

**FINAL DRAINAGE REPORT
FOR
MC CLINTOCK STATION LOT A
(VOLLMER ROAD RV STORAGE)**

**Prepared For:
Scott Belknap
3603 First Light Drive
Castle Rock, CO 80109**

**June 2022
Project No. 25251.00**

**Prepared By:
JR Engineering, LLC
5475 Tech Center Drive
Colorado Springs, CO 80919
719-593-2593**

Revise to PPR-2245



PCD File No. SF-XX-XXXX

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

Ryan Burns, Colorado P.E. # 0054412
For and On Behalf of JR Engineering, LLC

Date

DEVELOPER'S STATEMENT:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: Scott Belknap

By: _____

Title: _____

Address: 3603 First Light Drive
Castle Rock, CO 80109

El Paso County:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2 and Engineering Criteria Manual, as amended.

Jennifer Irvine, P.E.
County Engineer/ ECM Administrator

Date

Conditions:

Revise to Joshua Palmer, PE



CONTENTS

PURPOSE.....	2
GENERAL LOCATION AND DESCRIPTION.....	2
LOCATION.....	2
DESCRIPTION OF PROPERTY.....	2
FLOODPLAIN STATEMENT.....	2
DRAINAGE BASINS AND SUB-BASINS.....	3
EXISTING MAJOR BASIN DESCRIPTIONS.....	3
EXISTING SUB-BASIN DRAINAGE.....	3
PROPOSED SUB-BASIN DRAINAGE.....	4
DRAINAGE DESIGN CRITERIA.....	5
DEVELOPMENT CRITERIA REFERENCE.....	5
HYDROLOGIC CRITERIA.....	5
HYDRAULIC CRITERIA.....	5
DRAINAGE FACILITY DESIGN.....	6
SPECIFIC DETAILS.....	6
<i>Four Step Process to Minimize Adverse Impacts of Urbanization</i>	6
<i>Water Quality</i>	6
<i>Erosion Control Plan</i>	7
<i>Operation & Maintenance</i>	7
<i>Drainage & Bridge Fees</i>	7
SUMMARY.....	7
REFERENCES:.....	1

APPENDICES

- A. Vicinity Map, Soils, FEMA
- B. Hydrologic Calculations
- C. Hydraulic Calculations
- D. Reference Materials
- E. Drainage Maps



PURPOSE

This document is the Final Drainage Report for Mc Clintock Station Lot A herein know as “Vollmer Road RV Storage”. The purpose of this document is to identify and analyze the on and offsite drainage patterns and to ensure that post development runoff is routed through the site safely and in a manner that satisfies the requirements set forth by the El Paso County Drainage Criteria Manual. The following report is an analysis of the drainage for the site and surrounding areas.

GENERAL LOCATION AND DESCRIPTION

Location

Vollmer Road RV Storage herein known as “the site” is located in Section 34, Township 12 South, and Range 65 West of the 6th Principal Meridian. The site is bound on the northwest by existing Vollmer Road. Vollmer Road boards Wildridge Subdivision II Lot 1, Blocks 1 and 2 to the northwest of Vollmer Road. The property is bound to the east by the Sterling Ranch Filing 1 and by Lots B and C of the Mc Clintock Station Subdivision to the south. Vollmer Road RV Storage lies within the Sand Creek Drainage Basin. Flows from this site are tributary to Sand Creek. A vicinity map is presented in Appendix A.

Description of Property

Vollmer Road RV Storage consists of 4.15 acres and is presently undeveloped. Vegetation is sparse, consisting of native grasses. Existing site terrain generally slopes from north to south at grade rates that vary between 2% and 8%.

Vollmer Road RV Storage is currently zoned "I-2" for light industrial and manufacturing development. Improvements proposed for the site includes recycled asphalt drives and parking, fencing, storm drainage improvements, drainage swales, and a detention pond. A full spectrum detention pond is proposed to be constructed to provide water quality treatment and detain storm water for the development.

Soils for this project are classified as Pring Coarse Sandy Loam (71), which is characterized as Hydrologic Soil Types "B". Group B soils exhibit moderate infiltration rate when thoroughly wet, and consist primarily of deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. A soil map of the site can be found in Appendix A.

There are no major drainage ways or known irrigation facilities located on the project site. There are no known existing onsite utilities.

Floodplain Statement

Based on the FEMA FIRM Maps number 08041C0533G, dated December 7, 2018, all of the proposed development lies within Zone X. Zone X is defined as area outside the Special Flood



Final Drainage Report
Mc Clintock Station Lot A (Vollmer Road RV Storage)
Hazard Area (SFHA) and higher than the elevation of the 0.2-percent-annual-chance (or 500-year) flood. A FIRM Map is presented in Appendix A.

DRAINAGE BASINS AND SUB-BASINS

Existing Major Basin Descriptions

The Vollmer Road RV Storage site consists of 4.15 acres and is located in the Sand Creek Drainage Basin. The site area was previously studied in the "Sand Creek Drainage Basin Planning Study" (DBPS) prepared by Stantec, January 2021.

The Sand Creek DBPS assumed the Vollmer Road RV Storage property to have an undeveloped use for the site. However, the site is zoned I-2 for light industrial and manufacturing development. The site generally drains from northwest to southeast. Currently, the site is undeveloped. Sand Creek is located east of the site running north to south.

Downstream flow patterns have been studied in "Final Drainage Report for Barbarick Subdivision, Portions of Lots 1, 2 and Lots 3 & 4," by Matrix Design Group, June 2016, and "Woodmen Storage Final Drainage Report", by Calibre Engineering Inc, Revised February 2010. Applicable excerpts from these reports can be found in Appendix D.

A summary of peak runoff for the basins and designated design points are depicted on the Existing Conditions Drainage Map in the appendix.

Existing Sub-basin Drainage

Basin EX-1 ($Q_5=0.1$ cfs, $Q_{100}=1.0$ cfs) is 0.48 acres of open space. Runoff from this basin drains overland flows to the south east to DP 1. Flows from Basins EX-1 and VX-7 combine at DP1.1 ($Q_5=0.3$ cfs, $Q_{100}=1.5$ cfs) where flow continues onto Lot B of the McClintock Station Subdivision.

Basin EX-2 ($Q_5=0.5$ cfs, $Q_{100}=3.9$ cfs) is 2.22 acres of open space. Runoff from this basin overland flows southeast to DP 2. Flows from Basins VX-5, VX-6 and EX-2 combine at DP2.1 ($Q_5=1.2$ cfs, $Q_{100}=5.7$ cfs) and continues onto Lot C of the McClintock Station Subdivision.

Basin EX-3 ($Q_5=0.2$ cfs, $Q_{100}=1.5$ cfs) is 0.88 acres of open space. Runoff from this basin overland flows south to DP 3 and onto Lot C of the McClintock Station Subdivision.

Basin EX-4 ($Q_5=0.2$ cfs, $Q_{100}=1.3$ cfs) is 0.56 of open space. Runoff from this basin overland flows east across the property line to DP4 and onto Homestead at Sterling Ranch Filing No. 1. Runoff is then captured by an existing swale.

Basin VX-5 ($Q_5=0.7$ cfs, $Q_{100}=1.9$ cfs) is 0.41 acres and is comprised of the existing Vollmer Road and road side swale. Runoff from this offsite basin overland flows onto the site at DP5 where flow enters Basin EX-2.

Indicate whether this existing swale was designed with these flows contributing to it and are the higher or lower than assumed flows?

doesn't really
ve swale.
ow is directly to

Flows and area do not match VX-5 in hydrology spreadsheet in appendix. Please revise accordingly between report and appendix



Final Drainage Report

Mc Clintock Station Lot A (Vollmer Road RV Storage)

Basin VX-6 ($Q_5=0.6$ cfs, $Q_{100}=1.5$ cfs) is 0.41 acres and is comprised of the existing Vollmer Road and road side swale. Runoff from this offsite basin overland flows to the roadside ditch and then enter the site at DP6 where flow enters Basin EX-2.

Basin VX-7 ($Q_5=0.3$ cfs, $Q_{100}=0.7$ cfs) is 0.14 acres and is comprised of the existing Vollmer Road and road side swale. Runoff from this offsite basin overland flows onto the site at DP7 where flow enters Basin EX-1.

Proposed Sub-basin Drainage

The following is a description of the offsite and onsite basins for the developed condition. Calculations have been provided to show the proposed storm infrastructure will adequately convey flows. The following basins parameters and developed runoff were determined using the Rational Method. Calculation can be found in Appendix C.

Basin A ($Q_5=4.6$ cfs, $Q_{100}=9.2$ cfs) consists of approximately 1.62 acres and consists of recycled asphalt parking and drives, and landscaping. Runoff from this basin is conveyed via sheet flow across the proposed asphalt mat to DP1, where flow enters Basin C and combines with flows from Basins B and C at DP3.1 ($Q_5=11.5$ cfs, $Q_{100}=22.6$ cfs) where flow will be captured and treated in the full spectrum detention pond.

Basin B ($Q_5=7.2$ cfs, $Q_{100}=13.0$ cfs) consists of approximately 1.74 acres and consists of recycled asphalt parking and drives, and landscaping. Runoff from this basin is conveyed via sheet flow across the proposed asphalt mat and then will be captured and treated at DP2, where flow enters Basin C and combines with flows from Basins A and C at DP3.1 ($Q_5=11.5$ cfs, $Q_{100}=22.6$ cfs) where flow will be captured and treated in the full spectrum detention pond.

Basin C ($Q_5=0.1$ cfs, $Q_{100}=0.9$ cfs) consists of approximately 0.32 acres and consists the proposed full spectrum pond that includes a concrete trickle channel and outlet structure. Runoff from this basin is conveyed via sheet flow down to the bottom of the pond and through the trickle channel to the outlet structure to DP3. Runoff from Basin C combines with flows from Basins A and B at DP3.1 ($Q_5=11.5$ cfs, $Q_{100}=22.6$ cfs) where flow will be captured and treated in the full spectrum detention pond.

Basin D ($Q_5=0.0$ cfs, $Q_{100}=0.3$ cfs) is 0.10 acres of landscaping. Runoff from this basin overland flows east across the property line to DP4 and onto Homestead at Sterling Ranch Filing No. 1. Runoff is then captured by an existing swale. **Indicate contributing flow to existing swale is less.**

Basin E ($Q_5=0.1$ cfs, $Q_{100}=0.5$ cfs) is 0.16 acres of landscaping. Runoff from this basin drains via overland flow to the south east across the site boundary and onto Lot C of the McClintock Station Subdivision at DP 5.

Basin F ($Q_5=0.1$ cfs, $Q_{100}=0.6$ cfs) is 0.17 acres of landscaping. Runoff from this basin drains via overland flow to the south across the site boundary and onto Lot B of the McClintock Station Subdivision to DP6.



Basin G ($Q_5=0.9$ cfs, $Q_{100}=2.5$ cfs) is 0.84 acres and is comprised of the existing Vollmer Road, existing and proposed road side swale, and a portion of the access road. Runoff from this basin overland flows to the roadside ditch and then enters the proposed culvert under the access road at DP7, flow continues to DP8.1 where flows from Basins G and H combine, and continues to flow existing drainage patterns to the southwest.

Basin H ($Q_5=0.1$ cfs, $Q_{100}=0.2$ cfs) is 0.03 acres and is comprised of the existing Vollmer Road, proposed road side swale, and a portion of the access road. Runoff from this basin overland flows to the roadside ditch DP8, flow continues to DP8.1 where flows from Basins G and H combine, and continues to flow existing drainage patterns to the southwest.

DP9 ($Q_5=0.5$ cfs, $Q_{100}=4.1$ cfs) is the outfall point for the proposed full spectrum water quality and detention pond. Flow will leave the site via a proposed 18" RCP storm pipe and enter the existing ten foot public utility and drainage easement. The outlet structure for the ponds shall reduce the release rates for all storm events to less than historic rates to minimize adverse impacts to downstream stormwater facilities. Flow from DP9 will continue to follow existing drainage patterns.

DRAINAGE DESIGN CRITERIA

Development Criteria Reference

Storm drainage analysis and design criteria for the project were taken from the "*City of Colorado Spring/El Paso County Drainage Criteria Manual*" Volumes 1 and 2 (EPCDCM), dated October 12, 1994, the "*Urban Storm Drainage Criteria Manual*" Volumes 1 - 3 (USDCM) and Chapter 6 and Section 3.2.1 of Chapter 13 of the "*Colorado Springs Drainage Criteria Manual (CCSDCM)*", dated May 2014, as adopted by El Paso County.

Hydrologic Criteria

All hydrologic data was obtained from the "El Paso Drainage Criteria Manual" Volumes 1 and 2, and the "Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual" Volumes 1, 2, and 3. Onsite drainage improvements were designed based on the 5 year (minor) storm event and the 100-year (major) storm event. One hour point rainfall data for the storm events is identified in the table below. Rational Method calculations were prepared, in accordance with Section 3.0 of the EPCDCM. Rational method calculations are presented in Appendix B.

Table 1: 1-hr Point Rainfall Data

Storm	Rainfall (in.)
5-year	1.50
100-year	2.52

Hydraulic Criteria

Mile High Flood District's MHFD-Detention, Version 4.05 workbook was used for pond sizing. Required detention volumes and allowable release rates were designed per USDCM and



If Final report, calculations should all be final, not preliminary

Final Drainage Report
Mc Clintock Station Lot A (Vollmer Road RV Storage)
CCS/EPCDCM. Pond sizing spreadsheets are presented in Appendix C. The Manning's equation has been utilized as a preliminary sizing check for the proposed drive access culvert. Refer to Appendix C for pipe capacity calculations of the 18" RCP culvert.

DRAINAGE FACILITY DESIGN

Specific Details

Four Step Process to Minimize Adverse Impacts of Urbanization

In accordance with the El Paso County Drainage Criteria Manual Volume 2, this site has implemented the four step process to minimize adverse impacts of urbanization. The four step process includes reducing runoff volumes, stabilizing drainageways, treating the water quality capture volume (WQCV), and consider the need for Industrial Commercial BMP's.

Step 1, Reducing Runoff Volumes: The development of the project site consist of recycled asphalt parking and drives and landscaped areas. Proposed landscaped areas help disconnect impervious areas. Wherever possible runoff from the impervious areas will be routed to pervious areas to reduce runoff volumes and promote infiltration.

Step 2, Stabilize Drainageways: Drainage fees will be paid at the time of final platting. Drainage fees go towards channel stabilization projects throughout the drainage basin. The existing outfall for the site (DP9) is believed to be stable and sufficient in today's condition. The outfall will be analyzed for stability during final design and will be upgraded if needed.

Step 3, Treat the WQCV: Water Quality treatment for the site is provided in a proposed full spectrum water quality detention ponds located in the south west corner of the site. In general, the runoff from this site will be routed via overland flow to the proposed. A trickle channel is also incorporated into the ponds to minimize the amount of standing water. The outlet structure has been designed to detain the water quality capture volume (WQCV) for 40 hours, and the extended urban runoff volume (EURV) for 72 hours. Flows released from the pond will be reduced to less than historic rates. The pond will facilitate pollutant removal for the site, while also reducing peak stormwater rates down stream.

Step 4, Consider the need for Industrial and Commercial BMP's: Temporary BMPs will be utilized during construction to minimize off-site contaminates and to protect the downstream receiving waters, Site specific temporary source control BMPs that will be implement include, but are not limited to, silt fencing, construction vehicle tracking pads, designated fueling areas, covered storage areas, spill containment and control, etc. The permanent erosion control BMPs include recycled asphalt parking and drives, permanent vegetation, a storm culvert under the access, and a full spectrum water quality and detention pond.

Water Quality

Water quality for the site is provided by a private full-spectrum detention and water quality pond in the southeast corner of the site. Table 2 below shows the basin parameters. The proposed pond is sized. The WQCV for the pond shall be released within 40 hours and the EURV shall be released within 72 hours. Table 3 below gives the design storm results. The proposed pond will utilize a

Flows are lower than historic because of the pond, but you are changing the flow from sheet flow to concentrated flow. please confirm the existing outfall is suitable.

was this sentence cut off?



The entire site has to receive water quality per ECM I.7.1.C.a

Final Drainage Report
Mc Clintock Station Lot A (Vollmer Road RV Storage)

trickle channels, and outlet structure to dissipate energy and treat flows. The outlet structure for the ponds shall reduce the release rates for all storm events to less than historic rates to minimize adverse impacts to downstream stormwater facilities. A broad crested weir will be provided as an emergency spillway and will convey emergency flows to the existing drainage easement that runs along the southern property lines.

include a forebay

Show drainage easement on GEC plan and site plan.

Table 2 - Watershed Design Parameters

Watershed Area	3.68 AC
Percent Impervious	77.0%
Watershed Slope	0.015 ft/ft

Table 3 - Design Storm Results

Design Storm Period	Volume (AC-FT)	Depth (FT)	Q _{out 100} (CFS)
WQCV	0.096	2.25	0.0
EURV	0.315	4.01	0.2
100-YR	0.431	4.73	4.1

Include what the design volumes are in the table.

Erosion Control Plan

The El Paso County Drainage Criteria Manual specifies an Erosion Control Plan and associated cost estimate must be submitted with each Final Drainage Report. The Erosion Control Plan for Vollmer RV Storage has been submitted with this report.

Operation & Maintenance

delete

In order to ensure the function and effectiveness of the stormwater infrastructure, maintenance activities such as inspection, routine maintenance, restorative maintenance, rehabilitation and repair, are required. All proposed drainage structures within the any platted County ROW will be owned and maintained by El Paso County. All proposed drainage structures within the property will be owned and maintained by Scott Belknap. An Inspection & Maintenance Plan has been submitted concurrently with this final drainage report that details the required maintenance activities and intervals to ensure proper function of all stormwater infrastructures in the future. The full spectrum detention pond will be owned & maintained by Scott Belknap.

Drainage & Bridge Fees

The site lies within the Sand Creek Drainage Basin. It is assumed that all fees were paid at the time platting for Mc Clintock Station Lot A.

SUMMARY

The Mc Clintock Station Lot A known as the Vollmer RV Storage site consists of recycled asphalt parking and drive aisles, a proposed fill spectrum water quality and detention pond, and landscaped areas. The proposed development will not adversely affect downstream drainage



Final Drainage Report

Mc Clintock Station Lot A (Vollmer Road RV Storage)

infrastructure as the site will provide water quality and detention for the developed flows to release below historic rates. Establishment of maintenance procedures and the implementation of temporary and permanent BMP's will insure the site has no adverse drainage impacts on adjacent properties, surrounding developments, or downstream infrastructure. This report is in conformance with the latest El Paso County Stormwater Drainage Criteria requirements for this site.

Provide cost estimate for pond and update FAE.

Calculations need to be included for:

- sizing of spillway riprap
- trickle channel
- forebays
- riprap outlet protection at pond outlet and access culvert
- roadside ditch
- rundowns into pond from DP 1 & DP2

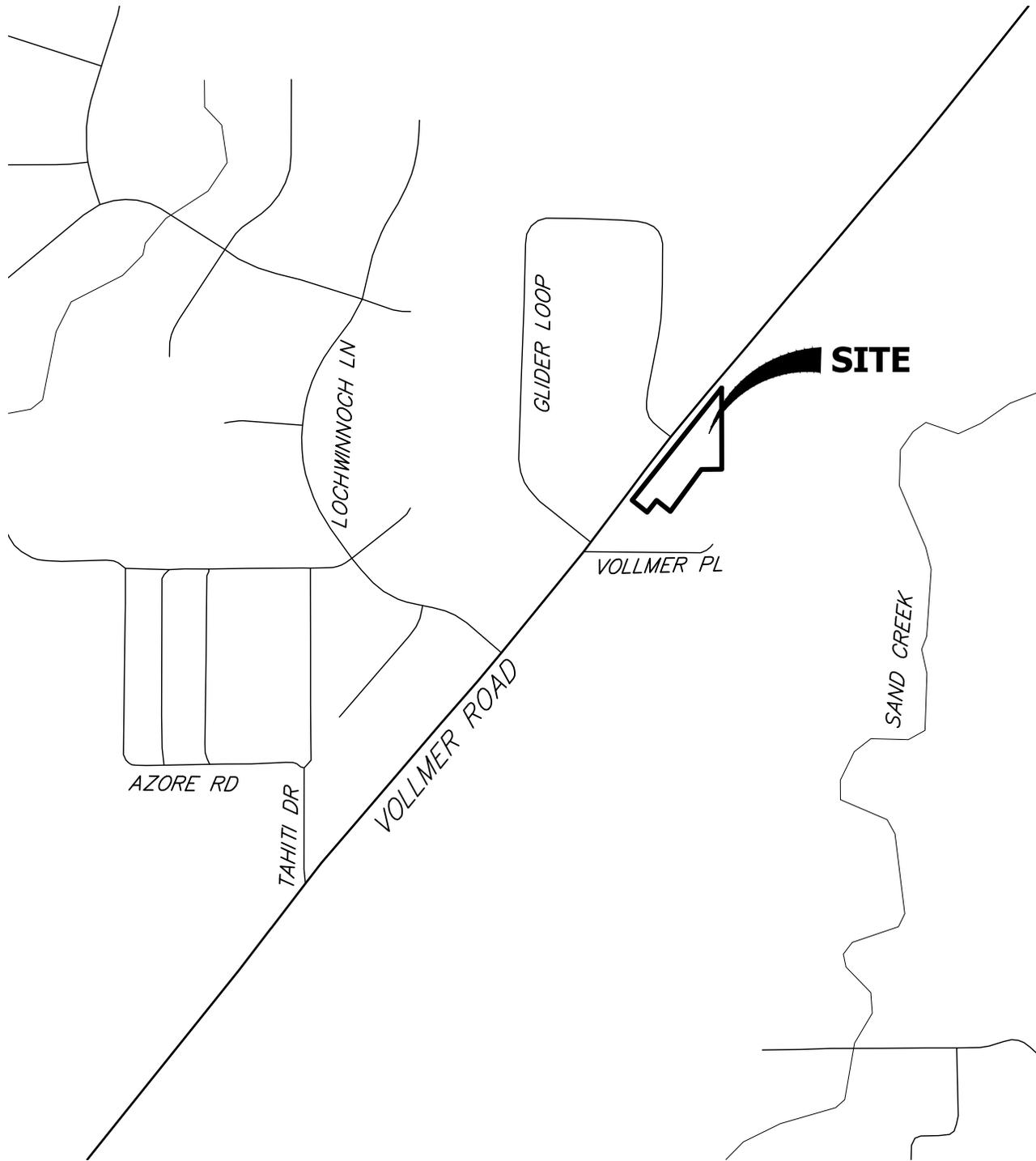
REFERENCES:

1. City of Colorado Springs Drainage Criteria Manual, Volume 1 & 2, Colorado Springs, CO, 2014.
2. El Paso County Drainage Criteria Manual Volume 1, El Paso County, CO, 1990.
3. El Paso County Drainage Criteria Manual Update (City Chapter 6), El Paso County, CO, 2015.
4. El Paso County Engineering Criteria Manual Revision 6, El Paso County, CO, 2016.
5. Final Drainage Report for Barbarick Subdivision, Portions of Lots 1, 2 and Lots 3 & 4, by Matrix Design Group, dated June 2016.
6. Sand Creek Drainage Basin Planning Study, by Stantec, dated January 2021.
7. Urban Storm Drainage Criteria Manual, Urban Drainage and Flood Control District, Latest Revision.

Final Drainage Report
Mc Clintock Station Lot A (Vollmer Road RV Storage)

APPENDIX A

Vicinity Map, Soils, FEMA



ORIGINAL SCALE: 1" = 1000'

VICINITY MAP
 VOLLMER RV STORAGE
 JOB NO. 25251.00
 02/22/2022
 SHEET 1 OF 1



J-R ENGINEERING

A Westrian Company

Centennial 303-740-9393 • Colorado Springs 719-593-2593
 Fort Collins 970-491-9888 • www.jrengineering.com

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Hydrologic Soil Group—El Paso County Area, Colorado



Soil Map may not be valid at this scale.

Map Scale: 1:1,410 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

-  A
-  A/D
-  B
-  B/D
-  C
-  C/D
-  D
-  Not rated or not available

Soil Rating Lines

-  A
-  A/D
-  B
-  B/D
-  C
-  C/D
-  D
-  Not rated or not available

Soil Rating Points

-  A
-  A/D
-  B
-  B/D

-  C
-  C/D
-  D
-  Not rated or not available

Water Features

 Streams and Canals

Transportation

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 19, Aug 31, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 11, 2018—Oct 20, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	4.0	100.0%
Totals for Area of Interest			4.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, NIMS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at <http://www.ngs.noaa.gov/>.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact **FEMA Map Service Center (MSC)** via the FEMA Map Information eXchange (FIRM) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at <http://www.msc.fema.gov/>.

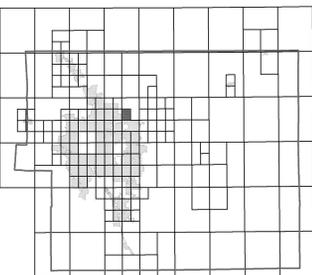
If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP (1-877-336-2627)** or visit the FEMA website at <http://www.fema.gov/business/nfp>.

El Paso County Vertical Datum Offset Table

Flooding Source	Vertical Datum Offset (ft)

REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION

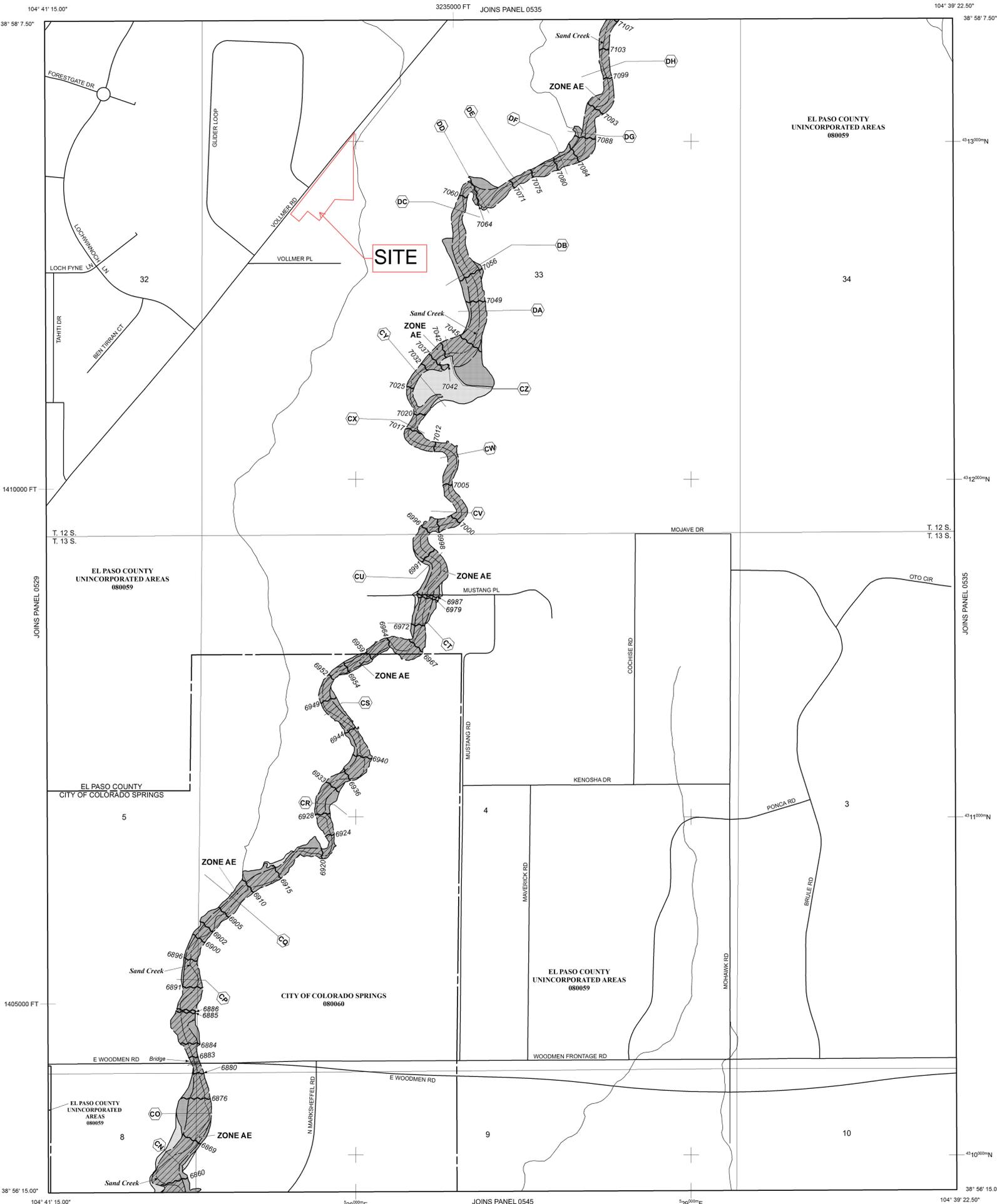
Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 65 WEST, AND TOWNSHIP 13 SOUTH, RANGE 65 WEST.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equalled or exceeded in any given year. The Special Flood Hazard Area is the area subject, to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
- ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
- ZONE AR** Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE
The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS
ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS
ZONE X Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS
OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- Floodplain boundary
- Floodway boundary
- Zone D Boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
- (EL 987) Base Flood Elevation value and value; elevation in feet*
- 513 Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

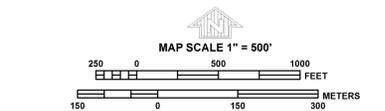
- A-A Cross section line
- 23-23 Transsect line
- 97° 07' 30.00" / 32° 22' 30.00" Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
- 4750000N / 6000000 FT 1000-meter Universal Transverse Mercator grid ticks, zone 13
- 5000-foot grid ticks: Colorado State Plane coordinate system, central zone (EPSG:3023), Lambert Conformal Conic Projection
- DX5510 Bench mark (see explanation in Notes to Users section of this FIRM panel)
- M1.5 River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index
EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.



NFIP **PANEL 0533G**

FIRM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY,
COLORADO
AND INCORPORATED AREAS

PANEL 533 OF 1300
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	08060	0533	G
EL PASO COUNTY	08059	0533	G

Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.

MAP NUMBER
08041C0533G

MAP REVISED
DECEMBER 7, 2018
Federal Emergency Management Agency

APPENDIX B

HYDROLOGIC CALCULATIONS

COMPOSITE % IMPERVIOUS CALCULATIONS - EXISTING CONDITIONS

Subdivision: MC CLINTOCK STATION
 Location: Colorado Springs

Vollmer Road RV Storage
25251.00
APL
REB
6/23/22

Basin ID	Total Area (ac)	Drives/Walks (100% Imp.)				Pasture/Meadow (2% Imp.)				Basins Total Weighted C		Basins Total Weighted % Imp.
		C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	
EX-1	0.48	0.90	0.96	0.00	0.0%	0.08	0.35	0.48	2.0%	0.08	0.35	2.0%
EX-2	2.22	0.90	0.96	0.00	0.0%	0.08	0.35	2.22	2.0%	0.08	0.35	2.0%
EX-3	0.88	0.90	0.96	0.00	0.0%	0.08	0.35	0.88	2.0%	0.08	0.35	2.0%
EX-4	0.56	0.90	0.96	0.00	0.0%	0.08	0.35	0.56	2.0%	0.08	0.35	2.0%
VX-5	0.27	0.90	0.96	0.08	28.1%	0.08	0.35	0.19	1.4%	0.31	0.52	29.5%
VX-6	0.41	0.90	0.96	0.12	30.4%	0.08	0.35	0.29	1.4%	0.33	0.54	31.8%
VX-7	0.14	0.90	0.96	0.05	34.3%	0.08	0.35	0.09	1.3%	0.36	0.56	35.6%
TOTAL	4.96											6.9%

Per COS DCM Table 6-6 C-Values for 2% impervious are 0.09 & 0.36. Please update values

STANDARD FORM SF-2 - EXISTING CONDITIONS TIME OF CONCENTRATION

Subdivision: MC CLINTOCK STATION
Location: El Paso County

Project Name: Vollmer Road RV Storage
Project No.: 25251.00
Calculated By: APL
Checked By: REB
Date: 6/23/22

SUB-BASIN						INITIAL/OVERLAND			TRAVEL TIME					t _c CHECK			FINAL
DATA						(T _i)			(T _t)					(URBANIZED BASINS)			
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C _s	C ₁₀₀	L (ft)	S _o (%)	t _i (min)	L _t (ft)	S _t (%)	K	VEL. (ft/s)	t _t (min)	COMP. t _c (min)	TOTAL LENGTH (ft)	Urbanized t _c (min)	
EX-1	0.48	B	2%	0.08	0.35	136	2.8%	15.4	0	0.0%	7.0	0.0	0.0	15.4	136.0	25.7	15.4
EX-2	2.22	B	2%	0.08	0.35	226	4.2%	17.2	296	2.6%	7.0	1.1	4.4	21.6	522.0	29.0	21.6
EX-3	0.88	B	2%	0.08	0.35	300	3.7%	20.7	78	2.2%	7.0	1.0	1.2	21.9	377.6	26.6	21.9
EX-4	0.56	B	2%	0.08	0.35	122	5.6%	11.5	0	0.0%	7.0	0.0	0.0	11.5	121.7	25.7	11.5
VX-5	0.27	B	30%	0.31	0.52	40	5.7%	5.1	0	0.0%	7.0	0.0	0.0	5.1	39.5	21.0	5.1
VX-6	0.41	B	32%	0.33	0.54	22	9.0%	3.2	455	2.5%	7.0	1.1	6.9	10.0	476.9	24.2	10.0
VX-7	0.14	B	36%	0.36	0.56	40	5.7%	4.7	0	0.0%	7.0	0.0	0.0	4.7	39.5	19.9	5.0

NOTES:

$$t_c = t_i + t_t$$

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

Where:

t_t = channelized flow time (travel time, min)

L_t = waterway length (ft)

S_o = waterway slope (ft/ft)

V_t = travel time velocity (ft/sec) = K√S_o

K = NRCS conveyance factor (see Table 6-2).

Equation 6-2
$$t_i = \frac{0.395(1.1 - C_s)\sqrt{L_i}}{S_o^{0.33}}$$

Where:

t_i = overland (initial) flow time (minutes)

C_s = runoff coefficient for 5-year frequency (from Table 6-4)

L_i = length of overland flow (ft)

S_o = average slope along the overland flow path (ft/ft).

Equation 6-4
$$t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.

L_t = length of channelized flow path (ft)

i = imperviousness (expressed as a decimal)

S_t = slope of the channelized flow path (ft/ft).

Equation 6-3

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

Equation 6-5

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

STANDARD FORM SF-3 - EXISTING CONDITIONS
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: MC CLINTOCK STATION
Location: El Paso County
Design Storm: 5-Year

Project Name: Vollmer Road RV Storage
Project No.: 25251.00
Calculated By: APL
Checked By: REB
Date: 6/23/22

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_r (min)	
	1	EX-1	0.48	0.08	15.4	0.04	3.48	0.1															Runoff from Basin EX-1, overland flows southeast, across the property line to Lot B at DP1.
	1.1								15.4	0.09	3.48	0.3											Runoff from Basins EX-1 and VX-7 combine at DP1.1 and continue onto Lot B
	2	EX-2	2.22	0.08	21.6	0.18	2.98	0.5															Runoff from Basin EX-2, overland flows southeast, across the property line to Lot C at DP2.
	2.1								21.6	0.40	2.98	1.2											Runoff from Basins EX-2, VX-6, and VX-5 combine at DP2.1 and continue onto Lot C
	3	EX-3	0.88	0.08	21.9	0.07	2.95	0.2															Runoff from Basin EX-3, overland flows southeast, across the property line to Lot C at DP3.
	4	EX-4	0.56	0.08	11.5	0.04	3.92	0.2															Runoff from Basin EX-4, overland flows east, across the property line to an existing swale in the Homestead at Stearling Ranch Development
	5	VX-5	0.27	0.31	5.1	0.08	5.15	0.4															Runoff from Basin VX-5, overland flows southeast, across Vollmer Road and into the Site at DP5.
	6	VX-6	0.41	0.33	10.0	0.13	4.12	0.6															Runoff from Basin VX-6, overland flows southeast, across Vollmer Road and into a road side swale, flow from the swale enters the Site at DP6.
	7	VX-7	0.14	0.36	5.0	0.05	5.17	0.3															Runoff from Basin VX-7, overland flows southeast, across Vollmer Road and into the Site at DP7.

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

STANDARD FORM SF-3 - EXISTING CONDITIONS
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: MC CLINTOCK STATION
Location: El Paso County
Design Storm: 100-Year

Project Name: Vollmer Road RV Storage
Project No.: 25251.00
Calculated By: APL
Checked By: REB
Date: 6/23/22

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q _{street} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t _r (min)	
	1	EX-1	0.48	0.35	15.4	0.17	5.85	1.0															Runoff from Basin EX-1, overland flows southeast, across the property line to Lot B at DP1.
	1.1								15.4	0.25	5.85	1.5											Runoff from Basins EX-1 and VX-7 combine at DP1.1 and continue onto Lot B
	2	EX-2	2.22	0.35	21.6	0.78	4.99	3.9															Runoff from Basin EX-2, overland flows southeast, across the property line to Lot C at DP2.
	2.1								21.6	1.14	4.99	5.7											Runoff from Basins EX-2, VX-6, and VX-5 combine at DP2.1 and continue onto Lot C
	3	EX-3	0.88	0.35	21.9	0.31	4.95	1.5															Runoff from Basin EX-3, overland flows southeast, across the property line to Lot C at DP3.
	4	EX-4	0.56	0.35	11.5	0.20	6.58	1.3															Runoff from Basin EX-4, overland flows east, across the property line to an existing swale in the Homestead at Stearling Ranch Development
	5	VX-5	0.27	0.52	5.1	0.14	8.65	1.2															Runoff from Basin VX-5, overland flows southeast, across Vollmer Road and into the Site at DP5.
	6	VX-6	0.41	0.54	10.0	0.22	6.92	1.5															Runoff from Basin VX-6, overland flows southeast, across Vollmer Road and into a road side swale, flow from the swale enters the Site at DP6.
	7	VX-7	0.14	0.56	5.0	0.08	8.68	0.7															Runoff from Basin VX-7, overland flows southeast, across Vollmer Road and into the Site at DP7.

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

COMPOSITE % IMPERVIOUS CALCULATIONS - PROPOSED CONDITIONS

Subdivision: MC CLINTOCK STATION
 Location: El Paso County

Vollmer Road RV Storage
25251.00
APL
REB
6/14/22

Basin ID	Total Area (ac)	Drives/Walks (100% Imp.)				Pasture/Meadow (2% Imp.)				Basins Total Weighted C		Basins Total Weighted % Imp.
		C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	
A	1.62	0.90	0.96	1.11	68.5%	0.08	0.35	0.51	0.6%	0.64	0.77	69.1%
B	1.74	0.90	0.96	1.70	97.7%	0.08	0.35	0.04	0.0%	0.88	0.95	97.7%
C	0.32	0.90	0.96	0.01	2.4%	0.08	0.35	0.31	2.0%	0.10	0.36	4.3%
D	0.10	0.90	0.96	0.00	0.0%	0.08	0.35	0.10	2.0%	0.08	0.35	2.0%
E	0.16	0.90	0.96	0.00	0.0%	0.08	0.35	0.16	2.0%	0.08	0.35	2.0%
F	0.17	0.90	0.96	0.01	8.2%	0.08	0.35	0.16	1.8%	0.15	0.40	10.0%
G	0.84	0.90	0.96	0.25	30.2%	0.08	0.35	0.59	1.4%	0.33	0.53	31.6%
H	0.03	0.90	0.96	0.02	58.0%	0.08	0.35	0.01	0.8%	0.56	0.70	58.8%
TOTAL	4.98											63.1%

Pond Total 3.68

77%

Per COS DCM Table 6-6 C-Values for 2% impervious are 0.09 & 0.36. Please update values

Did not see any area in Basin B that would not be "paved". Please verify this area

STANDARD FORM SF-2 - PROPOSED CONDITIONS TIME OF CONCENTRATION

Subdivision: MC CLINTOCK STATION
Location: El Paso County

Project Name: Vollmer Road RV Storage
Project No.: 25251.00
Calculated By: APL
Checked By: REB
Date: 6/14/22

SUB-BASIN						INITIAL/OVERLAND			TRAVEL TIME					tc CHECK			FINAL
DATA						(Ti)			(Ti)					(URBANIZED BASINS)			
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C ₅	C ₁₀₀	L (ft)	S _o (%)	t _i (min)	L _t (ft)	S _t (%)	K	VEL. (ft/s)	t _t (min)	COMP. t _c (min)	TOTAL LENGTH (ft)	Urbanized t _c (min)	
A	1.62	B	69%	0.64	0.77	45.0	10.0%	2.6	722.2	1.1%	20.0	2.1	5.7	8.3	767.2	20.4	8.3
B	1.74	B	98%	0.88	0.95	78.9	3.0%	2.5	556.9	1.1%	20.0	2.1	4.4	6.8	635.7	13.2	6.8
C	0.32	B	4%	0.10	0.36	27.9	21.0%	3.5	305.6	0.5%	20.0	1.4	3.6	7.1	333.5	32.8	7.1
D	0.10	B	2%	0.08	0.35	39.7	1.0%	11.8	0.0	0.0%	7.0	0.0	0.0	11.8	39.7	25.7	11.8
E	0.16	B	2%	0.08	0.35	22.5	5.5%	5.0	0.0	0.0%	7.0	0.0	0.0	5.0	22.5	25.7	5.0
F	0.17	B	10%	0.15	0.40	24.6	19.1%	3.2	0.0	0.0%	7.0	0.0	0.0	3.2	24.6	24.3	5.0
G	0.84	B	32%	0.33	0.53	22.0	9.4%	3.1	871.1	2.1%	7.0	1.0	14.2	17.4	893.1	28.1	17.4
H	0.03	B	59%	0.56	0.70	19.5	15.6%	1.8	14.5	0.6%	7.0	0.5	0.4	2.2	34.0	16.2	5.0

NOTES:

$$t_c = t_i + t_t$$

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

Where:

t_t = channelized flow time (travel time, min)

L_t = waterway length (ft)

S_o = waterway slope (ft/ft)

V_t = travel time velocity (ft/sec) = K√S_o

K = NRCS conveyance factor (see Table 6-2).

Equation 6-2

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_o^{0.33}}$$

Where:

t_i = overland (initial) flow time (minutes)

C₅ = runoff coefficient for 5-year frequency (from Table 6-4)

L_i = length of overland flow (ft)

S_o = average slope along the overland flow path (ft/ft).

Equation 6-4

$$t_t = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$

Where:

t_t = minimum time of concentration for first design point when less than t_c from Equation 6-1.

L_t = length of channelized flow path (ft)

i = imperviousness (expressed as a decimal)

S_t = slope of the channelized flow path (ft/ft).

This length seems long, since basin is just the pond. Please verify

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

STANDARD FORM SF-3 - PROPOSED CONDITIONS
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: MC CLINTOCK STATION
Location: El Paso County
Design Storm: 5-Year

Project Name: Vollmer Road RV Storage
Project No.: 25251.00
Calculated By: APL
Checked By: REB
Date: 6/23/22

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	A	1.62	0.64	8.3	1.04	4.41	4.6															Runoff from Basin A, overland flows southeast & becomes channelized and flows south west to DP1 where flow enters the proposed pond.
	2	B	1.74	0.88	6.8	1.53	4.70	7.2															Runoff from Basin B, overland flows & becomes channelized, runoff flows south west to DP2 where flow enters the proposed pond.
	3	C	0.32	0.10	7.1	0.03	4.64	0.1															Runoff from Basin C, flows down the sides of the pond and becomes channelized in the trickle channel that flows to DP3.
	3.1								8.3	2.60	4.41	11.5											Flows from Basins A,B, and C combine at DP3.1 in the proposed EDB pond.
	4	D	0.10	0.08	11.8	0.01	3.88	0.0															Runoff from Basin D, overland flows east, across the property line to an existng swale in the Homestead at Stearling Ranch Development
	5	E	0.16	0.08	5.0	0.01	5.17	0.1															Runoff from Basin E, overland flows southeast, across the property line and into the neighboring Parcel, Mc Clintock Station Lot C.
	6	F	0.17	0.15	5.0	0.02	5.17	0.1															Runoff from Basin F, overland flows southeast, across the property line and into the neighboring Parcel, Mc Clintock Station Lot B.
	7	G	0.84	0.33	17.4	0.28	3.30	0.9															Runoff from Basin G, overland flows southeast, across Vollmer Road & into a road side swale, flow enters the proposed 18" culvert at DP7.
	8	H	0.03	0.56	5.0	0.02	5.17	0.1															Runoff from Basin H, overland flows across Vollmer Road & proposed access & into a road side swale to DP 8.
	8.1								17.4	0.29	3.30	1.0											Flows from Basins G & H combine at DP8.1 and contiune to flow southwest along Vollmer Road.

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

STANDARD FORM SF-3 - PROPOSED CONDITIONS
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: MC CLINTOCK STATION
Location: El Paso County
Design Storm: 100-Year

Project Name: Vollmer Road RV Storage
Project No.: 25251.00
Calculated By: APL
Checked By: REB
Date: 6/23/22

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q_{street} (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_r (min)	
	1	A	1.62	0.77	8.3	1.24	7.40	9.2															Runoff from Basin A, overland flows southeast & becomes channelized and flows south west to DP1 where flow enters the proposed pond.
	2	B	1.74	0.95	6.8	1.65	7.90	13.0															Runoff from Basin B, overland flows & becomes channelized, runoff flows south west to DP2 where flow enters the proposed pond.
	3	C	0.32	0.36	7.1	0.12	7.80	0.9															Runoff from Basin C, flows down the sides of the pond and becomes channelized in the trickle channel that flows to DP3.
	3.1								8.3	3.01	7.40	22.3											Flows from Basins A,B, and C combine at DP3.1 in the proposed EDB pond.
	4	D	0.10	0.35	11.8	0.04	6.51	0.3															Runoff from Basin D, overland flows east, across the property line to an existing swale in the Homestead at Stearling Ranch Development
	5	E	0.16	0.35	5.0	0.06	8.68	0.5															Runoff from Basin E, overland flows southeast, across the property line and into the neighboring Parcel, Mc Clintock Station Lot C.
	6	F	0.17	0.40	5.0	0.07	8.68	0.6															Runoff from Basin F, overland flows southeast, across the property line and into the neighboring Parcel, Mc Clintock Station Lot B.
	7	G	0.84	0.53	17.4	0.45	5.54	2.5															Runoff from Basin G, overland flows southeast, across Vollmer Road & into a road side swale, flow enters the proposed 18" culvert at DP7.
	8	H	0.03	0.70	5.0	0.02	8.68	0.2															Runoff from Basin H, overland flows across Vollmer Road & proposed access & into a road side swale to DP 8.
	8.1								17.4	0.47	5.54	2.6											Flows from Basins G & H combine at DP8.1 and continue to flow southwest along Vollmer Road.

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

APPENDIX C

HYDRAULIC CALCULATIONS

Culvert Report

Drive Access Culvert

Invert Elev Dn (ft)	=	7067.17
Pipe Length (ft)	=	44.48
Slope (%)	=	0.50
Invert Elev Up (ft)	=	7067.39
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.012
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Groove end projecting (C)
Coeff. K,M,c,Y,k	=	0.0045, 2, 0.0317, 0.69, 0.2

Embankment

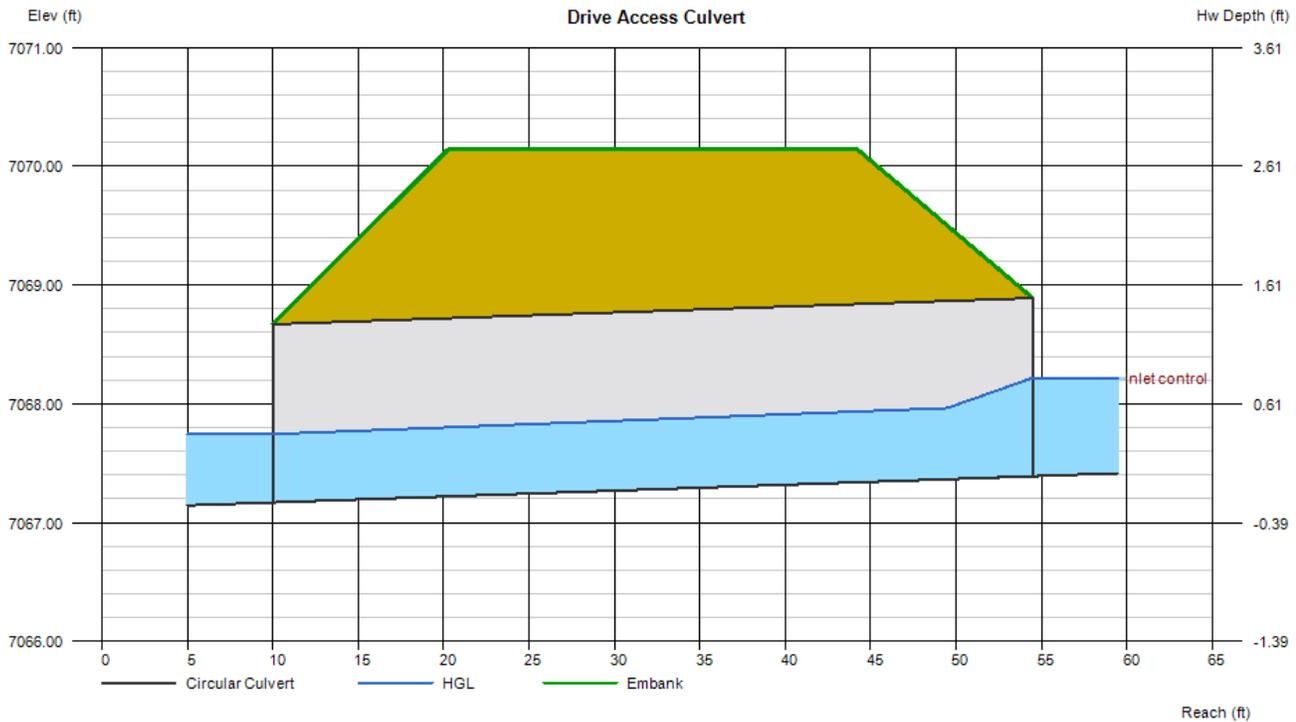
Top Elevation (ft)	=	7070.14
Top Width (ft)	=	24.00
Crest Width (ft)	=	15.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	2.50
Tailwater Elev (ft)	=	0.00

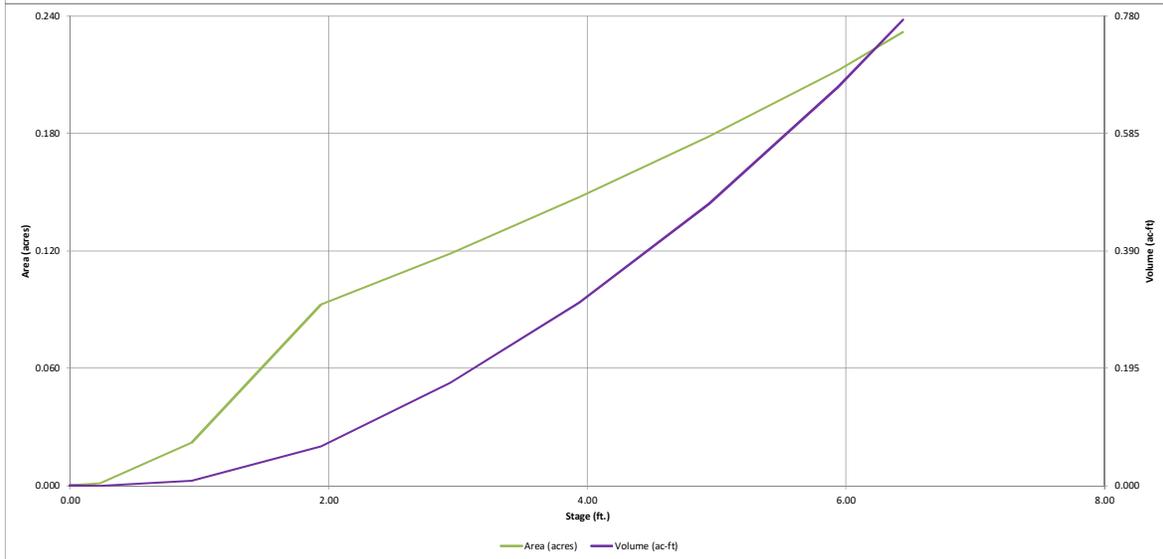
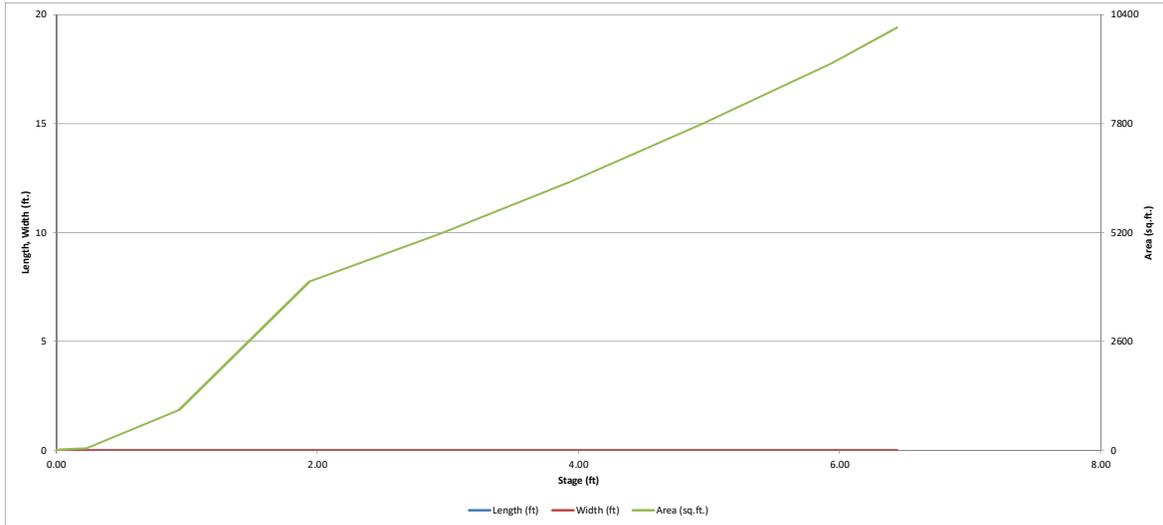
Highlighted

Qtotal (cfs)	=	2.50
Qpipe (cfs)	=	2.50
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	4.01
Veloc Up (ft/s)	=	3.77
HGL Dn (ft)	=	7067.75
HGL Up (ft)	=	7067.99
Hw Elev (ft)	=	7068.22
Hw/D (ft)	=	0.55
Flow Regime	=	Inlet Control



DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.05 (January 2022)

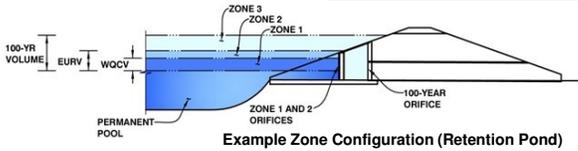


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.05 (January 2022)

Project: Vollmer RVStorage

Basin ID: Pond



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.25	0.095	Orifice Plate
Zone 2 (EURV)	4.01	0.218	Orifice Plate
Zone 3 (100-year)	5.02	0.168	Weir&Pipe (Restrict)
Total (all zones)		0.481	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
 Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = ft²
 Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Calculated Parameters for Plate

Centroid of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)
 Depth at top of Zone using Orifice Plate = 4.01 ft (relative to basin bottom at Stage = 0 ft)
 Orifice Plate: Orifice Vertical Spacing = N/A inches
 Orifice Plate: Orifice Area per Row = N/A sq. inches

WQ Orifice Area per Row = N/A ft²
 Elliptical Half-Width = N/A feet
 Elliptical Slot Centroid = N/A feet
 Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.34	2.67	3.00				
Orifice Area (sq. inches)	0.53	0.53	0.53	2.00				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft ²
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.01	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	2.00	N/A	feet
Overflow Weir Gate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	2.00	N/A	feet
Overflow Gate Type =	Type C Gate	N/A	
Debris Clogging % =	50%	N/A	%

	Zone 3 Weir	Not Selected	
Height of Gate Upper Edge, H ₁ =	4.01	N/A	feet
Overflow Weir Slope Length =	2.00	N/A	feet
Grate Open Area / 100-yr Orifice Area =	6.95	N/A	
Overflow Gate Open Area w/o Debris =	2.78	N/A	ft ²
Overflow Gate Open Area w/ Debris =	1.39	N/A	ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	5.00		inches

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	0.40	N/A	ft ²
Outlet Orifice Centroid =	0.25	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	1.11	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Calculated Parameters for Spillway

Spillway Invert Stage =	4.94	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	25.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Spillway Design Flow Depth =	0.28	feet
Stage at Top of Freeboard =	6.22	feet
Basin Area at Top of Freeboard =	0.22	acres
Basin Volume at Top of Freeboard =	0.72	acre-ft

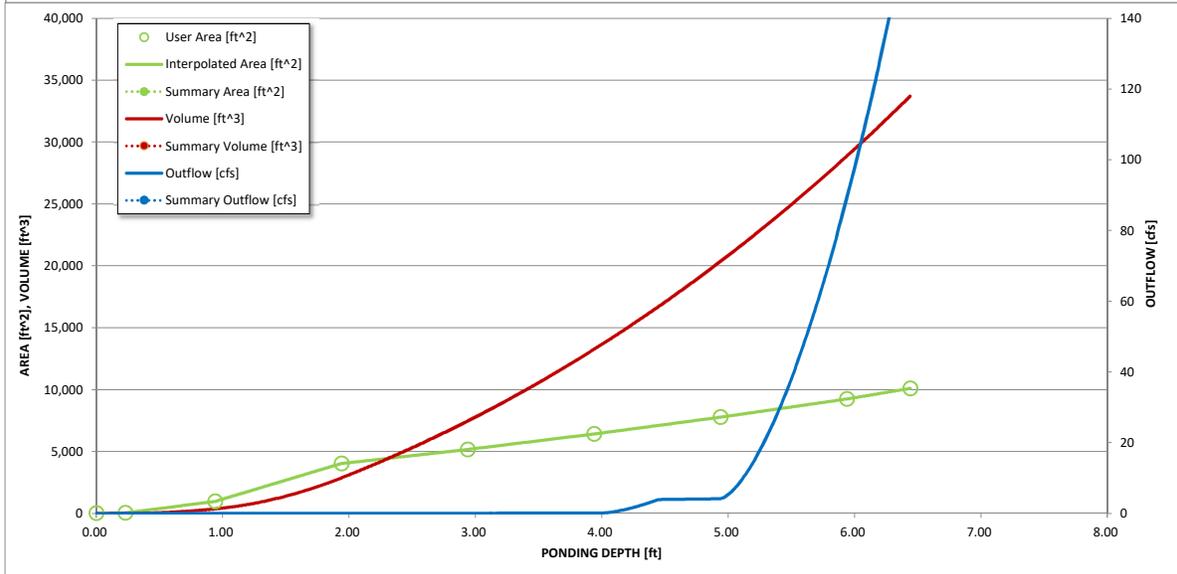
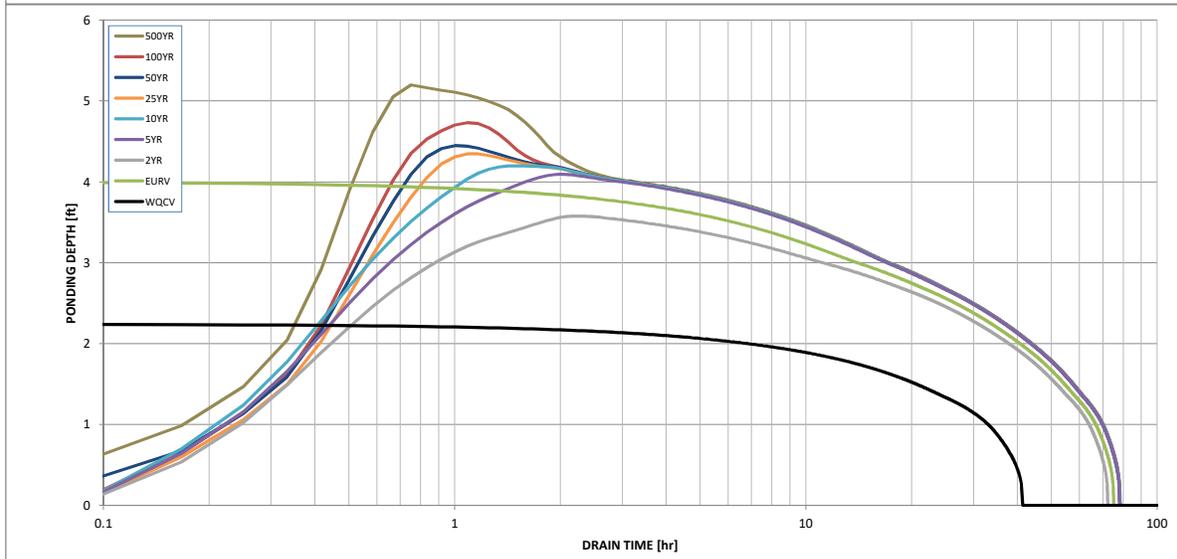
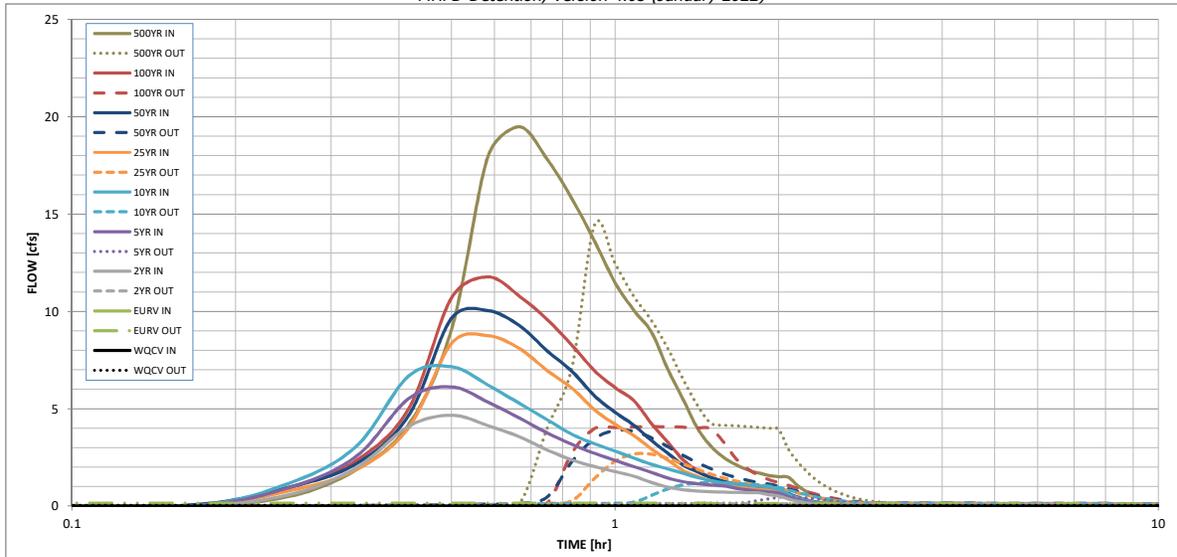
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	4.00
One-Hour Rainfall Depth (in) =	0.095	0.314	0.270	0.359	0.433	0.517	0.593	0.682	1.146
CUHP Runoff Volume (acre-ft) =	N/A	N/A	0.270	0.359	0.433	0.517	0.593	0.682	1.146
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.4	1.0	1.6	2.8	3.5	4.5	8.8
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A							
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.10	0.28	0.43	0.76	0.95	1.22	2.39
Peak Inflow Q (cfs) =	N/A	N/A	4.7	6.1	7.2	8.8	10.0	11.8	19.5
Peak Outflow Q (cfs) =	0.0	0.2	0.1	0.5	1.2	2.7	3.9	4.1	14.5
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.5	0.8	1.0	1.1	0.9	1.6
Structure Controlling Flow =	Plate	Overflow Weir 1	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1	Spillway
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	0.1	0.4	0.9	1.4	1.4	1.5
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	67	65	69	68	66	65	63	57
Time to Drain 99% of Inflow Volume (hours) =	40	72	70	75	74	73	73	72	69
Maximum Ponding Depth (ft) =	2.25	4.01	3.58	4.10	4.20	4.35	4.45	4.73	5.20
Area at Maximum Ponding Depth (acres) =	0.10	0.15	0.14	0.15	0.16	0.16	0.16	0.17	0.19
Maximum Volume Stored (acre-ft) =	0.096	0.315	0.252	0.327	0.342	0.366	0.382	0.431	0.513

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.05 (January 2022)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EUR [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.06	0.01	0.49
	0:15:00	0.00	0.00	0.58	0.94	1.16	0.78	0.96	0.95	1.86
	0:20:00	0.00	0.00	1.97	2.55	3.06	1.87	2.17	2.34	4.19
	0:25:00	0.00	0.00	4.11	5.53	6.69	4.04	4.67	5.02	9.12
	0:30:00	0.00	0.00	4.67	6.13	7.16	8.37	9.66	10.72	17.99
	0:35:00	0.00	0.00	4.13	5.34	6.22	8.76	10.05	11.78	19.49
	0:40:00	0.00	0.00	3.56	4.52	5.27	8.10	9.27	10.79	17.81
	0:45:00	0.00	0.00	2.88	3.75	4.44	6.96	7.97	9.58	15.78
	0:50:00	0.00	0.00	2.37	3.17	3.68	6.05	6.92	8.26	13.59
	0:55:00	0.00	0.00	2.03	2.71	3.20	4.94	5.66	6.95	11.47
	1:00:00	0.00	0.00	1.77	2.35	2.82	4.20	4.82	6.09	10.05
	1:05:00	0.00	0.00	1.53	2.02	2.46	3.62	4.15	5.41	8.93
	1:10:00	0.00	0.00	1.22	1.73	2.15	2.94	3.37	4.23	7.03
	1:15:00	0.00	0.00	0.98	1.44	1.91	2.37	2.72	3.28	5.49
	1:20:00	0.00	0.00	0.85	1.26	1.69	1.84	2.11	2.38	4.01
	1:25:00	0.00	0.00	0.78	1.15	1.47	1.54	1.76	1.83	3.08
	1:30:00	0.00	0.00	0.74	1.09	1.32	1.28	1.47	1.48	2.51
	1:35:00	0.00	0.00	0.72	1.04	1.21	1.12	1.28	1.26	2.13
	1:40:00	0.00	0.00	0.71	0.93	1.13	1.00	1.14	1.11	1.88
	1:45:00	0.00	0.00	0.70	0.84	1.08	0.93	1.06	1.00	1.70
	1:50:00	0.00	0.00	0.69	0.78	1.04	0.88	1.00	0.93	1.58
	1:55:00	0.00	0.00	0.59	0.73	0.97	0.84	0.96	0.89	1.51
	2:00:00	0.00	0.00	0.52	0.67	0.87	0.83	0.94	0.88	1.48
	2:05:00	0.00	0.00	0.37	0.48	0.62	0.59	0.66	0.62	1.06
	2:10:00	0.00	0.00	0.26	0.34	0.43	0.41	0.47	0.44	0.75
	2:15:00	0.00	0.00	0.18	0.23	0.30	0.29	0.33	0.31	0.52
	2:20:00	0.00	0.00	0.12	0.15	0.20	0.19	0.22	0.21	0.35
	2:25:00	0.00	0.00	0.08	0.10	0.13	0.13	0.15	0.14	0.23
	2:30:00	0.00	0.00	0.05	0.07	0.09	0.09	0.10	0.09	0.15
	2:35:00	0.00	0.00	0.03	0.04	0.05	0.05	0.06	0.05	0.09
	2:40:00	0.00	0.00	0.01	0.02	0.02	0.02	0.03	0.03	0.04
	2:45:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

APPENDIX D

REFERENCE MATERIALS



SAND CREEK DRAINAGE BASIN PLANNING STUDY FINAL REPORT JANUARY 2021

Prepared for:

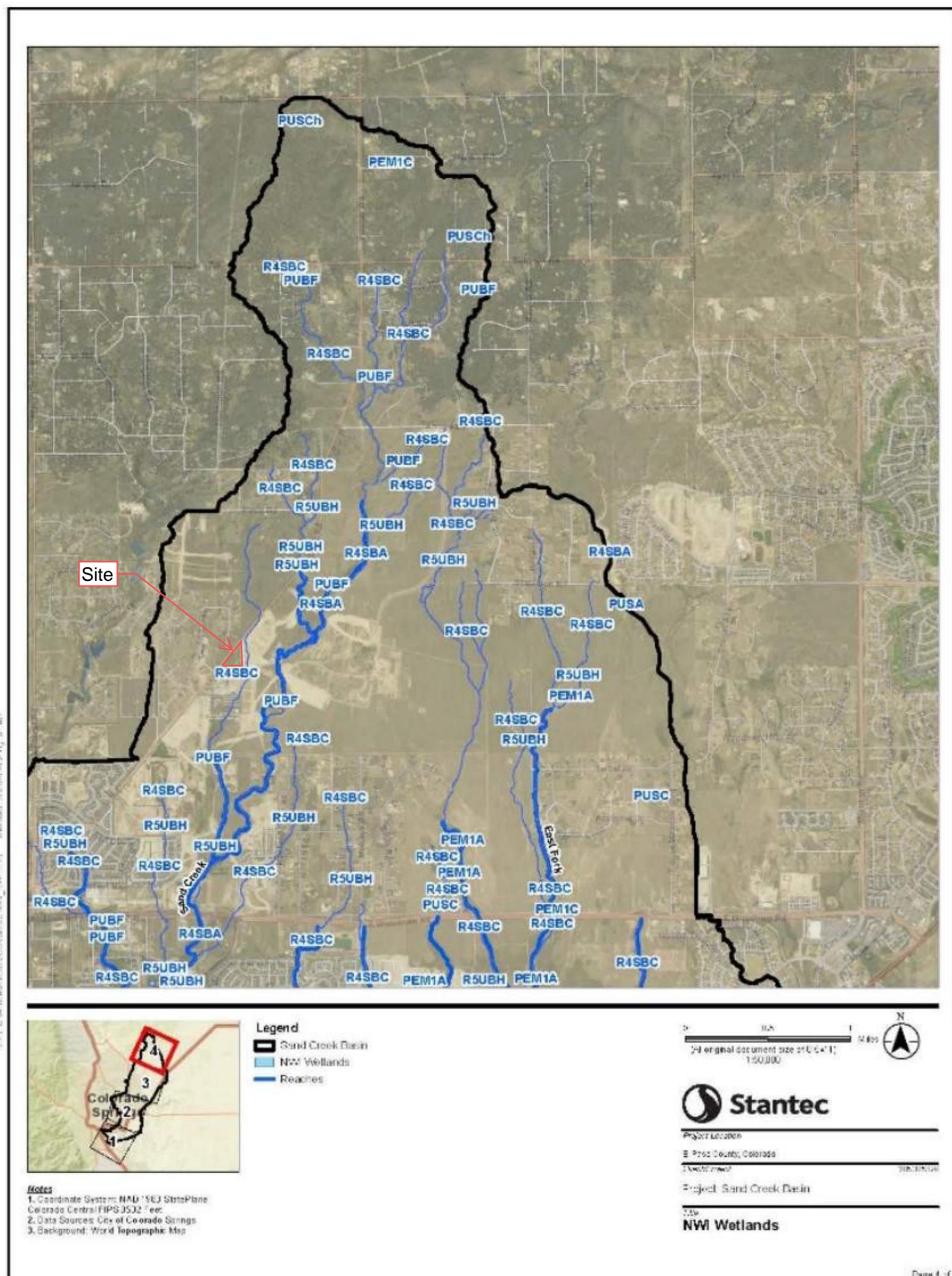


Prepared by:



SAND CREEK – SAND CREEK DRAINAGE BASIN PLANNING STUDY

Basin Characteristics and Environmental Resources



Disclaimer: This document has been prepared based on information provided by others as cited in the Notes section. Stantec has not verified the accuracy and/or completeness of this information and shall not be responsible for any errors or omissions which may be incorporated herein as a result. Stantec assumes no responsibility for data supplied in electronic format, and the recipient accepts full responsibility for verifying the accuracy and completeness of the data.

Figure 2-7: NWI Wetlands Located in Sand Creek Drainage Basin (Page 4)

SAND CREEK – SAND CREEK DRAINAGE BASIN PLANNING STUDY

Hydrology

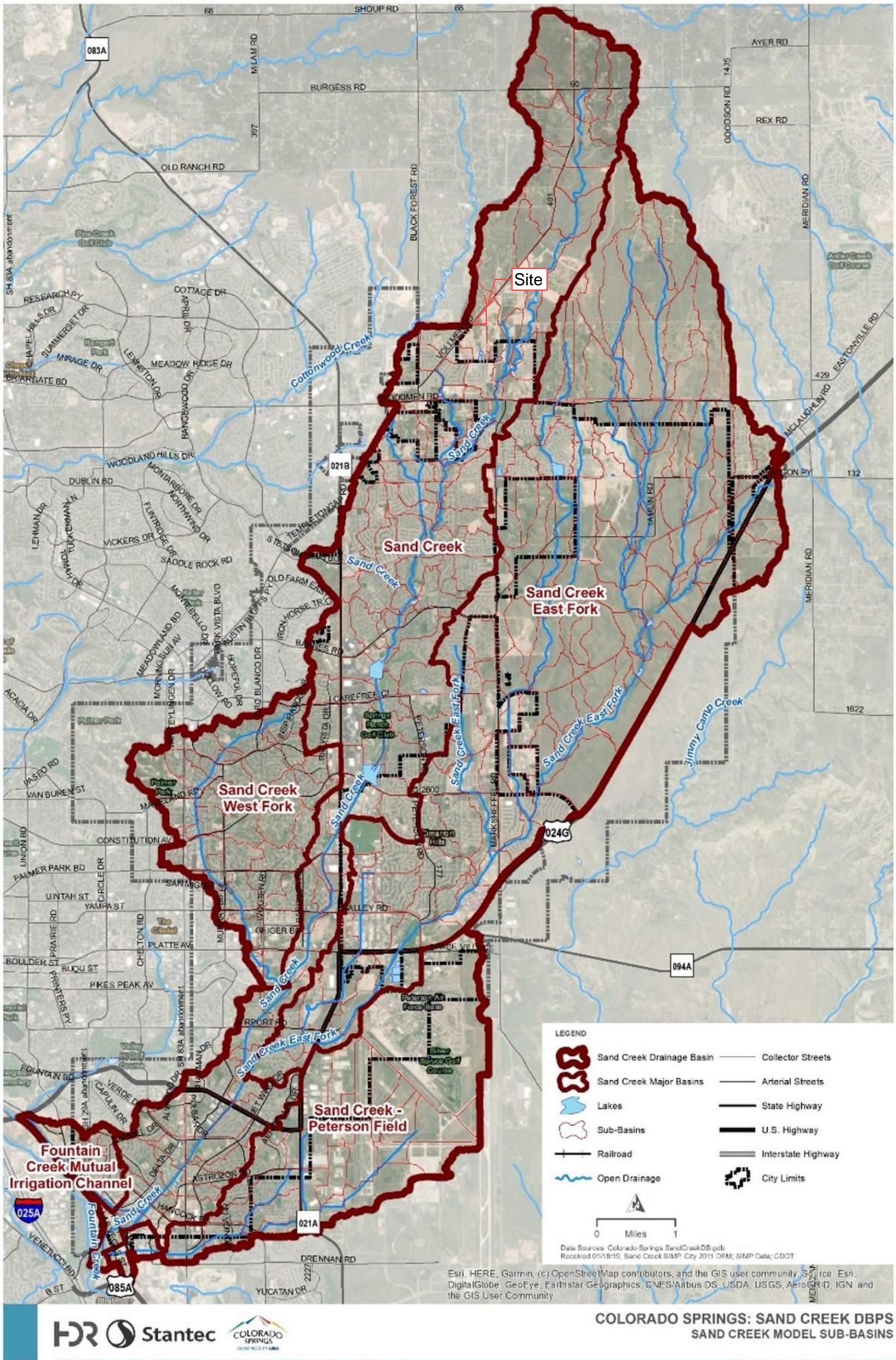


Figure 3-1. Major Sub-basin Map

SAND CREEK – SAND CREEK DRAINAGE BASIN PLANNING STUDY

Hydrology

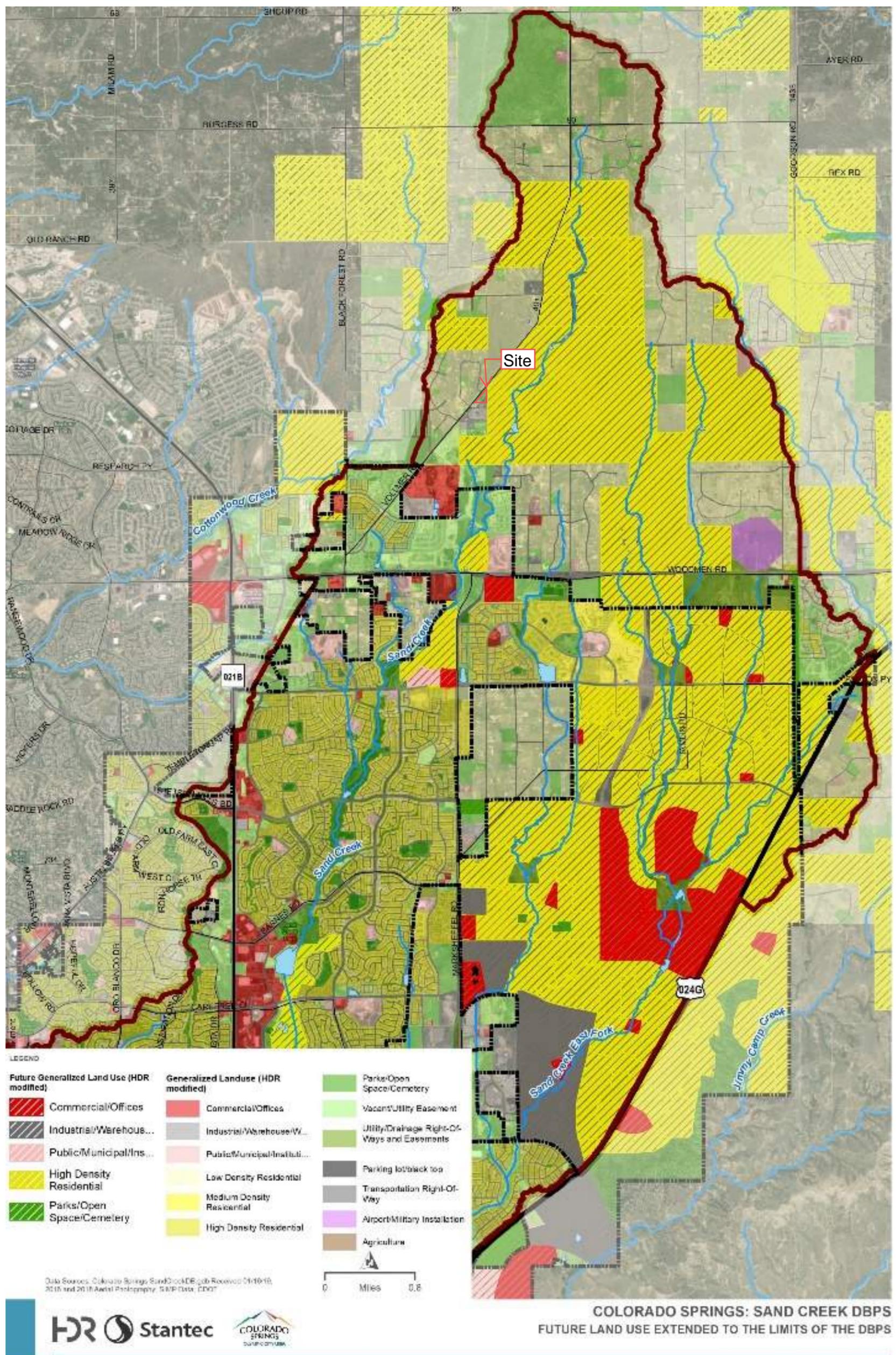
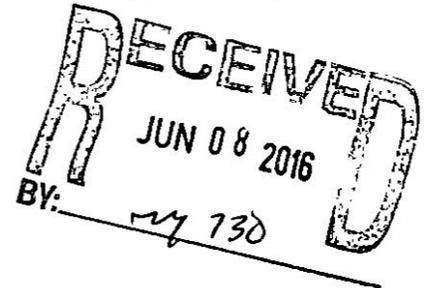


Figure 3-15. Future Land Use MapFuture Condition Model Results



FINAL DRAINAGE REPORT
For
BARBARICK SUBDIVISION,
PORTIONS OF LOTS 1, 2 and LOTS 3 & 4
El Paso County, Colorado
Sand Creek Drainage Basin

Prepared for:
El Paso County Development Services
Engineering Division



On Behalf of:
Wykota Construction
430 Beacon Light Road, Suite 130
Monument, CO 80132

Prepared by:



2435 Research Parkway, Suite 300
Colorado Springs, CO 80920
(719) 575-0100
Fax (719) 572-0208

June 6, 2016

15.789.001

plus the time of travel (t_t) in concentrated form, such as a swale or drainageway. A minimum T_c of 5 minutes and 10 minutes were used for the final calculations in developed and undeveloped conditions, respectively.

Storm Drain Systems

All proposed storm drain infrastructure will be located within private property and will be owned and maintained by the property owner.

The storm drain hydraulics is analyzed using *Bentley's FlowMaster*, *CulvertMaster* & *StormCAD* design software. Colorado Department of Transportation (CDOT) type inlets will be used where necessary.

The designated outfall locations for the proposed on-site storm drains are the natural drainage ways at the south end of the property. The proposed storm drain infrastructure will be discussed in more detail below.

EXISTING DRAINAGE REPORT DISCUSSION

The approved Barbarick Subdivision Final Drainage Report (BS-FDR) and the approved Woodmen Storage Final Drainage Report (WS-FDR) both apply to the existing general drainage conditions for this site. The off-site basins and general flow patterns in the BS-FDR and WS-FDR still apply. Excerpts from these reports are provided below for reference.

On-site and Off-Site Basin Descriptions from the BS-FDR and WS-FDR:

The following summary is taken from the Barbarick Subdivision Final Drainage Report (BS-FDR):

Off-site:

Off-site Basin O3 This basin encompasses approximately 7.03 acres and represents the area north and northwest of Lot 1. This basin drains into Lot 1 through a series of (2) 24" CMP pipes which control the flow of 14/36 cfs in the 5/100 year storm events.

Lots 1 & 2 – these lots are considered fully developed lots and drain north to south collecting at the existing concrete settling pond on Lot 2. This developed flow (20.8 cfs /57.2 cfs) combines with Off-site Basin O3 to total 30.5 cfs / 80.8 cfs in the greenbelt offsite south of Lot 2. At the time of development permit for these developed lots, a detention pond for water quality will be required, probably in the area of the existing concrete settling pond, that will accommodate Lots 1 and 2 west of the gas easement and flood plain area.

On-site:**On-site Basins A1 and B1 (for portions of Lots 1 and 2, and Lots 3 & 4)**

These basins encompass approximately 5.3 & 3.8 acres and represent the buildable portions of the property as described in the BS-FDR (see Basin Map from BS-FDR below). These basins were slated (in the BS-FDR) to drain into small detention ponds that would release to historic rates. These discharge rates were calculated to be 2.9/7.3 and 2.2/5.4 cfs (5/100 year). The BS-FDR does not include the drainage ways in any hydrology calculations due to the fact that this no-build drainage area was not planned on being developed. This drainage way allowed off-site flows from O1+O2 to pass-through Lots 3 & 4. The drainage way to the west of A1 passes through flows from offsite O3. Since the approval of this report, offsite tributary basins O1+O2 have been changed, and the development of the property encompasses the whole property, including the previously determined no-build area.

The following summary is taken from the Woodmen Storage Final Drainage Report (WS-FDR):

Off-site:

Design Point 5 - This design point encompasses approximately 19.69 acres and represents the tributary area north of the project site. This basin drains into a proposed detention pond near the northeast corner of the property and generates 57.4/92.7 cfs in the 10/100 year storm events, historic flows are 16.7/30.3 cfs. The releases rates from this pond are lower than historic 16.1 cfs/29.4 cfs in the 10/100-year storm events. These flows are conveyed along the east property line of the site and into the eastern natural drainage way that leaves the property to the south.

Review of the Sterling Ranch Preliminary Drainage Report (SR-PDR):

The Barbarick Subdivision is surrounded on three sides by the planned Sterling Ranch Development. The approved Sterling Ranch PDR was prepared by M&S Civil Consultants in May of 2015. This Sterling Ranch PDR re-analyzes runoff from Barbarick Subdivision and plans for storm drain improvements to convey this runoff to a full spectrum detention and water quality pond to be located down stream of Barbarick Subdivision as part of Sterling Ranch Phase One.

In summary; the Sterling Ranch PDR is planning on receiving 73.3/139.2 cfs (5/100 year) from Basin OS3. A 54" RCP is planned to convey this flow through Sterling Ranch. The Sterling Ranch PDR is planning on receiving 45/86 cfs (5/100 year) from OS2, encompasses Lots 1 & 2 and OS3 encompasses Lots 3 & 4 and the Basin north of Lot 3. A 48" RCP is planned to convey this flow through Sterling Ranch. The cumulative runoff from the northerly property and Lots 1 through 4 does not exceed the anticipated rates in the SR-PDR.

condition rangeland and generates 0.3/2.7 cfs in the 5/100 year storm events. This basin sheet flows offsite where it is captured in a small swale between the site and existing roadway and conveyed westerly to the low point south of the outfall of Basin H1.

These existing basins encompass the previously unmodelled drainage area from the BS-FDR. The total historic flow from the site is 3.8/34.6 cfs in the 5/100 year storm events. The following design point table is for combined allowable discharge rates from the property at respective locations including historic flows from the tributary upstream basins:

<u>Design Point</u>	<u>5/100 Release</u>	<u>Comments</u>
DP H1	16.7*/30.3 cfs	DP H5 WS-FDR - * is 10year
DP H2	13.7/35.5 cfs	O3 BS-FDR
DP H3	56.7 cfs	DPH1+H1+H3 (100-year)
DP H4	14.6/43.7 cfs	DPH2 + H2

Design Point H3 will release a flow lower than previously anticipated within the BS-FDR (52.9/170 cfs). It is the introduction of development within the Sterling Ranch site that has eliminated offsite flows from BS-FDR Basin O1 that significantly changed the drainage pattern. The historic release is now contained solely to the historic flows from WS-FDR design point H5 and the proposed onsite historic flows.

Design Point H4 will combine with the western half of Lots 1&2. Per the BS-FDR the combined portions of Lots 1&2 and O3 to release a combined flow of 30.5/80.8 cfs downstream. The flow anticipated in the BS-FDR appears consistent with the smaller basin analysis of this report and should be used for downstream analysis.

PROPOSED DRAINAGE DISCUSSION

Introduction

The proposed site will be developed differently than anticipated in the previous BS-FDR. The previous plan for this site maintained the existing native drainage way down the middle of Lots 1 & 2 and 3 & 4, thereby splitting the buildable area into the outer thirds of these lots. The native drainage way and "Drainage Boundary – No Build Area" (as shown on the Plat & FDR) will be eliminated with the proposed development. The proposed site and proposed drainage improvements will allow this native drainage way to be eliminated while maintaining the pass through of major flows. These modifications to the site and to the drainage patterns will allow a larger buildable area.

The existing retention pond, located just north of Lot 3, will be modified by others to become a water quality/detention pond pursuant to the WS-FDR. A new outlet works and a storm drain pipe will convey runoff from this detention pond (16.1/29.4 cfs in the 10/100 year storm events) discharging at the property line. This development is proposing a CDOT Type D inlet to capture the discharged flow and pipe it downstream along the east side of Lots 3 & 4 to discharge into the proposed Full Spectrum Extended

Detention Basin (EDB) in Lot 4. The EDB is designed to pass through, and not treat or detain, these offsite flows.

A new EDB will be provided in Lot 4. This detention basin will provide water quality treatment for portions of Lots 1 & 2, and Lots 3 & 4. In the approved Barbarick FDR there were to be two separate ponds. The new site development has been planned for a single pond to treat the developed flows. Tributary water sheet flow across the site to shallow swales that will direct runoff to the proposed EDB. The EDB will have a forebay at the confluence of the two pipe outfalls, a concrete trickle channel that terminates at a micropool structure, and is designed to treat the WQCV, EURV and 100-year detention.

A second SFB water quality with detention catchment basin will be provided at the south east/downstream end of Lot 2. This SFB will not have an outlet structure to release flows due to requirements from the gas main utility ownership of no structure to be built within the existing easements. There will be a small spillway to allow the release of large storm events. Runoff will be directed to the proposed SFB where possible.

Flow from the area north of Lot 1 (Basin O3) will pass through the site via two 24" culverts and will be discharged at the southern boundary of Lot 2, as historically done. An earthen channel will run north-south along the east side of the existing Lot 1 and Lot 2 developments. The channel is approximately 1-ft deep with 4:1 side slopes and will capture and convey any westerly flowing nuisance runoff from the proposed improvements to the sand filter detention pond as discussed in the original Barbarick Subdivision FDR, instead of the existing Lot 1 and 2 improved areas.

Runoff from the property is at historic flows and will not exceed the anticipated runoff as determined in the Sterling Ranch PDR. This is described in more detail below. The Sterling Ranch PDR includes an analysis of future drainage conditions and includes recommended infrastructure to convey this runoff. Since the Sterling Ranch surrounds the Barbarick Subdivision, it is appropriate to include the recommendations from the SR-PDR in this Proposed Drainage Discussion.

Proposed On-Site Basin Descriptions: (See Basin Map in the pocket)

On-site Basin D1 (D for Developed condition) - This developed basin encompasses approximately 11.4 acres - the majority of Lots 3 & 4 and small portions of Lots 1 & 2. This basin generates 19.7/56.0 cfs in the 5/100 year storm events and sheet flows into shallow swales that direct the runoff into the proposed EDB to be located in Lot 4. Lot 3 is based on Owner provided information for a gravel parking/vehicle storage area, and Lot 4 has been based on proposed building site improvements as identified in the rezoning application. Any changes to the land use will require an update to the Final Drainage Report; much like the original Barbarick Subdivision Final Drainage Report is being updated with the grading and Lot 4 development application.

On-site Basin D2 This undeveloped basin encompasses 1.2 acres and represents the south portion of Lot 4, below and south of the two detention ponds. This basin is historic in nature and generates 0.8/3.0 cfs and drains directly into a road side ditch within the Sterling Ranch development.

On-site Basin D3 This developed basin encompasses approximately 3.13 acres - the remaining proposed infill portions of Lots 1 and 2 (east of the currently built out Lots 1&2). As discussed in the original Barbarick Subdivision FDR, development of these areas will require a detention water quality pond. This basin generates 4.1/11.6 cfs in the 5/100 year storm events and sheet flows southerly to the proposed SFB located at the southern-most portion of Lot 2.

The following design point table is for combined allowable discharge rates from the property at respective locations including historic flows from the tributary upstream basins:

<u>Design Point</u>	<u>5/100 Year</u>	<u>Comments</u>
DP D1	85.4 cfs (100)	D1+O2 Pass Through
DP D2	48.9 cfs (100)	Pond Release+D2
DP D3	4.1/11.6 cfs	D3
DP D4	13.8/39.1 cfs	Pond Release +O3 Pass Through

All release flows downstream are at or below historic levels.

RECOMMENDED DESIGN

Off-site Detention Facility:

This shallow pond will be modified for the proposed development to the north as part of the WS-FDR. This will eliminate the retention properties in this pond, will provide detention for off-site flows, will provide a suitable outlet structure, and will remove accumulated sediment. The modified pond will store up to 1.52 acft (66,211 cuft) to the principal spillway (elevation = 7048.05). A summary of flows into and out of this pond:

<u>Off-site Pond Flow Summary (cfs)</u>	<u>5 year</u>	<u>100 year</u>
Proposed Flow into offsite pond (Basin G/DP 5)	<u>57.4</u>	<u>92.7</u>
Increase in peak flow due to development	46.2	51.3
Proposed flow out of modified pond	<u>16.1</u>	<u>29.4</u>
Reduction in peak flow	41.3	63.3

For complete pond design, refer to the WS-FDR.

Proposed 30" HDPE Storm Drain from Modified Off-site Detention Pond:

This storm drain will capture flows from the discharged offsite pond and route them along the perimeter of the property daylighting into the EDB in Lot 4. 4' precast concrete manholes will be used for maintenance access at all bends and grade breaks. A grouted riprap forebay will help dissipate energy at the outlet of the pipe, and allow for settling prior to entering the pond. See the Appendix for the hydraulic analysis of this storm drain (StormCAD).

In the event of an emergency and the offsite pond fails, developed flow (Q100=93.0 cfs) will overtop the pond and be collected between the proposed roadway and pond berm.. Flow not captured by the proposed inlet will bypass easterly to the proposed offsite swale between this property and the Sterling Ranch property and conveyed southerly.

Proposed 18" HDPE Storm Drain Culvert:

A 18" HDPE culvert will convey collected runoff from Lot 3 (Developed Q100 = 15.90cfs) through Lot 4 to the FSD Pond and join sheet flow from Lot 4 and the 30" piped bypass flow from basin O2. This culvert will be privately owned and maintained by the property owners. See the Appendix for open channel calculations.

On-site FSD - EDB Pond in Lot 4 (Basin D1):

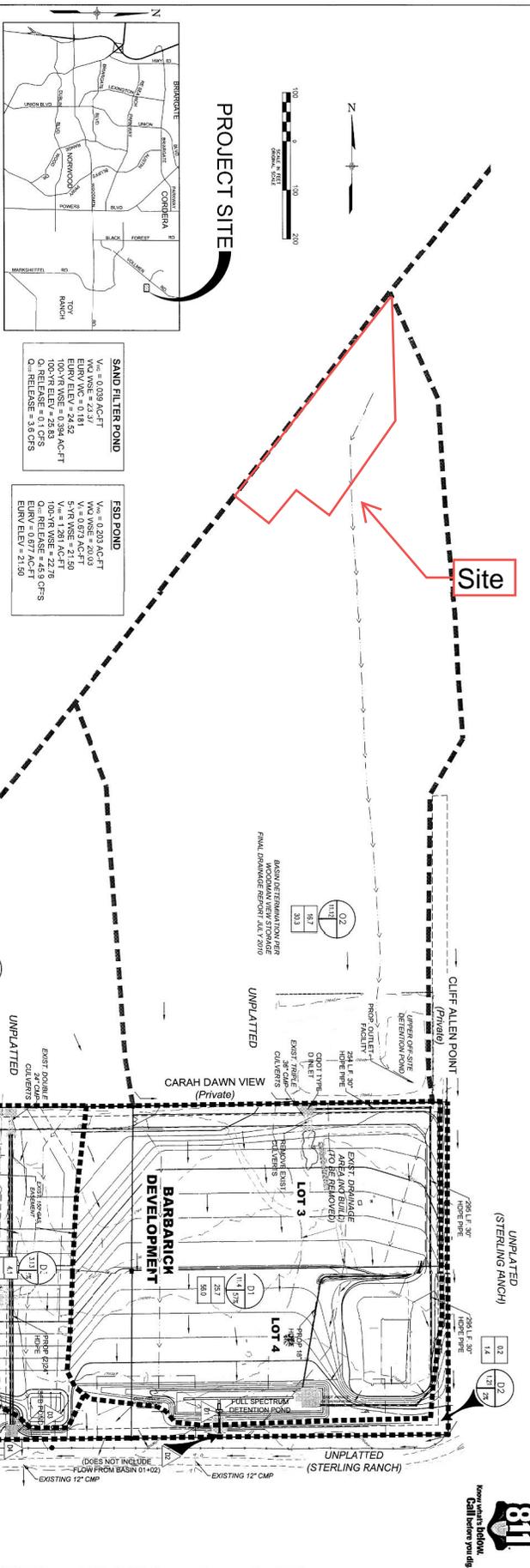
This On-site Full Spectrum Extended Detention Basin Pond provides water quality, EURV and 100-year detention. Onsite flows will combine with the 30-inch bypass flows from the north and pass through the EDB. The pond has been sized for the release of historic flows from Basin D1, as well as provides capacity for pass through conveyance of historic flows from the north.

The following table outlines the onsite existing and developed flow, required detention, and modifications to required detention utilizing the upstream over detention.

<u>On-site Basin Flow Summary (cfs)</u>	<u>5 year</u>	<u>100 year</u>
Existing On-site Flow at Pond	2.2	16.5
Developed On-site Flow (Basin D1)	<u>19.7</u>	<u>56.0</u>
Increase in peak flow due to development	17.5	39.5
Proposed Pass Through Flow from Off-Site Pond	<u>16.1*</u>	<u>29.4</u>
Proposed total flow out of EDB pond	<u>0.3</u>	<u>45.9**</u>

*Includes 10 year from WS-FDR

**Includes Pass Through flow of 29.4 cfs



VICINITY MAP
N.T.S.

BARBARICK DRAINAGE SUMMARY TABLE

BASIN	AREA (AC)	Q16 (CFS)	Q100 (CFS)	% IMP	COMMENT
D1	11.40	25.7	56.0	57%	D1 BASIN TO FSD +022 PASS THROUGH
D2	1.21	0.8	3.0	2%	POUND RELEASE + D2
D3	3.13	4.1	11.6	57%	D3 BASIN TO SFB
D4	11.12	16.7	30.3		POUND RELEASE + 03 PIPE PASS THROUGH
	7.03	13.7	35.5		REF: BARBARICK FDR 2005



REFERENCE DRAWINGS

NO.	DATE	DESCRIPTION	BY
		REVISIONS	
		BENCHMARK DATA (E.V.)	
		(DATA)	
		(DESCRIPTION/LOCATION)	

VERTICAL BENCHMARK

DATE: 10/20/2010 11:46:00 AM
PROJECT: BARBARICK FDR 2005
PROJECT SHEET: BARBARICK FDR 2005

PREPARED UNDER A DIRECT SUPERVISION FOR DESIGN GROUP, INC.

Matrix DESIGN GROUP
3265 Research Parkway, Suite 200
Ft. Worth, TX 76109
Phone: 718-275-0000
Fax: 718-275-0208

PROPOSED DRAINAGE PLAN

BARBARICK SUBDIVISION LOTS 1-4

DP02





WOODMAN VIEW STORAGE FINAL DRAINAGE REPORT

**JULY 2004
REVISED FEBRUARY 2010
REVISED MAY 2010
REVISED JULY 2010**

For:

**Woodmen View Storage
2720 Meridian Road
Peyton, CO 80831**



WOODMAN VIEW STORAGE
FINAL DRAINAGE REPORT
PAGE 2 of 5

2.2 Sub-Basin Description

- Historically, the runoff sheet-flows across the site to the south where it enters one of two draws to Sand Creek.
- A large upstream basin sheet-flows across the site.
- The offsite basin will continue to sheet-flow through the site in the developed conditions and is routed through the onsite detention pond.
- A swale is provided along the west property line to convey the discharge from the existing culvert under Vollmer Place.

3.0 DRAINAGE DESIGN CRITERIA

3.1 Development Criteria Reference and Constraints

- Previous studies for the proposed site or the surrounding areas are not available.
- The *Sand Creek Drainage Basin Planning Study* does not affect the proposed site.
- This study is in compliance with the following Standards except where stated herein:
 - *City of Colorado Springs and El Paso County Drainage Criteria Manual Volume 1 & 2*
- The simplicity and proposed use of the site do not create any drainage constraints.
- The proposed detention pond and outlet works must be constructed within the proposed site.

3.2 Hydrological Criteria

- Design rainfall is from the City/County's Criteria.
- The rational method was used to calculate peak runoff rates for the development.
- The 10-year storm was used as the minor event.
- The 100-year storm was used as the major event.
- Detention storage requirements were calculated using the Rational Stored Rate Method.

WOODMAN VIEW STORAGE
FINAL DRAINAGE REPORT
PAGE 3 of 5

- The Water Quality Capture Volume was calculated using the City/County's criteria.
- The combined runoff from the detention pond and developed undetained basins will be less than or equal to the total historic runoff rate from the site.

4.0 DRAINAGE FACILITY DESIGN

4.1 General Concepts

The following are concepts and typical drainage patterns of the proposed drainage system:

- Runoff generated in both the minor and major storm events will sheet-flow overland to the onsite detention pond.

- A swale is graded along the west property line to convey runoff from the north side of Vollmer Place and to keep onsite runoff from leaving the site.
- The proposed development is divided into seven basins (A, B, C, D, E, F, and G).
- Basins A-D are offsite basins. The offsite basins will continue to flow through the site and will be routed through the onsite detention pond.
- Basins E-G are made up entirely of the proposed development.
- Basin E will sheet-flow to the onsite detention pond.
- Basins F and G will be released from the site undetained.

Offsite runoff will be handled in the following ways:

- Offsite flows entering the site are conveyed through the site and proposed detention pond.

The following tables, charts, and figures are presented in the appendix of this report:

- Vicinity Map and Soils Map
- ¹FIRM Map
- Runoff computation sheets
- Detention Pond calculations
- Water Quality Capture Volume calculations
- ¹Pond Outfall Sizing spreadsheet
- ¹Restrictor Plate Sizing
- ¹Weir Design Spreadsheet
- ²Culvert Calculations

WOODMAN VIEW STORAGE
FINAL DRAINAGE REPORT
PAGE 4 of 5

- ²Riprap Sizing Calculations
- Tables and charts from *City of Colorado Springs and El Paso County Drainage Criteria Manual*

4.2 Specific Details

- It is anticipated the site will be developed in two phases.
- The detention facility must be constructed with the first phase.
- ²The flows released from the detention pond (16.1 cfs and 29.4 cfs) during the 10-year and 100-year events respectively, are equal to the historic flow rates at Design Point H5 (16.7 cfs and 30.3 cfs) less the developed flows released from the site undetained at Design Point 7 (0.6 cfs and 0.9 cfs).
- The detention volume was calculated using the City/County's Criteria.
- The WQCV was calculated using the City/County's Criteria.
- ¹ The outlet structure for the detention pond consists of a Modified Type D inlet. The rim of the inlet is set at the water quality water surface elevation and will collect the 10-year flow.
- ¹ The 100-year flow will outfall over a weir directly to one of the draws that drain to Sand Creek.
- An 18" HDPE culvert is provided at DP3 to convey the 100-year flow, 12.5cfs, from the onsite swale along the west property line to the onsite detention pond.
- Maintenance access to the detention pond will be provided via proposed drive aisles within the development and a gentle slope to the bottom of the pond per the City/Counties criteria.
- It is the responsibility of the property owner to maintain all drainage facilities.
- There are no immediate adverse impacts on downstream properties. The flows released from the site are equal to the historic flow rates through the site.



DETENTION POND CALCULATIONS

Woodman View Storage
El Paso County, CO

DETENTION POND CRITERIA

Peak release rate for the developed 10-yr and 100-yr events shall not exceed the historic rate for the drainage area

Criteria References:

El Paso County/City of Colorado Springs Drainage Criteria Manual
Urban Drainage and Flood Control District Criteria Manual

DETENTION POND RELEASE RATE CALCULATION

10-yr Historic Runoff (cfs)	10-yr Developed Runoff (cfs)
Design Point H5 = 16.7	Design Point 5 = 57.4
Design Point H7 = 15.3	Design Point 6 = 2.3
	Design Point 7 = 0.6

100-yr Historic Runoff (cfs)	100-yr Developed Runoff (cfs)
Design Point H5 = 30.3	Design Point 5 = 92.7
Design Point H7 = 30.0	Design Point 6 = 3.7
	Design Point 7 = 0.9

Allowable Release Rate at DP 5 (cfs)	
10-yr = 16.1	(DP H5 - DP 7)
100-yr = 29.4	(DP H5 - DP 7)

Allowable Release Rate at DP 6 (cfs)	
10-yr = 15.3	(Developed < Historic therefore no detention
100-yr = 30.0	at this location)

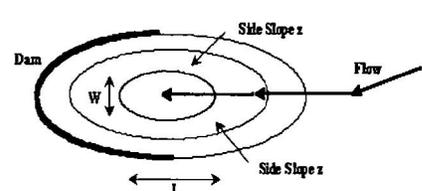
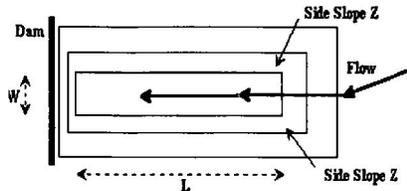
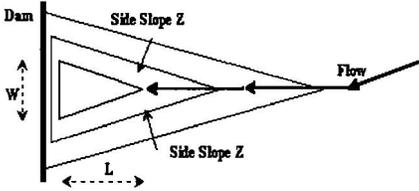
DETENTION POND VOLUME CALCULATION

Water Quality Capture Volume (WQCV) = 0.30	AC-FT	UDFCD WQCV Calculation	7045.74
10-yr Volume = 0.85	AC-FT	Rational Storage Rate Method	
10-yr Volume + WQCV = 1.15	AC-FT		7047.47
100-yr Volume = 1.37	AC-FT	Rational Storage Rate Method	
100-yr Volume + WQCV/2 = 1.52	AC-FT		7048.05

STAGE-STORAGE SIZING FOR POLYGONAL, ELLIPTICAL, OR IRREGULAR PONDS

Project: Woodman View Storage Final Drainage

Basin ID: _____



Design Information (Input):

Width of Pond Bottom, W = ft
 Length of Pond Bottom, L = ft
 Dam Side-slope (H:V), Z_d = ft/ft

Check Pond Shape

Right Triangle OR...
 Isosceles Triangle OR...
 Rectangle OR...
 Circle / Ellipse OR...
 Irregular (Use Override values in cells G32:G52)

Stage-Storage Relationship:

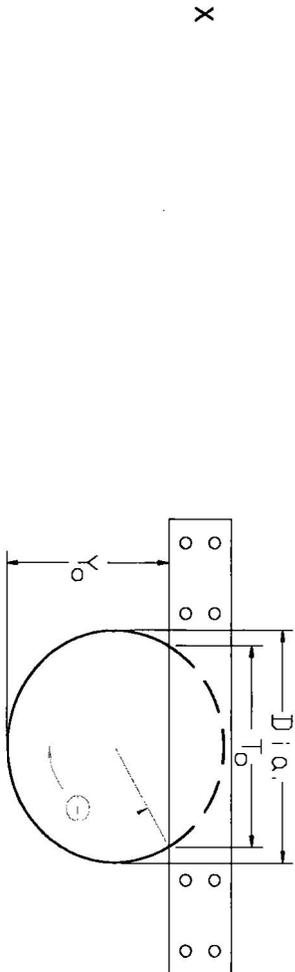
	MINOR	MAJOR	
Storage Requirement from Sheet 'Modified FAA':			acre-ft.
Storage Requirement from Sheet 'Hydrograph':			acre-ft.
Storage Requirement from Sheet 'Full-Spectrum':			acre-ft.

Labels for WQCV, Minor, & Major Storage Stages (input)	Stage (input) ft	Side Slope (H:V) ft/ft Below El. (input)	Pond Width at Stage (output) ft	Pond Length at Stage (output) ft	Surface Area at Stage (output) ft ²	Surface Area at Stage (output) ft ² User Override	Volume Below Stage (output) ft ³	Surface Area at Stage (output) acres	Volume Below Stage (output) acre-ft	Target Volumes for WQCV, Minor, & Major Storage Volumes (for goal seek)
	7043.00					0		0.000	0.000	
	7043.20		0.00	0.00		85	8	0.002	0.000	
	7043.40		0.00	0.00		541	71	0.012	0.002	
	7043.60		0.00	0.00		1,206	246	0.028	0.006	
	7043.80		0.00	0.00		1,802	547	0.041	0.013	
	7044.00		0.00	0.00		2,468	974	0.057	0.022	
	7044.20		0.00	0.00		3,221	1,542	0.074	0.035	
	7044.40		0.00	0.00		4,074	2,272	0.094	0.052	
	7044.60		0.00	0.00		5,029	3,182	0.115	0.073	
	7044.80		0.00	0.00		6,067	4,292	0.139	0.099	
	7045.00		0.00	0.00		7,256	5,624	0.167	0.129	
	7045.20		0.00	0.00		8,604	7,210	0.198	0.166	
	7045.40		0.00	0.00		10,126	9,083	0.232	0.209	
	7045.60		0.00	0.00		11,774	11,273	0.270	0.259	
WQCV	7045.80		0.00	0.00		13,756	13,826	0.316	0.317	0.30 REQUIRED
	7046.00		0.00	0.00		16,086	16,810	0.369	0.386	
	7046.20		0.00	0.00		18,669	20,286	0.429	0.466	
	7046.40		0.00	0.00		21,153	24,268	0.486	0.557	
	7046.60		0.00	0.00		22,506	28,634	0.517	0.657	
	7046.80		0.00	0.00		23,692	33,254	0.544	0.763	
	7047.00		0.00	0.00		24,730	38,096	0.568	0.875	
	7047.20		0.00	0.00		25,577	43,127	0.587	0.990	
	7047.40		0.00	0.00		26,259	48,310	0.603	1.109	
10-YR WSEL	7047.60		0.00	0.00		26,971	53,633	0.619	1.231	1.15 REQUIRED
	7047.80		0.00	0.00		27,873	59,118	0.640	1.357	
	7048.00		0.00	0.00		28,982	64,803	0.665	1.488	
100-YR WSEL	7048.20		0.00	0.00		30,276	70,729	0.695	1.624	1.52 REQUIRED
	7048.40		0.00	0.00		31,774	76,934	0.729	1.766	

RESTRICTOR PLATE SIZING FOR CIRCULAR VERTICAL ORIFICES

Project: Woodman View Storage Final Drainage

Basin ID: _____



Sizing the Restrictor Plate for Circular Vertical Orifices or Pipes (Input)

Water Surface Elevation at Design Depth
 Pipe/Vertical Orifice Entrance Invert Elevation
 Required Peak Flow through Orifice at Design Depth
 Pipe/Vertical Orifice Diameter (Inches)
 Orifice Coefficient

	#1 Vertical Orifice	#2 Vertical Orifice
Elev: WS =	7,047.74	
Elev: Invert =	7,042.67	
Q =	8.05	
Dia =	18.0	
C _o =	0.65	

Full-flow Capacity (Calculated)

Full-flow area
 Half Central Angle in Radians
 Full-flow capacity

Af =	1.77	sq ft
Theta =	3.14	rad
Qf =	19.2	cfs
Percent of Design Flow =	238%	

Calculation of Orifice Flow Condition

Half Central Angle ($0 < \text{Theta} < 3.1416$)
 Flow area
 Top width of Orifice (Inches)
 Height from Invert of Orifice to Bottom of Plate (feet)
 Elevation of Bottom of Plate
 Resultant Peak Flow Through Orifice at Design Depth

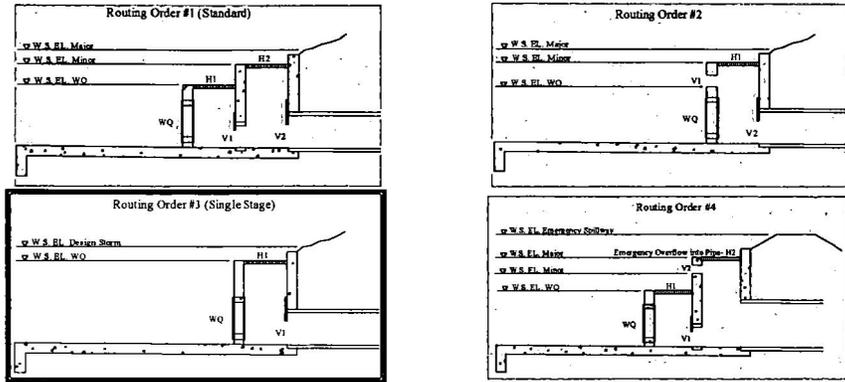
Theta =	1.42	rad
A _o =	0.71	sq ft
T _o =	17.78	Inches
Y _o =	0.63	feet
Elev Plate Bottom Edge =	7,043.30	feet
Q _o =	8.1	cfs

Width of Equivalent Rectangular Vertical Orifice
 Centroid Elevation of Equivalent Rectangular Vertical Orifice

Equivalent Width =	1.13	feet
Equiv. Centroid El. =	7,042.99	feet

STAGE-DISCHARGE SIZING OF THE WEIRS AND ORIFICES (INLET CONTROL)

Project: **Woodman View Storage Final Drainage**
 Basin ID: _____



Current Routing Order is #3

Design Information (Input):

Circular Opening: Diameter in Inches
 OR
 Rectangular Opening: Width in Feet
 Length (Height for Vertical)
 Percentage of Open Area After Trash Rack Reduction
 Orifice Coefficient
 Weir Coefficient
 Orifice Elevation (Bottom for Vertical)

	#1 Horiz.	#2 Horiz.	#1 Vert.	#2 Vert.	
Dia. =					inches
W =	2.92		1.55		ft.
L or H =	5.67		0.93		ft.
% open =	38		100		%
C _o =	0.65		0.65		
C _w =	2.55				
E _o =	7045.74		7,042.67		ft.

Calculation of Collection Capacity:

Net Opening Area (after Trash Rack Reduction)
 OPTIONAL: User-Override Net Opening Area
 Perimeter as Weir Length
 OPTIONAL: User-Override Weir Length

A _o =	6.21	1.44		sq. ft.
A _u =				sq. ft.
L _w =	13.53			ft.
L _w =				ft.
Top Elevation of Vertical Orifice Opening, Top =		7043.60		ft.
Center Elevation of Vertical Orifice Opening, Cen =		7043.14		ft.

Routing 3: Single Stage - Water flows through WQCV plate and #1 horizontal opening into #1 vertical opening. This flow will be applied to culvert sheet (#2 vertical & horizontal openings is not used).

Labels for WQCV, Minor, & Major Storage W.S. Elevations (input)	Water Surface Elevation ft (linked)	WQCV Plate/Riser Flow cfs (User-linked)	Horizontal Orifices				Vertical Orifices		Total Collection Capacity cfs (output)	Target Volumes for WQCV, Minor, & Major Storage Volumes (link for goal seek)
			#1 Horiz. Weir Flow cfs (output)	#1 Horiz. Orifice Flow cfs (output)	#2 Horiz. Weir Flow cfs (output)	#2 Horiz. Orifice Flow cfs (output)	#1 Vert. Collection Capacity cfs (output)	#2 Vert. Collection Capacity cfs (output)		
	7043.00	0.00	0.00	0.00	0.00	0.00	1.08	0.00	0.00	
	7043.20	0.01	0.00	0.00	0.00	0.00	2.20	0.00	0.01	
	7043.40	0.02	0.00	0.00	0.00	0.00	3.56	0.00	0.02	
	7043.60	0.03	0.00	0.00	0.00	0.00	5.12	0.00	0.03	
	7043.80	0.04	0.00	0.00	0.00	0.00	6.13	0.00	0.04	
	7044.00	0.05	0.00	0.00	0.00	0.00	6.99	0.00	0.05	
	7044.20	0.07	0.00	0.00	0.00	0.00	7.75	0.00	0.07	
	7044.40	0.09	0.00	0.00	0.00	0.00	8.45	0.00	0.09	
	7044.60	0.11	0.00	0.00	0.00	0.00	9.09	0.00	0.11	
	7044.80	0.13	0.00	0.00	0.00	0.00	9.69	0.00	0.13	
	7045.00	0.14	0.00	0.00	0.00	0.00	10.26	0.00	0.14	
	7045.20	0.17	0.00	0.00	0.00	0.00	10.79	0.00	0.17	
	7045.40	0.19	0.00	0.00	0.00	0.00	11.30	0.00	0.19	
	7045.60	0.21	0.00	0.00	0.00	0.00	11.79	0.00	0.21	
WQCV	7045.80	0.23	0.51	7.93	0.00	0.00	12.26	0.00	0.74	.30 REQUIRED
	7046.00	0.24	4.57	16.51	0.00	0.00	12.71	0.00	4.81	
	7046.20	0.25	10.76	21.97	0.00	0.00	13.15	0.00	11.01	
	7046.40	0.27	18.50	26.31	0.00	0.00	13.57	0.00	13.57	
	7046.60	0.28	27.52	30.03	0.00	0.00	13.98	0.00	13.98	
	7046.80	0.29	37.65	33.34	0.00	0.00	14.38	0.00	14.38	
	7047.00	0.30	48.80	36.35	0.00	0.00	14.77	0.00	14.77	
	7047.20	0.31	60.86	39.13	0.00	0.00	15.14	0.00	15.14	
	7047.40	0.33	73.79	41.73	0.00	0.00	15.51	0.00	15.51	
	7047.60	0.34	87.52	44.17	0.00	0.00	15.87	0.00	15.87	
10-YR WSEL	7047.80	0.35	102.01	46.48	0.00	0.00	16.22	0.00	16.22	1.15 REQUIRED
	7048.00	0.35	117.22	48.69	0.00	0.00	16.57	0.00	16.57	
100-YR	7048.20	0.36	133.12	50.80	0.00	0.00	16.90	0.00	16.90	1.52 REQUIRED
	7048.40	0.37	149.68	52.82	0.00	0.00	17.24	0.00	17.24	

STORM DRAINAGE SYSTEM DESIGN

WEIR DESIGN SPREADSHEET

PROJECT: Woodman View Storage
 CITY/COUNTY: Colorado Springs/El Paso

DATE: 7-May-10
 DESIGNER: JLT
 REVIEWER: TAJ

100 Year Weir must pass: 12.5 cfs Q = 100 year flow (29.4) - 100 year inlet capacity (16.9*)
 Emergency Weir must pass: 93.0 cfs Q = 100 year flow

Bottom of weir elevation = 7047.74 100-yr Available head = 0.31 feet
 100 Year water elev. = 7048.05 Emergency Overflow Available head= 1.26 feet
 Top of pond = 7049.00 Weir Coefficient = 3.1

Length of Rectangular Weir 22.8 FEET
 Side Slope 1 25% Angle 1 75.96 DEGREES
 Side Slope 2 25% Angle 2 75.96 DEGREES
 Total Angle For V-notch Weir 151.93

WSE	head (ft.)	Freeboard (ft.)	Rect weir (cfs)	v-notch (cfs)	total Q (cfs)	
7048.05	0.3	0.9	12.2	0.3	12.5	<----Q(100) Flow
7049.00	1.3	0.0	100.0	9.8	109.8	<----Q(Emergency) Flow

*Total Collection Capacity at 100-yr WSEL (see inlet control spreadsheet)

SUMMARY RUNOFF TABLE						
DESIGN POINT	BASIN	TOTAL AREA (ACRES)	Q10 (CFS)	TOTAL Q10 (CFS)	Q100 (CFS)	TOTAL Q100 (CFS)
HISTORIC						
H1	H-1	1.80	4.5	4.5	8.0	8.0
H2	H-2	0.45	2.2	2.2	3.4	3.4
H3	H-3	0.61	2.0	2.0	3.4	3.4
H4	H-4	1.80	6.0	6.0	10.4	10.4
H5	H-5	11.12	4.3	16.7	8.6	30.3
H6	H-6	5.63	13.2	13.2	25.7	25.7
H7	H-7	9.17	2.3	15.3	4.6	30.0
DEVELOPED						
1.00	A	1.80	4.5	4.5	8.0	8.0
2.00	B	0.85	2.2	2.2	3.4	3.4
3.00	C	2.64	2.9	7.2	4.9	12.5
4.00	D	7.43	22.8	22.8	43.0	43.0
5.00	E	19.69	32.5	57.4	52.1	92.7
6.00	F	0.48	2.3	2.3	3.7	3.7
7.00	G	0.12	0.6	2.9	0.9	4.6

WOODMEN VIEW S' ORAGE PLOT PLAN

LEGEND

DEVELOPED BASIN DESIGNATION
 AREA (AC) 2.00 0.02 10-YR RUNOFF COEFFICIENT 0.97 100-YR RUNOFF COEFFICIENT

HISTORIC BASIN DESIGNATION
 AREA (AC) 2.25 0.02 10-YR RUNOFF COEFFICIENT 0.90 100-YR RUNOFF COEFFICIENT

HISTORIC BASIN BOUNDARY

DEVELOPED BASIN BOUNDARY

DRAINAGE ARROW

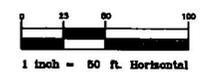
DESIGN POINT

EXISTING MAJOR CONTOUR (5')

EXISTING MINOR CONTOUR (1')

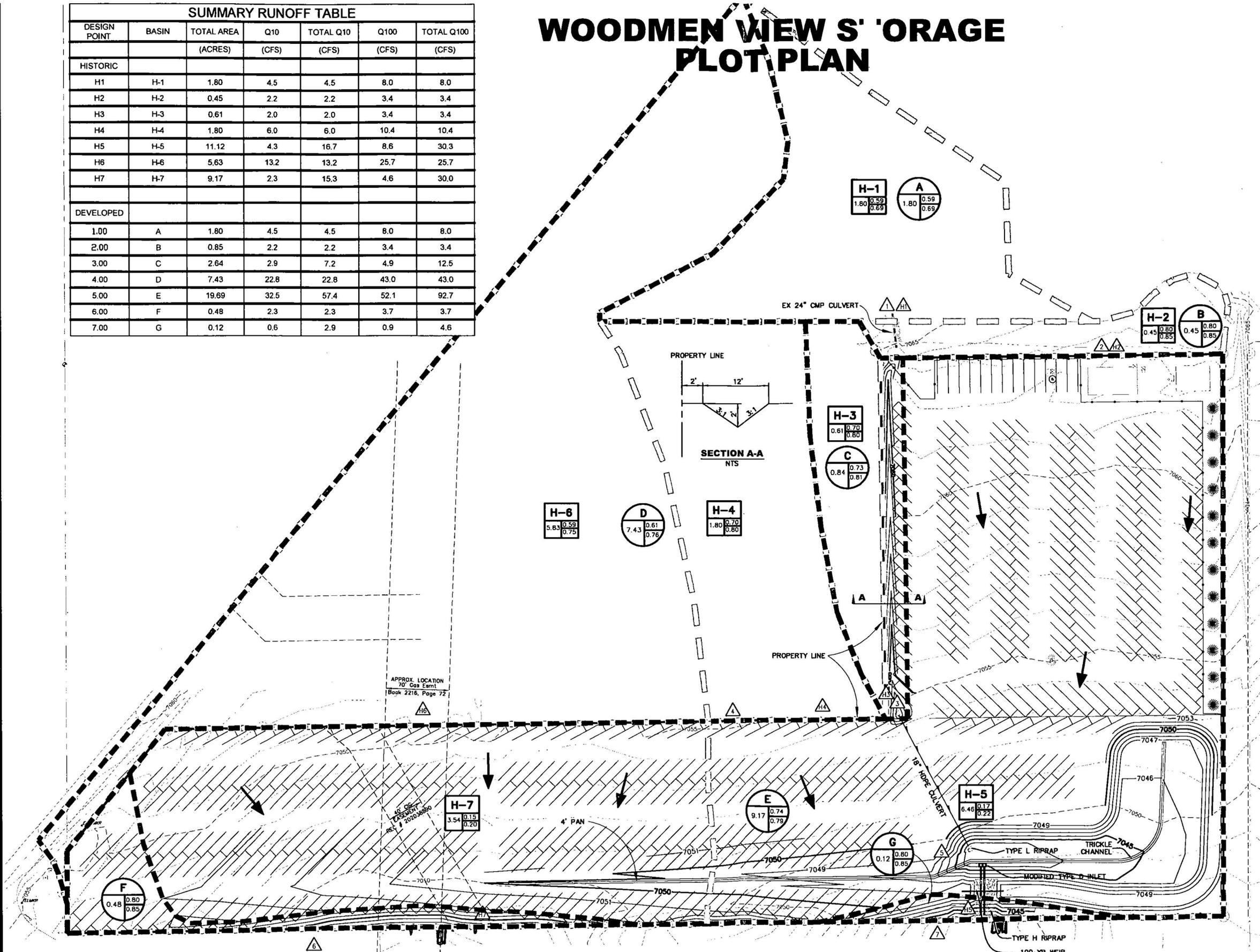
PROPOSED MAJOR CONTOUR (5')

PROPOSED MINOR CONTOUR (1')

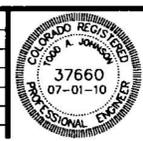


DETENTION POND

$V_{WQ} = 0.30$ AC-FT
 WQ WSE=7045.74
 $V_{10} = 1.15$ AC-FT
 10 YR WSE=7047.47
 $V_{100} = 1.52$ AC-FT
 100 YR WSE=7048.05
 Q_{10} RELEASE=16.1 cfs
 Q_{100} RELEASE=29.4 cfs



DATE	REVISION DESCRIPTION



Calibre
 ENGINEERING, INC.
 37660
 07-01-10
 PROFESSIONAL ENGINEER

WOODMEN VIEW STORAGE PLOT PLAN FINAL DRAINAGE REPORT
 JULY 1, 2010

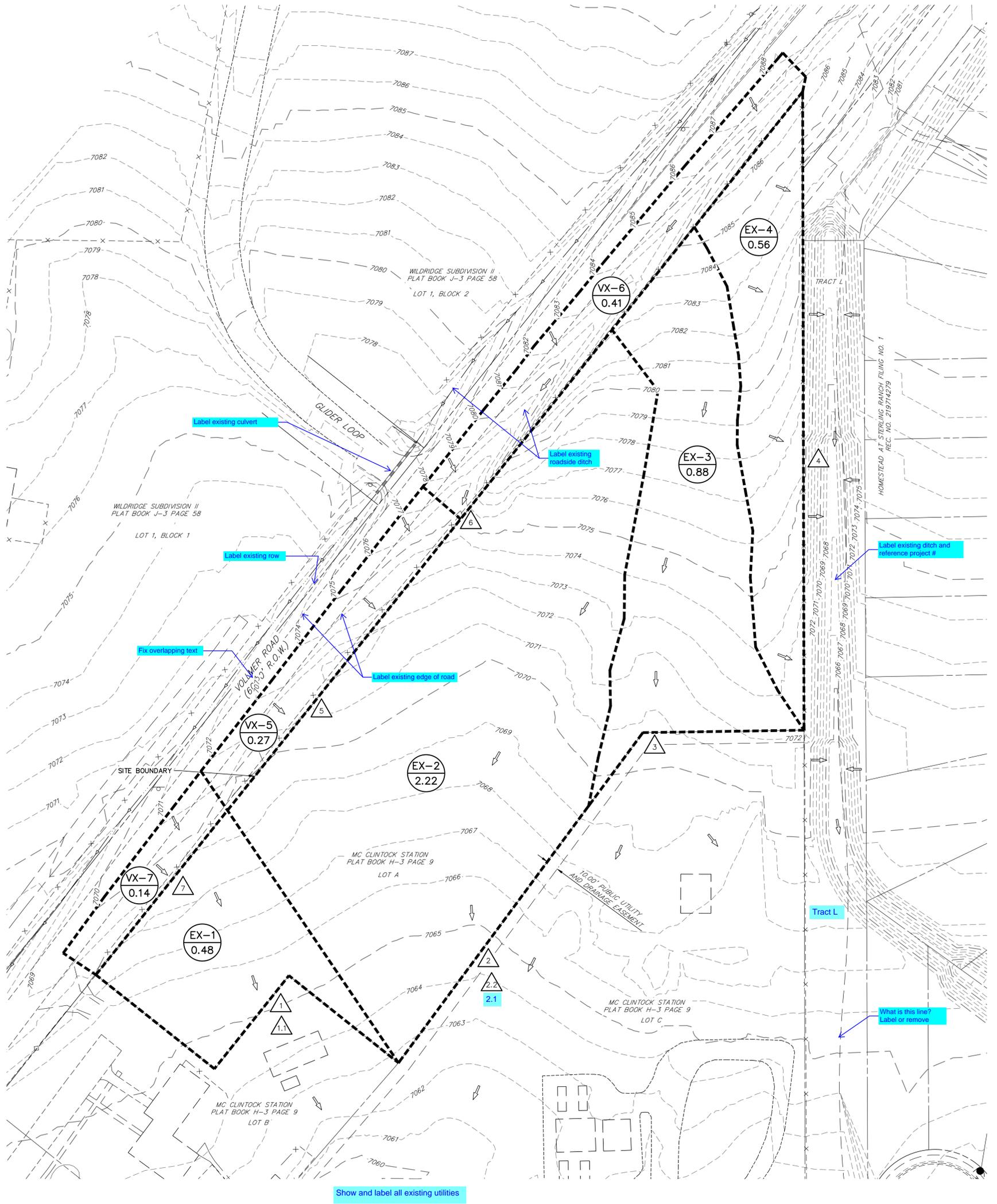
DR1
 of 1 sheets
 TAJ, EMB, GVM
 P.W. RYAN/CALIBRE

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APPENDIX E

DRAINAGE MAPS & PLANS

VOLLMER RV STORAGE EXISTING DRAINAGE MAP



BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q _s (cfs)	Q ₁₀₀ (cfs)
EX-1	0.48	2%	0.08	0.35	15.4	0.1	1.0
EX-2	2.22	2%	0.08	0.35	21.6	0.5	3.9
EX-3	0.88	2%	0.08	0.35	21.9	0.2	1.5
EX-4	0.56	2%	0.08	0.35	11.5	0.2	1.3
VX-5	0.27	30%	0.31	0.52	5.1	0.4	1.2
VX-6	0.41	32%	0.33	0.54	10.0	0.6	1.5
VX-7	0.14	36%	0.36	0.56	5.0	0.3	0.7

DESIGN POINT SUMMARY TABLE

DP#	Q _s -yr	Q ₁₀₀ -yr
1	0.1	1.0
1.1	0.3	1.5
2	0.5	3.9
2.1	1.2	5.7
3	0.2	1.5
4	0.2	1.3
5	0.4	1.2
6	0.6	1.5
7	0.3	0.7

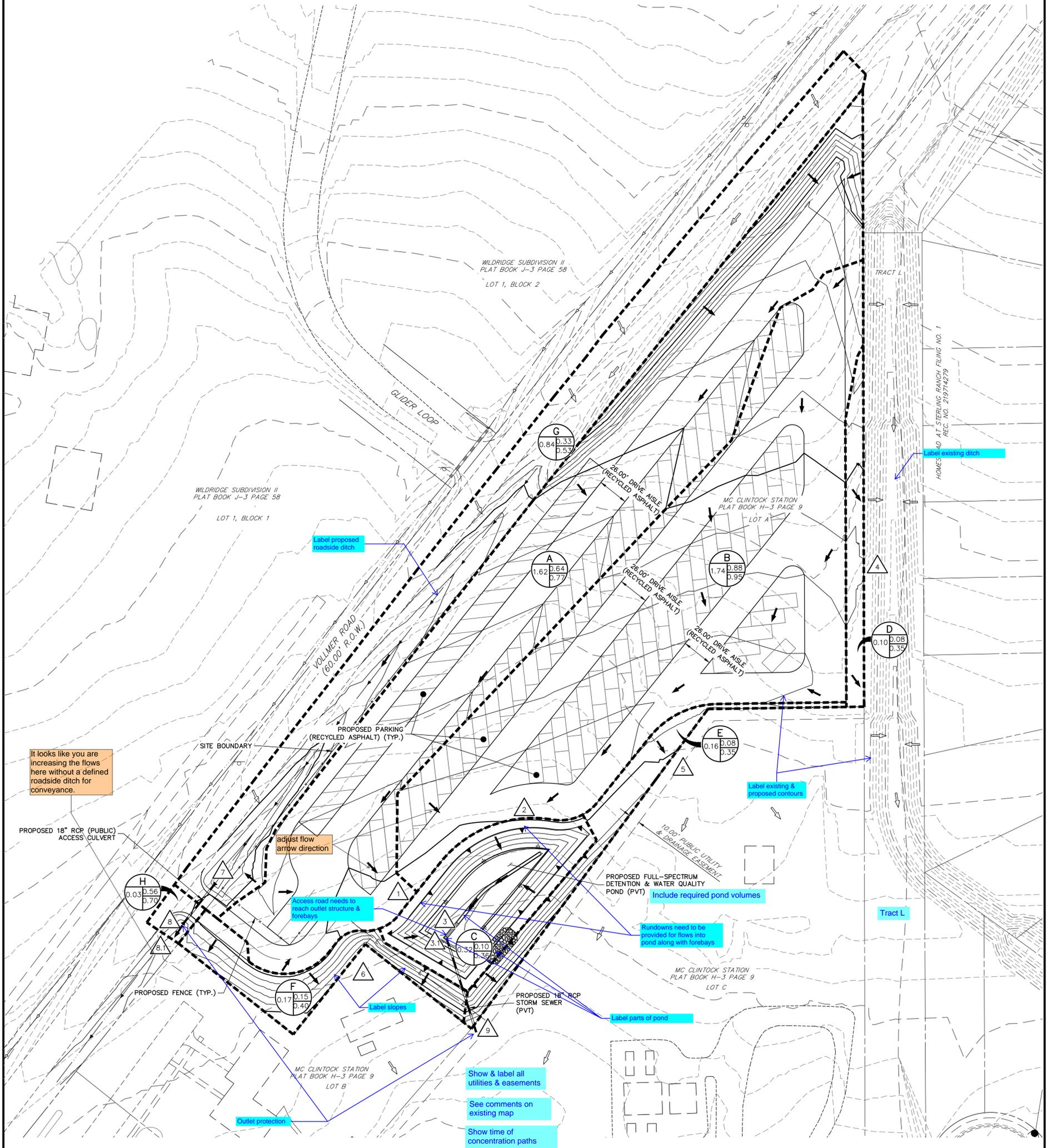
LEGEND:

- 6000--- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- DRAINAGE BASIN
- (A / B) A = BASIN DESIGNATION
B = AREA IN ACRES
- (1) DESIGN POINT
- ← EXISTING DRAINAGE ARROW

EXISTING DRAINAGE MAP
VOLLMER RV STORAGE
JOB NO. 25251.00
06/23/2022
SHEET 1 OF 1



VOLLMER RV STORAGE PROPOSED DRAINAGE MAP



Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q ₅ (cfs)	Q ₁₀₀ (cfs)
A	1.62	69%	0.64	0.77	8.3	4.6	9.2
B	1.74	98%	0.88	0.95	6.8	7.2	13.0
C	0.32	4%	0.10	0.36	7.1	0.1	0.9
D	0.10	2%	0.08	0.35	11.8	0.0	0.3
E	0.16	2%	0.08	0.35	5.0	0.1	0.5
F	0.17	10%	0.15	0.40	5.0	0.1	0.6
G	0.84	32%	0.33	0.53	17.4	0.9	2.5
H	0.03	59%	0.56	0.70	5.0	0.1	0.2

DP#	Q ₅ YR	Q ₁₀₀ YR
1	4.6	9.2
2	7.2	13.0
3	0.1	0.9
3.1	11.5	22.3
4	0.0	0.3
5	0.1	0.5
6	0.1	0.6
7	0.9	2.5
8	0.1	0.2
8.1	1.0	2.6
9	0.5	4.1

*-DP's release flows offsite

LEGEND:

- 6000 — PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- 6000 — EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- DRAINAGE BASIN

A = BASIN DESIGNATION
 B = AREA IN ACRES
 C = 5-YR RUNOFF COEFFICIENT
 D = 100-YR RUNOFF COEFFICIENT

DESIGN POINT
 DRAINAGE ARROW
 EXISTING DRAINAGE ARROW



PROPOSED DRAINAGE MAP
VOLLMER RV STORAGE
JOB NO. 25251.00
06/23/2022
SHEET 1 OF 1

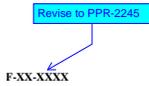
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X:\25251.00\Drawings\Sheet\Drawings\25251.00\DR.dwg, DR, 6/23/2022, 4:08:47 PM, CS

Drainage Report Final_V1.pdf Markup Summary

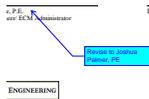
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Revise to PPR-2245

8/24/2022 9:33:28 AM (1)



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Author: lpackman
Date: 8/24/2022 9:33:28 AM
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Revise to Joshua Palmer, PE

8/25/2022 1:53:51 PM (1)

30.4%	0.09	0.35	0.29	1
34.3%	0.09	0.35	0.09	1

Per COS DCM Table 6-6 C-Values for 2% impervious are 0.09 & 0.36. Please update values.

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Date: 8/25/2022 1:53:51 PM
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Per COS DCM Table 6-6 C-Values for 2% impervious are 0.09 & 0.36. Please update values

8/25/2022 2:24:32 PM (1)

5	30.4%	0.09	0.35	0.29	1
Q	34.3%	0.09	0.35	0.09	1

Per COS DCM Table 6-6 C-Values for 2% impervious are 0.09 & 0.36. Please update values.

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Per COS DCM Table 6-6 C-Values for 2% impervious are 0.09 & 0.36. Please update values

8/25/2022 3:10:32 PM (1)

1	202	1.10	200	21	10
2	200	1.10	200	21	10
3	200	1.10	200	21	10
4	200	1.10	200	21	10
5	200	1.10	200	21	10
6	200	1.10	200	21	10
7	200	1.10	200	21	10
8	200	1.10	200	21	10
9	200	1.10	200	21	10
10	200	1.10	200	21	10

This length seems long, since basin is just the pond. Please verify.

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Date: 8/25/2022 3:10:32 PM
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This length seems long, since basin is just the pond. Please verify

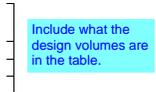
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Subject: Callout
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Date: 8/25/2022 4:22:57 PM
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If Final report, calculations should all be final, not preliminary

8/25/2022 4:25:33 PM (1)



Subject: Text Box
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Author: CDurham
Date: 8/25/2022 4:25:33 PM
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Include what the design volumes are in the table.

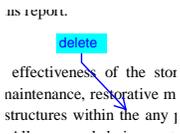
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100

8/25/2022 4:27:26 PM (1)



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delete

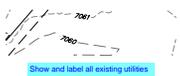
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Author: CDurham
Date: 8/25/2022 4:30:27 PM
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Calculations need to be included for:
-sizing of spillway riprap
-trickle channel
-forebays
-riprap outlet protection at pond outlet and access culvert
-roadside ditch
-rundowns into pond from DP 1 & DP2

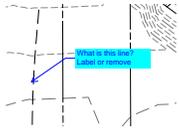
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Date: 8/25/2022 4:54:23 PM
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Show and label all existing utilities

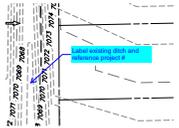
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Subject: Callout
Page Label: 62
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Date: 8/25/2022 4:54:49 PM
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Space:

What is this line? Label or remove

8/25/2022 4:55:47 PM (1)



Subject: Callout
Page Label: 62
Author: CDurham
Date: 8/25/2022 4:55:47 PM
Status:
Color: ■
Layer:
Space:

Label existing ditch and reference project #

8/25/2022 4:56:24 PM (1)



Subject: Text Box
Page Label: 62
Author: CDurham
Date: 8/25/2022 4:56:24 PM
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2.1

8/25/2022 4:56:46 PM (1)



Subject: Callout
Page Label: 62
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Fix overlapping text

8/25/2022 4:57:21 PM (1)



Subject: Callout
Page Label: 62
Author: CDurham
Date: 8/25/2022 4:57:21 PM
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Label existing culvert

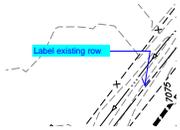
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Date: 8/25/2022 4:57:52 PM
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Label existing edge of road

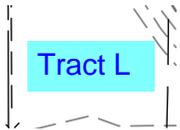
8/25/2022 4:58:08 PM (1)



Subject: Callout
Page Label: 62
Author: CDurham
Date: 8/25/2022 4:58:08 PM
Status:
Color: ■
Layer:
Space:

Label existing row

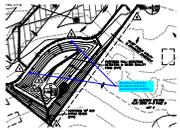
8/25/2022 4:59:09 PM (1)



Subject: Text Box
Page Label: 62
Author: CDurham
Date: 8/25/2022 4:59:09 PM
Status:
Color: ■
Layer:
Space:

Tract L

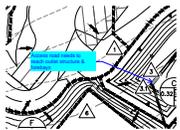
8/25/2022 5:00:13 PM (1)



Subject: Callout
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:00:13 PM
Status:
Color: ■
Layer:
Space:

Rundowns need to be provided for flows into pond along with forebays

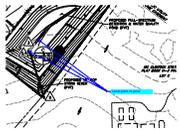
8/25/2022 5:00:45 PM (1)



Subject: Callout
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:00:45 PM
Status:
Color: ■
Layer:
Space:

Access road needs to reach outlet structure & forebays

8/25/2022 5:01:25 PM (1)



Subject: Callout
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:01:25 PM
Status:
Color: ■
Layer:
Space:

Label parts of pond

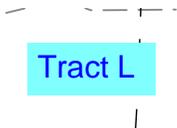
8/25/2022 5:05:19 PM (1)



Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:05:19 PM
Status:
Color: ■
Layer:
Space:

Show & label all utilities & easements

8/25/2022 5:05:59 PM (1)



Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:05:59 PM
Status:
Color: ■
Layer:
Space:

Tract L

8/25/2022 5:06:08 PM (1)



Subject: Callout
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:06:08 PM
Status:
Color: ■
Layer:
Space:

Label existing ditch

8/25/2022 5:06:32 PM (1)



Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:06:32 PM
Status:
Color: ■
Layer:
Space:

See comments on existing map

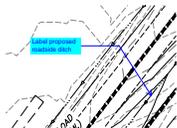
8/25/2022 5:07:01 PM (1)



Subject: Callout
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:07:01 PM
Status:
Color: ■
Layer:
Space:

Label existing & proposed contours

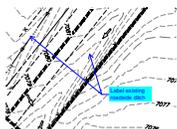
8/25/2022 5:07:30 PM (1)



Subject: Callout
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:07:30 PM
Status:
Color: ■
Layer:
Space:

Label proposed roadside ditch

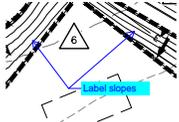
8/25/2022 5:07:56 PM (1)



Subject: Callout
Page Label: 62
Author: CDurham
Date: 8/25/2022 5:07:56 PM
Status:
Color: ■
Layer:
Space:

Label existing roadside ditch

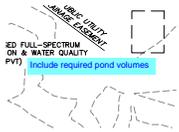
8/25/2022 5:08:32 PM (1)



Subject: Callout
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:08:32 PM
Status:
Color: ■
Layer:
Space:

Label slopes

8/25/2022 5:09:15 PM (1)



Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:09:15 PM
Status:
Color: ■
Layer:
Space:

Include required pond volumes

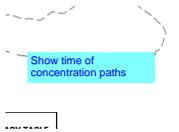
8/25/2022 5:10:15 PM (1)



Subject: Callout
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:10:15 PM
Status:
Color: ■
Layer:
Space:

Outlet protection

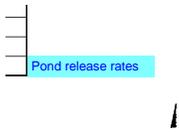
8/25/2022 5:10:33 PM (1)



Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:10:33 PM
Status:
Color: ■
Layer:
Space:

Show time of concentration paths

8/25/2022 5:10:56 PM (1)



Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:10:56 PM
Status:
Color: ■
Layer:
Space:

Pond release rates

8/25/2022 5:11:25 PM (1)

7	0.7	0.2
8	0.1	0.2
8.1	1.0	2.6
9	0.5	4.1

*-DP's release flows offsite

Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:11:25 PM
Status:
Color: ■
Layer:
Space:

*-DP's release flows offsite

8/25/2022 5:12:17 PM (1)

4 *

Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:12:17 PM
Status:
Color: ■
Layer:
Space:

*

8/25/2022 5:12:26 PM (1)

5 *

Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:12:26 PM
Status:
Color: ■
Layer:
Space:

*

8/25/2022 5:12:31 PM (1)

6 *

Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:12:31 PM
Status:
Color: ■
Layer:
Space:

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8/25/2022 5:12:37 PM (1)

.1 *

Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:12:37 PM
Status:
Color: ■
Layer:
Space:

*

8/25/2022 5:12:44 PM (1)

9 *

Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:12:44 PM
Status:
Color: ■
Layer:
Space:

*

8/25/2022 5:13:30 PM (1)

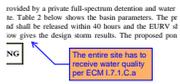


Include water quality exhibit showing what areas are treated by ponds and what is not treated

Subject: Text Box
Page Label: 63
Author: CDurham
Date: 8/25/2022 5:13:30 PM
Status:
Color: ■
Layer:
Space:

Include water quality exhibit showing what areas are treated by ponds and what is not treated

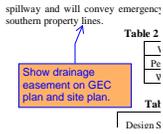
8/29/2022 11:47:44 AM (1)



Subject: Callout
Page Label: 8
Author: lpackman
Date: 8/29/2022 11:47:44 AM
Status:
Color: ■
Layer:
Space:

The entire site has to receive water quality per ECM I.7.1.C.a

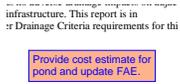
8/29/2022 11:58:46 AM (1)



Subject: Callout
Page Label: 9
Author: lpackman
Date: 8/29/2022 11:58:46 AM
Status:
Color: ■
Layer:
Space:

Show drainage easement on GEC plan and site plan.

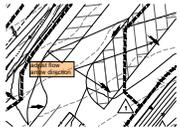
8/29/2022 3:10:44 PM (1)



Subject: Text Box
Page Label: 10
Author: lpackman
Date: 8/29/2022 3:10:44 PM
Status:
Color: ■
Layer:
Space:

Provide cost estimate for pond and update FAE.

9/2/2022 3:53:17 PM (1)



Subject: Engineer
Page Label: 63
Author: dotprete
Date: 9/2/2022 3:53:17 PM
Status:
Color: ■
Layer:
Space:

adjust flow arrow direction

9/5/2022 6:21:03 AM (1)



Subject: Engineer
Page Label: 63
Author: dotprete
Date: 9/5/2022 6:21:03 AM
Status:
Color: ■
Layer:
Space:

It looks like you are increasing the flows here without a defined roadside ditch for conveyance.

9/5/2022 6:22:26 AM (1)



Subject: Engineer
Page Label: 8
Author: dotprete
Date: 9/5/2022 6:22:26 AM
Status:
Color: ■
Layer:
Space:

The existing outfall for the site (DP9) is believed to be stable and sufficient in today's condition.

9/5/2022 6:24:11 AM (1)

small volume and per
Step 2. Stabilize. Drain
from go towards channel
the site (DPO) is before
for stability during final
fill.
Step 3. Test the WQ
parameters water quality
the runoff from this +
then incorporated into
has been designed to
possible other runoff
reduced to less than 10
reducing peak volumes

Subject: Engineer
Page Label: 8
Author: dotprete
Date: 9/5/2022 6:24:11 AM
Status:
Color: ■
Layer:
Space:

Flows are lower than historic because of the pond, but you are changing the flow from sheet flow to concentrated flow. please confirm the existing outfall is suitable.

9/5/2022 6:51:00 AM (1)

asphalt part
spectrum w:
Water Quali
Water quali
the southeast
sized. The V
within 72 h

Subject: Engineer
Page Label: 8
Author: dotprete
Date: 9/5/2022 6:51:00 AM
Status:
Color: ■
Layer:
Space:

was this sentence cut off?

9/5/2022 6:51:36 AM (1)

for the
diverse
agency
ng the

Subject: Engineer
Page Label: 9
Author: dotprete
Date: 9/5/2022 6:51:36 AM
Status:
Color: ■
Layer:
Space:

include a forebay