

# **DRAINAGE LETTER REPORT**

**for**

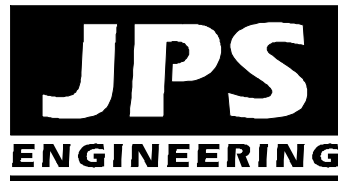
## **INTELIFAB LOT 1, MAYBERRY, COLORADO SPRINGS FILING NO. 2**

**Prepared for:**

**Hammers Construction Inc.**  
1411 Woolsey Heights  
Colorado Springs, CO 80915

March 30, 2020

**Prepared by:**



**19 E. Willamette Ave.  
Colorado Springs, CO 80903  
(719)-477-9429  
[www.jpsengr.com](http://www.jpsengr.com)**

**JPS Project No. 030502  
PCD File No. PPR-20\_\_**

**PCD File no. PPR2012**

**INTELIFAB**  
**LOT 1, MAYBERRY, COLORADO SPRINGS FILING NO. 2**  
**DRAINAGE REPORT STATEMENTS**

**1. Engineer's Statement:**

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan for the drainage basin. I accept responsibility for liability caused by negligent acts, errors or omissions on my part in preparing this report:

\_\_\_\_\_  
John P. Schwab          Colorado P.E. No. 29891

**2. Developer's Statement:**

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

By:

\_\_\_\_\_  
Printed Name:  
Title:

\_\_\_\_\_  
Date

**3. El Paso County Statement:**

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2, and Engineering Criteria Manual as amended.

\_\_\_\_\_  
Jennifer Irvine, P.E.  
County Engineer / ECM Administrator

\_\_\_\_\_  
Date

Conditions:

## I. INTRODUCTION

Please revise this statement as the site development plan cannot be approved until after the replat is approved.

### A. Property Location and Description

The Intelifab manufacturing facility is a proposed light industrial building to be constructed in the Mayberry, Colorado Springs (formerly known as “Ellicott Town Center”) Master Plan area of eastern El Paso County, Colorado. The 1.5-acre property is described as Lot 1, Mayberry, Colorado Springs Filing No. 2. Intelifab manufactures framing components used in the building industry. Lot 1 will be created upon recording of the subdivision plat for Mayberry, Colorado Springs Filing No. 2, which is a replat of tracts within Mayberry, Colorado Springs Filing No. 1.

The project site is located at the northwest corner of Springs Road and Cattlemen Run. A re-zoning is currently in process to establish the zoning of this property and adjoining property to the east as CS (Commercial Service).

State Highway 94 (SH94) adjoins the north boundary of this property, and the west boundary of the site adjoins an existing water storage tank parcel owned by Cherokee Metropolitan District. The south and east boundaries of the property adjoin undeveloped parts of the Mayberry, Colorado Springs (fka “Ellicott Town Center”) Master Plan area. Upon recording of Mayberry, Colorado Springs Filing No. 1, this property will be located within Tract L of Filing No. 1, and the Filing No. 2 replat is currently in process, which will establish the subject Lot 1 as a buildable lot.

The Intelifab project consists of a proposed 23,284 square-foot manufacturing building with associated parking and site improvements. Access will be provided by a driveway entrance onto Springs Road at the east boundary of the site, and an additional driveway entrance onto Cattlemen Run along the south boundary of the site.

comments were provided on the site plan regarding this access. Revise accordingly.

### B. Scope

In support of the El Paso County Site Development Plan submittal for this project, this report is intended to meet the requirements of a site-specific “Letter Type” drainage report in accordance with El Paso County drainage criteria. This report will provide a summary of site drainage issues impacting the proposed development. The report will analyze impacts from upstream drainage patterns, site-specific developed drainage patterns, and impacts on downstream facilities. This report is based on the guidelines and criteria presented in the City of Colorado Springs and El Paso County “Drainage Criteria Manual.”

### C. References

City of Colorado Springs & El Paso County “Drainage Criteria Manual,” revised October 12, 1994, Volumes 1 and 2.

JPS Engineering, Inc., "Final Drainage Report for Mayberry, Colorado Springs (fka "Ellicott Town Center") Filing No. 2," revised March 30, 2020.

JPS Engineering, Inc., "Final Drainage Report for Mayberry, Colorado Springs (fka "Ellicott Town Center") Filing No. 1," revised March 30, 2020.

As the revised drainage report for filing 2 plat has yet to be submitted the review of these runoffs will remain pending.

## II. EXISTING / PROPOSED DRAINAGE CONDITIONS

Drainage planning for this lot has been addressed in the "Final Drainage Report for Mayberry, Colorado Springs Filing No. 1" and the "Final Drainage Report for Mayberry, Colorado Springs Filing No. 2" by JPS Engineering. As noted in the subdivision drainage reports, on-site soils are comprised of Truckton loamy sand soils. These soils are classified as hydrologic soils group A (rapid permeability). The existing topography within this site slopes downward to the southeast with a grade of approximately 1-3 percent.

The subject property, Lot 1, has identified as Basin C2.7 in the subdivision drainage reports, and drainage from this lot has been planned to sheet flow in a southeasterly direction to the to the curb and gutter along the west side of Springs Road. The subdivision drainage report identifies peak flows of  $Q_5 = 5.2$  cfs and  $Q_{100} = 10.3$  cfs for Basin C2.7. Developed flows in the curb and gutter along the west side of Springs Road flow south to Design Point #C2.8A at the corner of Springs Road and Village Main Street. Developed peak flows at Design Point #C2.8A are calculated as  $Q_5 = 13.4$  cfs and  $Q_{100} = 28.9$  cfs.

Surface runoff from the developed site will continue to follow historic drainage patterns towards the south property boundary. The proposed building pad will be graded with protective slopes to provide positive drainage away from the face of the building. A high point will be graded in the northwest corner of the site, and drainage swales will be graded along the north and west faces of the building to convey developed flows around the building. Curb and gutter will be installed along the outer perimeter of the new parking areas to convey surface drainage to curb chases in the adjoining public streets, ultimately directing developed flows from this site to the curb and gutter along the west side of Springs Road.

Please also include discussion of the crosspans to be installed in the parking areas.

Provide a design calculation for the proposed grass lined drainage swale.

Stormwater quality mitigation and detention will be provided by routing developed flows through the subdivision detention pond south of the property. Temporary Detention Pond C2.8 will be constructed at the northwest corner of Springs Road and Village Main Street, and this pond has been sized to mitigate developed drainage impacts from Basins C2.6, C2.7, and C2.8 during the initial phase of development. In conjunction with future development of Filing No. 4 southeast of this site, the larger sub-regional Detention Pond D will be constructed further to the southeast, after which temporary Detention Pond C2.8 will be removed.

Please indicate how the developed flows will enter pond C2.8 from Springs Road. The previously submitted/reviewed TIS indicated that this road is to be developed to urban non-residential collector standards (i.e. curb/gutter) with the filing no. 1 final plat.

Note that the revised report dated March 2020 has not been submitted for review to verify this value therefore the review of this item will remain pending. The previous submitted report (dated July 2019) for filing 2 plat calculated the impervious as 70%.

The subdivision drainage report assumed full commercial / light industrial development of this site, and the proposed site development plan is entirely consistent with the approved subdivision drainage plan. The calculated impervious area of the Lot 1 site development is 80 percent, which matches the assumptions in the subdivision drainage reports (revised March, 2020). The proposed Site Development Plan for the Intelifab building project includes landscaped areas around the perimeter of the site, as well as a 40-foot right-of-way dedication along SH94, so the total impervious area remains below the maximum impervious area anticipated in the subdivision drainage report.

Hydrologic calculations are detailed in the attached spreadsheet (Appendix A), and peak flows are identified on Figure D1.12. The contractor will need to implement standard best management practices for erosion control during construction, as depicted on the Site Grading and Erosion Control Plans.

The 40' ROW is dedicated at the final plat so it would have been accounted for at that time.

### III. DRAINAGE PLANNING FOUR STEP PROCESS

El Paso County Drainage Criteria require drainage planning to include a Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long-term source controls.

As stated in DCM Volume 2, the Four Step Process is applicable to all new and re-development projects with construction activities that disturb 1 acre or greater or that disturb less than 1 acre but are part of a larger common plan of development. The Four Step Process has been implemented as follows in the planning of this project:

#### Step 1: Employ Runoff Reduction Practices

- Minimize Directly Connected Impervious Areas: Roof drain downspouts will be directed to flow over grass-lined drainage swales where possible. Drainage from the north side of the building will flow through a grass-lined drainage swale prior to reaching the downstream public street.

#### Step 2: Stabilize Drainageways

- There are no drainageways directly adjacent to this project site.

#### Step 3: Provide Water Quality Capture Volume (WQCV)

- EDB: The developed site will drain through an off-site Extended Detention Basin (EDB) south of the property. Detention Pond C2.8. Has been designed to provide stormwater detention and water quality for this site. The extended detention basin will capture and slowly discharge the WQCV over an extended release period.

#### Step 4: Consider Need for Industrial and Commercial BMPs

- The property owner will implement a Stormwater Management Plan (SWMP) which includes proper housekeeping and spill containment procedures.

- Site drainage will be routed through the downstream Extended Detention Basin (EDB) to minimize discharge of contaminants to the downstream drainage system.

#### **IV. FLOODPLAIN IMPACTS**

This site is located beyond the limits of any FEMA 100-year floodplain boundaries as shown in the FEMA floodplain map for this area, FIRM Panel No. 08041C0810G, dated December 7, 2018.

#### **V. DRAINAGE BASIN FEES**

This site is located within the Ellicott Consolidated Drainage Basin. No public drainage improvements are required for development of this site. The Ellicott Consolidated Drainage Basin does not have a drainage or bridge fee requirement, and any required drainage fees would have been addressed at the subdivision stage, so there are no applicable drainage fees required with the Site Development Plan.

#### **VI. SUMMARY**

The developed drainage patterns associated with the proposed Intelifab manufacturing facility will remain consistent with historic conditions and the overall drainage plan for this commercial subdivision. Developed flows from the site will continue to follow historic drainage patterns, flowing to the subdivision detention pond southeast of this site.

The subdivision detention pond will mitigate developed drainage impacts and meet the County's stormwater quality requirements for this site. Proper maintenance of the subdivision detention pond, in conjunction with proper erosion control practices, will ensure that this developed site has no significant adverse impact on downstream or surrounding areas.

ELLICOTT TOWN CENTER  
RATIONAL METHOD - HYDROLOGIC CALCULATIONS

DEVELOPED FLOWS

BASIN	DESIGN POINT	AREA (AC)	C		Overland Flow			Channel flow					TOTAL Tc <sup>(4)</sup> (MIN)	TOTAL Tc <sup>(4)</sup> (MIN)	INTENSITY <sup>(6)</sup>			PEAK FLOW Q5 <sup>(6)</sup> (CFS)	PEAK FLOW Q100 <sup>(6)</sup> (CFS)		
			5-YEAR	100-YEAR	LENGTH (FT)	SLOPE (FT/FT)	Tco <sup>(1)</sup> (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE COEFFICIENT C	SLOPE (FT/FT)	SCS <sup>(2)</sup> VELOCITY (FT/S)	Tt <sup>(3)</sup> (MIN)			5-YR (IN/HR)	100-YR (IN/HR)	5.34			7.26	8.68
<b>FILING NO. 1</b>																					
A1A	A1A	2.80	0.355	0.555	40	0.020	6.8	2035	15.00	0.011	1.57	21.6	28.4	28.4	2.56	4.30	2.55	6.68			
C1.2	C1.2	7.97	0.490	0.620				1000	20.00	0.009	1.90	8.8	8.8	8.8	4.32	7.26	16.88	35.87			
C1.7A	C1.7A	0.58	0.375	0.545				680	20.00	0.013	2.28	5.0	5.0	5.0	5.17	8.68	1.12	2.74			
C1.7B	C1.7B	4.34	0.490	0.620	100	0.020	8.9	400	20.00	0.01	2.00	3.3	12.2	12.2	3.83	6.43	8.15	17.31			
C1.7A,C1.7B	C1.7B1	4.92	0.476	0.611					20.00				12.2	12.2	3.83	6.43	8.97	19.33			
C1.2,C1.7	C1.2D	12.89	0.485	0.617									12.2	12.2	3.83	6.43	23.95	51.15			
C1.3		3.02	0.375	0.545				280	20.00	0.01	2.00	2.3	2.3	5.0	5.17	8.68	5.85	14.29			
C1.2,C1.3,C1.7	C1.3A	15.91	0.464	0.603					20.00	0.01	2.00	2.5	14.5	14.5	3.57	5.99	26.34	57.47			
C1.4		3.23	0.375	0.545				300	20.00	0.01	2.00	2.5	2.5	5.0	5.17	8.68	6.26	15.28			
C1.2-C1.4,C1.7	C1.4A	19.14	0.449	0.593					20.00	0.01	2.00	2.5	17.0	17.0	3.33	5.59	28.62	63.45			
C1.5		3.18	0.375	0.545				300	20.00	0.01	2.00	2.5	2.5	5.0	5.17	8.68	6.16	15.04			
C1.2-C1.5,C1.7	C1.5A	22.32	0.438	0.586									19.5	19.5	3.12	5.25	30.55	68.61			
C1.1	C1.1	9.38	0.226	0.447	100	0.017	13.4	1800	20.00	0.01	2.00	15.0	28.4	28.4	2.56	4.30	5.43	18.04			
C1.6		1.98	0.375	0.545				280	20.00	0.01	2.00	2.3	2.3	5.0	5.17	8.68	3.84	9.37			
C1.1,C1.6	C1.6B	11.36	0.252	0.464					20.00	0.01	2.00	2.3	30.7	30.7	2.44	4.10	7.00	21.62			
C1.1-C1.7	C1.7A	33.68	0.376	0.545									30.7	30.7	2.44	4.10	30.96	75.30			
C1.8		3.89	0.375	0.545				600	20.00	0.016	2.53	4.0	4.0	5.0	5.17	8.68	7.54	18.40			
C1.9		3.60	0.375	0.545				580	20.00	0.012	2.19	4.4	4.4	5.0	5.17	8.68	6.98	17.03			
C1.8,C1.9	C1.9A	7.49	0.375	0.545					20.00				8.4	8.4	4.40	7.38	12.35	30.14			
C1.1-C1.9	C1.9B	41.17	0.376	0.545									30.7	30.7	2.44	4.10	37.84	92.05			
<b>FILING NO. 2</b>																					
C2.1		5.59	0.242	0.457	100	0.016	13.4	650	20.00	0.01	2.00	5.4	18.8	18.8	3.18	5.34	4.30	13.63			
C2.2		4.03	0.375	0.545				460	20.00	0.01	2.00	3.8	3.8	5.0	5.17	8.68	7.81	19.06			
C2.3		2.76	0.375	0.545				260	20.00	0.01	2.00	2.2	2.2	5.0	5.17	8.68	5.35	13.06			
C2.1-C2.3	C2.3A	12.38	0.315	0.505					20.00				21.0	21.0	3.02	5.06	11.76	31.64			
C2.4		4.98	0.375	0.545				560	20.00	0.012	2.19	4.3	4.3	5.0	5.17	8.68	9.65	23.56			
C2.5		4.12	0.375	0.545				330	20.00	0.01	2.00	2.8	2.8	5.0	5.17	8.68	7.99	19.49			
C2.1-C2.5	C2.5A	21.48	0.341	0.522					20.00				23.8	23.8	2.83	4.75	20.73	53.27			

BASIN	DESIGN POINT	AREA (AC)	C		Overland Flow			Channel flow						TOTAL		INTENSITY <sup>(6)</sup>		PEAK FLOW				
			5-YEAR	100-YEAR	LENGTH (FT)	SLOPE (FT/FT)	T <sub>co</sub> (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE C	SLOPE (FT/FT)	VELOCITY (FT/S)	T <sub>t</sub> (MIN)	T <sub>c</sub> (MIN)	T <sub>c</sub> (MIN)	5-YR (IN/HR)	100-YR (IN/HR)	Q5 <sup>(6)</sup> (CFS)	Q100 <sup>(6)</sup> (CFS)				
			0.080	0.350											3.97	6.66	6.48	47.58				
C4		20.40	0.080	0.350									15.00	0.011	1.57	11.1	11.1	3.97	6.66	6.48	47.58	
Tc C2.5A TO DP-D2B													15.00	0.01	1.50	27.2						
C2.1-C2.5,C4	C4.1	41.88	0.214	0.438													30.7	2.44	4.10	21.91	75.25	
C2.6		2.76	0.375	0.545	100	0.020	10.5	550	20.00	0.016	2.53	3.6	20.00	0.013	2.28	2.9	14.2	3.61	6.06	3.73	9.11	
C2.7		2.14	0.490	0.620	100	0.020	8.9	400	20.00	0.013	2.28	2.9	20.00	0.013	2.28	2.9	11.8	3.88	6.52	4.07	8.65	
C2.8		3.00	0.413	0.579			0.0	250	20.00	0.012	2.19	1.9	20.00	0.012	2.19	1.9	1.9	5.17	8.68	6.40	15.08	
C2.6-C2.8	C2.8A	7.90	0.421	0.578													16.1	3.42	5.74	11.37	26.20	
D1.2		2.99	0.427	0.579			0.0	300	20.00	0.01	2.00	2.5	20.00	0.01	2.00	2.5	2.5	5.17	8.68	6.60	15.03	
C2.6-C2.8,D1.2	D1.2A	10.89	0.426	0.578													18.6	3.20	5.37	14.85	33.83	
D1.1		3.60	0.490	0.545			0.0	750	20.00	0.011	2.10	6.0	20.00	0.011	2.10	6.0	6.0	4.91	8.24	8.65	16.16	
D1.3		2.87	0.427	0.579			0.0	280	20.00	0.01	2.00	2.3	20.00	0.01	2.00	2.3	2.3	5.17	8.68	6.33	14.42	
C2.6-C2.8,D1.1-D1.3	D1.3A	17.36	0.439	0.571													20.9	3.02	5.08	23.04	50.31	
D1.4		4.19	0.375	0.545			0.0	550	20.00	0.012	2.19	4.2	20.00	0.012	2.19	4.2	4.2	5.17	8.68	8.12	19.82	
D1.5		5.09	0.375	0.545			0.0	280	20.00	0.01	2.00	2.3	20.00	0.01	2.00	2.3	2.3	5.17	8.68	9.87	24.08	
D1.6		3.33	0.375	0.545			0.0	1060	20.00	0.01	2.00	8.8	20.00	0.01	2.00	8.8	8.8	4.32	7.25	5.39	13.15	
C2.6-C2.8,D1.1-D1.6	D1.6A	29.97	0.412	0.560													25.1	2.75	4.62	33.96	77.46	
PHASE 2																						
D2		44.58	0.341	0.523																		
C2.6-C2.8,D1.1-D1.6,D2	D2A	74.55	0.370	0.538	100	0.020	11.0	1750	20.00	0.011	2.10	13.9	20.00	0.011	2.10	13.9	24.9	2.76	4.63	41.94	107.95	
C2,C4,D	D2B	116.43	0.314	0.502													23.8	2.83	4.75	103.49	277.70	
C3		74.48	0.375	0.545	100	0.020	10.5	3000	20.00	0.011	2.10	23.8	20.00	0.011	2.10	23.8	34.4	2.28	3.82	63.60	155.10	
C2,C2,C4,D	D2C	190.91	0.338	0.519													51.0	1.69	2.83	108.77	280.15	

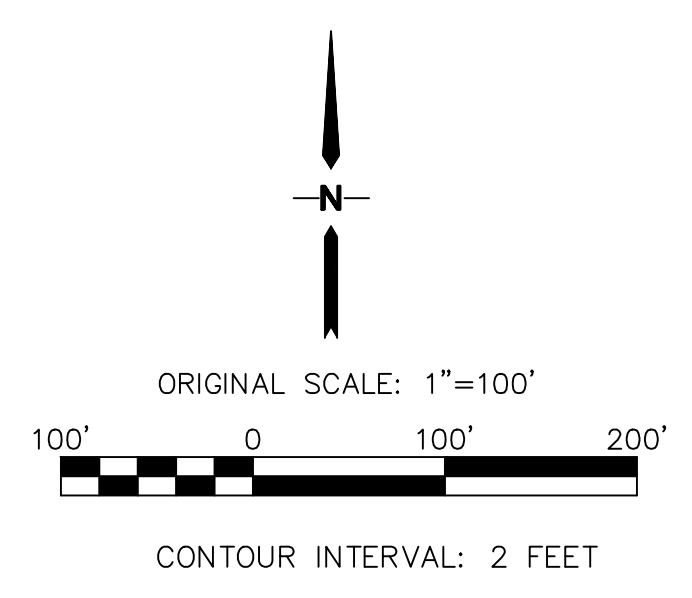
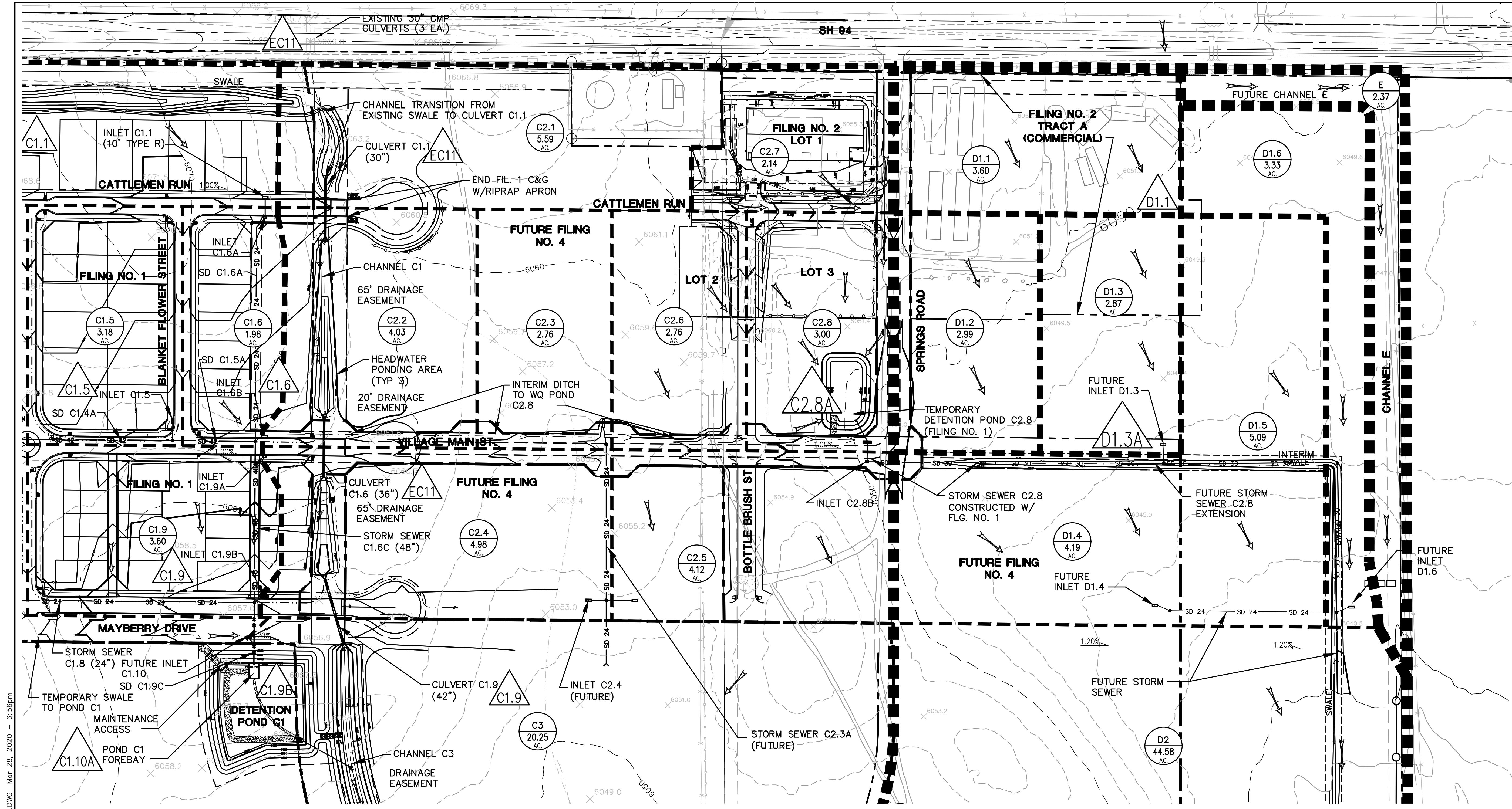
These runoff coefficients are for a 70% impervious per table 6-6 of the DCM. The narrative indicated that the imperviousness of the site is 80%. Based on the proposed site plan it appears that the imperviousness is in the +/-80% range. Revise accordingly.

Please calculate the actual imperviousness/ runoff of the proposed site to compare/prove that it is less than or equal to what is indicated in the drainage report provided with the final plat.

These flows do not match the narrative. Revise accordingly.



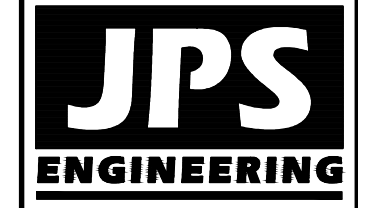
- 1) OVERLAND FLOW  $T_{co} = (0.395 * (1.1 - \text{RUNOFF COEFFICIENT}) * (\text{OVERLAND FLOW LENGTH})^{0.5}) / (\text{SLOPE})^{0.333}$
- 2) SCS VELOCITY =  $C * ((\text{SLOPE}(\text{FT}/\text{FT})^{0.5})$   
 $C = 2.5$  FOR HEAVY MEADOW  
 $C = 5$  FOR TILLAGE/FIELD  
 $C = 7$  FOR SHORT PASTURE AND LAWNS  
 $C = 10$  FOR NEARLY BARE GROUND  
 $C = 15$  FOR GRASSED WATERWAY  
 $C = 20$  FOR PAVED AREAS AND SHALLOW PAVED SWALES
- 3) MANNING'S CHANNEL TRAVEL TIME =  $L/V$  (WHEN CHANNEL VELOCITY IS KNOWN)
- 4)  $T_c = T_{co} + T_t$   
 \*\*\* IF TOTAL TIME OF CONCENTRATION IS LESS THAN 5 MINUTES, THEN 5 MINUTES IS USED
- 5) INTENSITY BASED ON I-D-F EQUATIONS IN CITY OF COLORADO SPRINGS DRAINAGE CRITERIA MANUAL  
 $I_s = -1.5 * \ln(T_c) + 7.583$   
 $I_{100} = -2.52 * \ln(T_c) + 12.735$
- 6)  $Q = C_i A$



- LEGEND:**
- PROPERTY LINES
  - DRAINAGE BASIN BOUNDARY
  - SUB-BASIN BOUNDARY
  - ← PROPOSED FLOW DIRECTION ARROW
  - 6490 EXISTING CONTOURS
  - 6490 PROPOSED CONTOURS
  - 1.5% PROPOSED STREET PROFILE GRADE
  - △ DESIGN POINT
  - DEVELOPED BASIN DESIGNATION
  - BASIN AREA (ACRES)

SUMMARY HYDROLOGY TABLE		
DESIGN POINT	Q5 (CFS)	Q100 (CFS)
C2.8A	13.4	28.9
D1.1	10.4	20.8
D1.3A	26.8	57.1

# MAYBERRY, COLORADO SPRINGS - FILING NO. 1-2



19 E. Willamette Ave.  
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CALL UTILITY NOTIFICATION  
CENTER OF COLORADO  
1-800-922-1987  
CALL BEFORE YOU DIG  
BEFORE YOU DIG GRADE, OR EXCAVATE  
FOR THE MARKING OF UNDERGROUND  
MEMBER UTILITIES

No.	REVISION	DATE

## EAST SITE / FILING NO. 2 DEVELOPED DRAINAGE PLAN

HORZ. SCALE: 1"=100'  
VERT. SCALE: N/A  
SURVEYED: N/A  
CREATED: 3/28/19  
PROJECT NO: 090001  
DRAWN: RMD  
DESIGNED: JPS  
CHECKED: JPS  
LAST MODIFIED: 3/30/20  
MODIFIED BY: BJJ

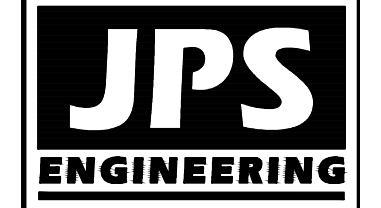
**SHEET: D1.12**

C:\Users\Owner\Desktop\psprojects\090001\Ellipse\TTC\dwg\Civil\F1\01.11.DWG Mar 28, 2020 - 6:56pm

**GRADING & EROSION CONTROL (GEC) PLAN SHEET INDEX**

- C1.1 SITE GRADING & EROSION CONTROL PLAN
- C2.1 CIVIL NOTES & DETAILS
- C2.2 EROSION CONTROL NOTES & DETAILS

NOTE: STORMWATER DETENTION AND WATER QUALITY FOR THIS SITE (INCLUDING TEMPORARY SEDIMENT BASIN) IS PROVIDED IN OFF-SITE DETENTION POND C2.8



19 E. Willamette Ave.  
Colorado Springs, CO 80903  
PH: 719-477-9429  
FAX: 719-471-0766



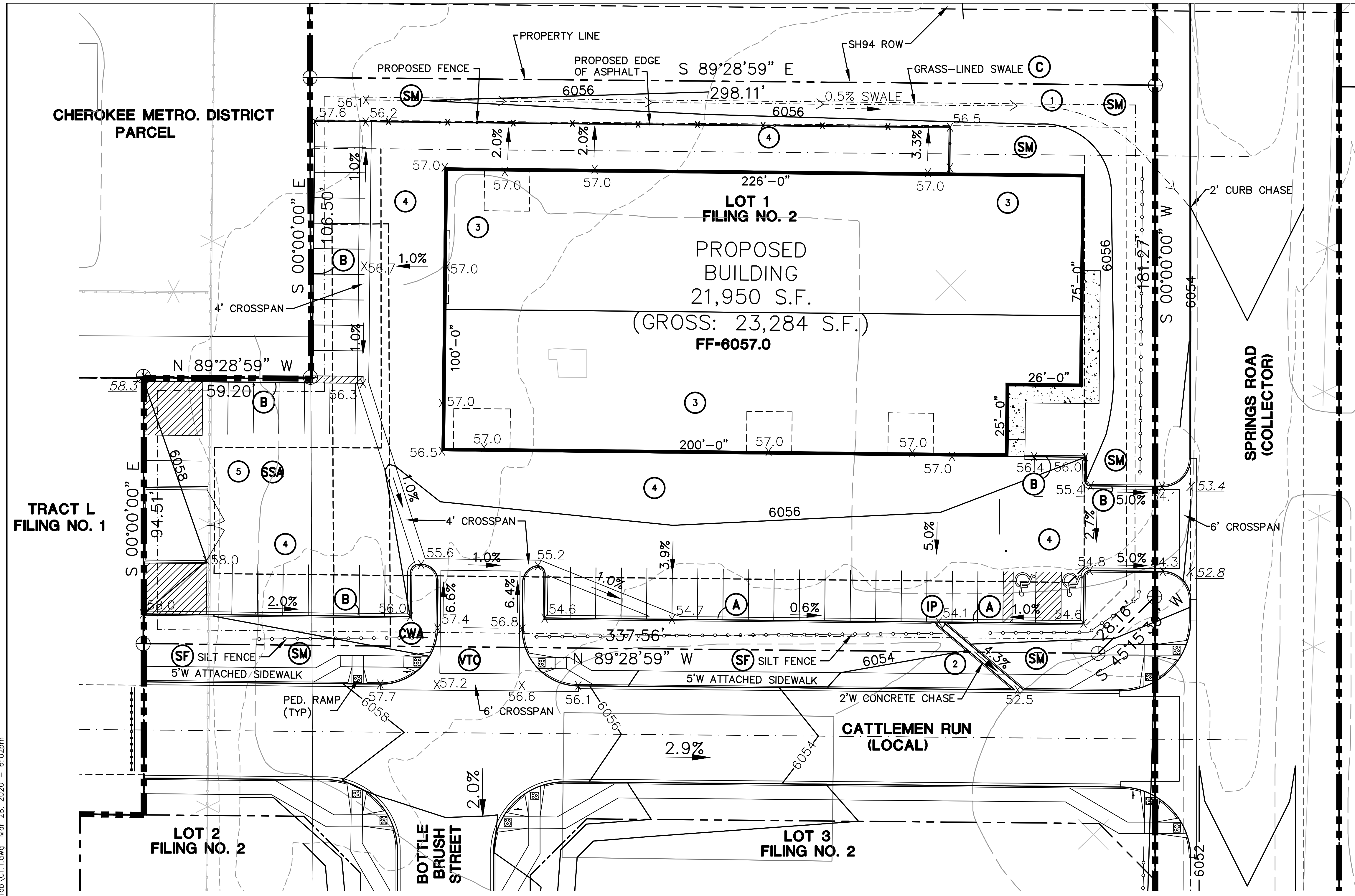
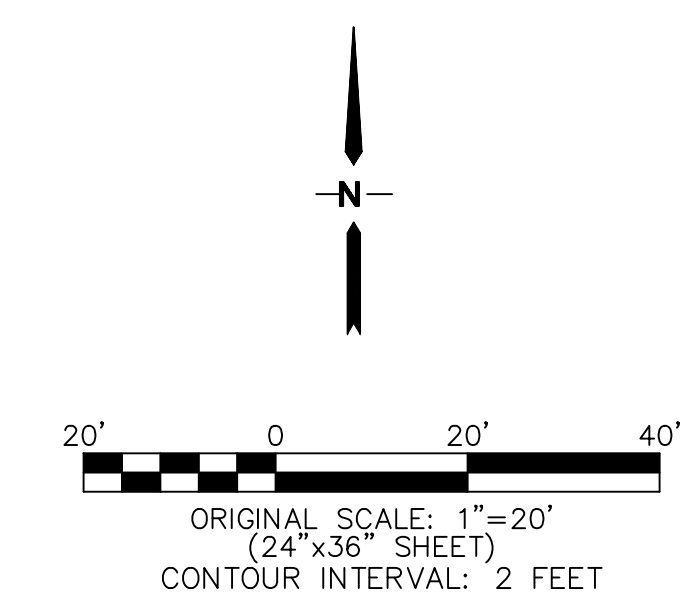
CALL UTILITY NOTIFICATION CENTER OF COLORADO  
1-800-922-1987  
BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.

**INTELIFAB LOT 1, MAYBERRY, COLORADO SPRINGS, FIL. NO. 2**

**TRACT A FILING NO. 2**

**KEYED NOTES:**

- 1 TOPSOIL & STRIPPINGS STOCKPILE AREA (SP)
- 2 CONTRACTOR MAY WASTE EXCESS CUT MATERIAL OR BORROW SUITABLE FILL MATERIAL FROM THIS AREA. MAINTAIN POSITIVE DRAINAGE & MATCH INTO EXISTING GRADES WITH 3:1 MAX. SLOPE.
- 3 PREPARE AND COMPACT BUILDING FOUNDATION & SLAB PER PROJECT GEOTECHNICAL REPORT
- 4 PARKING LOT PAVING PER GEOTECHNICAL REPORT (4" ASPHALT OVER 6" AGGREGATE BASE UNLESS NOTED OTHERWISE)
- 5 STORAGE AREA FOR BUILDING MATERIALS, EQUIPMENT, CONSTRUCTION WASTE, & STOCKPILES (CONTRACTOR MAY ADJUST AS NEEDED)

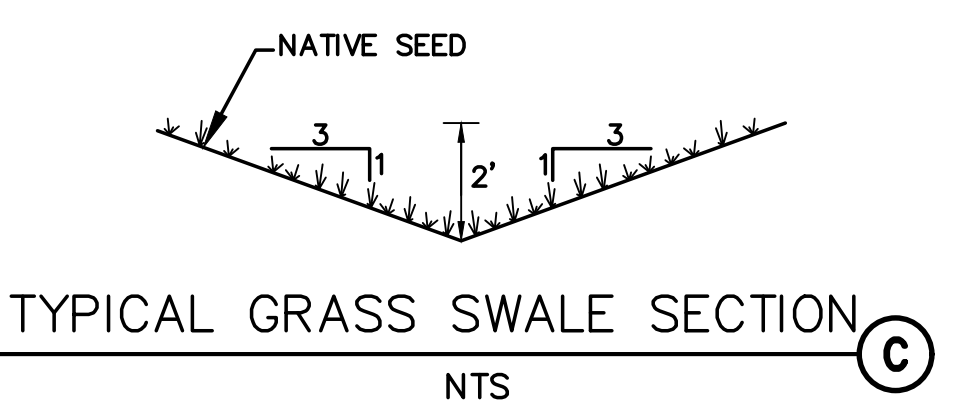
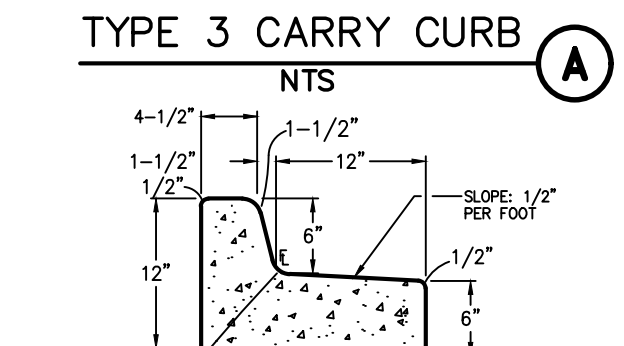
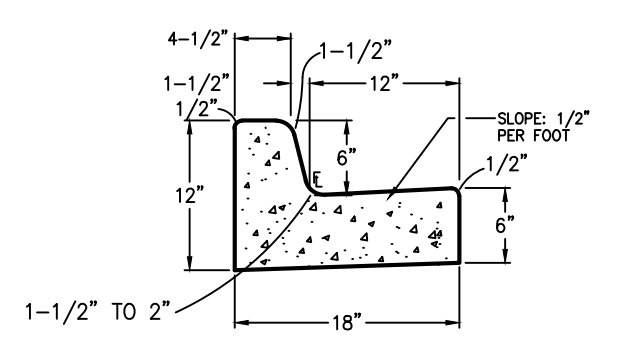


**LEGEND:**

- BOUNDARY LINES
- 6490 EXISTING CONTOURS
- 6490 PROPOSED CONTOURS
- x 99.00 PROPOSED SPOT ELEVATION (FLOWLINE)
- PROPOSED STREET PROFILE GRADE
- CUT-FILL DEMARCATION LINES
- FLOW DIRECTION ARROWS

**EROSION CONTROL LEGEND:**

- IP INLET PROTECTION (GRAVEL FILTER)
- SF SILT FENCE
- VTC VEHICLE TRACKING CONTROL PAD (PER EPC STD. DETAIL)
- SM SEED AND MULCH
- STB STRAW BALE BARRIER
- TSB TEMPORARY SEDIMENT BASIN
- RR RIPRAP
- CWA CONCRETE WASHOUT AREA
- SSA STABILIZED STAGING AREA



HORZ. SCALE: 1"=20'	DRAWN: BJJ
VERT. SCALE: N/A	DESIGNED: JPS
SURVEYED: UP&E	CHECKED: JPS
CREATED: 3/20/20	LAST MODIFIED: 3/30/20
PROJECT NO: 090001	MODIFIED BY: BJJ

**C1.1**

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