

**ADDENDUM 2 TO THE
FINAL DRAINAGE REPORT
FOR
HOMESTEAD AT STERLING RANCH FILING NO. 2**

**EPCD File No. SF-19-004/CDR-20-012
PCD File No. VR234**

Prepared For:

**SR Land, LLC
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Colorado Springs, CO 80903**

**April 17, 2023
Project No. 25188.29**

**Prepared By:
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**ADDENDUM 2 TO THE FINAL DRAINAGE REPORT FOR HOMESTEAD AT
STERLING RANCH FILING NO. 2**

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.



Mike Bramlett, Colorado P.E. # 32314
For and On Behalf of JR Engineering, LLC

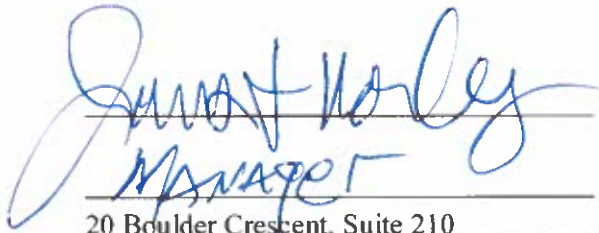


DEVELOPER'S STATEMENT:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: SR Land, LLC

By:


Manager

Title:

Address: 20 Boulder Crescent, Suite 210
Colorado Springs, CO 80903

El Paso County:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2 and Engineering Criteria Manual, as amended.

Joshua Palmer, P.E.
County Engineer/ ECM Administrator

Date

Conditions:



JR ENGINEERING

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STERLING RANCH FILING NO. 2**

Table of Contents

PURPOSE.....	1
PROPOSED DRAINAGE CHARACTERISTICS.....	1
Detailed Drainage Discussion (Design Points)	1
Water Quality Provisions	3
Construction Cost opinion – Homestead at Sterling Ranch Fil. No. 2	4

REVISED APPENDIX MATERIALS

Composite Percent Impervious & Composite Runoff Coefficient Calculations	
SF-2 Minor Basins X1, X2, W1, & Y1	
SF-3 Minor Basins X1, X2, W1, & Y1	
SF-3 Major Basins X1, X2, W1, & Y1	
MHFD Detention workbook Sand Filter 1	
MHFD Detention workbook Sand Filter 2	
Emergency Spillway Protection Sizing	
Proposed basin map (limited to Basins X1, X2, W1, & Y1)	



ADDENDUM 2 TO THE FINAL DRAINAGE REPORT FOR HOMESTEAD AT STERLING RANCH FILING NO. 2

PURPOSE

This document is an Addendum 2 to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2. The purpose of this report is to update the approved “Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2”. The scope of the updates included in this addendum are limited to proposed Basins W1 & X1 due to the addition of a new lot 2 (Basin X1) and the modification to lot 24 (new lot 1, Basin W1), making it smaller. More specifically, this Addendum 2 proposes updated design information for two common Sand Filters, one to serve basin X1: lots 36-41 & new lot 2, and one to serve basins W1, X2, & Y1, lots 13-24, 28-35, & new lot 1.

The text below replaces the original corresponding text from the Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2.

PROPOSED DRAINAGE CHARACTERISTICS

DETAILED DRAINAGE DISCUSSION (DESIGN POINTS)

BASIN X1 (0.91 acres), consists of existing residential backyards of lots 36-41 and proposed new lot along the eastern boundary of the site. Italicized text within this section is original and unchanged. Basin X1 increased in size due to the additional lot 2 that will be treated within existing Sand Filter 1. Previously the area for the new lot was located within a tract and was assumed to drain freely to the existing Sand Creek. In accordance with the previous design, a swale/berm and area drain will be used for the new lot to collect the runoff and pipe it directly to existing Sand Filter 1. *Runoff in this basin will be directed via backyard swales/berms towards the rear of the lots where it will be collected in a 12” Nyoplast Drain Basin w/ a 12” dome grate placed in the rear southwest corner of each lot. Each lot will have a half foot berm/swale along the rear and low side of lot that directs water to each inlet, see the detail included on the drainage map, GESC, and Storm plans. Should the inlet in any lot become clogged, the berm will overtop, and flow will continue to Sand Creek. Per the original drainage report by MandS, the estimated flows per each lot in basin X1 = 0.2 cfs for the 100 year storm. The 12” Nyoplast Drain Basins are sized to collect all flows ($Q_5 = 0.7$ cfs, $Q_{100} = 2.6$ cfs) in both the 5 and 100 year storms. Collected flows will then be piped to a proposed full-spectrum sand filter, with a 12-hour drain time and a 4” perforated underdrain. The treated flows from the sand filter will be discharged via an outlet structure to the adjacent Sand Creek. See the comparison table at the end of the Detailed Drainage Discussion section to see the changes to Basin X1.*

BASIN X2 has no proposed changes from the “Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2”.



ADDENDUM 2 TO THE FINAL DRAINAGE REPORT FOR HOMESTEAD AT STERLING RANCH FILING NO. 2

BASIN W1 (0.84 acres), consists of existing residential backyards of lots 19-23 and a proposed adjustment to lot 24 (new lot 1) along the southeastern boundary of the site. Italicized text within this section is original and unchanged. Basin W1 decreased in size due to the adjustment to lot 24 decreasing the lot area. Only the runoff from lot areas are collected and treated in existing Sand Filter 2 and since the lot size is decreasing, the swale/berm configuration along the back of the lot is also decreasing in length. *Runoff in this basin will be directed via backyard swales towards the rear of the lots where it will be collected in a 12" Nyoplast Drain Basin w/ a 12" dome grate placed in the rear corner of each lot (DP3). Each lot will have a half foot berm/swale along the rear and low side of lot that directs water to each inlet, see the detail included on the drainage map, GESC, and Storm plans. Should the inlet in any lot become clogged, the berm will overtop, and flow will continue to Sand Creek per existing drainage patterns. The 12" Nyoplast Drain Basins are sized to collect all flows ($Q_5 = 0.6$ cfs, $Q_{100} = 2.2$ cfs) in both the 5 and 100 year storms. Collected flows will then be piped southwest via 12" HDPE pipe following the rear lot lines towards DP3.1, where flows in the pipe combine with collected flows from Basin X2 ($Q_5 = 1.3$ cfs, $Q_{100} = 4.6$ cfs). See the comparison table at the end of the Detailed Drainage Discussion section to see the changes to Basin W1.*

BASIN Y1 has no proposed changes from the "Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2".

The basin characteristics, hydrologic parameters, runoff and rational calcs for Basins X1, X2, W1, and Y1 have remained consistent with the approved Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2. However, the size of the basins X1 and W1 has changed and therefore revised composite percent impervious, SF-2, & SF-3 forms are included in the appendix section of this report. A revised basin map, showing the changes within the above described basins is also attached to this report. See the table below for a comparison of the changed basins in the previously approved Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2 as well as the changes proposed in this Addendum 2.

Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2					
Basin	Area	C ₅	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
X1	0.78	0.22	0.46	0.6	2.3
W1	0.86	0.22	0.46	0.6	2.2
Addendum 2 to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2					
Basin	Area	C ₅	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
X1	0.91	0.22	0.46	0.7	2.6
W1	0.84	0.22	0.46	0.6	2.2

ADDENDUM 2 TO THE FINAL DRAINAGE REPORT FOR HOMESTEAD AT STERLING RANCH FILING NO. 2

WATER QUALITY PROVISIONS

The design of the full-spectrum Sand Filter 1 (North Sand Filter) outlet structure changed to accept the additional area of the new proposed lot 2 within Basin X1. Italicized text within this section is original and unchanged. Runoff from this additional lot will be collected in a drain basin and piped via a private storm sewer into the Sand Filter 1. The new proposed flared end section (FES) in the sand filter will be protected by Type L riprap per the previously approved design. The runoff will be treated in the full-spectrum sand filter, and discharged into Sand Creek via an existing private storm sewer system. The increased Basin X1 area from 0.78 acres to 0.91 acres (as shown in the Detailed Drainage Discussion section) caused changes to the outlet structure in order to meet the required drain times. Specifically, the underdrain orifice diameter increased from 0.42 inches to 0.46 inches and the height of the restrictor plate above the outlet pipe invert increased from 1.90 inches to 2.50 inches. *Sand Filter 1 was designed to have a 12 hour WQCV drain time and a peak outflow for the 100 year design storm of 0.9 cfs which is less than or equal to pre-development peak flows. The peak discharge rate of the proposed sand filter is at or below the historic flows for the basin which it serves.* See the comparison table at the end of the Water Quality Provisions section to see the changes to Sand Filter 1.

The design of the full-spectrum Sand Filter 2 (South Sand Filter) remained unchanged with the decrease of lot 24 (new lot 1) within Basin W1. Italicized text within this section is original and unchanged. Runoff from the decreased lot 24 area will still be collected in a drain basin, treated in the full-spectrum sand filter, and discharged into Sand Creek via a private storm sewer system. The decreased Basin W1 area from 0.86 acres to 0.84 acres (as shown in the Detailed Drainage Discussion section) was a small enough change overall for the Sand Filter 2 basin that no changes to the outlet structure design are required order to satisfy drain times. *Sand Filter 2 was designed to have a 12 hour WQCV drain time and a peak outflow for the 100 year design storm of 2.7 cfs which is less than or equal to pre-development peak flow rates. The peak discharge rate of the proposed sand filter is at or below the historic flows for the basins which it serves.* See the comparison table at the end of the Water Quality Provisions section to see the changes to Sand Filter 2.

The proposed sand filters calculations were updated using the MHFD Detention workbook and printouts are included in the Hydraulic Calculations section of this report.

Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2							
Name	Watershed Area (acres)	Req. WQCV (acre-feet)	Req. EURV (acre-feet)	Req. 100-year Volume (acre-feet)	Provided WQCV (acre-feet)	Provided EURV (acre-feet)	Provided 100-year Volume (acre-feet)
Sand Filter 1	0.78	0.007	0.013	0.027	0.007	0.020	0.042
Sand Filter 2	2.74	0.025	0.045	0.094	0.026	0.070	0.123
Addendum 2 to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2							
Name	Watershed Area (acres)	Req. WQCV (acre-feet)	Req. EURV (acre-feet)	Req. 100-year Volume (acre-feet)	Provided WQCV (acre-feet)	Provided EURV (acre-feet)	Provided 100-year Volume (acre-feet)
Sand Filter 1	0.91	0.008	0.015	0.031	0.008	0.023	0.043
Sand Filter 2	2.72	0.024	0.044	0.093	0.024	0.070	0.122



ADDENDUM 2 TO THE FINAL DRAINAGE REPORT FOR HOMESTEAD AT STERLING RANCH FILING NO. 2

CONSTRUCTION COST OPINION – HOMESTEAD AT STERLING RANCH FIL. NO. 2

Drainage improvements are planned with the development of Homestead at Sterling Ranch Filing No. 2. A majority of the construction costs have been accounted for in the “Master Development Drainage Report for Sterling Ranch Filing Nos. 1&2, and Final Drainage Report for Sterling Ranch Filing No.1” prepared by MS Civil Consultants, dated April 2017. Further cost updates were included in “Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2” by JR Engineering, dated June 2021. Any additional improvements and costs are listed below.

The following list of drainage improvements is Non-Reimbursable.

Item	Description	Additional Quantity	Unit	Unit Cost	Cost
1	12" Storm Pipe	108	LF	\$ 26.00	\$ 2,808.00
2	12" Nyloplast Drain Basin w/ 12" Dome Grate	1	EA	\$1,000.00	\$ 1,000.00
3	12"-15" FES	1	EA	\$ 350.00	\$ 350.00
4	Soil Riprap (9") [Type L]	3	CY	\$ 180.00	\$ 540.00

**ADDENDUM 2 TO THE FINAL DRAINAGE REPORT FOR HOMESTEAD AT
STERLING RANCH FILING NO. 2**

REVISED APPENDIX MATERIAL



COMPOSITE % IMPERVIOUS & COMPOSITE RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Homestead at Sterling Ranch Filing No. 2
 Location: Colorado Springs

Project Name: Homestead at Sterling Ranch Filing No. 2
 Project No.: 2000-5188.29
 Calculated By: GAG
 Checked By: _____
 Date: 12/20/22

Basin ID	Total Area (ac)	M & S Report (1/2 ac lots)				Roofs (90% Impervious)				Gravel (80% Impervious)				Lawns (0% Impervious)				Basins Total Weighted C		Basins Total Weighted % Imp.
		C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	
W1	0.84	0.22	0.46	0.84	25.0%	0.73	0.81		0.0%	0.59	0.70		0.0%	0.08	0.35		0.0%	0.22	0.46	25.0%
X1	0.91	0.22	0.46	0.91	25.0%	0.73	0.81		0.0%	0.59	0.70		0.0%	0.08	0.35		0.0%	0.22	0.46	25.0%
X2	1.04	0.22	0.46	1.04	25.0%	0.73	0.81		0.0%	0.59	0.70		0.0%	0.08	0.35		0.0%	0.22	0.46	25.0%
Y1	0.84	0.22	0.46	0.84	25.0%	0.73	0.81		0.0%	0.59	0.70		0.0%	0.08	0.35		0.0%	0.22	0.46	25.0%
TOTAL	3.63																			25.0%
Grey-boxes indicate no change from the approved "Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2" by JR Engineering dated 6/16/21.																				

STANDARD FORM SF-2 TIME OF CONCENTRATION

Subdivision: Homestead at Sterling Ranch Filing No. 2
Location: Colorado Springs

Project Name: Homestead at Sterling Ranch Filing No. 2
Project No.: 2000-5188.29
Calculated By: GAG
Checked By: _____
Date: 12/20/22

SUB-BASIN						INITIAL/OVERLAND			TRAVEL TIME					tc CHECK			FINAL
DATA						(Ti)			(Tt)					(URBANIZED BASINS)			
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C5	C100	L (ft)	So (%)	ti (min)	Lt (ft)	St (%)	K	VEL. (ft/s)	tt (min)	COMP. tc (min)	TOTAL LENGTH (ft)	Urbanized tc (min)	tc (min)
W1	0.84	B	25%	0.22	0.46	100	2.0%	12.6	50	0.0%	20.0	0.2	4.2	16.8	150.0	28.4	16.8
X1	0.91	B	25%	0.22	0.46	100	2.0%	12.6	50	2.5%	20.0	3.2	0.3	12.9	150.0	22.2	12.9
X2	1.04	B	25%	0.22	0.46	100	2.0%	12.6	50	2.5%	20.0	3.2	0.3	12.9	150.0	22.2	12.9
Y1	0.84	B	25%	0.22	0.46	100	2.0%	12.6	50	2.5%	20.0	3.2	0.3	12.9	150.0	22.2	12.9
Grey-boxes indicate no change from the approved "Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2" by JR Engineering dated 6/16/21.																	

Grey-boxes indicate no change from the approved "Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2" by JR Engineering dated 6/16/21.

NOTES:

$$t_c = t_i + t_t$$

Equation 6-2

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_o^{0.33}}$$

Equation 6-3

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

Where:

t_i = overland (initial) flow time (minutes)

C₅ = runoff coefficient for 5-year frequency (from Table 6-4)

L_i = length of overland flow (ft)

S_o = average slope along the overland flow path (ft/ft).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

Equation 6-4

$$t_t = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$

Equation 6-5

Where:

t_t = channelized flow time (travel time, min)

L_t = waterway length (ft)

S_o = waterway slope (ft/ft)

V_t = travel time velocity (ft/sec) = K√S_o

K = NRCS conveyance factor (see Table 6-2).

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.

L_t = length of channelized flow path (ft)

i = imperviousness (expressed as a decimal)

S_t = slope of the channelized flow path (ft/ft).

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Homestead at Sterling Ranch Filing No. 2
Location: Colorado Springs
Design Storm: 5-Year

Project Name: Homestead at Sterling Ranch Filing No. 2
Project No.: 2000-5188
Calculated By: GAG
Checked By:
Date: 12/20/22

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C^*A (Ac)	I (in/hr)	Q (cfs)	t_c (min)	C^*A (ac)	I (in/hr)	Q (cfs)	Q_{pipe} (cfs)	C^*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	X1	0.91	0.22	12.9	0.20	3.75	0.7					0.7	0.20	0.5	18	250	1.8	2.3	Runoff from Basin X1, collected by private 12" Nyoplast Drain Basins, Piped via 12" HDPE to pvt. sand filter @ DP1.1
	2	X2	1.04	0.22	12.9	0.23	3.75	0.9					0.9	0.23	1.5	12	950	2.8	5.6	Runoff from Basin X2, collected by private 12" Nyoplast Drain Basins, Piped via 12" HDPE to DP3.1
	3	W1	0.84	0.22	16.8	0.18	3.35	0.6					0.6	0.18	1.5	12	250	2.7	1.5	Runoff from Basin W1, collected by private 12" Nyoplast Drain Basins, Piped via 12" HDPE to DP3.1
	3.1								20.0	0.41	3.09	1.3								Combined flow in private 12" HDPE pipe @ DP3.1, piped to private sand filter @ DP-4.1
	4	Y1	0.84	0.22	12.9	0.18	3.75	0.7					0.7	0.18	1.5	15	350	2.4	2.4	Runoff from Basin Y1, collected by private 12" Nyoplast Drain Basins, Piped via 15" HDPE to DP4.1
	4.1								22.5	0.59	2.91	1.7								Combined flow in private 15" HDPE pipe @ DP4.1, inflow to proposed private sand filter

Notes:

Street and Pipe C^*A values are determined by Q/I using the catchment's intensity value.

All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

Grey-boxes indicate no change from the approved "Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2" by JR Engineering dated 6/16/21.

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Homestead at Sterling Ranch Filing No. 2
Location: Colorado Springs
Design Storm: 100-Year

Project Name: Homestead at Sterling Ranch Filing No. 2
Project No.: 2000-5188.
Calculated By: GAG
Checked By:
Date: 12/20/22

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	(in/hr)	Q (cfs)	t_c (min)	C*A (ac)	(in/hr)	Q (cfs)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	1	X1	0.91	0.46	12.9	0.42	6.29	2.6					2.6	0.42	0.5	12	250	2.3	1.8	Runoff from Basin X1, collected by private 12" Nyoplast Drain Basins, Piped via 12" HDPE to pvt. sand filter @ DP1.1
	2	X2	1.04	0.46	12.9	0.48	6.29	3.0					3.0	0.48	1.0	12	950	3.1	5.1	Runoff from Basin X2, collected by private 12" Nyoplast Drain Basins, Piped via 12" HDPE to DP3.1
	3	W1	0.84	0.46	16.8	0.39	5.62	2.2					2.2	0.39	1.0	12	250	2.9	1.5	Runoff from Basin W1, collected by private 12" Nyoplast Drain Basins, Piped via 12" HDPE to DP3.1
	3.1								19.4	0.87	5.26	4.6								Combined flow in private 12" HDPE pipe @ DP3.1, piped to private sand filter @ DP-4.1
	4	Y1	0.84	0.46	12.9	0.39	6.29	2.5					2.5	0.39	1.5	15	350	3.3	1.8	Runoff from Basin Y1, collected by private 15" Nyoplast Drain Basins, Piped via 12" HDPE to DP4.1
	4.1								21.2	1.26	5.04	6.4								Combined flow in private 15" HDPE pipe @ DP4.1, inflow to proposed private sand filter

Notes:

Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.

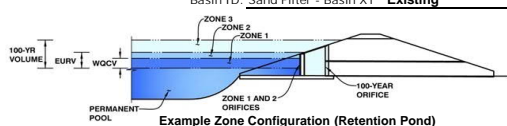
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

Grey-boxes indicate no change from the approved "Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2" by JR Engineering dated 6/16/21.

MHFD-Detention, Version 4.04 (February 2021)

Project: Homestead at Sterling Ranch Filing No. 2

Basin ID: Sand Filter - Basin X1 - Existing



Watershed Information

Selected BMP Type =	SF	
Watershed Area =	0.91	acres
Watershed Length =	450	ft
Watershed Length to Centroid =	225	ft
Watershed Slope =	0.020	ft/ft
Watershed Imperviousness =	25.00%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Group C/D =	0.0%	percent
Target WQCV Drain Time =	12.0	hours
Location for 1-hr Rainfall Depth =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WQCV) =	0.008	acre-feet
Excess Urban Runoff Volume (EURV) =	0.023	acre-feet
2-yr Runoff Volume ($P1 = 1.19$ in.) =	0.024	acre-feet
5-yr Runoff Volume ($P1 = 1.5$ in.) =	0.041	acre-feet
10-yr Runoff Volume ($P1 = 1.75$ in.) =	0.056	acre-feet
25-yr Runoff Volume ($P1 = 2$ in.) =	0.081	acre-feet
50-yr Runoff Volume ($P1 = 2.25$ in.) =	0.099	acre-feet
100-yr Runoff Volume ($P1 = 2.52$ in.) =	0.121	acre-feet
500-yr Runoff Volume ($P1 = 3.14$ in.) =	0.174	acre-feet
Approximate 2-yr Detention Volume =	0.016	acre-feet
Approximate 5-yr Detention Volume =	0.023	acre-feet
Approximate 10-yr Detention Volume =	0.036	acre-feet
Approximate 25-yr Detention Volume =	0.043	acre-feet
Approximate 50-yr Detention Volume =	0.045	acre-feet
Approximate 100-yr Detention Volume =	0.054	acre-feet

Define Zones and Basin Geometry

Zone 1 Volume (WOCV) =	0.008	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.015	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.031	acre-feet
Total Detention Basin Volume =	0.054	acre-feet
Initial Surcharge Volume (ISV) =	N/A	ft ³
Initial Surcharge Depth (ISD) =	N/A	ft
Total Available Detention Depth (H_{total}) =	N/A	ft
Depth of Trickle Channel (H_{TC}) =	N/A	ft
Slope of Trickle Channel (S_{TC}) =	N/A	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	ft:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

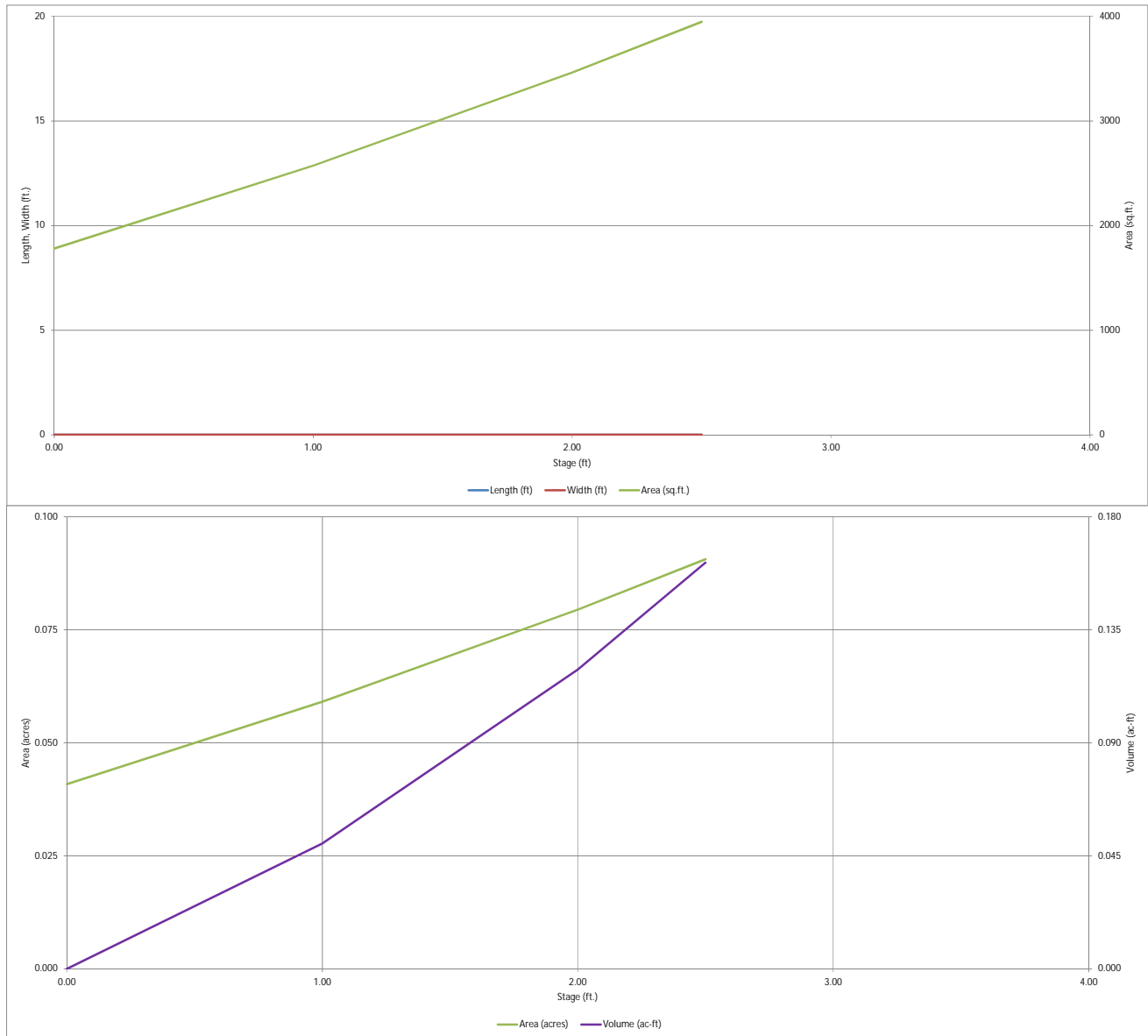
Initial Surcharge Area (A_{ISV})	=	user	ft^2
Surcharge Volume Length (L_{ISV})	=	user	
Surcharge Volume Width (W_{ISV})	=	user	
Depth of Basin Floor (H_{1LOOR})	=	user	ft
Length of Basin Floor (L_{1LOOR})	=	user	
Width of Basin Floor (W_{1LOOR})	=	user	
Area of Basin Floor (A_{1LOOR})	=	user	ft^2
Volume of Basin Floor (V_{1LOOR})	=	user	ft^3
Depth of Main Basin (H_{MAIN})	=	user	ft
Length of Main Basin (L_{MAIN})	=	user	
Width of Main Basin (W_{MAIN})	=	user	
Area of Main Basin (A_{MAIN})	=	user	ft^2
Volume of Main Basin (V_{MAIN})	=	user	ft^3
Calculated Total Basin Volume (V_{TOTAL})	=	user	acre-feet

Depth Increment = f

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DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

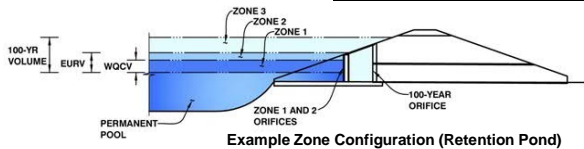


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: Homestead at Sterling Ranch Filing No. 2

Basin ID: Sand Filter - Basin X1 - Existing



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	0.20	0.008	Filtration Media
Zone 2 (EURV)	0.51	0.015	Rectangular Orifice
Zone 3 (100-year)	1.08	0.031	Weir&Pipe (Restrict)
Total (all zones)		0.054	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = 2.10 ft (distance below the filtration media surface)

Underdrain Orifice Diameter = 0.46 inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = 0.0 ft²

Underdrain Orifice Centroid = 0.02 feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)

Depth at top of Zone using Orifice Plate = N/A ft (relative to basin bottom at Stage = 0 ft)

Orifice Plate: Orifice Vertical Spacing = N/A inches

Orifice Plate: Orifice Area per Row = N/A inches

Calculated Parameters for Plate

WQ Orifice Area per Row = N/A ft²

Elliptical Half-Width = N/A feet

Elliptical Slot Centroid = N/A feet

Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

Zone 2 Rectangular Not Selected

Invert of Vertical Orifice = 0.35 N/A ft (relative to basin bottom at Stage = 0 ft)

Depth at top of Zone using Vertical Orifice = 0.51 N/A ft (relative to basin bottom at Stage = 0 ft)

Vertical Orifice Height = 2.00 N/A inches

Vertical Orifice Width = 4.00 inches

Calculated Parameters for Vertical Orifice

Zone 2 Rectangular Not Selected

Vertical Orifice Area = 0.06 N/A ft²

Vertical Orifice Centroid = 0.08 N/A feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe)

Zone 3 Weir Not Selected

Overflow Weir Front Edge Height, H_o = 0.75 N/A ft (relative to basin bottom at Stage = 0 ft)

Overflow Weir Front Edge Length = 2.21 N/A feet

Overflow Weir Grate Slope = 0.00 N/A H:V

Horiz. Length of Weir Sides = 2.21 N/A feet

Overflow Grate Type = Type C Grate N/A

Debris Clogging % = 50% N/A %

Height of Grate Upper Edge, H_i = 0.75 N/A feet

Overflow Weir Slope Length = 2.21 N/A feet

Grate Open Area / 100-yr Orifice Area = 28.68 N/A

Overflow Grate Open Area w/o Debris = 3.40 N/A ft²

Overflow Grate Open Area w/ Debris = 1.70 N/A ft²

Calculated Parameters for Overflow Weir

Zone 3 Weir Not Selected

Height of Grate Upper Edge, H_i = 0.75 N/A feet

Overflow Weir Slope Length = 2.21 N/A feet

Grate Open Area / 100-yr Orifice Area = 28.68 N/A

Overflow Grate Open Area w/o Debris = 3.40 N/A ft²

Overflow Grate Open Area w/ Debris = 1.70 N/A ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Zone 3 Restrictor Not Selected

Depth to Invert of Outlet Pipe = 2.10 N/A ft (distance below basin bottom at Stage = 0 ft)

Outlet Pipe Diameter = 12.00 N/A inches

Restrictor Plate Height Above Pipe Invert = 2.50 inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

Zone 3 Restrictor Not Selected

Outlet Orifice Area = 0.12 N/A ft²

Outlet Orifice Centroid = 0.12 N/A feet

Half-Central Angle of Restrictor Plate on Pipe = 0.95 N/A radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 2.50 ft (relative to basin bottom at Stage = 0 ft)

Spillway Crest Length = 5.00 feet

Spillway End Slopes = 4.00 H:V

Freeboard above Max Water Surface = 1.50 feet

Spillway Design Flow Depth = 0.14 feet

Stage at Top of Freeboard = 4.14 feet

Basin Area at Top of Freeboard = 0.09 acres

Basin Volume at Top of Freeboard = 0.16 acre-ft

Calculated Parameters for Spillway

Spillway Design Flow Depth = 0.14 feet

Stage at Top of Freeboard = 4.14 feet

Basin Area at Top of Freeboard = 0.09 acres

Basin Volume at Top of Freeboard = 0.16 acre-ft

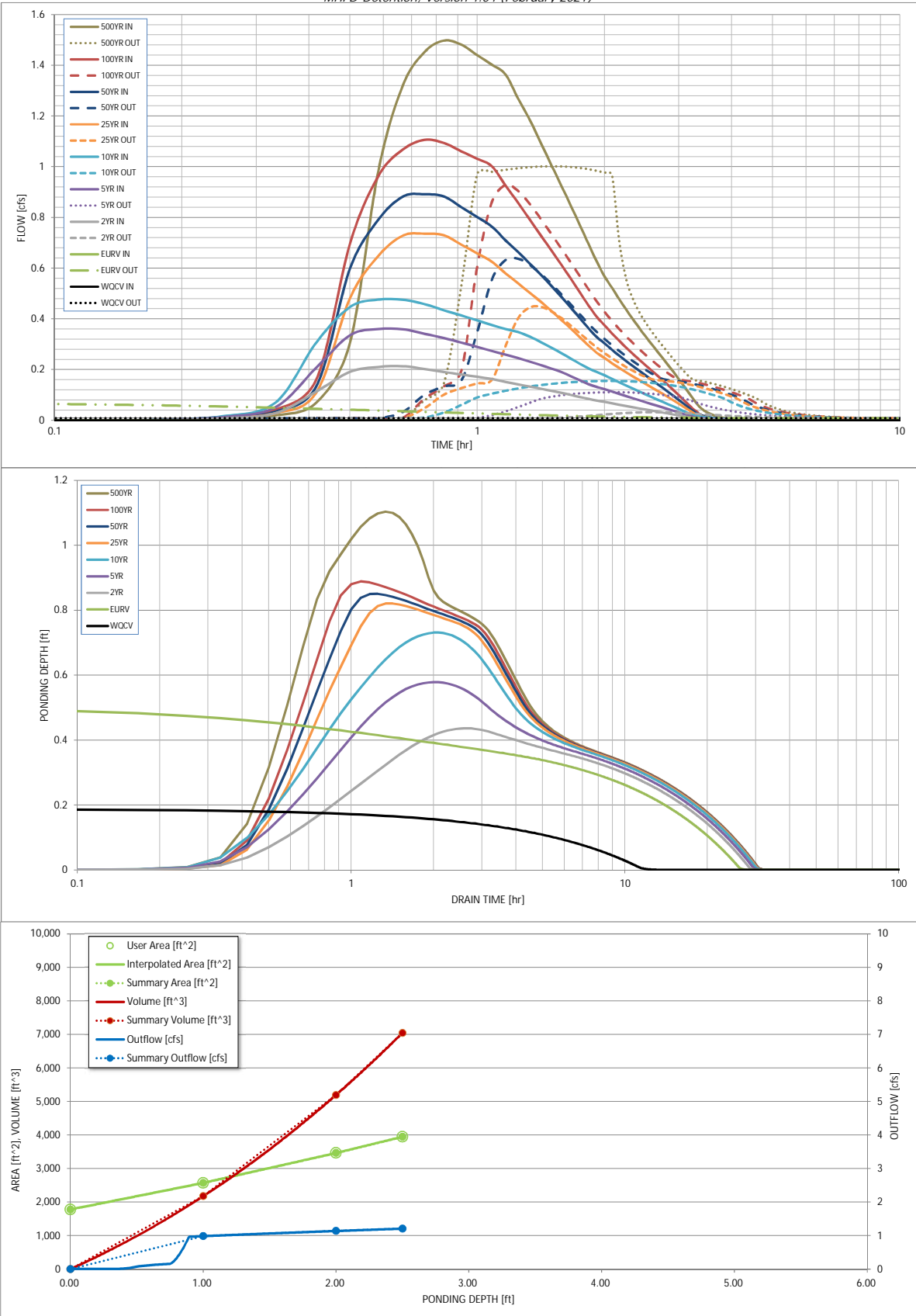
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.14
One-Hour Rainfall Depth (in)	N/A	N/A	0.008	0.023	0.024	0.041	0.056	0.081	0.099
CUHP Runoff Volume (acre-ft)	N/A	N/A	0.008	0.023	0.024	0.041	0.056	0.081	0.099
Inflow Hydrograph Volume (acre-ft)	N/A	N/A	0.008	0.023	0.024	0.041	0.056	0.081	0.099
CUHP Predevelopment Peak Q (cfs)	N/A	N/A	0.1	0.2	0.3	0.6	0.7	0.9	1.2
OPTIONAL Override Predevelopment Peak Q (cfs)	N/A	N/A	0.1	0.2	0.3	0.6	0.7	0.9	1.2
Predevelopment Unit Peak Flow, q (cfs/acre)	N/A	N/A	0.08	0.22	0.33	0.61	0.76	0.97	1.36
Peak Inflow Q (cfs)	N/A	N/A	0.2	0.4	0.5	0.7	0.9	1.1	1.5
Peak Outflow Q (cfs)	0.0	0.1	0.0	0.1	0.2	0.4	0.6	0.9	1.0
Ratio Peak Outflow to Predevelopment Q	N/A	N/A	N/A	0.6	0.5	0.8	0.9	1.0	0.8
Structure Controlling Flow	Filtration Media	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1
Max Velocity through Grate 1 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	0.1	0.1	0.2
Max Velocity through Grate 2 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours)	12	26	28	28	28	27	27	26	24
Time to Drain 99% of Inflow Volume (hours)	12	26	29	29	30	30	30	29	29
Maximum Ponding Depth (ft)	0.19	0.51	0.44	0.58	0.73	0.82	0.85	0.89	1.10
Area at Maximum Ponding Depth (acres)	0.04	0.05	0.05	0.05	0.05	0.06	0.06	0.06	0.06
Maximum Volume Stored (acre-ft)	0.008	0.023	0.019	0.026	0.035	0.040	0.041	0.043	0.056

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

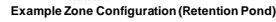
Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:15:00	0.00	0.00	0.01	0.01	0.02	0.01	0.01	0.01	0.02
	0:20:00	0.00	0.00	0.03	0.05	0.07	0.03	0.03	0.04	0.07
	0:25:00	0.00	0.00	0.11	0.21	0.31	0.11	0.14	0.16	0.31
	0:30:00	0.00	0.00	0.19	0.34	0.45	0.48	0.60	0.70	0.98
	0:35:00	0.00	0.00	0.21	0.36	0.48	0.65	0.79	0.96	1.32
	0:40:00	0.00	0.00	0.21	0.36	0.48	0.73	0.88	1.07	1.45
	0:45:00	0.00	0.00	0.20	0.34	0.46	0.74	0.89	1.11	1.50
	0:50:00	0.00	0.00	0.19	0.32	0.43	0.73	0.88	1.09	1.48
	0:55:00	0.00	0.00	0.18	0.31	0.41	0.69	0.84	1.06	1.44
	1:00:00	0.00	0.00	0.17	0.29	0.39	0.66	0.80	1.03	1.40
	1:05:00	0.00	0.00	0.16	0.27	0.38	0.63	0.76	1.00	1.36
	1:10:00	0.00	0.00	0.15	0.26	0.36	0.58	0.71	0.93	1.27
	1:15:00	0.00	0.00	0.14	0.25	0.35	0.54	0.66	0.86	1.18
	1:20:00	0.00	0.00	0.13	0.23	0.33	0.50	0.62	0.79	1.09
	1:25:00	0.00	0.00	0.12	0.22	0.31	0.47	0.57	0.73	1.00
	1:30:00	0.00	0.00	0.12	0.20	0.29	0.43	0.53	0.67	0.92
	1:35:00	0.00	0.00	0.11	0.19	0.26	0.40	0.49	0.61	0.85
	1:40:00	0.00	0.00	0.10	0.17	0.24	0.36	0.44	0.56	0.77
	1:45:00	0.00	0.00	0.09	0.16	0.22	0.33	0.40	0.50	0.70
	1:50:00	0.00	0.00	0.08	0.14	0.20	0.30	0.36	0.45	0.63
	1:55:00	0.00	0.00	0.08	0.13	0.19	0.27	0.33	0.41	0.57
	2:00:00	0.00	0.00	0.07	0.12	0.18	0.25	0.30	0.38	0.52
	2:05:00	0.00	0.00	0.07	0.11	0.16	0.23	0.28	0.34	0.48
	2:10:00	0.00	0.00	0.06	0.10	0.15	0.21	0.26	0.31	0.44
	2:15:00	0.00	0.00	0.06	0.10	0.14	0.19	0.23	0.29	0.40
	2:20:00	0.00	0.00	0.05	0.09	0.13	0.17	0.21	0.26	0.36
	2:25:00	0.00	0.00	0.05	0.08	0.11	0.16	0.20	0.24	0.33
	2:30:00	0.00	0.00	0.04	0.07	0.10	0.14	0.18	0.22	0.30
	2:35:00	0.00	0.00	0.04	0.06	0.09	0.13	0.16	0.20	0.27
	2:40:00	0.00	0.00	0.03	0.06	0.08	0.12	0.14	0.18	0.24
	2:45:00	0.00	0.00	0.03	0.05	0.07	0.10	0.13	0.16	0.22
	2:50:00	0.00	0.00	0.03	0.04	0.06	0.09	0.11	0.14	0.19
	2:55:00	0.00	0.00	0.02	0.04	0.05	0.08	0.10	0.12	0.16
	3:00:00	0.00	0.00	0.02	0.03	0.04	0.07	0.08	0.10	0.14
	3:05:00	0.00	0.00	0.02	0.02	0.04	0.05	0.06	0.08	0.11
	3:10:00	0.00	0.00	0.01	0.02	0.03	0.04	0.05	0.06	0.08
	3:15:00	0.00	0.00	0.01	0.01	0.02	0.03	0.04	0.04	0.06
	3:20:00	0.00	0.00	0.01	0.01	0.02	0.02	0.03	0.03	0.04
	3:25:00	0.00	0.00	0.01	0.01	0.01	0.01	0.02	0.02	0.03
	3:30:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.02	0.02
	3:35:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.02
	3:40:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01
	3:45:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01
	3:50:00	0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.00	0.01
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

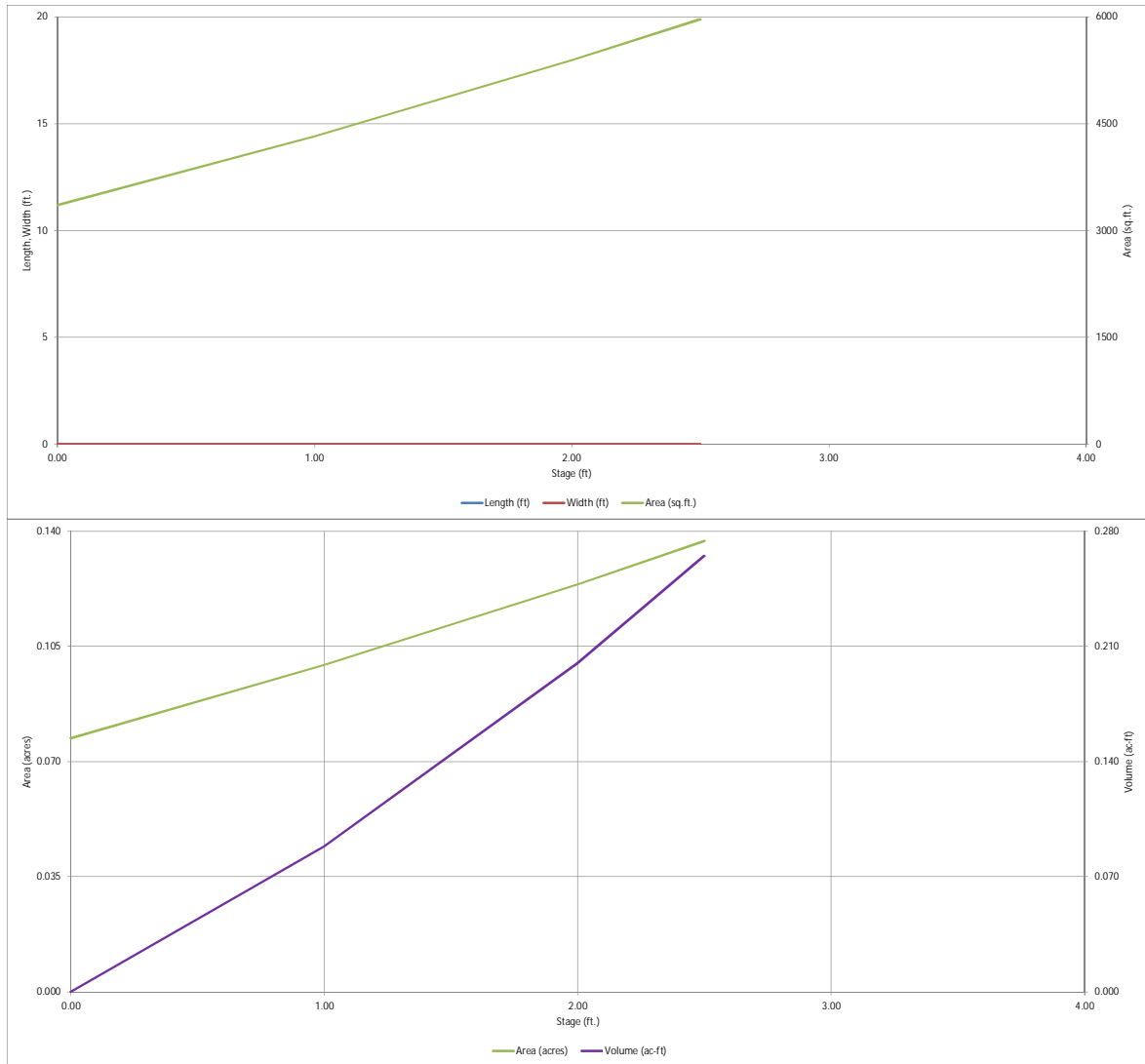
MHFD-Detention, Version 4.04 (February 2021)

Basin ID: Sand Filter - Basin Y1, W1, X2 - Existing

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DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

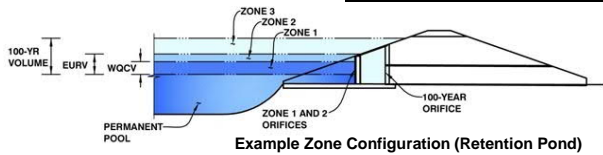


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: Homestead at Sterling Ranch Filling No. 2

Basin ID: Sand Filter - Basin Y1, W1, X2 - Existing



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	0.31	0.024	Filtration Media
Zone 2 (EURV)	0.80	0.044	Rectangular Orifice
Zone 3 (100-year)	1.69	0.093	Weir&Pipe (Restrict)
Total (all zones)		0.162	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = 2.10 ft (distance below the filtration media surface)

Underdrain Orifice Diameter = 0.80 inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = 0.0 ft²

Underdrain Orifice Centroid = 0.03 feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)

Depth at top of Zone using Orifice Plate = N/A ft (relative to basin bottom at Stage = 0 ft)

Orifice Plate: Orifice Vertical Spacing = N/A inches

Orifice Plate: Orifice Area per Row = N/A inches

Calculated Parameters for Plate

WQ Orifice Area per Row = N/A ft²

Elliptical Half-Width = N/A feet

Elliptical Slot Centroid = N/A feet

Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = 0.33 ft (relative to basin bottom at Stage = 0 ft)

Depth at top of Zone using Vertical Orifice = 0.80 ft (relative to basin bottom at Stage = 0 ft)

Vertical Orifice Height = 2.00 inches

Vertical Orifice Width = 4.00 inches

Calculated Parameters for Vertical Orifice

Zone 2 Rectangular = 0.06 ft²

Zone 2 Rectangular = 0.08 feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

Overflow Weir Front Edge Height, H_o = 1.00 ft (relative to basin bottom at Stage = 0 ft)

Overflow Weir Front Edge Length = 2.21 feet

Overflow Weir Grate Slope = 0.00 H:V

Horiz. Length of Weir Sides = 2.21 feet

Overflow Grate Type = Type C Grate

Debris Clogging % = 50%

Calculated Parameters for Overflow Weir

Height of Grate Upper Edge, H_u = 1.00 feet

Overflow Weir Slope Length = 2.21 feet

Grate Open Area / 100-yr Orifice Area = 10.98

Overflow Grate Open Area w/o Debris = 3.40 ft²

Overflow Grate Open Area w/ Debris = 1.70 ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = 2.10 ft (distance below basin bottom at Stage = 0 ft)

Outlet Pipe Diameter = 15.00 inches

Restrictor Plate Height Above Pipe Invert = 4.50 inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

Outlet Orifice Area = 0.31 ft²

Outlet Orifice Centroid = 0.22 feet

Half-Central Angle of Restrictor Plate on Pipe = 1.16 radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 2.00 ft (relative to basin bottom at Stage = 0 ft)

Spillway Crest Length = 10.00 feet

Spillway End Slopes = 4.00 H:V

Freeboard above Max Water Surface = 1.50 feet

Calculated Parameters for Spillway

Spillway Design Flow Depth = 0.22 feet

Stage at Top of Freeboard = 3.72 feet

Basin Area at Top of Freeboard = 0.14 acres

Basin Volume at Top of Freeboard = 0.26 acre-ft

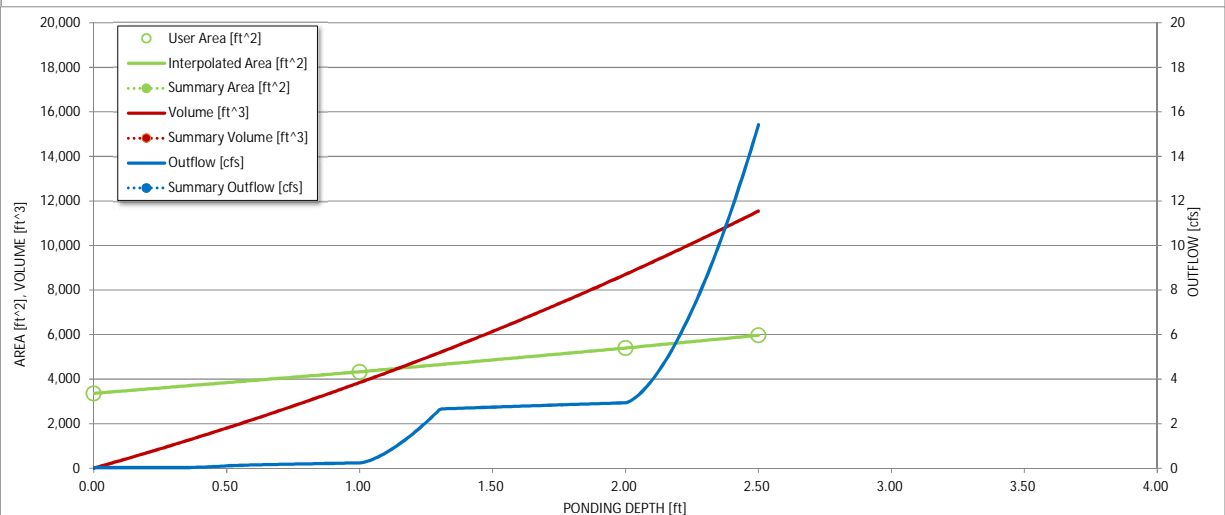
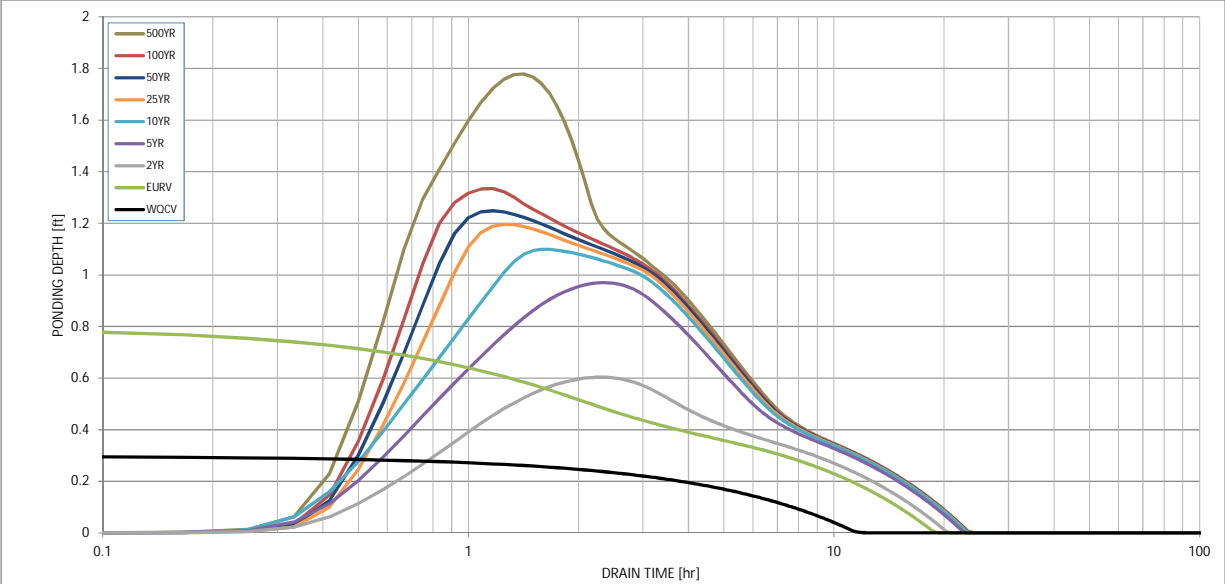
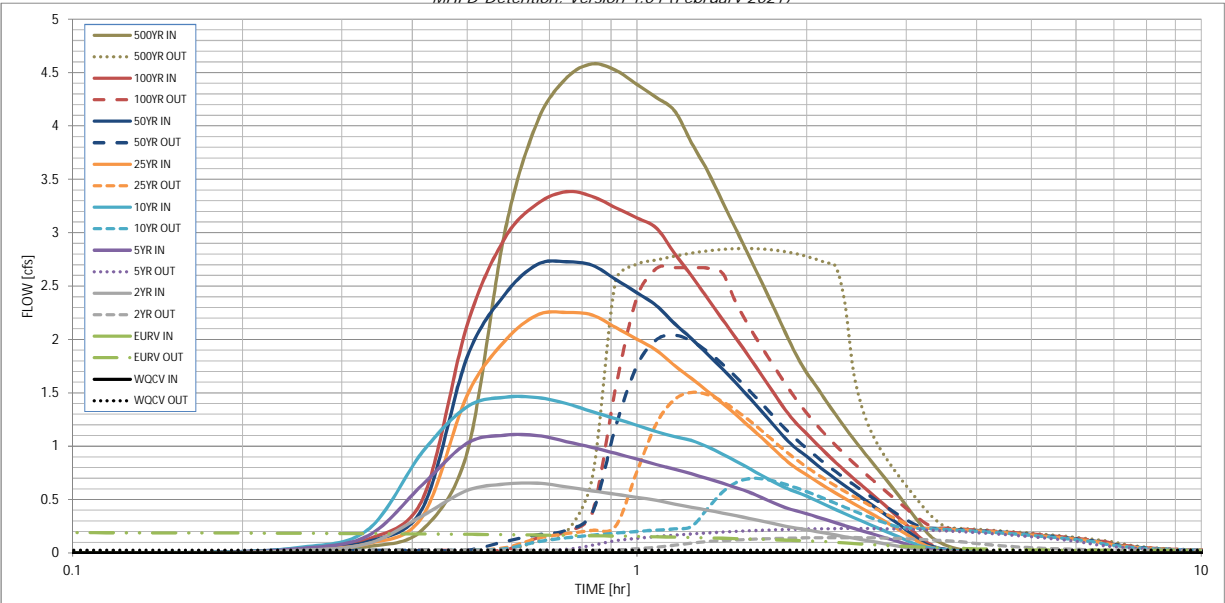
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.14
One-Hour Rainfall Depth (in)	0.024	0.069	0.071	0.122	0.169	0.243	0.297	0.372	0.512
CUHP Runoff Volume (acre-ft)	N/A	N/A	0.071	0.122	0.169	0.243	0.297	0.372	0.512
Inflow Hydrograph Volume (acre-ft)	N/A	N/A	0.2	0.6	0.9	1.7	2.1	2.7	3.8
CUHP Predevelopment Peak Q (cfs)	N/A	N/A							
OPTIONAL Override Predevelopment Peak Q (cfs)	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre)	N/A	N/A	0.08	0.22	0.34	0.62	0.78	1.00	1.40
Peak Inflow Q (cfs)	N/A	N/A	0.7	1.1	1.5	2.3	2.7	3.4	4.6
Peak Outflow Q (cfs)	0.0	0.2	0.1	0.2	0.7	1.5	2.0	2.7	2.9
Ratio Peak Outflow to Predevelopment Q	N/A	N/A	N/A	0.4	0.8	0.9	1.0	1.0	0.7
Structure Controlling Flow	Filtration Media	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1
Max Velocity through Grate 1 (fps)	N/A	N/A	N/A	N/A	0.1	0.4	0.5	0.7	0.7
Max Velocity through Grate 2 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours)	11	18	20	21	21	20	19	18	16
Time to Drain 99% of Inflow Volume (hours)	12	19	20	22	22	22	22	22	21
Maximum Ponding Depth (ft)	0.30	0.81	0.60	0.97	1.10	1.20	1.25	1.33	1.78
Area at Maximum Ponding Depth (acres)	0.08	0.10	0.09	0.10	0.10	0.10	0.11	0.11	0.12
Maximum Volume Stored (acre-ft)	0.024	0.070	0.050	0.085	0.097	0.108	0.113	0.122	0.172

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

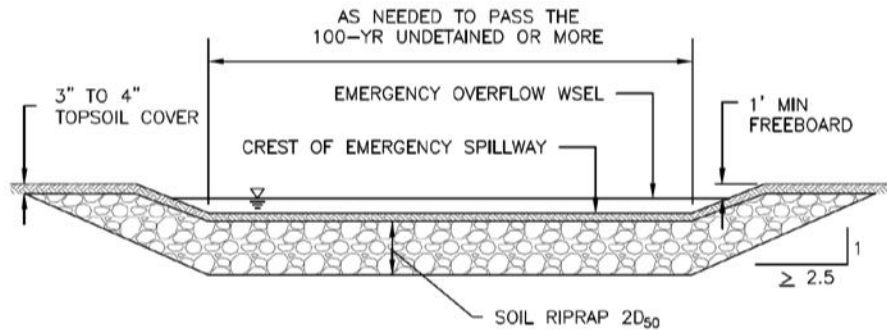
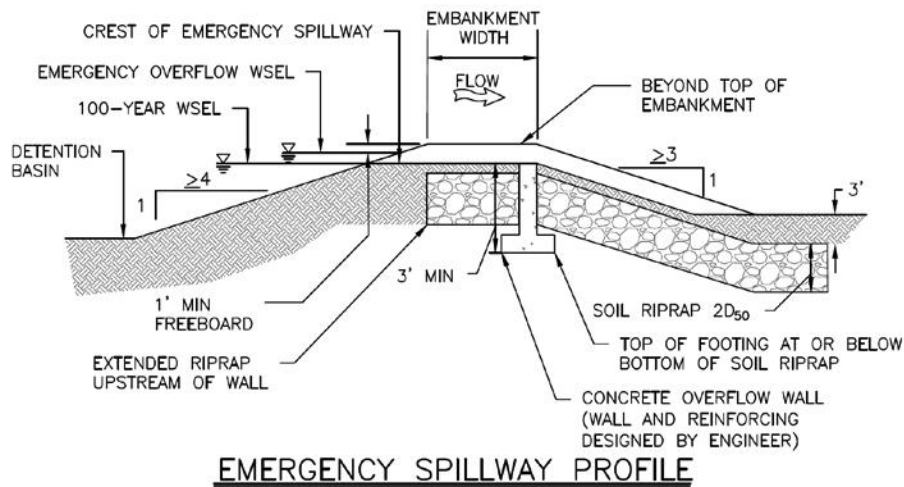
DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	0:15:00	0.00	0.00	0.02	0.04	0.05	0.03	0.04	0.04	0.06
	0:20:00	0.00	0.00	0.09	0.16	0.22	0.09	0.11	0.13	0.22
	0:25:00	0.00	0.00	0.35	0.64	0.94	0.34	0.42	0.50	0.94
	0:30:00	0.00	0.00	0.59	1.03	1.37	1.47	1.83	2.13	3.01
	0:35:00	0.00	0.00	0.64	1.10	1.46	1.99	2.42	2.95	4.04
	0:40:00	0.00	0.00	0.65	1.10	1.46	2.23	2.70	3.27	4.45
	0:45:00	0.00	0.00	0.62	1.04	1.40	2.25	2.73	3.38	4.58
	0:50:00	0.00	0.00	0.58	0.99	1.32	2.23	2.70	3.34	4.52
	0:55:00	0.00	0.00	0.55	0.93	1.26	2.11	2.56	3.23	4.39
	1:00:00	0.00	0.00	0.52	0.88	1.20	2.00	2.44	3.14	4.27
	1:05:00	0.00	0.00	0.49	0.83	1.14	1.90	2.32	3.04	4.14
	1:10:00	0.00	0.00	0.46	0.78	1.09	1.75	2.15	2.80	3.84
	1:15:00	0.00	0.00	0.43	0.74	1.05	1.63	2.01	2.60	3.58
	1:20:00	0.00	0.00	0.40	0.70	0.99	1.52	1.86	2.39	3.30
	1:25:00	0.00	0.00	0.37	0.65	0.93	1.41	1.73	2.19	3.03
	1:30:00	0.00	0.00	0.35	0.61	0.86	1.29	1.59	2.01	2.77
	1:35:00	0.00	0.00	0.32	0.57	0.79	1.18	1.45	1.83	2.53
	1:40:00	0.00	0.00	0.30	0.52	0.72	1.08	1.32	1.66	2.29
	1:45:00	0.00	0.00	0.27	0.47	0.66	0.97	1.19	1.49	2.06
	1:50:00	0.00	0.00	0.25	0.42	0.61	0.87	1.07	1.34	1.85
	1:55:00	0.00	0.00	0.23	0.39	0.57	0.79	0.98	1.21	1.69
	2:00:00	0.00	0.00	0.22	0.37	0.53	0.73	0.90	1.12	1.56
	2:05:00	0.00	0.00	0.20	0.34	0.49	0.67	0.83	1.02	1.42
	2:10:00	0.00	0.00	0.18	0.31	0.45	0.61	0.76	0.93	1.30
	2:15:00	0.00	0.00	0.17	0.28	0.41	0.56	0.69	0.85	1.18
	2:20:00	0.00	0.00	0.15	0.26	0.37	0.51	0.63	0.77	1.07
	2:25:00	0.00	0.00	0.14	0.23	0.33	0.47	0.57	0.70	0.97
	2:30:00	0.00	0.00	0.12	0.21	0.30	0.42	0.52	0.64	0.88
	2:35:00	0.00	0.00	0.11	0.19	0.27	0.38	0.47	0.57	0.79
	2:40:00	0.00	0.00	0.10	0.16	0.24	0.34	0.41	0.51	0.70
	2:45:00	0.00	0.00	0.09	0.14	0.21	0.30	0.36	0.45	0.62
	2:50:00	0.00	0.00	0.07	0.12	0.18	0.26	0.31	0.39	0.53
	2:55:00	0.00	0.00	0.06	0.10	0.15	0.22	0.26	0.33	0.45
	3:00:00	0.00	0.00	0.05	0.08	0.12	0.18	0.22	0.26	0.36
	3:05:00	0.00	0.00	0.04	0.06	0.09	0.14	0.17	0.21	0.28
	3:10:00	0.00	0.00	0.03	0.05	0.07	0.10	0.12	0.15	0.20
	3:15:00	0.00	0.00	0.02	0.03	0.05	0.07	0.08	0.10	0.14
	3:20:00	0.00	0.00	0.02	0.03	0.04	0.05	0.06	0.07	0.11
	3:25:00	0.00	0.00	0.01	0.02	0.04	0.04	0.05	0.05	0.08
	3:30:00	0.00	0.00	0.01	0.02	0.03	0.03	0.04	0.04	0.06
	3:35:00	0.00	0.00	0.01	0.02	0.03	0.02	0.03	0.03	0.04
	3:40:00	0.00	0.00	0.01	0.01	0.02	0.02	0.02	0.02	0.03
	3:45:00	0.00	0.00	0.01	0.01	0.02	0.01	0.02	0.02	0.02
	3:50:00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.02
	3:55:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01
	4:00:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01
	4:05:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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CHOOSE TYPE VL

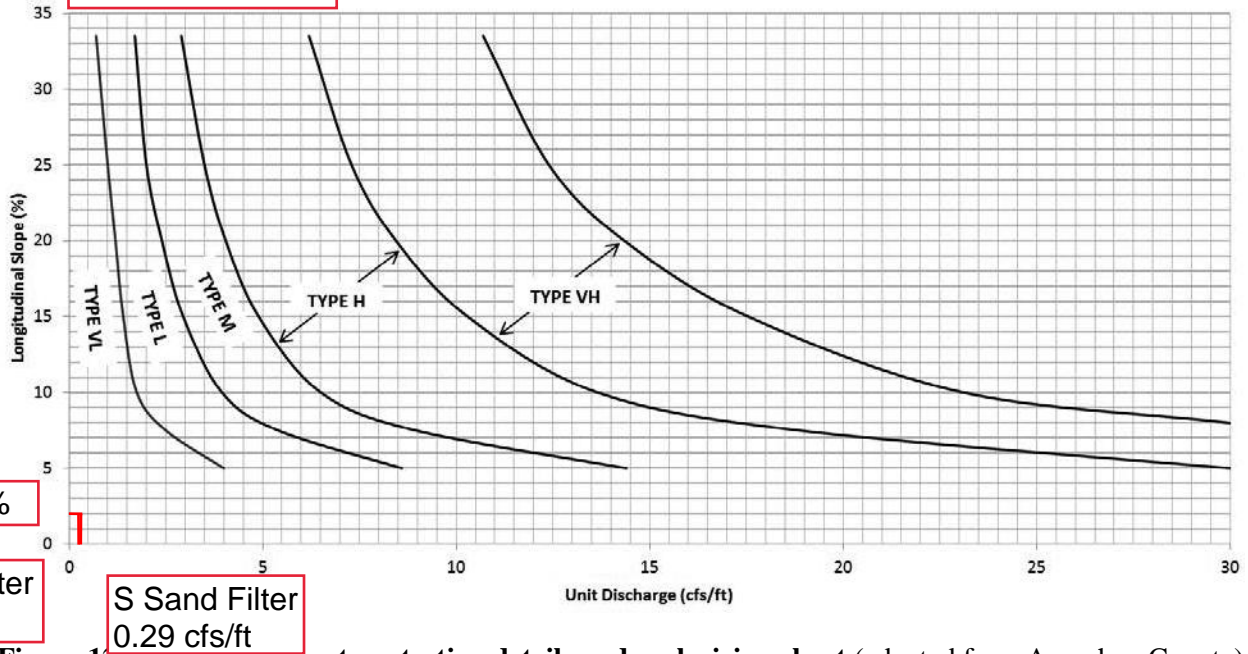


Figure 12-21. Embankment protection details and rock sizing chart (adapted from Arapahoe County)

HOMESTEAD AT STERLING RANCH FILING NO. 2

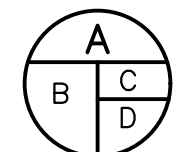
COUNTY OF EL PASO, STATE OF COLORADO

PROPOSED DRAINAGE MAP (ADDENDUM)

BASIN SUMMARY TABLE							
Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q _s (cfs)	Q ₁₀₀ (cfs)
X1	0.91	25%	0.22	0.46	12.9	0.7	2.6
X2	1.04	25%	0.22	0.46	12.9	0.9	3.0
W1	0.84	25%	0.22	0.46	16.8	0.6	2.2
Y1	0.84	25%	0.22	0.46	12.9	0.7	2.5
Grey-boxes indicate no change from the approved "Addendum to the Final Drainage Report for Homestead at Sterling Ranch Filing No. 2" by JR Engineering dated 6/16/21.							

DESIGN POINT		
DP	Q _S	Q ₁₀₀
	Total	Total
1	0.7	2.6
2	0.9	2.2
3	0.6	2.5
3.1	1.3	4.6
4	0.7	2.5
4.1	1.7	6.4

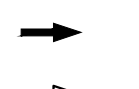
LEGEND



BASIN ID
A: SUB-BASIN DESIGNATION
B: AREA (AC)
C: 5 YEAR STORM COEFFICIENT
D: 100 YEAR STORM COEFFICIENT



DRAINAGE DISCHARGE
DESIGN POINT



PROPOSED FLOW DIRECTION



HISTORIC FLOW DIRECTION



EXISTING DRAINAGE BASIN



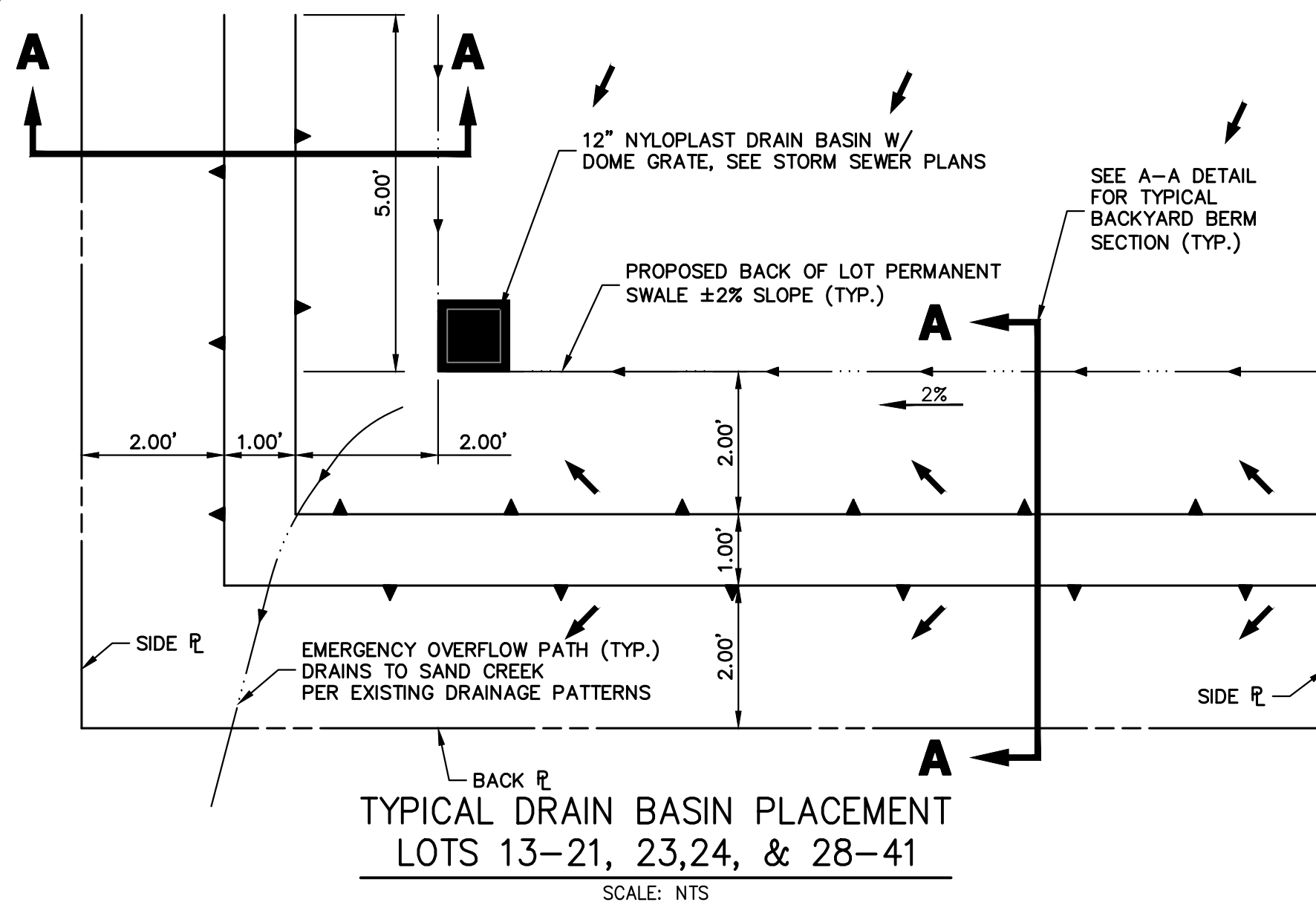
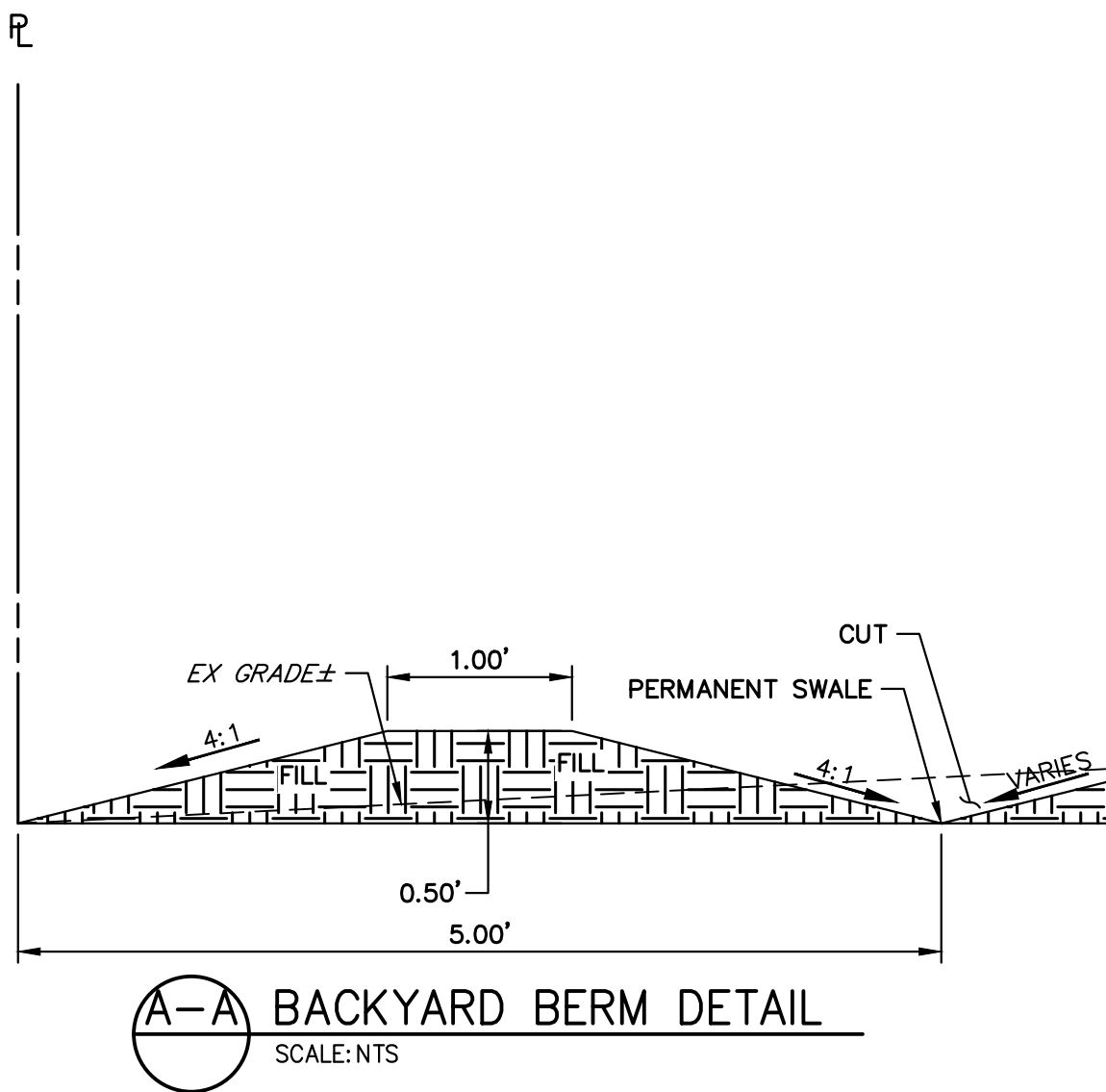
PROPOSED DRAINAGE BASIN



EXISTING STORM SEWER



PROPOSED STORM SEWER



PROPOSED DRAINAGE MAP
(ADDENDUM)
HOMESTEAD AT STERLING RANCH FILING NO. 2
JOB NO. 25188.29
04/17/2023
SHEET 1 OF 1



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Fort Collins 970-491-9888 • www.jrengineering.com