Preliminary and Final
DRAINAGE PLAN AND REPORT
ROCKY TOP MOTEL AND CAMPGROUND

10090 W Highway 24
A portion of the NW ¼, Section 9, Township 13 South, Range 68 West
EL PASO COUNTY

June 14, 2019

EPC STORMWATER REVIEW COMMENTS IN ORANGE BOXES WITH BLACK TEXT

Updated
August 16, 2021

Revised
January 4, 2022

Revised
April 13, 2022

Revised
August 22, 2022

Revises
December 19, 2022

Prepared for

G & D Enterprises
10090 West Highway 24
Green Mountain Falls, CO 80819

County File No.: PPR2140

Oliver E. Watts, Consulting Engineer, Inc.
Colorado Springs, Colorado

OLIVER E. WATTS, PE-LS
OLIVER E. WATTS, CONSULTING ENGINEER, INC.
CIVIL ENGINEERING AND SURVEYING
614 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907
(719) 593-0173
fax (719) 265-9660
olliewatts@aol.com
Celebrating over 43 years in business

December 19, 2022

El Paso County Planning and Community Development
2880 International Circle
Colorado Springs, CO 80910

ATTN: *Joshua Palmer, P.E.*

SUBJECT: Drainage Plan and Report
Rocky Top Motel and Campground

Transmitted herewith for your review and approval is the drainage plan and report for The Rocky Top Motel and Campground in El Paso County. This report is prepared and a result of Craig Dossey's letter of May 2, 2019 regarding an alleged violation of County grading regulations. It has been revised per the 10-7-21 County Review and our subsequent meetings and your review of December 12, 2022.. This report will accompany the submittal of other land use applications. Please contact me if I may provide any further information.

Oliver E. Watts, Consulting Engineer, Inc.

BY: _____
Oliver E. Watts, President

Encl:
Drainage Report 6 pages
Runoff Computations, 3 pages
UD Computations, 4 pages
FEMA Panel No. 08041C0952 G
SCS Soils Map
Backup Information, 5 sheets
Drainage Plan, Dwg 19-5341-02 & -07

Please sign this page and the next one electronically so that all pages of the report do not have to be scanned. It is much easier for us if the report is in the original state (ie: still a searchable pdf and not skewed from scanning).

Other option is just print, sign, and scan the two signature pages only and them insert them into the rest of the electronic (not scanned) pdf.

For a tutorial on how to do this with Adobe, you can check out this video:

https://www.youtube.com/watch?v=jPvzRRDd8ho

1. ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Oliver E. Watts, Consulting Engineer, Inc.

_____

Oliver E. Watts Colo. PE-LS No. 9853

2. OWNERS / DEVELOPER'S STATEMENT:

I the owner / developer have read and will comply with all of the requirements specified in this drainage report and plan.

G & D Enterprises, Corp.

By: _____
Daniel P. Nieman, owner
10090 West Highway 24
Green Mountain Falls, CO 80819
684-9044

3. EL PASO COUNTY:

Filed in accordance with the requirements of the El Paso Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Criteria Manual, as amended.

_____ _____

Joshua Palmer P.E., date
County Engineer / ECM Administrator

Conditions:

Please state whether or not the offsite flow was reverted to historic conditions by the addition of this retaining wall. If not then please analyze the conveyance to the outfall to ensure it is adequate.

4. LOCATION AND DESCRIPTION:

The Rocky Top Motel and Campground is located in a portion of the NW ¼, Section 9, Township 13 South, Range 68 West, of the 6th P.M., in El Paso County. The address, located at 10090 West Highway 24, is adjacent to Green Mountain Falls, on the north side of Highway 24 as shown in detail on the enclosed drainage plan. This facility has been in use at this location since 1947 as a motel and since 1950 as a camp ground. A use application for RV storage has been recently submitted to the County for this additional use. A detailed site survey is submitted as part of the enclosed drainage plan to delineate current conditions.

The County issued a notice of violation dated May 2, 2019, in reply to neighborhood complaints itemizing items that needed to be completed to reply to violations of grading in excess of one acre and the un-permitted use as RV storage. The County is considering any construction dating back to March 10, 2008 to be included in the disturbed area. This would include the paving of the primary north-south and east-west access road by asphalt, the grading of the proposed tent areas, and some of the RV sites, and two RV storage areas adjacent to Highway 24. The southeast 0.611 acre RV storage area and the southwest 0.38 acre site were vacated and reclaimed and are considered stable, and not included in the limit of disturbance. The tent site in the southwest corner of the site was graded and restored and the 0.393 acre portion is also not included in the proposed area of disturbance.

This text is contradictory to each other. The area should be included in the LOD since drainage paths were adjusted and impervious surfaces were added.

Much of the grading reported by the neighbors involved repair and maintenance, and only those areas within roadways are considered exempt. The owner has had to contend with erosion from stormwater runoff created by an addition of a culvert across the Lucky 4 Road to the west of the site. A rock retaining wall along the road was added for protection, which is within the 0.393 acre tent site area of disturbance. This is a private road that is not maintained by the County.

The proposed additional work requested by the client is as follows:

Rec Room addition north portion of property	0.035 ac. disturbance
RV site wall addition northeast portion	0.144
Garage and wall addition behind motel area	0.331
West PLD pond work	0.264
East PLD pond work	0.330
Total proposed work	1.104 ac. disturbance

5. FLOOD PLAIN STATEMENT:

This subdivision is not within the limits of a flood plain or flood hazard area, according to FEMA map panel number 08041C0467 G, dated December 7, 2018, a copy of which is enclosed for reference. Note that the site is in Zone D on said Firmette

Does not include 0.393ac tent site area that is shown on the drainage map.

6. METHOD AND CRITERIA:

The method used for all computations is that specified in the City-County Drainage Criteria Manual, using the rational method for areas of the size of the development. All computations are enclosed for reference and review.

The soils in the subdivision have been mapped by the local USDA/SCS office, and a soils map and

is enclosed for reference, indication that all soils in this area are of hydrologic group "A". The soils in this area are largely usable as gravel surfacing and are excellent as a construction material. Infiltration is maximum and runoff is held to a minimum.

is this developments flows the only flows accepted by the Hwy 24 culverts? Concern is whether the existing culvert is capable of accepting this developments flows and other tributary areas. Please address.

Unresolved previous comment:
Clarify that these flows are with the proposed/developed conditions since the sub-section heading is just "on-site runoff" it's unclear.

Computations are enclosed to show the historic drainage conditions prior to construction of any existing facilities (pre-1947). The drainage pattern has remained unchanged, and is increased due to development over the years. Historic and developed runoffs are described as follows.

B. Drainage Inflows:

As shown on the enclosed drainage plan one small area (Basin O-1) will drain into the property near the northwest corner, creating 0.15 cfs / 1.1 cfs (5-year / 100-year runoffs) from a small vacant grassed site. This runoff is in the undeveloped historic state.

C. On Site Runoff:

On site runoff has existed in the current state for many years. Improvements include the motel area and improvements, including paving, to the road system. Other improvements include regrading the area for use as campground and tented areas and increases in runoff are minimal as described improvements are made. The type "A" soils of the site exhibit minimal runoff, which is not significantly increased with gravel or similar surfacing used for dust control

The above mentioned inflow will combine with runoff from Basin A for a total of 4.0 cfs/ 10.6 cfs at the location shown on the drainage plan along the entrance road. The historic runoff for this area is 0.85 cfs \ 6.2 cfs. This basin is a mixture of part of the paved road and graveled campground sites graded into the natural terrain and areas of native vegetation covering steeper boundary areas. This will combine with runoff from Basin B, consisting of the motel site, paved roads and parking. The 0.61 acre RV parking site has been abandoned and reclaimed. The total runoff at the outfall point into Highway 24 is 5.6 cfs / 17.2 cfs, compared with the historic value of 1.49 cfs / 11.1 cfs. This runoff is well within the 21.4 cfs capacity of the existing downstream 24" cmp shown on the drainage plan, as shown by the enclosed computations. A sand filter basin is provided at the subdivision boundary for water quality. Computations are enclosed.

Please clarify which culvert this is on the drainage map.

Revise per comments on previous page regarding this area being considered part of the LOD since it is not fully reclaimed: grading changes and impervious surfaces added with the retaining walls.

Basin C is the Southwesterly third of the site, containing graveled campground sites, tent sites, and a gravel road. The 0.38 acre RV storage site has been abandoned and reclaimed. The total runoff at the historic outfall point into Highway 24 is 3.2 cfs / 9.1 cfs, compared with the historic value of 0.748 cfs / 5.7 cfs. Some 24" cmp culverts exist within the site and below the outfall point, as shown on the drainage plan. The first has a computed capacity of 33.5 cfs and will safely accommodate this total runoff as shown in the computations. Highway 24 culverts have proved historically adequate and will remain so as far as this development is concerned. A sand filter basin is provided at the subdivision boundary for water quality. Computations are enclosed

8. WATER QUALITY REQUIREMENTS:

The total historic and proposed development work on the site is largely mitigated by the Type A soils of the area. Two proposed sand filter basins are proposed at the outfall points development for this purpose. The proposed grading is shown on the enclosed drainage

provide the capacity of the Hwy 24 culvert that is accepting this flow. It is not clear if the above capacity listed is for the Hwy 24 culvert.

Please clearly state whether or not detention is needed and why.

For what purpose? Clarify that the SFBs are there to provide WQ treatment of the WQCV.

the grading plan that accompany the total submittal. The work is minimal and necessary erosion BMP's are proposed.

In this section, also discuss any applicable WQ exclusions. For areas that need WQ treatment (like the paved road for example) but don't appear to be tributary to either pond. So for the paved road, the recommended applicable exclusion is per ECM App I.7.1.C.1 (which allows for 20% not to exceed 1 acre of the applicable development site area to not be treated).

9. COST ESTIMATE:

All facilities are private.

Item No.	Description	Quantity	Unit Cost	Cost
1	West Sand Filter Basin	1 ea	LS	$ 2000.00
2	East Sand Filter Basin	1 ea	LS	2500.00
3	24" CMP Storm Sewer	101 LF	30.00	3030.00
4	12" PVC Storm Sewer	44 LF	25.00	1100.00
5	Firebaugh Grated Inlet	1 ea	1500.00	1500.00
6	CDOT Grated Inlet	1 ea	2500.00	2500.00
7	Riprap	10 CY	100.00	1000.00
	Subtotal Construction Cost			$ 13630.00
	Engineering	10%		1363.00
	Total Estimated Cost			$ 14993.00

10. SUMMARY

The motel and campground have existed at this address since 1947 and 1950 respectively. The proposed facilities will mitigate the effects of historic development as well as proposed improvements. Those installed since March, 2008 have been specifically addressed. There will be no adverse effects on downstream or surrounding properties.

The drainage analysis has been prepared in accordance with the current El Paso County Drainage Criteria Manuel. Supporting information and calculations are included in this report.

MAJOR BASIN	SUB BASIN	AREA		BASIN		Tc MIN	I		SOIL GRP	DEV. TYPE	C		FLOW		RETURN PERIOD	
		PLANIM READ	ACRES	LENGTH	HEIGHT								qp	qp		
FOUNTAIN CR	O-1	COGO	0.66	100	4	20			A	MDW	0.08	0.35			5	100
				+200	6	+1										
						21	2.9	4.8					0.15	1.1	5	100
	+A	COGO	3.12	+420	34	+1.2			A	MDW	0.08	0.35	15%			
				V=5.7						GRAVEL	0.50	0.70	85%			
										MIX	0.437	0.648				
	TOTAL	COGO	3.78			22.2	2.8	4.7	A	MIX	0.375	0.596	4.0	10.6	5	100
	+B	COGO	3.13	+360	34	+1.0			A	ROOF	0.73	0.81	2%			
				V=6.1						GRAVEL	0.50	0.70	20%			
										MDW	0.08	0.35	70%			
										MIX	0.215	0.478				
	TOTAL	COGO	6.91	43%		23.2	2.7	4.6	A	MIX	0.302	0.542	5.6	17.2	5	100
	C	COGO	2.97	100	2	14.7			A	GRAVEL	0.50	0.70	60%			
			V=5.4	+640	46	+2.0				MDW	0.08	0.35	40%			
				45%		16.7	3.3	5.5	A	MIX	0.332	0.560	3.2	9.1	5	100

HYDROLOGICAL COMPUTATION – BASIC DATA
PROJ: ROCKY TOP MOTEL & CAMPGROUND BY: O.E. WATTS
RATIONAL METHOD DATE: 6-14-19, 8-22-21

OLIVER E. WATTS, CONSULTING ENGINEER, INC.
614 ELKTON DRIVE COLORADO SPRINGS, CO 80907

Is this the total impervious of the site?If so please show your work.

MAJOR BASIN	SUB BASIN	AREA		BASIN		Tc MIN	I in./hr.		SOIL GRP	DEV. TYPE	C		FLOW 5-ry qp -CFS-	FLOW 100-yr qp -CFS-	RETURN PERIOD -years-	
		PLANIM READ	ACRES	LENGTH -FT.-	HEIGHT -FT.-											
HISTORIC	O-1	COGO	0.66	100	4	20			A	MDW	0.08	0.35			5	100
				+200	6	+1										
						21	2.9	4.8					0.15	1.1	5	100
	+A	COGO	3.13	+420	34	+1.2										
	TOTAL		3.748			22.2	2.8	4.7	A	MDW	0.08	0.35	0.85	6.2	5	100
	+B	COGO	3.13	+360	34	+1.0										
	TOTAL		6.91			23.2	2.7	4.6	A	MDW	0.08	0.35	1.49	11.1	5	100
	C	COGO	2.97	100	2	14.7										
				+640	46	+2.0										
						16.7	3.3	5.5	A	MDW	0.08	0.35	0.78	5.7	5	100

HYDROLOGICAL COMPUTATION – BASIC DATA
PROJ: ROCKY TOP MOTEL & CAMPGROUND BY: O.E. WATTS
RATIONAL METHOD DATE: August 24, 2022

OLIVER E. WATTS, CONSULTING ENGINEER, INC.
614 ELKTON DRIVE COLORADO SPRINGS, CO 80907

STREET AND STORM SEWER CALCULATIONS

STREET	LOCATION	DISTANCE	ELEVATION & SLOPE	TOTAL RUNOFF	STREET FLOW / CAPACITY	PIPE FLOW	TYPE PIPE, CATCH BASIN & SLOPE %
PRIVATE	B SFB	32	TOP 30.0 INV 26.5	5.6/172		17.2	24"CMP hi=0.62' S=0.60% MIN CDOT INLET WS EL 30.35
	SPILLWAY		-0-			17.2	WIER, MIN. W=8'
	OUTFALL	37	1.3%			17.2	24" CMP
	EXISTING	65'	3%			17.2	24" CMP CAP=21.4
	C SFB		TOP 24.00 INV. 22.5	3.7/9.1		9.1	24"CMP hi=0.24' S=0.20% MIN. FB INLET WS 24.40
	SPILLWAY		-0-			9.1	WIER, MIN. W=5'
	OUITFALL	44	5.6%			9.1	12" PVC
	EXISTING	80'	7.5%			9.1	24" CMP, CAP =33.5

please provide hydraulic calcs for the culverts and spillways

Design Procedure Form: Sand Filter (SF)

UD-BMP (Version 3.07, March 2018) — Sheet 1 of 2

Designer:	O.E. WATTS
Company:	Oliver E. Watts, CE
Date:	December 26, 2022
Project:	Rocky Top Motel and Campground
Location:	Basin C SFB southwest corner

1. Basin Storage Volume		
A) Effective Imperviousness of Tributary Area, I_a (100% if all paved and roofed areas upstream of sand filter)	I_a = 45.0	%
B) Tributary Area's Imperviousness Ratio (i = I_a/100)	i = 0.450	
C) Water Quality Capture Volume (WQCV) Based on 12-hour Drain Time WQCV= 0.8 * (0.91* i^3 - 1.19 * i^2 + 0.78 * i)	WQCV = 0.15	watershed inches
D) Contributing Watershed Area (including sand filter area)	Area = 129,700	sq ft
E) Water Quality Capture Volume (WQCV) Design Volume V_{WQCV} = WQCV / 12 * Area	V_{WQCV} =	cu ft
F) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm	d_6 = 2.52	in
G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume	$V_{WQCV\ OTHER}$ =	cu ft
H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)	$V_{WQCV\ USER}$ = 1,300	cu ft
2. Basin Geometry		
A) WQCV Depth	D_{WQCV} = 2.0	ft
B) Sand Filter Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred). Use "0" if sand filter has vertical walls.	Z = 4.00	ft / ft
C) Minimum Filter Area (Flat Surface Area)	A_{Min} = 730	sq ft
D) Actual Filter Area	A_{Actual} = 730	sq ft
E) Volume Provided	V_T = 1303	cu ft
3. Filter Material	Choose One: (●) 18" CDOT Class B or C Filter Material; ( ) Other (Explain): TYPE A SOIL	
4. Underdrain System		
A) Are underdrains provided?	Choose One: (●) YES; ( ) NO	
B) Underdrain system orifice diameter for 12 hour drain time		
i) Distance From Lowest Elevation of the Storage Volume to the Center of the Orifice	y = 1.5	ft
ii) Volume to Drain in 12 Hours	Vol_{12} = 1,300	cu ft
iii) Orifice Diameter, 3/8" Minimum	D_O = 7/8	in

Review 3: please provide calculation as to how the impervious % was determined.
Review 4: Unresolved
Review 5: unresolved

Delineate this area on the drainage map. It doesn't appear that the flows from the paved road (that need to be treated) are tributary to the pond. So we need to see this delineation in order to confirm which areas are being treated and are apart of this 129,700sq ft that is listed on this spreadsheet. See comment on page 6 above about possible exclusions. All areas within the LOD and/or disturbed since 2008 will need to with be shown as tributary to one of the ponds are shown to have an appropriate exclusion apply.

1.5ft here does not match what is shown on the plans. The plans show the bottom of the pond at 22ft elevation and inv of 4" pipe at connection to 18" pipe at 20.74ft. So something isn't right. Revise calcs and/or plans as needed to remove this discrepancy.

Design Procedure Form: Sand Filter (SF)

Designer: O.E. WATTS
Company: Oliver E. Watts, CE
Date: December 26, 2022
Project: Rocky Top Motel and Campground
Location: Basin C SFB southwest corner

5. Impermeable Geomembrane Liner and Geotextile Separator Fabric A) Is an impermeable liner provided due to proximity of structures or groundwater contamination?	Choose One ○ YES ◉ NO
6. Inlet / Outlet Works A) Describe the type of energy dissipation at inlet points and means of conveying flows in excess of the WQCV through the outlet	RIPRAPPED INLET AND SPILLWAY

Notes:

Design Procedure Form: Sand Filter (SF)

UD-BMP (Version 3.07, March 2018) — Sheet 1 of 2

Designer:	O.E. WATTS
Company:	Oliver E. Watts, CE
Date:	December 26, 2022
Project:	Rocky Top Motel and Campground
Location:	BASIN B SFB SOUTHEAST CORNER

1. Basin Storage Volume			
A) Effective Imperviousness of Tributary Area, I_a (100% if all paved and roofed areas upstream of sand filter)	I_a =	43.0	%
B) Tributary Area's Imperviousness Ratio (i = I_a/100)	i =	0.430	
C) Water Quality Capture Volume (WQCV) Based on 12-hour Drain Time WQCV= 0.8 * (0.91* i^3 - 1.19 * i^2 + 0.78 * i)	WQCV =	0.15	watershed inches
D) Contributing Watershed Area (including sand filter area)	Area =	136,300	sq ft
E) Water Quality Capture Volume (WQCV) Design Volume V_{WQCV} = WQCV / 12 * Area	V_{WQCV} =		cu ft
F) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm	d_6 =	2.52	in
G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume	$V_{WQCV\ OTHER}$ =		cu ft
H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)	$V_{WQCV\ USER}$ =	1,300	cu ft
2. Basin Geometry			
A) WQCV Depth	D_{WQCV} =	2.0	ft
B) Sand Filter Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred). Use "0" if sand filter has vertical walls.	Z =	4.00	ft / ft
C) Minimum Filter Area (Flat Surface Area)	A_{Min} =	733	sq ft
D) Actual Filter Area	A_{Actual} =	730	sq ft ACTUAL FLAT AREA < MINIMUM FLAT AREA
E) Volume Provided	V_T =	1313	cu ft
3. Filter Material	Choose One: (●) 18" CDOT Class B or C Filter Material; ( ) Other (Explain):		
	TYPE A SOIL		
4. Underdrain System			
A) Are underdrains provided?	Choose One: (●) YES; ( ) NO		
B) Underdrain system orifice diameter for 12 hour drain time			
i) Distance From Lowest Elevation of the Storage Volume to the Center of the Orifice	y =	1.5	ft
ii) Volume to Drain in 12 Hours	Vol_{12} =	1,300	cu ft
iii) Orifice Diameter, 3/8" Minimum	D_O =	7/8	in

Review 3: please provide calculation as to how the impervious % was determined.
Review 4: Unresolved
Review 5: unresolved

Delineate this area on the drainage map. It doesn't appear that the flows from the paved road (that need to be treated) are tributary to the pond. So we need to see this delineation in order to confirm which areas are being treated and are apart of this 136,300sq ft that is listed on this spreadsheet. See comment on page 6 above about possible exclusions. All areas within the LOD and/or disturbed since 2008 will need to with be shown as tributary to one of the ponds are shown to have an appropriate exclusion apply.

Cannot confirm whether or now 1.5ft here does matches what is shown on the plans. The plans show the bottom of the pond at 28ft elevation and inv of 4" pipe at connection to inlet box is unknown (not shown in plans). Revise calcs and/or plans as needed to clarify and to reflect this 1.5ft distance.

Design Procedure Form: Sand Filter (SF)

Designer:	O.E. WATTS
Company:	Oliver E. Watts, CE
Date:	December 26, 2022
Project:	Rocky Top Motel and Campground
Location:	BASIN B SFB SOUTHEAST CORNER

5. Impermeable Geomembrane Liner and Geotextile Separator Fabric A) Is an impermeable liner provided due to proximity of structures or groundwater contamination?	Choose One ○ YES ◉ NO
6. Inlet / Outlet Works A) Describe the type of energy dissipation at inlet points and means of conveying flows in excess of the WQCV through the outlet	RIPRAPPED INLET AND SPILLWAY

Notes:

National Flood Hazard Layer FIRMette

FEMA

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

- **SPECIAL FLOOD HAZARD AREAS**
  - Without Base Flood Elevation (BFE) *Zone A, V, A99*
  - With BFE or Depth *Zone AE, AO, AH, VE, AR*
  - Regulatory Floodway
- **OTHER AREAS OF FLOOD HAZARD**
  - 0.2% Annual Chance Flood Hazard, Area of 1% annual chance flood with average depth less than one foot or with drainag areas of less than one square mile *Zone*
  - Future Conditions 1% Annual Chance Flood Hazard *Zone X*
  - Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
  - Area with Flood Risk due to Levee *Zone D*
- **OTHER AREAS**
  - NO SCREEN — Area of Minimal Flood Hazard *Zone X*
  - Effective LOMRs
  - Area of Undetermined Flood Hazard *Zone*
- **GENERAL STRUCTURES**
  - Channel, Culvert, or Storm Sewer
  - Levee, Dike, or Floodwall
- **OTHER FEATURES**
  - 20.2 — Cross Sections with 1% Annual Chance
  - 17.5 — Water Surface Elevation
  - Coastal Transect
  - 513 — Base Flood Elevation Line (BFE)
  - Limit of Study
  - Jurisdiction Boundary
  - Coastal Transect Baseline
  - Profile Baseline
  - Hydrographic Feature
- **MAP PANELS**
  - Digital Data Available
  - No Digital Data Available
  - Unmapped

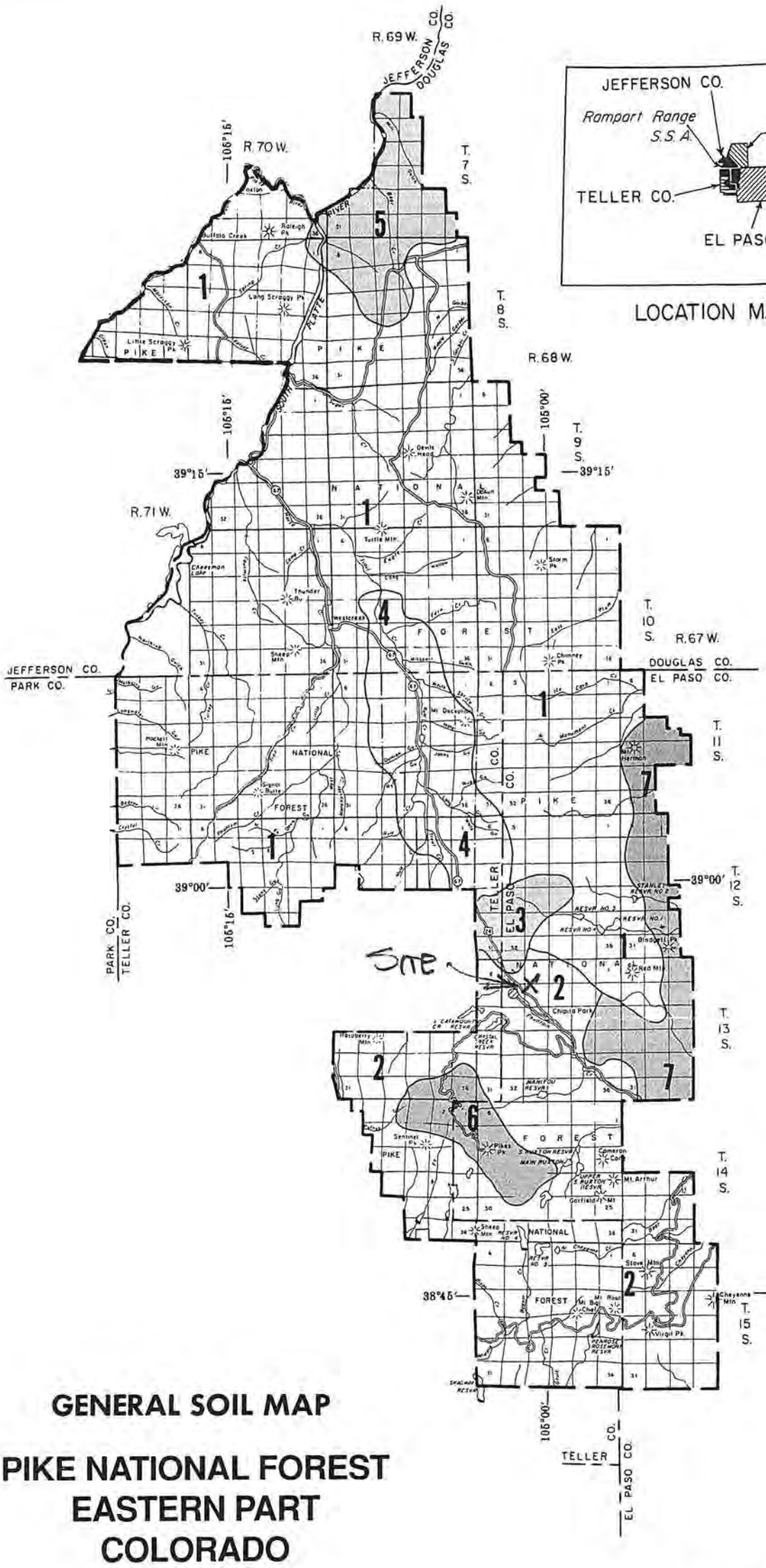
N

The pin displayed on the map is an approximate point selected by the user and does not represe an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 6/14/2019 at 10:34:12 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



LOCATION MAP

SOIL LEGEND

1 Sphinx-Legault-Rock outcrop: Rock outcrop and shallow, somewhat excessively drained soils that formed in material weathered from granite

2 Catamount-Ivywild-Legault-Rock outcrop: Rock outcrop and shallow and moderately deep, somewhat excessively drained, and excessively drained soils that formed in material weathered from granite

3 Sphinx-Tecolote-Condie: Shallow and deep, somewhat excessively drained and well drained soils that formed in material weathered from granite or in colluvium over weathered granite

4 Boyett-Frenchcreek-Pendant: Deep and shallow, well drained and somewhat excessively drained soils that formed in material weathered from limestone and granite, and in alluvium derived from mixed red arkosic sandstone

5 Security-Cathedral-Rock outcrop: Rock outcrop and moderately deep and shallow, well drained soils that formed in material weathered from mixed schist, gneiss, and granite

6 Sachett-Cirque land: Cirque land and shallow, excessively drained soils that formed in material weathered from granite

7 Tecolote-Pendant: Deep and shallow, well drained and somewhat excessively drained soils that formed in material weathered from limestone and in cobbly or stony colluvium over weathered granite

Compiled 1986

GENERAL SOIL MAP

PIKE NATIONAL FOREST EASTERN PART COLORADO

JULY 1992

5 0 5 10 MILES

Scale 1:362,057

1 inch equals approximately 5.7 miles

PARTS OF DOUGLAS, EL PASO, JEFFERSON, AND TELLER COUNTIES, COLORADO

OLIVER E. WATTS
CONSULTING ENGINEER, INC.
COLORADO SPRINGS

ROCKY TOP MOTEL AND CAMPGROUND
SCS SOILS MAP

U.S. DEPARTMENT OF AGRICULTURE
FOREST SERVICE
SOIL CONSERVATION SERVICE
COLORADO AGRICULTURAL EXPERIMENT STATION

$$t_c = t_i + t_t \qquad \text{(Eq. 6-7)}$$

Where:

t_c = time of concentration (min)

t_i = overland (initial) flow time (min)

t_t = travel time in the ditch, channel, gutter, storm sewer, etc. (min)

3.2.1 Overland (Initial) Flow Time

The overland flow time, t_i, may be calculated using Equation 6-8.

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \qquad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \qquad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)

C_v = conveyance coefficient (from Table 6-7)

S_w = watercourse slope (ft/ft)

Table 6-6. Runoff Coefficients for Rational Method
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis--Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

3.2 Time of Concentration

One of the basic assumptions underlying the Rational Method is that runoff is a function of the average rainfall rate during the time required for water to flow from the hydraulically most remote part of the drainage area under consideration to the design point. However, in practice, the time of concentration can be an empirical value that results in reasonable and acceptable peak flow calculations.

For urban areas, the time of concentration (t_c) consists of an initial time or overland flow time (t_i) plus the travel time (t_t) in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For non-urban areas, the time of concentration consists of an overland flow time (t_i) plus the time of travel in a concentrated form, such as a swale or drainageway. The travel portion (t_t) of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or drainageway. Initial time, on the other hand, will vary with surface slope, depression storage, surface cover, antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. The time of concentration is represented by Equation 6-7 for both urban and non-urban areas.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

*For buried riprap, select C_v value based on type of vegetative cover.

The travel time is calculated by dividing the flow distance (in feet) by the velocity calculated using Equation 6-9 and converting units to minutes.

The time of concentration (t_c) is then the sum of the overland flow time (t_i) and the travel time (t_t) per Equation 6-7.

3.2.3 First Design Point Time of Concentration in Urban Catchments

Using this procedure, the time of concentration at the first design point (typically the first inlet in the system) in an urbanized catchment should not exceed the time of concentration calculated using Equation 6-10. The first design point is defined as the point where runoff first enters the storm sewer system.

$$t_c = \frac{L}{180} + 10 \qquad \text{(Eq. 6-10)}$$

Where:

t_c = maximum time of concentration at the first design point in an urban watershed (min)

L = waterway length (ft)

Equation 6-10 was developed using the rainfall-runoff data collected in the Denver region and, in essence, represents regional "calibration" of the Rational Method. Normally, Equation 6-10 will result in a lesser time of concentration at the first design point and will govern in an urbanized watershed. For subsequent design points, the time of concentration is calculated by accumulating the travel times in downstream drainageway reaches.

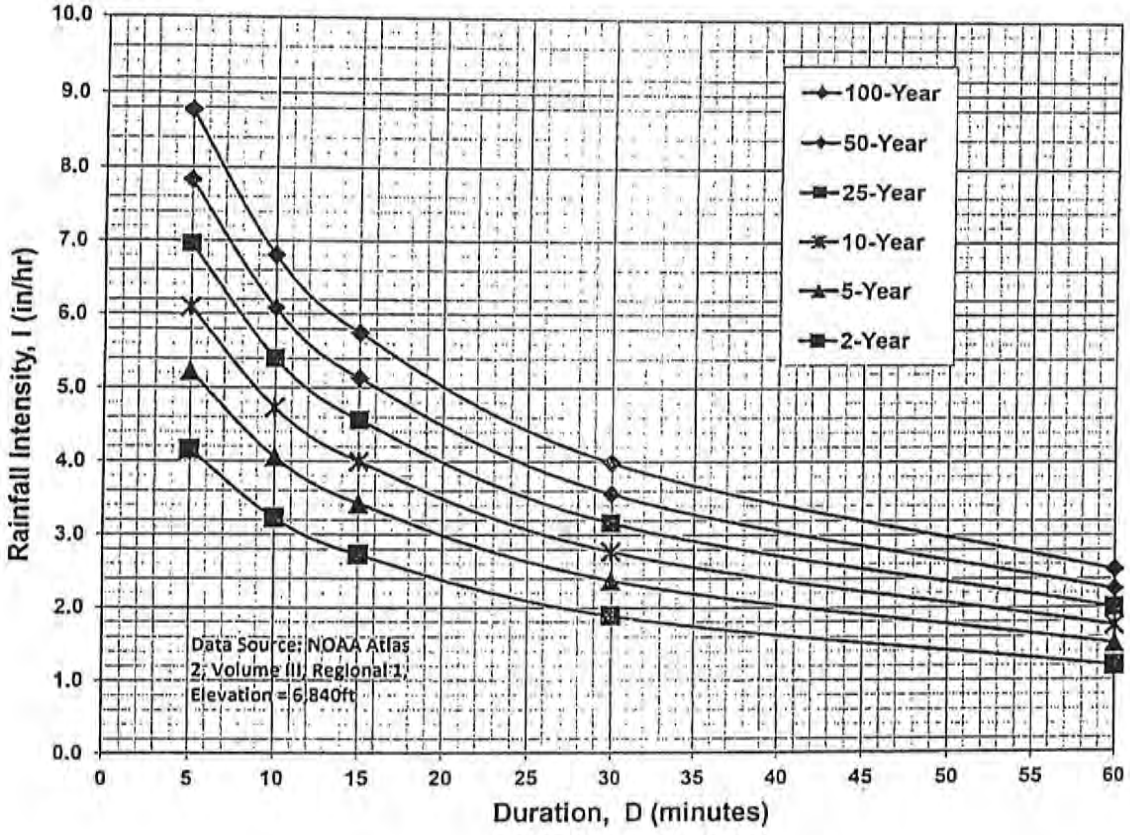
3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

3.2.5 Post-Development Time of Concentration

As Equation 6-8 indicates, the time of concentration is a function of the 5-year runoff coefficient for a drainage basin. Typically, higher levels of imperviousness (higher 5-year runoff coefficients) correspond to shorter times of concentration, and lower levels of imperviousness correspond to longer times of

Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



IDF Equations

$I_{100} = -2.52 \ln(D) + 12.735$

$I_{50} = -2.25 \ln(D) + 11.375$

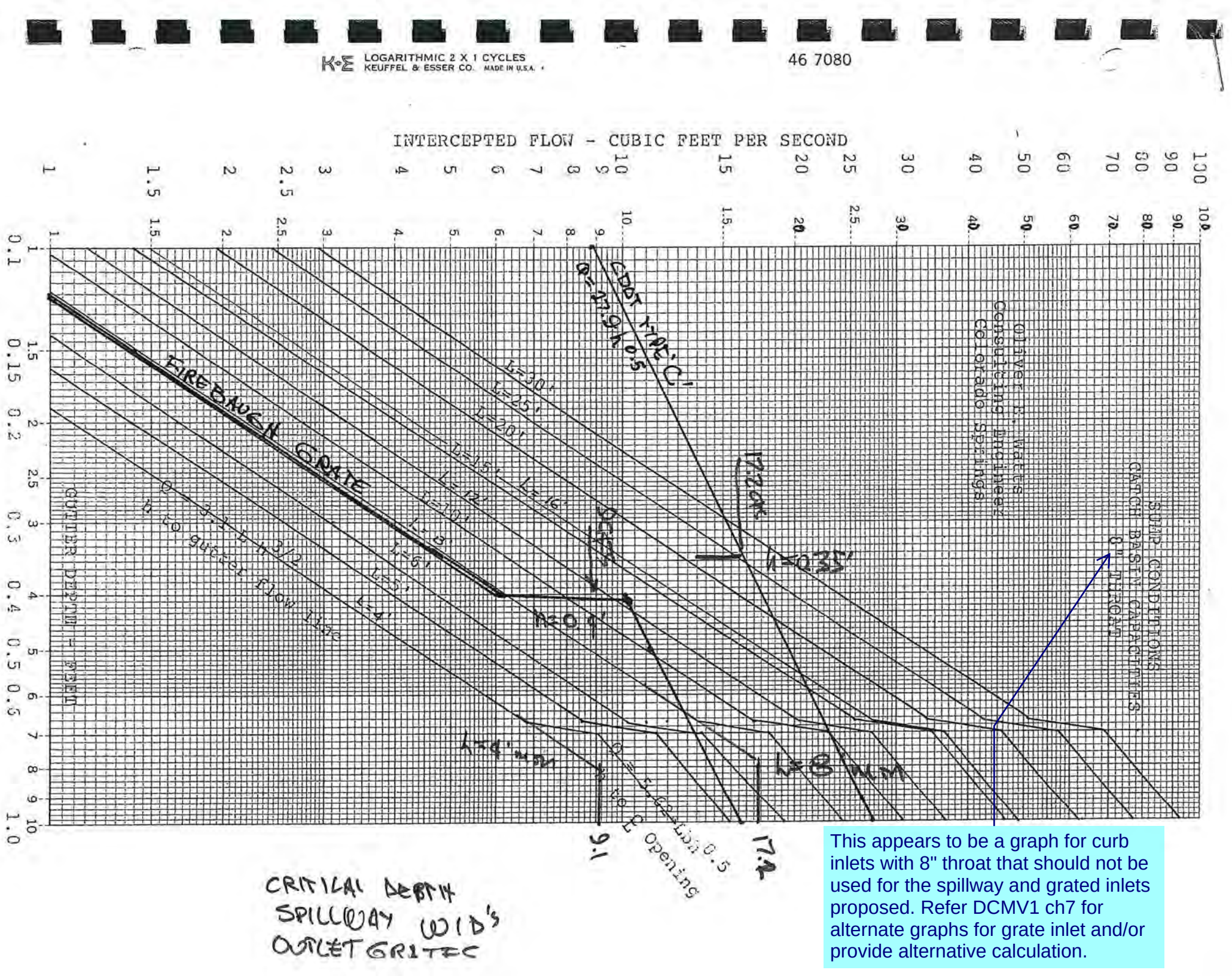
$I_{25} = -2.00 \ln(D) + 10.111$

$I_{10} = -1.75 \ln(D) + 8.847$

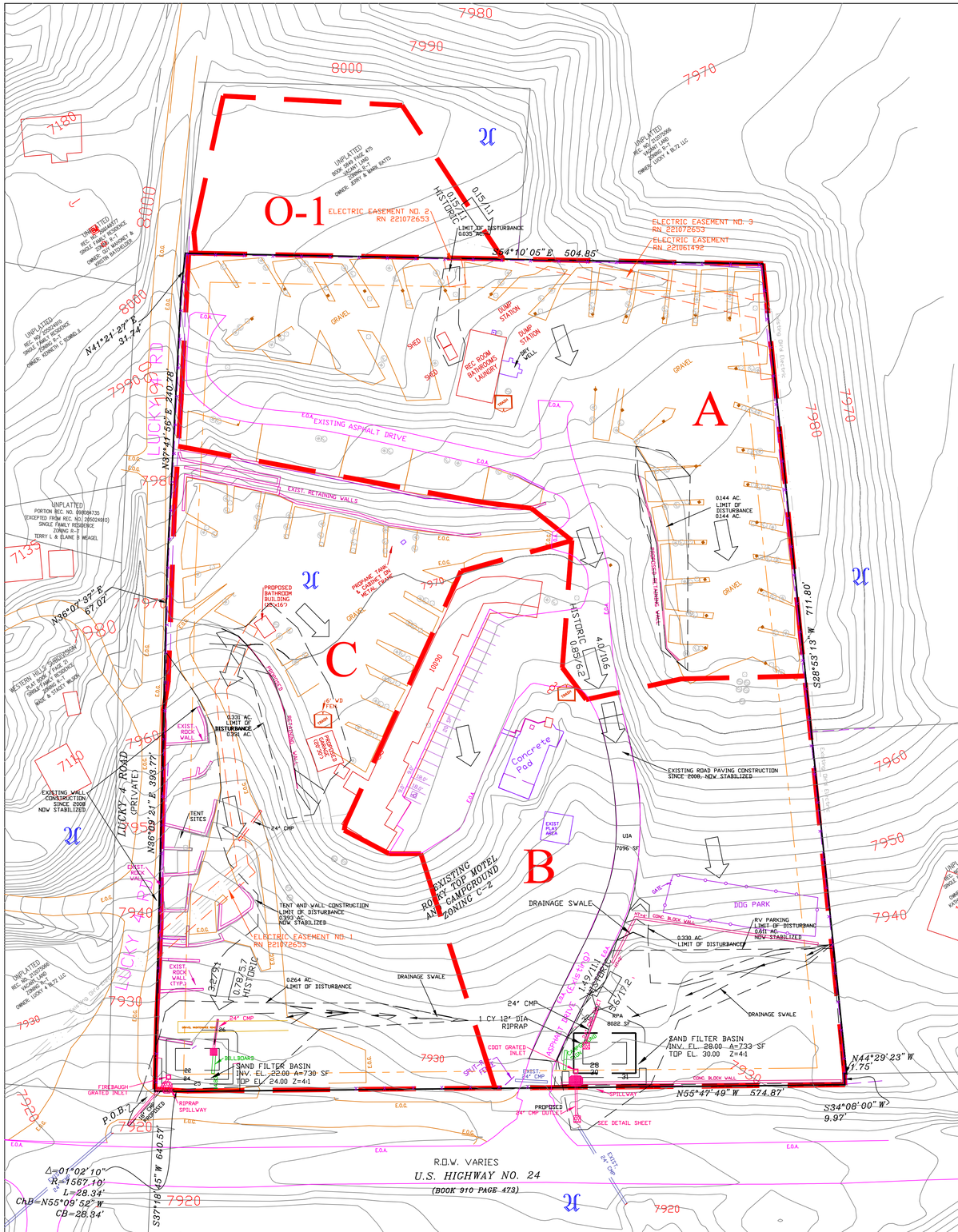
$I_{5} = -1.50 \ln(D) + 7.583$

$I_{2} = -1.19 \ln(D) + 6.035$

Note: Values calculated by equations may not precisely duplicate values read from figure.

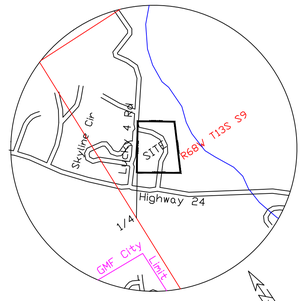


This appears to be a graph for curb inlets with 8" throat that should not be used for the spillway and grated inlets proposed. Refer DCMV1 ch7 for alternate graphs for grate inlet and/or provide alternative calculation.



Per comment on UD-BMP spreadsheets above, delineate approximate areas tributary to each pond, with excluded areas also clearly delineated.

It might be best to do this on a separate map so this map does not become too busy.



VICINITY MAP

1"=1000'

Scale 1" = 50'

Contour Interval: 2'

NGVD, 1929 ADJUSTMENT

CONTOUR LEGEND:

ORIGINAL CONTOURS:
2'
10'
FINISH CONTOURS:
1'
5'

LEGEND:

10.5/20.4 RUNOFF IN CFS 5-YEAR/100-YEAR

A LIMIT OF DRAINAGE BASIN AND DESIGNATION

EXISTING STORM SEWER AS LABELED

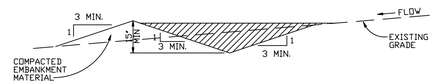
PROPOSED STORM SEWER AS LABELED

B LIMIT OF SOILS TYPE AND GROUP

DRAINAGE BASIN SUMMARY

RUNOFF IN CFS

BASIN	DEVELOPED 5-YEAR	DEVELOPED 100-YEAR	HISTORIC 5-YEAR	HISTORIC 100-YEAR
O-1	0.15	1.1	0.15	1.1
O-1 + A	4.0	10.6	0.85	6.2
O-1 + A + B	5.6	17.2	1.49	11.1
C	3.2	9.1	0.78	5.7



DRAINAGE SWALE

NTS

DRAWN BY: O.E. WATTS	APPROVED BY:	
DATE: 6-13-19	PROJ. NO.	
DWG. NO.: 19-5341-02	DWG.	
TOPOGRAPHY BY: CITY FIMS 6-12-19		
SURVEY INFORMATION BY: RAMPART JOB NO. 18384		

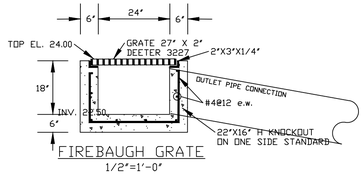
REVISIONS	
8-16-21 UPDATED	OEW
12-30-21 REVISED PER COUNTY REVIEW COMMENTS	OEW
~~8-23-22 REVISED PER COUNTY REVIEW COMMENTS~~	~~OEW~~
12-22-22 REVISED PER COUNTY REVIEW COMMENTS	OEW

OLIVER E. WATTS
CONSULTING ENGINEER
COLORADO SPRINGS

PROJECT
ROCKY TOP MOTEL & CAMPGROUND
PART NW1/4 SECTION 9, T.13S., R.68W., 6TH P.M.
EL PASO COUNTY

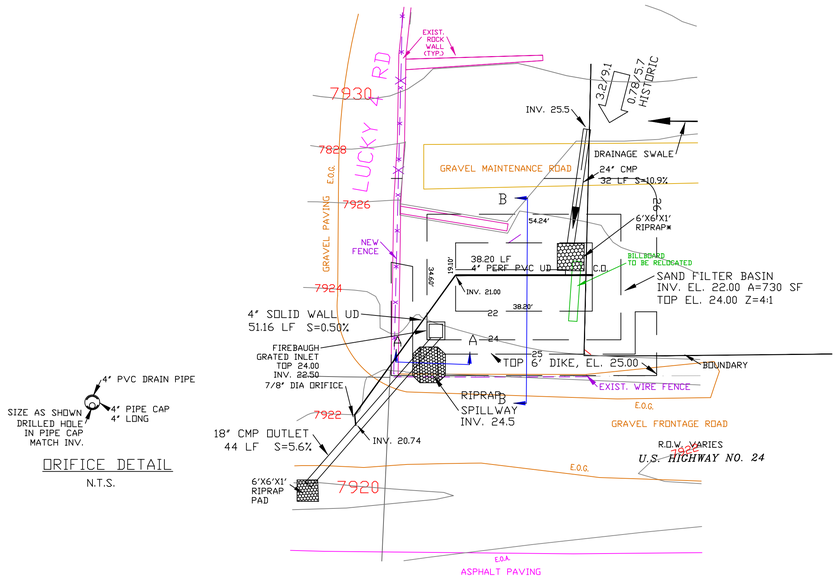
SHT. NAME
DRAINAGE PLAN

SHT. NO.
1
OF
1



FIREBAUGH GRATE

1/2"=1'-0"

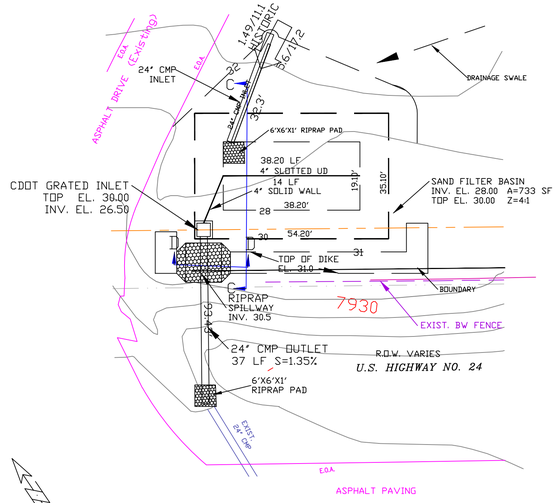


WEST SFB DETAILS

1"=20'

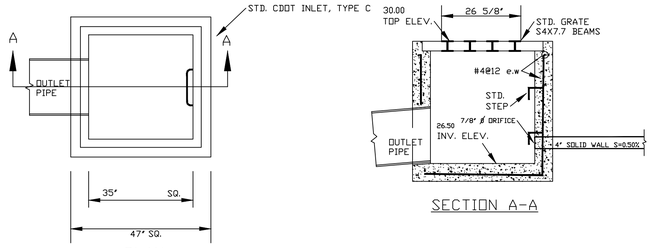
ORIFICE DETAIL

N.T.S.



EAST SFB DETAILS

1"=20'

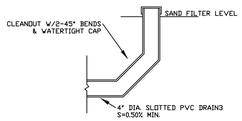


PLAN

SECTION A-A

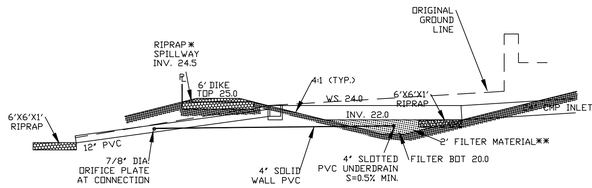
CDOT INLET

1/2"=1'-0"



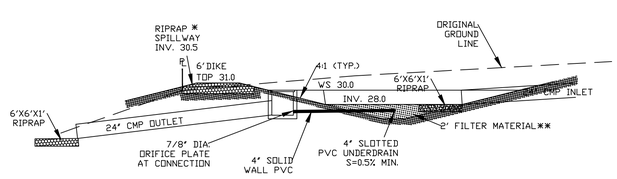
CLEANOUT DETAIL

N.T.S.



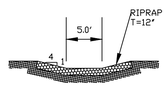
SECTION B-B

1"=10'



SECTION C-C

1"=10'

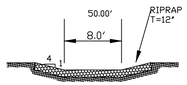


SECTION A-A SPILLWAY

1"=10'

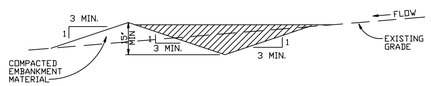
*RIPRAP IS TO BE TYPE 'L' OR 'VL', MIXED WITH 35% NATIVE SOIL BY WEIGHT, COVERED WITH 4" MIN TOPSOIL

** FILTER MATERIAL IS TO BE
85% SAND, ASTM C-33
15" PEAT MIX
RESEED PER MHFD TABLE B-6 MIX FOR DRYLAND GRASSES
RESEED SIDE SLOPES
DO NOT RESEED POND BOTTOM OR RIPRAP PADS



SECTION D-D SPILLWAY

1"=10'



DRAINAGE SWALE

NTS

Prepared by the office of:
Oliver E. Watts, Consulting Engineer, Inc.
614 Elkton Drive
Colorado Springs, CO 80907
(719) 593-0173
Olliewatts@aol.com
Celebrating 42 years in Business

DRAWN BY: O.E. WATTS DATE: 8-16-21 DWG. NO.: 19-5341-05 TOPOGRAPHY BY: CITY FIMS 6-12-19 SURVEY INFORMATION BY: RAMPART JOB NO. 18384	APPROVED BY: PROJ. NO. DWG.	REVISIONS 8-16-21 UPDATED — OEW 12-31-21 REVISED PER COUNTY REVIEW COMMENTS — OEW 8-22-22 REVISED PER COUNTY REVIEW COMMENTS — OEW 12-22-22 REVISED PER COUNTY REVIEW COMMENTS — OEW	OLIVER E. WATTS CONSULTING ENGINEER COLORADO SPRINGS	PROJECT ROCKY TOP MOTEL & CAMPGROUND PART NW1/4 SECTION 9, T.13S., R.68W., 6TH P.M. EL PASO COUNTY	SHT. NAME EROSION CONTROL PLAN	SHT. NO. 2 OF