



ENTECH
ENGINEERING, INC.

505 ELKTON DRIVE
COLORADO SPRINGS, CO 80907
PHONE (719) 531-5599
FAX (719) 531-5238

August 9, 2023

El Paso County
3460 Marksheffel Road
Colorado Springs, Colorado 80922

Attn: Gilbert LaForce
Engineering

Subject: Utility Trench Backfill – Storm Sewer
Within the Public Streets Rights of Ways of:

Forest Lakes, Filing No. 7
El Paso County, Colorado
Job No. 220622

Dear Mr. LaForce:

Representatives of Entech Engineering, Inc. periodically visited the site and performed field density testing during the subject construction processes. Testing was performed in general compliance with the frequency indicated in the El Paso County Standard Specifications. Results of field density tests have previously been provided. Field density tests were taken by Entech Engineering, Inc. personnel with test results presented in reports dated July 25 and October 12, 2022. The results indicate densities which meet those specified in the El Paso County Standard Specifications.

If you have further questions regarding this letter, or the construction from a geotechnical point of view, please call.

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Daniel P. Stegman

DPS/lu

Entech Job No. 220622
AA Projects/2022/220622 - filing 7 - utb-storm

Reviewed by:

Austin M. Nossokoff, P.E.





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ENGINEERING, INC.

505 ELKTON DRIVE
COLORADO SPRINGS, CO 80907
PHONE (719) 531-5599
FAX (719) 531-5238

August 9, 2023

El Paso County
3460 Marksheffel Road
Colorado Springs, Colorado 80922

Attn: Gilbert LaForce
Engineering

Subject: Utility Trench Backfill, Curb and Gutter Sub-grade, CTS Preparation
and Placement Testing.
Within the Public Streets Rights of Ways of:

Forest Lakes, Filing No. 7
El Paso County, Colorado
Job No. 220622

Dear Mr. LaForce:

Representatives of Entech Engineering, Inc. periodically visited the site and performed field density testing during the subject construction processes. Testing was performed in general compliance with the frequency indicated in the El Paso County Standard Specifications. Results of field density tests have previously been provided. Field density tests were taken by Entech Engineering, Inc. personnel with test results presented in reports dated May 26 through November 21, 2022. Results generally indicate densities, which meet those specified in the El Paso County Standard Specifications.

Representatives of Entech Engineering, Inc. also periodically observed the curb and gutter subgrade and the CTS preparation processes for the street improvements. These materials appeared to be in general compliance with the specifications presented in the El Paso County Standard Specifications and appeared to be consistent with the materials assumed in the pavement design for the project.

If you have further questions regarding this letter, or the construction from a geotechnical point of view, please call.

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Daniel P. Stegman

DPS/lu

Entech Job Nos. 220622
AAprojects/2022/220622 - filing 7 - utb-cgs

Reviewed by:

Austin M. Nossokoff, P.E.





August 15, 2023

El Paso County
Development Services Division
2880 International Circle
Colorado Springs, CO 80910

RE: Forest Lakes Filing No. 7 – Public & Private Storm Sewer

The public & private storm drainage facilities for Forest Lakes Filing No. 7 consist of:

- (3) 5' Type-R Inlet
- (5) 10' Type-R Inlet
- (8) 15' Type-R Inlet
- (2) CDOT Type C Inlet
- (3) CDOT Type D Inlet
- (21) Type 1 Storm Manholes
- (7) Type 2 Storm Manhole
- (1) Pond Outlet Box
- (318 LF) 18" RCP Storm Drain
- (883 LF) 24" RCP Storm Drain
- (923 LF) 30" RCP Storm Drain
- (591 LF) 36" RCP Storm Drain
- (864 LF) 42" RCP Storm Drain
- (1403 LF) 48" RCP Storm Drain

The above listed storm was recently installed by Forest Lakes Residential Development, LLC and per the approved drainage report, the system drains to (1) permanent detention/water quality facility.

Classic Consulting Engineers & Surveyors has reviewed the attached letter from Entech Engineering, Inc. Based upon this information and information gathered during periodic site visits to the project during the installation of the storm sewer facilities, Classic Consulting Engineers & Surveyors is of the opinion that the storm drainage facilities have been constructed in general compliance with the approved design plans and specifications as filed with the County.

On behalf of Forest Lakes Residential Development LLC, Classic Consulting Engineers & Surveyors, LLC hereby requests probationary inspection of these facilities by County Engineering so that the warranty period can begin.





STATEMENT OF ENGINEER IN RESPONSIBLE CHARGE:

To the best of my knowledge, information and belief, the referenced public & private storm drainage facilities have been constructed in general compliance with the approved design plans and specifications as filed with the El Paso County.



Kyle R. Campbell, P.E.
Colorado No. 29794
Attachment: Entech Engineering, Inc. Certification

Seal & Signature of P.E.

ml/117570/Letters/CERT STORM – FILING 7.doc





August 15, 2023

El Paso County
Development Services Division
2880 International Circle
Colorado Springs, CO 80910

RE: Forest Lakes Filing No. 7 – Street Improvements

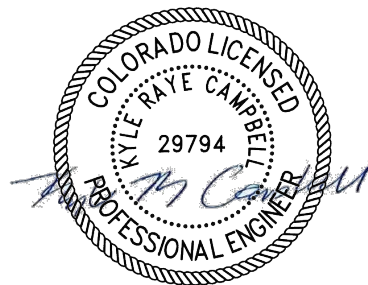
The street improvements for Forest Lakes Filing 7 consist of curb and gutter, paving, cross pans, sidewalk, and pedestrian ramps. Classic Consulting Engineers & Surveyors, LLC has reviewed the attached letter from Entech Engineering. Based upon this information and information gathered during periodic site visits to the project during the construction of the street improvements, Classic Consulting Engineers & Surveyors, LLC is of the opinion that the street improvements have been constructed in general compliance with the approved design plans and specifications prepared by Classic Consulting, Engineering, and Surveying, as filed with the County.

On behalf of Forest Lakes Residential Development, LLC, Classic Consulting Engineers & Surveyors, LLC hereby requests probationary inspection of these facilities by County Engineering so that the warranty period can begin.

STATEMENT OF ENGINEER IN RESPONSIBLE CHARGE:

To the best of my knowledge, information and belief, the referenced public street improvements have been constructed in general compliance with the approved design plans and specifications as filed with the El Paso County.

Kyle R. Campbell, P.E.
Colorado No. 29794



Attachment: Entech Engineering, Inc. Certification

ml/117570/letters/CERT STREET- FILING 7.doc





August 15, 2023

El Paso County
Planning and Community Development Department
2880 International Circle, Suite 110
Colorado Springs, CO 80910

ATTN: Inspections Staff

RE: Forest Lakes Filing 7 - Pond A

This letter is intended to provide documentation with the County Inspection Staff that the Pond A facility in Forest Lakes Filing 7 has been constructed per design. Classic Consulting has reviewed the final constructed facility and recently gathered survey as-builts confirming the appropriate size and design. Based upon this information and information gathered during periodic site visits to the project during construction, Classic Consulting is of the opinion that this stormwater BMP has been constructed in general compliance with the approved construction plans and specifications as filed with El Paso County.

STATEMENT OF ENGINEER IN RESPONSIBLE CHARGE:

To the best of my knowledge, information and belief, the referenced Forest Lakes Filing 7 - Pond A improvements have been constructed in general compliance with the approved design plans and specifications as filed with El Paso County.

Seal & Signature of P.E.



Kyle R. Campbell, P.E.
Colorado No. #29794
For an on behalf of Classic Consulting Engineers and Surveyors, LLC

mal/117570/PE CERT POND A-FILING 7.doc

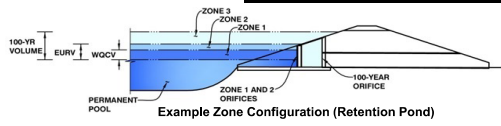


DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

Project: **FOREST LAKES FILING 7**

Basin ID: **POND A - ASBUILT**



Watershed Information

Selected BMP Type =	EDB
Watershed Area =	38.29 acres
Watershed Length =	2,063 ft
Watershed Length to Centroid =	800 ft
Watershed Slope =	0.028 ft/ft
Watershed Imperviousness =	43.20% percent
Percentage Hydrologic Soil Group A =	0.0% percent
Percentage Hydrologic Soil Group B =	100.0% percent
Percentage Hydrologic Soil Groups C/D =	0.0% percent
Target WQCV Drain Time =	40.0 hours
Location for 1-hr Rainfall Depths =	User Input

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Optional User Overrides

Water Quality Capture Volume (WQCV) =	0.601 acre-feet	acre-feet
Excess Urban Runoff Volume (EURV) =	1.748 acre-feet	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	1.696 acre-feet	1.19 inches
5-yr Runoff Volume (P1 = 1.5 in.) =	2.495 acre-feet	1.50 inches
10-yr Runoff Volume (P1 = 1.75 in.) =	3.208 acre-feet	1.75 inches
25-yr Runoff Volume (P1 = 2 in.) =	4.210 acre-feet	2.00 inches
50-yr Runoff Volume (P1 = 2.25 in.) =	5.000 acre-feet	2.25 inches
100-yr Runoff Volume (P1 = 2.52 in.) =	6.021 acre-feet	2.52 inches
500-yr Runoff Volume (P1 = 3.1 in.) =	7.930 acre-feet	3.10 inches
Approximate 2-yr Detention Volume =	1.299 acre-feet	
Approximate 5-yr Detention Volume =	1.802 acre-feet	
Approximate 10-yr Detention Volume =	2.446 acre-feet	
Approximate 25-yr Detention Volume =	2.713 acre-feet	
Approximate 50-yr Detention Volume =	2.843 acre-feet	
Approximate 100-yr Detention Volume =	3.229 acre-feet	

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.601 acre-feet
Zone 2 Volume (EURV - Zone 1) =	1.147 acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	1.481 acre-feet
Total Detention Basin Volume =	3.229 acre-feet
Initial Surcharge Volume (ISV) =	user ft ³
Initial Surcharge Depth (ISD) =	user ft
Total Available Detention Depth (H _{total}) =	user ft
Depth of Trickle Channel (H _{TC}) =	user ft
Slope of Trickle Channel (S _{TC}) =	user ft/ft
Slopes of Main Basin Sides (S _{main}) =	user H:V
Basin Length-to-Width Ratio (R _{L/W}) =	user
Initial Surcharge Area (A _{ISV}) =	user ft ²
Surcharge Volume Length (L _{ISV}) =	user ft
Surcharge Volume Width (W _{ISV}) =	user ft
Depth of Basin Floor (H _{FLOOR}) =	user ft
Length of Basin Floor (L _{FLOOR}) =	user ft
Width of Basin Floor (W _{FLOOR}) =	user ft
Area of Basin Floor (A _{FLOOR}) =	user ft ²
Volume of Basin Floor (V _{FLOOR}) =	user ft ³
Depth of Main Basin (H _{MAIN}) =	user ft
Length of Main Basin (L _{MAIN}) =	user ft
Width of Main Basin (W _{MAIN}) =	user ft
Area of Main Basin (A _{MAIN}) =	user ft ²
Volume of Main Basin (V _{MAIN}) =	user ft ³
Calculated Total Basin Volume (V _{total}) =	user acre-feet

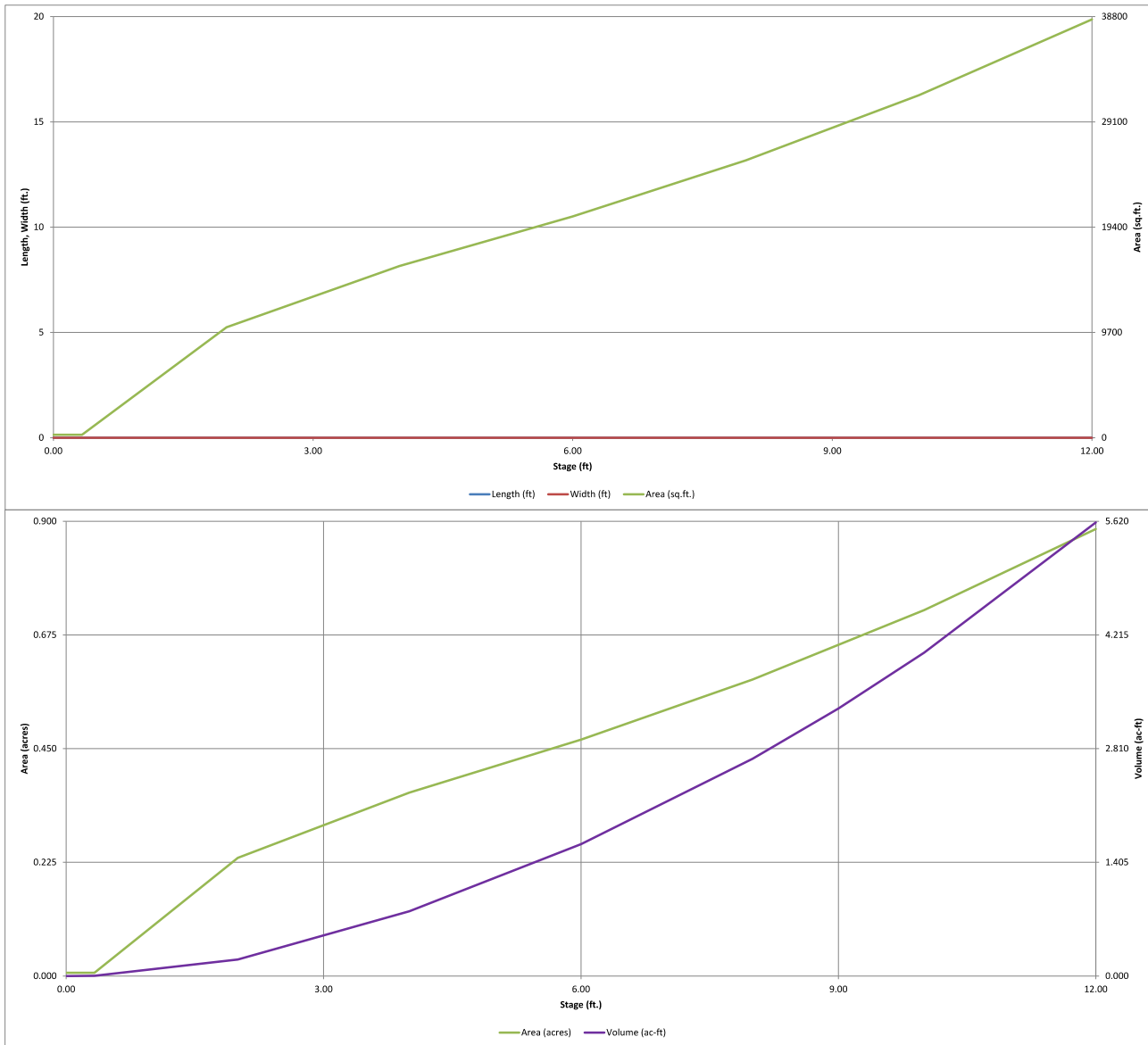
Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft ²)	Optional Override Area (ft ²)	Area (acre)	Volume (ft ³)	Volume (ac-ft)
Top of Micropool	0.00					274	0.006		
	0.33					274	0.006	90	0.002
	2.00					10,178	0.234	8,818	0.202
	4.00					15,818	0.363	34,814	0.799
	6.00					20,380	0.468	71,012	1.630
	8.00					25,558	0.587	116,950	2.685
	9.00					28,551	0.655	144,004	3.306
	10.00					31,544	0.724	174,052	3.996
	12.00					38,533	0.885	244,129	5.604

**AS-BUILTS POND 'A'
FOREST LAKES FILING 7
SF-21-49**



DETENTION BASIN STAGE-STORAGE TABLE BUILDER

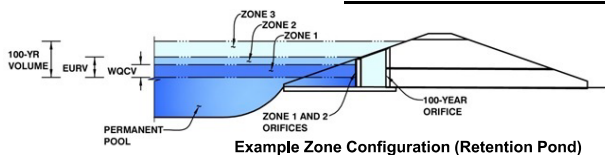
MHFD-Detention, Version 4.04 (February 2021)



DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: FOREST LAKES FILING 7
Basin ID: POND A - ASBUILT



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.43	0.601	Orifice Plate
Zone 2 (EURV)	6.25	1.147	Orifice Plate
Zone 3 (100-year)	8.89	1.481	Weir&Pipe (Circular)
Total (all zones)		3.229	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft ²
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	6.23	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	24.90	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft ²
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	2.08	4.15					
Orifice Area (sq. inches)	2.00	6.00	6.00					

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft ²
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	6.23	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	8.00	N/A	feet
Overflow Weir Grate Slope =	4.00	N/A	H:V
Horiz. Length of Weir Sides =	4.00	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H _u =	7.23	N/A	feet
Overflow Weir Slope Length =	4.12	N/A	feet
Grate Open Area / 100-yr Orifice Area =	4.68	N/A	
Overflow Grate Open Area w/o Debris =	22.96	N/A	ft ²
Overflow Grate Open Area w/ Debris =	11.48	N/A	ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Circular	Not Selected	
Depth to Invert of Outlet Pipe =	0.20	N/A	ft (distance below basin bottom at Stage = 0 ft)
Circular Orifice Diameter =	30.00	N/A	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Circular	Not Selected	
Outlet Orifice Area =	4.91	N/A	ft ²
Outlet Orifice Centroid =	1.25	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	N/A	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	8.91	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	36.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth =	0.88	feet
Stage at Top of Freeboard =	10.79	feet
Basin Area at Top of Freeboard =	0.79	acres
Basin Volume at Top of Freeboard =	4.59	acre-ft

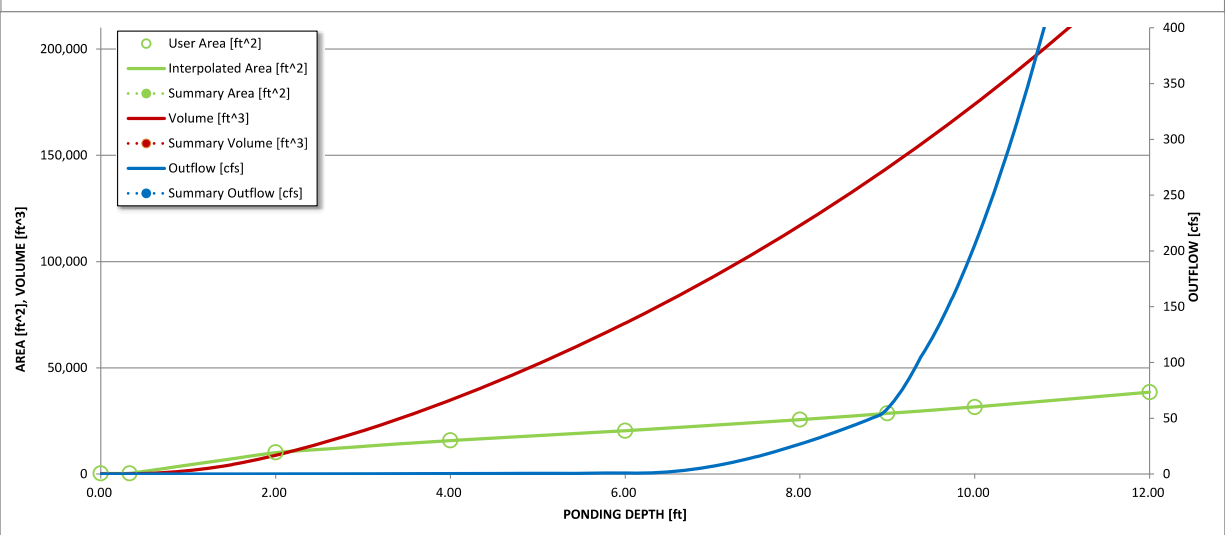
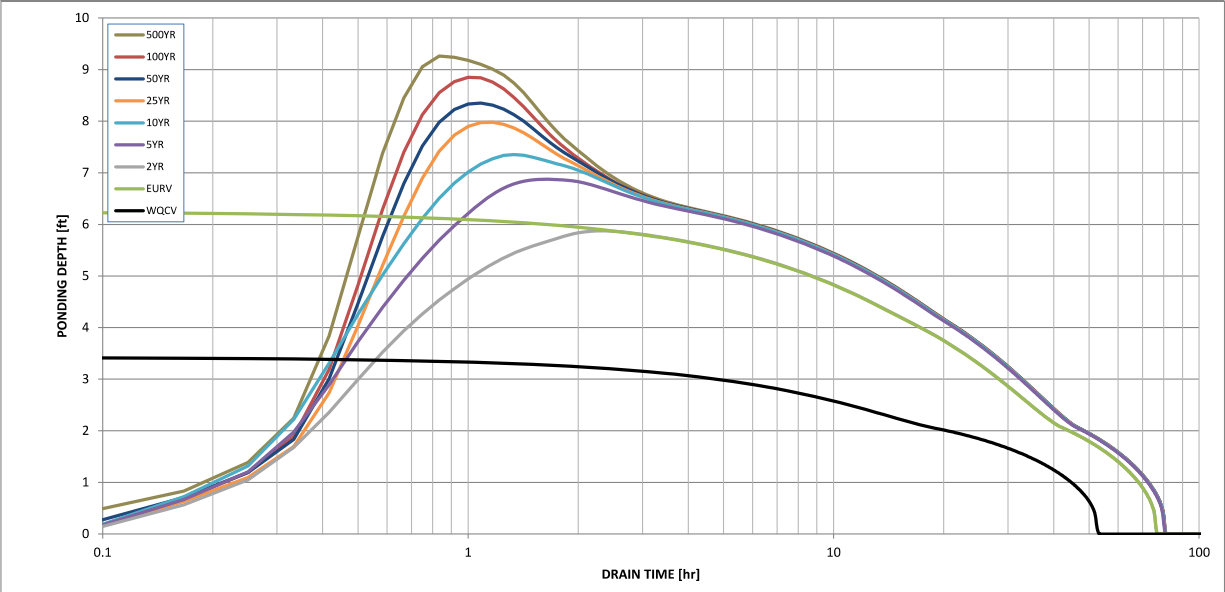
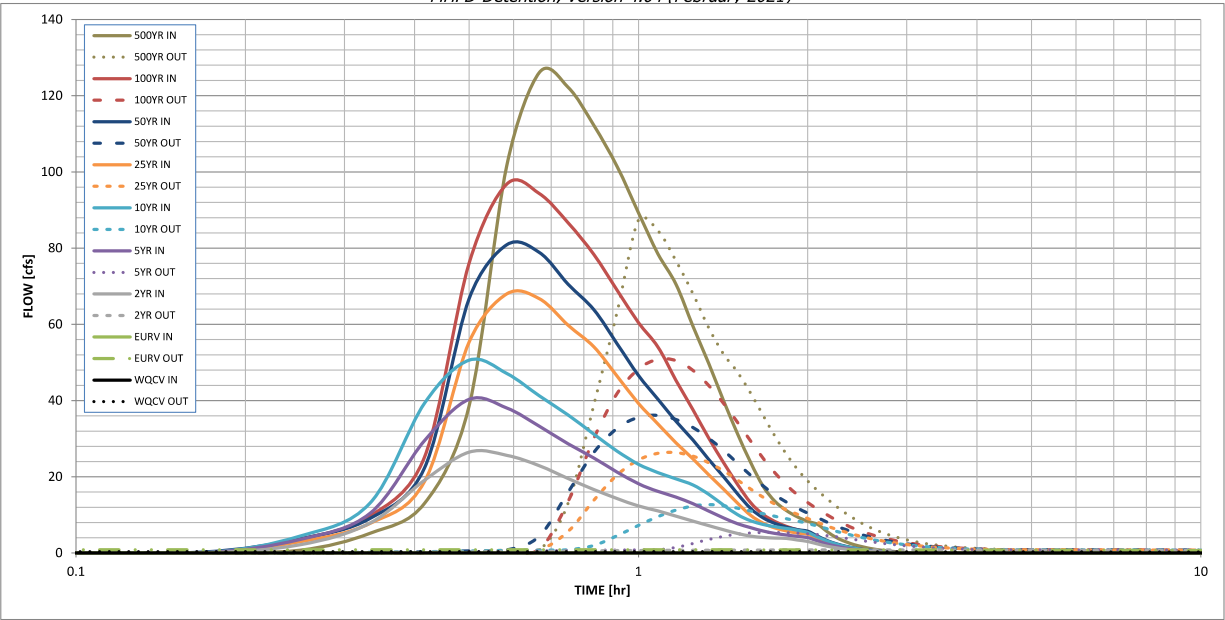
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.10
One-Hour Rainfall Depth (in) =	0.601	1.748	1.696	2.495	3.208	4.210	5.000	6.021	7.930
CUHP Runoff Volume (acre-ft) =	N/A	N/A	1.696	2.495	3.208	4.210	5.000	6.021	7.930
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	4.4	12.3	18.6	32.9	41.3	52.8	72.2
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A							
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.12	0.32	0.49	0.86	1.08	1.38	1.88
Peak Inflow Q (cfs) =	N/A	N/A	26.5	40.3	50.6	68.0	80.9	96.8	126.1
Peak Outflow Q (cfs) =	0.4	0.9	0.8	5.4	12.7	26.3	36.1	50.8	87.3
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.4	0.7	0.8	0.9	1.0	1.2
Structure Controlling Flow =	Plate	Overflow Weir 1	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	0.00	N/A	0.2	0.5	1.1	1.5	2.2	2.7
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	48	66	66	66	63	59	55	51	45
Time to Drain 99% of Inflow Volume (hours) =	51	72	72	75	74	72	70	69	66
Maximum Ponding Depth (ft) =	3.43	6.25	5.87	6.87	7.35	7.98	8.35	8.85	9.26
Area at Maximum Ponding Depth (acres) =	0.33	0.48	0.46	0.52	0.55	0.58	0.61	0.64	0.67
Maximum Volume Stored (acre-ft) =	0.603	1.749	1.570	2.060	2.316	2.667	2.894	3.202	3.479

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.25	0.02	0.75
	0:15:00	0.00	0.00	2.17	3.56	4.42	2.97	3.73	3.63	5.17
	0:20:00	0.00	0.00	7.87	10.44	13.12	7.78	9.08	9.72	13.07
	0:25:00	0.00	0.00	19.13	29.54	39.38	18.81	22.41	25.25	38.59
	0:30:00	0.00	0.00	26.48	40.33	50.59	55.34	66.71	76.07	101.60
	0:35:00	0.00	0.00	25.65	38.07	47.11	67.97	80.89	96.81	126.12
	0:40:00	0.00	0.00	22.91	33.25	41.24	66.73	78.86	94.31	122.21
	0:45:00	0.00	0.00	19.53	28.64	36.19	59.75	70.53	86.65	112.19
	0:50:00	0.00	0.00	16.66	24.93	31.18	54.08	63.85	78.30	101.41
	0:55:00	0.00	0.00	14.27	21.23	26.72	46.40	54.91	68.95	89.33
	1:00:00	0.00	0.00	12.36	18.18	23.29	39.28	46.60	60.47	78.50
	1:05:00	0.00	0.00	11.05	16.16	21.11	33.87	40.35	54.07	70.49
	1:10:00	0.00	0.00	9.65	14.68	19.44	28.90	34.55	45.30	59.50
	1:15:00	0.00	0.00	8.37	12.94	17.86	24.72	29.64	37.60	49.80
	1:20:00	0.00	0.00	7.20	11.02	15.43	20.54	24.60	30.22	40.00
	1:25:00	0.00	0.00	6.10	9.23	12.57	16.78	20.04	23.75	31.34
	1:30:00	0.00	0.00	5.13	7.70	10.08	13.11	15.57	18.05	23.75
	1:35:00	0.00	0.00	4.44	6.66	8.45	9.88	11.67	13.19	17.47
	1:40:00	0.00	0.00	4.09	5.76	7.50	7.81	9.23	10.11	13.55
	1:45:00	0.00	0.00	3.93	5.13	6.89	6.58	7.75	8.27	11.15
	1:50:00	0.00	0.00	3.84	4.70	6.45	5.80	6.80	7.01	9.49
	1:55:00	0.00	0.00	3.41	4.38	6.00	5.26	6.14	6.13	8.33
	2:00:00	0.00	0.00	3.02	4.03	5.40	4.92	5.71	5.50	7.49
	2:05:00	0.00	0.00	2.36	3.15	4.19	3.82	4.41	4.12	5.61
	2:10:00	0.00	0.00	1.80	2.37	3.13	2.83	3.25	2.96	4.03
	2:15:00	0.00	0.00	1.36	1.78	2.33	2.11	2.42	2.20	2.98
	2:20:00	0.00	0.00	1.03	1.33	1.72	1.57	1.80	1.64	2.22
	2:25:00	0.00	0.00	0.77	0.98	1.26	1.16	1.32	1.22	1.65
	2:30:00	0.00	0.00	0.56	0.70	0.92	0.84	0.95	0.89	1.20
	2:35:00	0.00	0.00	0.41	0.50	0.67	0.61	0.69	0.65	0.88
	2:40:00	0.00	0.00	0.29	0.35	0.48	0.44	0.50	0.47	0.63
	2:45:00	0.00	0.00	0.19	0.24	0.32	0.30	0.34	0.32	0.43
	2:50:00	0.00	0.00	0.11	0.15	0.19	0.19	0.21	0.20	0.26
	2:55:00	0.00	0.00	0.05	0.08	0.10	0.10	0.11	0.10	0.14
	3:00:00	0.00	0.00	0.02	0.03	0.04	0.04	0.04	0.04	0.05
	3:05:00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

