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# Final Drainage Report

## **Monument Small Engine Storage**

**Project No. 61092**

**June 15, 2018**

PCD File No. PPR-18-013

# **Final Drainage Report**

for

**Monument Small Engine Storage**

**Project No. 61092**

**June 15, 2018**

prepared for

**David P. Hellbusch**

137 N. Monument Lake Road

Monument, CO 80132

719.448.3230

prepared by

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61092-Monument Small Engine-FDR.odt

# Statements and Acknowledgments

## Engineer's Statement

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

\_\_\_\_\_  
David R. Gorman, P.E.  
For and on Behalf of MVE, Inc.

Colorado No. 31672

\_\_\_\_\_  
Date

## Developer's Statement

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

\_\_\_\_\_  
David P. Hellbusch  
Owner  
137 N. Monument Lake Rd  
Monument, CO 80132

\_\_\_\_\_  
Date

## El Paso County

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

\_\_\_\_\_  
Jennifer Irvine, P.E.,  
County Engineer / ECM Administrator

\_\_\_\_\_  
Date

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# Final Drainage Report

The purpose of this Final Drainage Report is to identify drainage patterns and quantities within and affecting the proposed Monument Small Engine Storage site. This drainage report is in support of a site plan approval for rezone application to allow outdoor RV storage on the property. The report will “identify specific solutions to problems on-site and off-site resulting from the proposed project. The report and included maps present results of hydrologic and drainage facilities analyses. The report will discuss the recommended drainage improvements to the site and identify drainage requirements relative to the proposed project. This report has been prepared and submitted in accordance with the requirements of the El Paso County development approval process. An Appendix is included with this report with pertinent calculations and graphs used in the drainage analyses and design.

## 1 General Location and Description

### 1.1 Location

The proposed Monument Small Engine Storage site is located within the southwest one-quarter of the northeast one-quarter of Section 15, Township 11 South, Range 67 west of the 6th principal meridian in El Paso County, Colorado. The 3.89± acre site is situated north of the intersection of Mitchell Avenue and Monument Lake Road. The site is generally west of the Denver & Rio Grand Railroad, north of Monument Lake Road at Mitchell Avenue. The site contains an existing business using the address of 137 N. Monument Lake Road. The El Paso County Assessor's Schedule Number for the site is 711510008. The proposed site has never been platted. A **Vicinity Map** is included in the **Appendix**.

The south edge of the site is adjacent to an unplatted parcel containing a single-family residence and zoned PRD (Planned Residential Development). This parcel is located in the Town of Monument and separates the site from Monument Lake Road (60' R.O.W.) which is located along the south edge of the parcel. Mount Herman RV Storage Subdivision, located along the east side of the site, is located within the Town of Monument, is zoned PID (Planned Industrial Development) and contains an RV storage business. Unplatted and undeveloped property, located within El Paso County and zoned RR-5 (Residential Rural), lies north of the site. Unplatted property zoned RR-5 and containing a single family residence is located adjacent to the east side of the site.

Crystal Creek, a tributary to Monument Creek, flows northeast to southwest through the adjacent property to north. The site is located in El Paso County's Crystal Creek Drainage Basin.

### 1.2 Description of Property

The Monument Small Engine Storage site 3.89± acres and is zoned CS (Commercial Service). The property is the location of a business with an existing shop, garage building, shed, gravel drives and parking areas, mostly concentrated at the southeast portion.

The site has been used for many years as an equipment and engine repair shop with an approved Variance of Use from El Paso County. A past use of the site has been as a plant and tree nursery with green house buildings, gravel drives and open space. Terraced railroad tie retaining walls are located in the western portion of the site. A gravel parking area is located in the northeastern portion. A gravel access drive runs through the site from north to south that is also used to serve the

adjacent residence. Ground cover in the non gravels areas is typically native grasses and weeds with sparse trees and shrubs scattered. The condition of the ground cover on the site varies from poor to good, depending on location within the site. Former drive areas are relatively bare, but the nursery areas are in good condition. Access to the site is via one point off of North Monument Lake Road from the south. Power poles and lines are also located near and through the site.

The site slopes generally from east to west with grades ranging from 1% to 6% with intermittent walls or short terraces. There lowest point on the site is the southwester corner. An existing stormwater quality pond exists on the adjacent property on the east which outfalls onto the site near the eastern boundary in the center. An existing 12" CMP culvert discharges from the pond into the site and the outflow is intercepted by a shallow ditch, which directs the flows to the northwest corner and then offsite. No significant drainageways flow through the site and no significant drainage improvements or drainage facilities currently exist on the site.

According to the National Resource Conservation Service, the soil in the area of the Monument Small Engine Storage site consists of Tomah-Crowfoot loamy sand (map unit 92). The soil is deep and well drained. Permeability is moderately rapid, surface runoff is slow, and the hazard of erosion is slight to moderate. Tomah-Crowfoot loamy sand is classified as being part of Hydrologic Soil Group B. A portion of the Soil Map and data tables from the National Cooperative Soil Survey and relevant Official Soil Series Descriptions (OSD) are included in the **Appendix**.<sup>1 2</sup>

There are no major drainageways in the Monument Small Engine Storage site. However, Crystal Creek is located to the north of the site. Crystal Creek drains into Monument Reservoir and is tributary to Monument Creek.

The current Flood Insurance Study of the region includes Flood Insurance Rate Maps (FIRM), effective on March 17, 1997.<sup>3</sup> The proposed subdivision is included in two Community Panels Numbered 08041C0276 F and 08041C0260 of the Flood Insurance Rate Maps for the El Paso County. No part of the site is shown to be included in a 100-year flood hazard area as determined by FEMA. A portion of the current FEMA Flood Insurance Rate Maps with the site delineated is included in the **Appendix**.

## 2 Drainage Basins and Sub-Basins

### 2.1 Major Basin Descriptions

The Monument Small Engine Storage site is located in the Crystal Creek Drainage Basin (FOFO5300) of the Fountain Creek Major Drainage Basin (FO). This basin drains to Monument Creek. The *Dirty Woman Creek and Crystal Creek Drainage Basin Planning Study* provides development recommendations and requirements for drainage development in the Crystal Creek Drainage Basin (DBPS).<sup>4</sup> The Crystal Creek Drainage Basin encompasses a part of the northwest portion of the Town of Monument and extends to the north and east. The drainage basin and Crystal Creek drains southwest into Monument Creek. The Monument Small Engine Storage site is located east of Crystal Creek as it flows offsite towards Monument Creek . The site is located in sub-basin CC 185, upstream of Design Point 185 of the Drainage Basin Planning Study. The closest drainage improvements that the study recommends is offsite at the Monument Lake Road crossing of Crystal Creek. No other improvements are recommended on or near the project site. A copy of a portion of the "***Drainage Area Identification Study***"<sup>5</sup> map, showing the site location within the Basin is included in the **Appendix**. The proposed Monument Small Engine Storage project is in conformance with the DBPS.

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1 WSS  
2 OSD  
3 FIRM  
4 DBPS  
5 Drain. Area Ident. Study

## 2.2 Other Drainage Reports

The adjacent property in the east was studied as detailed in the report “Final Drainage Report, Mount Herman RV Storage Subdivision, Town of Monument Colorado” by M.V.E., Inc. dated January 30, 2014.<sup>6</sup> The Mount Herman report establishes the offsite basin conditions to the east, including the existing storm water quality sand filter basin. The offsite basin and pond have been re-evaluated in this report using the current drainage criteria adopted by the County and those updated results are included in this Monument Small Engine Storage drainage report.

## 2.3 Sub-Basin Description

The existing drainage patterns of the Monument Small Engine Storage are described by two on-site drainage basins and one offsite basin. All of these basins are previously disturbed or developed to a degree as described below. All existing basin delineations and data are depicted on the attached **Existing Drainage Map**.

### 2.3.1 Existing Drainage Patterns (Off-Site)

Existing off site sub-basin OSA1 is located to the east of the site, containing the developed Lot 1 Mount Herman RV Storage Subdivision with existing RV storage area, pond area, landscape area and railroad right-of-way. The basin drains to the existing water quality pond and then into the site by way of an exiting 12” CMP pipe from the pond along with the pond spillway located on the property line. This flow enters the onsite basin A1 and continues through the site.

### 2.3.2 Existing Drainage Patterns (On-Site)

The site generally drains to the west, but contains a gravel drive and ditch that drains to towards the northwest corner of the site. The northeast portion of the site, combined with flows from off-site Basin OSA1 drain towards the northwest corner of the site. The remaining portion of the site drains west towards the site's western boundary.

Existing sub-basin A1 is located in the northeastern portion of the site and contains some gravel parking area and slope leading to the existing ditch and gravel drive. The existing 12” CMP from offsite Basin OSA1 outfalls into Basin A1. All flows from Basin A1 exit the site at the southwest corner into the adjacent site to the west, which is also owned by the owner of the adjacent property to the east. These flows continue west through the adjacent properties to Crystal Creek.

Existing sub-basin A2 is also located in the northeastern portion of the site, just south of sub-basin A1. The sub-basin contains gravel drive, leach field area, and pasture/meadow. All flows from sub-basin A2 exit the site at the west property line. The western adjacent property is owned by the owner of the adjacent property to the east. These flows continue west through the adjacent property and into Crystal Creek.

Existing sub-basin B1 is located on the southerly and westerly portion of the site, containing the existing shop, garage, and shed buildings, paved parking and walk areas, gravel drive and parking areas and open meadow areas with tiered railroad tie walls. The previously existing greenhouse buildings in this basin have been removed. The flows generated by this basin drain to the west and exit the site by sheet flow into the adjacent site to the west. These flows continue west through the adjacent properties to Crystal Creek. All flows from the site eventually enter Monument Reservoir and then Monument Creek.

## 3 Drainage Design Criteria

### 3.1 Development Criteria Reference

This Final Drainage Report for Monument Small Engine Storage has been prepared according to the report guidelines presented in the latest edition of *El Paso County Drainage Criteria Manual* (DCM)<sup>7</sup>. The County has also adopted portions of the City of Colorado Springs Drainage Criteria Manual

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<sup>6</sup> Mount Herman FDR

<sup>7</sup> DCM Section 4.3 and Section 4.4



Volumes 1 and 2, especially concerning the calculation of rainfall runoff flow rates.<sup>8 9</sup> The hydrologic analysis is based on a collection of data from the DCM, the NRCS Web Soil Survey<sup>10</sup>, Existing topographic data by Cornerstone Surveying, and proposed site plan by RMJ Designs.

### 3.2 Hydrologic Criteria

For this Final Drainage Report, the Rational Method as described in the *Drainage Criteria Manual* has been used for all Storm Runoff calculations, as the development and all sub-basins are less than 130 acres in area. “Colorado Springs Rainfall Intensity Duration Frequency” curves, Figure 6-5 in the DCM, was used to obtain the design rainfall values; a copy is included in the **Appendix**. The “Overland (Initial) Flow Equation” (Eq. 6-8) in the DCM, and Manning's equation with estimated depths were used in time of concentration calculations. “Runoff Coefficients for Rational Method”, Table 6-6 in the DCM, was utilized as a guide in estimating runoff coefficient and Percent Impervious values; a copy is included in the **Appendix**. Peak runoff discharges were calculated for each drainage sub-basin for both the 5-year storm event and the 100-year storm event with the Rational Method formula, (Eq. 6-5) in the DCM.<sup>11</sup>

The “Water Quality Control Volume procedure, Section 3.2.3 of the *Urban Drainage and Flood Control District Drainage Criteria Manual, Volume 3* (UDFCD)<sup>12 13</sup> method was used for water quality volume calculations with the aid of the “UD-BMP\_v3.06” spreadsheet developed by the Urban Drainage and Flood Control District. Storm routing calculation through the proposed water quality basin was performed using triangular hydrographs based on the rational method peak discharges and times of concentrations with the aid of the detention design spreadsheet, “UD-Detention\_v3.07”, developed by the Urban Drainage and Flood Control District.<sup>14</sup>

## 4 Drainage Facility Design

### 4.1 General Concept

The intent of the drainage concept presented in this Final Drainage Report is to maintain the existing drainage patterns on the site while addressing water quality requirements for the site. Major and minor storm flows will continue to be safely conveyed through the site and downstream.

The existing and proposed drainage hydrologic conditions are described in more detail below. Input data and results for all calculations are included in the **Appendix**. Drainage maps for the hydrology are also included in the **Appendix**.

### 4.2 Specific Details

#### 4.2.1 Existing Hydrologic Conditions

The off-site drainage area east of the site, Basin OSA1, contains the existing RV storage area, pond area, landscape area and railroad right-of-way. The sub-basin is 4.05 acres in area and drains westerly and into the existing offsite water quality pond. Basin OSA1 generates peak storm runoff discharges of  $Q_5 = 7.6$  cfs and  $Q_{100} = 16.6$  cfs (existing flows) which enter the pond, which is a full-infiltration type Sand Filter basin. The Water Quality Capture Volume (WQCV) is captured and percolates into the ground. Runoff greater than the WQCV from the existing pond discharges into the site by way of the 12” CMP and emergency spillway with peak flows of  $Q_5 = 2.8$  cfs and  $Q_{100} = 12.1$  cfs (existing flows), which enter the site into sub-basin A1. In the 100 year rainfall event, approximately 4.4 cfs exits the pond from the existing 12” CMP and is discharged onto the site, while approximately 7.7 cfs exits through the emergency spillway crest and into the site. Once in the site, these flows continue to drain to the northwest corner.

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8 CS DCM Vol 1  
 9 CS DCM Vol 2  
 10 WSS  
 11 DCM  
 12 UDFCD V.2  
 13 UDFCDV.3  
 14 UDFCD

The stated flows entering the site from sub-basin OSA1 are an existing condition which is greater than flows from a pre-developed condition. However, this site and the easterly offsite property was previously used for many years as a landscape nursery with significant areas dedicated to greenhouse buildings and landscape operations characterized with packed gravel drives and material storage. In the current condition, the greenhouse buildings have been removed and storage area are gone. The current offsite condition with recycled asphalt surface is not significantly different than the previously existing (at least 38 years) with greenhouses and compacted gravel. The developed flows from the sub-basin OSA1 travel through the site and enters Crystal Creek located approximately 250 feet northwest of the site. The Crystal Creek Basin extends more than two miles to the north of the site with a contributing area of more than 550 acres upstream of the site. The peak flows from off-site sub-basin OSA1, along with the on-site flows discussed below, reach Crystal Creek and pass downstream well before the peak discharges of the much larger basin travel by the area. The small increase flows have no effect on peak flows in Crystal Creek and do not present a hazard to the downstream properties, drainage basin, or drainageways.

Existing sub-basin A1 is 1.42 acres in area located on the northern portion of the site and contained a gravel drive and meadow area. The sub-basin accepts the offsite flows from sub-basin OSA1 as described above. Sub-basin A1 produces peak discharges of  $Q_5 = 0.7$  cfs and  $Q_{100} = 3.4$  cfs (existing flows) which drain westerly to the existing gravel drive and ditch, joining the off-site flows from Basin OSA1. The combined flows of  $Q_5 = 3.5$  cfs and  $Q_{100} = 15.5$  cfs (existing flows) continue towards the northwest corner of the site to existing Design Point 1 (DP1). The flows continue westerly offsite, through the adjacent property and into Crystal Creek.

Existing sub-basin A2, located in the northwestern portion of the site and just south of sub-basin A1, is 0.32 acres in area. Sub-basin A2 contains a gravel drive, leach field and meadow/pasture area. Peak storm runoff rates are  $Q_5 = 0.2$  cfs and  $Q_{100} = 1.0$  cfs (existing flows) which drain westerly into the adjacent property, in a combination of sheet flow and an area of concentrated flow in the southwest edge of the basin. The concentrated flow occurs in an existing swale flowing to the west through the adjacent property towards Crystal Creek. Flows from existing Design Point 1 also flow into this swale. The combined discharges of Design Point 1 and sub-basin A2 are  $Q_5 = 3.7$  cfs and  $Q_{100} = 16.3$  cfs (existing flows). The flows continue westerly offsite, through the adjacent property and into Crystal Creek.

Existing sub-basin B1 (2.15 acres) is comprised of the southerly and westerly portion of the site and contains the existing shop, garage, and shed buildings, paved parking and walk areas, gravel drive and parking areas and open meadow areas with tiered railroad tie walls. The sub-basin generates flows of  $Q_5 = 2.0$  cfs and  $Q_{100} = 6.1$  cfs (existing flow), which drains westerly and exit the site by sheet flow into the adjacent property. These flows, along with the flows from existing DP1 and sub-basin A2, continue offsite to Crystal Creek and into Monument Reservoir and Monument Creek as described in Section 2.3 above.

The **Existing Drainage Map** depicts the existing topographic mapping, drainage basin delineations, drainage patterns, existing drives, drainage facilities, and runoff quantities with a data table including drainage areas and flow rates.

#### 4.2.2 Proposed Hydrologic Conditions

Water quality treatment for the new disturbed and impervious areas on the site will be provided by two proposed Sand Filter Basins which will capture, contain, treat and release the Water Quality Capture Volume (WQCV). No detention for flood control is being provided because the downstream effects of the minor increases in peak flow rates are negligible. Several greenhouse buildings have been removed from the site in recent months and additional buildings are to be removed in the proposed site plan. This building removal partially offsets the additional parking areas on the site. The parking area surface is not completely impervious, but rather compacted recycled asphalt material, which allows a degree of perviousness on the site. Additionally, the developed flows from the offsite and onsite sub-basins travel through the site and enters Crystal Creek located approximately 250 feet northwest of the site. The Crystal Creek Basin extends more than two miles to the north of the site with a contributing area of more than 550 acres upstream of the site. The peak flows from off-site sub-basin OSA1, along with the on-site flows discussed below, reach Crystal

Creek and pass downstream well before the peak discharges of the much larger basin travel by the area. The small increase flows have no effect on peak flows in Crystal Creek and do not present a hazard to the downstream properties, drainage basin, or drainageways and no storm detention is required in addition to the WQCV.

The off-site drainage basin, OSA1 (4.05 acres), will continue to drain into the site as in existing conditions. The discharges entering the site from the existing offsite water quality pond are  $Q_5 = 2.8$  cfs and  $Q_{100} = 12.1$  cfs (proposed flow) which enter proposed onsite sub-basin A1. The existing pond discharges into the site by way of the 12" CMP and emergency spillway with peak flows of  $Q_5 = 2.8$  cfs and  $Q_{100} = 12.1$  cfs (existing flows), which enter the site into sub-basin A1. In the 100 year rainfall event, approximately 4.4 cfs exits the pond from the existing 12" CMP and is discharged onto the site, while approximately 7.7 cfs exits through the emergency spillway crest and into the site. Once in the site, these flows continue to drain to the northwest corner.

Proposed sub-basin A1 (1.42 acres) will be more developed with RV parking area and the parking surface will be stabilized with recycled asphalt product. The sub-basin will continue to accept the off-site flows from sub-basin OSA1 to the east. These flows will continue to be conveyed in the existing drainage ditch/swale located along the east edge of the existing gravel drive. The developed discharges generated by sub-basin A1 are  $Q_5 = 2.7$  cfs and  $Q_{100} = 6.0$  cfs (proposed flow), which drains westerly to the existing ditch/swale and combine with the off-site flows from sub-basin OSA1 until reaching the northwest corner of the site at Design Point 1 (DP1). The combined discharges at Design Point 1 (DP 1) are  $Q_5 = 5.5$  cfs and  $Q_{100} = 18.1$  cfs (proposed flow). These flows will enter the proposed water quality sand filter basin to be located in the northwest corner of the site. The proposed water quality sand filter basin will be 780 cubic feet in volume and have a sand surface area of 340 square feet. The proposed water quality basin will have an under drain with metered outfall, a 12" hdpe outfall pipe and an 15' wide emergency spillway stabilized with soil covered rip-rap. Outflows from the pond are  $Q_5 = 3.8$  cfs and  $Q_{100} = 16.0$  cfs (proposed flow), which represents an increase from existing conditions of 0.3 cfs in the 5-year rainfall event and 0.5 cfs in the 100-year event. In the 100-year rainfall event, approximately 1.6 cfs exits the pond from the 12" standpipe inlet located just above the WQCV elevation in the pond and the remaining 14.4 cfs exits by way of the emergency spillway. Pond outflows will be discharged to a culvert and swale in sub-basin A2 directing the flows to the south and then to the adjacent property to the west.

Proposed sub-basin A2 (0.32 acres) contains the existing gravel drive, existing leach field, meadow area, and proposed drain swale. The sub-basin drains to the west and also accepts the flows from DP1 sand filter basin located to the north. The discharges generated by sub-basin A2 are  $Q_5 = 0.2$  cfs and  $Q_{100} = 1.0$  cfs (proposed flow), which drains westerly to the proposed swale and then combines with the flows from DP1 pond outfall. The combined discharges of pond outfall and sub-basin A2 are  $Q_5 = 4.0$  cfs and  $Q_{100} = 17.0$  cfs (proposed flows). This combined flow represents a slight increase from existing conditions of 0.3 cfs in the 5-year rainfall event and 0.7 cfs in the 100-year event. These flows will enter the adjacent property top the west at DP4. The proposed drain swale will be stabilized with permanent turf reinforcement mat and feature a rip-rap spreading apron at the site property boundary. The swale flows will be spread out over a large width leading to the downstream existing swale, reducing velocities depth at the property line and is a suitable outfall of the pond outflows. These flows will continue westerly through the adjacent property and flow towards Crystal Creek, the same as existing conditions. The existing stable flow path through the adjacent site is adequate to carry the existing and developed flows. Flow velocities in the existing flow path are not erosive and require no special lining. The flow path delivers the flows west to Crystal Creek.

Proposed sub-basin B1 (0.86 acres) is comprised of the southeastern portion of the site. The sub-basin will contain the proposed new shop building and paved parking. The existing garage and shed will be removed and the existing gravel drive will be reconfigured. A concrete parking area will also be installed adjacent to the building. The developed discharges from sub-basin B1 are  $Q_5 = 1.6$  cfs and  $Q_{100} = 3.6$  cfs (proposed flows). These flows travel overland to the west and will be directed by new drainage swales on the north side and west side of the new building to a new water quality sand filter basin at DP2. The proposed water quality sand filter basin will be 550 cubic feet in volume and have a sand surface area of 260 square feet. The proposed water quality basin will have an under

drain with metered outfall, a 12" hdpe outfall pipe and an 8' wide emergency spillway stabilized with soil covered rip-rap. The sand filter basin will discharge to the west into sub-basin B2 with peak outflows of  $Q_5 = 0.8$  cfs and  $Q_{100} = 2.4$  cfs (proposed flow). In the 100-year rainfall event, approximately 1.4 cfs exits the pond from the 12" standpipe inlet located just above the WQCV elevation in the pond and the remaining 1.0 cfs exits by way of the emergency spillway. These flows continue to drain west over the existing meadow surface into sub-basin B2.

Developed sub-basin B2 (1.29 acres) is the western section of the site that is well vegetated. Sub-basin B2 will remain the same as in existing conditions. The sub-basin generates storm runoff peak discharges of  $Q_5 = 0.8$  cfs and  $Q_{100} = 3.4$  cfs (proposed flows), which join the flows from sub-basin B1. The combined flows of DP 2 Pond outflows and sub-basin B2 are  $Q_5 = 1.6$  cfs and  $Q_{100} = 5.8$  cfs (proposed flows), which represents a decrease from existing conditions of 0.4 cfs in the 5-year rainfall event and 0.3 cfs in the 100-year event. These combined flows leave the site to west in sheet flow mode as in existing conditions.

### 4.3 Erosion Control

During future construction, best management practices (BMP's) for erosion control will be employed based on the previously referenced City of Colorado Springs Drainage Criteria Manual Volume 2 and the Erosion Control Plan for the site. During Construction, silt fencing, sediment control log, vehicle tracking control and concrete washout area will be in place to minimize erosion from the site. Silt Fencing will be placed along the northern and western sides of the disturbed areas. This will inhibit suspended sediment from leaving the site during construction. Silt fencing is to remain in place until the parking area is stabilized with the recycled asphalt and until vegetation is reestablished in the other disturbed areas which are to be reseeded. Vehicle tracking control will be placed at the access point in the private driveway connecting to Monument Lake Road. Inlet protection will be placed at the water quality pond outlet locations. BMP's will be utilized as deemed necessary by the contractor, engineer, owner, or County inspector and are not limited to the measures described above. The water quality sand filters will also serve as sediment traps until construction is complete.

### 4.4 Water Quality Enhancement Best Management Practices

The Sand Filter Basins described above will provide storage for the Water Quality Capture Volume (WQCV) for the site. A Grading and Erosion Control Plan for the construction of the site has been prepared in accordance with the provisions of the DCM. Placement of construction stormwater BMP's will as required by the plan will limit soil erosion and deposition by stormwater flowing over the site.

The El Paso County Engineering Criteria Manual (Appendix I, Section I.7.2 ) requires the consideration of a "Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long term source controls". The Four Step Process is incorporated in this project and the elements are discussed below.

1) Runoff Reduction Practices are employed in this project. Impervious surfaces have been reduced as much as practically possible. Some of the existing structures on the site are being removed, reducing impervious surfaces. There is only minimal concrete or other hard surfaces proposed. RV storage area will be stabilized with recycled asphalt surfacing, which remains a partially pervious surface. Minimized Directly Connected Impervious Areas (MDCIA) is employed on the project because runoff passes through the western open space meadow area before leaving the site.

2) All drainage paths on the site are stabilized with pavement or appropriate landscape treatment. The water quality ponds are intended to intercept flows from developed areas. Additionally, the pond outflow points will have rip rap protection.

3) The project contains no potentially hazardous uses. All developed areas drain into a proposed a WQCV BMP.

4) The site contains no storage of potentially harmful substances or use of potentially harmful substances. No Site Specific or Other Source Control BMP's are required.

## **5 Opinion of Probable Cost for Drainage Facilities**

There is no public or private storm drain system proposed for development of this site. The private water quality facilities will be constructed as part of the site grading plan at the time of building construction. The property owner is responsible for construction and maintenance of the water quality facilities.

## **6 Drainage and Bridge Fees**

The site is not being platted. No Drainage or Bridge Fees are due for this project.

## **7 Conclusion**

This Final Drainage Report presents existing and proposed drainage conditions for the proposed Monument Small Engine Storage project. The development will have negligible and inconsequential effects on the existing site drainage and drainage conditions downstream. Water Quality treatment will be provided. The proposed project will not, with respect to stormwater runoff, negatively impact the adjacent properties and downstream properties.

# References

- NRCS Web Soil Survey.* United States Department of Agriculture, Natural Resources Conservation Service ("<http://websoilsurvey.nrcs.usda.gov/app/WebSoilSurvey.aspx>", accessed March, 2018).
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*Drainage Criteria Manual (Volume 2)*. Urban Drainage and Flood Control District (Denver, Colorado: Urban Drainage and Flood Control District, Rev. April, 2008).

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## **8 General Maps and Supporting Data**

- Vicinity Map
- Portions of Flood Insurance Rate Map
- Portion of Drainage Area Identification Study Map
- NRCS Soil Map and Tables
- SCS Soil Type Descriptions
- Hydrologic Soil Group Map and Tables



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Percent Imperviousness Calculation for proposed Lots 1 & 2

## **10 Hydraulic Calculations**

Sand Filter Basin Calculations  
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## **11 Report Maps**

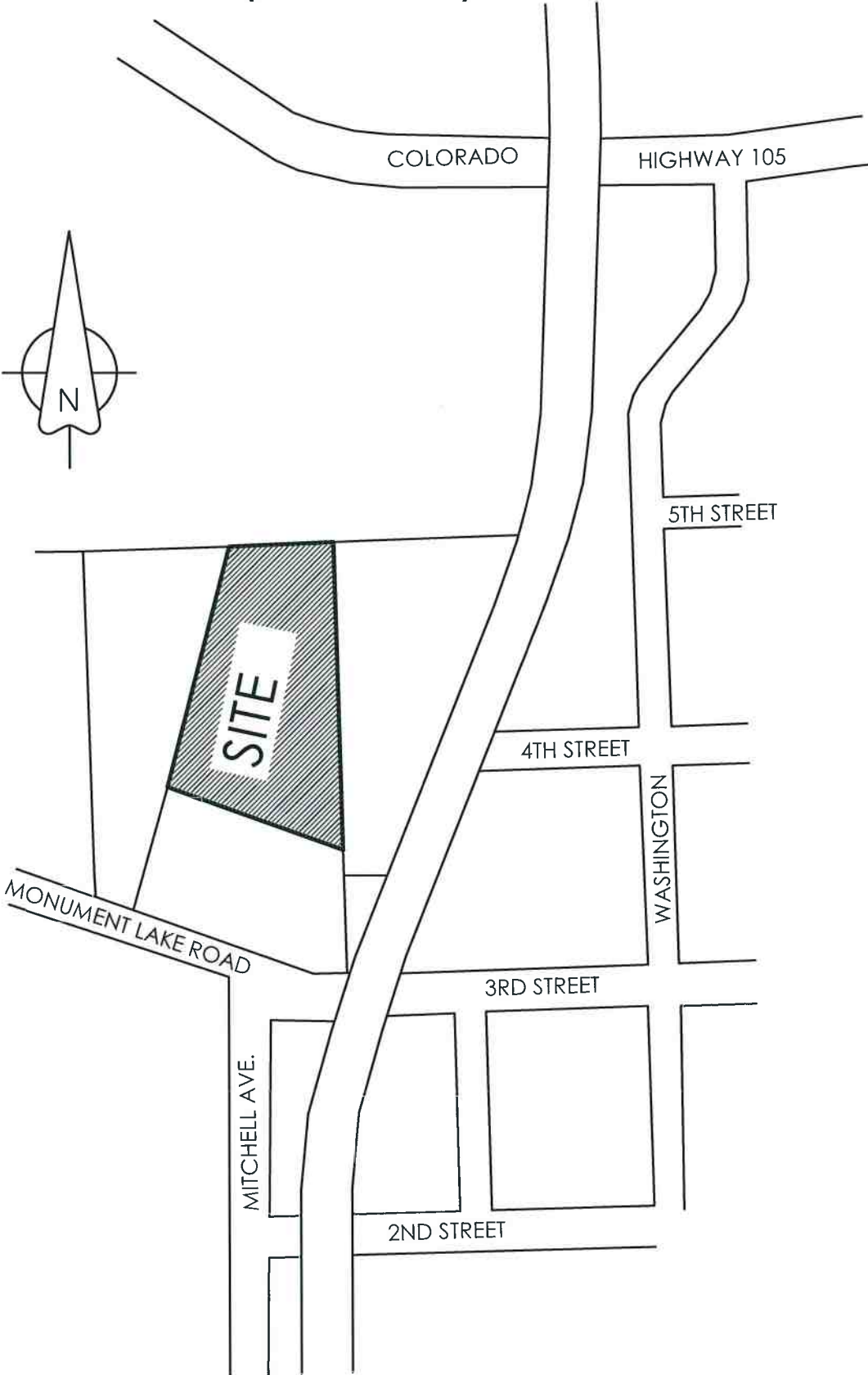
Existing Condition Hydraulic Analysis Map (Map Pocket)  
Proposed Condition Hydraulic Analysis Map (Map Pocket)

# | Appendices

## **8 General Maps and Supporting Data**

- Vicinity Map
- Portions of Flood Insurance Rate Map
- Portion of Drainage Area Identification Study Map
- NRCS Soil Map and Tables
- SCS Soil Type Descriptions
- Hydrologic Soil Group Map and Tables

# VICINITY MAP (no scale)





APPROXIMATE SCALE IN FEET



**NATIONAL FLOOD INSURANCE PROGRAM**

**FIRM  
FLOOD INSURANCE RATE MAP  
EL PASO COUNTY,  
COLORADO AND  
INCORPORATED AREAS**

**PANEL 260 OF 1300**  
(SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS:  
COMMUNITY

EL PASO COUNTY,  
UNINCORPORATED AREAS  
MONUMENT, TOWN OF  
PALMER LAKE, TOWN OF

NUMBER PANEL SUFFIX  
080064 0260 F  
080064 0260 F  
080064 0260 F

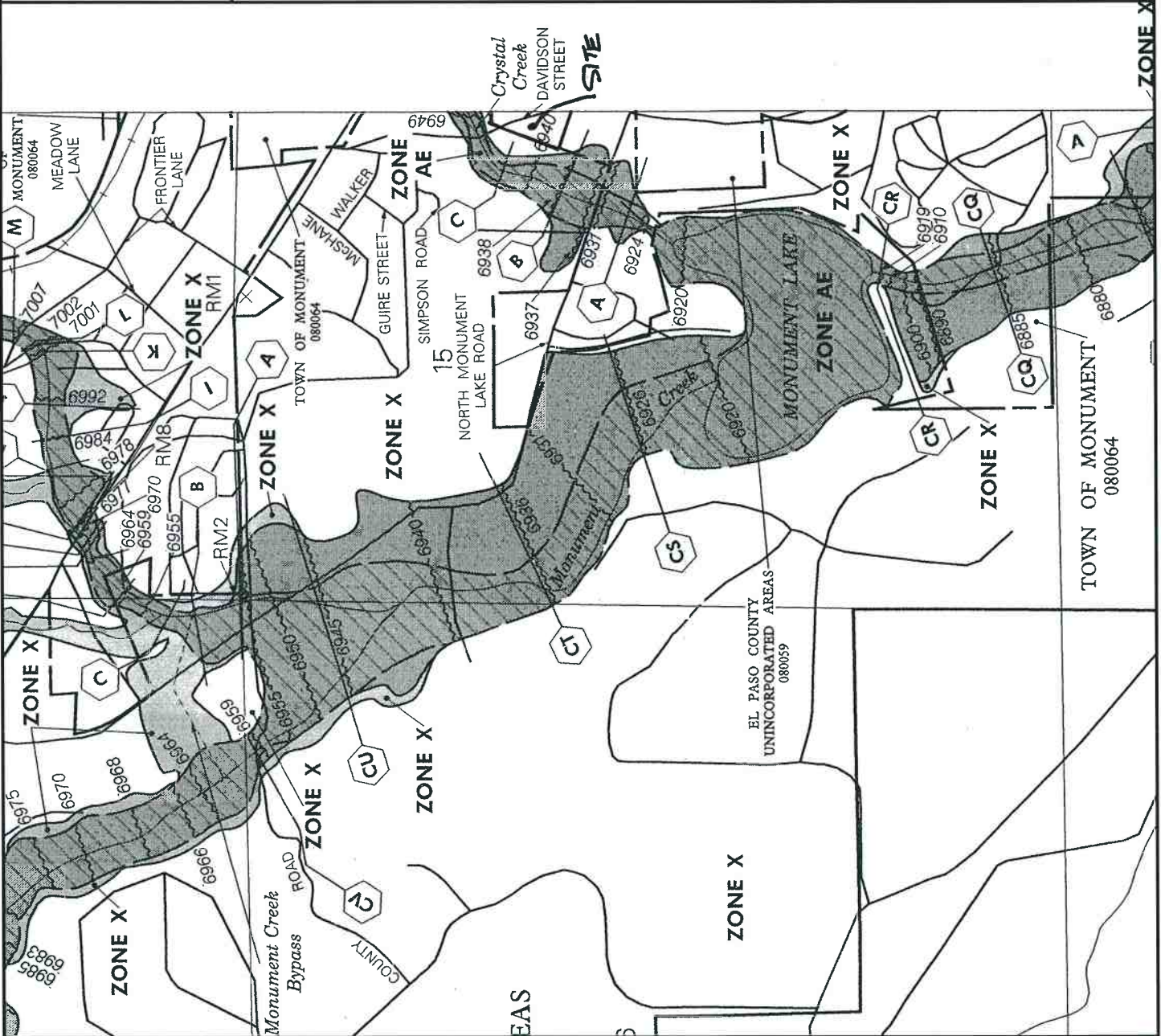
**MAP NUMBER  
08041C0260 F**

**EFFECTIVE DATE:  
MARCH 17, 1997**



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at [www.msc.fema.gov](http://www.msc.fema.gov)



ZONE X

ZONE AE

ZONE CR

ZONE CQ

ZONE X

ZONE X

ZONE X



**NATIONAL FLOOD INSURANCE PROGRAM**

**FIRM FLOOD INSURANCE RATE MAP**  
EL PASO COUNTY,  
COLORADO AND  
INCORPORATED AREAS

**PANEL 276 OF 1300**  
(SEE MAP INDEX FOR PANELS NOT PRINTED)

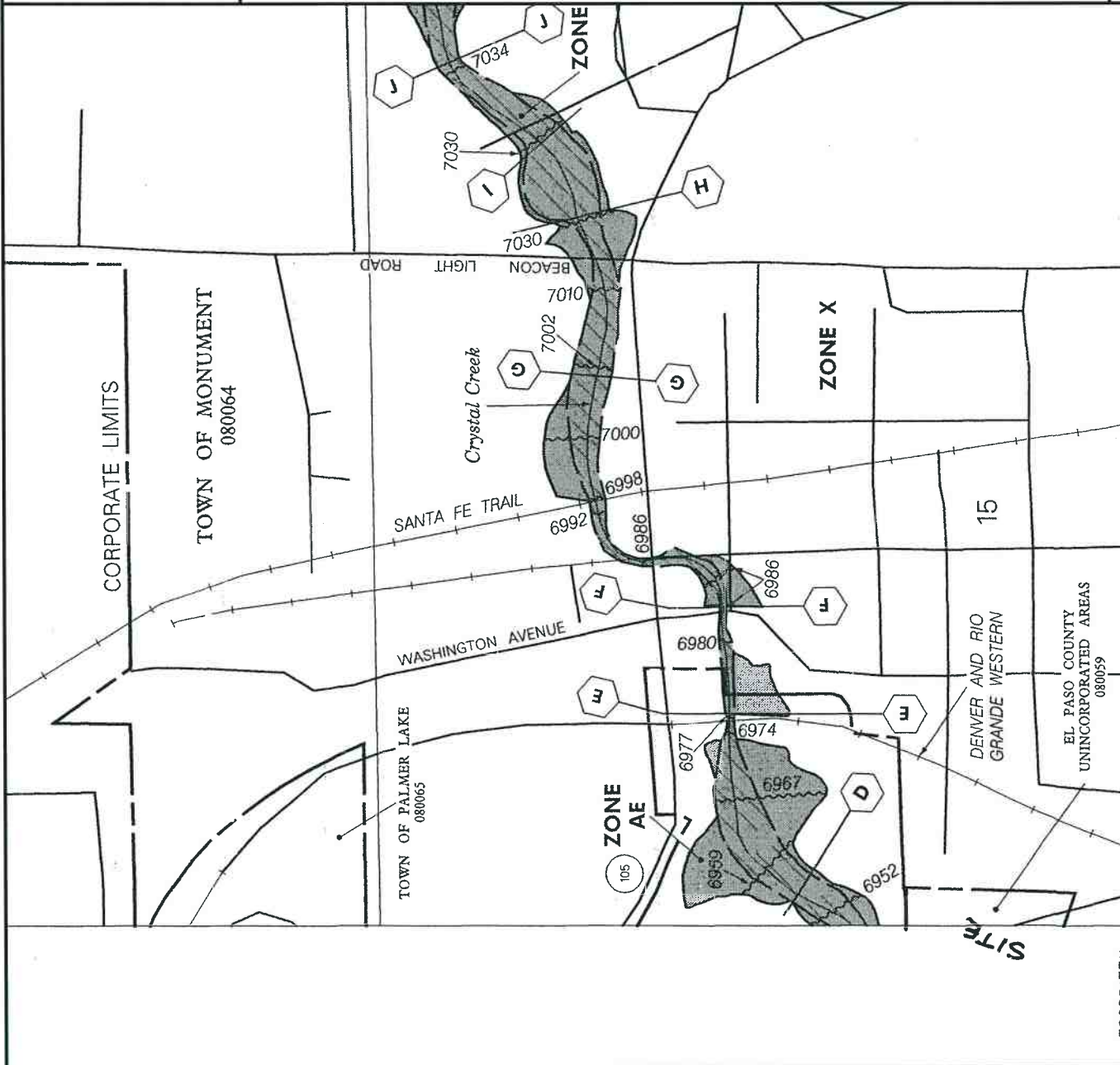
CONTAINS:	COMMUNITY	NUMBER	PANEL	SUFFIX
EL PASO COUNTY, UNINCORPORATED AREAS		080068	0276	F
TOWN OF PALMER LAKE, TOWN OF		080065	0276	F

**MAP NUMBER**  
08041C0276 F  
**EFFECTIVE DATE:**  
MARCH 17, 1997



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at [www.msc.fema.gov](http://www.msc.fema.gov)



39°05'37"  
104°52'30"



ARPENDER CREEK  
PLPL0400

BALD MOUNTAIN  
PLPL0200

CRYSTAL CREEK  
FOMO 5300

PALMER LAKE

PALMER LAKE  
FOMO5400

DIRTY WOMAN CREEK  
FOMO5200

HERRY MOUNTAIN  
FOMO5600 SITE

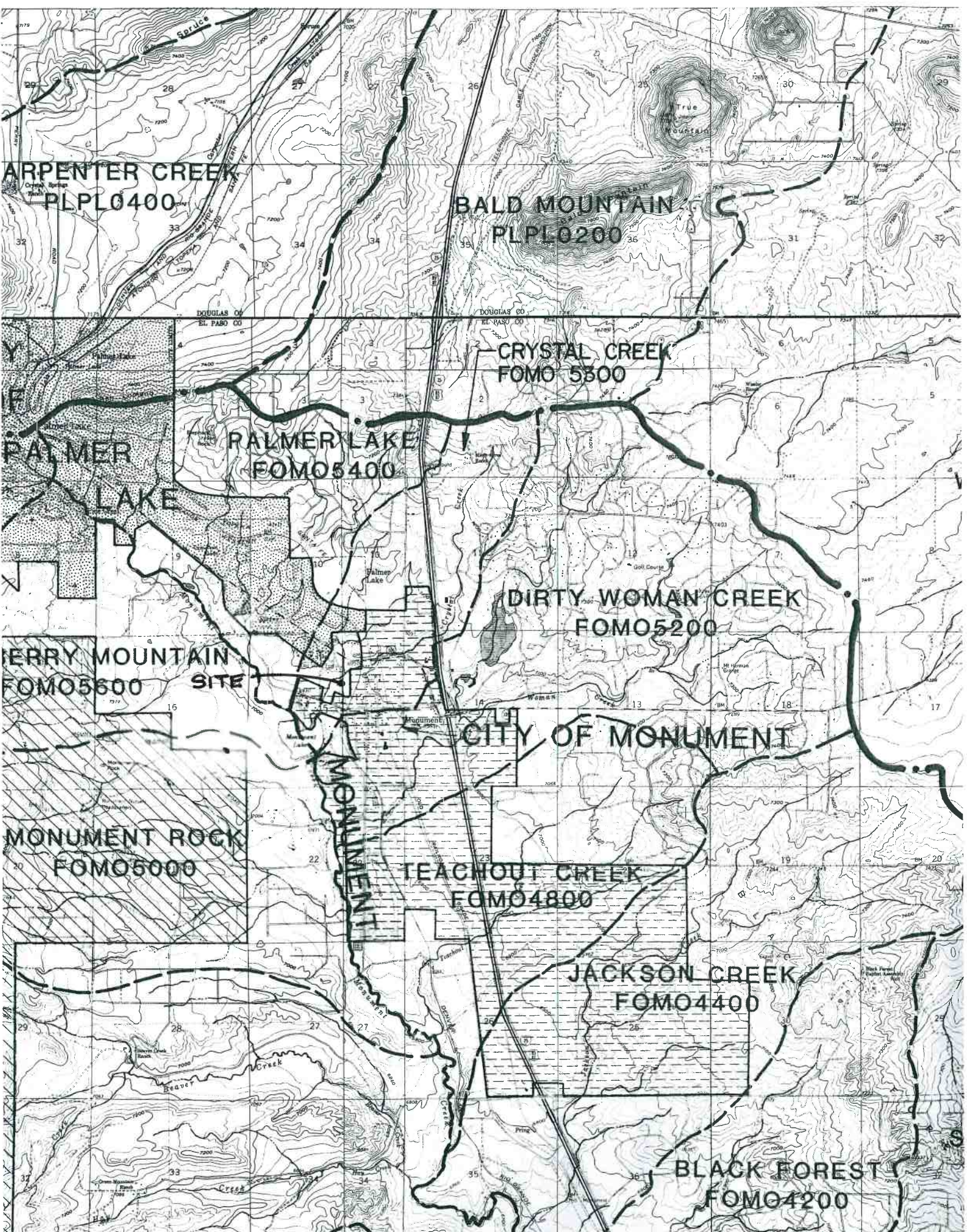
CITY OF MONUMENT

MONUMENT ROCK  
FOMO5000

TEACHOUT CREEK  
FOMO4800

JACKSON CREEK  
FOMO4400

BLACK FOREST  
FOMO4200





Soil Map—El Paso County Area, Colorado  
(Monument Small Engine)



Soil Map may not be valid at this scale.

Map Scale: 1:1,070 if printed on A portrait (8.5" x 11") sheet.



## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
































































Soil Survey Area: El Paso County Area, Colorado  
Survey Area Data: Version 15, Oct 10, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Feb 22, 2014—Mar 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## MAP LEGEND

 Area of Interest (AOI)	 Area of Interest (AOI)	 Spoil Area
 Soils	 Soil Map Unit Polygons	 Stony Spot
 Soil Map Unit Lines	 Soil Map Unit Lines	 Very Stony Spot
 Soil Map Unit Points	 Soil Map Unit Points	 Wet Spot
 Special Point Features	 Special Point Features	 Other
 Blowout	 Blowout	 Special Line Features
 Borrow Pit	 Borrow Pit	 Streams and Canals
 Clay Spot	 Clay Spot	 Transportation
 Closed Depression	 Closed Depression	 Rails
 Gravel Pit	 Gravel Pit	 Interstate Highways
 Gravelly Spot	 Gravelly Spot	 US Routes
 Landfill	 Landfill	 Major Roads
 Lava Flow	 Lava Flow	 Local Roads
 Marsh or swamp	 Marsh or swamp	 Background
 Mine or Quarry	 Mine or Quarry	 Aerial Photography
 Miscellaneous Water	 Miscellaneous Water	
 Perennial Water	 Perennial Water	
 Rock Outcrop	 Rock Outcrop	
 Saline Spot	 Saline Spot	
 Sandy Spot	 Sandy Spot	
 Severely Eroded Spot	 Severely Eroded Spot	
 Sinkhole	 Sinkhole	
 Slide or Slip	 Slide or Slip	
 Sodic Spot	 Sodic Spot	

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
1	Alamosa loam, 1 to 3 percent slopes	0.1	1.8%
92	Tomah-Crowfoot loamy sands, 3 to 8 percent slopes	3.2	98.2%
<b>Totals for Area of Interest</b>		<b>3.3</b>	<b>100.0%</b>



cludes some of each of the two or more dominant soils, and the pattern and proportion are somewhat similar in all areas. Elbeth-Pring complex, 5 to 30 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils that could be mapped individually but are mapped as one unit because there is little value in separating them. The pattern and proportion of the soils are not uniform. An area shown on the map has at least one of the dominant (named) soils or may have all of them. Olney and Vona soils, eroded, is an undifferentiated group in this survey area.

Most map units include small, scattered areas of soils other than those that appear in the name of the map unit. Some of these soils have properties that differ substantially from those of the dominant soil or soils and thus could significantly affect use and management of the map unit. These soils are described in the description of each map unit. Some of the more unusual or strongly contrasting soils that are included are identified by a special symbol on the soil map.

Most mapped areas include places that have little or no soil material and support little or no vegetation. Such places are called *miscellaneous areas*; they are delineated on the soil map and given descriptive names. Pits, gravel is an example. Some of these areas are too small to be delineated and are identified by a special symbol on the soil map.

The military impact area described in some map units consists of a large area on the Fort Carson Military Reservation. It is used as an artillery and bombing target area. This area has not been surveyed, but most of the soils mapped adjacent to the area are in the Heldt, Kim, Midway, Razor, and Wiley series. It is estimated that most of the impact area is Razor-Midway complex.

The acreage and proportionate extent of each map unit are given in table 4, and additional information on properties, limitations, capabilities, and potentials for many soil uses is given for each kind of soil in other tables in this survey. (See "Summary of tables.") Many of the terms used in describing soils are defined in the Glossary.

## Soil descriptions

**1—Alamosa loam, 1 to 3 percent slopes.** This deep, poorly drained soil formed in alluvium on flood plains and fans. Elevation ranges from 7,200 to 7,700 feet. The average annual precipitation is about 18 inches, the average annual air temperature is about 42 degrees F, and the average frost-free period is about 120 days.

Typically, the surface layer is dark gray loam about 6 inches thick. The subsoil is clay loam about 27 inches thick; it is very dark gray in the upper part and gray in the lower part. The substratum is dark greenish gray and light gray sandy clay loam and sandy loam. Mottles are common in the subsoil and substratum.

Included with this soil in mapping are small areas of Ellicott loamy coarse sand, 0 to 5 percent slopes;

Cruckton sandy loam, 1 to 9 percent slopes; Peyton sandy loam, 1 to 5 percent slopes; and Pring coarse sandy loam, 3 to 8 percent slopes.

Permeability of this Alamosa soil is moderately slow. Effective rooting depth is 60 inches or more. Available water capacity is high. Organic matter content of the surface layer is high. This soil has a high water table, usually between May and October. Surface runoff is slow, and the hazard of erosion is slight.

This soil is used mostly for native hay or pasture.

The potential plant community is mainly slender wheatgrass, Baltic rush, Nebraska sedge, timothy, and reedgrasses. Willows are a part of the plant community.

If the range has deteriorated, it consists mostly of Kentucky bluegrass and willows. If overgrazing is severe, denuding of the soil and gulying are possible and reestablishment of a good plant cover is very difficult. Where seeding is practical, smooth brome, orchardgrass, Garrison creeping foxtail, or reed canarygrass should be used.

Wet areas of this soil are well suited to shallow water developments, which encourage wetland wildlife such as waterfowl and a number of shore birds. Because of the availability of moisture, this soil provides excellent waterfowl nesting cover. Rangeland wildlife, such as deer and cottontail, use the areas where excellent cover is provided by willows, rushes, and other wetland vegetation. Wildlife on this soil can best be aided by using proper livestock grazing practices and allowing natural vegetation, such as willows and cattails, to grow.

This soil has poor potential for homesites. The main limitations for this use are a high water table and the hazard of flooding. Capability subclass Vw.

**2—Ascalon sandy loam, 1 to 3 percent slopes.** This deep, well drained soil formed in mixed alluvium and wind-laid material on uplands. Elevation ranges from 5,500 to 6,500 feet. The average annual precipitation is about 15 inches, the average annual air temperature is about 48 degrees F, and the average frost-free period is about 140 days.

Typically, the surface layer is brown sandy loam about 8 inches thick. The subsoil is brown, yellowish brown, and pale brown sandy clay loam about 22 inches thick. The substratum is calcareous, very pale brown sandy loam and loamy sand.

Included with this soil in mapping are small areas of Bresser sandy loam, 0 to 3 percent slopes; Olney sandy loam, 0 to 3 percent slopes; Vona sandy loam, 1 to 3 percent slopes; and Fort Collins loam, 0 to 3 percent slopes.

Permeability of this Ascalon soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Organic matter content of the surface layer is medium. Surface runoff is slow, and the hazards of erosion and soil blowing are moderate.

This soil is used mainly as cropland.

A typical rotation is wheat and summer fallow. Summer fallow is necessary because rainfall is insufficient for yearly cropping. Feed grains such as millet are used as a



strength. Special designs for buildings and roads are required to offset these limitations. Methods of sewage disposal other than septic tank absorption fields are needed because of the limited depth to bedrock. Capability subclass VIe.

**92—Tomah-Crowfoot loamy sands, 3 to 8 percent slopes.** These gently sloping to moderately sloping soils are on alluvial fans, hills, and ridges in the uplands. Elevation ranges from about 7,300 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 42 degrees F, and the average frost-free period is about 120 days.

The Tomah soil makes up about 50 percent of the complex, the Crowfoot soil about 30 percent, and other soils about 20 percent.

Included with these soils in mapping are areas of Elbeth sandy loam, 3 to 8 percent slopes; Kettle gravelly loamy sand, 3 to 8 percent slopes; and Pring coarse sandy loam, 3 to 8 percent slopes.

The Tomah soil is deep and well drained. It formed in alluvium or residuum derived from arkose beds. Typically, the surface layer is dark grayish brown loamy sand about 10 inches thick. The subsurface layer is very pale brown coarse sand about 12 inches thick. The subsoil, about 26 inches thick, is a matrix of very pale brown coarse sand in which are embedded many thin bands and lamellae of pale brown coarse sandy clay loam. The substratum is very pale brown coarse sand to a depth of 60 inches or more.

Permeability of the Tomah soil is moderately rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is slow, and the hazard of erosion is slight to moderate.

The Crowfoot soil is deep and well drained. It formed in sediment weathered from arkosic sandstone. Typically, the surface layer is grayish brown loamy sand about 12 inches thick. The subsurface layer is very pale brown sand about 11 inches thick. The subsoil is light yellowish brown sandy clay loam about 13 inches thick. The substratum is very pale brown coarse sand to a depth of about 68 inches.

Permeability of the Crowfoot soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is slow, and the hazard of erosion is slight to moderate.

This complex is used as rangeland, for wildlife habitat, and as homesites.

Native vegetation is mainly mountain muhly, bluestem, mountain brome, needleandthread, and blue grama. These soils are subject to invasion by Kentucky bluegrass and Gambel oak. Noticeable forbs are hairy goldenrod, geranium, milkvetch, low larkspur, fringed sage, and buckwheat.

Properly locating livestock watering facilities helps to control grazing. Timely deferment of grazing is needed to protect the plant cover.

Windbreaks and environmental plantings are fairly well suited to these soils. Blowing sand and moderate available water capacity are the principal limitations for the

establishment of trees and shrubs. The soils are so loose that trees need to be planted in shallow furrows and plant cover needs to be maintained between the rows. Supplemental irrigation may be needed to insure survival. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, and Siberian elm. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

These soils are best suited to habitat for openland wildlife such as pronghorn antelope and sharp-tailed grouse. Although sharp-tailed grouse are not plentiful, they could be encouraged on these soils, especially where brush species are interspersed with grasses and forbs. If these soils are used as rangeland, wildlife production can be increased by managing livestock grazing to preclude overuse of the more desirable grass species and depletion of the various brush species.

These soils have good potential for use as homesites. The main limitation of the Crowfoot soil is frost-action potential. Roads and streets need to be designed to minimize frost-heave damage. Maintaining the existing vegetation on building sites during construction helps to control erosion. Capability subclass IVe.

**93—Tomah-Crowfoot loamy sands, 8 to 15 percent slopes.** These moderately sloping to strongly sloping soils are on alluvial fans, hills, and ridges in the uplands. Elevation ranges from about 7,300 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 42 degrees F, and the average frost-free period is about 120 days.

The Tomah soil makes up about 50 percent of the complex, the Crowfoot soil about 30 percent, and other soils about 20 percent.

Included with these soils in mapping are areas of Elbeth sandy loam, 8 to 15 percent slopes; Peyton-Pring complex, 8 to 15 percent slopes; and Kettle gravelly loamy sand, 8 to 40 percent slopes.

The Tomah soil is deep and well drained. It formed in alluvium or residuum derived from arkose beds. Typically, the surface layer is dark grayish brown loamy sand about 10 inches thick. The subsurface layer is very pale brown coarse sand about 12 inches thick. The subsoil, about 26 inches thick, consists of a matrix of very pale brown coarse sandy clay loam. The substratum is very pale brown coarse sand to a depth of 60 inches or more.

Permeability of the Tomah soil is moderately rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is medium, and the hazard of erosion is moderate. Some gullies are present in some drainageways and along stock trails.

The Crowfoot soil is deep and well drained. It formed in sediment weathered from arkosic sandstone. Typically, the surface layer is grayish brown loamy sand about 12 inches thick. The subsurface layer is very pale brown sand about 11 inches thick. The subsoil is light yellowish brown sandy clay loam about 13 inches thick. The substratum is very pale brown coarse sand to a depth of about 68 inches.

Hydrologic Soil Group—El Paso County Area, Colorado  
(Monument Small Engine)



Map Scale: 1:1,070 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge ticks: UTM Zone 13N WGS84










































Natural Resources  
Conservation Service

Web Soil Survey  
National Cooperative Soil Survey

3/5/2018  
Page 1 of 4

## MAP LEGEND

 Area of Interest (AOI)	 C
 Soils	 C/D
 Soil Rating Polygons	 D
 A	 Not rated or not available
 A/D	 Water Features
 B	 Streams and Canals
 B/D	 Transportation
 C	 Rails
 C/D	 Interstate Highways
 D	 US Routes
 Not rated or not available	 Major Roads
 Soil Rating Lines	 Local Roads
 A	 Background
 A/D	 Aerial Photography
 B	
 B/D	
 C	
 C/D	
 D	
 Not rated or not available	
 Soil Rating Points	
 A	
 A/D	
 B	
 B/D	

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:  
Coordinate System: Web Mercator (EPSG:3857)

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This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado  
Survey Area Data: Version 15, Oct 10, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Feb 22, 2014—Mar 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
1	Alamosa loam, 1 to 3 percent slopes	D	0.1	1.8%
92	Tomah-Crowfoot loamy sands, 3 to 8 percent slopes	B	3.2	98.2%
<b>Totals for Area of Interest</b>			<b>3.3</b>	<b>100.0%</b>

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.



## **9 Hydrologic Calculations**

Runoff Coefficients and Percent Imperviousness Table 6-6

Colorado Springs Rainfall Intensity Duration Frequency Table 6-5

Hydrologic Calculations Summary Form SF-1 for Existing & Developed Conditions

Hydrologic Calculations Summary 5-yr Form SF-2 for Existing & Developed Conditions

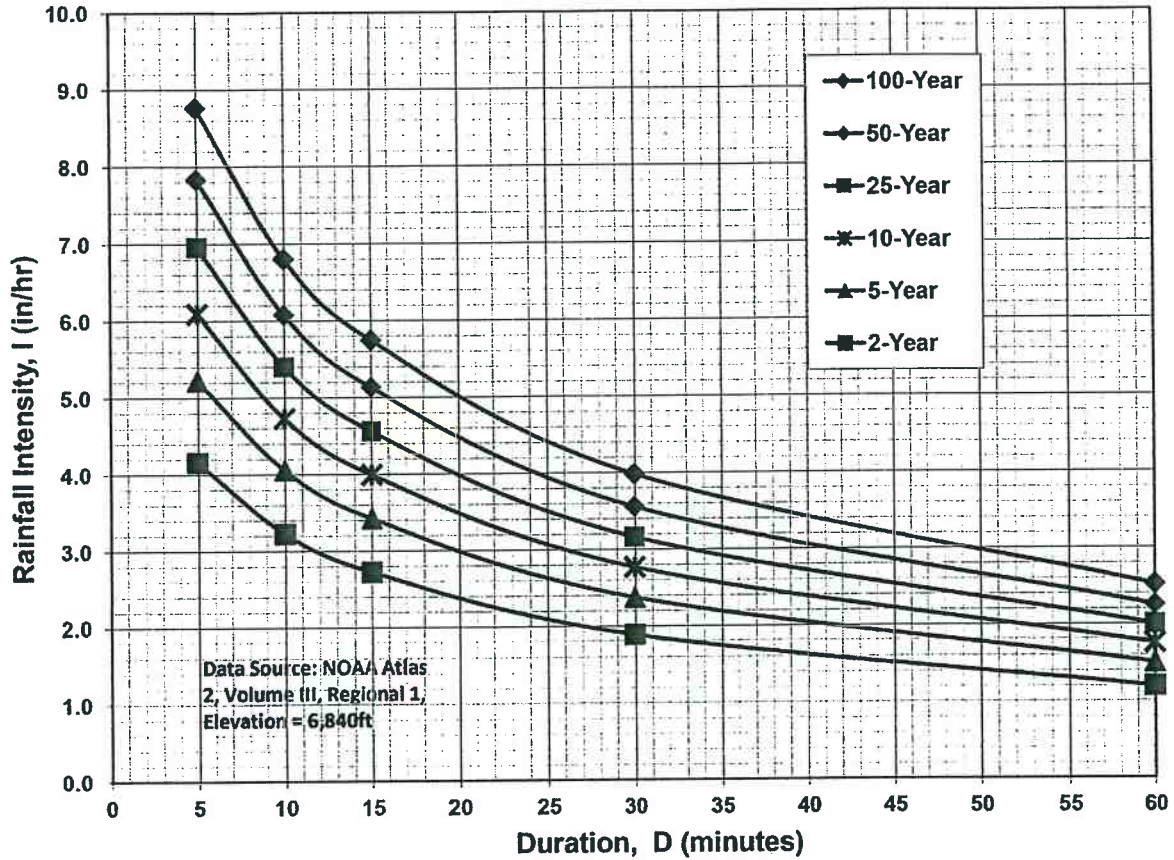
Hydrologic Calculations Summary 100-yr Form SF-2 for Existing & Developed Conditions

Percent Imperviousness Calculation for proposed Lots 1 & 2

**Table 6-6. Runoff Coefficients for Rational Method**  
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients																	
		2-year		5-year		10-year		25-year		50-year		100-year							
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D						
Business																			
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.87	0.87	0.87	0.87	0.88	0.88	0.89	0.89	
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	
Residential																			
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.62	0.65	
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.54	0.58	
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.47	0.47	0.47	0.47	0.47	0.47	0.47	0.52	0.57	
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.46	0.46	0.46	0.46	0.46	0.46	0.46	0.51	0.56	
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.44	0.44	0.44	0.44	0.44	0.44	0.44	0.50	0.55	
Industrial																			
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.72	0.74	
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.80	0.80	0.80	0.80	0.81	0.81	0.82	0.83	
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.40	0.40	0.40	0.40	0.40	0.39	0.39	0.46	0.52	
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.42	0.42	0.42	0.42	0.41	0.41	0.41	0.48	0.54	
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.54	0.58	
Undeveloped Areas																			
Historic Flow Analysis--																			
Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.38	0.38	0.38	0.38	0.31	0.36	0.36	0.45	0.51	
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.37	0.37	0.37	0.37	0.35	0.35	0.35	0.44	0.50	
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.37	0.37	0.37	0.37	0.30	0.35	0.35	0.44	0.50	
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.94	0.94	0.94	0.94	0.95	0.96	0.96	0.95	0.96	
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.51	0.51	0.51	0.51	0.48	0.51	0.51	0.55	0.59	
Streets																			
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.94	0.94	0.94	0.94	0.95	0.96	0.96	0.95	0.96	
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.70	0.70	0.70	0.68	0.70	0.70	0.72	0.72	0.74	
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.94	0.94	0.94	0.94	0.95	0.96	0.96	0.95	0.96	
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.80	0.80	0.80	0.80	0.81	0.81	0.82	0.83	
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.37	0.37	0.37	0.37	0.30	0.35	0.35	0.44	0.50	

Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



**IDF Equations**

$$I_{100} = -2.52 \ln(D) + 12.735$$

$$I_{50} = -2.25 \ln(D) + 11.375$$

$$I_{25} = -2.00 \ln(D) + 10.111$$

$$I_{10} = -1.75 \ln(D) + 8.847$$

$$I_5 = -1.50 \ln(D) + 7.583$$

$$I_2 = -1.19 \ln(D) + 6.035$$

Note: Values calculated by equations may not precisely duplicate values read from figure.

**Time of Concentration (Modified from Standard Form SF-1)**

Sub-Basin	Sub-Basin Data			Overland			Shallow Channel			Channelized				t <sub>c</sub> Check				
	Area (Acres)	C <sub>5</sub>	C <sub>100</sub> /CN	% Imp.	L <sub>0</sub> (ft)	S <sub>0</sub> (%)	t <sub>i</sub> (min)	L <sub>0t</sub> (ft)	S <sub>0t</sub> (ft/ft)	V <sub>osc</sub> (ft/s)	t <sub>i</sub> (min)	L <sub>0c</sub> (ft)	S <sub>0c</sub> (ft/ft)	V <sub>oc</sub> (ft/s)	t <sub>c</sub> (min)	L (min)	t <sub>c,alt</sub> (min)	t <sub>c</sub> (min)
EX-OSA1	4.05	0.48	0.62	62%	100	3%	7.7	335	0.022	1.5	3.7	0	0.000	0.0	0.0	435	N/A	11.4
EX-A1	1.42	0.13	0.39	8%	100	3%	11.9	50	0.140	3.7	0.2	285	0.032	3.0	1.6	435	N/A	13.7
EX-A2	0.32	0.17	0.41	13%	75	6%	8.0	94	0.074	2.7	0.6	0	0.000	0.0	0.0	169	N/A	8.6
EX-B1	2.15	0.26	0.48	27%	100	2%	12.0	195	0.041	1.4	2.3	85	0.082	4.5	0.3	380	N/A	14.7
DV-A1	1.42	0.45	0.60	58%	100	3%	8.0	50	0.140	3.7	0.2	285	0.032	3.1	1.5	435	N/A	9.7
DV-A2	0.32	0.17	0.41	14%	75	6%	8.0	94	0.074	2.7	0.6	0	0.000	0.0	0.0	169	N/A	8.5
DV-B1	0.86	0.46	0.62	54%	100	2%	9.2	50	0.060	2.4	0.3	115	0.009	1.6	1.2	265	N/A	10.8
DV-B2	1.29	0.15	0.40	10%	100	6%	9.5	50	0.020	1.0	0.8	85	0.082	3.6	0.4	235	N/A	10.8

Job No.: **61092**  
 Project: **Monument Small Engine Storage**  
 Design Storm: **5-Year Storm (20% Probability)**  
 Jurisdiction: **DCM**

Date: **6/22/18 0:39**  
 Calcs By: **D. Gorman**  
 Checked By:

**Sub-Basin and Combined Flows** (Modified from Standard Form SF-2)

DP	Sub-Basin	Area (Acres)	C5	Direct Runoff			Combined Runoff			Streetflow			Pipe Flow			Travel Time			
				t <sub>c</sub> (min)	CA (Acres)	I5 (in/hr)	Q5 (cfs)	t <sub>c</sub> (min)	CA (Acres)	I5 (in/hr)	Slope (%)	Length (ft)	Q (cfs)	Slope (%)	Mnngs	Length (ft)	D <sub>Pipe</sub> (in)	Length (ft)	V <sub>osc</sub> (ft/s)
EX DP1	EX-OSA1	4.05	0.48	11.4	1.93	3.93	7.6												
	EX-A1	1.42	0.13	13.7	0.19	3.65	0.7												
EX DP1+A2	OSA1,A1	5.47	0.17	8.6	0.05	4.36	0.2	13.7											
	EX-A2	0.32	0.17	8.6	0.05	4.36	0.2												
DV DP1	OSA1,A1,A2	5.79	0.26	14.7	0.56	3.56	2.0	13.7											
	EX-B1	2.15	0.26	14.7	0.56	3.56	2.0												
DV DP4	DV-A1	1.42	0.45	9.7	0.64	4.17	2.7	9.7											
	OSA1,A1	5.47	0.17	8.5	0.05	4.37	0.2												
DV DP2	DV-A2	0.32	0.17	8.5	0.05	4.37	0.2	8.5											
	OSA1,A1,A2	5.79	0.46	10.8	0.39	4.01	1.6												
DV DP3	DV-B1	0.86	0.15	10.8	0.19	4.02	0.8	10.8											
	DV-B2	1.29	0.15	10.8	0.19	4.02	0.8												
	B1,B2	2.15	0.15	10.8	0.19	4.02	0.8	10.8											

DCM:  $I = C1 * \ln(t_c) + C2$   
 C1: 1.5  
 C2: 7.583

Job No.: **61092**  
 Project: **Monument Small Engine Storage**  
 Design Storm: **100-Year Storm (1% Probability)**  
 Jurisdiction: **DCM**

Date: **6/22/18 0:39**  
 Calcs By: **D. Gorman**  
 Checked By:

**Sub-Basin and Combined Flows** (Modified from Standard Form SF-2)

DP	Sub-Basin	Area (Acres)	C100	Direct Runoff			Combined Runoff			Streetflow			Pipe Flow			Travel Time				
				t <sub>c</sub> (min)	CA (Acres)	I100 (in/hr)	Q100 (cfs)	t <sub>c</sub> (min)	CA (Acres)	I100 (in/hr)	Q100 (cfs)	Slope (%)	Length (ft)	Q (cfs)	Slope (%)	Mnngs n	Length (ft)	D <sub>pipe</sub> (in)	Length (ft)	V <sub>ave</sub> (ft/s)
EX DP1	EX-OSA1	4.05	0.62	11.4	2.52	6.60	16.6													
	EX-A1	1.42	0.39	13.7	0.55	6.13	3.4													
	OSA1,A1	5.47	0.41	8.6	0.13	7.33	1.0	15.5												
EX DP1+A2	OSA1,A1,A2	5.79	0.48	14.7	1.02	5.97	6.1	16.3												
	EX-B1	2.15	0.48	14.7	1.02	5.97	6.1	16.3												
DV DP1	DV-A1	1.42	0.60	9.7	0.86	7.00	6.0													
	OSA1,A1	5.47	0.60	8.5	0.13	7.33	1.0	18.1	(A1 + Pond)											
DV DP4	DV-A2	0.92	0.41	8.5	0.13	7.33	1.0													
	OSA1,A1,A2	5.79	0.62	10.8	0.53	6.74	3.6	17.0	(A2 + Pond)											
DV DP2	DV-B1	0.86	0.62	10.8	0.53	6.74	3.6													
	DV-B2	1.29	0.40	10.8	0.51	6.75	3.4	5.8	(B2 + Pond)											
DV DP3	B1/B2	2.15	0.40	10.8	0.51	6.75	3.4	10.8												

DCM:  $I = C1 * \ln(t_c) + C2$   
 C1: 2.52  
 C2: 12.735



## Sub-Basin EX-OSA1 Runoff Calculations (offsite pond inflow)

Job No.: 61092 Date: 6/22/18 0:39  
 Project: Monument Small Engine Storage Calcs by: D. Gorman  
 Checked by: \_\_\_\_\_  
 Jurisdiction: DCM Soil Type: B  
 Runoff Coefficient: Surface Type Urbanization: Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	137,170	3.15	0.57	0.59	0.63	0.66	0.68	0.7	80%
Landscaping	780	0.02	0.03	0.09	0.17	0.26	0.31	0.36	2%
Pasture/Meadow	38,537	0.88	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>176,487</b>	<b>4.05</b>	<b>0.45</b>	<b>0.48</b>	<b>0.52</b>	<b>0.57</b>	<b>0.60</b>	<b>0.62</b>	<b>62.2%</b>

176487

### Basin Travel Time

-  
Shallow Channel Ground Cover: Nearly bare ground

$L_{max, Overland}$  100 ft  $C_v$  10

	L (ft)	$\Delta Z_o$ (ft)	$S_o$ (ft/ft)	v (ft/s)	t (min)	$t_{Att}$ (min)
Total	435	11	-	-	-	-
Initial Time	100	3	0.032	-	7.7	N/A DCM Eq. 6-8
Shallow Channel	335	8	0.022	1.5	3.7	- DCM Eq. 6-9
Channelized			0.000	0.0	0.0	- V-Ditch
				$t_c$	<b>11.4 min.</b>	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	3.14	3.93	4.59	5.24	5.90	6.60
<b>Runoff (cfs)</b>	5.7	7.6	9.7	12.1	14.2	16.6
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	5.7	7.6	9.7	12.1	14.2	16.6

DCM:  $i = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

### Notes

## Sub-Basin Ex-A1 Runoff Calculations

Job No.: 61092

Date: 6/22/18 0:39

Project: Monument Small Engine Storage

Calcs by: D. Gorman

Jurisdiction: DCM  
Runoff Coefficient: Surface Type

Checked by: \_\_\_\_\_

Soil Type: B  
Urbanization: Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	6,295	0.14	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	55,489	1.27	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>61,784</b>	<b>1.42</b>	<b>0.08</b>	<b>0.13</b>	<b>0.20</b>	<b>0.29</b>	<b>0.34</b>	<b>0.39</b>	<b>8.2%</b>

61784

### Basin Travel Time

	Shallow Channel Ground Cover		Nearly bare ground			
	$L_{max, Overland}$	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	$v$ (ft/s)	$t$ (min)	$t_{Alt}$ (min)
Total	435	19	-	-	-	-
Initial Time	100	3	0.032	-	11.9	N/A DCM Eq. 6-8
Shallow Channel	50	7	0.140	3.7	0.2	- DCM Eq. 6-9
Channelized	285	9	0.032	3.0	1.6	- V-Ditch
				$t_c$	<b>13.7 min.</b>	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.92	3.65	4.26	4.87	5.48	6.13
Runoff (cfs)	0.3	0.7	1.2	2.0	2.6	3.4
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.3	0.7	1.2	2.0	2.6	3.4

DCM:  $I = C1 * \ln(t_c) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

### Notes



# EX Combined Sub-Basin Runoff Calculations (DP1)

Includes Basins EX-A1

+ offsite pond outflow

Job No.: **61092**

Date: **6/22/18 0:39**

Project: **Monument Small Engine Storage**

Calcs by: **D. Gorman**

Checked by: \_\_\_\_\_

Jurisdiction: **DCM**

Soil Type: **B**

Runoff Coefficient: **Surface Type**

Urbanization: **Non-Urban**

## Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	6,295	0.14	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	55,489	1.27	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>61,784</b>	<b>1.42</b>	<b>0.08</b>	<b>0.13</b>	<b>0.20</b>	<b>0.29</b>	<b>0.34</b>	<b>0.39</b>	<b>8.2%</b>

## Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. $\Delta Z_0$ (ft)	$Q_i$ (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	EX-A1	-	435	19	-	-	-	-	13.7
Channelized-1									
Channelized-2									
Channelized-3									
Total			435	19					
								$t_c$ (min)	13.7

## Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas: Offsite Basin OSA1 Existing Pond Outflow

$Q_{Minor}$ : 2.8 (cfs) - 5-year Storm

$Q_{Major}$ : 12.1 (cfs) - 100-year Storm

## Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.92	3.65	4.26	4.87	5.48	6.13
Site Runoff (cfs)	0.31	<b>0.68</b>	1.20	2.02	2.63	<b>3.36</b>
OffSite Runoff (cfs)	-	<b>2.80</b>	-	-	-	<b>12.10</b>
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	-	<b>3.5</b>	-	-	-	<b>15.5</b>

DCM:  $i = C1 * \ln(tc) + C2$

C1: 1.19, 1.5, 1.75, 2, 2.25, 2.52

C2: 6.035, 7.583, 8.847, 10.111, 11.375, 12.735

## Notes

Runoff from pond outflows and other contributing basins are assumed constant, disregarding differing times of concentration.

## Sub-Basin Ex-A2 Runoff Calculations

Job No.:	61092	Date:	6/22/18 0:39
Project:	Monument Small Engine Storage	Calcs by:	D. Gorman
Jurisdiction:	DCM	Checked by:	
Runoff Coefficient:	Surface Type	Soil Type:	B
		Urbanization:	Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	2,360	0.05	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	11,765	0.27	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>14,125</b>	<b>0.32</b>	<b>0.11</b>	<b>0.17</b>	<b>0.23</b>	<b>0.32</b>	<b>0.36</b>	<b>0.41</b>	<b>13.4%</b>

14125

### Basin Travel Time

	Shallow Channel Ground Cover		Nearly bare ground			
	$L_{max, Overland}$	100 ft			$C_v$	10
	L (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	v (ft/s)	t (min)	$t_{Alt}$ (min)
Total	169	12	-	-	-	-
Initial Time	75	5	0.063	-	8.0	N/A DCM Eq. 6-8
Shallow Channel	94	7	0.074	2.7	0.6	- DCM Eq. 6-9
Channelized			0.000	0.0	0.0	- V-Ditch
				$t_c$	8.6 min.	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	3.48	4.36	5.09	5.82	6.55	7.33
<b>Runoff (cfs)</b>	0.1	0.2	0.4	0.6	0.8	1.0
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	0.1	0.2	0.4	0.6	0.8	1.0

DCM:  $t = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

### Notes

## EX Combined Sub-Basin Runoff Calculations (DP1 + A2)

Includes Basins EX-A1 EX-A2

+ offsite pond outflow

Job No.: 61092

Date: 6/22/18 0:39

Project: Monument Small Engine Storage

Calcs by: D. Gorman

Checked by: \_\_\_\_\_

Jurisdiction **DCM**  
Runoff Coefficient **Surface Type**

Soil Type **B**  
Urbanization **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	8,655	0.20	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	67,254	1.54	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>75,909</b>	<b>1.74</b>	<b>0.08</b>	<b>0.14</b>	<b>0.20</b>	<b>0.30</b>	<b>0.34</b>	<b>0.39</b>	<b>9.1%</b>

### Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. $\Delta Z_0$ (ft)	$Q_i$ (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	EX-A1	-	435	19	-	-	-	-	13.7
Channelized-1									
Channelized-2									
Channelized-3									
Total			435	19					
								$t_c$ (min)	13.7

### Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas Offsite Basin OSA1 Existing Pond Outflow

$Q_{Minor}$  2.8 (cfs) - 5-year Storm

$Q_{Major}$  12.1 (cfs) - 100-year Storm

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.92	3.65	4.26	4.87	5.48	6.13
Site Runoff (cfs)	0.42	<b>0.88</b>	1.52	2.52	3.28	<b>4.17</b>
OffSite Runoff (cfs)	-	<b>2.80</b>	-	-	-	<b>12.10</b>
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	-	<b>3.7</b>	-	-	-	<b>16.3</b>

$$DCM: i = C1 * \ln(tc) + C2$$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

### Notes

Runoff from pond outflows and other contributing basins are assumed constant, disregarding differing times of concentration.

## Sub-Basin Ex-B1 Runoff Calculations

Job No.: 61092 Date: 6/22/18 0:39  
 Project: Monument Small Engine Storage Calcs by: D. Gorman  
 Checked by: \_\_\_\_\_  
 Jurisdiction: DCM Soil Type: B  
 Runoff Coefficient: Surface Type Urbanization: Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Roofs	3,525	0.08	0.71	0.73	0.75	0.78	0.8	0.81	90%
Gravel	22,940	0.53	0.57	0.59	0.63	0.66	0.68	0.7	80%
Paved	3,660	0.08	0.89	0.9	0.92	0.94	0.95	0.96	100%
Pasture/Meadow	63,448	1.46	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>93,573</b>	<b>2.15</b>	<b>0.21</b>	<b>0.26</b>	<b>0.32</b>	<b>0.40</b>	<b>0.44</b>	<b>0.48</b>	<b>26.9%</b>

93573

### Basin Travel Time

	Shallow Channel Ground Cover		Short Pasture/Lawns			
	$L_{max, Overland}$	100 ft			$C_v$	7
	L (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	v (ft/s)	t (min)	$t_{Att}$ (min)
Total	380	17	-	-	-	-
Initial Time	100	2	0.020	-	12.0	N/A DCM Eq. 6-8
Shallow Channel	195	8	0.041	1.4	2.3	- DCM Eq. 6-9
Channelized	85	7	0.082	4.5	0.3	- V-Ditch
				$t_c$	<b>14.7 min.</b>	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.84	3.56	4.15	4.74	5.33	5.97
Runoff (cfs)	1.3	2.0	2.9	4.0	5.0	6.1
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	1.3	2.0	2.9	4.0	5.0	6.1

DCM:  $t_c = C1 * \ln(t_c) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

### Notes

## Sub-Basin Dv-A1 Runoff Calculations

Job No.: 61092  
 Project: Monument Small Engine Storage  
 Jurisdiction: DCM  
 Runoff Coefficient: Surface Type

Date: 6/22/18 0:39  
 Calcs by: D. Gorman  
 Checked by: \_\_\_\_\_  
 Soil Type: B  
 Urbanization: Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	45,031	1.03	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	16,960	0.39	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>61,991</b>	<b>1.42</b>	<b>0.42</b>	<b>0.45</b>	<b>0.50</b>	<b>0.55</b>	<b>0.58</b>	<b>0.60</b>	<b>58.1%</b>

61991

### Basin Travel Time

	Shallow Channel Ground Cover		Nearly bare ground			
	$L_{max, Overland}$	$\Delta Z_o$ (ft)	$S_o$ (ft/ft)	$v$ (ft/s)	$t$ (min)	$t_{All}$ (min)
Total	435	19	-	-	-	-
Initial Time	100	3	0.032	-	8.0	N/A DCM Eq. 6-8
Shallow Channel	50	7	0.140	3.7	0.2	- DCM Eq. 6-9
Channelized	285	9	0.032	3.1	1.5	- V-Ditch
				$t_c$	<b>9.7 min.</b>	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	3.33	4.17	4.86	5.56	6.25	7.00
<b>Runoff (cfs)</b>	2.0	2.7	3.5	4.3	5.1	6.0
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	2.0	2.7	3.5	4.3	5.1	6.0

DCM:  $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

### Notes

**DEY**  
**Combined Sub-Basin Runoff Calculations (DP1 pond inflow)**

Includes Basins DV-A1

+ OSA1 pond outflow

Job No.: **61092**

Date: **6/22/18 0:39**

Project: **Monument Small Engine Storage**

Calcs by: **D. Gorman**

Jurisdiction: **DCM**

Soil Type: **B**

Runoff Coefficient: **Surface Type**

Urbanization: **Non-Urban**

**Basin Land Use Characteristics**

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	45,031	1.03	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	16,960	0.39	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>61,991</b>	<b>1.42</b>	<b>0.42</b>	<b>0.45</b>	<b>0.50</b>	<b>0.55</b>	<b>0.58</b>	<b>0.60</b>	<b>58.1%</b>

**Basin Travel Time**

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. ΔZ <sub>0</sub> (ft)	Q <sub>i</sub> (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	DV-A1	-	435	19	-	-	-	-	9.7
Channelized-1									
Channelized-2									
Channelized-3									
Total			435	19					
								<b>t<sub>c</sub> (min)</b>	<b>9.7</b>

**Contributing Offsite Flows** (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas	Offsite Basin	OSA1 Existing Pond Outflow
Q <sub>Minor</sub>	2.8 (cfs) - 5-year Storm	
Q <sub>Major</sub>	12.1 (cfs) - 100-year Storm	

**Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.33	4.17	4.86	5.56	6.25	7.00
Site Runoff (cfs)	1.99	<b>2.67</b>	3.45	4.33	5.13	<b>6.02</b>
OffSite Runoff (cfs)	-	<b>2.80</b>	-	-	-	<b>12.10</b>
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	-	<b>5.5</b>	-	-	-	<b>18.1</b>

DCM:  $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

**Notes**

Runoff from pond outflows and other contributing basins are assumed constant, disregarding differing times of concentration.

## Sub-Basin Dv-A2 Runoff Calculations

Job No.: 61092 Date: 6/22/18 0:39  
 Project: Monument Small Engine Storage Calcs by: D. Gorman  
 Checked by: \_\_\_\_\_  
 Jurisdiction: DCM Soil Type: B  
 Runoff Coefficient: Surface Type Urbanization: Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	2,362	0.05	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	11,556	0.27	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>13,918</b>	<b>0.32</b>	<b>0.11</b>	<b>0.17</b>	<b>0.23</b>	<b>0.32</b>	<b>0.36</b>	<b>0.41</b>	<b>13.6%</b>

13918

### Basin Travel Time

Shallow Channel Ground Cover Nearly bare ground

	L <sub>max, Overland</sub> (ft)	ΔZ <sub>0</sub> (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)	C <sub>v</sub>
Total	169	12	-	-	-	-	10
Initial Time	75	5	0.063	-	8.0	N/A	DCM Eq. 6-8
Shallow Channel	94	7	0.074	2.7	0.6	-	DCM Eq. 6-9
Channelized			0.000	0.0	0.0	-	V-Ditch
				<b>t<sub>c</sub></b>	<b>8.5 min.</b>		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.48	4.37	5.09	5.82	6.55	7.33
Runoff (cfs)	0.1	0.2	0.4	0.6	0.8	1.0
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.1	0.2	0.4	0.6	0.8	1.0

DCM:  $i = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10,111	11,375	12,735

### Notes

DEW  
**Combined Sub-Basin Runoff Calculations (DP1 pond outflow + A2)**

Includes Basins DV-A2

+ DP1 pond outflow

Job No.: 61092  
 Project: Monument Small Engine Storage  
 Jurisdiction: DCM  
 Runoff Coefficient: Surface Type

Date: 6/22/18 0:39  
 Calcs by: D. Gorman  
 Checked by: \_\_\_\_\_  
 Soil Type: B  
 Urbanization: Non-Urban

**Basin Land Use Characteristics**

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Gravel	2,362	0.05	0.57	0.59	0.63	0.66	0.68	0.7	80%
Pasture/Meadow	11,556	0.27	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>13,918</b>	<b>0.32</b>	<b>0.11</b>	<b>0.17</b>	<b>0.23</b>	<b>0.32</b>	<b>0.36</b>	<b>0.41</b>	<b>13.6%</b>

**Basin Travel Time**

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. ΔZ <sub>0</sub> (ft)	Q <sub>i</sub> (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	DV-A2	-	169	12	-	-	-	-	8.5
Channelized-1									
Channelized-2									
Channelized-3									
Total			169	12					
								t <sub>c</sub> (min)	8.5

**Contributing Offsite Flows** (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas: DP1 Pond Outflow  
 Q<sub>Minor</sub>: 3.8 (cfs) - 5-year Storm  
 Q<sub>Major</sub>: 16.0 (cfs) - 100-year Storm

**Rainfall Intensity & Runoff**

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	3.48	4.37	5.09	5.82	6.55	7.33
<b>Site Runoff (cfs)</b>	0.13	0.23	0.38	0.59	0.76	0.96
<b>OffSite Runoff (cfs)</b>	-	3.80	-	-	-	16.00
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	-	4.0	-	-	-	17.0

DCM:  $t = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

**Notes**

Runoff from pond outflows and other contributing basins are assumed constant, disregarding differing times of concentration.



## Sub-Basin Dv-B1 Runoff Calculations (DP2 pond inflow)

Job No.: 61092 Date: 6/22/18 0:39  
 Project: Monument Small Engine Storage Calcs by: D. Gorman  
 Checked by: \_\_\_\_\_  
 Jurisdiction: DCM Soil Type: B  
 Runoff Coefficient: Surface Type Urbanization: Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Roofs	3,200	0.07	0.71	0.73	0.75	0.78	0.8	0.81	90%
Gravel	15,860	0.36	0.57	0.59	0.63	0.66	0.68	0.7	80%
Paved	4,810	0.11	0.89	0.9	0.92	0.94	0.95	0.96	100%
Pasture/Meadow	13,657	0.31	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>37,527</b>	<b>0.86</b>	<b>0.42</b>	<b>0.46</b>	<b>0.50</b>	<b>0.56</b>	<b>0.59</b>	<b>0.62</b>	<b>54.3%</b>

37527

### Basin Travel Time

Shallow Channel Ground Cover: Nearly bare ground

$L_{max, Overland}$ : 100 ft  $C_v$ : 10

	L (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	$v$ (ft/s)	$t$ (min)	$t_{Alt}$ (min)
Total	265	6	-	-	-	-
Initial Time	100	2	0.020	-	9.2	N/A DCM Eq. 6-8
Shallow Channel	50	3	0.060	2.4	0.3	- DCM Eq. 6-9
Channelized	115	1	0.009	1.6	1.2	- V-Ditch
$t_c$					<b>10.8 min.</b>	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	3.20	4.01	4.68	5.35	6.02	6.74
<b>Runoff (cfs)</b>	1.2	1.6	2.0	2.6	3.0	3.6
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	1.2	1.6	2.0	2.6	3.0	3.6

DCM:  $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6,035	7,583	8,847	10,111	11,375	12,735

### Notes

## Sub-Basin Dv-B2 Runoff Calculations

Job No.: 61092 Date: 6/22/18 0:39  
 Project: Monument Small Engine Storage Calcs by: D. Gorman  
 Checked by: \_\_\_\_\_  
 Jurisdiction: DCM Soil Type: B  
 Runoff Coefficient: Surface Type Urbanization: Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Roofs	640	0.01	0.71	0.73	0.75	0.78	0.8	0.81	90%
Gravel	6,518	0.15	0.57	0.59	0.63	0.66	0.68	0.7	80%
Paved	-	0.00	0.89	0.9	0.92	0.94	0.95	0.96	100%
Pasture/Meadow	48,889	1.12	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>56,047</b>	<b>1.29</b>	<b>0.09</b>	<b>0.15</b>	<b>0.21</b>	<b>0.30</b>	<b>0.35</b>	<b>0.40</b>	<b>10.3%</b>

56047

### Basin Travel Time

	Shallow Channel Ground Cover		Short Pasture/Lawns				
	$L_{max, Overland}$ (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	$v$ (ft/s)	$t$ (min)	$t_{Att}$ (min)	$C_v$
Total	235	14	-	-	-	-	7
Initial Time	100	6	0.060	-	9.5	N/A	DCM Eq. 6-8
Shallow Channel	50	1	0.020	1.0	0.8	-	DCM Eq. 6-9
Channelized	85	7	0.082	3.6	0.4	-	V-Ditch
				$t_c$	<b>10.8 min.</b>		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.21	4.02	4.69	5.36	6.03	6.75
Runoff (cfs)	0.4	0.8	1.3	2.1	2.7	3.4
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.4	0.8	1.3	2.1	2.7	3.4

DCM:  $I = C1 * in (tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

### Notes

## Combined Sub-Basin Runoff Calculations (DP3)

Includes Basins DV-B2

+ DP2 pond outflow

Job No.: 61092

Date: 6/22/18 0:39

Project: Monument Small Engine Storage

Calcs by: D. Gorman

Jurisdiction: DCM  
Runoff Coefficient: Surface Type

Checked by: \_\_\_\_\_

Soil Type: B  
Urbanization: Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Roofs	640	0.01	0.71	0.73	0.75	0.78	0.8	0.81	90%
Gravel	6,518	0.15	0.57	0.59	0.63	0.66	0.68	0.7	80%
Paved	-	0.00	0.89	0.9	0.92	0.94	0.95	0.96	100%
Pasture/Meadow	48,889	1.12	0.02	0.08	0.15	0.25	0.3	0.35	0%
<b>Combined</b>	<b>56,047</b>	<b>1.29</b>	<b>0.09</b>	<b>0.15</b>	<b>0.21</b>	<b>0.30</b>	<b>0.35</b>	<b>0.40</b>	<b>10.3%</b>

### Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. $\Delta Z_0$ (ft)	$Q_i$ (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	DV-B2	-	235	14	-	-	-	-	10.8
Channelized-1									
Channelized-2									
Channelized-3									
<b>Total</b>			<b>235</b>	<b>14</b>					
								$t_c$ (min)	<b>10.8</b>

### Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas: DP2 pond outflow

$Q_{Minor}$ : 0.8 (cfs) - 5-year Storm  
 $Q_{Major}$ : 2.4 (cfs) - 100-year Storm

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	3.21	4.02	4.69	5.36	6.03	6.75
<b>Site Runoff (cfs)</b>	0.38	<b>0.76</b>	1.28	2.09	2.71	<b>3.44</b>
<b>OffSite Runoff (cfs)</b>	-	<b>0.80</b>	-	-	-	<b>2.40</b>
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	-	<b>1.6</b>	-	-	-	<b>5.8</b>

DCM:  $I = C1 * \ln(tc) + C2$

C1: 1.19, 1.5, 1.75, 2, 2.25, 2.52  
C2: 6.035, 7.583, 8.847, 10.111, 11.375, 12.735

### Notes

Runoff from pond outflows and other contributing basins are assumed constant, disregarding differing times of concentration.

## **10 Hydraulic Calculations**

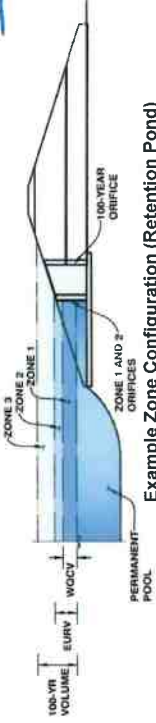
Sand Filter Basin Calculations  
Ditch Capacity Calculations

# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: Mounty Herman RV Storage

Basin ID: Existing WQ/Detention Pond Basin OSA1 (existing pond - offsite)



Example Zone Configuration (Retention Pond)

### Required Volume Calculation

Selected BMP Type =	SF	
Watershed Area =	4.05	acres
Watershed Length =	435	ft
Watershed Slope =	0.025	ft/ft
Watershed Imperviousness =	62.00%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Desired WQCV Drain Time =	12.0	hours
Location for 1-hr Rainfall Depths =	Denver - Capitol Building	
Water Quality Capture Volume (WQCV) =	0.066	acre-feet
Excess Urban Runoff Volume (EURV) =	0.273	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.225	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	0.302	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	0.391	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	0.511	acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	0.597	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	0.710	acre-feet
500-yr Runoff Volume (P1 = 3.4 in.) =	1.023	acre-feet
Approximate 2-yr Detention Volume =	0.211	acre-feet
Approximate 5-yr Detention Volume =	0.284	acre-feet
Approximate 10-yr Detention Volume =	0.364	acre-feet
Approximate 25-yr Detention Volume =	0.393	acre-feet
Approximate 50-yr Detention Volume =	0.409	acre-feet
Approximate 100-yr Detention Volume =	0.445	acre-feet

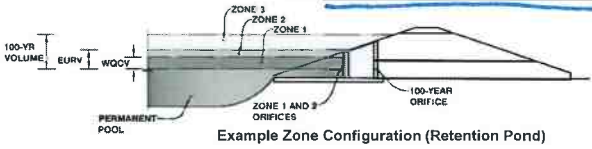
Optional User Override 1-hr Precipitation	inches
	1.19
	1.50
	1.75
	2.00
	2.25
	2.52
	3.40

Depth Increment =	0.2		Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft <sup>2</sup> )	Optional Override Area (ft <sup>2</sup> )	Area (acre)	Volume (ft <sup>3</sup> )	Volume (ac-ft)
	Stage (ft)	Media Surface								
Stage - Storage Description										
Media Surface	--	0.00	--	--	--	--	885	0.020	178	0.004
	--	0.20	--	--	--	--	997	0.023	384	0.009
	--	0.40	--	--	--	--	1,073	0.025	608	0.014
	--	0.60	--	--	--	--	1,167	0.027	849	0.019
	--	0.80	--	--	--	--	1,261	0.029	1,110	0.025
	--	1.00	--	--	--	--	1,355	0.031	1,419	0.033
	--	1.20	--	--	--	--	1,776	0.041	1,812	0.042
	--	1.40	--	--	--	--	2,197	0.050	2,289	0.063
	--	1.60	--	--	--	--	2,618	0.060	2,851	0.065
	--	1.80	--	--	--	--	3,039	0.070	3,497	0.080
12" CMP Inv EI	--	2.00	--	--	--	--	3,460	0.079	4,242	0.097
	--	2.20	--	--	--	--	3,646	0.084	4,990	0.115
	--	2.40	--	--	--	--	3,834	0.088	5,775	0.133
	--	2.60	--	--	--	--	4,020	0.092	6,598	0.151
	--	2.80	--	--	--	--	4,207	0.097	7,458	0.171
	--	3.00	--	--	--	--	4,394	0.101	8,356	0.192
	--	3.20	--	--	--	--	4,592	0.105	9,295	0.213
	--	3.40	--	--	--	--	4,790	0.110	10,273	0.236
Em Spwy Crest	--	3.60	--	--	--	--	4,989	0.115	11,290	0.259
	--	3.80	--	--	--	--	5,187	0.119	12,347	0.283
	--	4.00	--	--	--	--	5,385	0.124		
	--		--	--	--	--				
	--		--	--	--	--				
	--		--	--	--	--				
	--		--	--	--	--				
	--		--	--	--	--				

# Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: **Mount Herman RV Storage**  
 Basin ID: **Existing WQ/Detention Pond Basin OSA1**



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.80	0.066	Filtration Media
Zone 2 (5-year)		0.218	Circular Orifice
Zone 3			
		0.284	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	2.00	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	1.22	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	0.0	ft <sup>2</sup>
Underdrain Orifice Centroid =	0.05	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft <sup>2</sup>
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft <sup>2</sup>

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	2.00		ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	3.60		ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	12.00		inches

Calculated Parameters for Vertical Orifice

	Zone 2 Circular	Not Selected	
Vertical Orifice Area =	0.79		ft <sup>2</sup>
Vertical Orifice Centroid =	0.50		feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Not Selected	Not Selected	
Overflow Weir Front Edge Height, H <sub>o</sub> =	2.00		ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =			feet
Overflow Weir Slope =			H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =			feet
Overflow Grate Open Area % =			%, grate open area/total area
Debris Clogging % =			%

Calculated Parameters for Overflow Weir

	Not Selected	Not Selected	
Height of Grate Upper Edge, H <sub>u</sub> =			feet
Over Flow Weir Slope Length =			feet
Grate Open Area / 100-yr Orifice Area =			should be ≥ 4
Overflow Grate Open Area w/o Debris =			ft <sup>2</sup>
Overflow Grate Open Area w/ Debris =			ft <sup>2</sup>

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Not Selected	Not Selected	
Depth to Invert of Outlet Pipe =			ft (distance below basin bottom at Stage = 0 ft)
Circular Orifice Diameter =			inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Not Selected	Not Selected	
Outlet Orifice Area =			ft <sup>2</sup>
Outlet Orifice Centroid =			feet
Half-Central Angle of Restrictor Plate on Pipe =	N/A	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	3.60	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	20.00	feet
Spillway End Slopes =	3.00	H:V
Freeboard above Max Water Surface =	0.02	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth =	0.38	feet
Stage at Top of Freeboard =	4.00	feet
Basin Area at Top of Freeboard =	0.12	acres

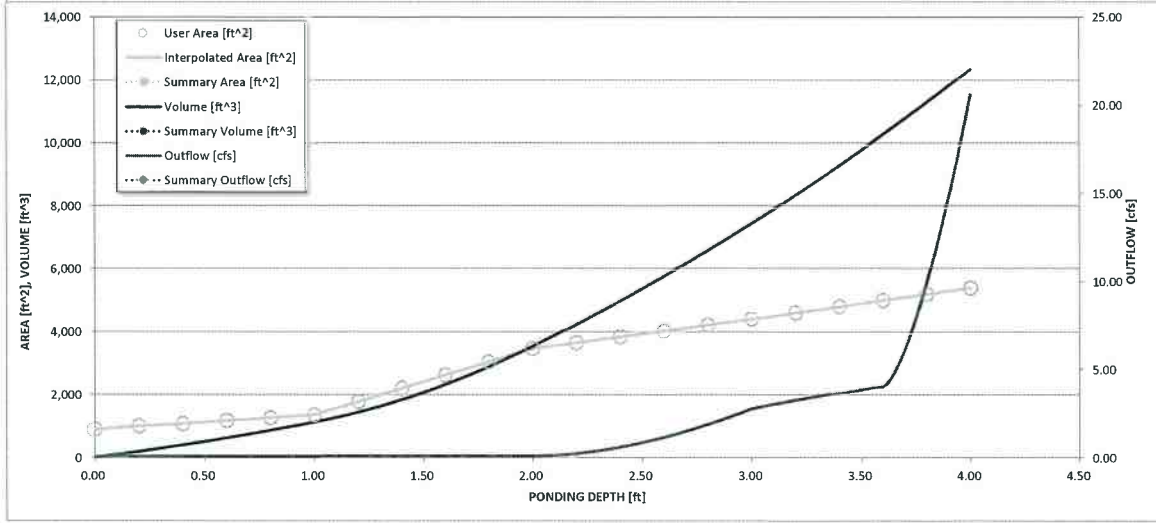
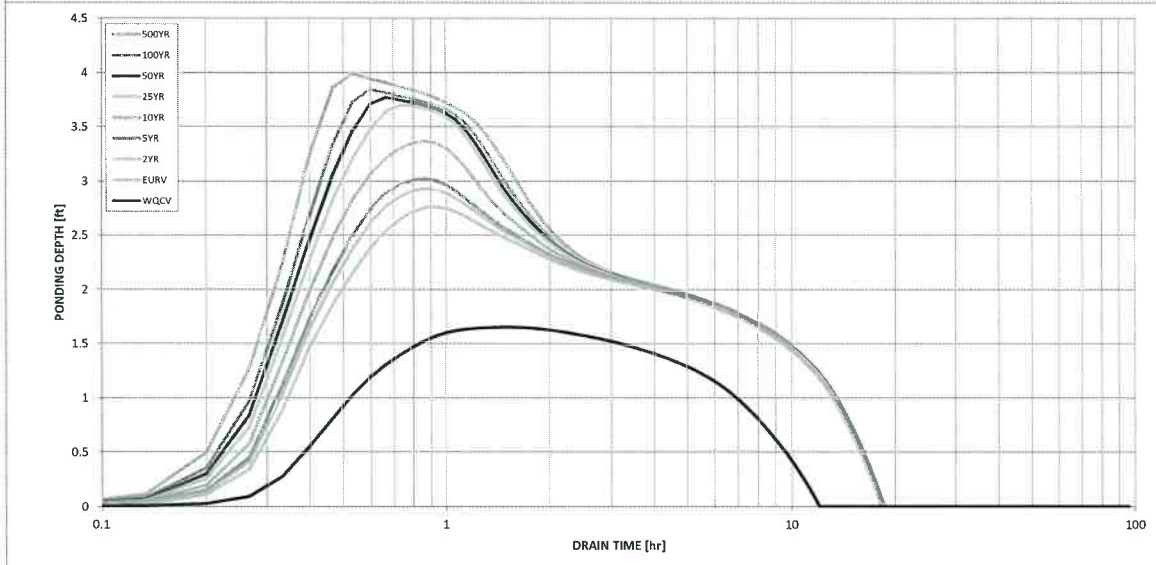
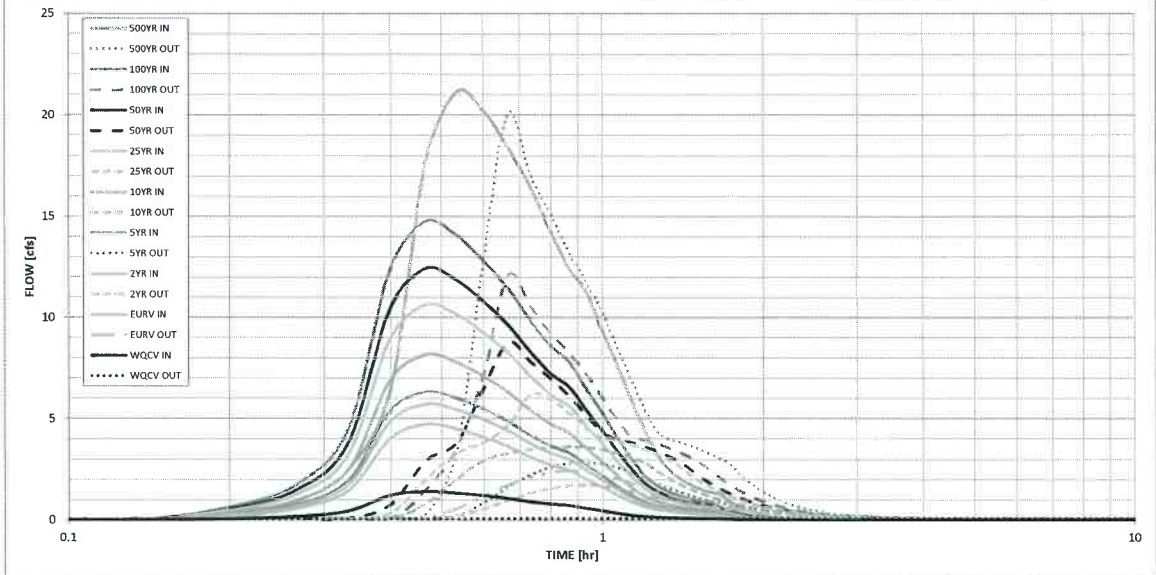
### Routed Hydrograph Results

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.40
Calculated Runoff Volume (acre-ft) =	0.066	0.273	0.225	0.302	0.391	0.511	0.597	0.710	1.023
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.066	0.273	0.225	0.302	0.391	0.511	0.597	0.710	1.024
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.02	0.03	0.28	0.87	1.20	1.60	2.55
Predevelopment Peak Q (cfs) =	0.0	0.0	0.1	0.1	1.1	3.5	4.9	6.5	10.3
Peak Inflow Q (cfs) =	1.4	5.7	4.7	6.3	8.2	10.6	12.4	14.7	21.1
Peak Outflow Q (cfs) =	0.1	2.4	1.7	2.8	3.6	6.2	8.8	12.1	20.1
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	25.1	3.2	1.3	1.8	1.9	1.9
Structure Controlling Flow =	Filtration Media	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Spillway	Spillway	Spillway	Spillway
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	12	17	17	17	16	15	15	14	13
Time to Drain 99% of Inflow Volume (hours) =	12	18	18	18	18	18	17	17	17
Maximum Ponding Depth (ft) =	1.65	2.93	2.76	3.02	3.37	3.70	3.77	3.85	3.99
Area at Maximum Ponding Depth (acres) =	0.06	0.10	0.10	0.10	0.11	0.12	0.12	0.12	0.12
Maximum Volume Stored (acre-ft) =	0.056	0.164	0.148	0.173	0.209	0.247	0.256	0.264	0.282



# Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)



**S-A-V-D Chart Axis Override**

	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

## Design Procedure Form: Sand Filter (SF)

UD-BMP (Version 3.06, November 2016)

Sheet 1 of 2

**Designer:** D. Gorman  
**Company:** M.V.E., Inc.  
**Date:** March 17, 2018  
**Project:** Monument Small Engine Storage  
**Location:** Basin A1 - Pond at DP1

<p>1. Basin Storage Volume</p> <p>A) Effective Imperviousness of Tributary Area, <math>I_a</math> (100% if all paved and roofed areas upstream of sand filter)</p> <p>B) Tributary Area's Imperviousness Ratio (<math>i = I_a/100</math>)</p> <p>C) Water Quality Capture Volume (WQCV) Based on 12-hour Drain Time <math>WQCV = 0.8 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i)</math></p> <p>D) Contributing Watershed Area (including sand filter area)</p> <p>E) Water Quality Capture Volume (WQCV) Design Volume <math>V_{WQCV} = WQCV / 12 * Area</math></p> <p>F) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm</p> <p>G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume</p> <p>H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)</p>	<p><math>I_a =</math> <u>43.6</u> %</p> <p><math>i =</math> <u>0.436</u></p> <p>WQCV = <u>0.15</u> watershed inches</p> <p>Area = <u>61,784</u> sq ft</p> <p><math>V_{WQCV} =</math> <u>780</u> cu ft</p> <p><math>d_6 =</math> _____ in</p> <p><math>V_{WQCV\ OTHER} =</math> _____ cu ft</p> <p><math>V_{WQCV\ USER} =</math> _____ cu ft</p>
<p>2. Basin Geometry</p> <p>A) WQCV Depth</p> <p>B) Sand Filter Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred). Use "0" if sand filter has vertical walls.</p> <p>C) Minimum Filter Area (Flat Surface Area)</p> <p>D) Actual Filter Area</p> <p>E) Volume Provided</p>	<p><math>D_{WQCV} =</math> <u>1.3</u> ft</p> <p><math>Z =</math> <u>3.00</u> ft / ft <span style="color: red;">DIFFICULT TO MAINTAIN, INCREASE WHERE POSSIBLE</span></p> <p><math>A_{Min} =</math> <u>337</u> sq ft</p> <p><math>A_{Actual} =</math> <u>357</u> sq ft</p> <p><math>V_r =</math> <u>781</u> cu ft</p>
<p>3. Filter Material</p>	<p>Choose One</p> <p><input checked="" type="radio"/> 18" CDOT Class B or C Filter Material</p> <p><input type="radio"/> Other (Explain): _____</p>
<p>4. Underdrain System</p> <p>A) Are underdrains provided?</p> <p>B) Underdrain system orifice diameter for 12 hour drain time</p> <p style="margin-left: 20px;">i) Distance From Lowest Elevation of the Storage Volume to the Center of the Orifice</p> <p style="margin-left: 20px;">ii) Volume to Drain in 12 Hours</p> <p style="margin-left: 20px;">iii) Orifice Diameter, 3/8" Minimum</p>	<p>Choose One</p> <p><input checked="" type="radio"/> YES</p> <p><input type="radio"/> NO</p> <p><math>y =</math> <u>2.0</u> ft</p> <p><math>Vol_{12} =</math> <u>780</u> cu ft</p> <p><math>D_o =</math> <u>5 / 8</u> in</p>

Design Procedure Form: Sand Filter (SF)

Sheet 2 of 2

Designer: D. Gorman  
Company: M.V.E., Inc.  
Date: March 17, 2018  
Project: Monument Small Engine Storage  
Location: Basin A1 - Pond at DP1

5. Impermeable Geomembrane Liner and Geotextile Separator Fabric

A) Is an impermeable liner provided due to proximity of structures or groundwater contamination?

Choose One

YES  NO

6-7. Inlet / Outlet Works

A) Describe the type of energy dissipation at inlet points and means of conveying flows in excess of the WQCV through the outlet

12" pvc pipe outlet and 8' wide emergency spillway  
\_\_\_\_\_  
\_\_\_\_\_

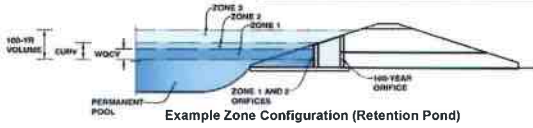
Notes: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: 61092 - Monument Small Engine Storage

Basin ID: Basin A1 sand filter DP 1



Example Zone Configuration (Retention Pond)

**Required Volume Calculation**

Selected BMP Type =	<b>SF</b>
Watershed Area =	1.42 acres
Watershed Length =	435 ft
Watershed Slope =	0.044 ft/ft
Watershed Imperviousness =	43.60% percent
Percentage Hydrologic Soil Group A =	0.0% percent
Percentage Hydrologic Soil Group B =	100.0% percent
Percentage Hydrologic Soil Groups C/D =	0.0% percent
Desired WQCV Drain Time =	12.0 hours
Location for 1-hr Rainfall Depths =	User Input
Water Quality Capture Volume (WQCV) =	0.018 acre-feet
Excess Urban Runoff Volume (EURV) =	0.065 acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.052 acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	0.072 acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	0.100 acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	0.146 acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	0.177 acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	0.218 acre-feet
500-yr Runoff Volume (P1 = 3.4 in.) =	0.327 acre-feet
Approximate 2-yr Detention Volume =	0.049 acre-feet
Approximate 5-yr Detention Volume =	0.068 acre-feet
Approximate 10-yr Detention Volume =	0.091 acre-feet
Approximate 25-yr Detention Volume =	0.101 acre-feet
Approximate 50-yr Detention Volume =	0.106 acre-feet
Approximate 100-yr Detention Volume =	0.121 acre-feet

Optional User Override 1-hr Precipitation

1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.40	inches

**Stage-Storage Calculation**

Zone 1 Volume (WQCV) =	0.018	acre-feet
Zone 2 Volume (2-year - Zone 1) =	0.031	acre-feet
Select Zone 3 Storage Volume (Optional) =		acre-feet
Total Detention Basin Volume =	0.049	acre-feet
Initial Surcharge Volume (ISV) =	N/A	ft <sup>3</sup>
Initial Surcharge Depth (ISD) =	N/A	ft
Total Available Detention Depth (H <sub>total</sub> ) =	user	ft
Depth of Trickle Channel (H <sub>TC</sub> ) =	N/A	ft
Slope of Trickle Channel (S <sub>TC</sub> ) =	N/A	ft/ft
Slopes of Main Basin Sides (S <sub>main</sub> ) =	user	H:V
Basin Length-to-Width Ratio (R <sub>L/W</sub> ) =	user	
Initial Surcharge Area (A <sub>ISV</sub> ) =	user	ft <sup>2</sup>
Surcharge Volume Length (L <sub>ISV</sub> ) =	user	ft
Surcharge Volume Width (W <sub>ISV</sub> ) =	user	ft
Depth of Basin Floor (H <sub>FLOOR</sub> ) =	user	ft
Length of Basin Floor (L <sub>FLOOR</sub> ) =	user	ft
Width of Basin Floor (W <sub>FLOOR</sub> ) =	user	ft
Area of Basin Floor (A <sub>FLOOR</sub> ) =	user	ft <sup>2</sup>
Volume of Basin Floor (V <sub>FLOOR</sub> ) =	user	ft <sup>3</sup>
Depth of Main Basin (H <sub>MAIN</sub> ) =	user	ft
Length of Main Basin (L <sub>MAIN</sub> ) =	user	ft
Width of Main Basin (W <sub>MAIN</sub> ) =	user	ft
Area of Main Basin (A <sub>MAIN</sub> ) =	user	ft <sup>2</sup>
Volume of Main Basin (V <sub>MAIN</sub> ) =	user	ft <sup>3</sup>
Calculated Total Basin Volume (V <sub>total</sub> ) =	user	acre-feet

Total detention volume is less than 100-year volume.

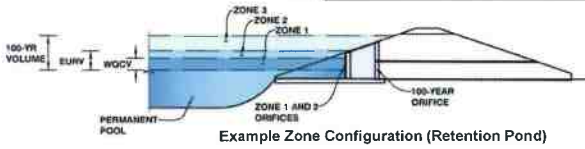
Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft <sup>2</sup> )	Optional Override Area (ft <sup>2</sup> )	Area (acre)	Volume (ft <sup>3</sup> )	Volume (ac-ft)
Media Surface	--	0.00	--	--	--	347	0.008		
	--	0.10	--	--	--	376	0.009	36	0.001
	--	0.20	--	--	--	405	0.009	71	0.002
	--	0.30	--	--	--	434	0.010	113	0.003
	--	0.40	--	--	--	463	0.011	157	0.004
<b>6354</b>	--	0.50	--	--	--	492	0.011	205	0.005
	--	0.60	--	--	--	534	0.012	256	0.006
	--	0.70	--	--	--	575	0.013	311	0.007
	--	0.80	--	--	--	617	0.014	370	0.008
	--	0.90	--	--	--	659	0.015	433	0.010
	--	1.00	--	--	--	701	0.016	501	0.011
	--	1.10	--	--	--	742	0.017	573	0.013
<b>WQCV (54.7)</b>	--	1.20	--	--	--	784	0.018	649	0.015
	--	1.30	--	--	--	826	0.019	729	0.017
	--	1.40	--	--	--	867	0.020	813	0.019
<b>6955 Emerg Spwy</b>	--	1.50	--	--	--	909	0.021	901	0.021
	--	1.60	--	--	--	962	0.022	994	0.023
	--	1.70	--	--	--	1,016	0.023	1,093	0.025
	--	1.80	--	--	--	1,069	0.025	1,196	0.027
	--	1.90	--	--	--	1,122	0.026	1,305	0.030
	--	2.00	--	--	--	1,176	0.027	1,420	0.033
	--	2.10	--	--	--	1,229	0.028	1,552	0.036
	--	2.20	--	--	--	1,282	0.029	1,677	0.039
	--	2.30	--	--	--	1,335	0.031	1,808	0.042
	--	2.40	--	--	--	1,389	0.032	1,944	0.045
<b>6956 Top Embkt</b>	--	2.50	--	--	--	1,442	0.033	2,086	0.048

## Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: 61092 - Monument Small Engine Storage

Basin ID: Basin A1 sand filter DP 1



	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.36	0.018	Filtration Media
Zone 2 (2-year)		0.031	Weir & Pipe (Restrict)
Zone 3			
		0.049	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	2.00	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	0.65	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	0.0	ft <sup>2</sup>
Underdrain Orifice Centroid =	0.03	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft <sup>2</sup>
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft <sup>2</sup>

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =			ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =			ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =			inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =			ft <sup>2</sup>
Vertical Orifice Centroid =			feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 2 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	1.36		ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	0.89		feet
Overflow Weir Slope =	0.00		H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	0.89		feet
Overflow Grate Open Area % =	81%		%, grate open area/total area
Debris Clogging % =	50%		%

Calculated Parameters for Overflow Weir

	Zone 2 Weir	Not Selected	
Height of Grate Upper Edge, H <sub>g</sub> =	1.36		feet
Over Flow Weir Slope Length =	0.89		feet
Grate Open Area / 100-yr Orifice Area =	0.82		should be ≥ 4
Overflow Grate Open Area w/o Debris =	0.64		ft <sup>2</sup>
Overflow Grate Open Area w/ Debris =	0.32		ft <sup>2</sup>

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 2 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	2.00		ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	12.00		inches
Restrictor Plate Height Above Pipe Invert =	12.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 2 Restrictor	Not Selected	
Outlet Orifice Area =	0.79		ft <sup>2</sup>
Outlet Orifice Centroid =	0.50		feet
Half-Central Angle of Restrictor Plate on Pipe =	3.14	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	2.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	8.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	0.50	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth =	0.26	feet
Stage at Top of Freeboard =	2.76	feet
Basin Area at Top of Freeboard =	0.03	acres

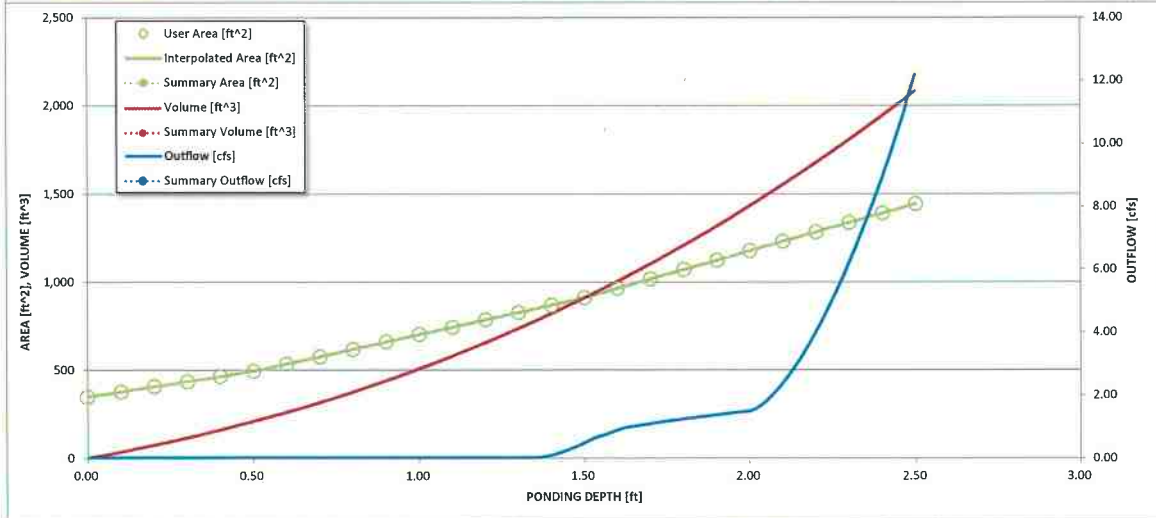
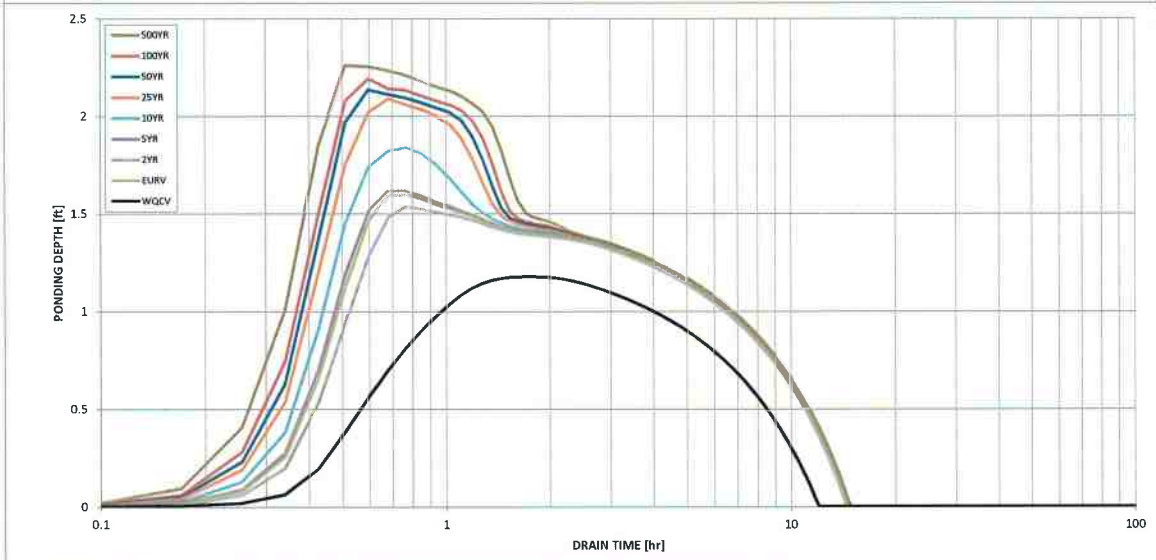
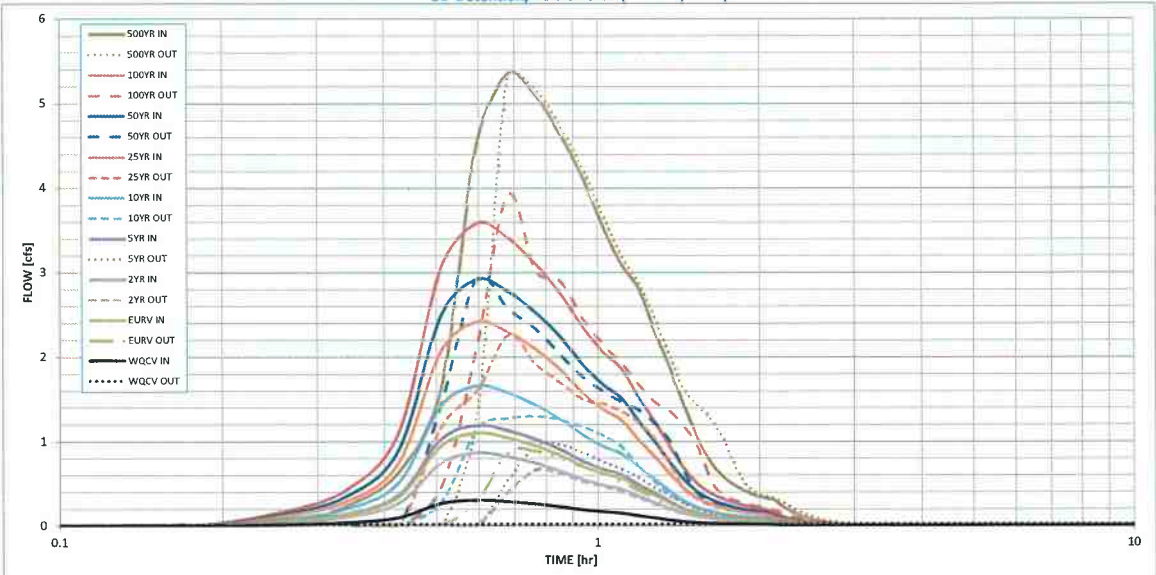
### Routed Hydrograph Results

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.40
Calculated Runoff Volume (acre-ft) =	0.018	0.065	0.052	0.072	0.100	0.146	0.177	0.218	0.327
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.018	0.066	0.051	0.071	0.100	0.146	0.177	0.218	0.327
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.19	0.64	0.89	1.19	1.91
Predevelopment Peak Q (cfs) =	0.0	0.0	0.0	0.0	0.3	0.9	1.3	1.7	2.7
Peak Inflow Q (cfs) =	0.3	1.1	0.9	1.2	1.7	2.4	2.9	3.6	5.3
Peak Outflow Q (cfs) =	0.0	0.9	0.7	1.0	1.3	2.3	2.9	3.9	5.3
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	33.8	4.8	2.5	2.3	2.3	2.0
Structure Controlling Flow =	Filtration Media	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Spillway	Spillway	Spillway	Spillway
Max Velocity through Grate 1 (fps) =	N/A	1.29	0.96	1.4	2.0	2.5	2.5	2.6	2.7
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	12	13	13	13	12	12	11	10	8
Time to Drain 99% of Inflow Volume (hours) =	12	14	14	14	14	14	14	13	12
Maximum Ponding Depth (ft) =	1.18	1.60	1.54	1.62	1.84	2.09	2.14	2.19	2.26
Area at Maximum Ponding Depth (acres) =	0.02	0.02	0.02	0.02	0.03	0.03	0.03	0.03	0.03
Maximum Volume Stored (acre-ft) =	0.015	0.023	0.022	0.023	0.029	0.035	0.036	0.038	0.040



# Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)



S-A-V-D Chart Axis Override

	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			



## Design Procedure Form: Sand Filter (SF)

UD-BMP (Version 3.06, November 2016)

Sheet 1 of 2

**Designer:** D. Gorman  
**Company:** M.V.E., Inc.  
**Date:** March 17, 2018  
**Project:** Monument Small Engine Storage  
**Location:** Basin B1 - Pond at DP2

<p>1. Basin Storage Volume</p> <p>A) Effective Imperviousness of Tributary Area, <math>I_a</math> (100% if all paved and roofed areas upstream of sand filter)</p> <p>B) Tributary Area's Imperviousness Ratio (<math>i = I_a/100</math>)</p> <p>C) Water Quality Capture Volume (WQCV) Based on 12-hour Drain Time WQCV = <math>0.8 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i)</math></p> <p>D) Contributing Watershed Area (including sand filter area)</p> <p>E) Water Quality Capture Volume (WQCV) Design Volume <math>V_{WQCV} = WQCV / 12 * Area</math></p> <p>F) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm</p> <p>G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume</p> <p>H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)</p>	<p><math>I_a =</math> <u>54.6</u> %</p> <p><math>i =</math> <u>0.546</u></p> <p>WQCV = <u>0.18</u> watershed inches</p> <p>Area = <u>37,324</u> sq ft</p> <p><math>V_{WQCV} =</math> <u>546</u> cu ft</p> <p><math>d_s =</math> _____ in</p> <p><math>V_{WQCV\ OTHER} =</math> _____ cu ft</p> <p><math>V_{WQCV\ USER} =</math> _____ cu ft</p>
<p>2. Basin Geometry</p> <p>A) WQCV Depth</p> <p>B) Sand Filter Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred). Use "0" if sand filter has vertical walls.</p> <p>C) Minimum Filter Area (Flat Surface Area)</p> <p>D) Actual Filter Area</p> <p>E) Volume Provided</p>	<p><math>D_{WQCV} =</math> <u>1.2</u> ft</p> <p><math>Z =</math> <u>3.00</u> ft / ft <b>DIFFICULT TO MAINTAIN, INCREASE WHERE POSSIBLE</b></p> <p><math>A_{Min} =</math> <u>255</u> sq ft</p> <p><math>A_{Actual} =</math> <u>262</u> sq ft</p> <p><math>V_T =</math> <u>548</u> cu ft</p>
<p>3. Filter Material</p>	<p>Choose One</p> <p><input checked="" type="radio"/> 18" CDOT Class B or C Filter Material</p> <p><input type="radio"/> Other (Explain): _____</p>
<p>4. Underdrain System</p> <p>A) Are underdrains provided?</p> <p>B) Underdrain system orifice diameter for 12 hour drain time</p> <p style="margin-left: 20px;">i) Distance From Lowest Elevation of the Storage Volume to the Center of the Orifice</p> <p style="margin-left: 20px;">ii) Volume to Drain in 12 Hours</p> <p style="margin-left: 20px;">iii) Orifice Diameter, 3/8" Minimum</p>	<p>Choose One</p> <p><input checked="" type="radio"/> YES</p> <p><input type="radio"/> NO</p> <p><math>y =</math> <u>2.0</u> ft</p> <p><math>Vol_{12} =</math> <u>546</u> cu ft</p> <p><math>D_o =</math> <u>1 / 2</u> in</p>

Design Procedure Form: Sand Filter (SF)

Sheet 2 of 2

Designer: D. Gorman  
Company: M.V.E., Inc.  
Date: March 17, 2018  
Project: Monument Small Engine Storage  
Location: Basin B1 - Pond at DP2

5. Impermeable Geomembrane Liner and Geotextile Separator Fabric

A) Is an impermeable liner provided due to proximity of structures or groundwater contamination?

Choose One

YES  NO

6-7. Inlet / Outlet Works

A) Describe the type of energy dissipation at inlet points and means of conveying flows in excess of the WQCV through the outlet

12" pvc pipe outlet and 8' wide emergency spillway  
\_\_\_\_\_  
\_\_\_\_\_

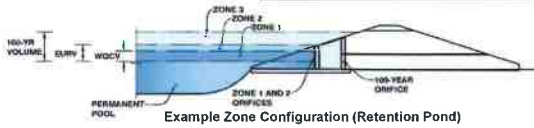
Notes: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

## DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: 61092 - Monument Small Engine Storage

Basin ID: Basin B1 sand filter DP 2



### Required Volume Calculation

Selected BMP Type =	SF	
Watershed Area =	0.86	acres
Watershed Length =	265	ft
Watershed Slope =	0.023	ft/ft
Watershed Imperviousness =	54.60%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Desired WQCV Drain Time =	12.0	hours
Location for 1-hr Rainfall Depths =	User Input	
Water Quality Capture Volume (WQCV) =	0.013	acre-feet
Excess Urban Runoff Volume (EURV) =	0.051	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.041	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	0.056	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	0.074	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	0.100	acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	0.119	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	0.143	acre-feet
500-yr Runoff Volume (P1 = 3.4 in.) =	0.210	acre-feet
Approximate 2-yr Detention Volume =	0.038	acre-feet
Approximate 5-yr Detention Volume =	0.052	acre-feet
Approximate 10-yr Detention Volume =	0.068	acre-feet
Approximate 25-yr Detention Volume =	0.074	acre-feet
Approximate 50-yr Detention Volume =	0.078	acre-feet
Approximate 100-yr Detention Volume =	0.086	acre-feet

Optional User Override 1-hr Precipitation	
1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.40	inches

### Stage-Storage Calculation

Zone 1 Volume (WQCV) =	0.013	acre-feet
Zone 2 Volume (2-year - Zone 1) =	0.026	acre-feet
Select Zone 3 Storage Volume (Optional) =		
Total Detention Basin Volume =	0.038	acre-feet
Initial Surcharge Volume (ISV) =	N/A	ft <sup>3</sup>
Initial Surcharge Depth (ISD) =	N/A	ft
Total Available Detention Depth (H <sub>total</sub> ) =	user	ft
Depth of Trickle Channel (H <sub>TC</sub> ) =	N/A	ft
Slope of Trickle Channel (S <sub>TC</sub> ) =	N/A	ft/ft
Slopes of Main Basin Sides (S <sub>main</sub> ) =	user	H:V
Basin Length-to-Width Ratio (R <sub>L/W</sub> ) =	user	
Initial Surcharge Area (A <sub>ISV</sub> ) =	user	ft <sup>2</sup>
Surcharge Volume Length (L <sub>ISV</sub> ) =	user	ft
Surcharge Volume Width (W <sub>ISV</sub> ) =	user	ft
Depth of Basin Floor (H <sub>1,00R</sub> ) =	user	ft
Length of Basin Floor (L <sub>1,00R</sub> ) =	user	ft
Width of Basin Floor (W <sub>1,00R</sub> ) =	user	ft
Area of Basin Floor (A <sub>1,00R</sub> ) =	user	ft <sup>2</sup>
Volume of Basin Floor (V <sub>1,00R</sub> ) =	user	ft <sup>3</sup>
Depth of Main Basin (H <sub>MAIN</sub> ) =	user	ft
Length of Main Basin (L <sub>MAIN</sub> ) =	user	ft
Width of Main Basin (W <sub>MAIN</sub> ) =	user	ft
Area of Main Basin (A <sub>MAIN</sub> ) =	user	ft <sup>2</sup>
Volume of Main Basin (V <sub>MAIN</sub> ) =	user	ft <sup>3</sup>
Calculated Total Basin Volume (V <sub>total</sub> ) =	user	acre-feet

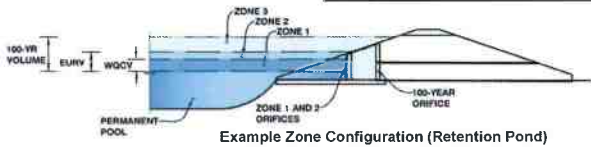
Total detention volume is less than 100-year volume.

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft <sup>2</sup> )	Optional Override Area (ft <sup>2</sup> )	Area (acre)	Volume (ft <sup>3</sup> )	Volume (ac-ft)
Media Surface	--	0.00	--	--	--	299	0.007		
	--	0.10	--	--	--	325	0.007	31	0.001
	--	0.20	--	--	--	352	0.008	62	0.001
	--	0.30	--	--	--	378	0.009	98	0.002
	--	0.40	--	--	--	404	0.009	137	0.003
	--	0.50	--	--	--	431	0.010	178	0.004
	--	0.60	--	--	--	457	0.010	222	0.005
	--	0.70	--	--	--	483	0.011	269	0.006
	--	0.80	--	--	--	509	0.012	318	0.007
	--	0.90	--	--	--	536	0.012	370	0.009
	--	1.00	--	--	--	562	0.013	425	0.010
	--	1.10	--	--	--	594	0.014	482	0.011
WQCV (6963.20)	--	1.20	--	--	--	626	0.014	543	0.012
	--	1.30	--	--	--	658	0.015	607	0.014
	--	1.40	--	--	--	690	0.016	674	0.015
	--	1.50	--	--	--	722	0.017	744	0.017
	--	1.60	--	--	--	754	0.017	818	0.019
	--	1.70	--	--	--	786	0.018	895	0.021
	--	1.80	--	--	--	818	0.019	974	0.022
	--	1.90	--	--	--	850	0.020	1,057	0.024
6964 Emrg Splyw	--	2.00	--	--	--	882	0.020	1,144	0.026
	--	2.10	--	--	--	943	0.022	1,244	0.029
	--	2.20	--	--	--	1,003	0.023	1,341	0.031
	--	2.30	--	--	--	1,064	0.024	1,444	0.033
	--	2.40	--	--	--	1,124	0.026	1,554	0.036
	--	2.50	--	--	--	1,185	0.027	1,669	0.038
	--	2.60	--	--	--	1,246	0.029	1,791	0.041
	--	2.70	--	--	--	1,306	0.030	1,918	0.044
	--	2.80	--	--	--	1,367	0.031	2,052	0.047
	--	2.90	--	--	--	1,427	0.033	2,192	0.050
6965 Top Embkmt	--	3.00	--	--	--	1,488	0.034	2,338	0.054

## Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: 61092 - Monument Small Engine Storage  
Basin ID: Basin B1 sand filter DP 2



	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.20	0.013	Filtration Media
Zone 2 (2-year)	2.51	0.026	Weir&Pipe (Restrict)
Zone 3			
		0.038	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = 2.00 ft (distance below the filtration media surface)  
Underdrain Orifice Diameter = 0.54 inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = 0.0 ft<sup>2</sup>  
Underdrain Orifice Centroid = 0.02 feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)  
Depth at top of Zone using Orifice Plate = N/A ft (relative to basin bottom at Stage = 0 ft)  
Orifice Plate: Orifice Vertical Spacing = N/A inches  
Orifice Plate: Orifice Area per Row = N/A inches

Calculated Parameters for Plate

WQ Orifice Area per Row = N/A ft<sup>2</sup>  
Elliptical Half-Width = N/A feet  
Elliptical Slot Centroid = N/A feet  
Elliptical Slot Area = N/A ft<sup>2</sup>

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = Not Selected ft (relative to basin bottom at Stage = 0 ft)  
Depth at top of Zone using Vertical Orifice = Not Selected ft (relative to basin bottom at Stage = 0 ft)  
Vertical Orifice Diameter = Not Selected inches

Calculated Parameters for Vertical Orifice

Vertical Orifice Area = Not Selected ft<sup>2</sup>  
Vertical Orifice Centroid = Not Selected feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 2 Weir	Not Selected	
Overflow Weir Front Edge Height, H <sub>o</sub> =	1.20		ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	0.78		feet
Overflow Weir Slope =	0.00		H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	0.78		feet
Overflow Grate Open Area % =	81%		% , grate open area/total area
Debris Clogging % =	50%		%

Calculated Parameters for Overflow Weir

	Zone 2 Weir	Not Selected	
Height of Grate Upper Edge, H <sub>u</sub> =	1.20		feet
Over Flow Weir Slope Length =	0.78		feet
Grate Open Area / 100-yr Orifice Area =	0.63		should be ≥ 4
Overflow Grate Open Area w/o Debris =	0.49		ft <sup>2</sup>
Overflow Grate Open Area w/ Debris =	0.25		ft <sup>2</sup>

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 2 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	2.00		ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	12.00		inches
Restrictor Plate Height Above Pipe Invert =	12.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 2 Restrictor	Not Selected	
Outlet Orifice Area =	0.79		ft <sup>2</sup>
Outlet Orifice Centroid =	0.50		feet
Half-Central Angle of Restrictor Plate on Pipe =	3.14	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 2.00 ft (relative to basin bottom at Stage = 0 ft)  
Spillway Crest Length = 8.00 feet  
Spillway End Slopes = 4.00 H:V  
Freeboard above Max Water Surface = 0.50 feet

Calculated Parameters for Spillway

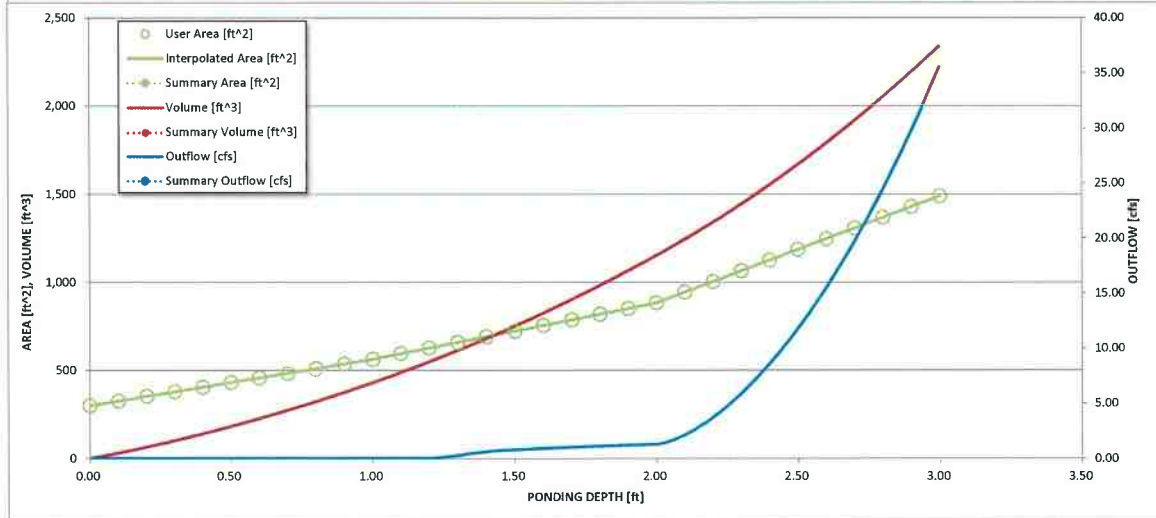
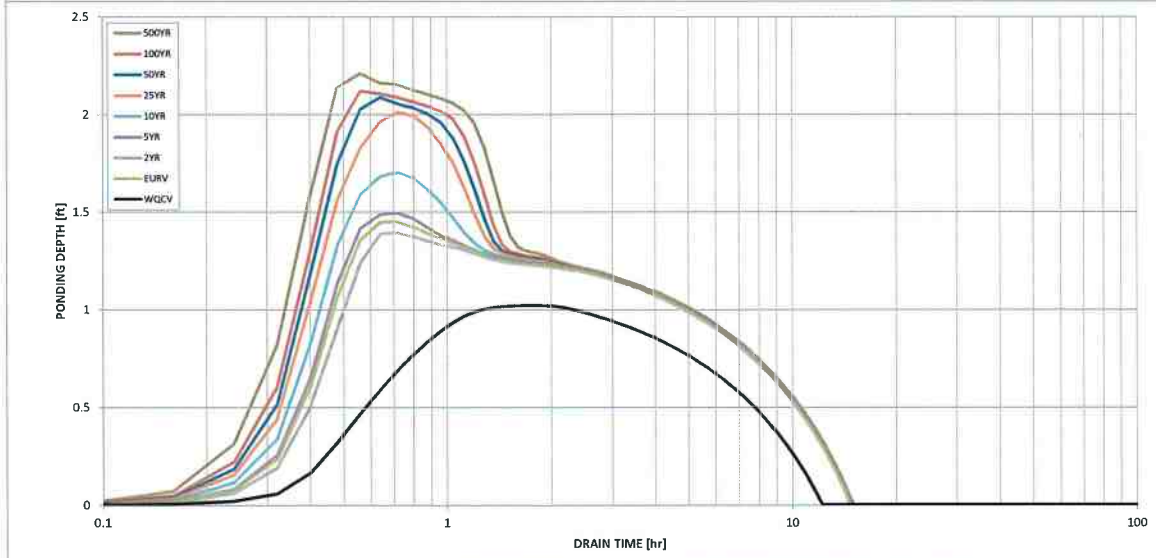
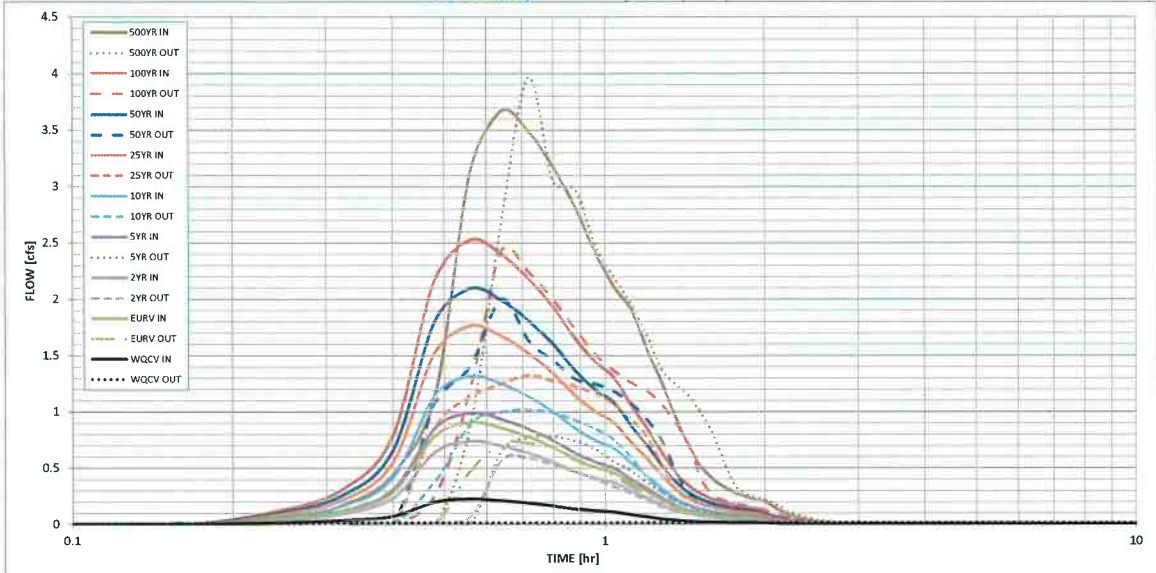
Spillway Design Flow Depth = 0.26 feet  
Stage at Top of Freeboard = 2.76 feet  
Basin Area at Top of Freeboard = 0.03 acres

### Routed Hydrograph Results

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.40
Calculated Runoff Volume (acre-ft) =	0.013	0.051	0.041	0.056	0.074	0.100	0.119	0.143	0.210
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.012	0.050	0.041	0.055	0.074	0.100	0.119	0.143	0.209
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.21	0.70	0.96	1.30	2.07
Predevelopment Peak Q (cfs) =	0.0	0.0	0.0	0.0	0.2	0.6	0.8	1.1	1.8
Peak Inflow Q (cfs) =	0.2	0.9	0.7	1.0	1.3	1.8	2.1	2.5	3.7
Peak Outflow Q (cfs) =	0.0	0.7	0.6	0.8	1.0	1.3	2.0	2.4	4.0
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	41.5	5.6	2.2	2.4	2.2	2.2
Structure Controlling Flow =	Filtration Media	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Spillway	Spillway	Spillway	Spillway
Max Velocity through Grate 1 (fps) =	N/A	1.40	1.08	1.5	2.0	2.6	2.7	2.8	2.9
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	12	13	13	13	13	12	11	10	9
Time to Drain 99% of Inflow Volume (hours) =	12	14	14	14	14	14	14	13	13
Maximum Ponding Depth (ft) =	1.02	1.45	1.39	1.50	1.71	2.01	2.09	2.12	2.21
Area at Maximum Ponding Depth (acres) =	0.01	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Maximum Volume Stored (acre-ft) =	0.010	0.016	0.015	0.017	0.021	0.027	0.028	0.029	0.031

# Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)



S-A-V-D Chart Axis Override

	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

# Channel Report

## proposed V-Ditch Basin B1 100-yr total flow (SWALE A)

### Triangular

Side Slopes (z:1) = 4.00, 4.00  
Total Depth (ft) = 1.00

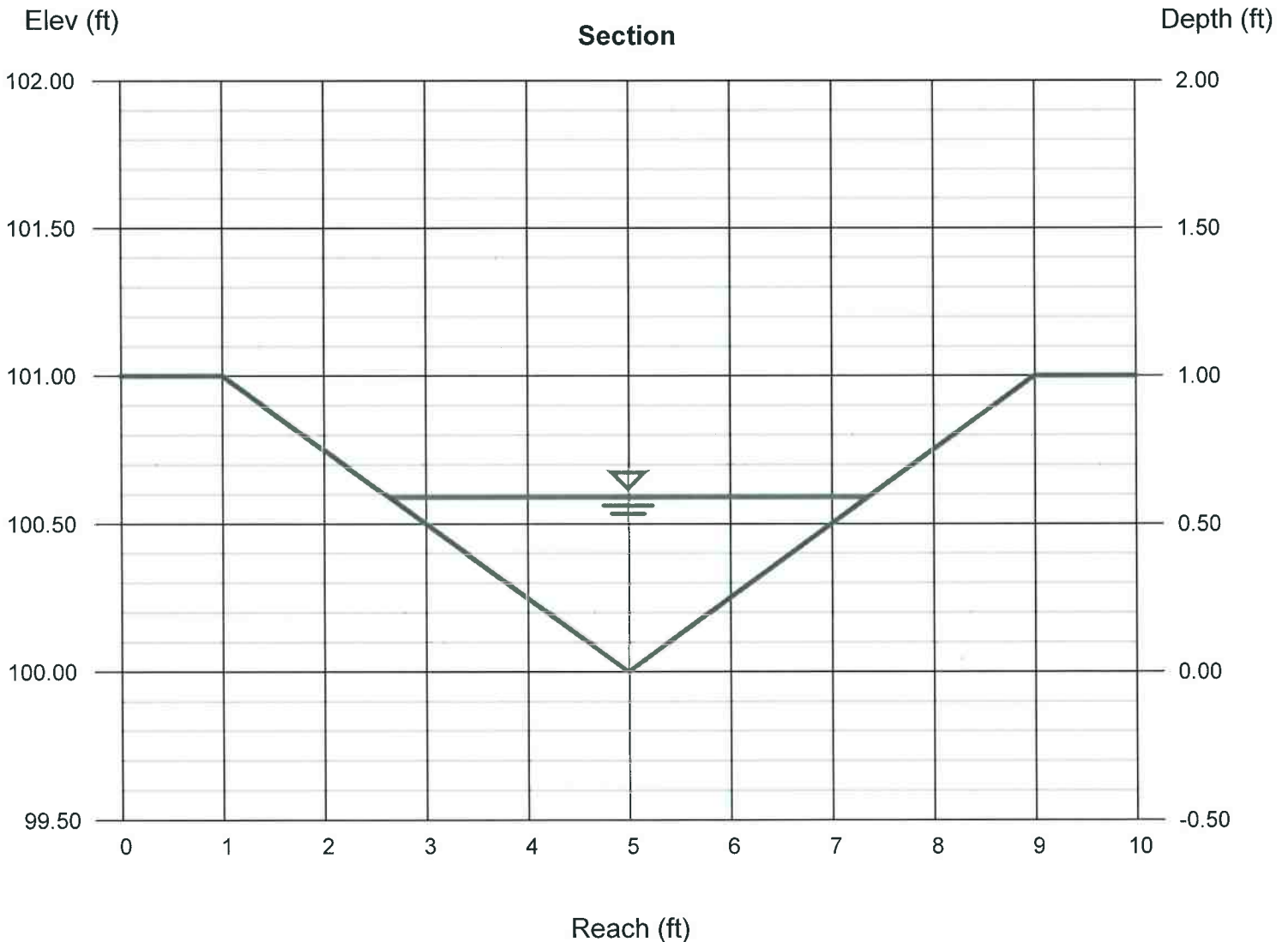
Invert Elev (ft) = 100.00  
Slope (%) = 1.50  
N-Value = 0.025

### Calculations

Compute by: Known Q  
Known Q (cfs) = 4.40

### Highlighted

Depth (ft) = 0.59  
Q (cfs) = 4.400  
Area (sqft) = 1.39  
Velocity (ft/s) = 3.16  
Wetted Perim (ft) = 4.87  
Crit Depth, Yc (ft) = 0.60  
Top Width (ft) = 4.72  
EGL (ft) = 0.75





# Channel Report

## Pond Outfall - Swale B

### Trapezoidal

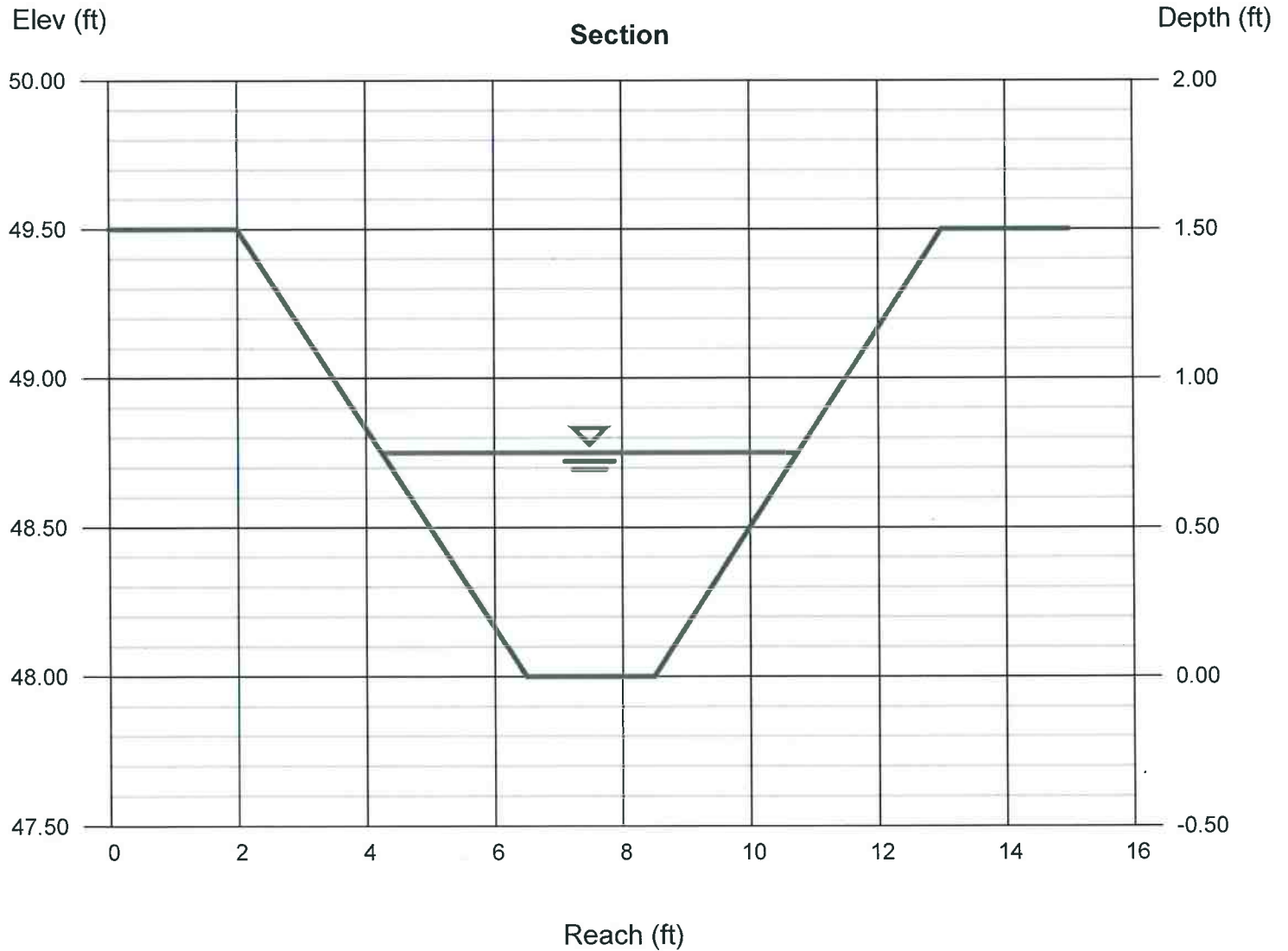
Bottom Width (ft) = 2.00  
Side Slopes (z:1) = 3.00, 3.00  
Total Depth (ft) = 1.50  
Invert Elev (ft) = 48.00  
Slope (%) = 4.00  
N-Value = 0.035

### Highlighted

Depth (ft) = 0.75  
Q (cfs) = 16.00  
Area (sqft) = 3.19  
Velocity (ft/s) = 5.02  
Wetted Perim (ft) = 6.74  
Crit Depth,  $Y_c$  (ft) = 0.85  
Top Width (ft) = 6.50  
EGL (ft) = 1.14

### Calculations

Compute by: Known Q  
Known Q (cfs) = 16.00



# Channel Report

## offsite west existing swale

### Triangular

Side Slopes (z:1) = 15.00, 50.00  
Total Depth (ft) = 1.00

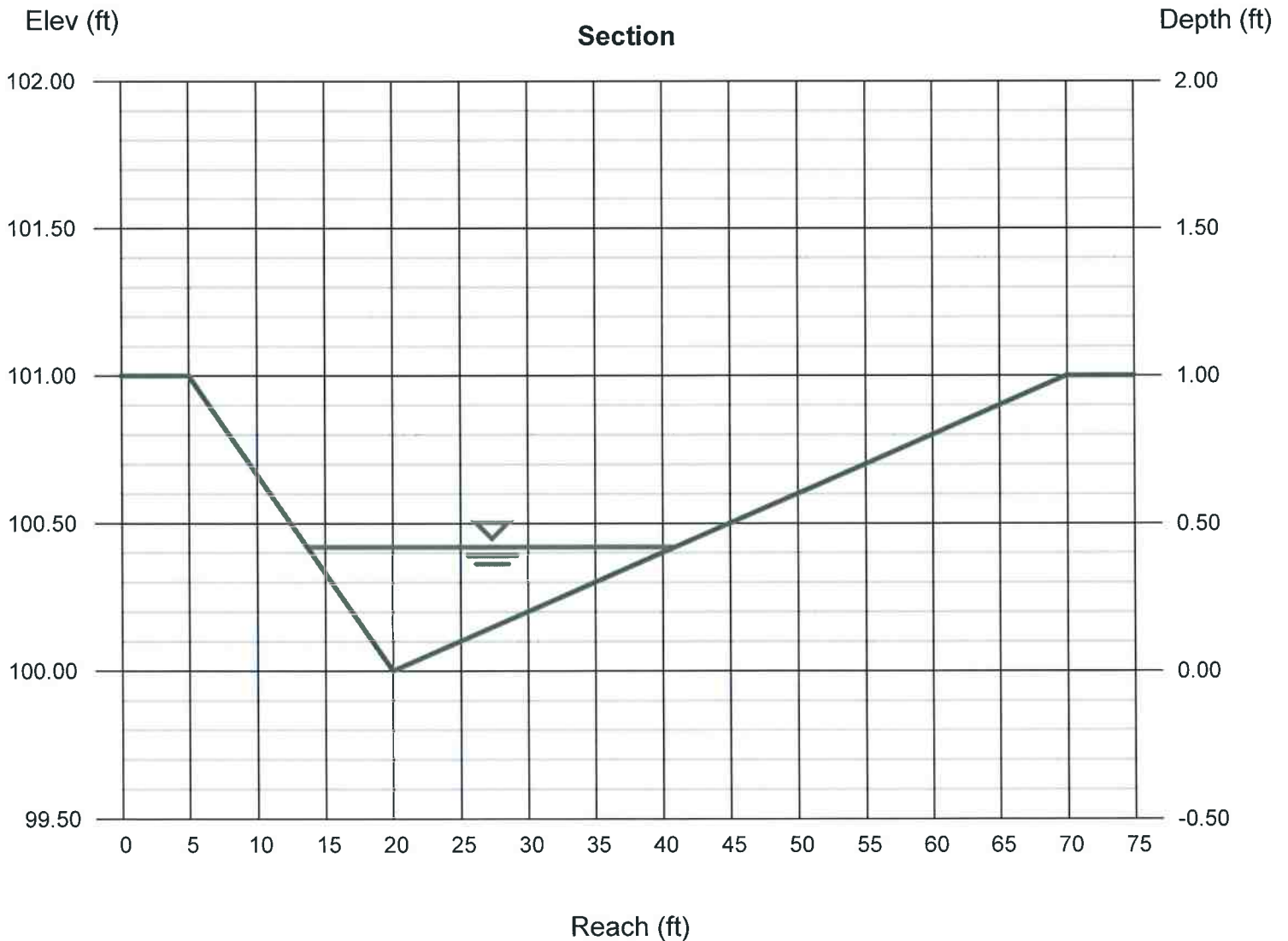
Invert Elev (ft) = 100.00  
Slope (%) = 3.80  
N-Value = 0.035

### Calculations

Compute by: Known Q  
Known Q (cfs) = 16.00

### Highlighted

Depth (ft) = 0.42  
Q (cfs) = 16.00  
Area (sqft) = 5.73  
Velocity (ft/s) = 2.79  
Wetted Perim (ft) = 27.32  
Crit Depth, Yc (ft) = 0.44  
Top Width (ft) = 27.30  
EGL (ft) = 0.54



# Culvert Report

## new 24 in CMP Driveway Culvert

Site plan shows one 24" pipe.  
Calculations show two 18" pipes. Revise either the plans or calculations so all information matches.

Invert Elev Dn (ft)	=	48.00
Pipe Length (ft)	=	47.00
Slope (%)	=	1.91
Invert Elev Up (ft)	=	48.90
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	2
n-Value	=	0.012
Culvert Type	=	Circular Corrugate Metal Pipe
Culvert Entrance	=	Projecting
Coeff. K,M,c,Y,k	=	0.034, 1.5, 0.0553, 0.54, 0.9

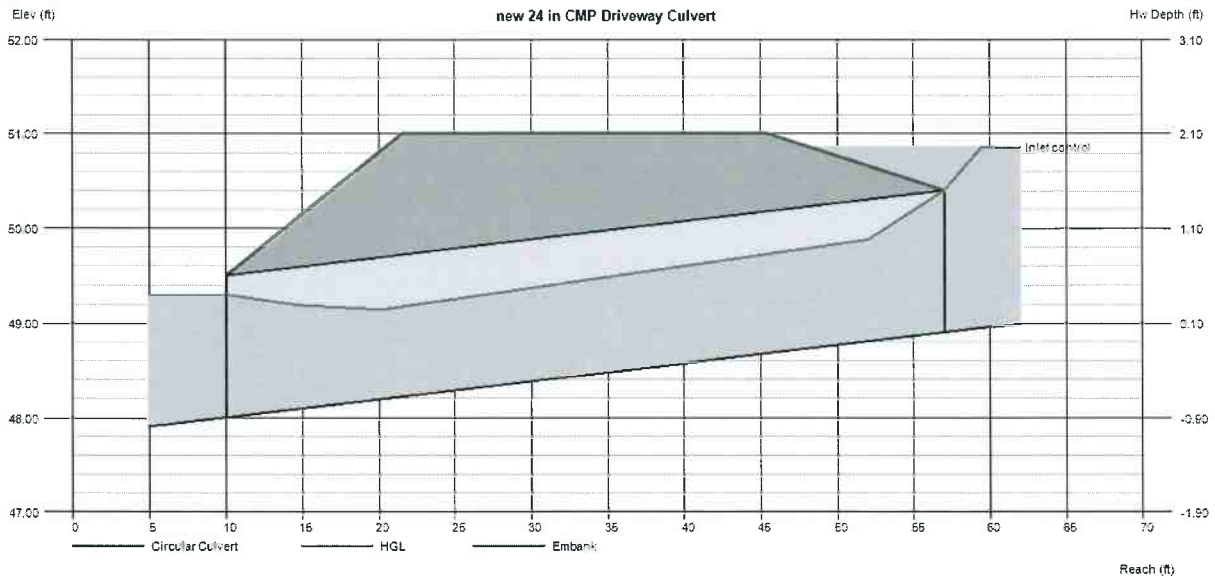
Qmin (cfs)	=	16.00
Qmax (cfs)	=	16.00
Tailwater Elev (ft)	=	(dc+D)/2

### Embankment

Top Elevation (ft)	=	51.00
Top Width (ft)	=	24.00
Crest Width (ft)	=	50.00

### Highlighted

Qtotal (cfs)	=	16.00
Qpipe (cfs)	=	16.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	4.92
Veloc Up (ft/s)	=	5.79
HGL Dn (ft)	=	49.30
HGL Up (ft)	=	49.99
Hw Elev (ft)	=	50.86
Hw/D (ft)	=	1.31
Flow Regime	=	Inlet Control



# Weir Report

## DP 1 Pond Emergency Spillway - 100-yr

### Trapezoidal Weir

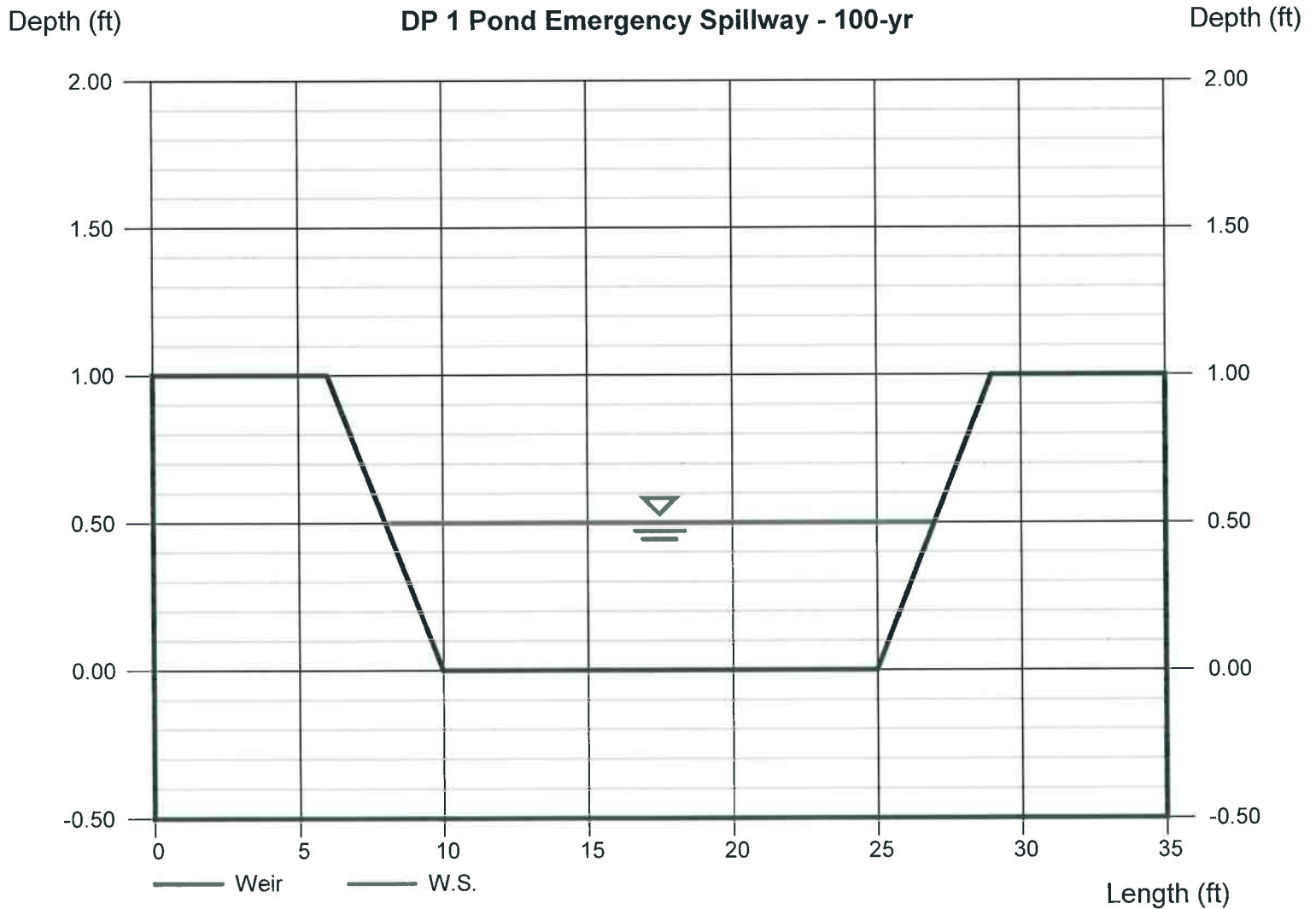
Crest = Sharp  
Bottom Length (ft) = 15.00  
Total Depth (ft) = 1.00  
Side Slope (z:1) = 4.00

### Highlighted

Depth (ft) = 0.50  
Q (cfs) = 18.10  
Area (sqft) = 8.50  
Velocity (ft/s) = 2.13  
Top Width (ft) = 19.00

### Calculations

Weir Coeff. Cw = 3.10  
Compute by: Known Q  
Known Q (cfs) = 18.10



# Weir Report

## DP 2 Pond Emergency Spillway - 100-yr

### Trapezoidal Weir

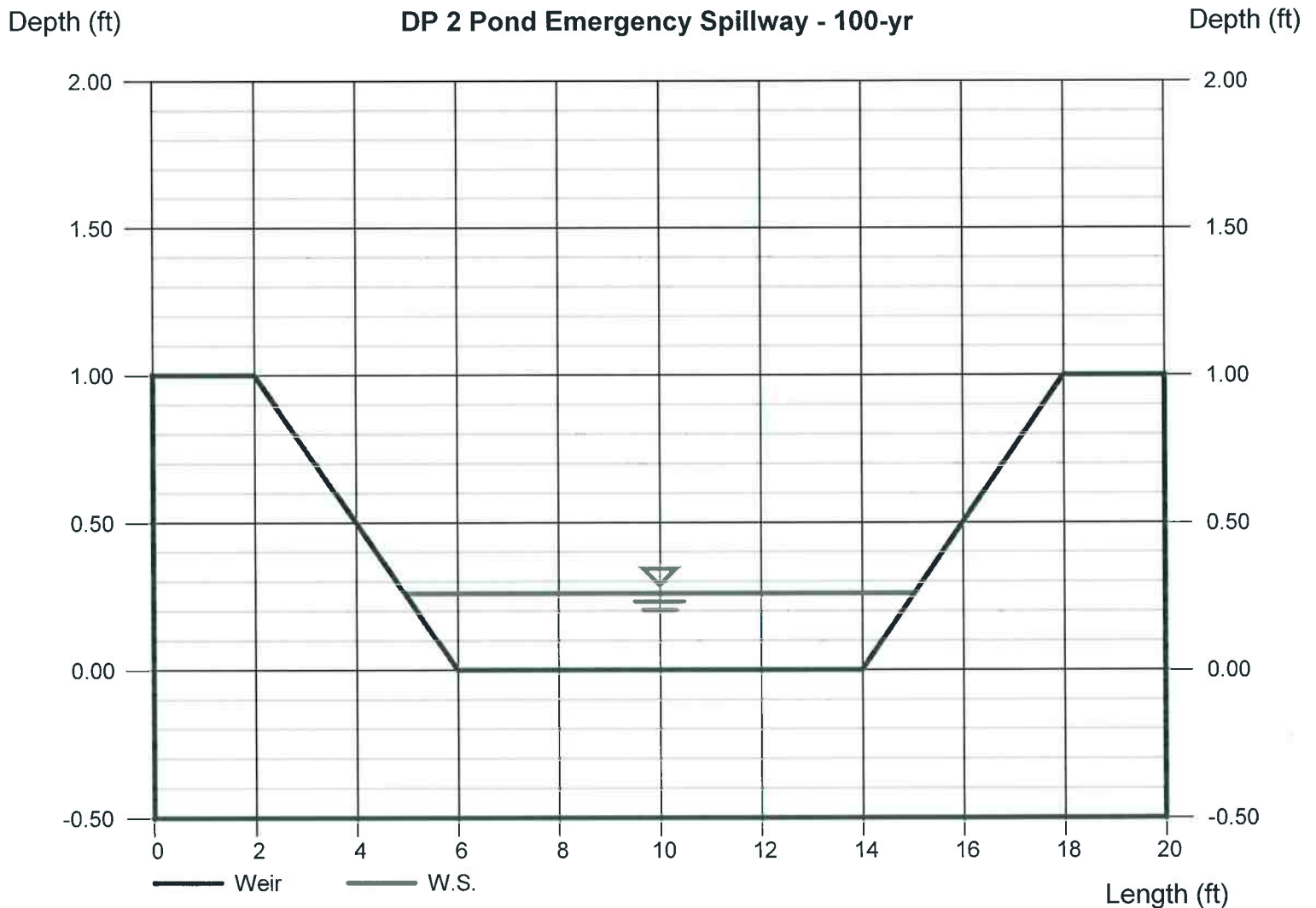
Crest = Sharp  
Bottom Length (ft) = 8.00  
Total Depth (ft) = 1.00  
Side Slope (z:1) = 4.00

### Highlighted

Depth (ft) = 0.26  
Q (cfs) = 3.600  
Area (sqft) = 2.35  
Velocity (ft/s) = 1.53  
Top Width (ft) = 10.08

### Calculations

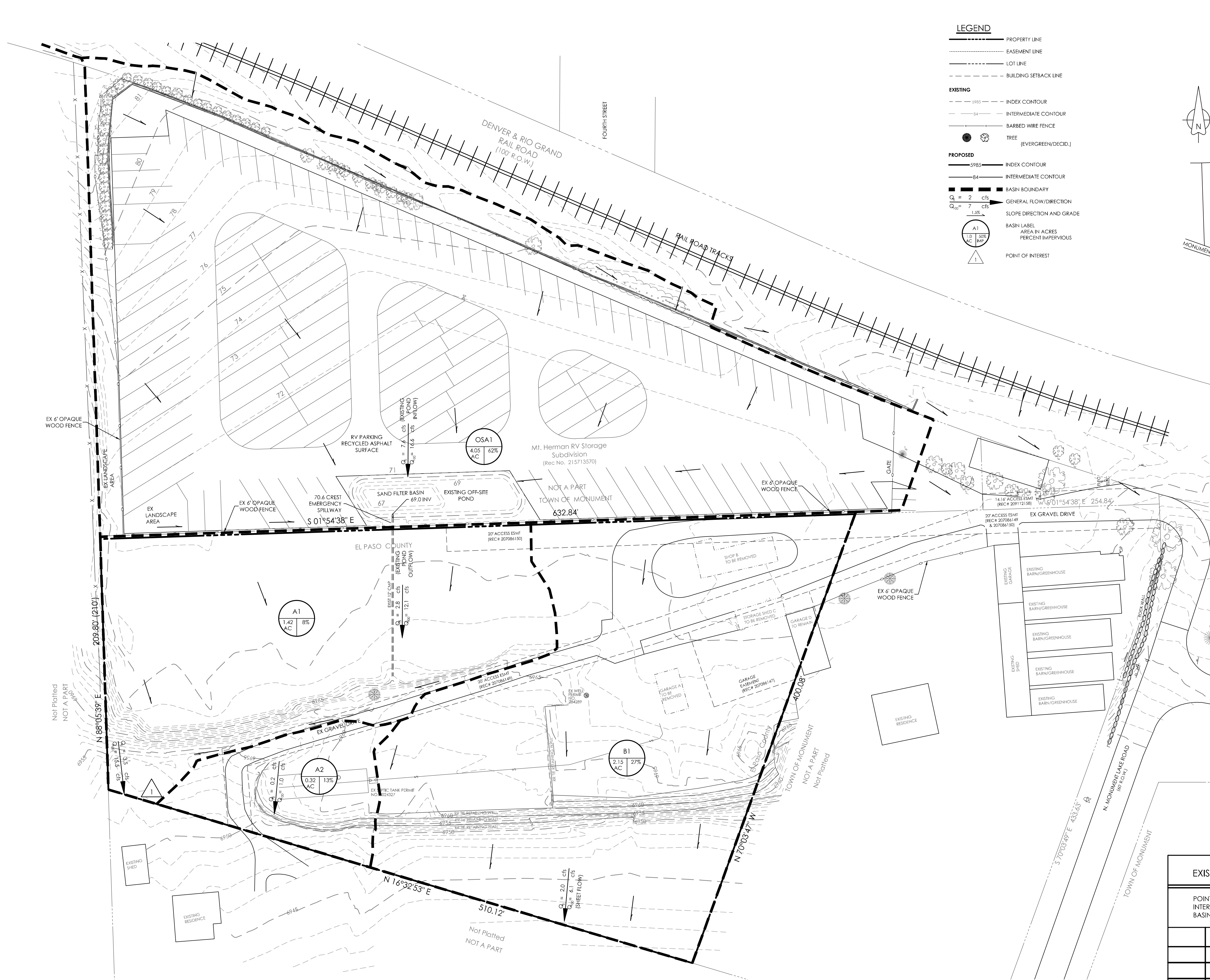
Weir Coeff. Cw = 3.10  
Compute by: Known Q  
Known Q (cfs) = 3.60



## **11 Report Maps**

Existing Condition Hydraulic Analysis Map (Map Pocket)  
Proposed Condition Hydraulic Analysis Map (Map Pocket)





**LEGEND**

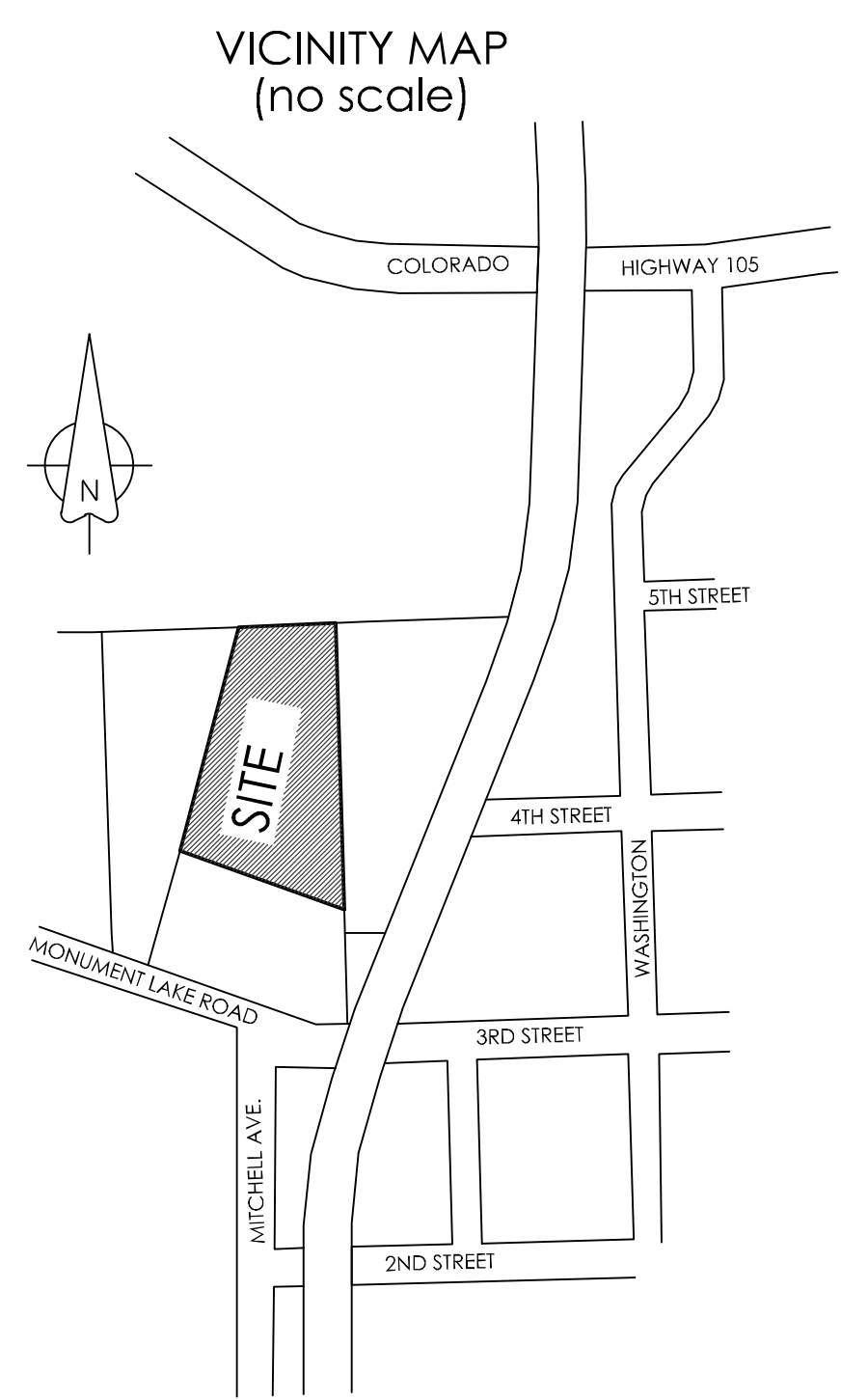
PROPERTY LINE  
 EASEMENT LINE  
 LOT LINE  
 BUILDING SETBACK LINE

**EXISTING**

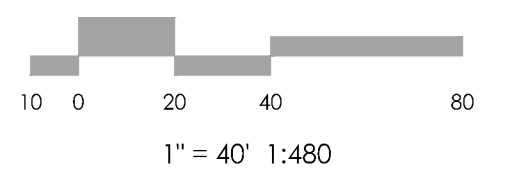
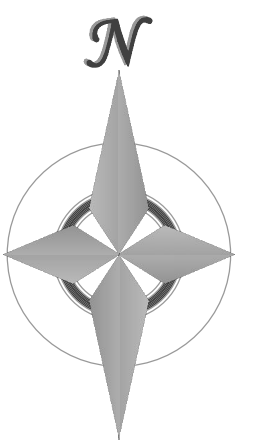
INDEX CONTOUR  
 INTERMEDIATE CONTOUR  
 BARBED WIRE FENCE  
 TREE (EVERGREEN/DECID.)

**PROPOSED**

INDEX CONTOUR  
 INTERMEDIATE CONTOUR  
 BASIN BOUNDARY  
 GENERAL FLOW/DIRECTION  
 SLOPE DIRECTION AND GRADE  
 BASIN LABEL  
 AREA IN ACRES  
 PERCENT IMPERVIOUS  
 POINT OF INTEREST



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REVISIONS

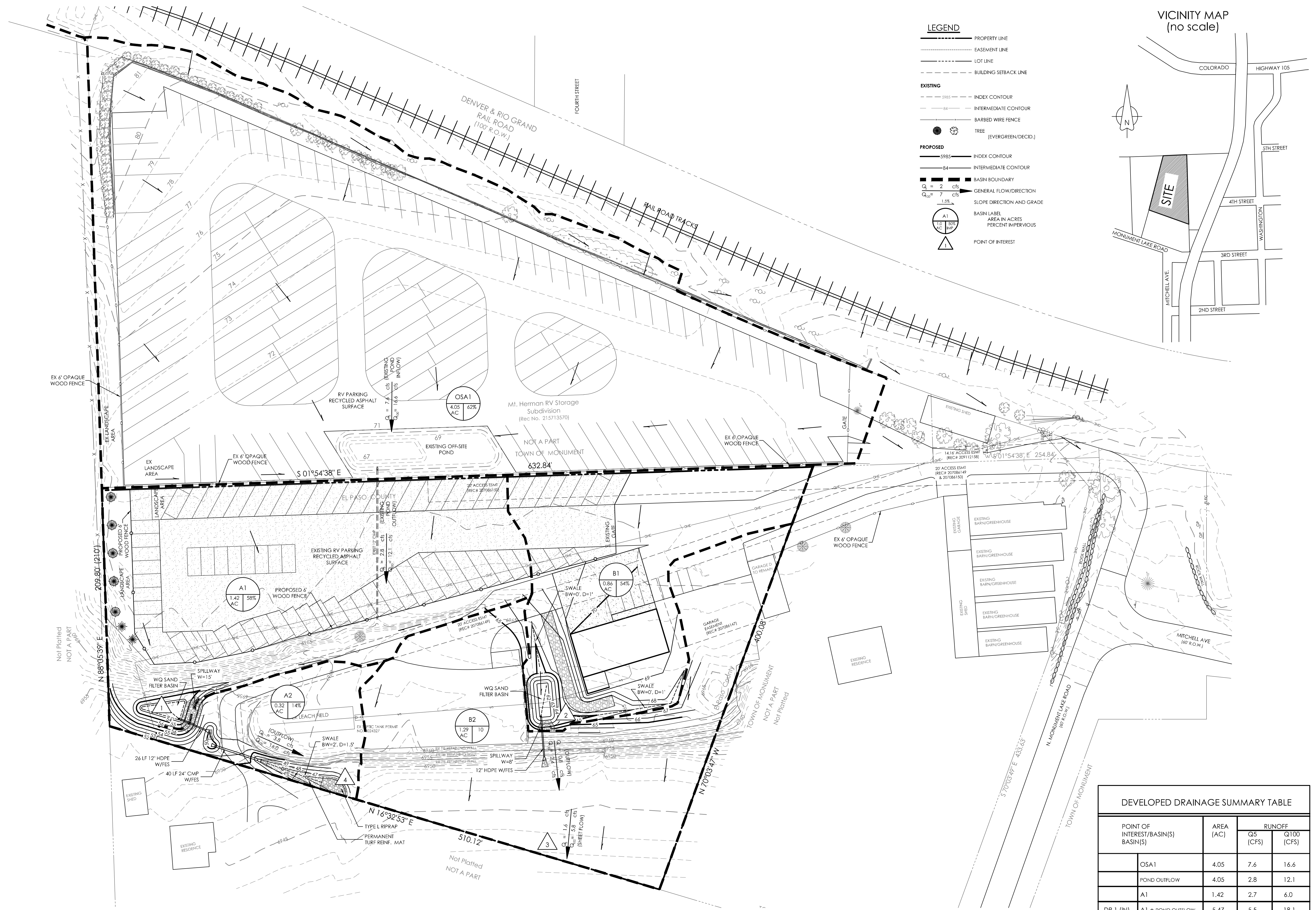
DESIGNED BY \_\_\_\_\_  
 DRAWN BY \_\_\_\_\_  
 CHECKED BY \_\_\_\_\_  
 AS-BUILT BY \_\_\_\_\_  
 CHECKED BY \_\_\_\_\_

**Monument Small Engine Storage**  
**Existing Drainage Map**

**EXISTING DRAINAGE SUMMARY TABLE**

POINT OF INTEREST/BASIN(S)	AREA (AC)	RUNOFF	
		Q5 (CFS)	Q100 (CFS)
OSA1	4.05	7.6	16.6
POND OUTFLOW	4.05	2.8	12.1
A1	1.42	0.7	3.4
DP 1	A1 + POND OUTFLOW	3.5	15.5
A2	0.32	0.2	1.0
DP1 + A2	5.79	3.7	16.3
B1	2.15	2.0	6.1

MVE PROJECT 61092  
 MVE DRAWING-Ex-Drain-Map



**LEGEND**

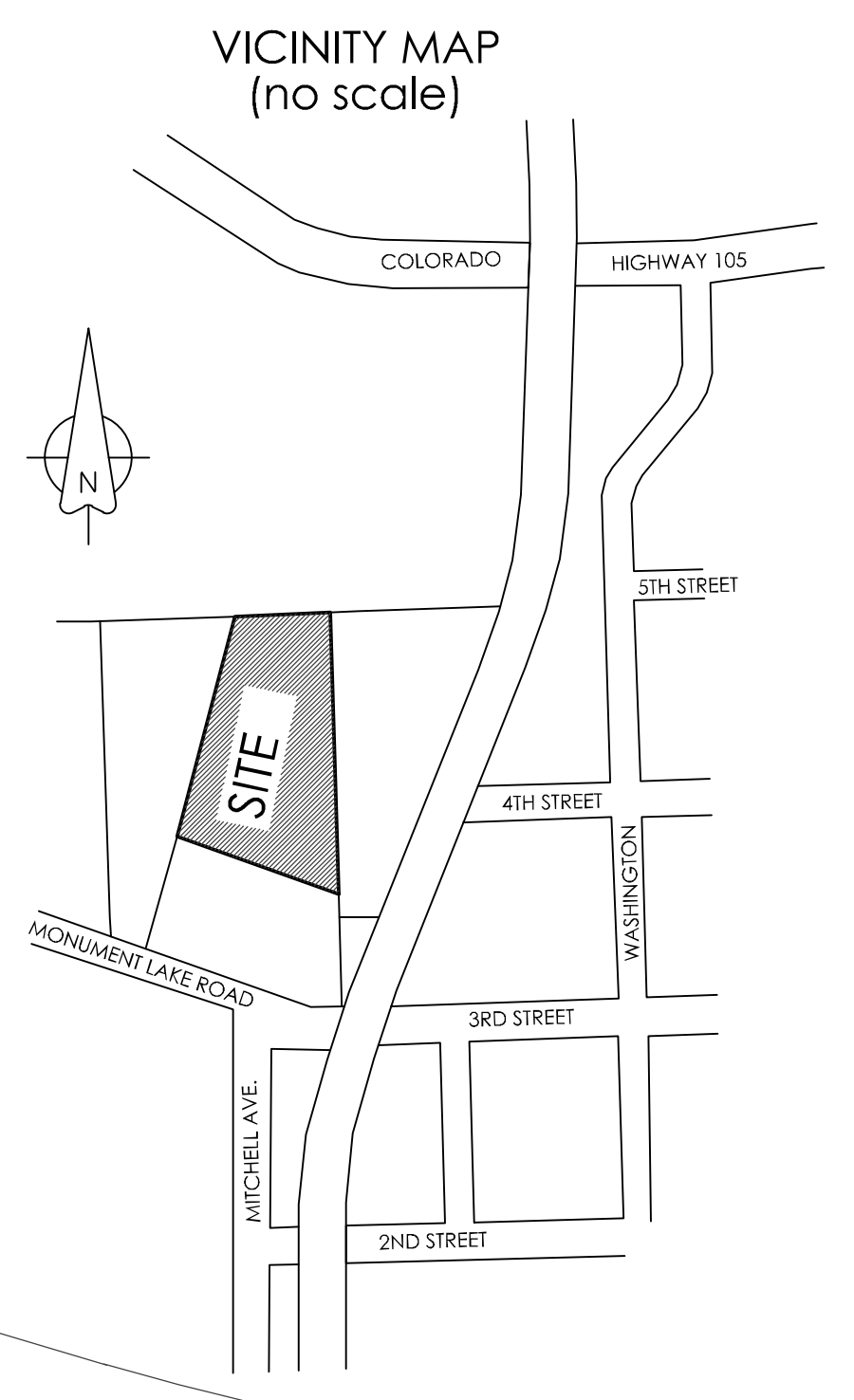
PROPERTY LINE  
 EASEMENT LINE  
 LOT LINE  
 BUILDING SETBACK LINE

**EXISTING**

INDEX CONTOUR  
 INTERMEDIATE CONTOUR  
 BARBED WIRE FENCE  
 TREE [EVERGREEN/DECID.]

**PROPOSED**

INDEX CONTOUR  
 INTERMEDIATE CONTOUR  
 BASIN BOUNDARY  
 GENERAL FLOW/DIRECTION  
 SLOPE DIRECTION AND GRADE  
 BASIN LABEL  
 AREA IN ACRES  
 PERCENT IMPERVIOUS  
 POINT OF INTEREST



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REVISIONS

**DEVELOPED DRAINAGE SUMMARY TABLE**

POINT OF INTEREST/BASIN(S)	AREA (AC)	RUNOFF	
		Q5 (CFS)	Q100 (CFS)
OSA1	4.05	7.6	16.6
POND OUTFLOW	4.05	2.8	12.1
A1	1.42	2.7	6.0
DP 1 (IN)	A1 + POND OUTFLOW	5.47	18.1
DP 1 (OUT)	A1 + POND OUTFLOW	5.47	16.0
A2	0.32	0.2	1.0
DP 4	DP1 (OUT) + A2	5.79	17.0
DP 2 (IN)	B1	0.86	3.6
DP 2 (OUT)	B1	0.8	2.4
B2	1.29	0.8	3.4
DP 3	B2 + B1 OUTFLOW	2.15	5.8

DESIGNED BY \_\_\_\_\_  
 DRAWN BY \_\_\_\_\_  
 CHECKED BY \_\_\_\_\_  
 AS-BUILTS BY \_\_\_\_\_  
 CHECKED BY \_\_\_\_\_

**Monument Small Engine Storage**

**Developed Drainage Map**

MVE PROJECT 61092  
 MVE DRAWING -PP-Drain-Map

June 15, 2018  
 SHEET 1 OF 1

Z:\1092\_3\Drawings\1092-PP-Drain-Map.dwg 6/21/18 10:52:37 AM DKS

# Markup Summary

dsdgrimm (1)

## Report

Site plan shows one 24" pipe. Calculations show two 18" pipes. Revise either the plans or calculations so all information matches.

1 (ft)	= 45.00	Qmin (cfs)	= 18.00
2 (ft)	= 47.00	Qmax (cfs)	= 18.00
3 (ft)	= 48.00	Talwater Elev (ft)	= 50+0.02
4 (ft)	= 49.00		
5 (ft)	= 50.00		
6 (ft)	= 51.00		
7 (ft)	= 52.00		
8 (ft)	= 53.00		
9 (ft)	= 54.00		
10 (ft)	= 55.00		
11 (ft)	= 56.00		
12 (ft)	= 57.00		
13 (ft)	= 58.00		
14 (ft)	= 59.00		
15 (ft)	= 60.00		
16 (ft)	= 61.00		
17 (ft)	= 62.00		
18 (ft)	= 63.00		
19 (ft)	= 64.00		
20 (ft)	= 65.00		
21 (ft)	= 66.00		
22 (ft)	= 67.00		
23 (ft)	= 68.00		
24 (ft)	= 69.00		
25 (ft)	= 70.00		
26 (ft)	= 71.00		
27 (ft)	= 72.00		
28 (ft)	= 73.00		
29 (ft)	= 74.00		
30 (ft)	= 75.00		
31 (ft)	= 76.00		
32 (ft)	= 77.00		
33 (ft)	= 78.00		
34 (ft)	= 79.00		
35 (ft)	= 80.00		
36 (ft)	= 81.00		
37 (ft)	= 82.00		
38 (ft)	= 83.00		
39 (ft)	= 84.00		
40 (ft)	= 85.00		
41 (ft)	= 86.00		
42 (ft)	= 87.00		
43 (ft)	= 88.00		
44 (ft)	= 89.00		
45 (ft)	= 90.00		
46 (ft)	= 91.00		
47 (ft)	= 92.00		
48 (ft)	= 93.00		
49 (ft)	= 94.00		
50 (ft)	= 95.00		
51 (ft)	= 96.00		
52 (ft)	= 97.00		
53 (ft)	= 98.00		
54 (ft)	= 99.00		
55 (ft)	= 100.00		

**Subject:** Engineer  
**Page Label:** 69  
**Author:** dsdgrimm  
**Date:** 7/23/2018 1:21:35 PM  
**Color:** ■

Site plan shows one 24" pipe. Calculations show two 18" pipes. Revise either the plans or calculations so all information matches.