	T _c (3) (MIN)	T _c (4) (MIN)	TOTAL INTENSITY (5)	PEAK FLOW (6)
					LENGTH COEFFICIENT (FT)	T _{co} (1) (MIN)	(FT/FT)			5-YR (IN/HR)	Q5 (6) (CFS)	
A1A	A1A	2.80	0.355	0.555	40	0.020	6.8	2035	15.00	0.011	1.57	21.6
C1.2	C1.2	7.97	0.490	0.620		0.0	1000	20.00	0.009	1.90	8.8	4.32
C1.7A	C1.7A	0.58	0.375	0.545		0.0	680	20.00	0.013	2.28	5.0	5.17
C1.7B	C1.7B	4.34	0.490	0.620	100	0.020	8.9	400	20.00	0.01	2.00	3.3
C1.7A,C1.7B	C1.7B1	4.92	0.476	0.611								
C1.2,C1.7	C1.2D	12.89	0.485	0.617								
C1.3	C1.3	3.02	0.375	0.545		0.0	280	20.00	0.01	2.00	2.3	5.0
C1.2,C1.3,C1.7	C1.3A	15.91	0.464	0.603								
C1.4	C1.4	3.23	0.375	0.545		0.0	300	20.00	0.01	2.00	2.5	5.0
C1.2-C1.4,C1.7	C1.4A	19.14	0.449	0.593								
C1.5	C1.5	3.18	0.375	0.545		0.0	300	20.00	0.01	2.00	2.5	5.0
C1.2-C1.5,C1.7	C1.5A	22.32	0.438	0.586								
C1.1	C1.1	9.38	0.226	0.447	100	0.017	13.4	1800	20.00	0.01	2.00	15.0
C1.6	C1.6	1.98	0.375	0.545		0.0	280	20.00	0.01	2.00	2.3	5.0
C1.1-C1.6	C1.6B	11.36	0.252	0.464								
C1.1-C1.7	C1.7A	33.68	0.376	0.545								
C1.8	C1.8	3.89	0.375	0.545		0.0	600	20.00	0.016	2.53	4.0	5.0
C1.9	C1.9	3.60	0.375	0.545		0.0	580	20.00	0.012	2.19	4.4	4.4
C1.8,C1.9	C1.9A	7.49	0.375	0.545								
C1.1-C1.9	C1.9B	41.17	0.376	0.545								
FILING NO. 2												
C2.1		5.59	0.242	0.457	100	0.016	13.4	650	20.00	0.01	2.00	5.4
C2.2		4.03	0.375	0.545		0.0	460	20.00	0.01	2.00	3.8	5.0
C2.3		2.76	0.375	0.545		0.0	260	20.00	0.01	2.00	2.2	5.0
C2.1-C2.3		C2.3A	12.38	0.315	0.505							
C2.4			4.98	0.375	0.545		0.0	560	20.00	0.012	2.19	4.3
C2.5			4.12	0.375	0.545		0.0	330	20.00	0.01	2.00	2.8
C2.1-C2.5		C2.5A	21.48	0.341	0.522							

Overland Flow										Channel flow								
BASIN	DESIGN POINT	AREA (AC)	C	CHANNEL CONVEYANCE			SCS (2)	SLOPE VELOCITY (FT/SEC)	Tc (4) (MIN)	Tt (3) (MIN)	TOTAL	INTENSITY (6)			PEAK FLOW			
				5-YEAR	100-YEAR	LENGTH (FT)	SLOPE (FT/FT)	Tco (1) (MIN)	COEFFICIENT C			5-YR (IN/HR)	100-YR (IN/HR)	Q5 (6) (CFS)	Q100 (6) (CFS)			
C4		20.40	0.080	0.350		0.0	1050	15.00	0.011	1.57	11.1	11.1	3.97	6.66	6.48	47.58		
Tc C2.5A TO DP-D2B						2450	15.00	0.01	1.50	27.2								
C2.1-C2.5,C4	C4.1	41.88	0.214	0.438							30.7	30.7	2.44	4.10	21.91	75.25		
C2.6		2.76	0.375	0.545	100	0.020	10.5	550	20.00	0.016	2.53	3.6	14.2	14.2	3.61	6.06	3.73	9.11
C2.7		2.14	0.490	0.620	100	0.020	8.9	400	20.00	0.013	2.28	2.9	11.8	11.8	3.88	6.52	4.07	8.66
C2.8		3.00	0.413	0.579	0.0	250	20.00	0.012	2.19	1.9	1.9	5.0	5.17	8.68	6.40	15.08		
C2.6-C2.8	C2.8A	7.90	0.421	0.578							16.1	16.1	3.42	5.74	11.37	26.20		
D1.2		2.99	0.427	0.579	0.0	300	20.00	0.01	2.00	2.5	2.5	5.0	5.17	8.68	6.60	15.03		
C2.6-C2.8,D1.2	D1.2A	10.89	0.426	0.578							18.6	18.6	3.20	5.37	14.85	33.83		
D1.1		3.60	0.490	0.545	0.0	750	20.00	0.011	2.10	6.0	6.0	6.0	4.91	8.24	8.65	16.16		
D1.3		2.87	0.427	0.579	0.0	280	20.00	0.01	2.00	2.3	2.3	5.0	5.17	8.68	6.33	14.42		
C2.6-C2.8,D1.1-D1.3	D1.3A	17.36	0.439	0.571							20.9	20.9	3.02	5.08	23.04	50.31		
D1.4		4.19	0.375	0.545	0.0	550	20.00	0.012	2.19	4.2	4.2	5.0	5.17	8.68	8.12	19.82		
D1.5		5.09	0.375	0.545	0.0	280	20.00	0.01	2.00	2.3	2.3	5.0	5.17	8.68	9.87	24.08		
D1.6		3.33	0.375	0.545	0.0	1060	20.00	0.01	2.00	8.8	8.8	8.8	4.32	7.25	5.39	13.15		
C2.6-C2.8,D1.1-D1.6	D1.6A	29.97	0.412	0.560							25.1	25.1	2.75	4.62	33.96	77.46		
PHASE 2																		
D2		44.58	0.341	0.523	100	0.020	11.0	1750	20.00	0.011	2.10	13.9	24.9	2.76	4.63	41.94	107.95	
C2.6-C2.8,D1.1-D1.6,D2	D2A	74.55	0.370	0.538							4.3	5.0	5.17	8.68	142.57	348.11		
C2,C4,D	D2B	116.43	0.314	0.502							23.8	23.8	2.83	4.75	103.49	277.70		
C3		74.48	0.375	0.545	100	0.020	10.5	3000	20.00	0.011	2.10	23.8	34.4	2.28	3.82	63.60	155.10	
C2,C2,C4,D	D2C	190.91	0.338	0.519							51.0	51.0	1.69	2.83	108.77	280.15		

$$1) \text{OVERLAND FLOW } T_{co} = (0.395 * (1.1 - \text{RUNOFF COEFFICIENT}) * (\text{OVERLAND FLOW LENGTH}^{(0.5)} / (\text{SLOPE}^{(0.333)})))$$

$$2) \text{SCS VELOCITY} = C * ((\text{SLOPE (FT/FT)})^{0.5})$$

C = 2.5 FOR HEAVY MEADOW

C = 5 FOR TILLAGE/FIELD

C = 7 FOR SHORT PASTURE AND LAWNS

C = 10 FOR NEARLY BARE GROUND

C = 15 FOR GRASSED WATERWAY

C = 20 FOR PAVED AREAS AND SHALLOW PAVED SWALES

$$3) \text{MANNING'S CHANNEL TRAVEL TIME} = L/V \text{ (WHEN CHANNEL VELOCITY IS KNOWN)}$$

$$4) T_c = T_{co} + T_t$$

*** IF TOTAL TIME OF CONCENTRATION IS LESS THAN 5 MINUTES, THEN 5 MINUTES IS USED

5) INTENSITY BASED ON I-D-F EQUATIONS IN CITY OF COLORADO SPRINGS DRAINAGE CRITERIA MANUAL

$$I_5 = -1.5 * \ln(T_c) + 7.583$$

$$I_{100} = -2.52 * \ln(T_c) + 12.735$$

$$6) Q = CIA$$

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80903

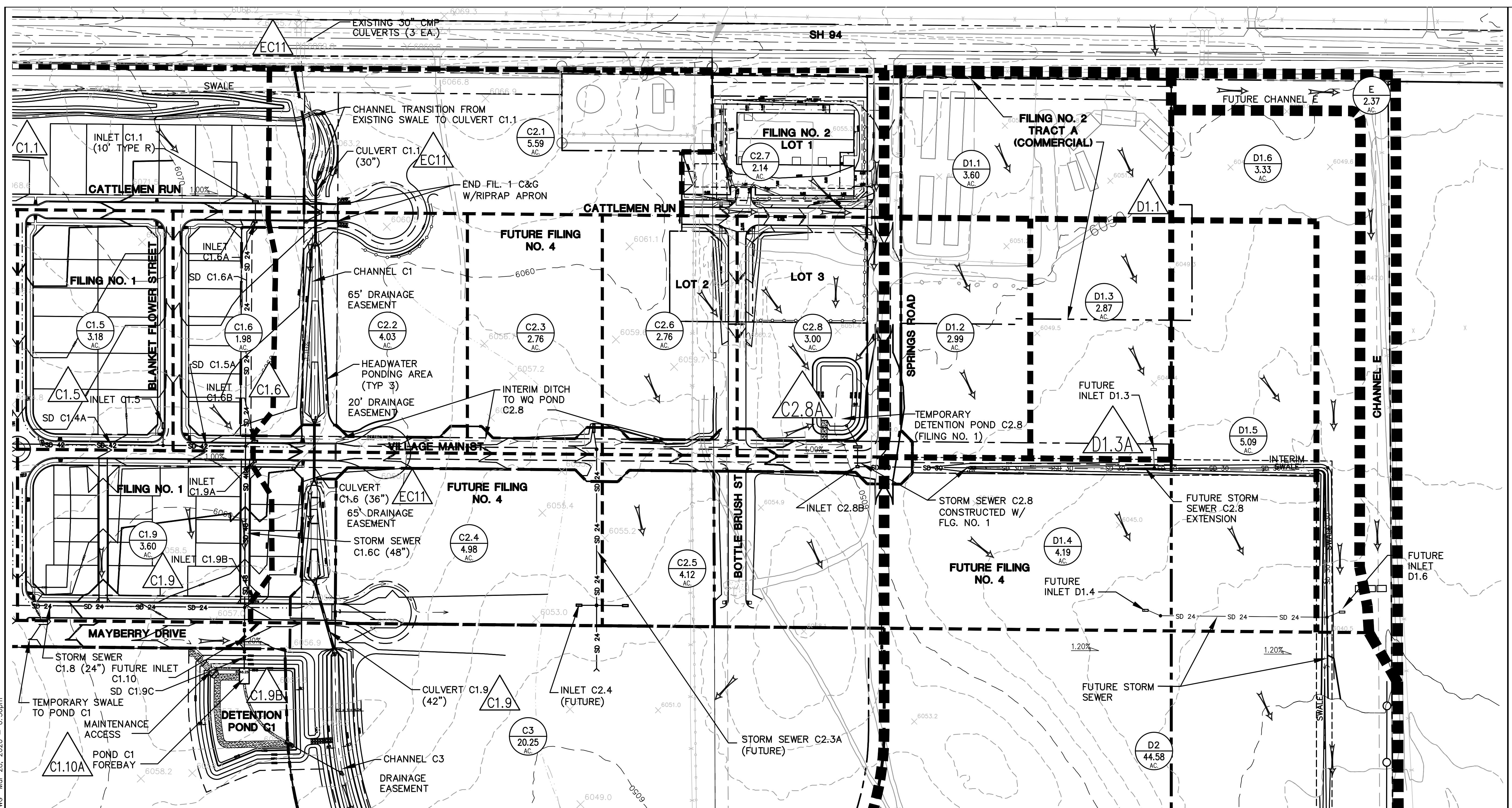
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MAYBERRY, COLORADO SPRINGS - FILING NO. 1-2

EAST SITE / FILING NO. 2 DEVELOPED DRAINAGE PLAN



LEGEND:

- PROPERTY LINES
- DRAINAGE BASIN BOUNDARY
- - - SUB-BASIN BOUNDARY
- ← PROPOSED FLOW DIRECTION ARROW
- EXISTING CONTOURS
- PROPOSED CONTOURS
- ▲ 1.5% PROPOSED STREET PROFILE GRADE
- △ DESIGN POINT
- (C1.9) DEVELOPED BASIN DESIGNATION
- (O/A 66.8 AC.) BASIN AREA (ACRES)

SUMMARY HYDROLOGY TABLE

DESIGN POINT	Q ₅ (CFS)	Q ₁₀₀ (CFS)
C2.8A	13.4	28.9
D1.1	10.4	20.8
D1.3A	26.8	57.1

HORZ. SCALE: 1"=100'	DRAWN: RMD
VERT. SCALE: N/A	DESIGNED: JPS
SURVEYED: RAMPART	CHECKED: JPS
CREATED: 3/28/19	LAST MODIFIED: 3/30/20
PROJECT NO: 090001	MODIFIED BY: BJJ

SHEET:
D1.12

