# FINAL DRAINAGE REPORT

# FOR

# CROSSROADS MIXED USE FILING NO. 1
# EL PASO COUNTY, COLORADO

APRIL 2022

Prepared for:
Crossroads Metropolitan District No. 1
Mr. Danny Mientka
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Prepared by:

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Project #18-003A
PCD Filing No.: SF 21-029

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**FOR**
**CROSSROADS MIXED USE FILING NO. 1**

**DRAINAGE PLAN STATEMENTS**

ENGINEERS STATEMENT

The attached drainage plan and report was prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Virgil A. Sanchez, P.E. #37160
For and on Behalf of M&S Civil Consultants, Inc


DEVELOPER'S STATEMENT

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

BY:_____
Danny Mientka –Owner

DATE: 4-22-2022

ADDRESS: Colorado Springs Equities LLC
90 South Cascade Avenue, Suite 1500
Colorado Springs, CO 80903

EL PASO COUNTY'S STATEMENT

Filed in accordance with the requirements of El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Manual, as amended.

APPROVED
Engineering Department
06/09/2022 3:07:47 PM
dsdnijkamp
EPC Planning & Community Development Department

BY:___ _____ DATE:_____

County Engineer / ECM Administrator

CONDITIONS:

# FINAL DRAINAGE REPORT
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**APPENDIX**

# FINAL DRAINAGE REPORT
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### Purpose

This Final Drainage Report for Crossroads Mixed Use Filing No. 1 is in support of the Final Plat, Preliminary Plan, and Construction Drawings of the subject site. This report functions to identify the existing and proposed runoff patterns and recommend proposed drainage improvements which are intended to safely convey runoff through the proposed development, while minimizing impacts to downstream facilities and adjacent properties.

The Final Plat and Construction Drawings for this site will be submitted concurrently with this report. Individual drainage letters and/or reports shall be required with the development of each lot not otherwise clearly analyzed by this report for Crossroads Mixed Use Filing No. 1. This report is subject to changes dependent upon future lot development. In such case, an updated report and accompanying drawings shall be submitted.

### Project Location and Description

The subject site is located at 0 Meadowbrook Parkway in the southwestern quarter of Section 8, Township 14 South, Range 65 West of the 6$^{th}$ P.M. in El Paso County, Colorado. The 29.049 Acre site is currently undeveloped. The site is bound to the west by undeveloped Softball West Subdivision Filing No. 2, to the north by Meadowbrook Crossing Subdivision, south by Highway 24, and to the east by Newt Drive.

The proposed site is will be developed into ten (10) commercial lots, one (1) multifamily residential lot, and three (3) tracts for detention and roadway use. The development will extend Meadowbrook Parkway to the west and will include a single lane roundabout to be constructed at the intersection of the Meadowbrook Parkway and Newt Drive. The property is within the commercial aviation district overlay. A rezone application has been approved to rezone 12.703 acres from CR to the RM-30 Zone.

The majority of the existing site is covered with native grasses with fair to good cover, the exception being portions of the future Meadowbrook Parkway corridor where exposed soils are present. Known earthwork operations for “borrow material” have occurred over a small area of the eastern portion of the site in early to mid 2019, but have since stabilized. A few dirt paths/trails are present along the far west end of the site, likely from recreational vehicles. Generally, the site slopes from east to west slightly greater than 1% with some localized depressions and general terrain undulations near the west boundary that have slopes ranging from 1- 20%. Some of these may be the results of previous earthwork activities. The site lies within the Sand Creek Drainage

Basin. No existing drainage facilities or improvements are onsite. No known irrigation systems or wells are present.

**Soils**

Soils in the project area have been determined to be Blakeland Loamy Sand (8) and Blendon Sandy Loam (10), which are characterized to be part of Hydrologic Soil Types "A" & "B" as determined from the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS) "Web Soils Survey". A soils map illustrating the site location and soil types is provided in the appendix of this report.

**Floodplain Statement**

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Nos. 08041C0754 G & 08041C0752 G, effective date December 7th, 2018, none of the site lies within a designated floodplain. A copy of these annotated maps can be found in the appendix. The Sand Creek East Fork Channel is located to the northwest of the adjacent Meadowbrook Crossing subdivision.

**Previous Studies**

The area which encompasses Crossroads Mixed Use Filing No. 1 has been previously studied. Below is a short outline of the assumptions regarding the lands of the subject site and those based upon the previously assembled and approved drainage reports and how the assumptions within them impact the subject site.

"Sand Creek Drainage Basin Planning Study, Preliminary Design Report", prepared by Kiowa Engineering Corporation, dated January 1993, revised March 1996.

- Establishes that the subject site falls within the East Fork Sand Creek Drainage Basin, a portion of the larger Sand Creek Watershed
- Establishes that there are no requirements for major infrastructure improvements and subsequently no drainage-improvement related reimbursements with the development of this parcel
- Drainage fees shall be required to plat the subdivision

"Claremont Business Park Filing No.2 prepared by Matrix Design Group, revised November 2006

- Establishes the drainage patterns of offsite Basins 0S-4 and E2 which are to be conveyed within the Meadowbrook Rights of Way
- Established up-gradient offsite drainage to be directed under Meadowbrook north to offsite East Fork Sand Creek Channel, and away from the subject site

"Final Drainage Report, Lot 1 24/94 Business Park Filing No.1 prepared by Core Engineering Group, dated July 14, 2016

- The development of the 24/94 Business Park FDR shows future curb inlets along the future Meadowbrook Parkway extension on the south and west corners of the intersection to capture runoff from up-gradient watersheds in addition to a proposed inlet which was to be located above the intersection at the northwest corner of the subject site.
- Establishes that flows from the parcel upstream of the convenience store (29/94 FDR Basin OS4) EX-B now to be collected by the extension of a 36" RCP along the south side of Meadowbrook Parkway. Runoff within the right of way/roadway separated out as Basin EX-A2.
- Continues assumption that flows from Newt Drive be conveyed north to East Fork Sand Creek.
- Evaluated pre-development drainage patterns for subject site including direct discharge flow rates to the CDOT rights of way of 1.9 and 14.5 cfs for the 5 and 100 year events, respectively. (Basin EX-E).

"Preliminary and Final Drainage Report Meadowbrook Crossing Filing No. 1 and Filing 2, El Paso County, Colorado prepared by Kiowa Engineering Corporation, dated July 25, 2017

- Proposed the installation of a future 10' Type R inlet at the southeast corner of Newt Drive and Meadowbrook Parkway with the extension of Meadowbrook Parkway to the west (along the northern boundary of the subject site). The inlet was to function to collect offsite runoff from a portion of the south half of Meadowbrook Parkway and Newt Drive north of Hwy 24. Intercepted runoff would be conveyed via a proposed 24" storm sewer to the existing storm sewer system within the Meadowbrook Crossings development.
- Proposed the installation of a 10' Type R inlet at the west end of future Meadowbrook Parkway. The inlet was to collect runoff from the north half of the future roadway. An 18" storm drain was proposed to convey collected runoff to the existing water quality pond located within the Meadowbrook Crossings Development. The report indicates a separate forebay or the modification of an existing forebay would be required.
- Shifted the location of the existing 10' Type R curb inlet to be installed upstream of the intersection of Newt Drive (as shown with the 24/94 Business Park FDR), flows in excess of the inlet capacity are to continue within the future Meadowbrook.

"Final Drainage Report for Meadowbrook Dirt Borrow Site, El Paso County Colorado, prepared by M&S Civil Consultants, November 2018.

- Evaluated onsite drainage patterns
- Excluded offsite runoff impacts from areas to the east of site.
- Allowed site to be utilized as a "borrow site" for offsite earthwork activities.

**Hydrologic Calculations**

Hydrologic calculations were performed using the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual and where applicable the Urban Storm Drainage Criteria

Manual. The Rational Method was used to estimate stormwater runoff anticipated from design storms with 5-year and 100-year recurrence intervals.

**Hydraulic Calculations**

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual. Grassed swale cross sections were analyzed using an open channel flow calculator with parameters such as a surface roughness coefficient of 0.025 (good condition w/ little to no stones or weeds). Topographical information was used to define swale geometry, and design point flows were used to obtain channel flow depths and velocities at their maximum design capacity for flood and erosion control considerations. Storm drains were designed using parameters and criteria summarized in Chapter 8 of El Paso County's Drainage Criteria Manual Vol. 1 and the City of Colorado Springs Drainage Criteria Manuals. Parameters such as Manning's values of 0.13 were used for concrete pipe flow, and design considerations for minimum freeboard and maximum velocities were applied. The relevant data sheets are included in the appendix of this report. Hydraulic grade line calculations for the storm system in the ultimate (future) condition are provided in the Appendix of this Final Drainage report.

**Drainage Criteria**

This drainage analysis has been prepared in accordance with current El Paso County Drainage Criteria Manual and, where applicable, City of Colorado Springs and Mile High Flood District Criteria Manuals. Calculations were performed to determine runoff quantities for the 5-year and 100-year frequency storms for developed conditions using the Rational Method as required for basins having areas less than 100 acres. See Appendix for supporting calculations.

**Historic (Pre-Grading) Drainage Characteristics**

The historic drainage patterns discussed within this report reflect the site conditions prior to the approval of the 2018 Meadowbrook Dirt Borrow Site Grading and Erosion Control Plan. This 'historic condition' generally coincides with the existing condition analysis and mapping that accompanied that project's hydrologic analysis.

The following excerpt is from the existing Drainage Characteristics section of the Final Drainage Report for Meadowbrook Dirt Borrow Site, El Paso County, Colorado, by M&S Civil Consultants and adequately describes the general site characteristics prior to grading.

"Site vegetation is sparse, consisting primarily of native grasses and weeds. The parcel possesses a ridgeline that bisects the parcel, directing runoff to the south and west boundaries, with slopes varying from 1% to 20%. A few small depressions are located on site, near the west boundary. For the purposes of hydrologic analysis, the small depressions are not considered to detain runoff."

Given the increase in breadth and scope of this study, significant consideration of the impacts of offsite drainage from the adjacent developments will be evaluated. This includes drainage from a small portion of Hwy 24 which combines with flows within existing from portions of Newt Drive. Runoff from this offsite area combines with by-pass flows from two inlets located within existing Meadowbrook Drive, prior to entering the site at its northeast corner. Runoff from these locations ultimately combines with onsite flows within the proposed Meadowbrook Parkway corridor, prior to discharging to downstream properties.

The detailed description of the historic (pre-grading) condition is as follows. Please refer to the historic conditions drainage map which is provided within the appendix of this report.

**Historic Conditions - Detailed Drainage Discussion**

**Design Point 1**

**Basins E2 and EX-A2** geometry were derived from their respective reports. Flow velocity equations, conveyance coefficients, and time of concentration equations have been modified since these reports were approved, therefore, these parameters were remodeled with El Paso's hydrologic criteria current to this report's date. Excerpts of reported calculations for these basins are provided in the Appendix for comparison. **Basin E2 (Claremont Business Park Filing No.2)** consists of a reported 3.86 developed acres of development located along the southeastern half of existing Meadowbrook Parkway, some 1200' northeast of the subject site. Runoff produced by the offsite development (CBPF2 Lot 46) is conveyed to Meadowbrook Parkway at flow rates of Q5=15.1 and Q100=28.6 cfs in the 5 and 100-year storm events respectively. The collected flows combine with runoff from **Basin EX-A2 (Lot 1 24/94 Business Park Filing No.1)** (Q5=2.5, Q100=4.5 cfs) which consists of 0.59 acres of the southeastern half of Meadowbrook Parkway, and is located immediately east of existing Newt Drive. The collected flows from the two basins culminate at **Design Point 1** at peak rates of Q5=14.2 and Q100=26.5 cfs. An existing 10' CDOT Type R at-grade inlet (**Inlet 1**) intercepts flows of Q5=8.4 and Q100=11.1 cfs, with subsequent by-pass flows of 5.8 and 15.4 cfs in the 5 and 100 year events. Surface flows continue west within the south half of existing Meadowbrook Parkway.

**Design Point 2**

**Basin OS-A (Meadowbrook Crossing Filing 1 and 2)** consists of 1.29 acres of the northern half of existing Meadowbrook Parkway located immediately east of Newt Drive. Runoff produced within this basin totals Q5=3.1 and Q100=6.0 cfs. These calculated flows differ 0.1 cfs from reported flows due to the significant digits used for the basin acreage in the flow calculation, yet can be viewed as conservative values since they are higher. An existing 10' CDOT Type R at grade inlet (**Inlet 2**) collects runoff of Q5=3.1 and Q100=5.3 cfs, with subsequent by-pass flows in only the 100 year event of 0.7 cfs. Runoff leaving the design point continuing west within the north half of existing Meadowbrook Parkway.

**Design Point 3**

**Basin OS-1** consists of approximately 1.28 developed acres of existing Newt Drive located along the eastern boundary of the site. Runoff produced within the basin (Q5=5.8 cfs, Q100=10.5 cfs)

combine with flow-by from **DP1** in the intersection at peak flow rates of 9.8 cfs, and 22.5 cfs in the 5 and 100-year storm events.

Surface runoff and by-pass flows from both **DP2** and **DP3** enter **Basin A** in the undeveloped rights of way of future Meadowbrook Parkway, at the northeast corner of the site.

**Design Point 4**

**Basin A** consists of 12.88 undeveloped acres located along the northern boundary that drains from east to west across the subject site. Runoff produced by this basin (Q5=1.9 cfs, Q100=14.2 cfs) combine with flowby from **DP2** and flows from **DP3.** The cumulative runoff at **DP4** of Q5=7.4 cfs and Q100=27.7 cfs discharges onto the adjacent property (Lot 1, Softball West Subdivision 2) along the western boundary of the site, approximately 250' to south of the northern property line.

**Design Point 5**

**Basin B** consists of 13.63 undeveloped acres located along the western side of the subject site. Runoff produced by the basin generally flows from northeast to southwest, discharging onto the adjacent property (Lot 2, Softball West Subdivision 2) approximately 200' north of the southern property line. Runoff reaching the boundary at **DP5** are calculated at 2.5 cfs and 18.2 cfs in the 5 year and 100 year events, respectively.

**Design Point 6**

**Basin C** currently consists of 5.89 undeveloped acres located along the southern boundary of the site. Runoff produced within the basin travels east to west as sheet flow before eventually discharging into the existing barrow ditch which travels along the US HWY 24 CDOT right of way**.** Peak runoff rates reaching the subdivision boundary at **DP6** are calculated at 1.2 cfs and 8.5 cfs in the 5 year and 100 year events respectively.

**Design Point 7**

**Basin OS-2** consists of 4.98 acres of a portion of the northern half of the US HWY 24 roadway and the adjoining, native, grass-lined barrow ditch. Runoff produced within the basin (Q5=8.7, Q100=19.6 cfs) combines with flows from **DP6** at cumulative peak runoff rates of 10.4 and 31.9 cfs in the 5 and 100 year storm events at **DP7**. A modeled hydraulic cross section of the ditch section at **DP7** calculates flow depths of 0.59 feet traveling at a velocity of approximately 4.02 feet per second. Input parameters for this analysis can be found in the Hydraulics section of the Appendix. The roadside ditch at this design point was selected as the suitable downstream outfall, therefore, intermediate events have been routed through the site to compare predevelopment to post development flows at this point. Calculations are provided in the Roadside Ditch Intermediate Events Routing Summary in the appendix.

Runoff from **Design Points 4 and 5** ultimately combine with the flows from **DP7** within the barrow ditch of US Hwy 24 several hundred feet downstream of the subject site. An existing 36" RCP culvert located at the interchange of HWY 24 and Peterson Road aids in conveying a portion of the runoff from the subject site and adjacent offsite areas under the roadway. Flows in excess of the culverts carrying capacity, overtop the roadway before rejoining within a subsequent drainage swale

that parallels the west bound HWY 24 on-ramp. Ultimately flows discharge into the East Fork of the Sand Creek via an existing riprap rundown. Site visits conducted by M&S Civil at the time of the writing of this report, found no significant signs of erosion or deposition along the aforementioned corridor.

A Drainageway Exhibit in the appendix of this report provides an aerial illustration of the aforementioned conveyance route to the channel, which will also serve to function as the emergency overflow path for the proposed site development.

**Existing Drainage Characteristics**

The subject site has been utilized as a "borrow site" to provide surplus earthwork to offsite developments in the area. This recent grading effort occurred during the spring and summer of 2019. At the request of El Paso County, an existing conditions drainage analysis has been provided to show the changes to the topography and drainage patterns as a result of this effort. As the only changes between the two conditions are onsite, the offsite drainage patterns calculations and assumptions determined within the historical analysis will remain the same. Specifically, basins **E2**, **EX-A2**, and **OS-2** remained the same. This correlates to **DP1**, **DP2**, and **DP3** remaining the same. It should be noted that the subject site was not disturbed to the full extent of the approved plan, with limited excavation primarily occurring within the eastern side of the site.

In the existing condition, vegetation remains sparse, consisting primarily of native grasses and weeds with good to fair cover. Areas disturbed by grading activities were reseeded and have since stabilized. With regards to historic versus existing drainage basin delineation, the bisecting parcel ridgeline has been relocated further to the south, which results in redirecting more of the runoff to the southwestern part of the site and less to the CDOT rights of way. The few small depressions remain on site, near the west boundary. For the purposes of hydrologic analysis, these small depressions will continue to not be evaluated for their ability to detain runoff. Ultimately, all runoff from the parcel is conveyed to the west towards existing drainage facilities located under Peterson Road and ultimately the East Fork of Sand Creek as in the historic condition.

This section only discusses the changes in basin geometry and drainage pattern and provides a direct comparison of the historic versus existing conditions, utilizing the same outfall (design) points, which have remained undisturbed.

**Design Point 4**

**Basin A** (Q5=1.5, Q100=11.1 cfs) currently consists of 11.02 acres which continues to drain from east to west eventually discharging along the western boundary of the site, approximately 250' south of the northern property line. Peak runoff, post-grading, has decreased to 7.1 and 25.5 cfs as compared to the historic condition flow rates of 7.4 and 27.7 cfs in the 5-year and 100-year events respectively.

**Design Point 5**

**Basin B** (Q5=2.0, Q100=14.5 cfs) consists of 17.31 acres that drains from northeast to southwest, eventually discharging along the western boundary of the site, approximately 200' north of the

southern property line. Peak runoff rates at this location are also than lower than the historic conditions with post grading flows of 2.0 cfs and 14.5 cfs, as compared to 2.5 cfs and 18.2cfs in the 5-year and 100-year events respectively. Despite the basin currently being larger in area than in the historic condition, a decrease in the peak flow rates occurs as a result of a longer flow path to the given design point.

**Design Point 6**

**Basin C** consists of 3.99 undeveloped acres that drains from east to west into the US HWY 24 Right of Way at the southern boundary of the site. The peak runoff at this location is less than the historic condition at an estimated 0.9 and 6.3 cfs, as compared to 1.2 and 8.5 cfs in the 5-year and 100-year events, respectively.

**Design Point 7**

**Basin OS-2** (Q5=8.7, Q100=19.6 cfs) consists of 4.98 acres of the northern half of the US HWY 24 roadway and adjoining native grass lined barrow ditch. Runoff produced within the basin combines with runoff from the subject site at lower cumulative peak runoff rates of 9.9 and 28.0 cfs in the 5 and 100-year storm events at **DP7** as compared to 10.4 and 31.9 cfs in the historic condition. A cross section of the ditch at this location was analyzed in the 100 yr event for comparison purposes and is provided in the appendix.

**Four Step Process**

**Step 1** **Employ Runoff Reduction Practices** – Approx. 2.54 acres of the proposed development is being set aside for a Full Spectrum Detention (FSD) Pond. Whenever possible, runoff produced within developable area containing impervious surfaces will be routed through landscaped areas or earthen swales (grass-lined where slope exceeds 2%) to minimize direct connection of impervious surfaces. In the interim, runoff will be reduced through the use of (4) temporary sediments ponds until the ground has been stabilized with vegetation or permanently developed.

**Step 2** **Stabilize Drainageways** – The development of this site is not anticipated to have negative effects on downstream drainage ways since flows released will be below historic rates. In the interim, the site proposes four temporary sedimentation ponds, before discharging at the southwest property corner of the site and onto an adjacent undeveloped property via riprap-lined spillways. This ensures that in this stage of the development negative effects on the downstream drainage ways will be avoided.

In the proposed and future conditions, the flow is discharged to the same location offsite through an RCP pipe outfall lined with rip rap. From here it continues southwest in CDOT's man-made roadside ditch until it reaches Peterson Road. It is then conveyed to the other side of the road, into a similar earthen channel, via a 36" CMP culvert. The drainage continues southwest in the right of way, until it reaches the East Fork Sand Creek Channel. Existing rip rap barriers are lined throughout this portion of the pathway approximately every 90-100 feet within the ditch to the channel bank. The Drainageway Exhibit provided in the Drainage Maps section of the Appendix provides a visual representation of this information. Roadside ditch calculations for various storm events are provided

for the selected suitable downstream outfall (project site's discharge location) to ensure the facility can adequately contain and convey the flows.

**Step 3 Provide Water Quality Capture Volume (WQCV)**– The site will use a Full Spectrum Detention (FSD) Pond to control developed runoff that is discharging into an existing CDOT ROW roadside ditch and ultimately into Sand Creek. The FSD pond's outlet structure will be designed to drain the water quality event storm in 40 hours, while reducing the 100 year peak discharge to approximately 90% of the predevelopment conditions.

**Step 4 Consider Need For Selecting Industrial And Commercial BMP's** – The proposed development will implement a Stormwater Management Plan including property housekeeping practices, spill containment procedures, and coverage of storage/handling areas. Specialized BMP's are not required since the vertical development of the commercial areas are unknown at this time.

**Future Drainage Characteristics**

The future site will be developed into ten (10) commercial lots, one (1) multifamily residential lot, and three (3) tracts for detention and roadway use. The proposed development will extend Meadowbrook Parkway to the west and will include a single lane roundabout to be constructed at the intersection of the Meadowbrook Parkway and Newt Drive to aid in traffic control. A proposed private looped roadway, consisting of Southern Rail Point and Pacific Rail Point will extend into the site to provide access and a utility corridor to both the commercial and residential developments. At this time, it is anticipated that the development and design of Lot 11 (by others) is planned to occur concurrently with the construction of the proposed utilities and infrastructure provided by this plan. A separate drainage letter or report will be required for that portion of the development.

The following summary generalizes the proposed drainage patterns and drainage improvements required to safely route developed runoff to downstream facilities.

A storm sewer pipe and inlet will be constructed at the southwest corner of the newly constructed roundabout to aid in collecting runoff reaching the site from offsite watersheds. These facilities will connect to the existing system located inside the existing Meadowbrook Subdivision. Bypassed flows and developed flows within the newly constructed Meadowbrook Parkway will be collected by a pair of sump inlets located at the west end of the roadway. The drainage facilities located with the rights of way will be public and all remaining onsite storm sewer and drainage improvements shall be private. A future, private, looped roadway will provide access and utility corridors for development. Private storm sewer mains, stubs, and inlets will be extended along these corridors to serve the development. The extension of these facilities beyond what is shown by this plan is likely with future development. Runoff collected by the infrastructure will be conveyed to a single full spectrum detention pond located in the southwest corner of the subdivision. The proposed outfall from the pond is planned to discharge into the existing barrow ditch located with the north half of the existing CDOT Right of Way. A drainage easement will be required from CDOT for the outfall and slope protection facilities that fall within the corridor. It

should be noted that the storm outfall will be shaped into the existing hillside and any soil riprap protection will be buried. Runoff leaving the site and entering the CDOT corridor will discharge at greater-than-historic rates in certain events, but the anticipated effects of this increase to downstream infrastructure has been concluded to be negligible. A detailed discussion of the comparative analysis associated with this conclusion is provided with **Design Point 16** below. The previous discharge points along the west boundary of the subject site, which also previously contributed to the barrow ditch will be virtually eliminated, further reducing the peak flow rates to downstream facilities.

**Future Detailed Drainage Discussion**

**Design Point 1**

**Basin E2 (Claremont Business Park Filing No.2)** consists of a reported 3.86 developed acres of development located along the southeastern half of existing Meadowbrook Parkway some 1200' northeast of the subject site. Runoff produced by the offsite development (CBPF2 Lot 46) is conveyed to Meadowbrook Parkway at flow rates of Q5=15.1 and Q100=28.6 cfs in the 5 and 100-year storm events respectively. The collected flows combine with runoff from **Basin EX-A2 (Lot 1 24/94 Business Park Filing No.1)** (Q5=2.5, Q100=4.5 cfs) which consists of 0.59 acres of the southeastern half of Meadowbrook Parkway located immediately east of existing Newt Drive. The collected flows from the two basins culminate at **Design Point 1** at peak rates of Q5=14.2 and Q100=26.5 cfs. An existing 10' CDOT Type R at-grade inlet (**Inlet 1**) intercepts flows of Q5=8.4 and Q100=11.1 cfs, with subsequent by-pass flows of 5.8 and 15.4 cfs in the 5 and 100 year events. Surface flows continue west within the south half of existing Meadowbrook Parkway.

**Design Point 2**

**Basin OS-A (Meadowbrook Crossing Filing 1 and 2)** consists of 1.29 acres of the northern half of existing Meadowbrook Parkway located immediately east of Newt Drive. Runoff produced within this basin totals Q5=3.1 and Q100=6.0 cfs. An existing 10' CDOT Type R at grade inlet (**Inlet 2**) collects runoff of Q5=3.1 and Q100=5.3 cfs, with subsequent by-pass flows in only the 100 year event of 0.7 cfs. Runoff leaving the design point continuing west within the north half of existing Meadowbrook Parkway.

**Design Point 3**

In accordance with the assumptions outlined within the Meadowbrook Subdivision Final Drainage Report, an offsite public storm sewer pipe and inlet will be constructed at the southwest corner of the proposed roundabout to aid in collecting runoff from a portion of the offsite watershed located to the east of the site. A new manhole is not anticipated to be required to connect the outfall to the existing pipe located inside the existing Meadowbrook Subdivision. As this area is already paved, increases to the imperviousness of this area are not anticipated.

**Basin OS-1** consists of approximately 1.40 acres of existing Newt Drive that will be retrofitted with new raised median as part of an intersection conversion to a roundabout. Runoff produced within the basin (Q5=6.4 and Q100=11.5 cfs) will combine with flow-by from **DP1** at peak rates of Q5=10.2 and Q100=23.3 cfs at a proposed public 10' at-grade inlet (**Inlet 3**: Q5=6.7, Q100=9.8 cfs

intercepted; Q5=3.5, Q100=13.5 cfs flowby) located at **DP3**. A proposed public 24" storm sewer (**PR1**) will convey water across the intersection to the existing 42" storm sewer with Meadowbrook Crossings in accordance with that subdivision's drainage report. The existing manhole connection has been determined to be sufficient following construction of this proposed inlet and storm sewer. It is important to note that this connection also remains feasible as the roundabout is not anticipated to significantly increase the overall imperviousness of the area above that of the existing condition. Runoff in excess of the inlet capacity will continue westward via the curb and gutter of Proposed Meadowbrook Parkway.

**Design Point 4**

**Basin A** consists of 1.67 acres of the north half of proposed Meadowbrook. Runoff within this basin (Q5=6.5 and Q100=11.6 cfs) combines with flow by from **DP2** for total flows of 6.5 and 12.4 cfs in the 5 year and 100 year events, respectively. A proposed 15' at-grade inlet (**Inlet 4:** Q5=6.5, Q100=10.6 cfs intercepted; Q5=0.0, Q100=1.8 cfs flowby) is located at the west end of the roadway just before the proposed temporary cul-de-sac. This inlet conveys intercepted flows to **PR1.5**, a proposed 24" RCP public storm sewer. Flowby from the 100 year event continues west to downstream infrastructure.

**Design Point 4.5**

1.8 cfs of flowby in the 100 year event continues west from **DP4** towards **Inlet 4.5**, a **NEENAH R-2501 Type C Grate** lid and frame at the low point of the cul-de-sac. Supporting calculations for this non-standard inlet are provided in the Appendix. This inlet is anticipated to reach a maximum depth of 0.5' in order to convey this flow underneath the roadway via a proposed public 24" storm sewer (**PR2**). The NEENAH inlet is to be removed and replaced with a standard CDOT 5' Type R inlet when the roadway cul de sac is removed and the roadway is extended to the west with future development. In the case of inlet clogging, overflow will collect at **DP5**, which has an additional 13.3 cfs capacity.

**Design Point 5**

**Basin B** consists of 1.48 acres of the south half of proposed Meadowbrook Parkway. Runoff produced within this basin (Q5=5.8 and Q100=10.3 cfs) combines with flow-by leaving **DP3** at peak flowrates of Q5=9.8, Q100=25.8 cfs. A proposed public 15' sump inlet (**Inlet 5:** Q5=9.8, Q100=25.8 cfs intercepted; no flowby) located at west end of the roadway will prevent developed flows from leaving exiting the roadway corridor. The intercepted runoff will combine with **PR2** flows in a 36" private storm sewer system (**PR3, by others**). Combined flows within the proposed system are calculated to reach peak rates of 16.2 and 37.9 cfs. The storm sewer system is to be planned by others through the multi-family site (Lot 11) but ultimately will tie back into the system at **DP15**. In case of inlet clogging, overflows will overtop the curb on the southern side onto the apartment site and be conveyed to the swale on the west side of the site.

**Design Point 6**

**Basin C** (Q5=18.7, Q100=34.5 cfs) consists of 4.61 acres of commercial lots (1-5 and portions of lot 6) located along the east side of the site. Earthen swales are proposed to convey flows along the basin edge to the proposed depression. Rip rap (Type H, D50=1.5 ft, 3' thickness) is proposed at the terminus and will protect the slopes of the depression. A future private 30" storm sewer (**PR4-PR7**)

is provided to collect and convey flows of Q5=18.7 and Q100=34.5 cfs in the 5 and 100-year storm event, respectively. **PR4.5** is a 30" private stub provided to assist in intercepting flows from future development of the commercial lots, and therefore does not receive any flows in this condition. Intercepted flows are conveyed west underground within the roadway tract. Rip rap sizing was determined with the use of the Steep Slope Rip Rap Design charts from the Surface Mining Water Diversion Manual and is provided in the appendix. Flow to the depression considered the 2:1 longitudinal slope into the depression, 2:1 side slopes in the depression, and was assumed to spread and encompass a 6' base width at the entry point of the depression from the swale. The rip rap sizing at this design point was conservatively used at other depressions around the site due to having the largest flow accumulation.

**Design Point 7**

**Basin D** consists of 2.22 acres of commercial lots located between Meadowbrook Parkway and the looped roadway. **Basin D**, which includes portions of Lots 9 and 10, will require a private 24" storm drain (**PR8**) to collect peak flows of Q5=9.3 and Q100=17.0 cfs from this basin in the 5 and 100 year storm events, respectively. Earthen swales are proposed to convey flows along the basin edge to the proposed depression. Rip rap (Type H, D50=1.5 ft, 3' thickness) is proposed at the terminus of the swale and will protect the slopes of the depression. Rip rap was conservatively sized using **DP6's** analysis.

**Design Point 8**

**Basin E** (Q5=4.1, Q100=7.4 cfs) consists of 1.04 acres of commercial lots and roadway located in the central portion of the site. A private 10' CDOT Type R at-grade inlet (**Inlet 6:** Q5=4.0, Q100=6.0 cfs intercepted; Q5=0.1, Q100=1.4 cfs flowby) is located on the north side of the roadway to intercept flows. Runoff bypassing this inlet continues to downstream infrastructure. Flows collected from the inlet combined with **PR8** and are conveyed to a box base manhole in the middle of the planned roadway via a private 30" (**PR9**) storm drain.

**Design Point 9**

**Basin E1** (Q5=6.4, Q100=11.7 cfs) consists of 1.67 acres of commercial lots and roadway located in the central portion of the site. A private 10' CDOT Type R at-grade inlet (**Inlet 7:** Q5=5.5, Q100=7.7 cfs intercepted; Q5=0.9, Q100=4.0 cfs flowby) is located on the south side of the roadway to intercept flows. Runoff bypassing this inlet continues to downstream infrastructure. Flows collected from the inlet are conveyed to a box base manhole in the middle of the planned roadway via a private 18" (**PR10**) storm drain. **PR7** and **PR9** also collect at this junction. A proposed 36" private storm sewer (**PR11**) will then convey flows west underground at peak flow rates of 35.0 and 60.5 cfs in the 5 and 100-year events. **PR12**, a 42" private storm sewer, then directs the system south from another manhole. Pipe flows from the proposed apartment site (**PR11.5**, private 24" RCP) combine with **PR12** in **PR12.5**, a proposed private 48" storm drain.

**Design Point 10**

**Basin G** (Q5=2.1, Q100=3.8 cfs) consists of 0.46 acres of multi-family lots and roadway located in the central portion of the site. A private 10' CDOT Type R sump inlet (**Inlet 8**: Q5=2.1, Q100=3.8 cfs; no flowby) located on the west side of the street functions to collect the runoff from **Basin G. PR13**, a proposed 18" private storm sewer, will direct runoff east to a box base manhole at peak

flow rates of 2.1 cfs and 3.8 cfs in the minor and major storm events, respectively. In the case of inlet clogging, overflow is directed to the swale at **DP13**.

**Design Point 11**

**Basin G1** (Q5=2.8, Q100=5.0 cfs) consists of 0.60 acres of commercial lots and roadway located in the central portion of the site. A private 15' CDOT Type R sump inlet (**Inlet 9:** Q5=3.7, Q100=15.3 cfs intercepted; no flowby), located on the east side of the street functions to collect the runoff from **Basin G1** as well as bypass flows from **DP8 and DP9**, totaling Q5=3.7 and Q100=15.3 cfs. **PR14**, a proposed 30" private storm sewer, will direct runoff west to an underground box base manhole at peak flow rates of 3.7 cfs and 15.3 cfs in the minor and major storm events, respectively. From the junction, flows from **PR12.5**, **PR13**, and **PR14** combine at **PR15** (Q5=48.0, Q100=93.7 cfs), a 48" private storm sewer, and are directed south. In the case of inlet clogging, overflows will overtop the curb and collect in the rip rap protected depression at **DP12**.

**Design Point 12**

**Basin F** consists of 2.57 acres of commercial lots (Lot 8 and portions of Lot 7) located along the southern boundary of the site. An earthen swale is proposed to convey flows to the depression. Rip rap (Type M, D50=1.5 ft, 3' thickness) is proposed at the terminus of the swale and will protect the slopes of the depression. Rip rap was conservatively sized using **DP6's** analysis. A private 24" storm drain (**PR16**) is provided to collect the basin flows of Q5=10.8 and Q100=19.6 cfs at **DP12** in the 5 and 100 year events, respectively. Intercepted flows are conveyed west underground to the main line where they combine with flows from **PR15** at a manhole junction. **PR17,** a private 48" RCP storm sewer directs the collected runoff to a manhole which joins with **PR21** (private 48" RCP) at peak flow rates of Q5=57.9 and Q100=112.1 cfs. The collected flows are conveyed southwest in **PR18** (Private 48" RCP) until discharging into the proposed forebay at **DP15**.

**Design Point 13**

**DP13** consists of a 2' bottom earthen swale that is designed to convey overflow runoff from the proposed apartment site (**Basin A-5 Overflow**: Q5=0.9, Q100=7.8 cfs, **Basin Z-1**: Q5=0.47, Q100=1.27 cfs, and **Basin D-1 Overflow**: Q5=0.0, Q100=1.5 cfs) to the northwest corner of the pond. This swale joins another on the west end of the property (**DP14**) that ultimately conveys flows into the pond. Overflows from the apartment site were obtained by using flowby from the "Final Drainage Report for Aura at Crossroads" MHFD inlet sheets, which are provided in the appendix. The maximum runoff expected at **DP13** is 1.3 and 10.9 cfs in the 5 and 100 year events, respectively. Calculations for this swale (Section C-C') are included in the appendix of this report.

**Design Point 14**

**DP14** represents the on-site portion of a proposed, v-shaped, earthen swale that collects flows not anticipated to be collected by the apartment site's storm sewer (**Basin Z-2**: Q5=0.57, Q100=1.43 cfs), and combines with flows from **Design Point 13**. Runoff collected within this swale (maximum Q5=2.0 cfs, Q100=9.7 cfs) is conveyed from north to south to the proposed FSD pond at **DP15**. Calculations for this swale before (Section B-B') and after (Section D-D') the junction are provided in the appendix of this report. Anticipated flows for **Basin Z-2** from "Final Drainage Report for Aura at Crossroads" were used to determine swale cross section prior to the junction location, and combined flows with **DP13** were used for after. North American Green SC-250

erosion control blanketing or approved equal shall be used as swale protection and was selected based on flow velocity.

**Design Point 15**

**Basin J** consists of 3.21 acres of the proposed Tract for the full spectrum detention pond. Runoff produced within this basin reaches peak runoff rates (Q5=2.3 and Q100=10.0 cfs) combines with flows from **DP14** and **PR18** (proposed 48" private RCP) in the pond. **PR19** (proposed 48" private RCP) represents the tie in point for the apartment site storm sewer, and conveys collected flows into the proposed forebay. The cumulative flows at **FSD Pond 1** are Q5=116.7 and Q100=235.0 cfs. Flow exiting the pond will be routed to the existing 5' bottom earthen swale (Proposed Section A-A' Analyses) in CDOT's Right of Way at **DP16** via 18" private **PR20** (Q5=1.2 and Q100=11.4 cfs). A rip rap pad (Type L, D50=9") is provided as outlet protection. Refer to the Appendix for rip rap sizing calculations.

**Design Point 16**

**Basin OS-2** consists of 4.98 acres. Approximately half of this basin is comprised of the paved surface of U.S. Highway 24, while the other half is comprised of the 5 foot bottom earthen swale in CDOT's Right of Way. Runoff produced within this basin (Q5=8.7 and Q100=19.6 cfs) flows from northeast to southwest, combining with outfall flows from **DP15**. This combination of runoff collects in the existing swale in the right of way. The pond releases flows at Q5=1.2, Q100=11.4 cfs. The cumulative flows at **DP16** are Q5=9.9 and Q100=31.0 cfs, which are lower than historic. See below discussion for predevelopment, historic, and future condition comparison for the intermediate events at this location. Flows from this design point continue to downstream infrastructure.

**Roadside Ditch Intermediate Events: Drainage Summary**

The MHFD Pond Routing worksheet shows that future discharge rates are greater than allowable predevelopment rates. This is largely a result of the small separation between intermediate events (~0.3') due to pond geometry and configuration limitations, as well as drainage pattern assumptions involved in the CUHP procedure. The pond has been designed for maximum, non-jurisdictional depth to allow for the most separation possible between the events and have the most flow control through the outlet structure. Since the three historic site discharge points have been condensed to a single point-discharge location in the future condition and predevelopment discharge peaks were shown to be greater using the MHFD approximation, a comparative rational analysis has been provided to more accurately describe and compare release rates at **Design Point 16**.

**Design Point 7** in the historic condition has been compared to **Design Point 16** in the future condition. Rational calculations for the 5, 10, 25, 50, and 100 year events for the swale are provided and compared with historic patterns in the "Roadside Ditch Intermediate Events: Routing Summary" in the Appendix. To accomplish a rational comparison, pond peak discharge flows from the MHFD worksheet were converted into "CA" values using rainfall intensities derived from **Basin OS-2** time of concentration, creating the conservative assumption that peak discharge rates from the pond occur at this point in time. Following the analysis, all except the 25 and 50 year events are lower than the historic condition. Historic flows for the 25 and 50 year events are reported as 21.1 and 26.2 cfs, respectively. Future flow conditions at the discharge point for these

two events are 23.2 and 27.7 cfs, respectively. This is approximately a 9% and 6% increase relative to the historic flows. Open channel cross section analyses were constructed to model the effects of the discharge increase in these two events. These cross sections have also been provided in the Appendix. An increase of ~.1 ft/sec from 3.6 and 3.8 ft/sec in the historic condition was observed for both events. Due to this relative increase, resulting velocities are less than 5 ft/sec, and the potential for a 5% error introduced in these successive calculations, flow and velocity increase impacts have been determined to be negligible in terms of erosion effects in the ditch and downstream infrastructure.

**Proposed Drainage Characteristics**

In the proposed condition Lot 11, (apartment site), Tract C, and Meadowbrook Parkway infrastructure will be constructed and Tract D (future 10 commercial lots) will remain undeveloped. Since the future (full-buildout) condition was used to size this infrastructure and has been shown to adequately convey site drainage to the downstream facilities, the undeveloped characteristics of Tract D cause lower contributions to overall flows that are conveyed to downstream facilities. Calculations have been provided in the appendix notating these characteristics. Parks and cemeteries runoff coefficients were used to analyze the undeveloped area drainage. Surface flows at **DP's 1-5**, **DP10**, **DP11**, **DP13**, and **DP14** remain the same as the future condition. Pipe flow analysis was simplified to a comparison of the affected upstream storm sewer (**PR11**) to the manhole junction at **PR17** for this condition since it has been shown that the entire system sufficiently serves the future condition. A detailed drainage discussion for the undeveloped portion of the site (Tract D) in the proposed condition is provided below that highlights and summarizes the results of this analysis.

**Design Point 7**

**Basin P1** consists of 8.97 undeveloped acres. Runoff produced within this basin (Q5=3.8 and Q100=20.7 cfs) flows from northeast to southwest and collects in a proposed swale parallel to Tract C that discharges into a temporary sediment basin (**SB2**). Flows from the sediment basin discharge into a proposed swale to the south to continue to downstream infrastructure. In the case of clogging, overflow will be directed to the swale to the south. Since no flows at this location enter the storm system, **PR11** and **PR12** convey no flow in the proposed condition. **PR11.5** conveys flows from the apartment site into the trunk main at **PR12.5** (Q5=6.9, Q100=13.8 cfs). Inlets 8 and 9 function as they do in the future condition and combine with **PR12.5** at cumulative flow rates of Q5=10.8 and Q100=27.0 cfs at **PR15** in the proposed condition. See below for continued discussion of the pipe conveyance to and from **DP12**.

**Design Point 12**

**Basin P2** consists of 3.04 undeveloped acres. Runoff produced within this basin (Q5=1.3 and Q100=7.2 cfs) flows from northeast to southwest, combining with outfall flows from **DP7**. Detention effects from the sediment basin was not considered, therefore, inflow was considered equal to outflow as a conservative analysis. This combination of runoff collects in a proposed swale parallel to Tract C that discharges into a proposed sediment basin (**SB3**). The sediment basin outfall discharges onto a rip rap protected depression at the design point. In the case of overflow, flows will be directed to this same location. Runoff then enters the storm drain system at

proposed 24" private RCP **PR16** (Q5=5.1, Q100=27.9 cfs). A manhole junction joins flows from **PR15** and **PR16** in **PR17** (Q5=15.9, Q100=54.7 cfs). The storm system at this location is considerably less than the future condition (Q5=57.0, Q100=110.1 cfs). Flows continue through the storm drain system until discharging in the eastern forebay of the FSD pond. Backwater effects in the storm system are considered negligible and were not analyzed due to the reduction in flows at the aforementioned entry points and sheer volumetric reduction in flow. An assumption was also made that the system (**PR11-PR18**) in this condition will not be pressurized from the results of this analysis.

**Water Quality Provisions and Maintenance**

The proposed full spectrum detention (FSD) pond functions to provide detention and water quality for the proposed development. This full spectrum detention pond will function to treat approximately 32.20 acres of 78.67% impervious, tributary area by providing 0.863 acre-feet of storage for the water quality event, 3.295 acre feet of storage at the EURV storm event, and 4.668 acre-feet of storage in the 100-year event. The 33' wide emergency spillway is designed with a foot of freeboard in the 100-year event. This spillway safely conveys flows to CDOT's Right of Way in the event of outlet clogging or failure, and will be armored with permanent erosion control fabric and Type M (D50=12") soil rip rap. Rip Rap sizing calculations for the embankment protection are provided in the appendix. The results show that the FSD pond remains functional in the 100-year event and the outlet structure is able to safely discharge flows to an existing swale and ultimately to Sand Creek. Discharge and velocity increases for the 25 and 50 year events in the swale have been determined to be negligible following a comparative, rational analysis that was developed to more accurately describe intermediate events at the site discharge location. The calculations for this analysis can be viewed in the Roadside Ditch Intermediate Events Drainage and Routing Summaries calculations in the appendix. Cross sections of the swale in these conditions can also be found there. The sizing for the full spectrum detention facility has been determined using the guidelines set forth in the Urban Drainage and Flood Control District Criteria Manual. Refer to the UDFCD MHFD-Detention, Version 4.03, Excel Workbook located within the appendix of this report for calculations.

The proposed FSD pond will be privately owned and maintained by Crossroads Metropolitan District No. 1. Access to the pond shall be granted to the owner/district and El Paso County for access and maintenance of the private facility. A private maintenance agreement document shall accompany this report submittal.

**Erosion Control**

It is the policy of the El Paso County that M&S Civil Consultants submit a grading and erosion control plan with the drainage report. The plan includes proposed silt fence, vehicle tracking control, (4) temporary sediment basins, and straw bale barriers as proposed erosion control measures. The plan also includes provisions for stockpiling, staging, and concrete washout areas. A stormwater management plan is provided to accompany the plans.

**2022 Drainage & Bridge Fees:**

| | | | | | | | |
|---|---|---|---|---|---|---|---|
| **Drainage Fees:** | 17.033 | x | 78.67% | x | $21,814.00 | = | $ 292304.57 |
| **Bridge Fees:** | 17.033 | x | 78.67% | x | $8,923.00 | = | $ 119,566.96 |
| | | | | | | **Total** | **$ 411,871.53** |

Drainage fees shall be paid at the time of platting. Tract D drainage fees are not included and will be paid at the time of platting. Future development of these lots shall require individual drainage reports.

**Construction Cost Estimate (Non-Reimbursable)**

| Item | Amount | Unit | Unit Cost | Total Cost |
|---|---|---|---|---|
| 10’ CDOT Type R Inlet | 4 | EA | $ 9,890.00 | $ 39,560.00 |
| 15’ CDOT Type R Inlet | 3 | EA | $ 13,002.00 | $ 39,006.00 |
| Custom Grate Inlet | 1 | EA | $ 5,000.00 | $ 5,000.00 |
| Type I MH | 8 | EA | $ 9,800.00 | $ 78,400.00 |
| Type II MH | 1 | EA | $ 6,000.00 | $ 6,000.00 |
| Rip Rap Aprons | 84.5 | CY | $ 65.00 | $ 5,492.50 |
| 18” SD | 113 | LF | $ 45.00 | $ 5,085.00 |
| 24” SD | 232 | LF | $ 81.00 | $ 18,792.00 |
| 30" SD | 432 | LF | $ 100.00 | $ 35,800.00 |
| 36" SD | 16 | LF | $ 124.00 | $ 1,984.00 |
| 42" SD | 396 | LF | $ 166.00 | $ 65,736.00 |
| 48" SD | 395 | LF | $ 202.00 | $ 79,790.00 |
| Concrete Channel | 2,416 | SF | $ 5.00 | $ 12,080.00 |
| Outlet Structure | 1 | EA | $ 15,000.00 | $ 15,000.00 |
| Forebay | 2 | EA | $ 8,000.00 | $ 16,000.00 |
| Gravel (Access) | 629 | CY | $ 52.00 | $ 32,708.00 |
| Spillway | 1 | EA | $ 20,000.00 | $ 20,000.00 |
| **TOTAL COST:** | **$** | | | **476,433.50** |

M & S Civil Consultants, Inc. (M & S) cannot and does not guarantee the construction cost will not vary from these opinions of probable costs. These opinions represent our best judgment as design professionals familiar with the construction industry and this development in particular. The above is only an estimate of the facility cost and drainage basin fee amounts in 2022.

**Summary:**

The construction of this site is for the purposes of creating a commercial tract, detention tract, and an apartment site in the proposed condition. In the future condition, the commercial tract is proposed to be platted into ten lots. This condition was analyzed to appropriately size the infrastructure for full buildout of the site. The site will be graded and all disturbed areas will be seeded. Post construction runoff will be discharged to downstream property at rates that are below historic discharge rates for the 5 and 100 year design events. Intermediate events within this swale were rationally analyzed since peak discharge rates were shown to be greater than predevelopment

peaks, largely due to pond geometry and configuration limitations. In the historic condition, the total flows leaving the site that reach the East Fork Sand Creek Channel are 10.4, 14.8, 21.1, 26.2, and 31.9 cfs in the 5, 10, 25, 50, and 100 year storm events, respectively. Flows leaving the site in the future condition for these respective events are 9.9, 13.7, 23.2, 27.7, and 31.0 cfs. Negligible erosion impacts have been concluded from the minor flow and velocity increase in flows in the 25 and 50 year events following this analysis. The ditch is also being adequately protected with rip rap and a concrete toe wall to prevent erosion and scouring at this location. Erosion control measures will be implemented throughout the site to prevent sediment migration. The construction of Crossroads Mixed Use Filing No. 1 shall not adversely affect adjacent or downstream property. Subsequent drainage reports will be required when the site is developed behind the uses defined within this report.

## References


1.) "El Paso County and City of Colorado Springs Drainage Criteria Manual".

2.) "Urban Storm Drainage Criteria Manual"

3.) SCS Soils Map for El Paso County.

4.) Flood Insurance Rate Map (FIRM), Federal Emergency Management Agency, Revised date December 7th, 2018.

5.) "Final Drainage Report for Claremont Business Park Filing No. 2", dated November 2006, by Matrix Design Group, Inc.

6.) "Preliminary and Final Drainage Report Meadowbrook Crossing Filing 1 and Filing 2", dated July 25, 2017, by Kiowa Engineering Corporation.

7.) "Final Drainage Report Lot 1 24/94 Business Park Filing No. 1 on Platte Avenue and Meadowbrook Parkway", dated April 28, 2016 and revised July 14, 2016, by Core Engineering Group, LLC.

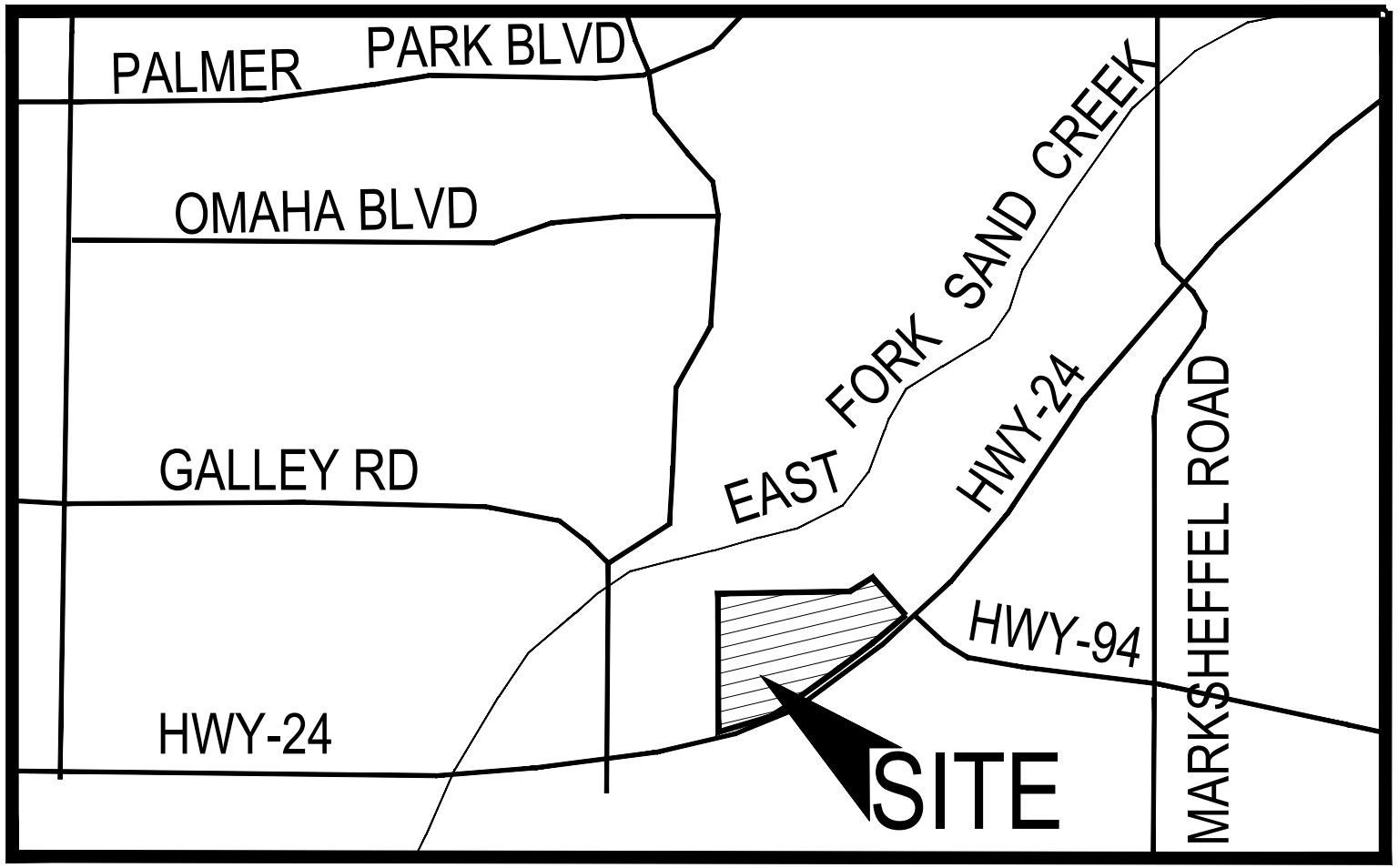
8.) "Final Drainage Report for Meadowbrook Dirt Borrow Site ", dated November 2018, by M&S Civil Consultants, Inc.

9.) "Sand Creek Drainage Basin Planning Study", revised March 1996, by Kiowa Engineering Corporation.

10.) “Final Drainage Report for Aura at Crossroads”, dated April 4th, 2022, by Harris Kocher Smith.

# APPENDIX

**VICINITY MAP**




# VICINITY MAP

N.T.S.

**SOILS MAP**


Map Scale: 1:4,120 if printed on A landscape (11" x 8.5") sheet.

N

Meters
0 50 100 200 300

Feet
0 200 400 800 1200

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

## MAP LEGEND

**Area of Interest (AOI)**

- Area of Interest (AOI)

**Soils**

- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

**Special Point Features**

- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

**Water Features**

- Streams and Canals

**Transportation**

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

**Background**

- Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 15, Oct 10, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 3, 2014—Jun 17, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Map Unit Legend

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
|---|---|---|---|
| 8 | Blakeland loamy sand, 1 to 9 percent slopes | 35.2 | 95.4% |
| 10 | Blendon sandy loam, 0 to 3 percent slopes | 1.7 | 4.6% |
| **Totals for Area of Interest** | | **36.9** | **100.0%** |

**FIRM PANELS**

# NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

**Coastal Base Flood Elevations** shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

**Base Map** information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, City of Fountain, Bureau of Land Management, National Oceanic and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

**Corporate limits** shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

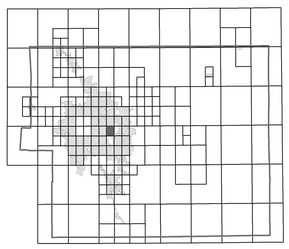
Contact **FEMA Map Service Center** (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

**El Paso County Vertical Datum Offset Table**

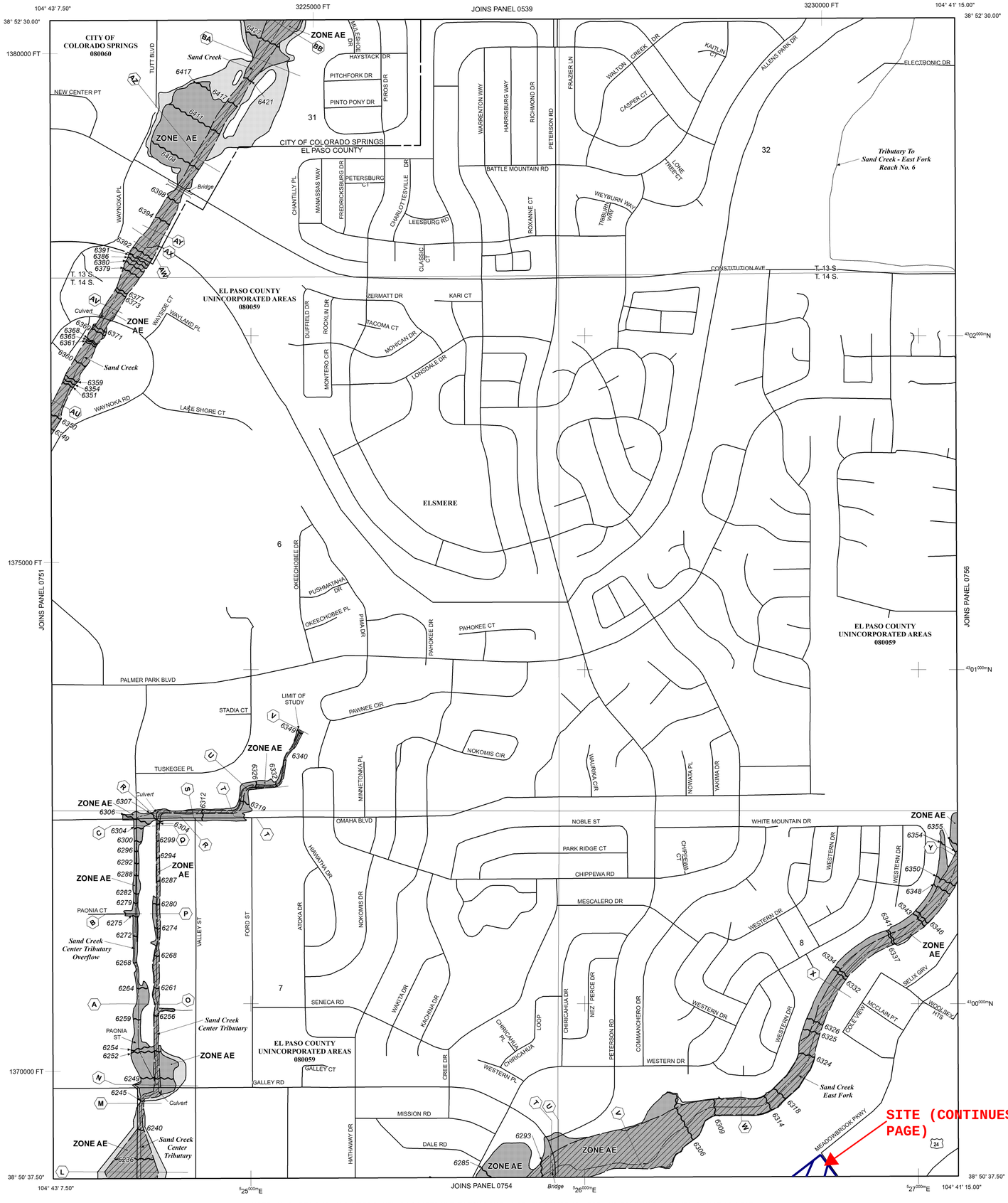
| Flooding Source | Vertical Datum Offset (ft) |
|---|---|
| REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION | |

**Panel Location Map**



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.




NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 13 SOUTH, RANGE 65 WEST, AND TOWNSHIP 14 SOUTH, RANGE 65 WEST.

# LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

**ZONE A** No Base Flood Elevations determined.
**ZONE AE** Base Flood Elevations determined.
**ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
**ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
**ZONE AR** Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
**ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
**ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
**ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

**ZONE X** Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

**ZONE X** Areas determined to be outside the 0.2% annual chance floodplain.
**ZONE D** Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- Floodplain boundary
- Floodway boundary
- Zone D Boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
- 513 — Base Flood Elevation line and value; elevation in feet*
- (EL 987) — Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

- A–A — Cross section line
- 23–23 — Transect line
- 97° 07' 30.00" / 32° 22' 30.00" — Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
- 4275000mN — 1000-meter Universal Transverse Mercator grid ticks, zone 13
- 6000000 FT — 5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
- DX5510 × — Bench mark (see explanation in Notes to Users section of this FIRM panel)
- M1.5 ● — River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
**MARCH 17, 1997**

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
**DECEMBER 7, 2018** - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

**MAP SCALE 1" = 500'**

250 0 500 1000 FEET
150 0 150 300 METERS

NFIP

**PANEL 0752G**

NATIONAL FLOOD INSURANCE PROGRAM

# FIRM

## FLOOD INSURANCE RATE MAP

## EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

**PANEL 752 OF 1300**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

| COMMUNITY | NUMBER | PANEL | SUFFIX |
|---|---|---|---|
| COLORADO SPRINGS, CITY OF | 080060 | 0752 | G |
| EL PASO COUNTY | 080059 | 0752 | G |

Notice: This map was reissued on 05/15/2020 to make a correction.This version replaces any previous versions. See the Notice-to-User Letter that accompanied this correction for details.

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

**MAP NUMBER**
**08041C0752G**

**MAP REVISED**
**DECEMBER 7, 2018**

Federal Emergency Management Agency

# NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

**Coastal Base Flood Elevations** shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

**Base Map** information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

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| Flooding Source | Vertical Datum Offset (ft) |
|---|---|

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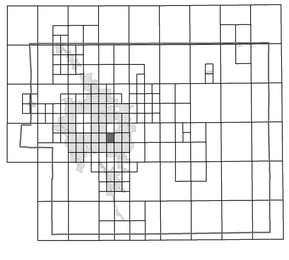
**Panel Location Map**



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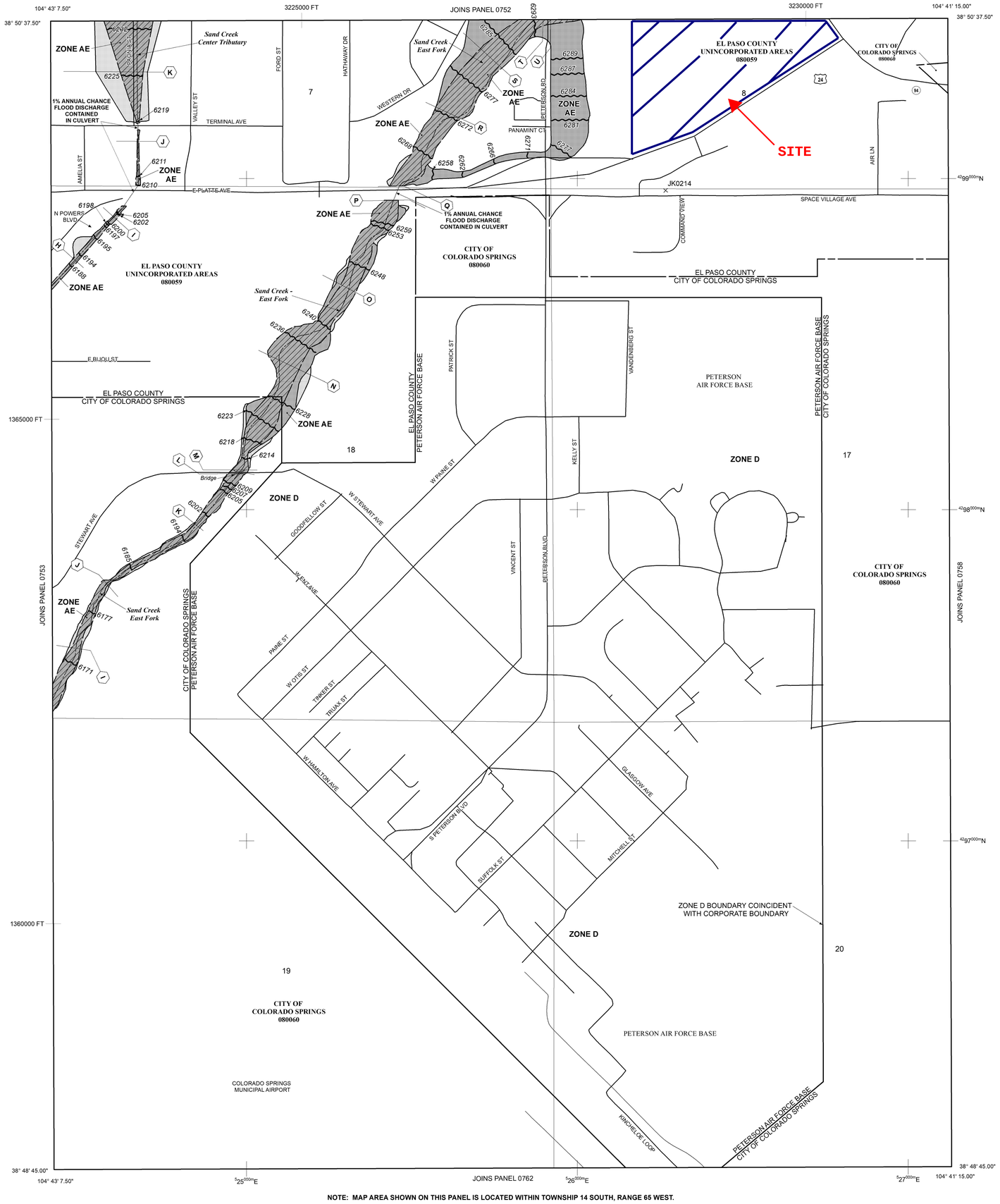


**NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 14 SOUTH, RANGE 65 WEST.**

# LEGEND

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OTHERWISE PROTECTED AREAS (OPAs)

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Floodway boundary
Zone D Boundary
CBRS and OPA boundary
Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
513 — Base Flood Elevation line and value; elevation in feet*
(EL 987) — Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

A–A Cross section line
23–23 Transect line
97° 07' 30.00" / 32° 22' 30.00" Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
4275000mN 1000-meter Universal Transverse Mercator grid ticks, zone 13
6000000 FT 5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
DX5510 × Bench mark (see explanation in Notes to Users section of this FIRM panel)
M1.5 River Mile

MAP REPOSITORIES
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EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
**MARCH 17, 1997**

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
**DECEMBER 7, 2018** - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

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**MAP SCALE 1" = 500'**

250 0 500 1000 FEET
150 0 150 300 METERS

NFIP

**PANEL 0754G**

NATIONAL FLOOD INSURANCE PROGRAM

# FIRM

## FLOOD INSURANCE RATE MAP

## EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

**PANEL 754 OF 1300**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

| COMMUNITY | NUMBER | PANEL | SUFFIX |
|---|---|---|---|
| COLORADO SPRINGS, CITY OF | 080060 | 0754 | G |
| EL PASO COUNTY | 080059 | 0754 | G |

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**MAP NUMBER 08041C0754G**

**MAP REVISED DECEMBER 7, 2018**

**Federal Emergency Management Agency**

# HYDROLOGIC CALCULATIONS

# *Crossroads Mixed Use Filing No. 1*
# *FINAL DRAINAGE REPORT*
# *(Historic Area Runoff Coefficient Summary)*

| | | | *STREETS / DEVELOPED* | | | *OVERLAND / DEVELOPED* | | | *WEIGHTED* | |
|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **TOTAL AREA** *(SF)* | **TOTAL AREA** *(Acres)* | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **$C_5$** | **$C_{100}$** |
| ***C*** | 256383.3 | 5.89 | 0.00 | 0.90 | 0.96 | 5.89 | 0.08 | 0.35 | ***0.08*** | ***0.35*** |
| ***A*** | 561176.6 | 12.88 | 0.00 | 0.90 | 0.96 | 12.88 | 0.08 | 0.35 | ***0.08*** | ***0.35*** |
| ***B*** | 593693.4 | 13.63 | 0.00 | 0.90 | 0.96 | 13.63 | 0.08 | 0.35 | ***0.08*** | ***0.35*** |
| ***OS-1*** | 55560.16 | 1.28 | 1.28 | 0.90 | 0.96 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***OS-2*** | 216993.7 | 4.98 | 2.49 | 0.90 | 0.96 | 2.49 | 0.08 | 0.35 | ***0.49*** | ***0.66*** |
| ***EX-A2****** | | 0.59 | 0.59 | 0.90 | 0.96 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***OS-A**** | | 1.29 | 1.29 | 0.62 | 0.72 | 0.00 | 0.08 | 0.35 | ***0.62*** | ***0.72*** |
| ***E2**** | | 3.86 | 3.86 | 0.80 | 0.90 | 0.00 | 0.08 | 0.35 | ***0.80*** | ***0.90*** |

*FROM FDR FOR CLAREMONT BUSINESS PARK FILING NO. 2

**FROM FDR FOR MEADOWBROOK CROSSING FILING 1 AND FILING 2

***FROM FDR FOR LOT 1 24/94 BUSINESS PARK FILING NO. 1 ON PLATTE AVENUE AND MEADOWBROOK PARKWAY

# *Crossroads Mixed Use Filing No. 1*
# *FINAL DRAINAGE REPORT*
# *(Historic Area Drainage Summary)*

| *From Area Runoff Coefficient Summary* | | | | *OVERLAND* | | | | *STREET / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | | *INTENSITY ^* | | *TOTAL FLOWS* | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **AREA TOTAL** | **$C_5$** | **$C_{100}$** | **$C_5$** | **Length** | **Height** | **$T_C$** | **Length** | **Slope** | **Velocity** | **$T_t$** | **TOTAL** | **CHECK** | **$I_5$** | **$I_{100}$** | **$Q_5$** | **$Q_{100}$** |
| | *(Acres)* | *From DCM Table 5-1* | | | *(ft)* | *(ft)* | *(min)* | *(ft)* | *(%)* | *(fps)* | *(min)* | *(min)* | *(min)* | *(in/hr)* | *(in/hr)* | *(c.f.s.)* | *(c.f.s.)* |
| ***C*** | 5.89 | 0.08 | 0.35 | 0.08 | 300 | 9 | 22.2 | 500 | 2.0% | 1.0 | 8.4 | 30.6 | 14.4 | 2.5 | 4.1 | ***1.2*** | ***8.5*** |
| ***A*** | 12.88 | 0.08 | 0.35 | 0.08 | 300 | 13 | 19.7 | 1350 | 1.6% | 0.9 | 25.2 | 44.8 | 19.2 | 1.9 | 3.2 | ***1.9*** | ***14.2*** |
| ***B*** | 13.63 | 0.08 | 0.35 | 0.08 | 300 | 11 | 20.8 | 750 | 1.7% | 0.9 | 13.7 | 34.5 | 15.8 | 2.3 | 3.8 | ***2.5*** | ***18.2*** |
| ***OS-1*** | 1.28 | 0.90 | 0.96 | 0.90 | 100 | 3 | 2.5 | 490 | 2.2% | 3.0 | 2.8 | 5.3 | 13.3 | 5.1 | 8.5 | ***5.8*** | ***10.5*** |
| ***OS-2*** | 4.98 | 0.49 | 0.66 | 0.49 | 85 | 8 | 4.8 | 1165 | 1.8% | 2.0 | 9.6 | 14.5 | 16.9 | 3.6 | 6.0 | ***8.7*** | ***19.6*** |
| ***EX-A2****** | 0.59 | 0.90 | 0.96 | 0.90 | 10 | 0.2 | 0.9 | 916 | 1.9% | 2.7 | 5.6 | 6.5 | 15.1 | 4.8 | 8.0 | ***2.5*** | ***4.5*** |
| ***OS-A**** | 1.29 | 0.62 | 0.72 | 0.62 | 40 | 0.8 | 4.4 | 1310 | 1.9% | 2.8 | 7.9 | 12.3 | 17.5 | 3.8 | 6.4 | ***3.1*** | ***6.0*** |
| ***E2**** | 3.86 | 0.80 | 0.90 | 0.80 | 50 | 1 | 3.0 | 400 | 1.3% | 2.3 | 2.9 | 6.0 | 12.5 | 4.9 | 8.2 | ***15.1*** | ***28.6*** |

^ Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

*VALUES DERIVED USING DATA FROM FDR FOR CLAREMONT BUSINESS PARK FILING NO. 2

**VALUES DERIVED USING DATA FROM FDR FOR MEADOWBROOK CROSSING FILING 1 AND FILING 2 PAGE 31

***VALUES DERIVED USING DATA FROM FDR FOR LOT 1 24/94 BUSINESS PARK FILING NO. 1 ON PLATTE AVENUE AND MEADOWBROOK PARKWAY

# *Crossroads Mixed Use Filing No. 1*
# *FINAL DRAINAGE REPORT*
# *(Historic Basin Routing Summary)*

| *From Area Runoff Coefficient Summary* | | | | *OVERLAND* | | | | *PIPE / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | *INTENSITY ** | | *TOTAL FLOWS* | | COMMENTS |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **DESIGN POINT** | **CONTRIBUTING BASINS** | **$CA_5$** | **$CA_{100}$** | **$C_5$** | **Length** *(ft)* | **Height** *(ft)* | **$T_C$** *(min)* | **Length** *(ft)* | **Slope** *(%)* | **Velocity** *(fps)* | **$T_t$** *(min)* | **TOTAL** *(min)* | **$I_5$** *(in/hr)* | **$I_{100}$** *(in/hr)* | **$Q_5$** *(c.f.s.)* | **$Q_{100}$** *(c.f.s.)* | |
| *1* | **E2** | 3.09 | 3.47 | | | | 6.0 | 916 | 1.9% | 2.7 | 5.6 | 11.6 | 3.9 | 6.6 | ***14.2*** | ***26.5*** | |
| | **EX-A2** | 0.53 | 0.57 | | | | | | | | | | | | | | |
| | | 3.62 | 4.04 | Tc for E2 Used | | | | | | | | | | | | | EXISTING 10' CDOT TYPE R AT GRADE INLET |
| *2* | **OS-A** | 0.80 | 0.93 | | | | | | | | | 12.3 | 3.8 | 6.4 | ***3.1*** | ***6.0*** | |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | EXISTING 10' CDOT TYPE R AT GRADE INLET |
| *3* | **OS-1** | 1.15 | 1.22 | | | | 11.6 | 150 | 1.0% | 2.0 | 1.3 | 12.8 | 3.8 | 6.3 | ***9.8*** | ***22.5*** | |
| | **FB-DP1** | 1.47 | 2.35 | | | | | | | | | | | | | | |
| | | 2.62 | 3.57 | Tc for DP1 Used | | | | | | | | | | | | | END OF PAVEMENT |
| *4* | **A** | 1.03 | 4.51 | | | | 12.8 | 1470 | 1.6% | 0.9 | 28.0 | 40.8 | 2.0 | 3.4 | ***7.4*** | ***27.7*** | |
| | **FB-DP2** | 0.00 | 0.10 | | | | | | | | | | | | | | |
| | **DP3** | 2.62 | 3.57 | | | | | | | | | | | | | | |
| | | 3.65 | 8.19 | Tc for DP3 Used | | | | | | | | | | | | | ADJACENT PARCEL (LOT 1) |
| *5* | **B** | 1.09 | 4.77 | | | | | | | | | 34.5 | 2.3 | 3.8 | ***2.5*** | ***18.2*** | |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | ADJACENT PARCEL (LOT 2) |
| *6* | **C** | 0.47 | 2.06 | | | | | | | | | 30.6 | 2.5 | 4.1 | ***1.2*** | ***8.5*** | |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | DISCHARGE TO CDOT ROW |
| *7* | **OS2** | 2.44 | 3.26 | | | | | | | | | 14.5 | 3.6 | 6.0 | ***10.4*** | ***31.9*** | |
| | **DP6** | 0.47 | 2.06 | | | | | | | | | | | | | | BARROW DITCH |
| | | 2.91 | 5.32 | Tc for OS2 Used | | | | | | | | | | | | | SW CORNER OF SITE/CDOT ROW |

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

# *Crossroads Mixed Use Filing No. 1*
# *FINAL DRAINAGE REPORT*
# *(Existing Area Runoff Coefficient Summary)*

| | | | *STREETS / DEVELOPED* | | | *OVERLAND / DEVELOPED* | | | *WEIGHTED* | |
|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **TOTAL AREA** *(SF)* | **TOTAL AREA** *(Acres)* | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **$C_5$** | **$C_{100}$** |
| ***C*** | 173960 | 3.99 | 0.00 | 0.90 | 0.96 | 5.89 | 0.08 | 0.35 | ***0.08*** | ***0.35*** |
| ***A*** | 480166.8 | 11.02 | 0.00 | 0.90 | 0.96 | 11.02 | 0.08 | 0.35 | ***0.08*** | ***0.35*** |
| ***B*** | 754121.6 | 17.31 | 0.00 | 0.90 | 0.96 | 17.31 | 0.08 | 0.35 | ***0.08*** | ***0.35*** |
| ***OS-1*** | 55560.16 | 1.28 | 1.28 | 0.90 | 0.96 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***OS-2*** | 216993.7 | 4.98 | 2.49 | 0.90 | 0.96 | 2.49 | 0.08 | 0.35 | ***0.49*** | ***0.66*** |
| ***EX-A2****** | | 0.59 | 0.59 | 0.90 | 0.96 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***OS-A**** | | 1.29 | 1.29 | 0.62 | 0.72 | 0.00 | 0.08 | 0.35 | ***0.62*** | ***0.72*** |
| ***E2**** | | 3.86 | 3.86 | 0.80 | 0.90 | 0.00 | 0.08 | 0.35 | ***0.80*** | ***0.90*** |

*FROM FDR FOR CLAREMONT BUSINESS PARK FILING NO. 2

**FROM TO FDR MEADOWBROOK CROSSING FILING 1 AND FILING 2

***FROM FDR LOT 1 24/94 BUSINESS PARK FILING NO. 1 ON PLATTE AVENUE AND MEADOWBROOK PARKWAY

# *Crossroads Mixed Use Filing No. 1*
# *FINAL DRAINAGE REPORT*
# *(Existing Area Drainage Summary)*

| *From Area Runoff Coefficient Summary* | | | | *OVERLAND* | | | | *STREET / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | | *INTENSITY ^* | | *TOTAL FLOWS* | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **AREA TOTAL** | **$C_5$** | **$C_{100}$** | **$C_5$** | **Length** | **Height** | **$T_C$** | **Length** | **Slope** | **Velocity** | **$T_t$** | **TOTAL** | **CHECK** | **$I_5$** | **$I_{100}$** | **$Q_5$** | **$Q_{100}$** |
| | *(Acres)* | *From DCM Table 5-1* | | | *(ft)* | *(ft)* | *(min)* | *(ft)* | *(%)* | *(fps)* | *(min)* | *(min)* | *(min)* | *(in/hr)* | *(in/hr)* | *(c.f.s.)* | *(c.f.s.)* |
| ***C*** | 3.99 | 0.08 | 0.35 | 0.08 | 120 | 2.8 | 15.3 | 555 | 1.5% | 0.9 | 10.6 | 25.9 | 13.8 | 2.7 | 4.5 | ***0.9*** | ***6.3*** |
| ***A*** | 11.02 | 0.08 | 0.35 | 0.08 | 165 | 8 | 13.8 | 1730 | 1.3% | 0.8 | 36.3 | 50.1 | 20.5 | 1.7 | 2.9 | ***1.5*** | ***11.1*** |
| ***B*** | 17.31 | 0.08 | 0.35 | 0.08 | 300 | 3 | 30.9 | 1390 | 1.2% | 0.8 | 29.7 | 60.6 | 19.4 | 1.4 | 2.4 | ***2.0*** | ***14.5*** |
| ***OS-1*** | 1.28 | 0.90 | 0.96 | 0.90 | 100 | 3 | 2.5 | 490 | 2.2% | 3.0 | 2.8 | 5.3 | 13.3 | 5.1 | 8.5 | ***5.8*** | ***10.5*** |
| ***OS-2*** | 4.98 | 0.49 | 0.66 | 0.49 | 85 | 8 | 4.8 | 1165 | 1.8% | 2.0 | 9.6 | 14.5 | 16.9 | 3.6 | 6.0 | ***8.7*** | ***19.6*** |
| ***EX-A2****** | 0.59 | 0.90 | 0.96 | 0.90 | 10 | 0.2 | 0.9 | 916 | 1.9% | 2.7 | 5.6 | 6.5 | 15.1 | 4.8 | 8.0 | ***2.5*** | ***4.5*** |
| ***OS-A**** | 1.29 | 0.62 | 0.72 | 0.62 | 40 | 0.8 | 4.4 | 1310 | 1.9% | 2.8 | 7.9 | 12.3 | 17.5 | 3.8 | 6.4 | ***3.1*** | ***6.0*** |
| ***E2**** | 3.86 | 0.80 | 0.90 | 0.80 | 50 | 1 | 3.0 | 400 | 1.3% | 2.3 | 2.9 | 6.0 | 12.5 | 4.9 | 8.2 | ***15.1*** | ***28.6*** |

^ Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

*VALUES DERIVED USING DATA FROM FDR FOR CLAREMONT BUSINESS PARK FILING NO. 2

**VALUES DERIVED USING DATA FROM FDR FOR MEADOWBROOK CROSSING FILING 1 AND FILING 2 PAGE 31

***VALUES DERIVED USING DATA FROM FDR FOR LOT 1 24/94 BUSINESS PARK FILING NO. 1 ON PLATTE AVENUE AND MEADOWBROOK PARKWAY

# *Crossroads Mixed Use Filing No. 1*
# *FINAL DRAINAGE REPORT*
# *(Existing Basin Routing Summary)*

| *From Area Runoff Coefficient Summary* | | | | *OVERLAND* | | | | *PIPE / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | *INTENSITY ** | | *TOTAL FLOWS* | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **DESIGN POINT** | **CONTRIBUTING BASINS** | **$CA_5$** | **$CA_{100}$** | **$C_5$** | **Length** *(ft)* | **Height** *(ft)* | **$T_C$** *(min)* | **Length** *(ft)* | **Slope** *(%)* | **Velocity** *(fps)* | **$T_t$** *(min)* | **TOTAL** *(min)* | **$I_5$** *(in/hr)* | **$I_{100}$** *(in/hr)* | **$Q_5$** *(c.f.s.)* | **$Q_{100}$** *(c.f.s.)* | COMMENTS |
| *1* | **E2** | 3.09 | 3.47 | | | | 6.0 | 916 | 1.9% | 2.7 | 5.6 | 11.6 | 3.9 | 6.6 | ***14.2*** | ***26.5*** | |
| | **EX-A2** | 0.53 | 0.57 | | | | | | | | | | | | | | EXISTING 10' CDOT TYPE R AT GRADE INLET |
| | | 3.62 | 4.04 | Tc for E2 Used | | | | | | | | | | | | | |
| *2* | **OS-A** | 0.80 | 0.93 | | | | | | | | | 12.3 | 3.8 | 6.4 | ***3.1*** | ***6.0*** | |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | EXISTING 10' CDOT TYPE R AT GRADE INLET |
| *3* | **OS-1** | 1.15 | 1.22 | | | | 11.6 | 150 | 1.0% | 2.0 | 1.3 | 12.8 | 3.8 | 6.3 | ***9.8*** | ***22.5*** | |
| | **FB-DP1** | 1.47 | 2.35 | | | | | | | | | | | | | | |
| | | 2.62 | 3.57 | Tc for DP1 Used | | | | | | | | | | | | | END OF PAVEMENT |
| *4* | **A** | 0.88 | 3.86 | | | | 12.8 | 1470 | 1.6% | 0.9 | 28.0 | 40.8 | 2.0 | 3.4 | ***7.1*** | ***25.5*** | |
| | **FB-DP2** | 0.00 | 0.10 | | | | | | | | | | | | | | |
| | **DP3** | 2.62 | 3.57 | | | | | | | | | | | | | | |
| | | 3.50 | 7.54 | Tc for DP3 Used | | | | | | | | | | | | | ADJACENT PARCEL (LOT 1) |
| *5* | **B** | 1.38 | 6.06 | | | | | | | | | 60.6 | 1.4 | 2.4 | ***2.0*** | ***14.5*** | |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | ADJACENT PARCEL (LOT 2) |
| *6* | **C** | 0.32 | 1.40 | | | | | | | | | 25.9 | 2.7 | 4.5 | ***0.9*** | ***6.3*** | |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | DISCHARGE TO CDOT ROW |
| *7* | **OS2** | 2.44 | 3.26 | | | | | | | | | 14.5 | 3.6 | 6.0 | ***9.9*** | ***28.0*** | |
| | **DP6** | 0.32 | 1.40 | | | | | | | | | | | | | | BARROW DITCH |
| | | 2.76 | 4.66 | Tc for OS2 Used | | | | | | | | | | | | | SW CORNER OF SITE/CDOT ROW |

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

# *CROSSROADS MIXED USE FILING NO. 1*
# *FINAL DRAINAGE CALCULATIONS*
# *(Future Area Runoff Coefficient Summary)*

| | | | *STREETS / COMMERC.* | | | *MULTI-FAMILY/PARKLAND* | | | *OVERLAND / UNDEVELOPED* | | | *WEIGHTED* | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **TOTAL AREA** *(Sq Ft)* | **TOTAL AREA** *(Acres)* | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **$C_5$** | **$C_{100}$** |
| *FUTURE BASINS* | | | | | | | | | | | | | |
| ***OS-A**** | | 1.29 | 1.29 | 0.62 | 0.72 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.62*** | ***0.72*** |
| ***E2**** | | 3.86 | 3.86 | 0.80 | 0.90 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.80*** | ***0.90*** |
| ***EX-A2***** | | 0.59 | 0.59 | 0.90 | 0.96 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***OS-1*** | ***60793.3017*** | 1.40 | 1.40 | 0.90 | 0.96 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***OS-2*** | ***216993.7096*** | 4.98 | 2.49 | 0.90 | 0.96 | 0.00 | 0.49 | 0.62 | 2.49 | 0.08 | 0.35 | ***0.49*** | ***0.66*** |
| ***A*** | ***72787.0873*** | 1.67 | 1.67 | 0.90 | 0.96 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***B*** | ***64490.3787*** | 1.48 | 1.48 | 0.90 | 0.96 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***C*** | ***200631.5748*** | 4.61 | 4.46 | 0.81 | 0.88 | 0.00 | 0.49 | 0.62 | 0.15 | 0.08 | 0.35 | ***0.79*** | ***0.86*** |
| ***D*** | ***96773.7602*** | 2.22 | 2.22 | 0.81 | 0.88 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.81*** | ***0.88*** |
| ***E*** | ***45497.7355*** | 1.04 | 0.24 | 0.90 | 0.96 | 0.80 | 0.81 | 0.88 | 0.00 | 0.08 | 0.35 | ***0.83*** | ***0.90*** |
| ***E1*** | ***72636.2925*** | 1.67 | 0.24 | 0.90 | 0.96 | 1.43 | 0.81 | 0.88 | 0.00 | 0.08 | 0.35 | ***0.82*** | ***0.89*** |
| ***F*** | ***112036.6061*** | 2.57 | 2.57 | 0.81 | 0.88 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.81*** | ***0.88*** |
| ***G*** | ***20057.4496*** | 0.46 | 0.46 | 0.90 | 0.96 | 0.00 | 0.49 | 0.62 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |
| ***J*** | ***139924.2472*** | 3.21 | 0.00 | 0.90 | 0.96 | 3.21 | 0.16 | 0.41 | 0.00 | 0.08 | 0.35 | ***0.16*** | ***0.41*** |
| ***A-5******** | ***159865.2*** | 3.67 | 0.00 | 0.90 | 0.96 | 3.67 | 0.68 | 0.79 | 0.00 | 0.08 | 0.35 | ***0.68*** | ***0.79*** |
| ***Z-1******** | ***16117.2*** | 0.37 | 0.00 | 0.90 | 0.96 | 0.37 | 0.33 | 0.52 | 0.00 | 0.08 | 0.35 | ***0.33*** | ***0.52*** |
| ***D-1******** | ***33976.8*** | 0.78 | 0.00 | 0.90 | 0.96 | 0.78 | 0.62 | 0.75 | 0.00 | 0.08 | 0.35 | ***0.62*** | ***0.75*** |
| ***Z-2******** | ***16552.8*** | 0.38 | 0.00 | 0.90 | 0.96 | 0.38 | 0.38 | 0.56 | 0.00 | 0.08 | 0.35 | ***0.38*** | ***0.56*** |
| ***G1*** | ***25962.0179*** | 0.60 | 0.60 | 0.90 | 0.96 | 0.00 | 0.16 | 0.41 | 0.00 | 0.08 | 0.35 | ***0.90*** | ***0.96*** |

*FROM FDR FOR CLAREMONT BUSINESS PARK FILING NO. 2
**FROM FDR FOR MEADOWBROOK CROSSING FILING 1 AND FILING 2
***FROM FDR FOR LOT 1 24/94 BUSINESS PARK FILING NO. 1 ON PLATTE AVENUE AND MEADOWBROOK PARKWAY
****FROM FDR FOR AURA AT CROSSROADS, DATED APRIL 4TH, 2022

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

# *CROSSROADS MIXED USE FILING NO. 1*
# *FINAL DRAINAGE REPORT*
# *(Future Drainage Summary)*

| *From Area Runoff Coefficient Summary* | | | | *OVERLAND* | | | | *STREET / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | | *INTENSITY #* | | *TOTAL FLOWS* | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **AREA TOTAL** *(Acres)* | **$C_5$** *From DCM Table 5-1* | **$C_{100}$** *From DCM Table 5-1* | **$C_5$** | **Length** *(ft)* | **Height** *(ft)* | **$T_C$** *(min)* | **Length** *(ft)* | **Slope** *(%)* | **Velocity** *(fps)* | **$T_t$** *(min)* | **TOTAL** *(min)* | **CHECK** *(min)* | **$I_5$** *(in/hr)* | **$I_{100}$** *(in/hr)* | **$Q_5$** *(c.f.s.)* | **$Q_{100}$** *(c.f.s.)* |
| ***Future Area Drainage Summary*** | | | | | | | | | | | | | | | | | |
| ***OS-A**** | 1.29 | 0.62 | 0.72 | 0.62 | 40 | 0.8 | 4.4 | 1310 | 1.9% | 2.8 | 7.9 | 12.3 | 17.5 | 3.8 | 6.4 | ***3.1*** | ***6.0*** |
| ***E2**** | 3.86 | 0.80 | 0.90 | 0.80 | 50 | 1 | 3.0 | 400 | 1.3% | 2.3 | 2.9 | 6.0 | 12.5 | 4.9 | 8.2 | ***15.1*** | ***28.6*** |
| ***EX-A2***** | 0.59 | 0.90 | 0.96 | 0.90 | 10 | 0.2 | 0.9 | 916 | 1.9% | 2.7 | 5.6 | 6.5 | 15.1 | 4.8 | 8.0 | ***2.5*** | ***4.5*** |
| ***OS-1*** | 1.40 | 0.90 | 0.96 | 0.90 | 100 | 3 | 2.5 | 490 | 2.2% | 3.0 | 2.7 | 5.2 | 13.3 | 5.1 | 8.6 | ***6.4*** | ***11.5*** |
| ***OS-2*** | 4.98 | 0.49 | 0.66 | 0.49 | 85 | 8 | 4.8 | 1165 | 1.8% | 2.0 | 9.6 | 14.5 | 16.9 | 3.6 | 6.0 | ***8.7*** | ***19.6*** |
| ***A*** | 1.67 | 0.90 | 0.96 | 0.90 | 30 | 0.6 | 1.6 | 1325 | 0.7% | 1.7 | 7.3 | 8.9 | 17.5 | 4.3 | 7.2 | ***6.5*** | ***11.6*** |
| ***B*** | 1.48 | 0.90 | 0.96 | 0.90 | 25 | 0.5 | 1.4 | 1335 | 0.7% | 1.7 | 7.3 | 8.8 | 17.6 | 4.3 | 7.3 | ***5.8*** | ***10.3*** |
| ***C*** | 4.61 | 0.79 | 0.86 | 0.79 | 50 | 1 | 3.2 | 260 | 1.5% | 2.4 | 1.4 | 5.0 | 11.7 | 5.2 | 8.7 | ***18.7*** | ***34.5*** |
| ***D*** | 2.22 | 0.81 | 0.88 | 0.81 | 50 | 1 | 2.9 | 200 | 1.5% | 2.4 | 1.1 | 5.0 | 11.4 | 5.2 | 8.7 | ***9.3*** | ***17.0*** |
| ***E*** | 1.04 | 0.83 | 0.90 | 0.83 | 60 | 1.2 | 3.0 | 700 | 1.0% | 2.0 | 3.8 | 6.8 | 14.2 | 4.7 | 7.9 | ***4.1*** | ***7.4*** |
| ***E1*** | 1.67 | 0.82 | 0.89 | 0.82 | 60 | 1.2 | 3.0 | 700 | 1.0% | 2.0 | 3.8 | 6.8 | 14.2 | 4.7 | 7.9 | ***6.4*** | ***11.7*** |
| ***F*** | 2.57 | 0.81 | 0.88 | 0.81 | 50 | 0.8 | 3.2 | 300 | 1.3% | 2.3 | 1.6 | 5.0 | 11.9 | 5.2 | 8.7 | ***10.8*** | ***19.6*** |
| ***G*** | 0.46 | 0.90 | 0.96 | 0.90 | 50 | 1 | 2.0 | 466 | 1.1% | 2.1 | 2.6 | 5.0 | 12.9 | 5.2 | 8.7 | ***2.1*** | ***3.8*** |
| ***J*** | 3.21 | 0.16 | 0.41 | 0.16 | 50 | 2 | 7.6 | 0 | 0.0% | 0.0 | 0.0 | 7.6 | 10.3 | 4.5 | 7.6 | ***2.3*** | ***10.0*** |
| ***A-5****** | 3.67 | 0.68 | 0.79 | 0.68 | REFER TO "FDR FOR AURA AT CROSSROADS" FOR DETAILS | | | | | | | | | | | ***8.72*** | ***17.06*** |
| ***Z-1****** | 0.37 | 0.33 | 0.52 | 0.33 | REFER TO "FDR FOR AURA AT CROSSROADS" FOR DETAILS | | | | | | | | | | | ***0.47*** | ***1.27*** |
| ***D-1****** | 0.78 | 0.62 | 0.75 | 0.62 | REFER TO "FDR FOR AURA AT CROSSROADS" FOR DETAILS | | | | | | | | | | | ***2.08*** | ***4.20*** |
| ***Z-2****** | 0.38 | 0.38 | 0.56 | 0.38 | REFER TO "FDR FOR AURA AT CROSSROADS" FOR DETAILS | | | | | | | | | | | ***0.57*** | ***1.43*** |
| ***G1*** | 0.60 | 0.90 | 0.96 | 0.90 | 50 | 1 | 2.0 | 466 | 1.1% | 2.1 | 2.6 | 5.0 | 12.9 | 5.2 | 8.7 | ***2.8*** | ***5.0*** |

# Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

*VALUES DERIVED USING DATA FROM FDR FOR CLAREMONT BUSINESS PARK FILING NO. 2

**VALUES DERIVED USING DATA FROM FDR MEADOWBROOK CROSSING FILING 1 AND FILING 2 PAGE 31

***VALUES DERIVED USING DATA FROM FDR LOT 1 24/94 BUSINESS PARK FILING NO. 1 ON PLATTE AVENUE AND MEADOWBROOK PARKWAY

****FROM FDR FOR AURA AT CROSSROADS, DATED APRIL 4TH, 2022

# CROSSROADS MIXED USE FILING NO. 1
# FINAL DRAINAGE REPORT
# (Future Basin Routing Summary)

| *From Area Runoff Coefficient Summary* | | | | *OVERLAND* | | | | *PIPE / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | *INTENSITY ** | | *TOTAL FLOWS* | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **DESIGN POINT** | **CONTRIBUTING BASINS** | **$CA_5$** | **$CA_{100}$** | **$C_5$** | **Length** *(ft)* | **Height** *(ft)* | **$T_C$** *(min)* | **Length** *(ft)* | **Slope** *(%)* | **Velocity** *(fps)* | **$T_t$** *(min)* | **TOTAL** *(min)* | **$I_5$** *(in/hr)* | **$I_{100}$** *(in/hr)* | **$Q_5$** *(c.f.s.)* | **$Q_{100}$** *(c.f.s.)* | COMMENTS |
| ***FUTURE DRAINAGE BASIN ROUTING SUMMARY*** | | | | | | | | | | | | | | | | | |
| *1* | **E2, EX-A2** | 3.62 | 4.04 | | | | 6.0 | 916 | 1.9% | 2.7 | 5.6 | 11.6 | 3.9 | 6.6 | ***14.2*** | ***26.5*** | Existing 10' CDOT Type R At-Grade Inlet (Public) |
| | | | | Tc for E2 Used | | | | | | | | | | | | | |
| *2* | **OS-A** | 0.80 | 0.93 | | | | | | | | | 12.3 | 3.8 | 6.4 | ***3.1*** | ***6.0*** | Existing 10' CDOT Type R At-Grade Inlet (Public) |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | |
| *3* | **OS-1, FB-DP1** | 2.73 | 3.69 | | | | 11.6 | 150 | 1.0% | 2.0 | 1.3 | 12.8 | 3.8 | 6.3 | ***10.2*** | ***23.3*** | Proposed 10' CDOT Type R At-Grade Inlet (Public) |
| | | | | Tc for DP1 Used | | | | | | | | | | | | | |
| *4* | **A, FB-DP2** | 1.50 | 1.71 | | | | | | | | | 8.9 | 4.3 | 7.2 | ***6.5*** | ***12.4*** | Proposed 15' CDOT Type R At-Grade Inlet (Public) |
| | | | | Tc for Basin A used | | | | | | | | | | | | | |
| *4.5* | **FB-DP4** | 0.00 | 0.25 | | | | | | | | | 8.9 | 4.3 | 7.2 | ***0.0*** | ***1.8*** | Proposed NEENAH R-2501 MH Lid and Frame (Public) |
| | | | | Tc for DP4 used | | | | | | | | | | | | | |
| *5* | **B, FB-DP3** | 2.28 | 3.56 | | | | | | | | | 8.8 | 4.3 | 7.3 | ***9.8*** | ***25.8*** | Proposed 15' CDOT Type R Sump Inlet (Public) |
| | | | | Tc for Basin B Used | | | | | | | | | | | | | |
| *6* | **C** | 3.62 | 3.98 | | | | | | | | | 5.0 | 5.2 | 8.7 | ***18.7*** | ***34.5*** | Future 30" RCP or PP Storm Sewer, Rip Rap Pad (Private) |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | |
| *7* | **D** | 1.80 | 1.96 | | | | | | | | | 5.0 | 5.2 | 8.7 | ***9.3*** | ***17.0*** | Future 24" RCP or PP Storm Sewer, Rip Rap Pad (Private) |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | |
| *8* | **E** | 0.87 | 0.94 | | | | | | | | | 6.8 | 4.7 | 7.9 | ***4.1*** | ***7.4*** | Future 10' CDOT Type R At-Grate Inlet (Private) |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | |
| *9* | **E1** | 1.37 | 1.49 | | | | | | | | | 6.8 | 4.7 | 7.9 | ***6.4*** | ***11.7*** | Future 10' CDOT Type R At-Grade Inlet (Private) |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | |
| *10* | **G** | 0.41 | 0.44 | | | | | | | | | 5.0 | 5.2 | 8.7 | ***2.1*** | ***3.8*** | Proposed 10' CDOT Type R Sump Inlet (Private) |
| | | | | Tc for Basin G Used | | | | | | | | | | | | | |
| *11* | **G1** | 0.51 | 1.32 | | | | | | | | | 5.6 | 5.0 | 8.4 | ***3.7*** | ***15.3*** | Proposed 15' CDOT Type R Sump Inlet (Private) |
| | **FB-DP8** | 0.02 | 0.18 | | | | | | | | | | | | | | |
| | **FB-DP9** | 0.20 | 0.51 | | | | | | | | | | | | | | |
| | | **0.73** | **1.83** | Weighted Tc Used | | | | | | | | | | | | | |
| *12* | **F** | 2.08 | 2.26 | | | | | | | | | 5.0 | 5.2 | 8.7 | ***10.8*** | ***19.6*** | Proposed 24" RCP or PP Storm Sewer (Private) |
| | | | | See Area Drainage Sheet for Input | | | | | | | | | | | | | |
| *13* | **Basin A-5 (Overflow)** | 0.23 | 1.32 | | | | | | | | | 12.8 | 3.8 | 6.3 | ***1.3*** | ***10.9*** | Proposed 2' Bottom Earthen Swale, Rip Rap Rundown |
| | **Basin Z-1** | 0.12 | 0.20 | | | | | | | | | | | | | | |
| | **Basin D-1 (Overflow)** | 0.00 | 0.21 | | | | | | | | | | | | | | |
| | | **0.36** | **1.72** | Weighted Tc Used | | | | | | | | | | | | | |
| *14* | **Basin Z-2** | 0.14 | 0.03 | | | | | | | | | 11.1 | 4.0 | 6.7 | ***2.0*** | ***9.7*** | Proposed Triangular Earthen Swale (Private) |
| | **DP 13** | 0.36 | 1.43 | | | | | | | | | | | | | | |
| | | **0.50** | **1.46** | Tc for Basin Z-2 Used | | | | | | | | | | | | | |
| *15* | **J, DP14, PR19,** | 24.15 | 28.95 | | | | | | | | | 6.3 | 4.8 | 8.1 | ***116.7*** | ***235.0*** | Full Spectrum Extended Detention Basin (Private) |
| | **PR18** | | | Weighted Tc Used | | | | | | | | | | | | | |
| *16* | **POND OUTFALL** | 2.77 | 5.16 | | | | | | | | | 14.5 | 3.6 | 6.0 | ***9.9*** | ***31.0*** | HISTORIC FLOW IN CDOT BARROW DITCH |
| | **OS-2** | | | | | | | | | | | | | | | | Q5= 10.4 CFS, Q100 = 31.9 CFS |
| | | | | Tc for Basin OS-2 Used | | | | | | | | | | | | | PER HISTORIC DRAINAGE ANALYSIS |

* Intensity equations assume a minimum travel time of 5 minutes.

Overflow- obtain flows from inlet sheets provided in Background Information Section of Appendix

CVW
Date: 1/31/2022
Checked by: DLM

# *CROSSROADS MIXED USE FILING NO. 1*
# *FINAL DRAINAGE CALCULATIONS*
# *(Future Storm Sewer Routing Summary)*

| *PIPE RUN* | *Contributing Pipes/Design Points* | *Equivalent $CA_5$* | *Equivalent $CA_{100}$* | *Maximum $T_C$* | *Intensity** | | *Flow* | | PIPE SIZE |
|---|---|---|---|---|---|---|---|---|---|
| | | | | | *$I_5$* | *$I_{100}$* | *$Q_5$* | *$Q_{100}$* | |
| *1* | **DP3 (INLET 3)** | 1.78 | 1.55 | 12.8 | 3.8 | 6.3 | ***6.7*** | ***9.8*** | 24" SD |
| *1.5* | **DP4 (INLET 4)** | 1.50 | 1.46 | 8.9 | 4.3 | 7.2 | ***6.5*** | ***10.6*** | 24" SD |
| *2* | **PR1.5, DP4.5 (INLET 4.5)** | 1.50 | 1.71 | 9.0 | 4.3 | 7.2 | ***6.4*** | ***12.3*** | 24" SD |
| *3* | **PR2, DP5 (INLET 5)** | 3.78 | 5.27 | 9.0 | 4.3 | 7.2 | ***16.2*** | ***37.9*** | 36" SD |
| *4* | **DP6** | 3.62 | 3.98 | 5.0 | 5.2 | 8.7 | ***18.7*** | ***34.5*** | 30" SD |
| *4.5* | **Future Commercial Lot** | 0.00 | 0.00 | 0.0 | 0.0 | 0.0 | ***0.0*** | ***0.0*** | 30" SD |
| *5* | **PR4, PR4.5** | 3.62 | 3.98 | 5.0 | 5.2 | 8.7 | ***18.7*** | ***34.5*** | 30" SD |
| *6* | **PR5** | 3.62 | 3.98 | 5.0 | 5.2 | 8.7 | ***18.7*** | ***34.5*** | 30" SD |
| *7* | **PR6** | 3.62 | 3.98 | 5.0 | 5.2 | 8.7 | ***18.7*** | ***34.5*** | 30" SD |
| *8* | **DP7** | 1.80 | 1.96 | 5.0 | 5.2 | 8.7 | ***9.3*** | ***17.0*** | 24" SD |
| *9* | **PR8, DP8 (Inlet 6)** | 2.65 | 2.72 | 6.8 | 4.7 | 7.9 | ***12.5*** | ***21.4*** | 30" SD |
| *10* | **DP9 (Inlet 7)** | 1.17 | 0.98 | 6.8 | 4.7 | 7.9 | ***5.5*** | ***7.7*** | 18" SD |
| *11* | **PR7, PR9, PR10** | 7.45 | 7.67 | 6.8 | 4.7 | 7.9 | ***35.0*** | ***60.5*** | 36" SD |
| *11.5** | **SEE FDR FOR AURA AT CROSSROADS** | 1.93 | 2.30 | 14.6 | 3.6 | 6.0 | ***6.9*** | ***13.8*** | 30" SD |
| *12* | **PR11** | 7.45 | 7.67 | 7.0 | 4.7 | 7.8 | ***34.7*** | ***60.0*** | 42" SD |
| *12.5* | **PR12, PR11.5** | 9.38 | 9.97 | 7.2 | 4.6 | 7.8 | ***43.3*** | ***77.4*** | 48" SD |
| *13* | **DP10 (Inlet 8)** | 0.41 | 0.44 | 5.0 | 5.2 | 8.7 | ***2.1*** | ***3.8*** | 18" SD |
| *14* | **DP11 (Inlet 9)** | 0.73 | 1.83 | 5.6 | 5.0 | 8.4 | ***3.7*** | ***15.3*** | 30" SD |
| *15* | **PR12.5, PR13, PR14** | 10.52 | 12.24 | 7.5 | 4.6 | 7.7 | ***48.0*** | ***93.7*** | 48" SD |
| *16* | **DP12** | 2.08 | 2.26 | 5.0 | 5.2 | 8.7 | ***10.8*** | ***19.6*** | 24" SD |
| *17* | **PR15, PR16** | 12.61 | 14.50 | 7.7 | 4.5 | 7.6 | ***57.0*** | ***110.1*** | 48" SD |
| *18* | **PR17, PR21** | 13.09 | 15.08 | 8.2 | 4.4 | 7.4 | ***57.9*** | ***112.1*** | 48" SD |
| *19** | **SEE FDR FOR AURA AT CROSSROADS** | 10.05 | 11.09 | 15.0 | 3.5 | 5.9 | ***35.4*** | ***65.5*** | 48" SD |
| *20* | **POND OUTFALL** | PER | MHFD | WKSHT | | | ***1.2*** | ***11.4*** | 18" SD |
| *21** | **SEE FDR FOR AURA AT CROSSROADS** | 0.48 | 0.58 | 8.8 | 4.3 | 7.3 | ***2.1*** | ***4.2*** | 30" SD |

*REFER TO FDR FOR AURA AT CROSSROADS FOR CONTRIBUTING PIPE FLOW DETAILS

DP - Design Point
EX - Existing Design Point
FB- Flow By from Design Point
INT- Intercepted Flow from Design Point

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

# *CROSSROADS MIXED USE FILING NO. 1*
# *FINAL DRAINAGE CALCULATIONS*
# *(Proposed Area Runoff Coefficient Summary)*

| | | | *STREETS / COMMERC.* | | | *MULTI-FAMILY/PARKLAND* | | | *DISTURBED & UNDEVELOPED* | | | *WEIGHTED* | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **TOTAL AREA** *(Sq Ft)* | **TOTAL AREA** *(Acres)* | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **AREA** *(Acres)* | **$C_5$** | **$C_{100}$** | **$C_5$** | **$C_{100}$** |
| ***TRACT D PROPOSED BASINS*** | | | | | | | | | | | | | |
| ***P1*** | ***390703.7678*** | 8.97 | 0.00 | 0.90 | 0.96 | 0.00 | 0.38 | 0.56 | 8.97 | 0.12 | 0.39 | ***0.12*** | ***0.39*** |
| ***P2*** | ***132430.7607*** | 3.04 | 0.00 | 0.90 | 0.96 | 0.00 | 0.16 | 0.41 | 3.04 | 0.12 | 0.39 | ***0.12*** | ***0.39*** |

Calculated by: CVW
Date: 2/7/2022
Checked by: DLM

# *CROSSROADS MIXED USE FILING NO. 1*
# *FINAL DRAINAGE REPORT*
# *(Proposed Drainage Summary)*

| *From Area Runoff Coefficient Summary* | | | | *OVERLAND* | | | | *STREET / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | | *INTENSITY #* | | *TOTAL FLOWS* | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **AREA TOTAL** *(Acres)* | **$C_5$** | **$C_{100}$** *From DCM Table 5-1* | **$C_5$** | **Length** *(ft)* | **Height** *(ft)* | **$T_C$** *(min)* | **Length** *(ft)* | **Slope** *(%)* | **Velocity** *(fps)* | **$T_t$** *(min)* | **TOTAL** *(min)* | **CHECK** *(min)* | **$I_5$** *(in/hr)* | **$I_{100}$** *(in/hr)* | **$Q_5$** *(c.f.s.)* | **$Q_{100}$** *(c.f.s.)* |
| ***Proposed Area Drainage Summary*** | | | | | | | | | | | | | | | | | |
| ***P1*** | 8.97 | 0.12 | 0.39 | 0.12 | 173 | 2 | 22.2 | 728 | 1.1% | 1.6 | 7.7 | 29.9 | 15.0 | 3.5 | 5.9 | ***3.8*** | ***20.7*** |
| ***P2*** | 3.04 | 0.12 | 0.39 | 0.12 | 175 | 2 | 22.4 | 525 | 1.5% | 1.9 | 4.7 | 27.1 | 13.9 | 3.6 | 6.1 | ***1.3*** | ***7.2*** |

# Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: CVW
Date: 2/7/2022
Checked by: DLM

# CROSSROADS MIXED USE FILING NO. 1
# FINAL DRAINAGE REPORT
# (Proposed Basin Routing Summary)

| *From Area Runoff Coefficient Summary* | | | | *OVERLAND* | | | | *PIPE / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | *INTENSITY ** | | *TOTAL FLOWS* | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **DESIGN POINT** | **CONTRIBUTING BASINS** | **$CA_5$** | **$CA_{100}$** | **$C_5$** | **Length** *(ft)* | **Height** *(ft)* | **$T_C$** *(min)* | **Length** *(ft)* | **Slope** *(%)* | **Velocity** *(fps)* | **$T_t$** *(min)* | **TOTAL** *(min)* | **$I_5$** *(in/hr)* | **$I_{100}$** *(in/hr)* | **$Q_5$** *(c.f.s.)* | **$Q_{100}$** *(c.f.s.)* | COMMENTS |
| ***PROPOSED DRAINAGE BASIN ROUTING SUMMARY*** | | | | | | | | | | | | | | | | | |
| *7* | **P1** | 1.08 | 3.50 | | | | 15.0 | | | | | 15.0 | 3.5 | 5.9 | ***3.8*** | ***20.7*** | Proposed Sediment Basin (SB2) |
| | | | | Tc for P1 Used | | | | | | | | | | | | | |
| *12* | **P2, DP7** | 1.44 | 4.68 | | | | 14.7 | | | | | 14.7 | 3.6 | 6.0 | ***5.1*** | ***27.9*** | Proposed Sediment Basin (SB3) |
| | | | | Weighted Tc Used | | | | | | | | | | | | | |

* Intensity equations assume a minimum travel time of 5 minutes.

CVW
Date: 2/7/2022
Checked by: DLM

# *CROSSROADS MIXED USE FILING NO. 1*
# *FINAL DRAINAGE CALCULATIONS*
## *(Proposed Storm Sewer Routing Summary)*

| *PIPE RUN* | *Contributing Pipes/Design Points* | *Equivalent $CA_5$* | *Equivalent $CA_{100}$* | *Maximum $T_C$* | *Intensity** | | *Flow* | | PIPE SIZE |
|---|---|---|---|---|---|---|---|---|---|
| | | | | | *$I_5$* | *$I_{100}$* | *$Q_5$* | *$Q_{100}$* | |
| *11* | **N/A** | 0.00 | 0.00 | 0.0 | 0.0 | 0.0 | ***0.0*** | ***0.0*** | 36" SD |
| *11.5** | **SEE FDR FOR AURA AT CROSSROADS** | 1.93 | 2.30 | 14.6 | 3.6 | 6.0 | ***6.9*** | ***13.8*** | 30" SD |
| *12* | **PR11** | 0.00 | 0.00 | 0.0 | 0.0 | 0.0 | ***0.0*** | ***0.0*** | 42" SD |
| *12.5* | **PR12, PR11.5** | 1.93 | 2.30 | 14.6 | 3.6 | 6.0 | ***6.9*** | ***13.8*** | 48" SD |
| *13* | **Inlet 8 (See Future Drainage)** | 0.41 | 0.44 | 5.0 | 5.2 | 8.7 | ***2.1*** | ***3.8*** | 18" SD |
| *14* | **Inlet 9 (See Future Drainage)** | 0.73 | 1.83 | 5.6 | 5.0 | 8.4 | ***3.7*** | ***15.3*** | 30" SD |
| *15* | **PR12.5 PR13, PR14** | 3.07 | 4.57 | 15.0 | 3.5 | 5.9 | ***10.8*** | ***27.0*** | 48" SD |
| *16* | **DP12** | 1.44 | 4.68 | 14.7 | 3.6 | 6.0 | ***5.1*** | ***27.9*** | 24" SD |
| *17* | **PR15, PR16** | 4.51 | 9.25 | 15.0 | 3.5 | 5.9 | ***15.9*** | ***54.7*** | 48" SD |

*REFER TO FDR FOR AURA AT CROSSROADS FOR CONTRIBUTING PIPE FLOW DETAILS

DP - Design Point
EX - Existing Design Point
FB- Flow By from Design Point
INT- Intercepted Flow from Design Point

Calculated by: CVW
Date: 2/7/2022
Checked by: DLM

# *Crossroads Mixed Use Filing No. 1*
# *FINAL DRAINAGE REPORT*
# *(Roadside Ditch Intermediate Events Drainage Summary)*

| *From Area Runoff Coefficient Summary* | | | | | | | *OVERLAND* | | | | *STREET / CHANNEL FLOW* | | | | *Time of Travel ($T_t$)* | | *INTENSITY ^* | | | | | *TOTAL FLOWS* | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **BASIN** | **AREA TOTAL** | **$C_5$** | **$C_{10}$** | **$C_{25}$** | **$C_{50}$** | **$C_{100}$** | **$C_5$** | **Length** | **Height** | **$T_C$** | **Length** | **Slope** | **Velocity** | **$T_t$** | **TOTAL** | **CHECK** | **$I_5$** | **$I_{10}$** | **$I_{25}$** | **$I_{50}$** | **$I_{100}$** | **$Q_5$** | **$Q_{10}$** | **$Q_{25}$** | **$Q_{50}$** | **$Q_{100}$** |
| | *(Acres)* | *From DCM Table 5-1* | | | | | | *(ft)* | *(ft)* | *(min)* | *(ft)* | *(%)* | *(fps)* | *(min)* | *(min)* | *(min)* | *(in/hr)* | *(in/hr)* | *(in/hr)* | *(in/hr)* | *(in/hr)* | *(c.f.s.)* | *(c.f.s.)* | *(c.f.s.)* | *(c.f.s.)* | *(c.f.s.)* |
| ***C*** | 5.89 | 0.08 | 0.15 | 0.25 | 0.30 | 0.35 | 0.1 | 300 | 9.0 | 22.2 | 300 | 2% | 1.0 | 8.4 | 30.6 | 14.4 | 2.9 | 3.6 | 4.2 | 4.8 | 5.4 | ***1.3*** | ***3.2*** | ***6.2*** | ***8.4*** | ***11.1*** |
| ***OS-2*** | 4.98 | 0.49 | 0.54 | 0.60 | 0.63 | 0.66 | 0.5 | 85 | 8.0 | 4.8 | 1165 | 1.8% | 2.0 | 9.6 | 14.5 | 16.9 | 3.6 | 4.2 | 4.8 | 5.4 | 6.0 | ***8.7*** | ***11.1*** | ***14.1*** | ***16.7*** | ***19.6*** |

^ Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

# *Crossroads Mixed Use Filing No. 1*
# *FINAL DRAINAGE REPORT*
# *(Roadside Ditch Intermediate Events: Routing Summary)*

| *From Area Runoff Coefficient Summary* | | | | | | | *OVERLAND* | | | | *PIPE / CHANNEL FLOW* | | | | $T_t$ | *INTENSITY ** | | | | | *TOTAL FLOWS* | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **DESIGN POINT** | **CONTRIBUTING BASINS** | **$CA_5$** | **CA10** | **CA25** | **CA50** | **$CA_{100}$** | **$C_5$** | **Length** | **Height** | **$T_C$** | **Length** | **Slope** | **Velocity** | **$T_t$** | **TOTAL** | **$I_5$** | **$I_{10}$** | **$I_{25}$** | **$I_{50}$** | **$I_{100}$** | **$Q_5$** | **Q10** | **Q25** | **Q50** | **$Q_{100}$** |
| *(CONDITION)* | | | | | | | | *(ft)* | *(ft)* | *(min)* | *(ft)* | *(%)* | *(fps)* | *(min)* | *(min)* | *(in/hr)* | *(in/hr)* | *(in/hr)* | *(in/hr)* | *(in/hr)* | *(c.f.s.)* | *(c.f.s.)* | *(c.f.s.)* | *(c.f.s.)* | *(c.f.s.)* |
| *7* | **OS2** | 2.44 | 2.67 | 2.96 | 3.11 | 3.26 | | | | 14.5 | | | | | 14.5 | 3.6 | 4.2 | 4.8 | 5.4 | 6.0 | ***10.4*** | ***14.8*** | ***21.1*** | ***26.2*** | ***31.9*** |
| *(Historic)* | **DP6 (Basin C)** | 0.47 | 0.88 | 1.47 | 1.77 | 2.06 | | | | | | | | | | | | | | | | | | | |
| | | 2.91 | 3.55 | 4.44 | 4.88 | 5.32 | Tc for OS2 Used | | | | | | | | | | | | | | | | | | |
| ***Peak Discharge*** | | **COLORADO URBAN HYDROGRAPH PROCEDURE USED TO DEVELOP PEAK FLOWS** | | | | | | | | | | | | | | | | | | | ***1.2*** | ***2.6*** | ***9.1*** | ***11.0*** | ***11.4*** |
| ***(From MHFD Worksheet)*** | | | | | | | | | | | | | | | | | | | | | | | | | |
| *16* | **OS2** | 2.44 | 2.67 | 2.96 | 3.11 | 3.26 | | | | 14.5 | | | | | 14.5 | 3.6 | 4.2 | 4.8 | 5.4 | 6.0 | ***9.9*** | ***13.7*** | ***23.2*** | ***27.7*** | ***31.0*** |
| *(Future)* | **POND OUTFALL (MHFD)** | 0.34 | 0.62 | 1.91 | 2.05 | 1.90 | | | | | | | | | | | | | | | | | | | |
| | | 2.78 | 3.29 | 4.87 | 5.17 | 5.16 | Tc for OS2 Used | | | | | | | | | | | | | | | | | | |

Calculated by: CVW
Date: 1/31/2022
Checked by: DLM

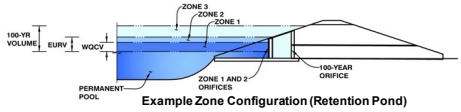
**HYDRAULIC CALCULATIONS**

# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

*MHFD-Detention, Version 4.03 (May 2020)*

**Project:** **CROSSROADS MIXED USE**

**Basin ID:** **POND 1**




**Example Zone Configuration (Retention Pond)**

## Watershed Information

| | | |
|---|---|---|
| Selected BMP Type = | **EDB** | |
| Watershed Area = | 32.20 | acres |
| Watershed Length = | 1,725 | ft |
| Watershed Length to Centroid = | 1,000 | ft |
| Watershed Slope = | 0.006 | ft/ft |
| Watershed Imperviousness = | 78.67% | percent |
| Percentage Hydrologic Soil Group A = | 95.4% | percent |
| Percentage Hydrologic Soil Group B = | 4.6% | percent |
| Percentage Hydrologic Soil Groups C/D = | 0.0% | percent |
| Target WQCV Drain Time = | 40.0 | hours |
| Location for 1-hr Rainfall Depths = | User Input | |

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

| | | | Optional User Overrides |
|---|---|---|---|
| Water Quality Capture Volume (WQCV) = | 0.859 | acre-feet | acre-feet |
| Excess Urban Runoff Volume (EURV) = | 3.293 | acre-feet | acre-feet |
| 2-yr Runoff Volume (P1 = 1.19 in.) = | 2.407 | acre-feet | 1.19 inches |
| 5-yr Runoff Volume (P1 = 1.5 in.) = | 3.122 | acre-feet | 1.50 inches |
| 10-yr Runoff Volume (P1 = 1.75 in.) = | 3.696 | acre-feet | 1.75 inches |
| 25-yr Runoff Volume (P1 = 2 in.) = | 4.394 | acre-feet | 2.00 inches |
| 50-yr Runoff Volume (P1 = 2.25 in.) = | 5.058 | acre-feet | 2.25 inches |
| 100-yr Runoff Volume (P1 = 2.52 in.) = | 5.833 | acre-feet | 2.52 inches |
| 500-yr Runoff Volume (P1 = 3.14 in.) = | 7.551 | acre-feet | inches |
| Approximate 2-yr Detention Volume = | 2.178 | acre-feet | |
| Approximate 5-yr Detention Volume = | 2.835 | acre-feet | |
| Approximate 10-yr Detention Volume = | 3.393 | acre-feet | |
| Approximate 25-yr Detention Volume = | 4.014 | acre-feet | |
| Approximate 50-yr Detention Volume = | 4.379 | acre-feet | |
| Approximate 100-yr Detention Volume = | 4.723 | acre-feet | |

## Define Zones and Basin Geometry

| | | |
|---|---|---|
| Zone 1 Volume (WQCV) = | 0.859 | acre-feet |
| Zone 2 Volume (EURV - Zone 1) = | 2.433 | acre-feet |
| Zone 3 Volume (100-year - Zones 1 & 2) = | 1.430 | acre-feet |
| Total Detention Basin Volume = | 4.723 | acre-feet |
| Initial Surcharge Volume (ISV) = | user | $ft^3$ |
| Initial Surcharge Depth (ISD) = | user | ft |
| Total Available Detention Depth ($H_{total}$) = | user | ft |
| Depth of Trickle Channel ($H_{TC}$) = | user | ft |
| Slope of Trickle Channel ($S_{TC}$) = | user | ft/ft |
| Slopes of Main Basin Sides ($S_{main}$) = | user | H:V |
| Basin Length-to-Width Ratio ($R_{L/W}$) = | user | |

| | | |
|---|---|---|
| Initial Surcharge Area ($A_{ISV}$) = | user | $ft^2$ |
| Surcharge Volume Length ($L_{ISV}$) = | user | ft |
| Surcharge Volume Width ($W_{ISV}$) = | user | ft |
| Depth of Basin Floor ($H_{FLOOR}$) = | user | ft |
| Length of Basin Floor ($L_{FLOOR}$) = | user | ft |
| Width of Basin Floor ($W_{FLOOR}$) = | user | ft |
| Area of Basin Floor ($A_{FLOOR}$) = | user | $ft^2$ |
| Volume of Basin Floor ($V_{FLOOR}$) = | user | $ft^3$ |
| Depth of Main Basin ($H_{MAIN}$) = | user | ft |
| Length of Main Basin ($L_{MAIN}$) = | user | ft |
| Width of Main Basin ($W_{MAIN}$) = | user | ft |
| Area of Main Basin ($A_{MAIN}$) = | user | $ft^2$ |
| Volume of Main Basin ($V_{MAIN}$) = | user | $ft^3$ |
| Calculated Total Basin Volume ($V_{total}$) = | **user** | acre-feet |

6287.68

Depth Increment = 2.00 ft

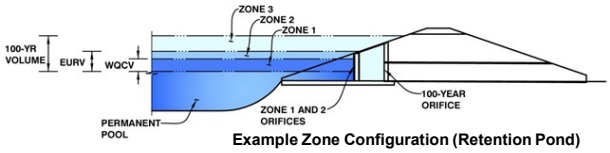
| Stage - Storage Description | Stage (ft) | Optional Override Stage (ft) | Length (ft) | Width (ft) | Area ($ft^2$) | Optional Override Area ($ft^2$) | Area (acre) | Volume ($ft^3$) | Volume (ac-ft) |
|---|---|---|---|---|---|---|---|---|---|
| **Top of Micropool** | -- | 0.00 | -- | -- | -- | 125 | 0.003 | | |
| **88** | -- | 0.40 | -- | -- | -- | 240 | 0.006 | 73 | 0.002 |
| **90** | -- | 2.40 | -- | -- | -- | 17,342 | 0.398 | 16,784 | 0.385 |
| **92** | -- | 4.40 | -- | -- | -- | 39,263 | 0.901 | 72,518 | 1.665 |
| **94** | -- | 6.40 | -- | -- | -- | 47,710 | 1.095 | 159,490 | 3.661 |
| **96** | -- | 8.40 | -- | -- | -- | 60,034 | 1.378 | 267,234 | 6.135 |

# DETENTION BASIN OUTLET STRUCTURE DESIGN

*MHFD-Detention, Version 4.03 (May 2020)*

**Project:** CROSSROADS MIXED USE

**Basin ID:** POND 1




**Example Zone Configuration (Retention Pond)**

| | Estimated Stage (ft) | Estimated Volume (ac-ft) | Outlet Type |
|---|---|---|---|
| Zone 1 (WQCV) | 3.35 | 0.859 | Orifice Plate |
| Zone 2 (EURV) | 6.06 | 2.433 | Orifice Plate |
| Zone 3 (100-year) | 7.32 | 1.430 | Weir&Pipe (Restrict) |
| Total (all zones) | | 4.723 | |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = N/A ft (distance below the filtration media surface)

Underdrain Orifice Diameter = N/A inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = N/A $ft^2$

Underdrain Orifice Centroid = N/A feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)

Depth at top of Zone using Orifice Plate = 6.08 ft (relative to basin bottom at Stage = 0 ft)

Orifice Plate: Orifice Vertical Spacing = N/A inches

Orifice Plate: Orifice Area per Row = N/A inches

Calculated Parameters for Plate

WQ Orifice Area per Row = N/A $ft^2$

Elliptical Half-Width = N/A feet

Elliptical Slot Centroid = N/A feet

Elliptical Slot Area = N/A $ft^2$

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (required) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|---|---|---|---|---|---|---|---|---|
| Stage of Orifice Centroid (ft) | 0.00 | 2.03 | 4.05 | | | | | |
| Orifice Area (sq. inches) | 3.77 | 6.25 | 12.60 | | | | | |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|---|---|---|---|---|---|---|---|---|
| Stage of Orifice Centroid (ft) | | | | | | | | |
| Orifice Area (sq. inches) | | | | | | | | |

User Input: Vertical Orifice (Circular or Rectangular)

| | Not Selected | Not Selected | |
|---|---|---|---|
| Invert of Vertical Orifice = | N/A | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Depth at top of Zone using Vertical Orifice = | N/A | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Vertical Orifice Diameter = | N/A | N/A | inches |

Calculated Parameters for Vertical Orifice

| | Not Selected | Not Selected | |
|---|---|---|---|
| Vertical Orifice Area = | N/A | N/A | $ft^2$ |
| Vertical Orifice Centroid = | N/A | N/A | feet |

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

| | Zone 3 Weir | Not Selected | |
|---|---|---|---|
| Overflow Weir Front Edge Height, Ho = | 6.09 | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Overflow Weir Front Edge Length = | 5.70 | N/A | feet |
| Overflow Weir Grate Slope = | 0.00 | N/A | H:V |
| Horiz. Length of Weir Sides = | 2.91 | N/A | feet |
| Overflow Grate Open Area % = | 70% | N/A | %, grate open area/total area |
| Debris Clogging % = | 50% | N/A | % |

Calculated Parameters for Overflow Weir

| | Zone 3 Weir | Not Selected | |
|---|---|---|---|
| Height of Grate Upper Edge, $H_t$ = | 6.09 | N/A | feet |
| Overflow Weir Slope Length = | 2.91 | N/A | feet |
| Grate Open Area / 100-yr Orifice Area = | 13.14 | N/A | |
| Overflow Grate Open Area w/o Debris = | 11.61 | N/A | $ft^2$ |
| Overflow Grate Open Area w/ Debris = | 5.81 | N/A | $ft^2$ |

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

| | Zone 3 Restrictor | Not Selected | |
|---|---|---|---|
| Depth to Invert of Outlet Pipe = | 0.33 | N/A | ft (distance below basin bottom at Stage = 0 ft) |
| Outlet Pipe Diameter = | 18.00 | N/A | inches |
| Restrictor Plate Height Above Pipe Invert = | 9.00 | | inches |

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

| | Zone 3 Restrictor | Not Selected | |
|---|---|---|---|
| Outlet Orifice Area = | 0.88 | N/A | $ft^2$ |
| Outlet Orifice Centroid = | 0.43 | N/A | feet |
| Half-Central Angle of Restrictor Plate on Pipe = | 1.57 | N/A | radians |

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage= 7.30 ft (relative to basin bottom at Stage = 0 ft)

Spillway Crest Length = 33.00 feet

Spillway End Slopes = 3.00 H:V

Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway

Spillway Design Flow Depth= 0.85 feet

Stage at Top of Freeboard = 9.15 feet

Basin Area at Top of Freeboard = 1.38 acres

Basin Volume at Top of Freeboard = 6.13 acre-ft

Routed Hydrograph Results

*The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).*

| Design Storm Return Period = | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year | 500 Year |
|---|---|---|---|---|---|---|---|---|---|
| One-Hour Rainfall Depth (in) = | N/A | N/A | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 | 3.14 |
| CUHP Runoff Volume (acre-ft) = | 0.859 | 3.293 | 2.407 | 3.122 | 3.696 | 4.394 | 5.058 | 5.833 | 7.551 |
| Inflow Hydrograph Volume (acre-ft) = | N/A | N/A | 2.407 | 3.122 | 3.696 | 4.394 | 5.058 | 5.833 | 7.551 |
| CUHP Predevelopment Peak Q (cfs) = | N/A | N/A | 0.2 | 0.3 | 0.5 | 5.1 | 9.2 | 14.8 | 26.5 |
| OPTIONAL Override Predevelopment Peak Q (cfs) = | N/A | N/A | | | | | | | |
| Predevelopment Unit Peak Flow, q (cfs/acre) = | N/A | N/A | 0.01 | 0.01 | 0.01 | 0.16 | 0.29 | 0.46 | 0.82 |
| Peak Inflow Q (cfs) = | N/A | N/A | 33.3 | 42.7 | 49.7 | 61.8 | 71.7 | 83.5 | 108.3 |
| Peak Outflow Q (cfs) = | 0.5 | 1.3 | 1.1 | 1.2 | 2.6 | 9.1 | 11.0 | 11.4 | 40.0 |
| Ratio Peak Outflow to Predevelopment Q = | N/A | N/A | N/A | 3.7 | 5.6 | 1.8 | 1.2 | 0.8 | 1.5 |
| Structure Controlling Flow = | Plate | Plate | Plate | Plate | Overflow Weir 1 | Overflow Weir 1 | Outlet Plate 1 | Outlet Plate 1 | Spillway |
| Max Velocity through Grate 1 (fps) = | N/A | N/A | N/A | N/A | 0.1 | 0.7 | 0.8 | 0.8 | 0.9 |
| Max Velocity through Grate 2 (fps) = | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Time to Drain 97% of Inflow Volume (hours) = | 38 | 67 | 61 | 66 | 69 | 68 | 67 | 66 | 62 |
| Time to Drain 99% of Inflow Volume (hours) = | **40** | 72 | 65 | 72 | 76 | 75 | 75 | 75 | 74 |
| Maximum Ponding Depth (ft) = | 3.35 | 6.06 | 5.02 | 5.70 | 6.19 | 6.44 | 6.75 | 7.28 | 7.72 |
| Area at Maximum Ponding Depth (acres) = | 0.63 | 1.06 | 0.96 | 1.03 | 1.07 | 1.10 | 1.14 | 1.22 | 1.28 |
| Maximum Volume Stored (acre-ft) = | 0.863 | 3.295 | 2.242 | 2.918 | 3.434 | 3.705 | 4.042 | 4.668 | 5.230 |

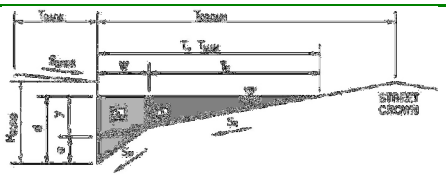
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use Existing Inlets

**Inlet ID:** INLET 1




**Gutter Geometry (Enter data in the blue cells)**

| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 14.0 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 26.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.010 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 20.0 | 26.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 6.0 | 12.0 | inches |
| Allow Flow Depth at Street Crown (leave blank for no) | | ☐ | ☐ | check = yes |

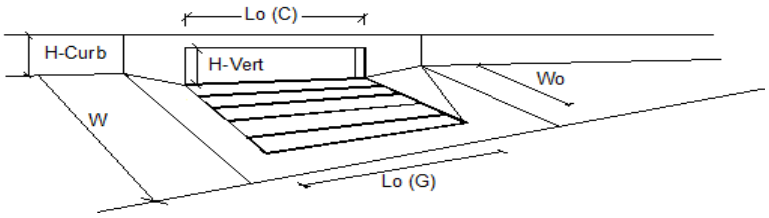
| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Spread Criterion** | **$Q_{allow}$ =** | **13.8** | **32.7** | **cfs** |

**WARNING: MINOR STORM max. allowable capacity is less than the design flow given on sheet 'Inlet Management'**

**Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

# INLET ON A CONTINUOUS GRADE

**Version 4.06 Released August 2018**




| **Design Information (Input)** | | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a') | $a_{LOCAL}$ = | 3.0 | 3.0 | inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | No = | 1 | 1 | |
| Length of a Single Unit Inlet (Grate or Curb Opening) | $L_o$ = | 10.00 | 10.00 | ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | $W_o$ = | N/A | N/A | ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | $C_f$-G = | N/A | N/A | |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | $C_f$-C = | 0.10 | 0.10 | |

| **Street Hydraulics: WARNING: Q > ALLOWABLE Q FOR MINOR STORM'** | | MINOR | MAJOR | |
|---|---|---|---|---|
| **Total Inlet Interception Capacity** | **Q =** | **8.4** | **11.1** | **cfs** |
| **Total Inlet Carry-Over Flow (flow bypassing inlet)** | **$Q_b$ =** | **5.8** | **15.4** | **cfs** |
| **Capture Percentage = $Q_a/Q_o$ =** | **C% =** | **59** | **42** | **%** |

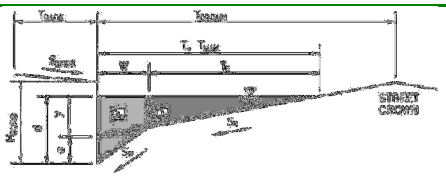
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use Existing Inlets

**Inlet ID:** INLET 2



**Gutter Geometry (Enter data in the blue cells)**

| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 14.0 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 26.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.010 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 20.0 | 26.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 6.0 | 12.0 | inches |
| Allow Flow Depth at Street Crown (leave blank for no) | | ☐ | ☐ | check = yes |

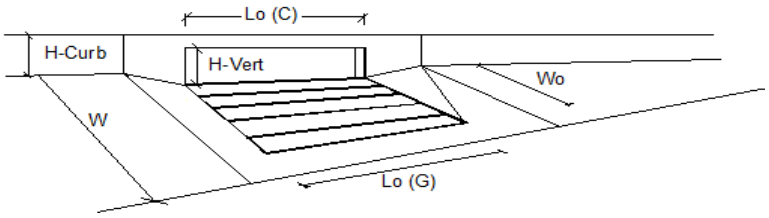
| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Spread Criterion** | **$Q_{allow}$ =** | **13.8** | **32.7** | **cfs** |

**Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

**Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

# INLET ON A CONTINUOUS GRADE

**Version 4.06 Released August 2018**




| **Design Information (Input)** | | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet | CDOT Type R Curb Opening | Type = | CDOT Type R Curb Opening | |
| Local Depression (additional to continuous gutter depression 'a') | | $a_{LOCAL}$ = | 3.0 | 3.0 inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | | No = | 1 | 1 |
| Length of a Single Unit Inlet (Grate or Curb Opening) | | $L_o$ = | 10.00 | 10.00 ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | | $W_o$ = | N/A | N/A ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | | $C_f$-G = | N/A | N/A |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | | $C_f$-C = | 0.10 | 0.10 |

| **Street Hydraulics: OK - Q < Allowable Street Capacity'** | | MINOR | MAJOR | |
|---|---|---|---|---|
| **Total Inlet Interception Capacity** | **Q =** | **3.1** | **5.3** | **cfs** |
| **Total Inlet Carry-Over Flow (flow bypassing inlet)** | **$Q_b$ =** | **0.0** | **0.7** | **cfs** |
| **Capture Percentage = $Q_a/Q_o$ =** | **C% =** | **100** | **88** | **%** |

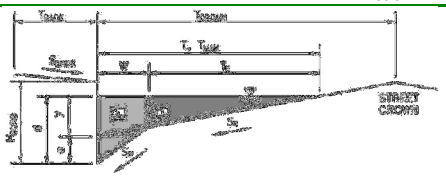
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use

**Inlet ID:** Inlet 3




**Gutter Geometry (Enter data in the blue cells)**

| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 7.5 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 27.0 | ft |
| Gutter Width | W = | 1.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.005 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 22.8 | 27.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 6.0 | 12.0 | inches |
| Allow Flow Depth at Street Crown (leave blank for no) | | ☐ | ☐ | check = yes |

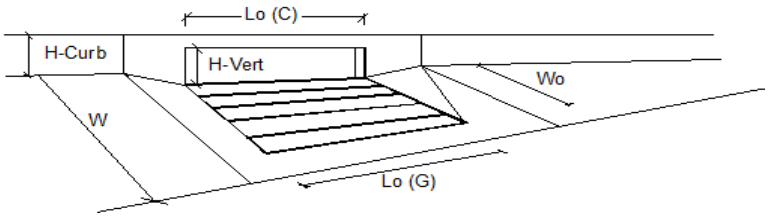
| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Spread Criterion** | **$Q_{allow}$ =** | **13.8** | **24.4** | **cfs** |

**Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

**Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

# INLET ON A CONTINUOUS GRADE

**Version 4.06 Released August 2018**




| **Design Information (Input)** | | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a') | $a_{LOCAL}$ = | 3.0 | 3.0 | inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | No = | 1 | 1 | |
| Length of a Single Unit Inlet (Grate or Curb Opening) | $L_o$ = | 10.00 | 10.00 | ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | $W_o$ = | N/A | N/A | ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | $C_f$-G = | N/A | N/A | |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | $C_f$-C = | 0.10 | 0.10 | |

| **Street Hydraulics: OK - Q < Allowable Street Capacity'** | | MINOR | MAJOR | |
|---|---|---|---|---|
| **Total Inlet Interception Capacity** | **Q =** | **6.7** | **9.8** | **cfs** |
| **Total Inlet Carry-Over Flow (flow bypassing inlet)** | **$Q_b$ =** | **3.5** | **13.5** | **cfs** |
| **Capture Percentage = $Q_a/Q_o$ =** | **C% =** | **65** | **42** | **%** |

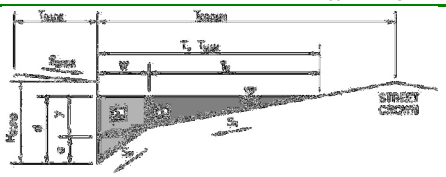
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use

**Inlet ID:** Inlet 4 - AT-GRADE



**Gutter Geometry (Enter data in the blue cells)**

| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 7.5 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 26.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.010 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 20.8 | 26.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 6.0 | 12.0 | inches |
| Allow Flow Depth at Street Crown (leave blank for no) | | ☐ | ☐ | check = yes |

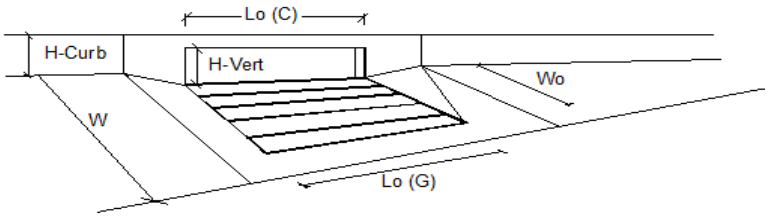
| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Spread Criterion** | **$Q_{allow}$ =** | **13.8** | **32.7** | **cfs** |

**Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

**Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

# INLET ON A CONTINUOUS GRADE

**Version 4.06 Released August 2018**




| **Design Information (Input)** | | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet | CDOT Type R Curb Opening | Type = | CDOT Type R Curb Opening | |
| Local Depression (additional to continuous gutter depression 'a') | | $a_{LOCAL}$ = | 3.0 | 3.0 inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | | No = | 1 | 1 |
| Length of a Single Unit Inlet (Grate or Curb Opening) | | $L_o$ = | 15.00 | 15.00 ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | | $W_o$ = | N/A | N/A ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | | $C_f$-G = | N/A | N/A |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | | $C_f$-C = | 0.10 | 0.10 |

| **Street Hydraulics: OK - Q < Allowable Street Capacity'** | | MINOR | MAJOR | |
|---|---|---|---|---|
| **Total Inlet Interception Capacity** | **Q =** | **6.5** | **10.6** | **cfs** |
| **Total Inlet Carry-Over Flow (flow bypassing inlet)** | **$Q_b$ =** | **0.0** | **1.8** | **cfs** |
| **Capture Percentage = $Q_a/Q_o$ =** | **C% =** | **100** | **85** | **%** |

**NEEHAH R-2501 TYPE C GRATE (100 YR)**

| | | | | | |
|---|---|---|---|---|---|
| width | N/A | area (sf) | 1.1 | open area x 50% | 0.55 |
| length | N/A | area blockage | 0.5 | | |
| perimeter (lf) | 4.9 | perimeter blockage | 3 | avail perm. (lf) | 1.9 |

| | | Orifice (cfs) | Weir (cfs) |
|---|---|---|---|
| 58 | 0 | 0.00 | 0.00 |
| 58.125 | 0.125 | 0.94 | 0.26 |
| 58.25 | 0.25 | 1.32 | 0.74 |
| 58.375 | 0.375 | 1.62 | 1.35 |
| 58.5 | 0.5 | 1.87 | 2.08 |
| 58.625 | 0.625 | 2.09 | 2.91 |
| 58.75 | 0.75 | 2.29 | 3.83 |
| 58.875 | 0.875 | 2.48 | 4.82 |
| 59 | 1 | 2.65 | 5.89 |
| 59.125 | 1.125 | 2.81 | 7.03 |
| 59.25 | 1.25 | 2.96 | 8.23 |
| 59.375 | 1.375 | 3.11 | 9.50 |
| 59.5 | 1.5 | 3.24 | 10.82 |
| 59.625 | 1.625 | 3.38 | 12.20 |
| 59.75 | 1.75 | 3.50 | 13.64 |
| 59.875 | 1.875 | 3.63 | 15.12 |
| 60 | 2 | 3.75 | 16.66 |
| 60.125 | 2.125 | 3.86 | 18.25 |
| 60.25 | 2.25 | 3.97 | 19.88 |
| 60.375 | 2.375 | 4.08 | 21.56 |
| 60.5 | 2.5 | 4.19 | 23.28 |
| 60.625 | 2.625 | 4.29 | 25.05 |
| 60.75 | 2.75 | 4.39 | 26.86 |
| 60.875 | 2.875 | 4.49 | 28.71 |
| 61 | 3 | 4.59 | 30.61 |
| 61.125 | 3.125 | 4.68 | 32.54 |
| 61.25 | 3.25 | 4.77 | 34.51 |

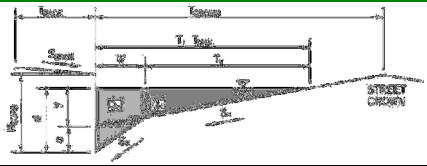
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use

**Inlet ID:** Inlet 5



**Gutter Geometry (Enter data in the blue cells)**

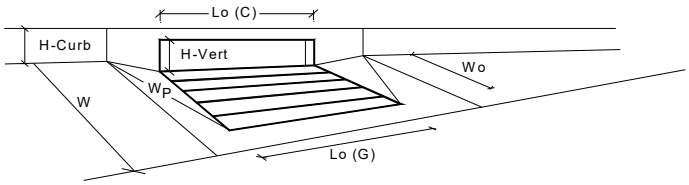
| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 7.5 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 26.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.000 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 20.8 | 26.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 6.0 | 12.0 | inches |
| Check boxes are not applicable in SUMP conditions | | ☐ | ☐ | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Depth Criterion** | **$Q_{allow}$ =** | **SUMP** | **SUMP** | **cfs** |

# INLET IN A SUMP OR SAG LOCATION

**Version 4.06 Released August 2018**




| Design Information (Input) | | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a' from above) | $a_{local}$ = | 3.00 | 3.00 | inches |
| Number of Unit Inlets (Grate or Curb Opening) | No = | 1 | 1 | |
| Water Depth at Flowline (outside of local depression) | Ponding Depth = | 6.0 | 12.0 | inches |
| **Grate Information** | | MINOR | MAJOR | ☑ Override Depths |
| Length of a Unit Grate | $L_o$ (G) = | N/A | N/A | feet |
| Width of a Unit Grate | $W_o$ = | N/A | N/A | feet |
| Area Opening Ratio for a Grate (typical values 0.15-0.90) | $A_{ratio}$ = | N/A | N/A | |
| Clogging Factor for a Single Grate (typical value 0.50 - 0.70) | $C_f$ (G) = | N/A | N/A | |
| Grate Weir Coefficient (typical value 2.15 - 3.60) | $C_w$ (G) = | N/A | N/A | |
| Grate Orifice Coefficient (typical value 0.60 - 0.80) | $C_o$ (G) = | N/A | N/A | |
| **Curb Opening Information** | | MINOR | MAJOR | |
| Length of a Unit Curb Opening | $L_o$ (C) = | 15.00 | 15.00 | feet |
| Height of Vertical Curb Opening in Inches | $H_{vert}$ = | 6.00 | 6.00 | inches |
| Height of Curb Orifice Throat in Inches | $H_{throat}$ = | 6.00 | 6.00 | inches |
| Angle of Throat (see USDCM Figure ST-5) | Theta = | 63.40 | 63.40 | degrees |
| Side Width for Depression Pan (typically the gutter width of 2 feet) | $W_p$ = | 2.00 | 2.00 | feet |
| Clogging Factor for a Single Curb Opening (typical value 0.10) | $C_f$ (C) = | 0.10 | 0.10 | |
| Curb Opening Weir Coefficient (typical value 2.3-3.7) | $C_w$ (C) = | 3.60 | 3.60 | |
| Curb Opening Orifice Coefficient (typical value 0.60 - 0.70) | $C_o$ (C) = | 0.67 | 0.67 | |
| **Grate Flow Analysis (Calculated)** | | MINOR | MAJOR | |
| Clogging Coefficient for Multiple Units | Coef = | N/A | N/A | |
| Clogging Factor for Multiple Units | Clog = | N/A | N/A | |
| **Grate Capacity as a Weir (based on Modified HEC22 Method)** | | MINOR | MAJOR | |
| Interception without Clogging | $Q_{wi}$ = | N/A | N/A | cfs |
| Interception with Clogging | $Q_{wa}$ = | N/A | N/A | cfs |
| **Grate Capacity as a Orifice (based on Modified HEC22 Method)** | | MINOR | MAJOR | |
| Interception without Clogging | $Q_{oi}$ = | N/A | N/A | cfs |
| Interception with Clogging | $Q_{oa}$ = | N/A | N/A | cfs |
| **Grate Capacity as Mixed Flow** | | MINOR | MAJOR | |
| Interception without Clogging | $Q_{mi}$ = | N/A | N/A | cfs |
| Interception with Clogging | $Q_{ma}$ = | N/A | N/A | cfs |
| **Resulting Grate Capacity (assumes clogged condition)** | **$Q_{Grate}$ =** | **N/A** | **N/A** | **cfs** |
| **Curb Opening Flow Analysis (Calculated)** | | MINOR | MAJOR | |
| Clogging Coefficient for Multiple Units | Coef = | 1.31 | 1.31 | |
| Clogging Factor for Multiple Units | Clog = | 0.04 | 0.04 | |
| **Curb Opening as a Weir (based on Modified HEC22 Method)** | | MINOR | MAJOR | |
| Interception without Clogging | $Q_{wi}$ = | 10.4 | 51.0 | cfs |
| Interception with Clogging | $Q_{wa}$ = | 9.9 | 48.8 | cfs |
| **Curb Opening as an Orifice (based on Modified HEC22 Method)** | | MINOR | MAJOR | |
| Interception without Clogging | $Q_{oi}$ = | 29.4 | 40.9 | cfs |
| Interception with Clogging | $Q_{oa}$ = | 28.1 | 39.1 | cfs |
| **Curb Opening Capacity as Mixed Flow** | | MINOR | MAJOR | |
| Interception without Clogging | $Q_{mi}$ = | 16.2 | 42.5 | cfs |
| Interception with Clogging | $Q_{ma}$ = | 15.5 | 40.6 | cfs |
| **Resulting Curb Opening Capacity (assumes clogged condition)** | **$Q_{Curb}$ =** | **9.9** | **39.1** | **cfs** |
| **Resultant Street Conditions** | | MINOR | MAJOR | |
| Total Inlet Length | L = | 15.00 | 15.00 | feet |
| Resultant Street Flow Spread (based on street geometry from above) | T = | 18.9 | 43.7 | ft.>T-Crown |
| Resultant Flow Depth at Street Crown | $d_{CROWN}$ = | 0.0 | 4.2 | inches |
| **Low Head Performance Reduction (Calculated)** | | MINOR | MAJOR | |
| Depth for Grate Midwidth | $d_{Grate}$ = | N/A | N/A | ft |
| Depth for Curb Opening Weir Equation | $d_{Curb}$ = | 0.34 | 0.83 | ft |
| Combination Inlet Performance Reduction Factor for Long Inlets | $RF_{Combination}$ = | 0.57 | 1.00 | |
| Curb Opening Performance Reduction Factor for Long Inlets | $RF_{Curb}$ = | 0.79 | 1.00 | |
| Grated Inlet Performance Reduction Factor for Long Inlets | $RF_{Grate}$ = | N/A | N/A | |
| | | MINOR | MAJOR | |
| **Total Inlet Interception Capacity (assumes clogged condition)** | **$Q_a$ =** | **9.9** | **39.1** | **cfs** |
| **Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)** | $Q_{PEAK\ REQUIRED}$ = | 9.8 | 25.8 | cfs |

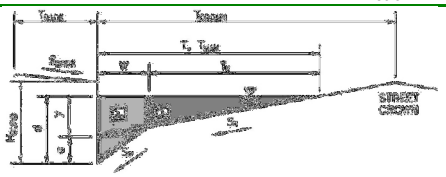
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use

**Inlet ID:** Inlet 6



**Gutter Geometry (Enter data in the blue cells)**

| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 7.5 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 14.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.020 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 14.0 | 14.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 4.4 | 8.8 | inches |
| Allow Flow Depth at Street Crown (leave blank for no) | | ☐ | ☑ | check = yes |

**Maximum Capacity for 1/2 Street based On Allowable Spread**

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Water Depth without Gutter Depression (Eq. ST-2) | y = | 3.36 | 3.36 | inches |
| Vertical Depth between Gutter Lip and Gutter Flowline (usually 2") | $d_C$ = | 2.0 | 2.0 | inches |
| Gutter Depression ($d_C$ - (W * $S_x$ * 12)) | a = | 1.51 | 1.51 | inches |
| Water Depth at Gutter Flowline | d = | 4.87 | 4.87 | inches |
| Allowable Spread for Discharge outside the Gutter Section W (T - W) | $T_X$ = | 12.0 | 12.0 | ft |
| Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7) | $E_O$ = | 0.425 | 0.425 | |
| Discharge outside the Gutter Section W, carried in Section $T_X$ | $Q_X$ = | 5.5 | 5.5 | cfs |
| Discharge within the Gutter Section W ($Q_T$ - $Q_X$) | $Q_W$ = | 4.1 | 4.1 | cfs |
| Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns) | $Q_{BACK}$ = | 0.0 | 0.0 | cfs |
| **Maximum Flow Based On Allowable Spread** | **$Q_T$ =** | **9.6** | **9.6** | **cfs** |
| Flow Velocity within the Gutter Section | V = | 6.3 | 6.3 | fps |
| V*d Product: Flow Velocity times Gutter Flowline Depth | V*d = | 2.6 | 2.6 | |

**Maximum Capacity for 1/2 Street based on Allowable Depth**

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Theoretical Water Spread | $T_{TH}$ = | 11.8 | 30.4 | ft |
| Theoretical Spread for Discharge outside the Gutter Section W (T - W) | $T_{X\,TH}$ = | 9.8 | 28.4 | ft |
| Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7) | $E_O$ = | 0.498 | 0.191 | |
| Theoretical Discharge outside the Gutter Section W, carried in Section $T_{X\,TH}$ | $Q_{X\,TH}$ = | 3.2 | 54.6 | cfs |
| Actual Discharge outside the Gutter Section W, (limited by distance $T_{CROWN}$) | $Q_X$ = | 3.2 | 42.0 | cfs |
| Discharge within the Gutter Section W ($Q_d$ - $Q_X$) | $Q_W$ = | 3.2 | 12.9 | cfs |
| Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns) | $Q_{BACK}$ = | 0.0 | 3.8 | cfs |
| Total Discharge for Major & Minor Storm (Pre-Safety Factor) | Q = | 6.4 | 58.7 | cfs |
| Average Flow Velocity Within the Gutter Section | V = | 5.7 | 9.9 | fps |
| V*d Product: Flow Velocity Times Gutter Flowline Depth | V*d = | 2.1 | 7.3 | |
| Slope-Based Depth Safety Reduction Factor for Major & Minor (d ≥ 6") Storm | R = | 1.00 | 0.83 | |
| **Max Flow Based on Allowable Depth (Safety Factor Applied)** | **$Q_d$ =** | **6.4** | **49.0** | **cfs** |
| Resultant Flow Depth at Gutter Flowline (Safety Factor Applied) | d = | 4.35 | 8.26 | inches |
| Resultant Flow Depth at Street Crown (Safety Factor Applied) | $d_{CROWN}$ = | 0.00 | 3.39 | inches |

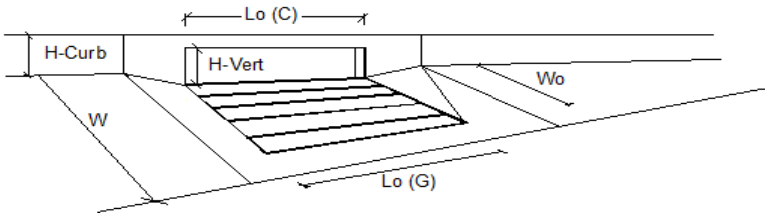
| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Depth Criterion** | **$Q_{allow}$ =** | **6.4** | **49.0** | **cfs** |

**Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

**Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

# INLET ON A CONTINUOUS GRADE

**Version 4.06 Released August 2018**




| **Design Information (Input)** | | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a') | $a_{LOCAL}$ = | 3.0 | 3.0 | inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | No = | 1 | 1 | |
| Length of a Single Unit Inlet (Grate or Curb Opening) | $L_o$ = | 10.00 | 10.00 | ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | $W_o$ = | N/A | N/A | ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | $C_f$-G = | N/A | N/A | |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | $C_f$-C = | 0.10 | 0.10 | |

| **Street Hydraulics: OK - Q < Allowable Street Capacity'** | | MINOR | MAJOR | |
|---|---|---|---|---|
| **Total Inlet Interception Capacity** | **Q =** | **4.0** | **6.0** | **cfs** |
| **Total Inlet Carry-Over Flow (flow bypassing inlet)** | **$Q_b$ =** | **0.1** | **1.4** | **cfs** |
| **Capture Percentage = $Q_a/Q_o$ =** | **C% =** | **98** | **81** | **%** |

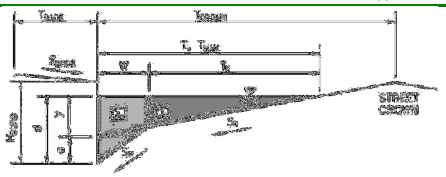
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use

**Inlet ID:** Inlet 7



**Gutter Geometry (Enter data in the blue cells)**

| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 7.5 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 14.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.020 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 14.0 | 14.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 4.4 | 8.8 | inches |
| Allow Flow Depth at Street Crown (leave blank for no) | | ☐ | ☑ | check = yes |

**Maximum Capacity for 1/2 Street based On Allowable Spread**

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Water Depth without Gutter Depression (Eq. ST-2) | y = | 3.36 | 3.36 | inches |
| Vertical Depth between Gutter Lip and Gutter Flowline (usually 2") | $d_C$ = | 2.0 | 2.0 | inches |
| Gutter Depression ($d_C$ - (W * $S_x$ * 12)) | a = | 1.51 | 1.51 | inches |
| Water Depth at Gutter Flowline | d = | 4.87 | 4.87 | inches |
| Allowable Spread for Discharge outside the Gutter Section W (T - W) | $T_X$ = | 12.0 | 12.0 | ft |
| Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7) | $E_O$ = | 0.425 | 0.425 | |
| Discharge outside the Gutter Section W, carried in Section $T_X$ | $Q_X$ = | 5.5 | 5.5 | cfs |
| Discharge within the Gutter Section W ($Q_T$ - $Q_X$) | $Q_W$ = | 4.1 | 4.1 | cfs |
| Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns) | $Q_{BACK}$ = | 0.0 | 0.0 | cfs |
| **Maximum Flow Based On Allowable Spread** | **$Q_T$ =** | **9.6** | **9.6** | **cfs** |
| Flow Velocity within the Gutter Section | V = | 6.3 | 6.3 | fps |
| V*d Product: Flow Velocity times Gutter Flowline Depth | V*d = | 2.6 | 2.6 | |

**Maximum Capacity for 1/2 Street based on Allowable Depth**

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Theoretical Water Spread | $T_{TH}$ = | 12.0 | 30.4 | ft |
| Theoretical Spread for Discharge outside the Gutter Section W (T - W) | $T_{X\,TH}$ = | 10.0 | 28.4 | ft |
| Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. ST-7) | $E_O$ = | 0.490 | 0.191 | |
| Theoretical Discharge outside the Gutter Section W, carried in Section $T_{X\,TH}$ | $Q_{X\,TH}$ = | 3.4 | 54.6 | cfs |
| Actual Discharge outside the Gutter Section W, (limited by distance $T_{CROWN}$) | $Q_X$ = | 3.4 | 42.0 | cfs |
| Discharge within the Gutter Section W ($Q_d$ - $Q_X$) | $Q_W$ = | 3.3 | 12.9 | cfs |
| Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns) | $Q_{BACK}$ = | 0.0 | 3.8 | cfs |
| Total Discharge for Major & Minor Storm (Pre-Safety Factor) | Q = | 6.7 | 58.7 | cfs |
| Average Flow Velocity Within the Gutter Section | V = | 5.8 | 9.9 | fps |
| V*d Product: Flow Velocity Times Gutter Flowline Depth | V*d = | 2.1 | 7.3 | |
| Slope-Based Depth Safety Reduction Factor for Major & Minor (d ≥ 6") Storm | R = | 1.00 | 0.83 | |
| **Max Flow Based on Allowable Depth (Safety Factor Applied)** | **$Q_d$ =** | **6.7** | **49.0** | **cfs** |
| Resultant Flow Depth at Gutter Flowline (Safety Factor Applied) | d = | 4.40 | 8.26 | inches |
| Resultant Flow Depth at Street Crown (Safety Factor Applied) | $d_{CROWN}$ = | 0.00 | 3.39 | inches |

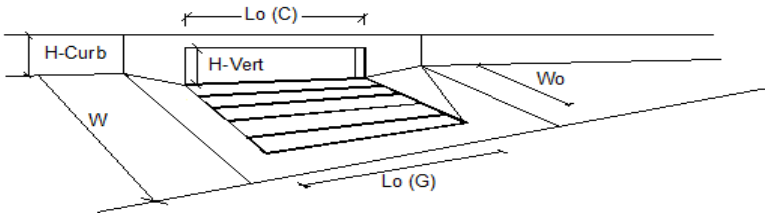
| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Depth Criterion** | **$Q_{allow}$ =** | **6.7** | **49.0** | **cfs** |

**Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

**Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

# INLET ON A CONTINUOUS GRADE

**Version 4.06 Released August 2018**




| **Design Information (Input)** | | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet | CDOT Type R Curb Opening | Type = | CDOT Type R Curb Opening | |
| Local Depression (additional to continuous gutter depression 'a') | | $a_{LOCAL}$ = | 3.0 | 3.0 inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | | No = | 1 | 1 |
| Length of a Single Unit Inlet (Grate or Curb Opening) | | $L_o$ = | 10.00 | 10.00 ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | | $W_o$ = | N/A | N/A ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | | $C_f$-G = | N/A | N/A |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | | $C_f$-C = | 0.10 | 0.10 |

| **Street Hydraulics: OK - Q < Allowable Street Capacity'** | | MINOR | MAJOR | |
|---|---|---|---|---|
| **Total Inlet Interception Capacity** | **Q =** | **5.5** | **7.7** | **cfs** |
| **Total Inlet Carry-Over Flow (flow bypassing inlet)** | **$Q_b$ =** | **0.9** | **4.0** | **cfs** |
| **Capture Percentage = $Q_a/Q_o$ =** | **C% =** | **86** | **66** | **%** |

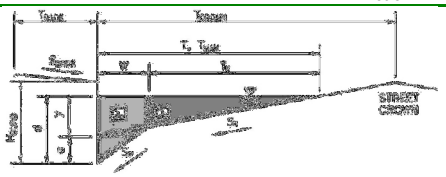
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use

**Inlet ID:** Inlet 8



**Gutter Geometry (Enter data in the blue cells)**

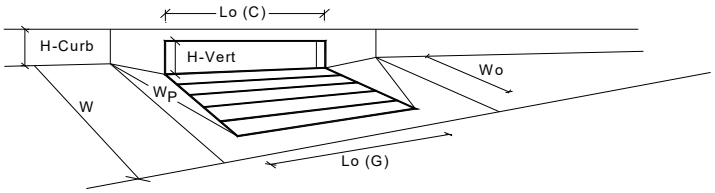
| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 7.5 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 14.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.000 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 14.0 | 14.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 4.4 | 12.0 | inches |
| Check boxes are not applicable in SUMP conditions | | ☐ | ☐ | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Depth Criterion** | **$Q_{allow}$ =** | **SUMP** | **SUMP** | **cfs** |

# INLET IN A SUMP OR SAG LOCATION

**Version 4.06 Released August 2018**




| **Design Information (Input)** | CDOT Type R Curb Opening | | | |
|---|---|---|---|---|
| | | MINOR | MAJOR | |
| Type of Inlet | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a' from above) | $a_{local}$ = | 3.00 | 3.00 | inches |
| Number of Unit Inlets (Grate or Curb Opening) | No = | 1 | 1 | |
| Water Depth at Flowline (outside of local depression) | Ponding Depth = | 4.4 | 8.0 | inches |
| **Grate Information** | | MINOR | MAJOR | ☑ Override Depths |
| Length of a Unit Grate | $L_o$ (G) = | N/A | N/A | feet |
| Width of a Unit Grate | $W_o$ = | N/A | N/A | feet |
| Area Opening Ratio for a Grate (typical values 0.15-0.90) | $A_{ratio}$ = | N/A | N/A | |
| Clogging Factor for a Single Grate (typical value 0.50 - 0.70) | $C_f$ (G) = | N/A | N/A | |
| Grate Weir Coefficient (typical value 2.15 - 3.60) | $C_w$ (G) = | N/A | N/A | |
| Grate Orifice Coefficient (typical value 0.60 - 0.80) | $C_o$ (G) = | N/A | N/A | |
| **Curb Opening Information** | | MINOR | MAJOR | |
| Length of a Unit Curb Opening | $L_o$ (C) = | 10.00 | 10.00 | feet |
| Height of Vertical Curb Opening in Inches | $H_{vert}$ = | 6.00 | 6.00 | inches |
| Height of Curb Orifice Throat in Inches | $H_{throat}$ = | 6.00 | 6.00 | inches |
| Angle of Throat (see USDCM Figure ST-5) | Theta = | 63.40 | 63.40 | degrees |
| Side Width for Depression Pan (typically the gutter width of 2 feet) | $W_p$ = | 2.00 | 2.00 | feet |
| Clogging Factor for a Single Curb Opening (typical value 0.10) | $C_f$ (C) = | 0.10 | 0.10 | |
| Curb Opening Weir Coefficient (typical value 2.3-3.7) | $C_w$ (C) = | 3.60 | 3.60 | |
| Curb Opening Orifice Coefficient (typical value 0.60 - 0.70) | $C_o$ (C) = | 0.67 | 0.67 | |
| **Low Head Performance Reduction (Calculated)** | | MINOR | MAJOR | |
| Depth for Grate Midwidth | $d_{Grate}$ = | N/A | N/A | ft |
| Depth for Curb Opening Weir Equation | $d_{Curb}$ = | 0.20 | 0.50 | ft |
| Combination Inlet Performance Reduction Factor for Long Inlets | $RF_{Combination}$ = | 0.41 | 0.75 | |
| Curb Opening Performance Reduction Factor for Long Inlets | $RF_{Curb}$ = | 0.82 | 1.00 | |
| Grated Inlet Performance Reduction Factor for Long Inlets | $RF_{Grate}$ = | N/A | N/A | |
| | | MINOR | MAJOR | |
| **Total Inlet Interception Capacity (assumes clogged condition)** | **$Q_a$ =** | **3.3** | **16.3** | **cfs** |
| **Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)** | $Q_{PEAK\ REQUIRED}$ = | 2.1 | 3.8 | cfs |

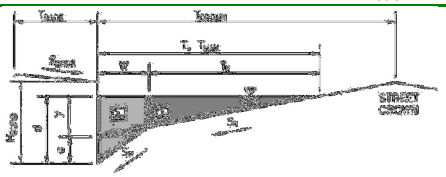
Version 4.06 Released August 2018

# ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads Mixed Use

**Inlet ID:** Inlet 9



**Gutter Geometry (Enter data in the blue cells)**

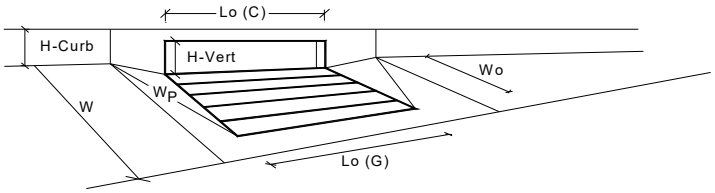
| | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 7.5 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.020 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 14.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.020 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.000 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 14.0 | 14.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 4.4 | 12.0 | inches |
| Check boxes are not applicable in SUMP conditions | | ☐ | ☐ | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| **MINOR STORM Allowable Capacity is based on Depth Criterion** | | | | |
| **MAJOR STORM Allowable Capacity is based on Depth Criterion** | **$Q_{allow}$ =** | **SUMP** | **SUMP** | **cfs** |

# INLET IN A SUMP OR SAG LOCATION

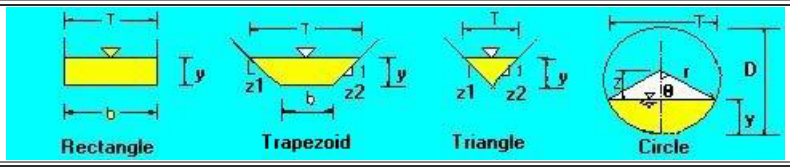
**Version 4.06 Released August 2018**




| **Design Information (Input)** | CDOT Type R Curb Opening | | | |
|---|---|---|---|---|
| | | MINOR | MAJOR | |
| Type of Inlet | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a' from above) | $a_{local}$ = | 3.00 | 3.00 | inches |
| Number of Unit Inlets (Grate or Curb Opening) | No = | 1 | 1 | |
| Water Depth at Flowline (outside of local depression) | Ponding Depth = | 4.4 | 8.0 | inches |
| **Grate Information** | | MINOR | MAJOR | ☑ Override Depths |
| Length of a Unit Grate | $L_o$ (G) = | N/A | N/A | feet |
| Width of a Unit Grate | $W_o$ = | N/A | N/A | feet |
| Area Opening Ratio for a Grate (typical values 0.15-0.90) | $A_{ratio}$ = | N/A | N/A | |
| Clogging Factor for a Single Grate (typical value 0.50 - 0.70) | $C_f$ (G) = | N/A | N/A | |
| Grate Weir Coefficient (typical value 2.15 - 3.60) | $C_w$ (G) = | N/A | N/A | |
| Grate Orifice Coefficient (typical value 0.60 - 0.80) | $C_o$ (G) = | N/A | N/A | |
| **Curb Opening Information** | | MINOR | MAJOR | |
| Length of a Unit Curb Opening | $L_o$ (C) = | 15.00 | 15.00 | feet |
| Height of Vertical Curb Opening in Inches | $H_{vert}$ = | 6.00 | 6.00 | inches |
| Height of Curb Orifice Throat in Inches | $H_{throat}$ = | 6.00 | 6.00 | inches |
| Angle of Throat (see USDCM Figure ST-5) | Theta = | 63.40 | 63.40 | degrees |
| Side Width for Depression Pan (typically the gutter width of 2 feet) | $W_p$ = | 2.00 | 2.00 | feet |
| Clogging Factor for a Single Curb Opening (typical value 0.10) | $C_f$ (C) = | 0.10 | 0.10 | |
| Curb Opening Weir Coefficient (typical value 2.3-3.7) | $C_w$ (C) = | 3.60 | 3.60 | |
| Curb Opening Orifice Coefficient (typical value 0.60 - 0.70) | $C_o$ (C) = | 0.67 | 0.67 | |
| **Low Head Performance Reduction (Calculated)** | | MINOR | MAJOR | |
| Depth for Grate Midwidth | $d_{Grate}$ = | N/A | N/A | ft |
| Depth for Curb Opening Weir Equation | $d_{Curb}$ = | 0.20 | 0.50 | ft |
| Combination Inlet Performance Reduction Factor for Long Inlets | $RF_{Combination}$ = | 0.41 | 0.75 | |
| Curb Opening Performance Reduction Factor for Long Inlets | $RF_{Curb}$ = | 0.67 | 0.89 | |
| Grated Inlet Performance Reduction Factor for Long Inlets | $RF_{Grate}$ = | N/A | N/A | |
| | | MINOR | MAJOR | |
| **Total Inlet Interception Capacity (assumes clogged condition)** | **$Q_a$ =** | **3.7** | **20.3** | **cfs** |
| **Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)** | $Q_{PEAK\ REQUIRED}$ = | 3.7 | 15.3 | cfs |

# HISTORIC DRAINAGE MAP: A-A' 100 YR ANALYSIS

Select Channel Type: Trapezoid



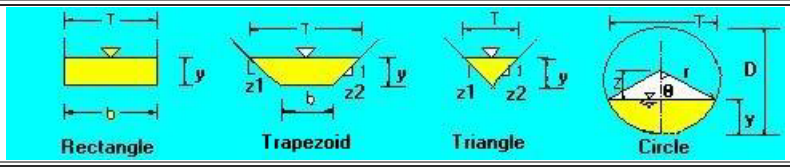

Depth from Q

Select unit system: Feet(ft)

| | | |
|---|---|---|
| Channel slope: .018 ft/ft | Water depth(y): 0.59 ft | Bottom width(b) 5 ft |
| Flow velocity 4.02 ft/s | LeftSlope (Z1): 19 to 1 (H:V) | RightSlope (Z2): 9.9 to 1 (H:V) |
| Flow discharge 31.9 ft^3/s | Input n value .025 or select n | |
| Calculate! | Status: Calculation finished | Reset |
| Wetted perimeter 22.04 ft | Flow area 7.94 ft^2 | Top width(T) 21.99 ft |
| Specific energy 0.84 ft | Froude number 1.18 | Flow status Supercritical flow |
| Critical depth 0.64 ft | Critical slope 0.0125 ft/ft | Velocity head 0.25 ft |

# EXISTING DRAINAGE MAP: A-A' 100 YR ANALYSIS

Select Channel Type: Trapezoid



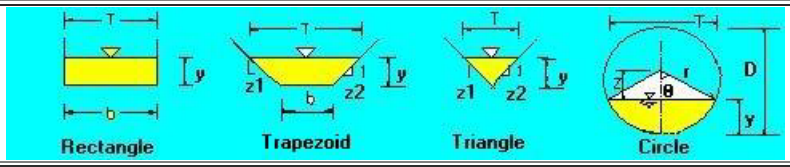

Depth from Q

Select unit system: Feet(ft)

| | | |
|---|---|---|
| Channel slope: .018 ft/ft | Water depth(y): 0.55 ft | Bottom width(b) 5 ft |
| Flow velocity 3.89 ft/s | LeftSlope (Z1): 19 to 1 (H:V) | RightSlope (Z2): 9.9 to 1 (H:V) |
| Flow discharge 28 ft^3/s | Input n value .025 or select n | |
| Calculate! | Status: Calculation finished | Reset |
| Wetted perimeter 21.04 ft | Flow area 7.2 ft^2 | Top width(T) 21 ft |
| Specific energy 0.79 ft | Froude number 1.17 | Flow status Supercritical flow |
| Critical depth 0.6 ft | Critical slope 0.0123 ft/ft | Velocity head 0.23 ft |

# FUTURE DRAINAGE MAP: A-A' 100 YR ANALYSIS

Select Channel Type: Trapezoid



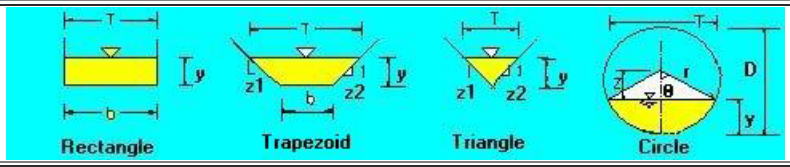

Depth from Q

Select unit system: Feet(ft)

| | | |
|---|---|---|
| Channel slope: .018 ft/ft | Water depth(y): 0.58 ft | Bottom width(b) 5 ft |
| Flow velocity 3.96 ft/s | LeftSlope (Z1): 19 to 1 (H:V) | RightSlope (Z2): 9.9 to 1 (H:V) |
| Flow discharge 31 ft^3/s | Input n value .025 or select n | |
| Calculate! | Status: Calculation finished | Reset |
| Wetted perimeter 21.9 ft | Flow area 7.83 ft^2 | Top width(T) 21.85 ft |
| Specific energy 0.83 ft | Froude number 1.17 | Flow status Supercritical flow |
| Critical depth 0.63 ft | Critical slope 0.0122 ft/ft | Velocity head 0.24 ft |

# FUTURE DRAINAGE MAP: B-B' 100 YR ANALYSIS

Select Channel Type: Trapezoid



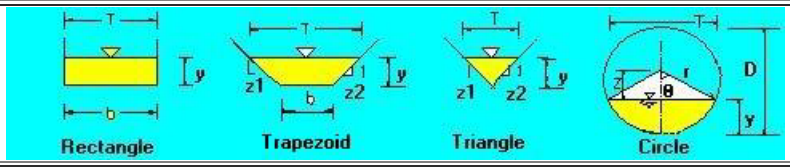

Depth from Q

Select unit system: Feet(ft)

| | | |
|---|---|---|
| Channel slope: .015 ft/ft | Water depth(y): 0.44 ft | Bottom width(b) 0 ft |
| Flow velocity 2.506355 ft/s | LeftSlope (Z1): 3 to 1 (H:V) | RightSlope (Z2): 3 to 1 (H:V) |
| Flow discharge 1.43 ft^3/s | Input n value .025 or select n | |
| Calculate! | Status: Calculation finished | Reset |
| Wetted perimeter 2.76 ft | Flow area 0.57 ft^2 | Top width(T) 2.62 ft |
| Specific energy 0.53 ft | Froude number 0.95 | Flow status Subcritical flow |
| Critical depth 0.43 ft | Critical slope 0.0154 ft/ft | Velocity head 0.1 ft |

# FUTURE DRAINAGE MAP: C-C' 100 YR ANALYSIS

Select Channel Type: Trapezoid



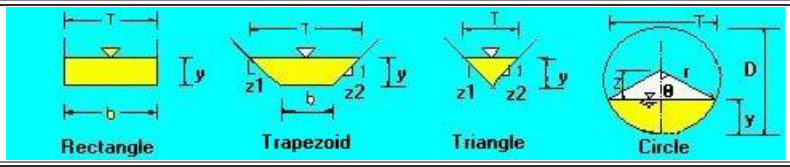

Depth from Q

Select unit system: Feet(ft)

| | | |
|---|---|---|
| Channel slope: .010 ft/ft | Water depth(y): 0.66 ft | Bottom width(b) 0 ft |
| Flow velocity 2.809 ft/s | LeftSlope (Z1): 14 to 1 (H:V) | RightSlope (Z2): 4 to 1 (H:V) |
| Flow discharge 10.9 ft^3/s | Input n value .025 or select n | |
| Calculate! | Status: Calculation finished | Reset |
| Wetted perimeter 11.92 ft | Flow area 3.88 ft^2 | Top width(T) 11.82 ft |
| Specific energy 0.78 ft | Froude number 0.86 | Flow status Subcritical flow |
| Critical depth 0.62 ft | Critical slope 0.0133 ft/ft | Velocity head 0.12 ft |

# FUTURE DRAINAGE MAP: D-D' 100 YR ANALYSIS

Select Channel Type: Trapezoid




Depth from Q

Select unit system: Feet(ft)

| | | |
|---|---|---|
| Channel slope: .015 ft/ft | Water depth(y): 0.89 ft | Bottom width(b) 0 ft |
| Flow velocity 4.066 ft/s | LeftSlope (Z1): 3 to 1 (H:V) | RightSlope (Z2): 3 to 1 (H:V) |
| Flow discharge 9.7 ft^3/s | Input n value .025 or select n | |
| Calculate! | Status: Calculation finished | Reset |
| Wetted perimeter 5.64 ft | Flow area 2.39 ft^2 | Top width(T) 5.35 ft |
| Specific energy 1.15 ft | Froude number 1.07 | Flow status Supercritical flow |
| Critical depth 0.92 ft | Critical slope 0.0124 ft/ft | Velocity head 0.26 ft |

***CROSSROADS MIXED USE FILING NO. 1***

***EMERGENCY SPILLWAY CALCULATIONS PRIVATE FSD POND (POND 1)***

| *Horizontal Broad-Crested Weir (Eqn 12-20 UDFCD)* | | | | | | |
|---|---|---|---|---|---|---|
| *Variable* | | | *Solve For* | | |
| | | | L (ft) | H (ft) | Q (cfs) |
| ***C*** | **3.00** | | | | |
| ***L*** | **33.00** | ft | **0.0** | **0.0** | **79.0** |
| ***H*** | **0.86** | ft | | | |
| ***Q*** | | cfs | | | |

| *Total Q* | **83.89** |
|---|---|

Equation 12-20

$$Q = C_{BCW} L H^{1.5}$$

| *Sloping Broad-Crested Weir (Eqn 12-21 UDFCD)* | | | | | | |
|---|---|---|---|---|---|---|
| *Variable* | | | *Solve For* | | |
| | | | Z (ft) | H (ft) | Q (cfs) |
| ***C*** | **3.00** | | | | |
| ***Z*** | **3.00** | ft | **0.0** | **0.0** | **2.5** |
| ***H*** | **0.86** | ft | | | |
| ***Q*** | | cfs | | | |

Equation 12-21

$$Q = \left(\frac{2}{5}\right) C_{BCW} Z H^{2.5}$$

Where:

$Q$ = discharge (cfs)

$C_{BCW}$ = broad-crested weir coefficient (This ranges from 2.6 to 3.0. A value of 3.0 is often used in practice.) See Hydraulic Engineering Circular No. 22 for additional information.

$L$ = broad-crested weir length (ft)

$H$ = head above weir crest (ft)

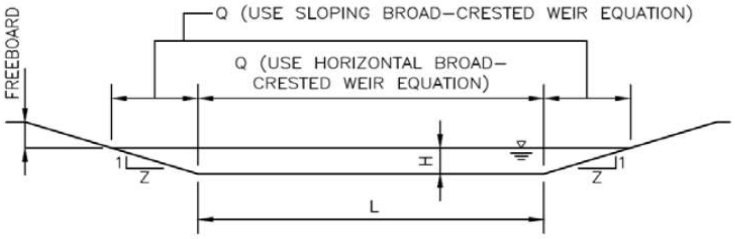



**Figure 12-20. Sloping broad-crest weir**

# DESIGN POINT 6 RIP RAP SIZING CHART

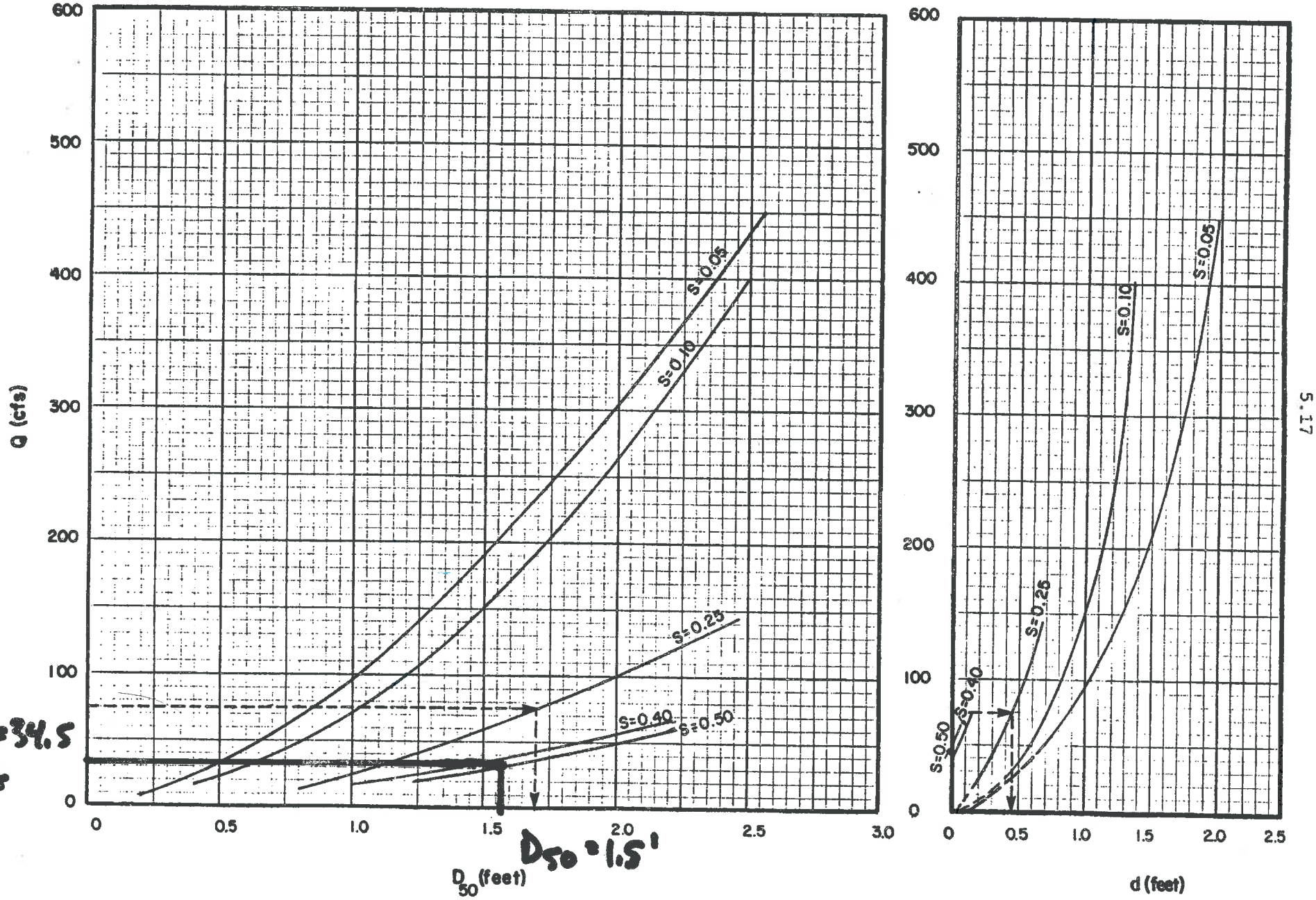



Figure 5.4. Steep slope riprap design, trapezoidal channels, 2:1 sideslopes, 6 ft base width.

CIVIL CONSULTANTS, INC.

212 N. Wahsatch Ave., Ste. 305
Colorado Springs, CO
719.955.5485

Project: **CROSSROADS MIXED USE FIL. NO. 1: POND STRUCTURES**

Date: **2/8/2022**

## Minimum Forebay Volume

- 3% of WQCV $= (0.03)(0.863\ ac\cdot ft)\left(43560\ \frac{ft^2}{ac}\right) = \frac{1127.77\ ft^3}{2\ forebays}$

$\approx$ **564 $ft^3$ / forebay (design)**

$(301\ ft^2)(2\ ft\ depth) = 602\ ft^3\ (Actual) > 564\ ft^3\ (Design)$ ∴ ✓

## Forebay Release and Configuration

- Release 2% of 100-yr Peak Discharge

$(0.02)(82.7\ cfs) = 1.65\ cfs\ (design) \leq 2.13\ cfs\ (actual)^*$ ∴ ✓

* See attached "Rectangular Contracted Weir Sheet" for actual flow determination

## Micropool Sizing

- 32.2 acres @ 78.7% imperviousness

Tributary Impervious Area $= (32.2\ acres)(78.7\%) = 25.3\ acres$

From Fig. 1 - Micropool Surface Area (SA) determination chart provided by City of Colorado Springs, [Design SA = 108 $ft^2$] < [Actual SA = 126 $ft^2$ SA] ∴ ✓

See Attached

1.) See attached "Open Channel Flow Calculator" sheet for low-flow channel capacity

# Rectangular Contracted Weir

This calculates the water flow rate over a rectangular contracted weir. This weir has a rectangular opening where the sides are straight up and down. A contracted weir means that the ditch or canal leading up to the weir is wider than the weir opening itself. The water before the weir should be held in a relatively calm and smooth pool. There should be air (not trapped) underneath the water leaving the weir. The Length is the bottom width of the weir. The height is measured from the bottom of the weir opening to the top of the water level ponded behind the weir (not the water level right as it leaves the weir). Learn more about the units used on this page.

Length:

3 in

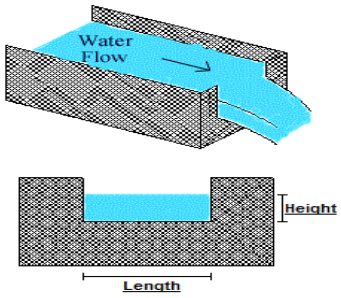
Height:

2 ft

Calculate

Flow Rate:

2.13 cfs




* Note: 1 point = 1/100 ft.

## The Equation

The Equation used to determine the flow rate $(Q)$ of a Rectangular Contracted Weir is:

$$Q = 3.247 \cdot L \cdot H^{1.48} - \frac{0.566 L^{1.9}}{1 + 2 \cdot L^{1.87}} \cdot H^{1.9}$$

Where:

$Q$ = Flow Rate in cfs.

$L$ = Bottom width of the weir in feet.

$H$ = Height of the upstream water above the weir crest in feet.

WSU Prosser – IAREC, 24106 N Bunn Rd, Prosser WA 99350-8694, 509-786-2226. Contact Us

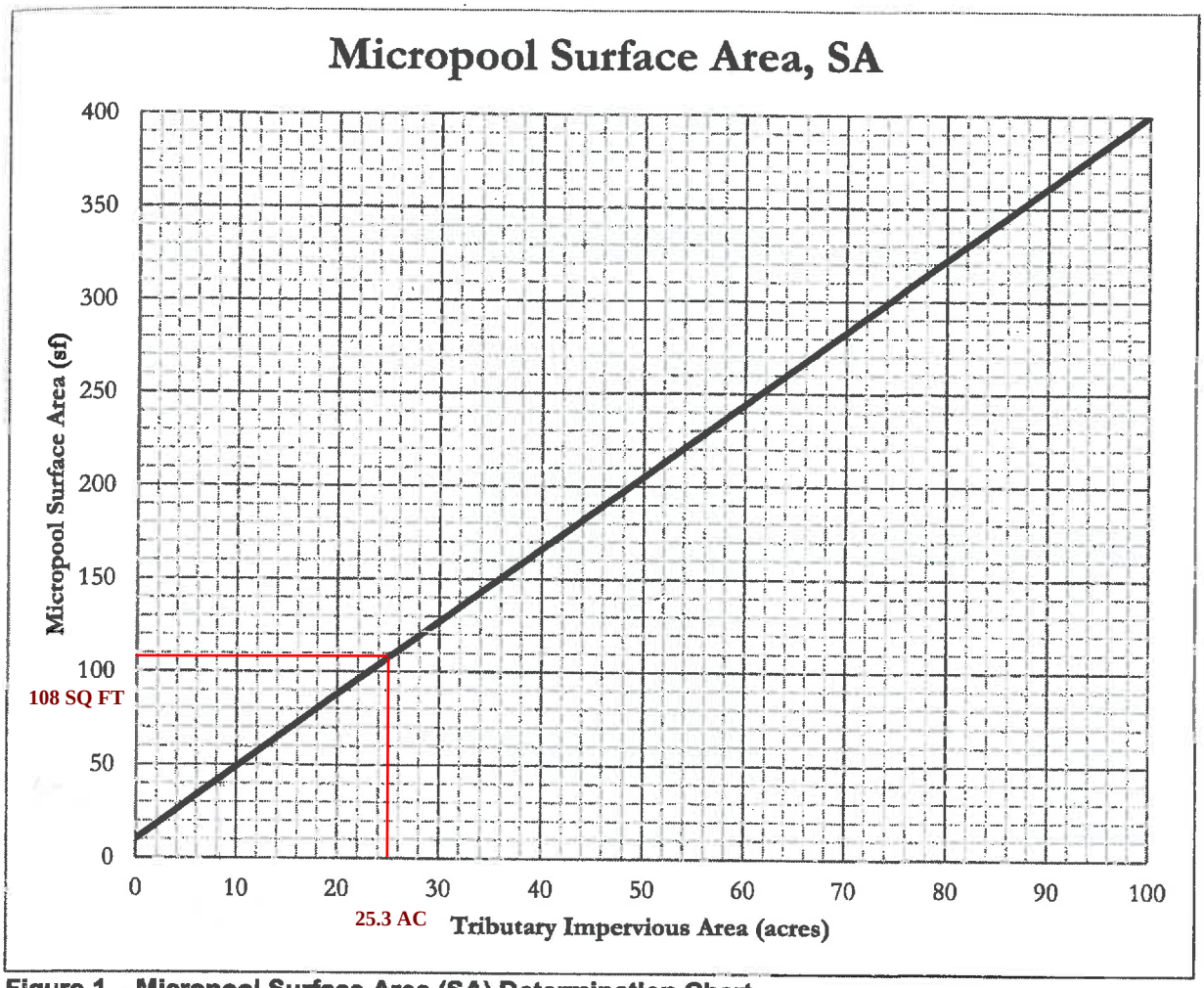



**Figure 1 – Micropool Surface Area (SA) Determination Chart**

The tributary impervious area is the effective number of impervious acres that will be treated by the extended detention basin (EDB). It is calculated by multiplying the tributary area to be treated by the impervious fraction of that area.

$$TIA = I \times A$$

**POND 1**
**78.7%x32.1AC= 25.3 AC**

*TIA* = Tributary impervious area (acres)
*I* = Imperviousness (fraction)
*A* = Tributary catchment area upstream (acres)

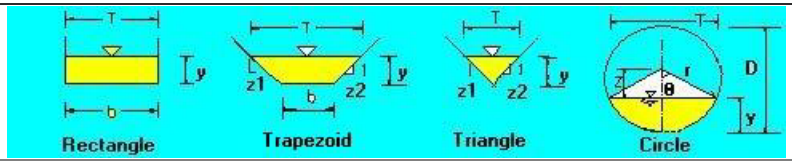
For EDBs with tributary impervious areas greater than 100 acres, the micropool surface area is 400 sf. The initial surcharge depth (ISD) is defined as the depth of the initial surcharge volume (ISV). The surface area determined using Figure 1 assumes an ISD of 4 inches. The initial surcharge volume is thus calculated by multiplying the micropool surface area by 4 inches.

$$ISV = SA \times 4\ inches$$

*ISV* = Initial surcharge volume (cf)
*SA* = Surface area (from Figure 1, sf)

# TRICKLE CHANNEL CAPACITY

Select Channel Type: Trapezoid




Velocity(V)&Discharge(Q)

Select unit system: Feet(ft)

| | | |
|---|---|---|
| Channel slope: .005 ft/ft | Water depth(y): .5 ft | Bottom width(b) 4 ft |
| Flow velocity 4.388 ft/s | LeftSlope (Z1): 0 to 1 (H:V) | RightSlope (Z2): 0 to 1 (H:V) |
| Flow discharge 8.776 ft^3/s | Input n value .013 or select n | |
| Calculate! | Status: Calculation finished | Reset |
| Wetted perimeter 5 ft | Flow area 2 ft^2 | Top width(T) 4 ft |
| Specific energy 0.8 ft | Froude number 1.09 | Flow status Supercritical flow |
| Critical depth 0.53 ft | Critical slope 0.0041 ft/ft | Velocity head 0.3 ft |

CIVIL CONSULTANTS, INC.

212 N. Wahsatch Ave., Ste. 305
Colorado Springs, CO
719.955.5485

Project: **CROSSROADS MIXED USE FIL. NO. 1: RIP RAP SIZING**

Date: **2/8/2022**

## Pond 1 Outfall: Rip Rap Sizing

*Based on outfall pipe condition, flow is subcritical ($F_r = 0.89$)

1. $\frac{Q}{D_c^{2.5}} = \frac{11.4\,cfs}{(1.5ft)^{2.5}} = 4.14 \leq 6.0$ ∴ Use Figure 9-38 (see attached) for rip rap sizing

2. $\frac{Y_t}{D_c} = \frac{0.58'}{1.5'} = 0.39$

From Figure 9-38, Type L Rip Rap Shall Be Used

## Pond 2 Spillway: Rip Rap Sizing

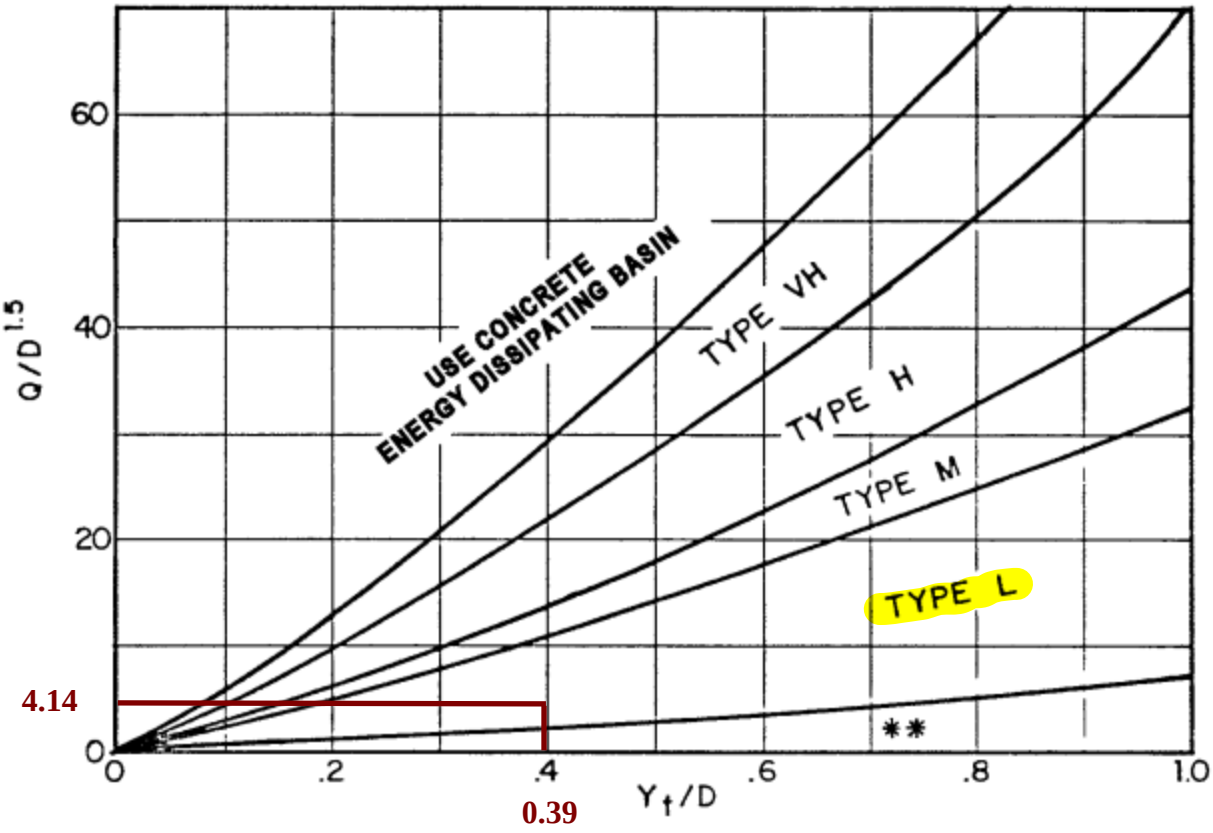
1. Unit Discharge (cfs/ft) = $\frac{82.7\,cfs}{33ft} = 2.51\,cfs/ft$

2. Longitudinal Slope = $\frac{1'}{5.5'} = 18.1\%$ ← Used steepest portion of slope

From Figure 12-21, Type M Rip Rap Shall Be Used

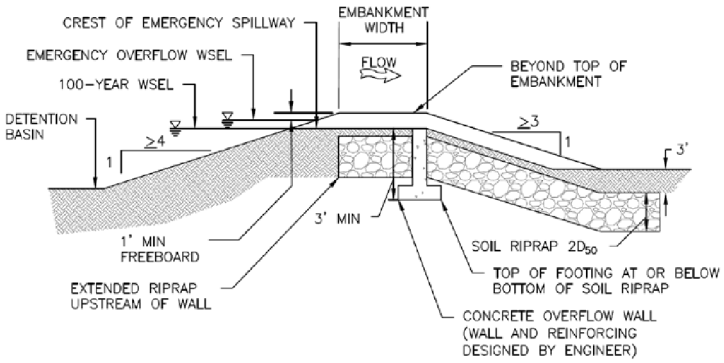
**(SEE ATTACHED FIGURE)**

**POND 1 OUTFALL: RIP RAP SIZING**

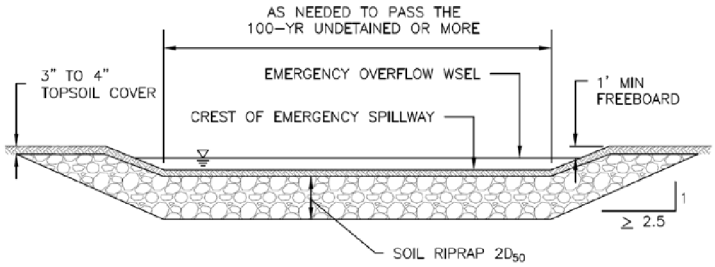



Use $D_a$ instead of D whenever flow is supercritical in the barrel.
** Use Type L for a distance of 3D downstream.

**Figure 9-38. Riprap erosion protection at circular conduit outlet (valid for Q/D2.5 ≤ 6.0)**

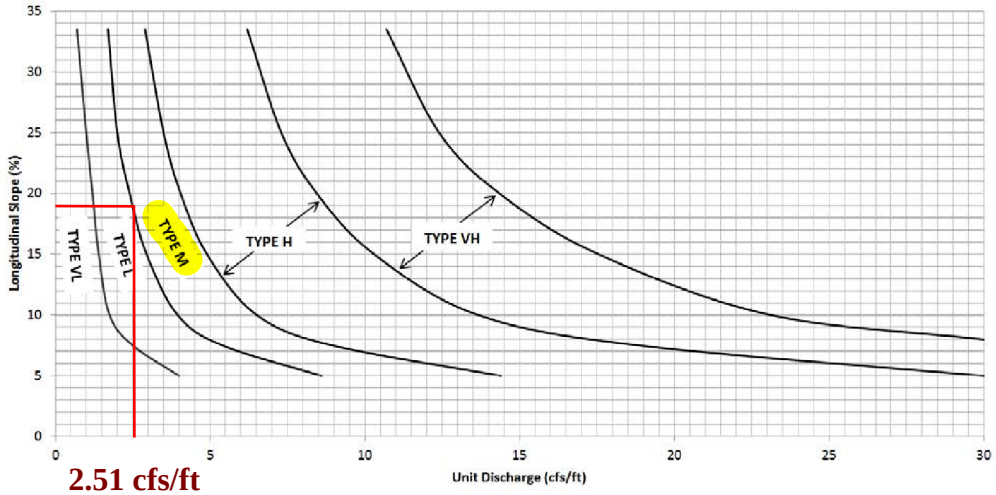



EMERGENCY SPILLWAY PROFILE




EMERGENCY SPILLWAY SECTION AND SPILLWAY CHANNEL

18%




2.51 cfs/ft

**Figure 12-21. Embankment protection details and rock sizing chart** (adapted from Arapahoe County)

SOIL RIPRAP NOTES:

1. ELEVATION TOLERANCES FOR THE SOIL RIPRAP SHALL BE 0.10 FEET. THICKNESS OF SOIL RIPRAP SHALL BE NO LESS THAN THICKNESS SHOWN AND NO MORE THAN 2–INCHES GREATER THAN THE THICKNESS SHOWN.

2. WHERE "SOIL RIPRAP" IS DESIGNATED ON THE CONTRACT DRAWINGS, RIPRAP VOIDS ARE TO BE FILLED WITH NATIVE SOIL. THE RIPRAP SHALL BE PRE–MIXED WITH THE NATIVE SOIL AT THE FOLLOWING PROPORTIONS BY VOLUME: 65PERCENT RIPRAP AND 35 PERCENT SOIL. THE SOIL USED FOR MIXING SHALL BE NATIVE TOPSOIL AND SHALL HAVE A MINIMUM FINES CONTENT OF 15 PERCENT. THE SOIL RIPRAP SHALL BE INSTALLED IN A MANNER THAT RESULTS IN A DENSE, INTERLOCKED LAYER OF RIPRAP WITH RIPRAP VOIDS FILLED COMPLETELY WITH SOIL. SEGREGATION OF MATERIALS SHALL BE AVOIDED AND IN NO CASE SHALL THE COMBINED MATERIAL CONSIST PRIMARILY OF SOIL; THE DENSITY AND INTERLOCKING NATURE OF RIPRAP IN THE MIXED MATERIAL SHALL ESSENTIALLY BE THE SAME AS IF THE RIPRAP WAS PLACED WITHOUT SOIL.

3. WHERE SPECIFIED (TYPICALLY AS "BURIED SOIL RIPRAP"), A SURFACE LAYER OF TOPSOIL SHALL BE PLACED OVER THE SOIL RIPRAP ACCORDING TO THE THICKNESS SPECIFIED ON THE CONTRACT DRAWINGS. THE TOPSOIL SURFACE LAYER SHALL BE COMPACTED TO APPROXIMATELY 85% OF MAXIMUM DENSITY AND WITHIN TWO PERCENTAGE POINTS OF OPTIMUM MOISTURE IN ACCORDANCE WITH ASTM D698. TOPSOIL SHALL BE ADDED TO ANY AREAS THAT SETTLE.

4. ALL SOIL RIPRAP THAT IS BURIED WITH TOPSOIL SHALL BE REVIEWED AND APPROVED BY THE ENGINEER PRIOR TO ANY TOPSOIL PLACEMENT.

| GRADATION FOR GRANULAR BEDDING | | |
|---|---|---|
| U.S. STANDARD SIEVE SIZE | PERCENT PASSING BY WEIGHT | |
| | TYPE I CDOT SECT. 703.01 | TYPE II CDOT SECT. 703.09 CLASS A |
| 3 INCHES | – | 90 – 100 |
| 1½ INCHES | – | – |
| ¾ INCHES | – | 20 – 90 |
| ⅜ INCHES | 100 | – |
| #4 | 95 – 100 | 0 – 20 |
| #16 | 45 – 80 | – |
| #50 | 10 – 30 | – |
| #100 | 2 – 10 | – |
| #200 | 0 – 2 | 0 – 3 |

RIPRAP BEDDING

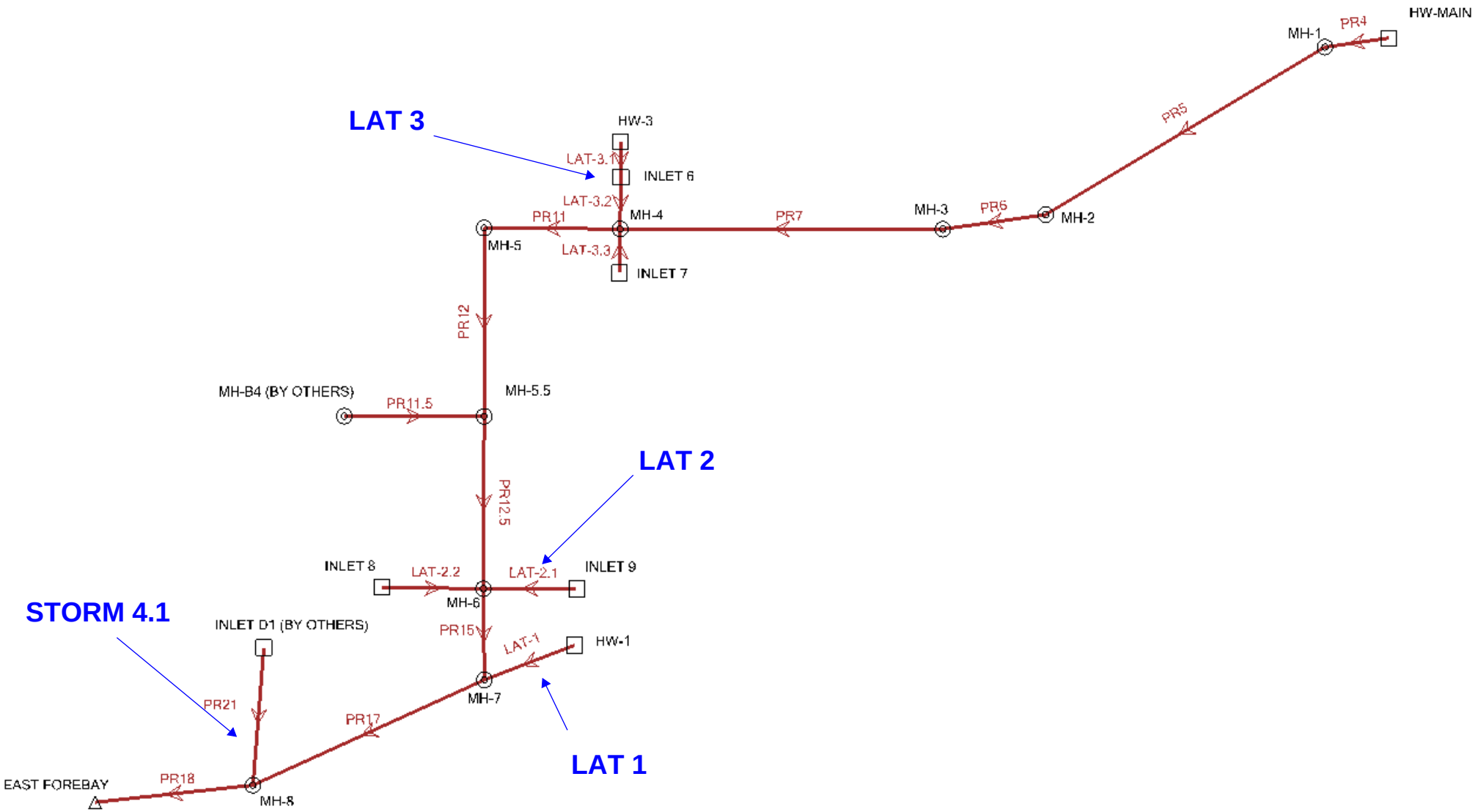
**Figure 8-34. Riprap and soil riprap placement and gradation (part 2 of 3)**

| THICKNESS REQUIREMENTS FOR GRANULAR BEDDING | | | |
|---|---|---|---|
| RIPRAP DESIGNATION | MINIMUM BEDDING THICKNESS (INCHES) | | |
| | FINE–GRAINED SOILS [1] | | COARSE–GRAINED SOILS [2] |
| | TYPE I (LOWER LAYER) | TYPE II (UPPER LAYER) | TYPE II |
| VL ($D_{50}$ = 6 IN) | 4 | 4 | 6 |
| L ($D_{50}$ = 9 IN) | 4 | 4 | 6 |
| M ($D_{50}$ = 12 IN) | 4 | 4 | 6 |
| H ($D_{50}$ = 18 IN) | 4 | 6 | 8 |
| VH ($D_{50}$ = 24 IN) | 4 | 6 | 8 |

NOTES:
1. MAY SUBSTITUTE ONE 12–INCH LAYER OF TYPE II BEDDING. THE SUBSTITUTION OF ONE LAYER OF TYPE II BEDDING SHALL NOT BE PERMITTED AT DROP STRUCTURES. THE USE OF A COMBINATION OF FILTER FABRIC AND TYPE II BEDDING AT DROP STRUCTURES IS ACCEPTABLE.
2. FIFTY PERCENT OR MORE BY WEIGHT RETAINED ON THE #40 SIEVE.

**Figure 8-34. Riprap and soil riprap placement and gradation (part 3 of 3)**

# STORM MAIN NETWORK LAYOUT

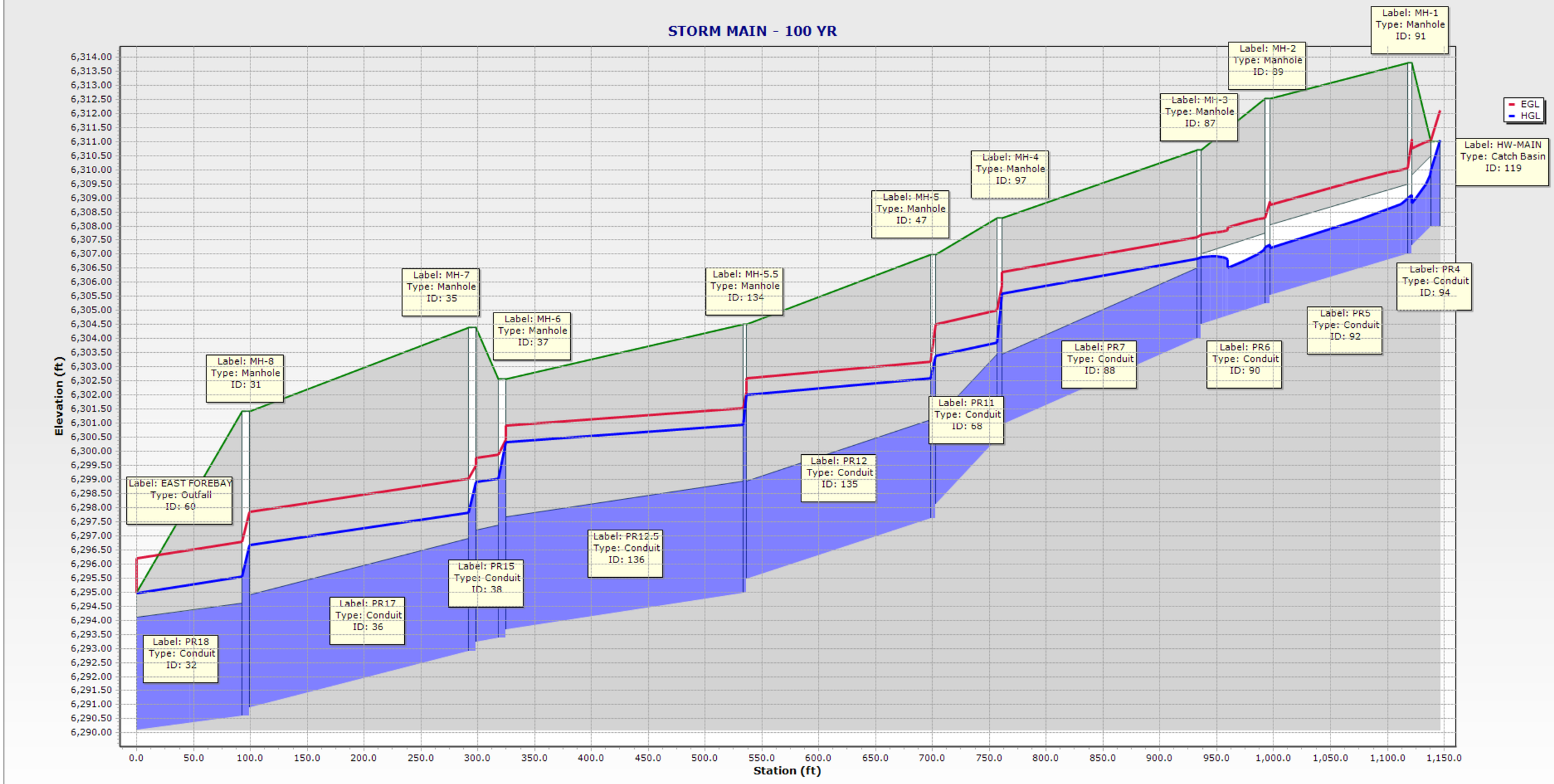


# STORM MAIN: 100 YR FLEXTABLE

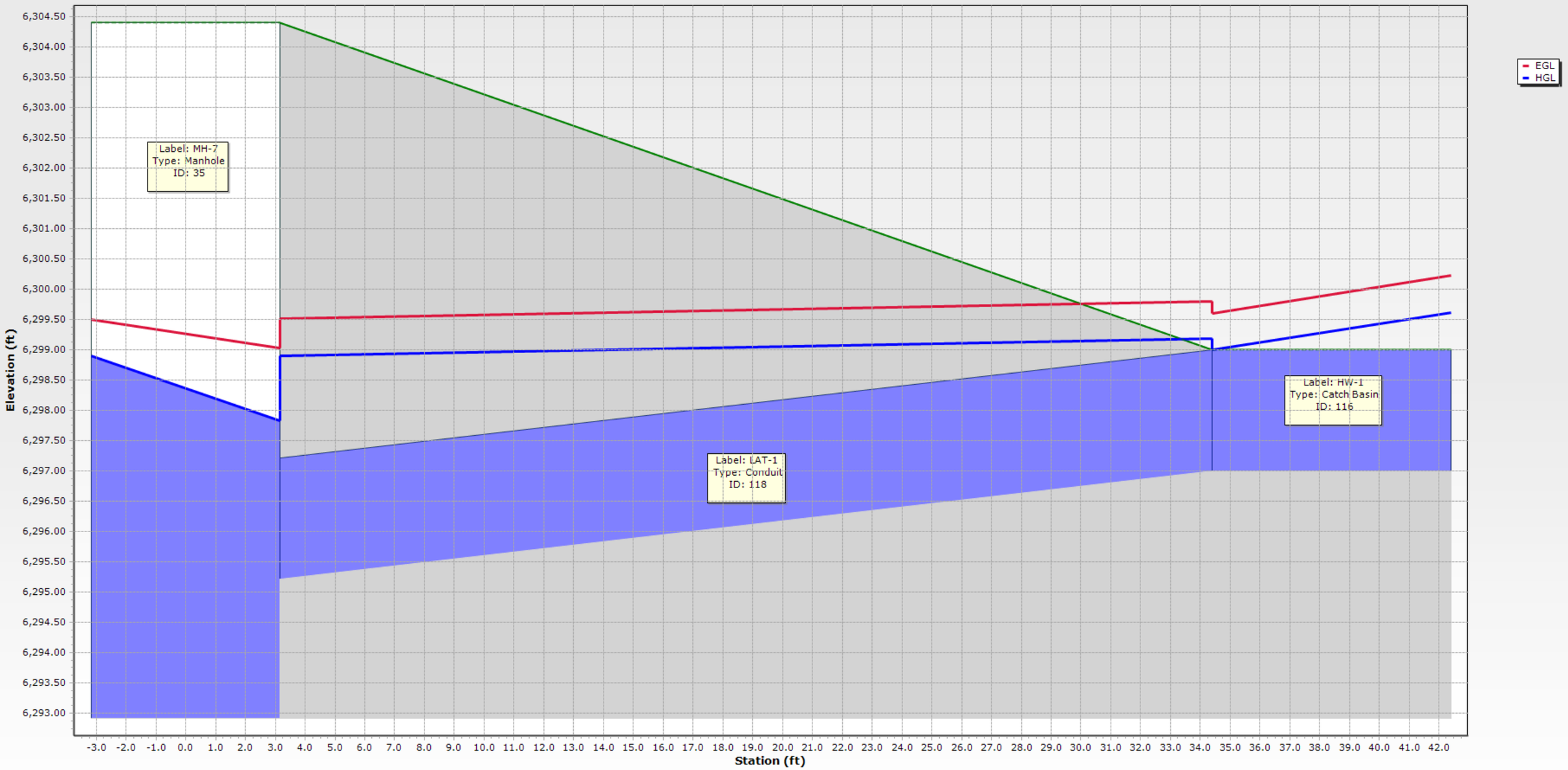
## FlexTable: Conduit Table

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| PR18 | 32 | MH-8 | 112.10 | 108.1 | 95.9 | 8.92 | 0.786 | (N/A) | 3.20 | 6,296.78 | 6,296.20 | 6,295.54 | 6,294.96 | 0.58 |
| PR17 | 36 | MH-7 | 110.10 | 76.3 | 199.1 | 8.76 | 1.474 | 2.62 | 3.17 | 6,299.02 | 6,297.85 | 6,297.83 | 6,296.66 | 1.17 |
| PR15 | 38 | MH-6 | 93.70 | 84.0 | 26.5 | 7.46 | 1.093 | 2.81 | 2.93 | 6,299.88 | 6,299.76 | 6,299.01 | 6,298.90 | 0.11 |
| LAT-2.1 | 46 | INLET 9 | 15.30 | 16.7 | 25.7 | 3.12 | 3.469 | 0.69 | 1.32 | 6,300.51 | 6,300.48 | 6,300.36 | 6,300.33 | 0.04 |
| PR11 | 68 | MH-4 | 60.50 | 45.3 | 58.4 | 8.56 | 3.102 | 1.42 | 2.51 | 6,305.00 | 6,304.51 | 6,303.86 | 6,303.38 | 0.48 |
| LAT-2.2 | 73 | INLET 8 | 3.80 | 16.2 | 15.7 | 2.15 | 3.181 | 0.41 | 0.75 | 6,300.42 | 6,300.40 | 6,300.35 | 6,300.33 | 0.02 |
| LAT-3.3 | 84 | INLET 7 | 7.70 | 15.6 | 2.5 | 4.36 | 6.691 | 0.40 | 1.08 | 6,305.90 | 6,305.88 | 6,305.60 | 6,305.59 | 0.01 |
| LAT-3.2 | 86 | INLET 6 | 21.40 | 41.0 | 27.8 | 4.36 | 1.932 | 1.11 | 1.57 | 6,305.96 | 6,305.88 | 6,305.66 | 6,305.59 | 0.08 |
| PR7 | 88 | MH-3 | 34.50 | 63.8 | 175.7 | 7.03 | 1.880 | 1.45 | 2.00 | 6,307.60 | 6,306.35 | 6,306.83 | 6,305.59 | 1.24 |
| PR6 | 90 | MH-2 | 34.50 | 75.3 | 60.1 | 10.25 | 1.522 | 1.62 | 2.00 | 6,308.29 | 6,307.69 | 6,307.25 | 6,306.91 | 0.34 |
| PR5 | 92 | MH-1 | 34.50 | 78.2 | 125.3 | 9.95 | 1.446 | 1.66 | 2.00 | 6,310.04 | 6,308.74 | 6,309.00 | 6,307.22 | 1.77 |
| PR4 | 94 | HW-MAIN | 34.50 | 47.7 | 22.5 | 14.56 | 2.635 | 1.22 | 2.00 | 6,311.04 | 6,310.76 | 6,310.00 | 6,308.80 | 1.20 |
| LAT-3.1 | 104 | HW-3 | 17.00 | 20.2 | 15.1 | 5.41 | 5.583 | 0.61 | 1.49 | 6,306.50 | 6,306.42 | 6,306.05 | 6,305.96 | 0.09 |
| LAT-1 | 118 | HW-1 | 19.60 | 40.1 | 38.4 | 6.24 | 3.161 | 0.88 | 1.59 | 6,299.79 | 6,299.51 | 6,299.19 | 6,298.90 | 0.29 |
| PR21 | 129 | INLET D1 (BY OTHERS) | 4.20 | 5.4 | 87.6 | 8.49 | 2.873 | 0.39 | 0.67 | 6,296.68 | 6,296.67 | 6,296.60 | 6,296.66 | -0.05 |
| PR12 | 135 | MH-5 | 60.00 | 52.2 | 165.6 | 6.24 | 1.786 | 1.80 | 2.43 | 6,303.18 | 6,302.59 | 6,302.58 | 6,301.99 | 0.59 |
| PR12.5 | 136 | MH-5.5 | 77.40 | 69.5 | 213.2 | 6.16 | 1.171 | 2.46 | 2.66 | 6,301.54 | 6,300.92 | 6,300.95 | 6,300.33 | 0.62 |
| PR11.5 | 138 | MH-B4 (BY OTHERS) | 13.80 | 47.7 | 110.3 | 2.81 | 1.055 | 1.22 | 1.25 | 6,302.24 | 6,302.11 | 6,302.11 | 6,301.99 | 0.13 |

| Upstream Structure Hydraulic Grade Line (In) (ft) | Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description |
|---|---|---|---|---|---|---|---|---|
| 6,296.66 | 8.76 | 0.900 | 1.11 | 6,295.00 | 6,301.41 | 6,290.10 | 6,290.60 | Circle - 48.0 in |
| 6,298.90 | 6.24 | 0.900 | 1.07 | 6,301.41 | 6,304.40 | 6,290.90 | 6,292.91 | Circle - 48.0 in |
| 6,300.33 | 2.15 | 1.520 | 1.31 | 6,304.40 | 6,302.57 | 6,293.21 | 6,293.37 | Circle - 48.0 in |
| 6,300.59 | 3.12 | 1.500 | 0.23 | 6,302.57 | 6,302.59 | 6,294.87 | 6,296.15 | Circle - 30.0 in |
| 6,305.59 | 4.36 | 1.520 | 1.73 | 6,306.99 | 6,308.27 | 6,298.11 | 6,300.45 | Circle - 36.0 in |
| 6,300.46 | 2.15 | 1.500 | 0.11 | 6,302.57 | 6,302.59 | 6,295.87 | 6,296.65 | Circle - 18.0 in |
| 6,306.04 | 4.36 | 1.500 | 0.44 | 6,308.27 | 6,307.05 | 6,301.95 | 6,302.50 | Circle - 18.0 in |
| 6,305.96 | 5.41 | 1.020 | 0.30 | 6,308.27 | 6,307.77 | 6,300.95 | 6,301.40 | Circle - 30.0 in |
| 6,306.91 | 7.11 | 0.100 | 0.08 | 6,308.27 | 6,310.70 | 6,300.95 | 6,304.00 | Circle - 30.0 in |
| 6,307.35 | 9.89 | 0.100 | 0.10 | 6,310.70 | 6,312.53 | 6,304.50 | 6,305.25 | Circle - 30.0 in |
| 6,309.10 | 11.25 | 0.100 | 0.10 | 6,312.53 | 6,313.79 | 6,305.55 | 6,307.00 | Circle - 30.0 in |
| 6,311.06 | 8.21 | 1.020 | 1.07 | 6,313.79 | 6,311.00 | 6,307.30 | 6,308.00 | Circle - 30.0 in |
| 6,306.46 | 5.41 | 1.020 | 0.46 | 6,307.77 | 6,306.00 | 6,301.90 | 6,304.00 | Circle - 24.0 in |
| 6,299.62 | 6.24 | 1.020 | 0.62 | 6,304.40 | 6,299.00 | 6,295.21 | 6,297.00 | Circle - 24.0 in |
| 6,296.72 | 2.28 | 1.500 | 0.12 | 6,301.41 | 6,301.20 | 6,292.40 | 6,295.60 | Circle - 30.0 in |
| 6,303.38 | 8.56 | 1.320 | 0.80 | 6,306.99 | 6,304.52 | 6,297.61 | 6,295.45 | Circle - 42.0 in |
| 6,301.99 | 2.81 | 1.770 | 1.04 | 6,304.52 | 6,302.57 | 6,294.95 | 6,293.67 | Circle - 48.0 in |
| 6,302.18 | 2.81 | 0.500 | 0.06 | 6,304.52 | 6,303.80 | 6,296.45 | 6,297.00 | Circle - 24.0 in |


STORM MAIN - 100 YR
Elevation (ft)
Station (ft)
EGL
HGL
Label: MH-1
Type: Manhole
ID: 91
Label: MH-2
Type: Manhole
ID: 89
Label: MH-3
Type: Manhole
ID: 87
Label: MH-4
Type: Manhole
ID: 97
Label: MH-5
Type: Manhole
ID: 47
Label: MH-5.5
Type: Manhole
ID: 134
Label: MH-6
Type: Manhole
ID: 37
Label: MH-7
Type: Manhole
ID: 35
Label: MH-8
Type: Manhole
ID: 31
Label: HW-MAIN
Type: Catch Basin
ID: 119
Label: PR4
Type: Conduit
ID: 94
Label: PR5
Type: Conduit
ID: 92
Label: PR6
Type: Conduit
ID: 90
Label: PR7
Type: Conduit
ID: 88
Label: PR11
Type: Conduit
ID: 68
Label: PR12
Type: Conduit
ID: 135
Label: PR12.5
Type: Conduit
ID: 136
Label: PR15
Type: Conduit
ID: 38
Label: PR17
Type: Conduit
ID: 36
Label: PR18
Type: Conduit
ID: 32
Label: EAST FOREBAY
Type: Outfall
ID: 60

## LAT 1 - 100 YR


EGL
HGL
Label: MH-7
Type: Manhole
ID: 35
Label: LAT-1
Type: Conduit
ID: 118
Label: HW-1
Type: Catch Basin
ID: 116
Elevation (ft)
6,304.50
6,304.00
6,303.50
6,303.00
6,302.50
6,302.00
6,301.50
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6,300.00
6,299.50
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6,295.00
6,294.50
6,294.00
6,293.50
6,293.00
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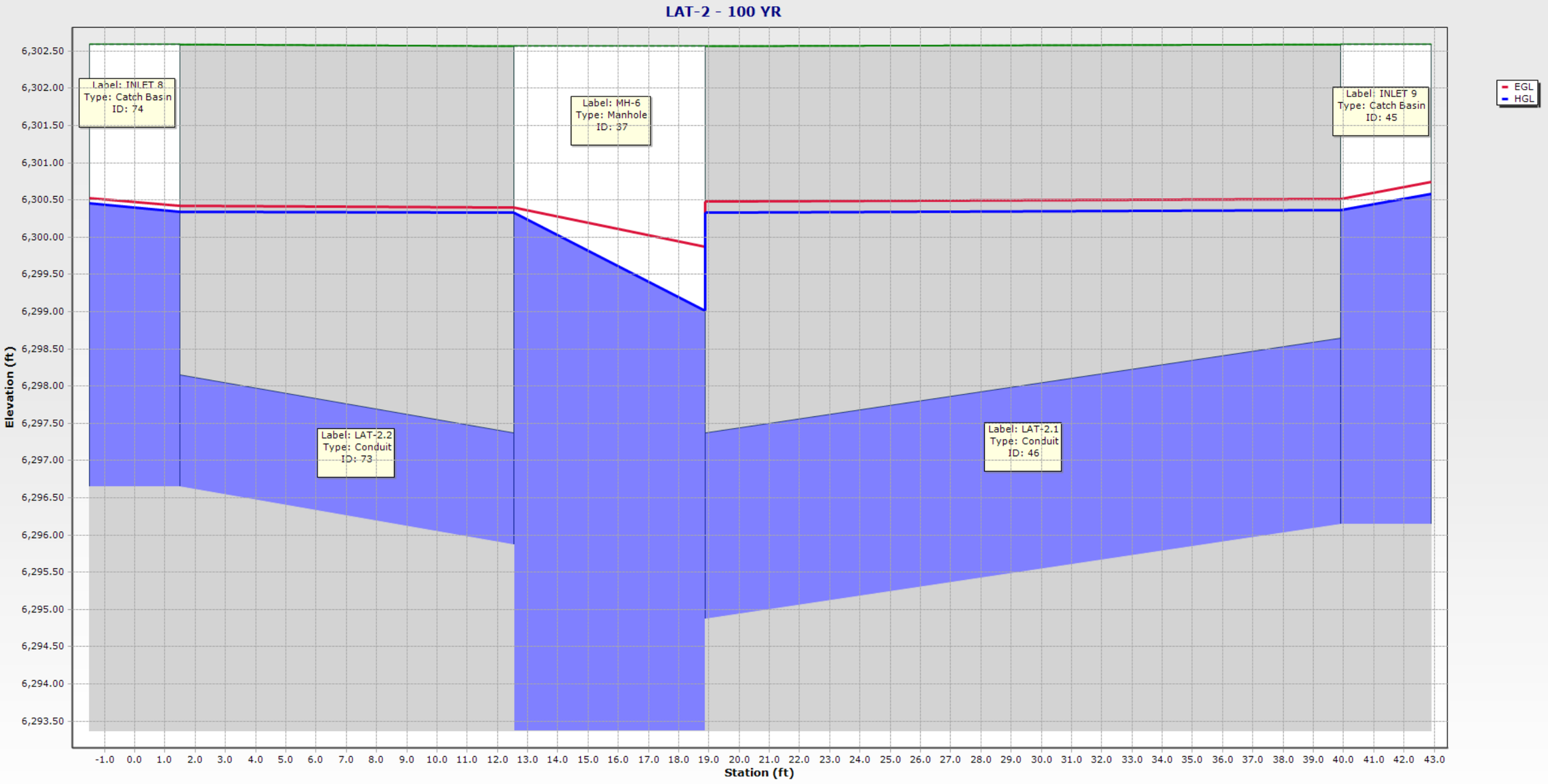
Station (ft)


LAT-2 - 100 YR
Label: INLET 8
Type: Catch Basin
ID: 74
Label: MH-6
Type: Manhole
ID: 37
Label: INLET 9
Type: Catch Basin
ID: 45
Label: LAT-2.2
Type: Conduit
ID: 73
Label: LAT-2.1
Type: Conduit
ID: 46
EGL
HGL
Elevation (ft)
6,302.50
6,302.00
6,301.50
6,301.00
6,300.50
6,300.00
6,299.50
6,299.00
6,298.50
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6,297.00
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6,296.00
6,295.50
6,295.00
6,294.50
6,294.00
6,293.50
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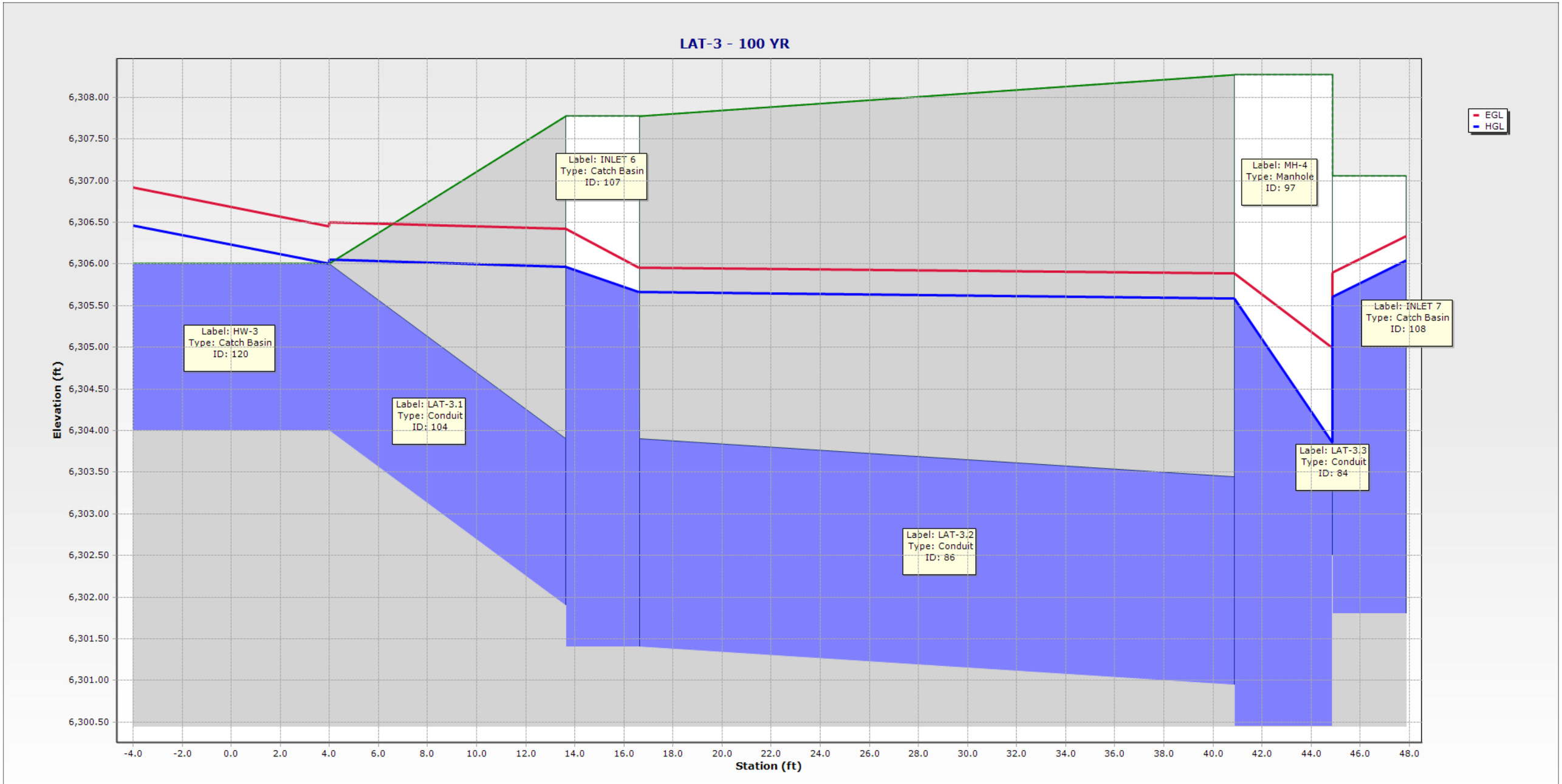
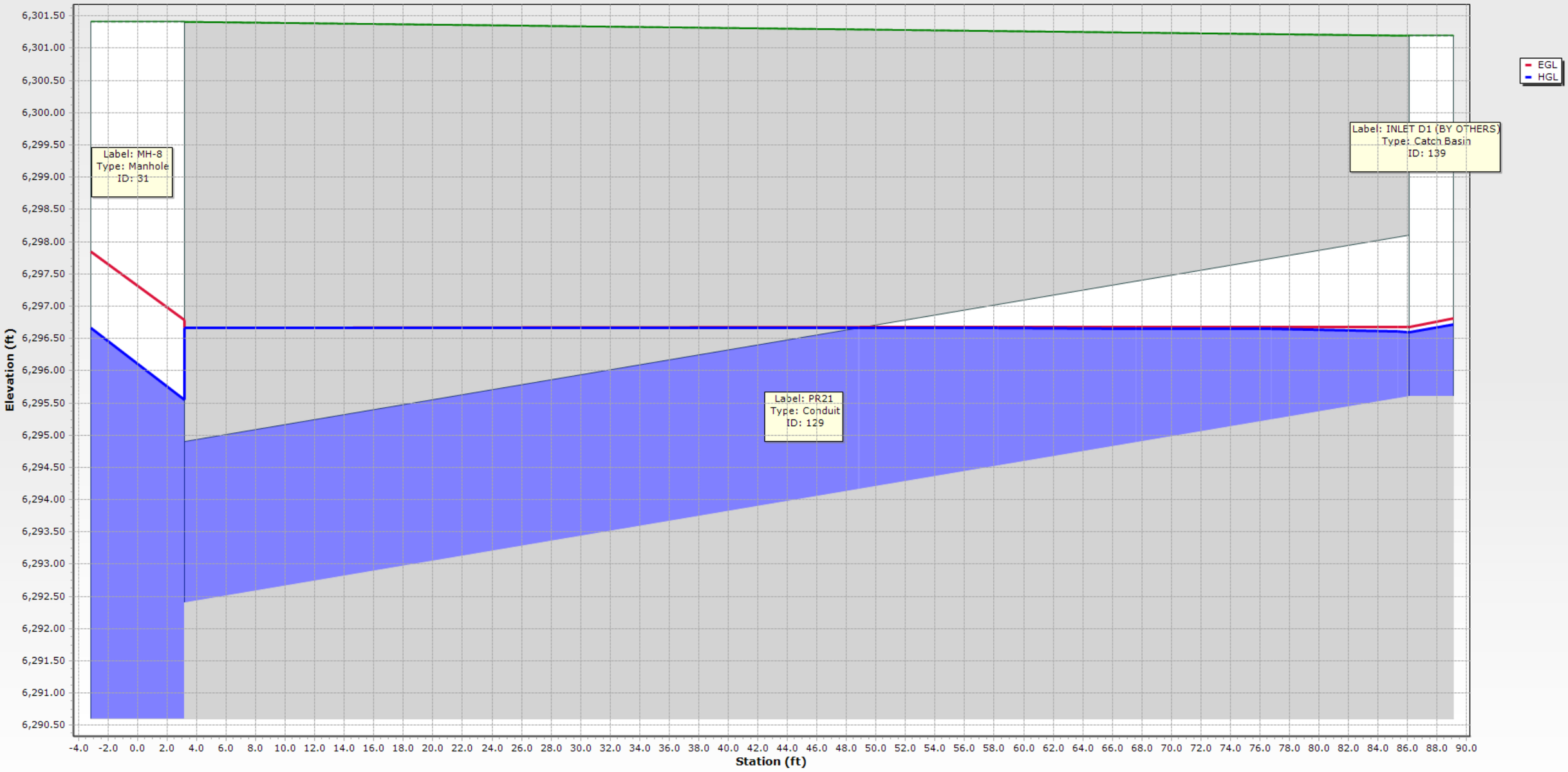
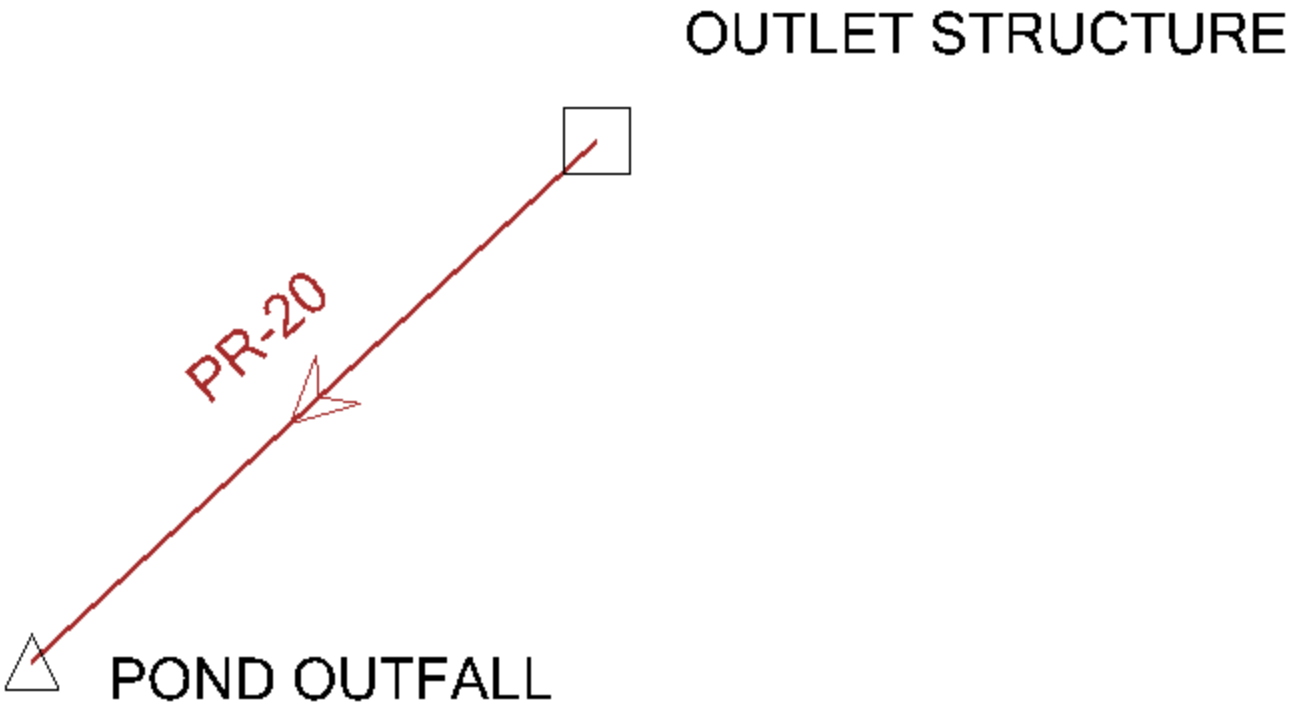
Station (ft)


LAT-3 - 100 YR
EGL
HGL
Label: HW-3
Type: Catch Basin
ID: 120
Label: LAT-3.1
Type: Conduit
ID: 104
Label: INLET 6
Type: Catch Basin
ID: 107
Label: LAT-3.2
Type: Conduit
ID: 86
Label: MH-4
Type: Manhole
ID: 97
Label: LAT-3.3
Type: Conduit
ID: 84
Label: INLET 7
Type: Catch Basin
ID: 108
Elevation (ft)
6,308.00
6,307.50
6,307.00
6,306.50
6,306.00
6,305.50
6,305.00
6,304.50
6,304.00
6,303.50
6,303.00
6,302.50
6,302.00
6,301.50
6,301.00
6,300.50
-4.0
-2.0
0.0
2.0
4.0
6.0
8.0
10.0
12.0
14.0
16.0
18.0
20.0
22.0
24.0
26.0
28.0
30.0
32.0
34.0
36.0
38.0
40.0
42.0
44.0
46.0
48.0
Station (ft)

## STORM 4.1 - 100 YR



# STORM 1 LAYOUT

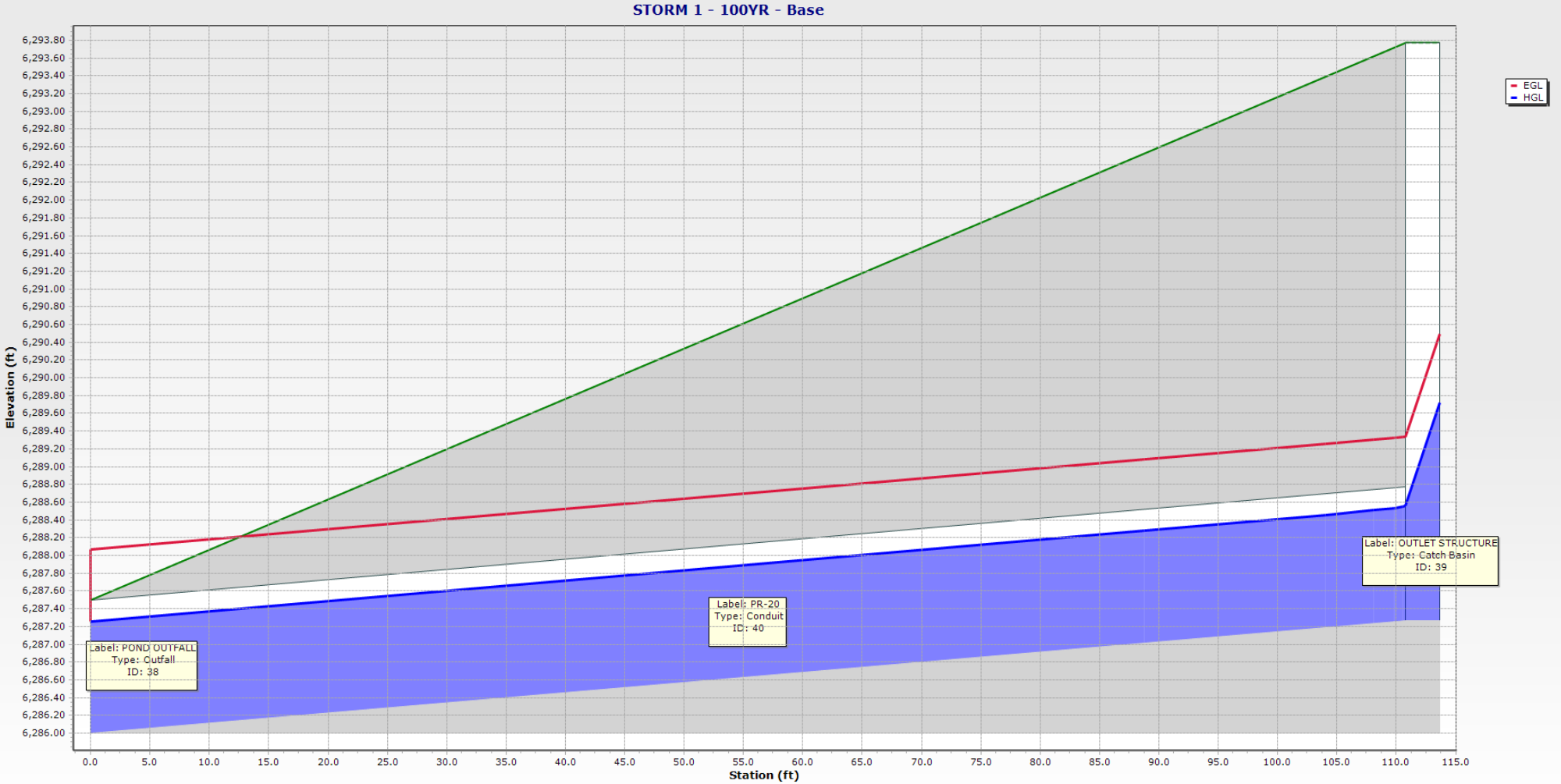


# STORM 1 - FLEXTABLE

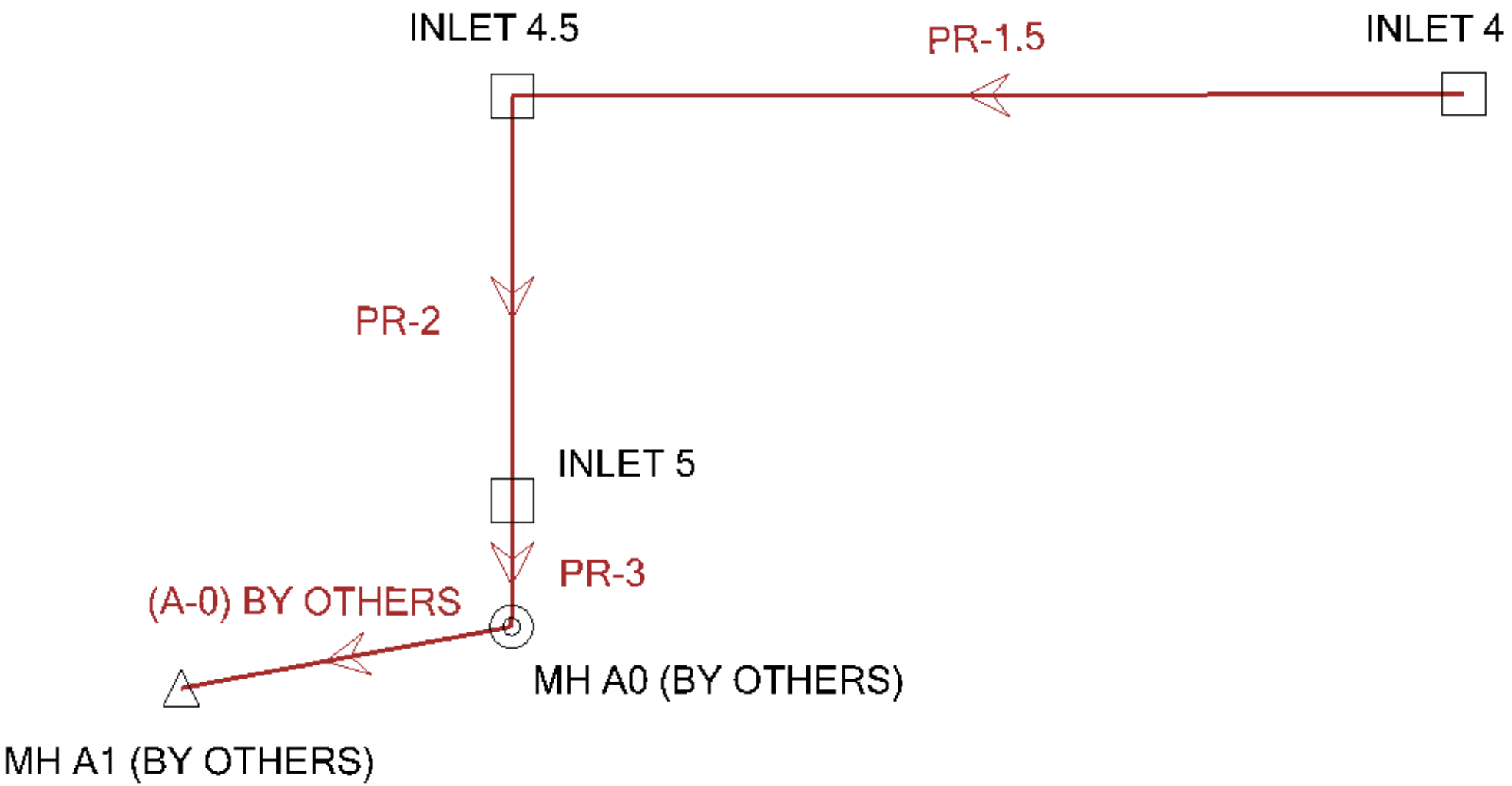
## FlexTable: Conduit Table

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| PR-20 | 40 | OUTLET STRUCTURE | 11.40 | 102.0 | 112.2 | 7.20 | 1.059 | 1.26 | 1.29 | 6,289.33 | 6,288.06 | 6,288.56 | 6,287.26 | 1.30 |

| Upstream Structure Hydraulic Grade Line (In) (ft) | Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description |
|---|---|---|---|---|---|---|---|---|
| 6,289.72 | 7.06 | 1.500 | 1.16 | 6,287.50 | 6,293.77 | 6,286.00 | 6,287.27 | Circle - 18.0 in |


STORM 1 - 100YR - Base
EGL
HGL
Elevation (ft)
6,293.80
6,293.60
6,293.40
6,293.20
6,293.00
6,292.80
6,292.60
6,292.40
6,292.20
6,292.00
6,291.80
6,291.60
6,291.40
6,291.20
6,291.00
6,290.80
6,290.60
6,290.40
6,290.20
6,290.00
6,289.80
6,289.60
6,289.40
6,289.20
6,289.00
6,288.80
6,288.60
6,288.40
6,288.20
6,288.00
6,287.80
6,287.60
6,287.40
6,287.20
6,287.00
6,286.80
6,286.60
6,286.40
6,286.20
6,286.00
0.0
5.0
10.0
15.0
20.0
25.0
30.0
35.0
40.0
45.0
50.0
55.0
60.0
65.0
70.0
75.0
80.0
85.0
90.0
95.0
100.0
105.0
110.0
115.0
Station (ft)
Label: POND OUTFALL
Type: Outfall
ID: 38
Label: PR-20
Type: Conduit
ID: 40
Label: OUTLET STRUCTURE
Type: Catch Basin
ID: 39

# STORM 2 NETWORK LAYOUT

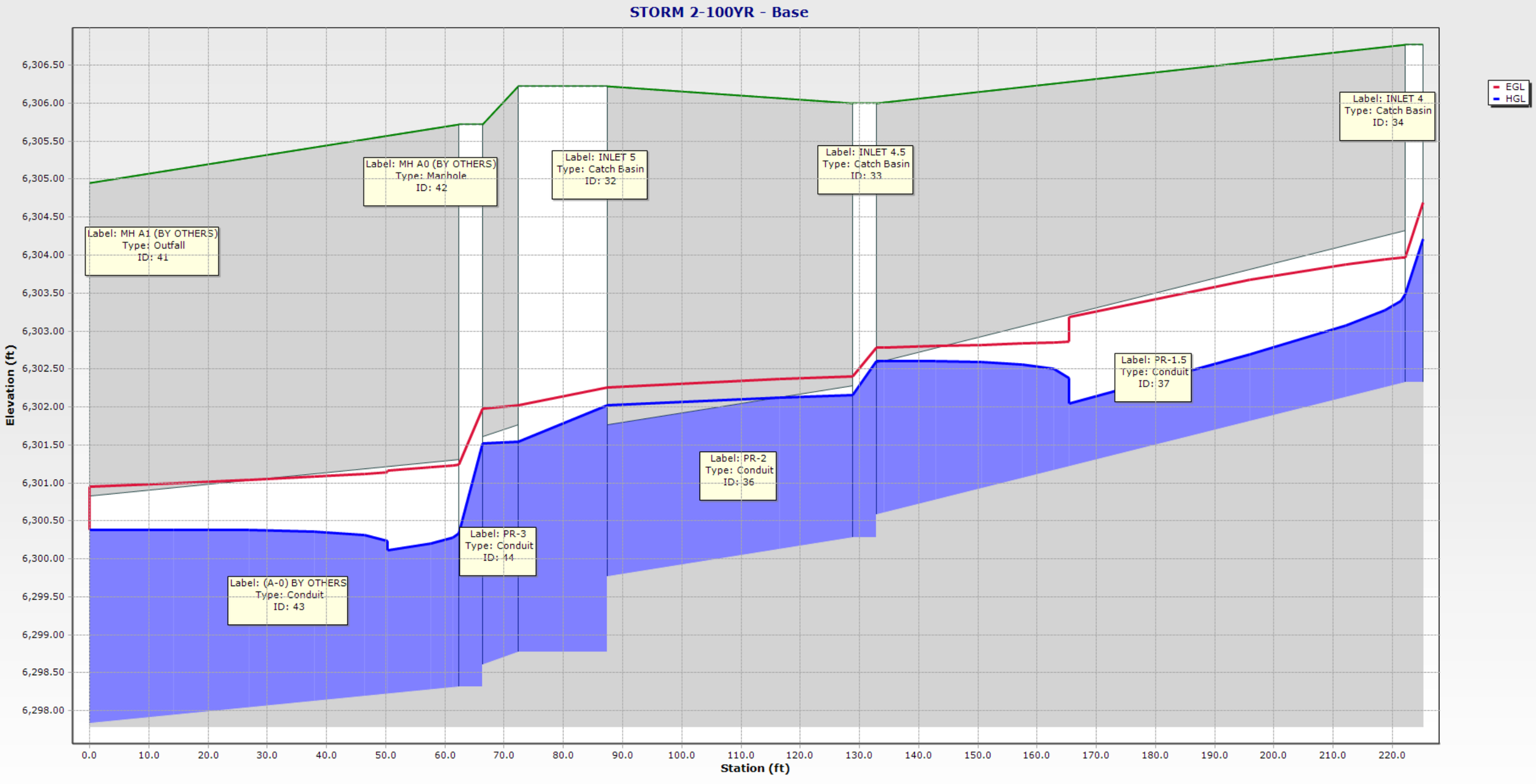


# STORM 2: 100 YR FLEXTABLE

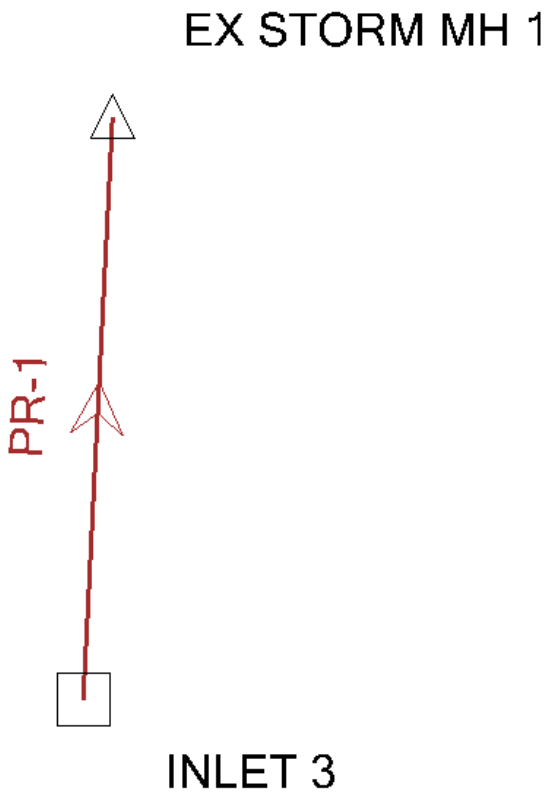
## FlexTable: Conduit Table

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| PR-2 | 36 | INLET 4.5 | 12.30 | 54.3 | 51.0 | 7.35 | 1.417 | 1.05 | 1.26 | 6,302.41 | 6,302.26 | 6,302.16 | 6,302.02 | 0.14 |
| PR-1.5 | 37 | INLET 4 | 10.60 | 34.2 | 92.8 | 8.93 | 2.025 | 0.81 | 1.17 | 6,303.97 | 6,302.78 | 6,303.49 | 6,302.60 | 0.89 |
| (A-0) BY OTHERS | 43 | MH A0 (BY OTHERS) | 38.65 | 67.1 | 64.3 | 8.74 | 1.256 | 1.80 | 2.02 | 6,301.24 | 6,300.95 | 6,300.33 | 6,300.38 | 0.05 |
| PR-3 | 44 | INLET 5 | 37.90 | 56.1 | 15.6 | 9.83 | 1.526 | 1.61 | 2.00 | 6,302.02 | 6,301.98 | 6,301.54 | 6,301.52 | 0.02 |

| Upstream Structure Hydraulic Grade Line (In) (ft) | Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description |
|---|---|---|---|---|---|---|---|---|
| 6,302.60 | 3.37 | 1.770 | 0.44 | 6,306.22 | 6,306.00 | 6,299.77 | 6,300.28 | Circle - 24.0 in |
| 6,304.21 | 5.57 | 1.500 | 0.72 | 6,306.00 | 6,306.77 | 6,300.58 | 6,302.32 | Circle - 24.0 in |
| 6,301.52 | 5.41 | 1.320 | 1.19 | 6,304.95 | 6,305.72 | 6,297.83 | 6,298.31 | Circle - 36.0 in |
| 6,302.02 | 3.92 | 1.000 | 0.48 | 6,305.72 | 6,306.22 | 6,298.61 | 6,298.77 | Circle - 36.0 in |


STORM 2-100YR - Base
Elevation (ft)
Station (ft)
6,306.50
6,306.00
6,305.50
6,305.00
6,304.50
6,304.00
6,303.50
6,303.00
6,302.50
6,302.00
6,301.50
6,301.00
6,300.50
6,300.00
6,299.50
6,299.00
6,298.50
6,298.00
0.0
10.0
20.0
30.0
40.0
50.0
60.0
70.0
80.0
90.0
100.0
110.0
120.0
130.0
140.0
150.0
160.0
170.0
180.0
190.0
200.0
210.0
220.0
EGL
HGL
Label: MH A1 (BY OTHERS)
Type: Outfall
ID: 41
Label: MH A0 (BY OTHERS)
Type: Manhole
ID: 42
Label: INLET 5
Type: Catch Basin
ID: 32
Label: INLET 4.5
Type: Catch Basin
ID: 33
Label: INLET 4
Type: Catch Basin
ID: 34
Label: PR-1.5
Type: Conduit
ID: 37
Label: PR-2
Type: Conduit
ID: 36
Label: PR-3
Type: Conduit
ID: 44
Label: (A-0) BY OTHERS
Type: Conduit
ID: 43

# STORM 3 NETWORK LAYOUT

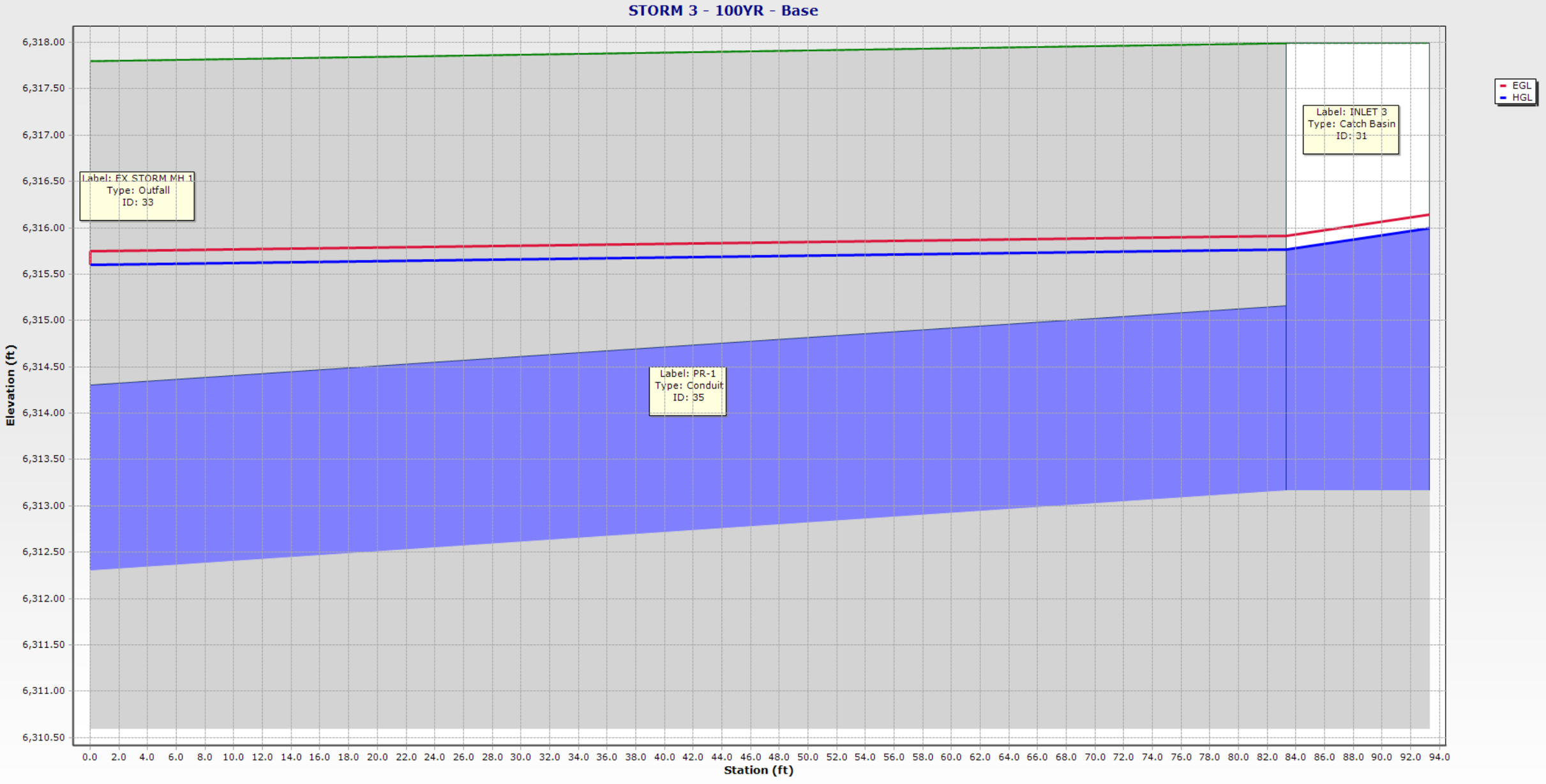


# STORM 3: 100 YR FLEXTABLE

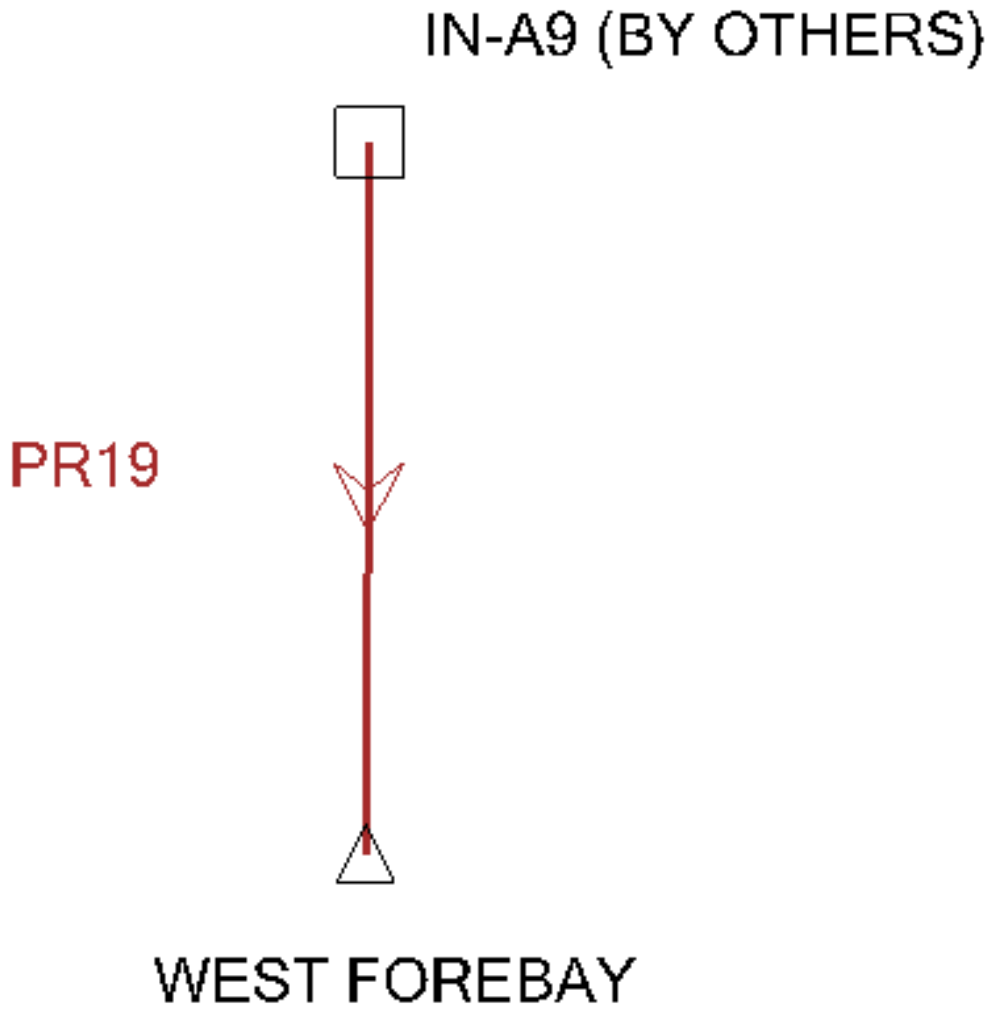
## FlexTable: Conduit Table

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| PR-1 | 35 | INLET 3 | 9.80 | 43.9 | 88.3 | 3.12 | 1.434 | 0.93 | 1.12 | 6,315.92 | 6,315.75 | 6,315.77 | 6,315.60 | 0.17 |

| Upstream Structure Hydraulic Grade Line (In) (ft) | Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description |
|---|---|---|---|---|---|---|---|---|
| 6,315.99 | 3.12 | 1.500 | 0.23 | 6,317.99 | 6,317.80 | 6,313.16 | 6,312.30 | Circle - 24.0 in |


STORM 3 - 100YR - Base
Label: EX STORM MH 1
Type: Outfall
ID: 33
Label: INLET 3
Type: Catch Basin
ID: 31
Label: PR-1
Type: Conduit
ID: 35
EGL
HGL
Elevation (ft)
6,318.00
6,317.50
6,317.00
6,316.50
6,316.00
6,315.50
6,315.00
6,314.50
6,314.00
6,313.50
6,313.00
6,312.50
6,312.00
6,311.50
6,311.00
6,310.50
0.0 2.0 4.0 6.0 8.0 10.0 12.0 14.0 16.0 18.0 20.0 22.0 24.0 26.0 28.0 30.0 32.0 34.0 36.0 38.0 40.0 42.0 44.0 46.0 48.0 50.0 52.0 54.0 56.0 58.0 60.0 62.0 64.0 66.0 68.0 70.0 72.0 74.0 76.0 78.0 80.0 82.0 84.0 86.0 88.0 90.0 92.0 94.0
Station (ft)

# STORM 5 NETWORK LAYOUT

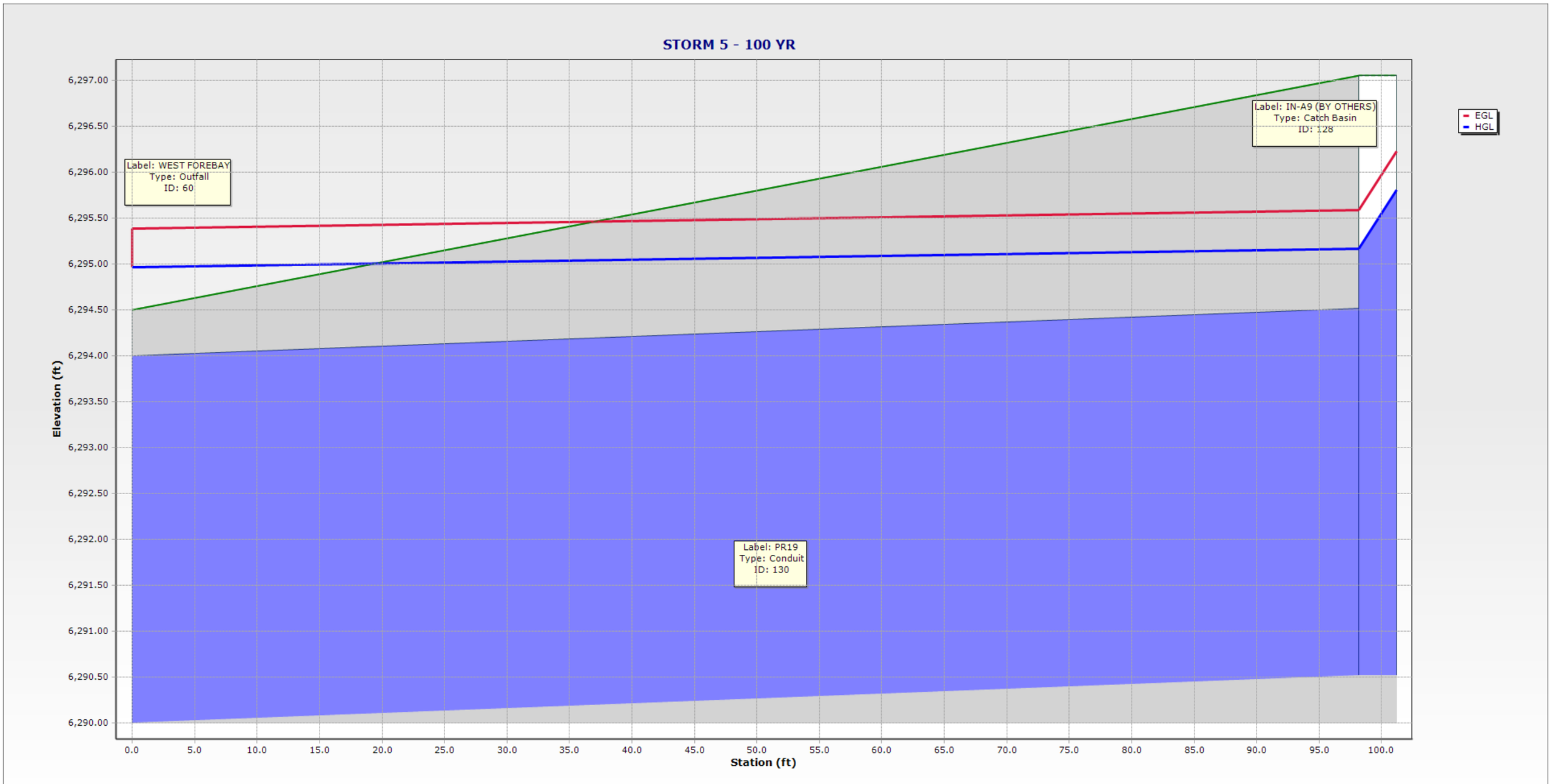


# STORM 5: 100 YR FLEXTABLE

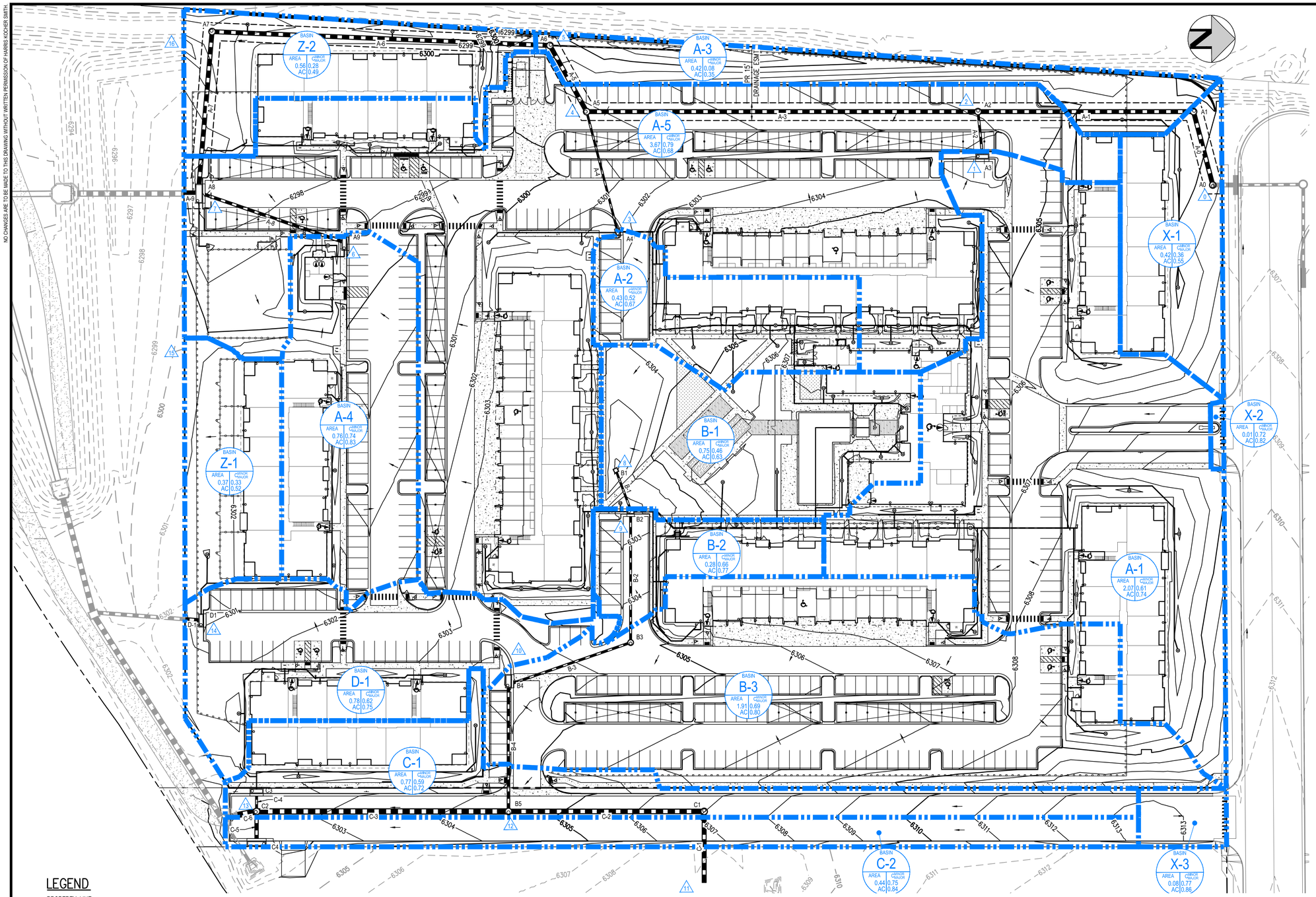
## FlexTable: Conduit Table

| Label | ID | Upstream Structure | Flow (cfs) | Flow / Capacity (Design) (%) | Length (Unified) (ft) | Velocity (ft/s) | Froude Number (Normal) | Depth (Normal) (ft) | Depth (Critical) (ft) | Energy Grade Line (In) (ft) | Energy Grade Line (Out) (ft) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Headloss (ft) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| PR19 | 130 | IN-A9 (BY OTHERS) | 65.50 | 63.1 | 99.7 | 5.21 | 1.117 | 2.31 | 2.44 | 6,295.59 | 6,295.38 | 6,295.17 | 6,294.96 | 0.21 |

| Upstream Structure Hydraulic Grade Line (In) (ft) | Upstream Structure Velocity (In-Governing) (ft/s) | Upstream Structure Headloss Coefficient | Upstream Structure Headloss (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) | Invert (Start) (ft) | Invert (Stop) (ft) | Conduit Description |
|---|---|---|---|---|---|---|---|---|
| 6,295.80 | 5.21 | 1.500 | 0.63 | 6,294.50 | 6,297.05 | 6,290.00 | 6,290.52 | Circle - 48.0 in |


STORM 5 - 100 YR
Label: WEST FOREBAY
Type: Outfall
ID: 60
Label: IN-A9 (BY OTHERS)
Type: Catch Basin
ID: 128
Label: PR19
Type: Conduit
ID: 130
EGL
HGL
Elevation (ft)
Station (ft)
6,297.00
6,296.50
6,296.00
6,295.50
6,295.00
6,294.50
6,294.00
6,293.50
6,293.00
6,292.50
6,292.00
6,291.50
6,291.00
6,290.50
6,290.00
0.0
5.0
10.0
15.0
20.0
25.0
30.0
35.0
40.0
45.0
50.0
55.0
60.0
65.0
70.0
75.0
80.0
85.0
90.0
95.0
100.0

# BACKGROUND




| STRUCTURE TABLE | |
|---|---|
| STRUCTURE ID | DESCRIPTION |
| A0 | TYPE I MANHOLE |
| A1 | TYPE I MANHOLE |
| A2 | TYPE I MANHOLE |
| A3 | INLET TYPE R 10' |
| A4 | INLET TYPE R 5' |
| A5 | TYPE I MANHOLE |
| A6 | TYPE I MANHOLE |
| A7 | TYPE I MANHOLE |
| A8 | INLET TYPE R 15' MOD |
| A9 | INLET TYPE R 10' |
| B1 | INLET TYPE C |

| STRUCTURE TABLE | |
|---|---|
| STRUCTURE ID | DESCRIPTION |
| B2 | INLET TYPE R 5' |
| B3 | TYPE II MANHOLE |
| B4 | INLET TYPE R 10' |
| B5 | TYPE I MANHOLE |
| C1 | TYPE I MANHOLE |
| C2 | TYPE I MANHOLE |
| C3 | INLET TYPE R 10' |
| C4 | INLET TYPE R 15' |
| D1 | INLET TYPE R 10' |

| PIPE TABLE | | | | | | |
|---|---|---|---|---|---|---|
| NAME | UPSTREAM STRUCTURE | DOWNSTREAM STRUCTURE | SIZE | LENGTH | SLOPE | MATERIAL |
| A-0 | A0 | A1 | 36" | 64.26' | 0.50% | RCP |
| A-1 | A1 | A2 | 36" | 182.49' | 0.50% | RCP |
| A-2 | A3 | A2 | 18" | 36.82' | 4.50% | RCP |
| A-3 | A2 | A5 | 36" | 331.29' | 0.50% | RCP |
| A-4 | A4 | A5 | 15" | 102.94' | 1.50% | RCP |
| A-5 | A5 | A6 | 36" | 64.06' | 2.03% | RCP |
| A-6 | A6 | A7 | 36" | 286.28' | 0.58% | RCP |
| A-7 | A7 | A8 | 36" | 130.14' | 0.50% | RCP |
| A-8 | A9 | A8 | 24" | 125.80' | 1.00% | RCP |
| A-9 | A8 | | 48" | 10.05' | 0.50% | RCP |
| B-1 | B1 | | 15" | 35.69' | 0.50% | RCP |
| B-2 | B2 | B3 | 18" | 109.40' | 0.50% | RCP |
| B-3 | B3 | B4 | 18" | 107.86' | 0.50% | RCP |
| B-4 | B4 | B5 | 24" | 110.33' | 0.50% | RCP |
| C-1 | | C1 | 36" | 60.28' | 2.00% | RCP |
| C-2 | C1 | B5 | 42" | 165.64' | 1.30% | RCP |
| C-3 | B5 | C2 | 48" | 213.24' | 0.60% | RCP |
| C-4 | C3 | C2 | 18" | 15.67' | 5.00% | RCP |
| C-5 | C4 | C2 | 30" | 25.67' | 5.00% | RCP |
| C-6 | C2 | | 48" | 16.52' | 0.60% | RCP |

| DESIGN POINT SUMMARY | | |
|---|---|---|
| DESIGN POINT | Q5 (CFS) | Q100 (CFS) |
| 0 | 16.48 | 38.58 |
| 1 | 4.61 | 9.36 |
| 2 | 19.17 | 43.45 |
| 3 | 0.88 | 1.90 |
| 4 | 19.60 | 44.42 |
| 5 | 19.55 | 45.20 |
| 6 | 2.76 | 5.24 |
| 7 | 30.16 | 65.43 |
| 8 | 1.38 | 3.17 |
| 9 | 2.06 | 4.57 |
| 10 | 6.85 | 13.80 |
| 11 | 35.00 | 60.50 |
| 12 | 41.65 | 73.75 |
| 13 | 44.47 | 79.25 |
| 14 | 2.08 | 4.20 |
| 15 | 0.47 | 1.27 |
| 16 | 0.57 | 1.43 |

| DIRECT RUNOFF SUMMARY | | | |
|---|---|---|---|
| SUBBASIN | AREA (AC) | Q5 (CFS) | Q100 (CFS) |
| X-1 | 0.42 | 0.58 | 1.50 |
| X-2 | 0.01 | 0.05 | 0.10 |
| X-3 | 0.08 | 0.26 | 0.50 |
| A-1 | 2.07 | 4.61 | 9.36 |
| A-2 | 0.43 | 0.88 | 1.90 |
| A-3 | 0.42 | 0.13 | 0.94 |
| A-4 | 0.76 | 2.76 | 5.24 |
| A-5 | 3.67 | 8.72 | 17.06 |
| B-1 | 0.75 | 1.38 | 3.17 |
| B-2 | 0.28 | 0.74 | 1.45 |
| B-3 | 1.91 | 4.89 | 9.52 |
| C-1 | 0.77 | 1.86 | 3.84 |
| C-2 | 0.44 | 1.39 | 2.64 |
| D-1 | 0.78 | 2.08 | 4.20 |
| Z-1 | 0.37 | 0.47 | 1.27 |
| Z-2 | 0.38 | 0.57 | 1.43 |

Know what's below. Call before you dig.

40 0 40 80
SCALE: 1" = 40'

DESIGNED BY: EEM
CHECKED BY: JDO
DRAWN BY: EEM

| ISSUE DATE: 08-06-2021 | |
|---|---|
| DATE | REVISION COMMENTS |
| 10-29-2021 | PER COUNTY COMMENTS |
| 01-13-2022 | PER COUNTY COMMENTS |

HARRIS KOCHER SMITH
1120 Lincoln Street, Suite 1000
Denver, Colorado 80203
P: 303.623.6300 F: 303.623.6311
HarrisKocherSmith.com

TRINSIC ACQUISITION COMPANY, LLC

# AURA AT CROSSROADS
## DRAINAGE PLAN

PROJECT #: 200823
SHEET NUMBER
1
1 OF 1

FILEPATH: K:\200823\ENGINEERING\DRAINAGE\DRAINAGE PLAN.DWG LAYOUT: LAYOUT1 (2)
5 XREFs: a-layout, p-base, p-base-hyot, p-legal, p-util-hyot
PLOTTED: TUE 04/05/22 5:38:38P BY: ETHAN MARKS

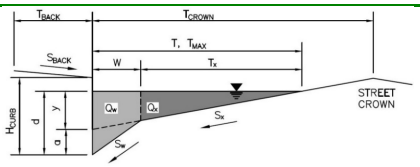
# Basin A-5 Overflow

*MHFD-Inlet, Version 5.01 (April 2021)*

## ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads
**Inlet ID:** A8




| Gutter Geometry: | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 15.0 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.150 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 30.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.010 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.020 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

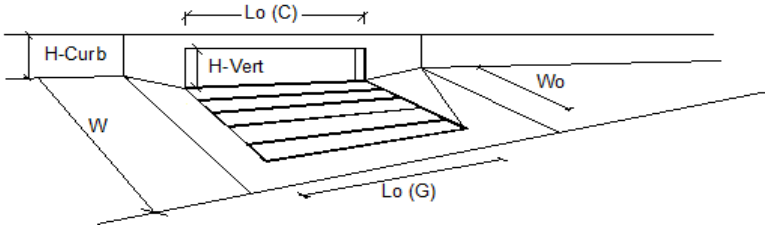
| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 30.0 | 30.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 6.0 | 6.0 | inches |
| Allow Flow Depth at Street Crown (check box for yes, leave blank for no) | | ☐ | ☐ | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| MINOR STORM Allowable Capacity is based on Spread Criterion | | | | |
| MAJOR STORM Allowable Capacity is based on Spread Criterion | **$Q_{allow}$ =** | **21.5** | **21.5** | **cfs** |

**Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**
**Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

## INLET ON A CONTINUOUS GRADE

*MHFD-Inlet, Version 5.01 (April 2021)*




| Design Information (Input) | | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet (CDOT Type R Curb Opening) | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a') | $a_{LOCAL}$ = | 3.0 | 3.0 | inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | No = | 1 | 1 | |
| Length of a Single Unit Inlet (Grate or Curb Opening) | $L_o$ = | 15.00 | 15.00 | ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | $W_o$ = | N/A | N/A | ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | $C_f$-G = | N/A | N/A | |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | $C_f$-C = | 0.10 | 0.10 | |
| **Street Hydraulics: OK - Q < Allowable Street Capacity'** | | MINOR | MAJOR | |
| Total Inlet Interception Capacity | **Q =** | **8.1** | **12.4** | **cfs** |
| Total Inlet Carry-Over Flow (flow bypassing inlet) | **$Q_b$ =** | **0.9** | **7.8** | **cfs** |
| Capture Percentage = $Q_a/Q_o$ = | **C% =** | **90** | **61** | **%** |

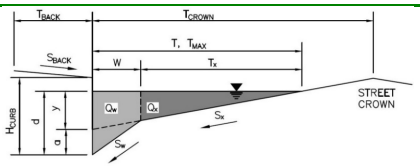
# Basin D-1 Overflow

*MHFD-Inlet, Version 5.01 (April 2021)*

## ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

**(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)**

**Project:** Crossroads
**Inlet ID:** D1




| Gutter Geometry: | | | |
|---|---|---|---|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK}$ = | 15.0 | ft |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK}$ = | 0.060 | ft/ft |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK}$ = | 0.020 | |
| Height of Curb at Gutter Flow Line | $H_{CURB}$ = | 6.00 | inches |
| Distance from Curb Face to Street Crown | $T_{CROWN}$ = | 30.0 | ft |
| Gutter Width | W = | 2.00 | ft |
| Street Transverse Slope | $S_X$ = | 0.010 | ft/ft |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_W$ = | 0.083 | ft/ft |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_O$ = | 0.020 | ft/ft |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET}$ = | 0.016 | |

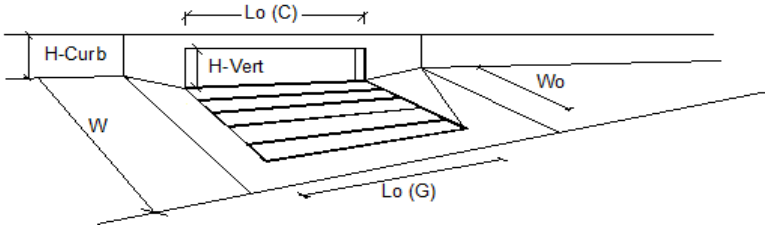
| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| Max. Allowable Spread for Minor & Major Storm | $T_{MAX}$ = | 30.0 | 30.0 | ft |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | $d_{MAX}$ = | 6.0 | 6.0 | inches |
| Allow Flow Depth at Street Crown (check box for yes, leave blank for no) | | ☐ | ☐ | |

| | | Minor Storm | Major Storm | |
|---|---|---|---|---|
| MINOR STORM Allowable Capacity is based on Spread Criterion | | | | |
| MAJOR STORM Allowable Capacity is based on Spread Criterion | **$Q_{allow}$ =** | **21.5** | **21.5** | **cfs** |

**Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**
**Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'**

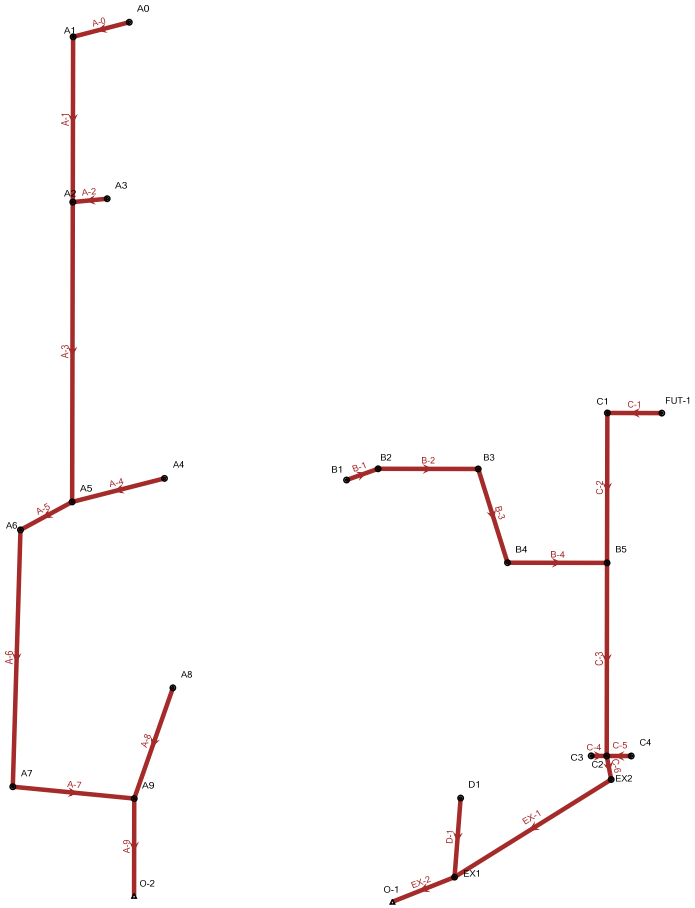
## INLET ON A CONTINUOUS GRADE

*MHFD-Inlet, Version 5.01 (April 2021)*




| Design Information (Input) | CDOT Type R Curb Opening | MINOR | MAJOR | |
|---|---|---|---|---|
| Type of Inlet | Type = | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a') | $a_{LOCAL}$ = | 3.0 | 3.0 | inches |
| Total Number of Units in the Inlet (Grate or Curb Opening) | No = | 1 | 1 | |
| Length of a Single Unit Inlet (Grate or Curb Opening) | $L_o$ = | 10.00 | 10.00 | ft |
| Width of a Unit Grate (cannot be greater than W, Gutter Width) | $W_o$ = | N/A | N/A | ft |
| Clogging Factor for a Single Unit Grate (typical min. value = 0.5) | $C_f$-G = | N/A | N/A | |
| Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1) | $C_f$-C = | 0.10 | 0.10 | |
| **Street Hydraulics: OK - Q < Allowable Street Capacity'** | | MINOR | MAJOR | |
| Total Inlet Interception Capacity | **Q =** | **2.4** | **5.3** | **cfs** |
| Total Inlet Carry-Over Flow (flow bypassing inlet) | **$Q_b$ =** | **0.0** | **1.5** | **cfs** |
| Capture Percentage = $Q_a/Q_o$ = | **C% =** | **100** | **78** | **%** |

# Aura at Crossroads
## Network Schematic



# Aura at Crossroads
## FlexTable: Conduit Table
### Active Scenario: 5 YR

| Label | Start Node | Stop Node | Length (Unified) (ft) | Rise (Unified) (ft) | Notes | Invert (Start) (ft) | Invert (Stop) (ft) | Slope (Calculated) (ft/ft) | System Known Flow (cfs) | Capacity (Full Flow) (cfs) | Velocity (ft/s) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| A-0 | A0 | A1 | 64.3 | 3.00 | 36" RCP | 6,298.31 | 6,297.99 | 0.005 | 22.65 | 47.06 | 6.59 | 6,299.84 | 6,299.46 | 6,305.84 | 6,305.38 |
| A-1 | A1 | A2 | 182.5 | 3.00 | 36" RCP | 6,297.79 | 6,296.88 | 0.005 | 22.65 | 47.10 | 6.60 | 6,299.32 | 6,298.35 | 6,305.38 | 6,304.10 |
| A-2 | A3 | A2 | 38.3 | 1.50 | 18" RCP | 6,299.84 | 6,298.18 | 0.043 | 4.61 | 21.88 | 9.81 | 6,300.66 | 6,298.67 | 6,302.56 | 6,304.10 |
| A-3 | A2 | A5 | 331.3 | 3.00 | 36" RCP | 6,296.68 | 6,295.04 | 0.005 | 24.63 | 46.92 | 6.72 | 6,298.28 | 6,296.58 | 6,304.10 | 6,300.87 |
| A-4 | A4 | A5 | 102.9 | 1.25 | 15" RCP | 6,298.13 | 6,296.59 | 0.015 | 0.88 | 7.90 | 4.25 | 6,298.50 | 6,296.87 | 6,302.89 | 6,300.87 |
| A-5 | A5 | A6 | 64.1 | 3.00 | 36" RCP | 6,294.84 | 6,293.54 | 0.020 | 24.95 | 94.98 | 11.32 | 6,296.45 | 6,294.66 | 6,300.87 | 6,300.83 |
| A-6 | A6 | A7 | 286.3 | 3.00 | 36" RCP | 6,293.34 | 6,291.67 | 0.006 | 24.86 | 50.94 | 7.16 | 6,294.95 | 6,293.72 | 6,300.83 | 6,297.03 |
| A-7 | A7 | A9 | 130.1 | 3.00 | 36" RCP | 6,291.47 | 6,290.82 | 0.005 | 24.86 | 47.14 | 6.76 | 6,293.69 | 6,293.61 | 6,297.03 | 6,297.91 |
| A-8 | A8 | A9 | 125.8 | 2.00 | 24" RCP | 6,293.78 | 6,292.52 | 0.010 | 2.76 | 22.64 | 4.88 | 6,294.36 | 6,293.51 | 6,299.57 | 6,297.91 |
| A-9 | A9 | O-2 | 104.4 | 4.00 | 48" RCP | 6,290.50 | 6,290.00 | 0.005 | 35.44 | 99.40 | 7.25 | 6,293.46 | 6,293.44 | 6,297.91 | 6,294.00 |
| B-1 | B1 | B2 | 35.7 | 1.25 | 15" RCP | 6,299.37 | 6,299.19 | 0.005 | 1.38 | 4.59 | 3.27 | 6,299.84 | 6,299.65 | 6,302.34 | 6,303.02 |
| B-2 | B2 | B3 | 109.4 | 1.50 | 18" RCP | 6,298.99 | 6,298.44 | 0.005 | 2.06 | 7.45 | 3.60 | 6,299.53 | 6,298.98 | 6,303.02 | 6,304.77 |
| B-3 | B3 | B4 | 107.9 | 1.50 | 18" RCP | 6,298.24 | 6,297.70 | 0.005 | 2.06 | 7.43 | 3.60 | 6,298.84 | 6,298.82 | 6,304.77 | 6,303.81 |
| B-4 | B4 | B5 | 110.3 | 2.00 | 24" RCP | 6,297.50 | 6,296.95 | 0.005 | 6.85 | 15.97 | 4.89 | 6,298.43 | 6,297.87 | 6,303.81 | 6,304.53 |
| C-1 | FUT-1 | C1 | 60.3 | 3.00 | 36" RCP | 6,300.45 | 6,298.11 | 0.039 | 35.00 | 131.40 | 15.73 | 6,302.37 | 6,299.29 | 6,308.24 | 6,307.03 |
| C-2 | C1 | B5 | 165.7 | 3.50 | 42" RCP | 6,297.61 | 6,295.45 | 0.013 | 35.00 | 114.88 | 10.48 | 6,299.44 | 6,296.78 | 6,307.03 | 6,304.53 |
| C-3 | B5 | C2 | 213.2 | 4.00 | 48" RCP | 6,294.95 | 6,293.67 | 0.006 | 41.65 | 111.28 | 8.22 | 6,296.88 | 6,295.37 | 6,304.53 | 6,302.67 |
| C-4 | C3 | C2 | 15.7 | 1.50 | 18" RCP | 6,296.65 | 6,295.87 | 0.050 | 1.86 | 23.41 | 7.92 | 6,297.16 | 6,296.18 | 6,299.96 | 6,302.67 |
| C-5 | C4 | C2 | 25.7 | 2.50 | 30" RCP | 6,296.15 | 6,294.87 | 0.050 | 3.60 | 91.53 | 9.05 | 6,296.77 | 6,295.23 | 6,300.16 | 6,302.67 |
| C-6 | C2 | EX2 | 26.5 | 4.00 | 48" RCP | 6,293.37 | 6,293.21 | 0.006 | 46.67 | 111.56 | 8.48 | 6,295.42 | 6,295.32 | 6,302.67 | 6,304.40 |
| D-1 | D1 | EX1 | 87.7 | 2.50 | 30" RCP | 6,295.49 | 6,292.40 | 0.035 | 2.08 | 77.00 | 6.80 | 6,295.96 | 6,293.49 | 6,301.22 | 6,298.00 |
| EX-1 | EX2 | EX1 | 204.3 | 4.00 | 48" RCP | 6,292.91 | 6,290.90 | 0.010 | 57.47 | 142.47 | 10.73 | 6,295.19 | 6,293.46 | 6,304.40 | 6,298.00 |
| EX-2 | EX1 | O-1 | 99.4 | 4.00 | 48" RCP | 6,290.60 | 6,290.00 | 0.006 | 59.55 | 111.59 | 9.03 | 6,293.44 | 6,293.44 | 6,298.00 | 6,295.00 |

# Aura at Crossroads
## FlexTable: Conduit Table
## Active Scenario: 100 YR

| Label | Start Node | Stop Node | Length (Unified) (ft) | Rise (Unified) (ft) | Notes | Invert (Start) (ft) | Invert (Stop) (ft) | Slope (Calculated) (ft/ft) | System Known Flow (cfs) | Capacity (Full Flow) (cfs) | Velocity (ft/s) | Hydraulic Grade Line (In) (ft) | Hydraulic Grade Line (Out) (ft) | Elevation Ground (Start) (ft) | Elevation Ground (Stop) (ft) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| A-0 | A0 | A1 | 64.3 | 3.00 | 36" RCP | 6,298.31 | 6,297.99 | 0.005 | 38.65 | 47.06 | 7.43 | 6,300.38 | 6,300.15 | 6,305.84 | 6,305.38 |
| A-1 | A1 | A2 | 182.5 | 3.00 | 36" RCP | 6,297.79 | 6,296.88 | 0.005 | 38.65 | 47.10 | 7.44 | 6,300.08 | 6,299.69 | 6,305.38 | 6,304.10 |
| A-2 | A3 | A2 | 38.3 | 1.50 | 18" RCP | 6,299.84 | 6,298.18 | 0.043 | 9.36 | 21.88 | 11.89 | 6,301.02 | 6,299.75 | 6,302.56 | 6,304.10 |
| A-3 | A2 | A5 | 331.3 | 3.00 | 36" RCP | 6,296.68 | 6,295.04 | 0.005 | 43.51 | 46.92 | 6.16 | 6,299.68 | 6,298.27 | 6,304.10 | 6,300.87 |
| A-4 | A4 | A5 | 102.9 | 1.25 | 15" RCP | 6,298.13 | 6,296.59 | 0.015 | 1.90 | 7.90 | 5.29 | 6,298.68 | 6,298.22 | 6,302.89 | 6,300.87 |
| A-5 | A5 | A6 | 64.1 | 3.00 | 36" RCP | 6,294.84 | 6,293.54 | 0.020 | 44.48 | 94.98 | 6.29 | 6,298.19 | 6,297.90 | 6,300.87 | 6,300.83 |
| A-6 | A6 | A7 | 286.3 | 3.00 | 36" RCP | 6,293.34 | 6,291.67 | 0.006 | 45.26 | 50.94 | 6.40 | 6,297.71 | 6,296.39 | 6,300.83 | 6,297.03 |
| A-7 | A7 | A9 | 130.1 | 3.00 | 36" RCP | 6,291.47 | 6,290.82 | 0.005 | 45.26 | 47.14 | 6.40 | 6,296.16 | 6,295.56 | 6,297.03 | 6,297.91 |
| A-8 | A8 | A9 | 125.8 | 2.00 | 24" RCP | 6,293.78 | 6,292.52 | 0.010 | 5.24 | 22.64 | 5.86 | 6,295.38 | 6,295.33 | 6,299.57 | 6,297.91 |
| A-9 | A9 | O-2 | 104.4 | 4.00 | 48" RCP | 6,290.50 | 6,290.00 | 0.005 | 65.49 | 99.40 | 5.21 | 6,295.18 | 6,294.96 | 6,297.91 | 6,294.00 |
| B-1 | B1 | B2 | 35.7 | 1.25 | 15" RCP | 6,299.37 | 6,299.19 | 0.005 | 3.17 | 4.59 | 4.03 | 6,300.16 | 6,300.07 | 6,302.34 | 6,303.02 |
| B-2 | B2 | B3 | 109.4 | 1.50 | 18" RCP | 6,298.99 | 6,298.44 | 0.005 | 4.57 | 7.45 | 4.43 | 6,299.96 | 6,299.83 | 6,303.02 | 6,304.77 |
| B-3 | B3 | B4 | 107.9 | 1.50 | 18" RCP | 6,298.24 | 6,297.70 | 0.005 | 4.57 | 7.43 | 2.59 | 6,299.81 | 6,299.61 | 6,304.77 | 6,303.81 |
| B-4 | B4 | B5 | 110.3 | 2.00 | 24" RCP | 6,297.50 | 6,296.95 | 0.005 | 13.80 | 15.97 | 5.72 | 6,298.93 | 6,298.29 | 6,303.81 | 6,304.53 |
| C-1 | FUT-1 | C1 | 60.3 | 3.00 | 36" RCP | 6,300.45 | 6,298.11 | 0.039 | 60.50 | 131.40 | 18.20 | 6,302.96 | 6,299.76 | 6,308.24 | 6,307.03 |
| C-2 | C1 | B5 | 165.7 | 3.50 | 42" RCP | 6,297.61 | 6,295.45 | 0.013 | 60.50 | 114.88 | 12.10 | 6,300.05 | 6,297.29 | 6,307.03 | 6,304.53 |
| C-3 | B5 | C2 | 213.2 | 4.00 | 48" RCP | 6,294.95 | 6,293.67 | 0.006 | 73.75 | 111.28 | 9.47 | 6,297.55 | 6,297.34 | 6,304.53 | 6,302.67 |
| C-4 | C3 | C2 | 15.7 | 1.50 | 18" RCP | 6,296.65 | 6,295.87 | 0.050 | 3.84 | 23.41 | 9.78 | 6,297.40 | 6,297.41 | 6,299.96 | 6,302.67 |
| C-5 | C4 | C2 | 25.7 | 2.50 | 30" RCP | 6,296.15 | 6,294.87 | 0.050 | 17.30 | 91.53 | 14.33 | 6,297.56 | 6,297.41 | 6,300.16 | 6,302.67 |
| C-6 | C2 | EX2 | 26.5 | 4.00 | 48" RCP | 6,293.37 | 6,293.21 | 0.006 | 93.89 | 111.56 | 9.95 | 6,297.32 | 6,297.21 | 6,302.67 | 6,304.40 |
| D-1 | D1 | EX1 | 87.7 | 2.50 | 30" RCP | 6,295.49 | 6,292.40 | 0.035 | 4.20 | 77.00 | 8.39 | 6,296.16 | 6,295.89 | 6,301.22 | 6,298.00 |
| EX-1 | EX2 | EX1 | 204.3 | 4.00 | 48" RCP | 6,292.91 | 6,290.90 | 0.010 | 113.49 | 142.47 | 9.03 | 6,296.98 | 6,295.71 | 6,304.40 | 6,298.00 |
| EX-2 | EX1 | O-1 | 99.4 | 4.00 | 48" RCP | 6,290.60 | 6,290.00 | 0.006 | 117.96 | 111.59 | 9.39 | 6,295.63 | 6,294.96 | 6,298.00 | 6,295.00 |

## IMPERVIOUSNESS AND RUNOFF COEFFICIENTS CALCULATIONS, POST DEVELOPMENT

CALC'D BY: EEM
DATE: 10/29/21
NRCS Hydrologic Soil Group: A/B

PROJECT: Aura at Crossroads
PROJ. NO: 200823

LAND USE TYPES (per Table 6-6 of Volume 1 Update of El Paso County DCM) :

| Land Use or Surface Characteristics | Imperviousness | Runoff Coefficients, C | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | 2-year | | 5-year | | 10-year | | 25-yr | | 50-yr | | 100-yr | |
| | | A/B | C/D | A/B | C/D | A/B | C/D | A/B | C/D | A/B | C/D | A/B | C/D |
| Paved Streets, Drives, Parking, Walks | 100% | 0.89 | 0.89 | 0.90 | 0.90 | 0.92 | 0.92 | 0.94 | 0.94 | 0.95 | 0.95 | 0.96 | 0.96 |
| Building Roofs | 90% | 0.71 | 0.73 | 0.73 | 0.75 | 0.75 | 0.77 | 0.78 | 0.80 | 0.80 | 0.82 | 0.81 | 0.83 |
| Lawns, Landscape Areas | 2% | 0.02 | 0.04 | 0.08 | 0.15 | 0.15 | 0.25 | 0.25 | 0.37 | 0.30 | 0.44 | 0.35 | 0.50 |

| Subbasin | Areas | ACRES | | | Imperviousness | Imperv. Acres | COMPOSITE RUNOFF COEFFICIENTS (Eq. 6-6) | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | Paved Streets, Drives, Parking, Walks | Building Roofs | Lawns, Landscape Areas | | | $C_2$= | $C_5$= | $C_{10}$= | $C_{25}$= | $C_{50}$= | $C_{100}$= |
| X-1 | 0.42 | 0.00 | 0.18 | 0.24 | 39.42% | 0.16 | 0.31 | 0.36 | 0.41 | 0.48 | 0.51 | 0.55 |
| X-2 | 0.01 | 0.01 | 0.00 | 0.00 | 77.91% | 0.01 | 0.69 | 0.72 | 0.75 | 0.78 | 0.80 | 0.82 |
| X-3 | 0.08 | 0.07 | 0.00 | 0.01 | 83.16% | 0.07 | 0.74 | 0.76 | 0.79 | 0.82 | 0.84 | 0.86 |
| A-1 | 2.07 | 0.87 | 0.59 | 0.61 | 68.37% | 1.42 | 0.58 | 0.61 | 0.65 | 0.69 | 0.72 | 0.74 |
| A-2 | 0.43 | 0.13 | 0.12 | 0.18 | 57.23% | 0.25 | 0.48 | 0.52 | 0.56 | 0.61 | 0.64 | 0.67 |
| A-3 | 0.42 | 0.00 | 0.00 | 0.42 | 2.00% | 0.01 | 0.02 | 0.08 | 0.15 | 0.25 | 0.30 | 0.35 |
| A-4 | 0.76 | 0.46 | 0.19 | 0.11 | 83.16% | 0.63 | 0.72 | 0.74 | 0.76 | 0.80 | 0.82 | 0.83 |
| A-5 | 3.67 | 1.92 | 0.94 | 0.81 | 75.80% | 2.78 | 0.65 | 0.68 | 0.71 | 0.75 | 0.77 | 0.79 |
| B-1 | 0.75 | 0.27 | 0.09 | 0.39 | 48.53% | 0.37 | 0.42 | 0.46 | 0.50 | 0.57 | 0.60 | 0.63 |
| B-2 | 0.28 | 0.12 | 0.10 | 0.06 | 74.95% | 0.21 | 0.63 | 0.66 | 0.69 | 0.73 | 0.75 | 0.77 |
| B-3 | 1.91 | 1.17 | 0.31 | 0.43 | 76.49% | 1.46 | 0.67 | 0.69 | 0.72 | 0.76 | 0.78 | 0.80 |
| D-1 | 0.78 | 0.39 | 0.15 | 0.23 | 68.64% | 0.53 | 0.59 | 0.62 | 0.66 | 0.70 | 0.73 | 0.75 |
| C-1 | 0.77 | 0.34 | 0.17 | 0.26 | 64.66% | 0.50 | 0.56 | 0.59 | 0.62 | 0.67 | 0.70 | 0.72 |
| C-2 | 0.44 | 0.36 | 0.00 | 0.08 | 81.51% | 0.36 | 0.73 | 0.75 | 0.77 | 0.81 | 0.83 | 0.84 |
| Z-1 | 0.37 | 0.00 | 0.14 | 0.23 | 35.36% | 0.13 | 0.28 | 0.33 | 0.38 | 0.45 | 0.49 | 0.52 |
| Z-2 | 0.38 | 0.01 | 0.17 | 0.21 | 42.47% | 0.16 | 0.34 | 0.38 | 0.43 | 0.50 | 0.53 | 0.56 |
| Total/Composite | 13.55 | 6.13 | 3.15 | 4.27 | 66.79% | 9.05 | 0.57 | 0.60 | 0.64 | 0.69 | 0.71 | 0.73 |

When multiple sub-basins are delineated, the composite C value calculation is:

$$C_c = (C_1A_1 + C_2A_2 + C_3A_3 + \ldots C_iA_i) / A_t \qquad \text{(Eq. 6-6)}$$

Where:

$C_c$ = composite runoff coefficient for total area

$C_i$ = runoff coefficient for subarea corresponding to surface type or land use

$A_i$ = area of surface type corresponding to $C_i$ (units must be the same as those used for total area)

$A_t$ = total area of all subareas for which composite runoff coefficient applies

i = number of surface types in the drainage area

CALCULATED BY: EEM
CHECKED BY: MW
DATE: 10/29/2021

**Standard Form SF-1 (Modified)**
**Time of Concentration, Post-Development**

JOB NO: 200823
PROJECT: Aura at Crossroads
REVISED: 10/29/2021

| SUB-BASIN DATA | | | INITIAL/OVERLAND TIME ($T_i$) | | | TRAVEL TIME ($T_t$) | | | | | $T_c$ CHECK (URBANIZED BASINS) | | | | FINAL $T_c$ | REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| BASIN | AREA (AC) | $C_5$ | LENGTH (FT) | SLOPE % | $T_i$ (MIN) | LENGTH (FT) | SLOPE % | $C_v$ | VELOCITY (FPS) | $T_t$ (MIN) | COMPOS. $T_c = T_i + T_t$ (MIN) | $L_t$, TOTAL LENGTH | AVG SLOPE | $T_c = (L/180) + 10$ (MIN) | (MIN) | |
| X-1 | 0.42 | 0.36 | 243.0 | 1.26 | 19.7 | 16.0 | 1.00 | 15 | 1.50 | 0.2 | 19.8 | 259 | 1.25 | 11.4 | 11.4 | to proposed area/landscape drains |
| X-2 | 0.01 | 0.72 | 28.9 | 5.29 | 2.2 | 285.0 | 1.30 | 20 | 2.28 | 2.1 | 5.0 | 313.9 | 1.67 | 11.7 | 5.0 | to existing off-site inlet |
| X-3 | 0.08 | 0.76 | 87.5 | 1.41 | 5.2 | 613.6 | 1.10 | 20 | 2.10 | 4.9 | 10.1 | 701.1 | 1.14 | 13.9 | 10.1 | to existing off-site inlet |
| A-1 | 2.07 | 0.61 | 362.0 | 1.13 | 16.4 | 318.0 | 1.18 | 20 | 2.17 | 2.4 | 18.8 | 680 | 1.15 | 13.8 | 13.8 | to proposed curb inlet |
| A-2 | 0.43 | 0.52 | 203.0 | 1.10 | 14.7 | 67.4 | 1.96 | 20 | 2.80 | 0.4 | 15.1 | 270.4 | 1.32 | 11.5 | 11.5 | to proposed curb inlet |
| A-3 | 0.42 | 0.08 | 70.2 | 3.59 | 10.2 | 378.0 | 2.00 | 15 | 2.12 | 3.0 | 13.2 | 448.16 | 2.25 | 12.5 | 12.5 | to proposed area inlet |
| A-4 | 0.76 | 0.74 | 52.4 | 2.18 | 3.7 | 282.0 | 1.10 | 20 | 2.10 | 2.2 | 6.0 | 334.4 | 1.27 | 11.9 | 6.0 | to proposed modified curb inlet |
| A-5 | 3.67 | 0.68 | 198.0 | 1.06 | 10.7 | 701.0 | 1.03 | 20 | 2.03 | 5.8 | 16.5 | 899 | 1.04 | 15.0 | 15.0 | to proposed curb inlet |
| B-1 | 0.75 | 0.46 | 180.0 | 2.11 | 12.3 | 5.0 | 1.00 | 10 | 1.00 | 0.1 | 12.4 | 185 | 2.08 | 11.0 | 11.0 | to proposed area inlet |
| B-2 | 0.28 | 0.66 | 182.5 | 1.28 | 10.1 | 76.0 | 1.70 | 20 | 2.61 | 0.5 | 10.5 | 258.5 | 1.40 | 11.4 | 10.5 | to proposed curb inlet |
| B-3 | 1.91 | 0.69 | 210.3 | 1.54 | 9.5 | 451.0 | 1.00 | 20 | 2.00 | 3.8 | 13.2 | 661.3 | 1.17 | 13.7 | 13.2 | to proposed curb inlet |
| D-1 | 0.78 | 0.62 | 125.0 | 1.48 | 8.6 | 40.0 | 2.60 | 20 | 3.22 | 0.2 | 8.8 | 165 | 1.75 | 10.9 | 8.8 | to proposed curb inlet |
| C-1 | 0.77 | 0.59 | 50.0 | 2.14 | 5.2 | 706.0 | 1.46 | 20 | 2.42 | 4.9 | 10.0 | 756 | 1.51 | 14.2 | 10.0 | to proposed curb inlet |
| C-2 | 0.44 | 0.75 | 13.0 | 0.15 | 4.4 | 754.0 | 1.48 | 20 | 2.43 | 5.2 | 9.5 | 767 | 1.45 | 14.3 | 9.5 | to proposed curb inlet |
| Z-1 | 0.37 | 0.33 | 90.0 | 0.24 | 21.5 | 277.4 | 1.06 | 20 | 2.06 | 2.2 | 23.8 | 367.4 | 0.86 | 12.0 | 12.0 | to proposed area/landscape drains |
| Z-2 | 0.38 | 0.38 | 126.0 | 2.25 | 11.3 | 67.0 | 1.22 | 7 | 0.77 | 1.4 | 12.7 | 193 | 1.89 | 11.1 | 11.1 | to landsacpe drains/grass swale to detention pond |

Estimating Time of Concentration ($T_c$):

$$t_c = t_i + t_t \qquad \text{(RO-2)}$$

in which:

$t_c$ = time of concentration (minutes)

$t_i$ = initial or overland flow time (minutes)

$t_t$ = travel time in the ditch, channel, gutter, storm sewer, etc. (minutes)

**2.4.1 Initial Flow Time**

The initial or overland flow time, $t_i$, may be calculated using equation RO-3:

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \qquad \text{(RO-3)}$$

in which:

$t_i$ = initial or overland flow time (minutes)

$C_5$ = runoff coefficient for 5-year frequency (from Table RO-5)

$L$ = length of overland flow (500 ft maximum for non-urban land uses, 300 ft maximum for urban land uses)

$S$ = average basin slope (ft/ft)

Estimating Overland Travel Time ($T_t$):

$$V = C_v S_w^{0.5} \qquad \text{(RO-4)}$$

in which:

$V$ = velocity (ft/sec)

$C_v$ = conveyance coefficient (from Table RO-2)

$S_w$ = watercourse slope (ft/ft)

**Table RO-2—Conveyance Coefficient, $C_v$**

| Type of Land Surface | Conveyance Coefficient, $C_v$ |
|---|---|
| Heavy meadow | 2.5 |
| Tillage/field | 5 |
| Short pasture and lawns | 7 |
| Nearly bare ground | 10 |
| Grassed waterway | 15 |
| Paved areas and shallow paved swales | 20 |

The time of concentration, $t_c$, is then the sum of the initial flow time, $t_i$, and the travel time, $t_t$, as per Equation RO-2.

CALCULATED BY: EEM

DATE: 3/24/2022

CHECKED BY: MW

PROJECT MANAGER: JDO

**Standard Form SF-2 (Modified)**

**Storm Drainage System Design**

**(Rational Method Procedure)** **Post Development**

JOB NO: 200823

PROJECT: Aura at Crossroads

DESIGN STORM: 5 YR

| | | DIRECT RUNOFF | | | | | | TOTAL RUNOFF | | | | INLET | | PIPE | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| SUBBASIN(s) | DESIGN POINT (DP) | AREA (AC) | RUNOFF COEFF | $T_c$ (min) | C x A (AC) | I (IN/HR) | Q (CFS) | $T_c$ (MIN) | Σ(C x A) (AC) | I (IN/HR) | Q (CFS) | INLET INTERCEPTION (CFS) | BYPASS (CFS) | DESIGN FLOW (CFS) | SLOPE (%) | PIPE SIZE (IN) | CAPACITY AT 80% (CFS) | LENGTH (FT) | VELOCITY (FPS) | $T_t$ (min) | REMARKS |
| Offsite Basin A | | 1.68 | 0.96 | 8.9 | 1.50 | 4.30 | 6.46 | | | | | | | | | | | | | | data per FDR for Crossroads Mixed Use (M&S Civil, August 2021) |
| Offsite Basin B | | 1.49 | 0.96 | 8.8 | 2.28 | 4.32 | 9.85 | | | | | | | | | | | | | | data per FDR for Crossroads Mixed Use (M&S Civil, August 2021) |
| | | | | | | | | 8.9 | 3.78 | 4.30 | 16.27 | | | | | | | | | | |
| X-1 | | 0.42 | 0.36 | 11.4 | 0.15 | 3.93 | 0.58 | | | | | | | | | | | | | | Flows offsite to Meadowbrook Curb Inlet |
| X-2 | | 0.01 | 0.72 | 5.0 | 0.01 | 5.17 | 0.05 | | | | | | | | | | | | | | Flows offsite to Meadowbrook Curb Inlet |
| X-3 | | 0.08 | 0.76 | 10.1 | 0.06 | 4.12 | 0.26 | | | | | | | | | | | | | | Flows offsite to Meadowbrook Curb Inlet |
| Offsite + X-1 + X-2 + X-3 | 0 | | | | | | | 10.1 | 4.00 | 4.12 | 16.48 | | | 16.48 | 0.75 | 30 | 32.23 | 246.75 | 6.6 | 0.63 | |
| A-1 | 1 | 2.07 | 0.61 | 13.8 | 1.26 | 3.65 | 4.61 | | | | | 4.40 | 0.21 | 4.61 | 2.00 | 18 | 13.48 | 38.51 | 7.6 | 0.08 | |
| DP0+DP1 | 2 | | | | | | | 13.9 | 5.27 | 3.64 | 19.17 | | | 19.17 | 0.93 | 36 | 58.35 | 331.00 | 8.3 | 0.67 | |
| A-2 | 3 | 0.43 | 0.52 | 11.5 | 0.22 | 3.92 | 0.88 | | | | | | | 0.88 | 1.50 | 15 | 7.18 | 103.00 | 5.8 | 0.29 | |
| DP2+DP3 | 4 | | | | | | | 14.5 | 5.49 | 3.57 | 19.60 | | | 19.60 | 2.00 | 36 | 85.57 | 65.00 | 12.1 | 0.09 | |
| A-3 | | 0.42 | 0.08 | 12.5 | 0.03 | 3.80 | 0.13 | | | | | | | | | | | | | | Landscape Area Drain to DP-5 |
| DP4+DP5 | 5 | | | | | | | 14.8 | 5.53 | 3.54 | 19.55 | | | 19.55 | 0.55 | 36 | 44.87 | 406.00 | 6.3 | 1.07 | |
| A-4 | 6 | 0.76 | 0.74 | 6.0 | 0.56 | 4.91 | 2.76 | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | 0.00 | 2.00 | 24 | 29.02 | 132.06 | 9.2 | 0.24 | |
| A-5 | | 3.67 | 0.68 | 15.0 | 2.48 | 3.52 | 8.72 | | | | | | | | | | | | | | |
| A-5+DP6 | 7 | | | | | | 8.93 | 15.0 | 8.56 | 3.52 | 30.16 | 6.80 | 2.13 | 30.16 | 0.50 | 48 | 4.14 | 131.29 | 3.4 | 0.65 | |
| B-1 | 8 | 0.75 | 0.46 | 11.0 | 0.35 | 3.98 | 1.38 | | | | | | | 1.38 | 0.50 | 15 | 4.14 | 35.33 | 3.4 | 0.17 | |
| B-2 | | 0.28 | 0.66 | 10.5 | 0.18 | 4.05 | 0.74 | | | | | | | 0.74 | 0.50 | 24 | 14.51 | 215.10 | 4.6 | 0.78 | |
| B-2+DP8 | 9 | | | | | | | 11.7 | 0.53 | 3.90 | 2.06 | | | | | | | | | | |
| B-3 | | 1.91 | 0.69 | 13.2 | 1.32 | 3.71 | 4.89 | | | | | | | | | | | | | | |
| B-3+DP8 | 10 | | | | | | | 13.2 | 1.85 | 3.71 | 6.85 | 4.60 | 0.29 | 6.85 | 0.50 | 30 | 26.31 | 110.00 | 5.4 | 0.34 | |
| Offsite Basin E | | 1.36 | 0.89 | 6.9 | 1.21 | 4.69 | 5.65 | | | | | | | | | | | | | | |
| | 11 | | | | | | | 14.2 | | | 35.00 | | | 35.00 | 1.10 | 36 | 63.46 | 226.00 | 9.0 | 0.42 | |
| DP10 + DP11 | 12 | | | | | | | 14.2 | 1.85 | 3.60 | 41.65 | | | 41.65 | 0.60 | 48 | 100.94 | 213.00 | 8.0 | 0.44 | 1/2 of Basin E in Crossroads Mixed Use PDR |
| C-1 | | 0.77 | 0.59 | 10.0 | 0.45 | 4.12 | 1.86 | | | | | | | 1.86 | 2.00 | 18 | 13.48 | 16.16 | 7.6 | 0.04 | Flow and Time of Travel is from Crossroads PDR |
| | | | | | | | 1.86 | | | | | | | | | | | | | | |
| C-2 | | 0.44 | 0.75 | 9.5 | 0.33 | 4.20 | 1.39 | | | | | | | 1.39 | 2.00 | 30 | 52.62 | 16.16 | 10.7 | 0.03 | |
| OS-1+OS-2+DP12 | 13 | | | | | | | 14.2 | 0.78 | 3.60 | 44.47 | | | 44.47 | 1.00 | 48 | 130.31 | 30.49 | 10.4 | 0.05 | |
| D-1 | 14 | 0.78 | 0.62 | 8.8 | 0.48 | 4.32 | 2.08 | | | | | | | | | | | | | | |
| | | | | | | | 2.08 | | | | | | | 0.00 | 3.50 | 30 | | 87.00 | | | |
| Z-1 | 15 | 0.37 | 0.33 | 12.0 | 0.12 | 3.85 | 0.47 | | | | | | | | | | | | | | Landscape drains and overflow into detention pond |
| Z-2 | 16 | 0.38 | 0.38 | 11.1 | 0.14 | 3.98 | 0.57 | | | | | | | | | | | | | | Swale that flows existing detention pond |

CALCULATED BY: EEM

DATE: 3/24/2022

CHECKED BY: MW

PROJECT MANAGER: JDO

**Standard Form SF-2 (Modified)**

**Storm Drainage System Design**

**(Rational Method Procedure)** **Post Development**

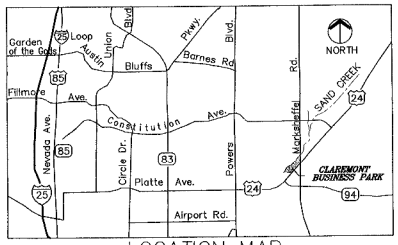
JOB NO: 200823

PROJECT: Aura at Crossroads

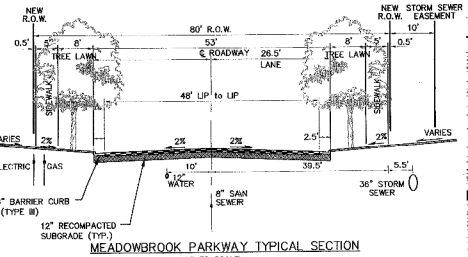
DESIGN STORM: 100 YR

| SUBBASIN(s) | DESIGN POINT (DP) | DIRECT RUNOFF | | | | | | TOTAL RUNOFF | | | | INLET | | PIPE | | | | | | | REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | AREA (AC) | RUNOFF COEFF | $T_c$ (min) | C x A (AC) | I (IN/HR) | Q (CFS) | $T_c$ (MIN) | Σ(C x A) (AC) | I (IN/HR) | Q (CFS) | INLET INTERCEPTION (CFS) | BYPASS (CFS) | DESIGN FLOW (CFS) | SLOPE (%) | PIPE SIZE (IN) | QFULL (CFS) | LENGTH (FT) | VELOCITY (FPS) | $T_t$ (min) | |
| Offsite Basin A | | 1.67 | 0.96 | 8.9 | 1.71 | 7.23 | 12.36 | | | | | | | | | | | | | | data per FDR for Crossroads Mixed Use (M&S Civil, August 2021) |
| Offsite Basin B | | 1.48 | 0.96 | 8.8 | 3.56 | 7.25 | 25.83 | | | | | | | | | | | | | | data per FDR for Crossroads Mixed Use (M&S Civil, August 2021) |
| | | | | | | | | 8.9 | 5.27 | 7.23 | 38.08 | | | | | | | | | | |
| X-1 | | 0.42 | 0.55 | 11.4 | 0.23 | 6.59 | 1.50 | | | | | | | | | | | | | | Flows offsite to Meadowbrook Curb Inlet |
| X-2 | | 0.01 | 0.82 | 5.0 | 0.01 | 8.68 | 0.10 | | | | | | | | | | | | | | Flows offsite to Meadowbrook Curb Inlet |
| X-3 | | 0.08 | 0.86 | 10.1 | 0.07 | 6.91 | 0.50 | | | | | | | | | | | | | | Flows offsite to Meadowbrook Curb Inlet |
| Offsite + X-1 + X-2 + X-3 | 0 | | | | | | | 10.1 | 5.58 | 6.91 | 38.58 | | | 38.58 | 0.75 | 30 | 35.52 | 246.75 | 7.2 | 0.57 | |
| A-1 | 1 | 2.07 | 0.74 | 13.8 | 1.53 | 6.12 | 9.36 | | | | | 6.80 | 2.56 | 9.36 | 2.00 | 18 | 14.86 | 38.51 | 8.4 | 0.08 | |
| DP0+DP1 | 2 | | | | | | | 13.9 | 7.11 | 6.11 | 43.45 | | | 43.45 | 0.93 | 36 | 64.32 | 331.00 | 9.1 | 0.61 | |
| A-2 | 3 | 0.43 | 0.67 | 11.5 | 0.29 | 6.58 | 1.90 | | | | | 1.70 | 0.20 | 1.90 | 1.50 | 15 | 7.91 | 103.00 | 6.4 | 0.27 | |
| DP2+DP3 | 4 | | | | | | | 14.5 | 7.40 | 6.00 | 44.42 | | | 44.42 | 2.00 | 36 | 94.33 | 65.00 | 13.3 | 0.08 | |
| A-3 | | 0.42 | 0.35 | 12.5 | 0.15 | 6.37 | 0.94 | | | | | | | | | | | | | | |
| DP4+DP5 | 5 | | | | | | | 14.5 | 7.55 | 5.99 | 45.20 | | | 45.20 | 0.55 | 36 | 49.46 | 406.00 | 7.0 | 0.97 | |
| A-4 | 6 | 0.76 | 0.83 | 6.0 | 0.64 | 8.24 | 5.24 | | | | | 4.80 | 0.44 | | | | | | | | |
| | | | | | | | | | | | | | | 4.80 | 2.00 | 24 | 31.99 | 132.06 | 10.2 | 0.22 | |
| A-5 | | 3.67 | 0.79 | 15.0 | 2.89 | 5.91 | 17.06 | | | | | | | | | | | | | | |
| A-5+DP6 | 7 | | | | | | 20.26 | 15.0 | 11.07 | 5.91 | 65.43 | 11.60 | 8.66 | 65.43 | 0.50 | 48 | 101.57 | 131.29 | 8.1 | 0.27 | |
| B-1 | 8 | 0.75 | 0.63 | 11.0 | 0.47 | 6.69 | 3.17 | | | | | | | 3.17 | 0.50 | 15 | 4.57 | 35.33 | 3.7 | 0.16 | |
| B-2 | | 0.28 | 0.77 | 10.5 | 0.21 | 6.80 | 1.45 | | | | | | | 1.45 | 0.50 | 24 | 16.00 | 215.10 | 5.1 | 0.70 | |
| B-2+DP8 | 9 | | | | | | | 11.2 | 0.69 | 6.65 | 4.57 | | | | | | | | | | |
| B-3 | | 1.91 | 0.80 | 13.2 | 1.53 | 6.23 | 9.52 | | | | | 6.90 | 2.62 | | | | | | | | |
| B-3+DP8 | 10 | | | | | | | 13.2 | 2.22 | 6.23 | 13.80 | | | 13.80 | 0.50 | 30 | 29.00 | 110.00 | 5.9 | 0.31 | |
| Offsite Basin E | | 1.36 | 0.89 | 6.9 | 1.21 | 7.87 | 9.49 | | | | | | | | | | | | | | |
| | 11 | | | | | | | 14.2 | | | 60.50 | | | 60.50 | 1.10 | 36 | 69.95 | 226.00 | 9.9 | 0.38 | |
| DP10 + DP11 | 12 | | | | | | | 14.6 | 2.22 | 5.98 | 73.75 | | | 73.75 | 0.50 | 48 | 101.57 | 213.00 | 8.1 | 0.44 | |
| C-1 | | 0.77 | 0.72 | 10.0 | 0.56 | 6.92 | 3.84 | | | | | | | 3.84 | 2.00 | 18 | 14.86 | 16.16 | 8.4 | 0.03 | 1/2 of Basin E in Crossroads Mixed Use PDR |
| | | | | | | | 4.79 | | | | | 4.00 | 0.79 | | | | | | | | Flow and Time of Travel is from Crossroads PDR |
| C-2 | | 0.44 | 0.84 | 9.5 | 0.37 | 7.05 | 2.64 | | | | | 1.70 | 0.94 | 2.64 | 2.00 | 30 | 58.01 | 16.16 | 11.8 | 0.02 | |
| OS-1+OS-2+DP12 | 13 | | | | | | | 15.0 | 0.93 | 5.91 | 79.25 | | | 79.25 | 1.00 | 48 | 143.64 | 30.49 | 11.4 | 0.04 | |
| D-1 | 14 | 0.78 | 0.75 | 8.8 | 0.58 | 7.25 | 4.20 | | | | | 4.60 | 2.23 | | | | | | | | |
| | | | | | | | 6.83 | | | | | | | 0.00 | 3.50 | 30 | 76.74 | 87.00 | 15.6 | 0.09 | |
| Z-1 | 15 | 0.37 | 0.52 | 12.0 | 0.20 | 6.46 | 1.27 | | | | | | | | | | | | | | Landscape drains and overflow into detention pond |
| Z-2 | 16 | 0.38 | 0.56 | 11.1 | 0.21 | 6.68 | 1.43 | | | | | | | | | | | | | | Swale that flows existing detention pond |

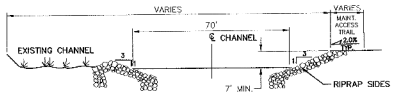
# DRAINAGE PLAN
# CLAREMONT BUSINESS PARK FILING NO. 2



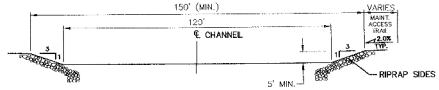

LOCATION MAP
NTS




MEADOWBROOK PARKWAY TYPICAL SECTION
NOT TO SCALE

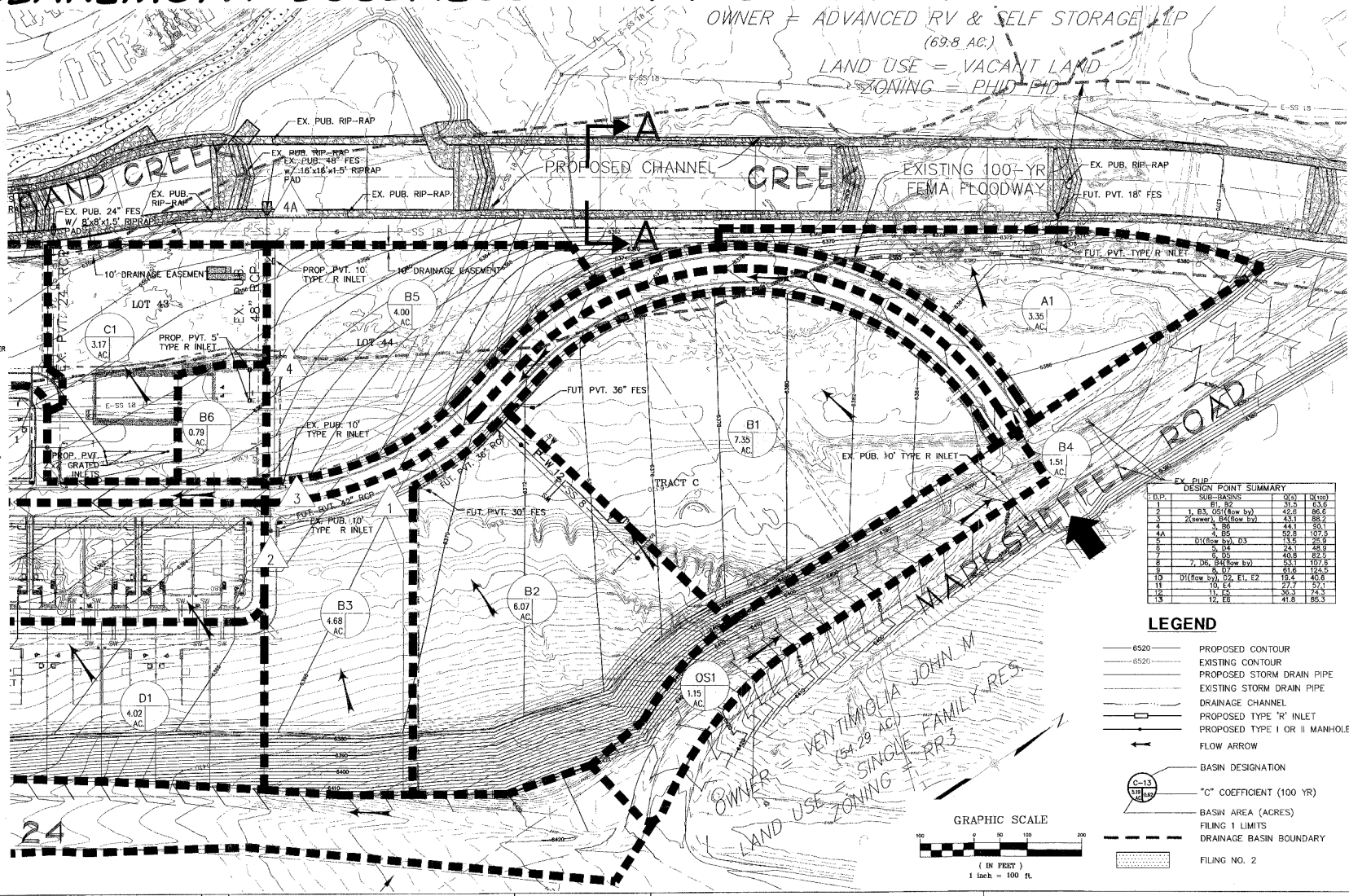



EAST FORK SAND CREEK
SECTION B-B
N.T.S.
FOR INFORMATION ONLY




EAST FORK SAND CREEK
SECTION A-A
N.T.S.
FOR INFORMATION ONLY

NOTE:
CONSTRUCTION OF THE EAST FORK SAND CREEK CHANNEL IMPROVEMENTS WILL BE REQUIRED TO REMOVE A PORTION OF CLAREMONT BUSINESS PARK FILING No. 1 FROM THE EXISTING 100-YEAR FLOODPLAIN (PER FEMA FLOOD INSURANCE RATE MAPS 752F AND 756F)




EX. PUP

DESIGN POINT SUMMARY

| D.P. | SUB-BASINS | Q(5) | Q(100) |
|---|---|---|---|
| 1 | B1, B2 | 31.5 | 63.6 |
| 2 | 1, B3, OS1(flow by) | 42.6 | 86.6 |
| 3 | 2(sewer), B4(flow by) | 43.1 | 88.2 |
| 4 | 3, B6 | 44.1 | 90.1 |
| 4A | 4, B5 | 52.8 | 107.5 |
| 5 | D1(flow by), D3 | 13.5 | 25.9 |
| 6 | 5, D4 | 24.1 | 48.9 |
| 7 | 6, D5 | 40.8 | 82.5 |
| 8 | 7, D6, B4(flow by) | 53.1 | 107.8 |
| 9 | 8, D7 | 61.6 | 124.5 |
| 10 | D1(flow by), D2, E1, E2 | 19.4 | 40.6 |
| 11 | 10, E4 | 27.7 | 57.1 |
| 12 | 11, E5 | 36.3 | 74.3 |
| 13 | 12, E6 | 41.8 | 85.3 |

## LEGEND

- 6520 — PROPOSED CONTOUR
- 6520 — EXISTING CONTOUR
- PROPOSED STORM DRAIN PIPE
- EXISTING STORM DRAIN PIPE
- DRAINAGE CHANNEL
- PROPOSED TYPE 'R' INLET
- PROPOSED TYPE I OR II MANHOLE
- FLOW ARROW
- BASIN DESIGNATION
- C-13 3.18 A 0.60 — "C" COEFFICIENT (100 YR)
- BASIN AREA (ACRES)
- FILING 1 LIMITS
- DRAINAGE BASIN BOUNDARY
- FILING NO. 2

GRAPHIC SCALE
100 0 50 100 200
( IN FEET )
1 inch = 100 ft.

REFERENCE DRAWINGS
-BASE-04.052.001
-HWY
-TOPO
-ROAD
-BASE
-TOPO-09-01-05-bas

| NO. | DATE | DESCRIPTION | BY |
|---|---|---|---|
| | | REVISIONS | |

BENCHMARK DATA(ELEV.)
(DATUM)
(DESCRIPTION/LOCATION)

S:\05.151.006(FILING2)\300 Water Resources\302-FDR\DR01-02.dwg
PLOT DATE: Nov 27, 2006 2:55pm

SUBDIVIDER
HAMMERS CONSTRUCTION INC.
3460 CAPITAL DRIVE
COLORADO SPRINGS, CO 80915-9710

FOR AND ON BEHALF OF MATRIX DESIGN GROUP, INC.

Matrix Design Group, Inc.
Integrated Design Solutions 2435 Research Parkway, Suite 300
Colorado Springs, CO 80920
Phone 719-575-0100
Fax 719-575-0208

CLAREMONT BUSINESS PARK
FINAL DRAINAGE PLAN
MASTER DEVELOPMENT DRAINAGE PLAN
FINAL DRAINAGE PLAN
FILING NO. 2

| | | | |
|---|---|---|---|
| DESIGNED BY: | RGC | SCALE | DATE ISSUED: SEPTEMBER 2006 |
| DRAWN BY: | GES | HORIZ: 1"= 100' | |
| CHECKED BY: | JPL | VERT: N/A | SHEET NO. 1 OF 2 SHEETS |

DR01

# DRAINAGE PLAN
# CLAREMONT BUSINESS PARK FILING NO. 2

| DESIGN POINT SUMMARY | | | |
|---|---|---|---|
| D.P. | SUB-BASINS | Q(5) | Q(100) |
| 1 | B1, B2 | 31.9 | 63.6 |
| 2 | 1, B3, OS1(flow by) | 42.6 | 88.6 |
| 3 | 2(sewer), B4(flow by) | 43.1 | 88.2 |
| 4 | 3, B6 | 44.1 | 90.1 |
| 4A | 4, B5 | 52.8 | 107.5 |
| 5 | D1(flow by), D3 | 13.5 | 25.9 |
| 6 | 5, D4 | 24.1 | 48.9 |
| 7 | 6, D5 | 40.8 | 82.5 |
| 8 | 7, D6, B4(flow by) | 53.1 | 107.6 |
| 9 | 8, D7 | 61.6 | 124.5 |
| 10 | D1(flow by), D2, E1, E2 | 19.4 | 40.6 |
| 11 | 10, E4 | 27.7 | 57.1 |
| 12 | 11, E6 | 36.3 | 74.3 |
| 13 | 12, E6 | 41.8 | 85.5 |
| OS4 | OS4 | 13.4 | 33.5 |

GRAPHIC SCALE
( IN FEET )
1 inch = 100 ft.

EAST

WEST FORK

STATE HIGHWAY

OWNER = COMMITTEE FOR CATHOLIC SECONDARY EDUCATION IN CO SPGS (34.29 Ac)
LAND USE = VACANT LAND
ZONING = PBC PID

OWNER = CYGNET LAND LLC (6.32 Ac)
LAND USE = UNIMPROVED
ZONING = RR3

PROPOSED 18" TRIANGULAR SWALE, 1' DEEP, 3:1 SIDE SLOPES SWALE TO BE CONTAINED WITHIN EASEMENT, OR AGREEMENT TO BE REACHED WITH PROPERTY OWNER.

PROPOSED 42" PUBLIC STORM SEWER; CAPPED BOTH ENDS

PROPOSED 18" PUBLIC CULVERT W/ 18" FES

PROPOSED CROSS PAN

PROPOSED PUBLIC STORM SEWER

FUT. PVT. 30" FES W/ 10'x10'x1.5' RIPRAP PAD

EX. PUB. 42" FES W/ 16'x16'x1.5' RIPRAP PAD

PROP. PUB. 42" FES W/ 14'x14'x1.5' RIPRAP PAD

EX. PUB. RIP-RAP

FUT. PVT. 18" FES W/ 8'x8'x1.5' RIPRAP PAD

10' DRAINAGE EASEMENT

30' UTILITY EASEMENT

PROP. PUB. 42" RCP

PROP. PUB. 36" RCP

PROP. PUB. 24" RCP

PROP. PUB. 18" RCP

EX. PUB. 24" RCP

EX. PUB. 30" RCP

EX. PUB. 21" RCP

PROP. PVT. 24" FES

PROP. PUB. 10' TYPE R INLET

PROP. PUB. 5' TYPE R INLET

PROP. PUB. 15' TYPE R INLET

PROP. PUB. 20' TYPE R INLET

PROP. PVT. 20' TYPE R INLET

EX. PUB. 20' TYPE R INLET

FUT. PVT. 10' TYPE R INLET

FUT. PVT. 5' TYPE R INLET

EX. PUB. 15' TYPE R INLET

LOT 31 LOT 30 LOT 29 LOT 28 LOT 27 LOT 26 LOT 46 LOT 45 LOT 35 LOT 32 LOT 33 LOT 34 LOT 36 LOT 37 LOT 38 LOT 22 LOT 21 LOT 24 LOT 19 LOT 25 LOT 18 LOT 23 LOT 20 LOT 14 LOT 13 LOT 15 LOT 12 LOT 16 LOT 11 LOT 17 LOT 10 LOT 39 LOT 40 LOT 41 LOT 42 LOT 9 LOT 8 LOT 7 LOT 6 LOT 5 LOT 4 LOT 3 LOT 2

E1 5.83 AC. | E2 3.86 AC. | E4 3.87 AC. | E5 4.0 AC. | E6 2.74 AC. | F1 7.72 AC.

D1 | D2 2.07 AC. | D3 2.02 AC. | D4 2.06 AC. | D5 7.69 AC. | D6 5.77 AC. | D7 4.99 AC. | C2 3.28 AC.

OS1 | OS2 7.52 AC. | OS3 0.79 AC. | OS4 15.61 AC.

| REFERENCE DRAWINGS | | | |
|---|---|---|---|
| X-BASE 04.052.001<br>x-util<br>X-TOPO<br>X-ROAD<br>X-BASE<br>X-TOPO-09 01-05.bas | NO. | DATE | DESCRIPTION BY |
| | | | REVISIONS |
| S:\05.151.006(FILING2)\300 Water Resources\302-FDR\DN01 02.dwg<br>PLOT DATE: Nov 27, 2006 2:34pm | | | BENCHMARK DATA(ELEV.)<br>(DATUM)<br>(DESCRIPTION/LOCATION) |

**SUBDIVIDER**
HAMMERS CONSTRUCTION INC.
3460 CAPITAL DRIVE
COLORADO SPRINGS, CO 80915-9710

FOR AND ON BEHALF OF MATRIX DESIGN GROUP, INC.


Matrix Design Group, Inc.
Integrated Design Solutions 2435 Research Parkway, Suite 300
Colorado Springs, CO 80920
Phone 719-575-0100
Fax 719-575-0208


CLAREMONT BUSINESS PAR

FINAL DRAINAGE PLAN

MASTER DEVELOPMENT DRAINAGE PLA
FINAL DRAINAGE PLAN
FILING NO. 2

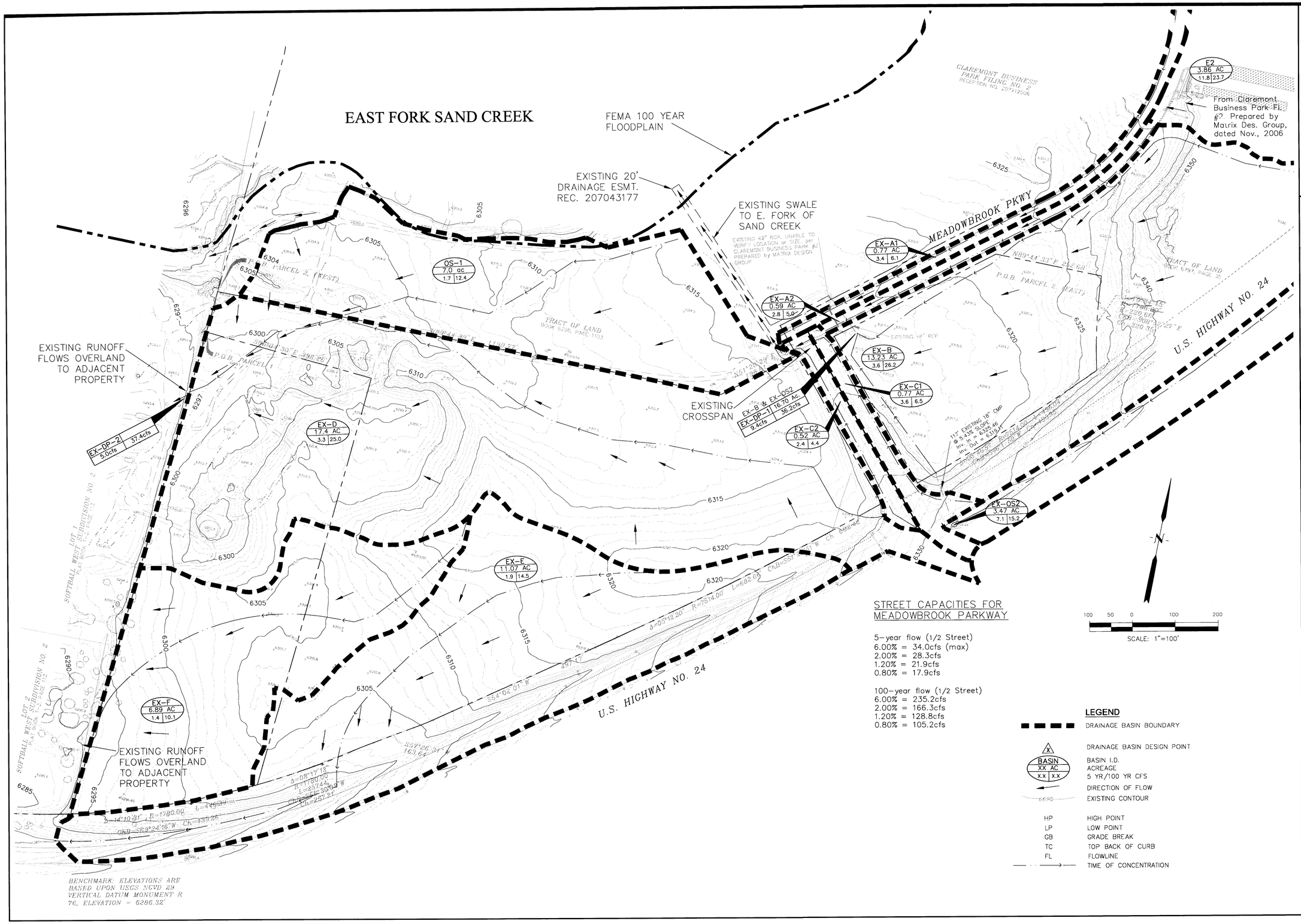
| | | | |
|---|---|---|---|
| DESIGNED BY: | RCC | SCALE | DATE ISSUED: SEPTEMBER 2006 |
| DRAWN BY: | GES | HORIZ: 1"= 100' | |
| CHECKED BY: | JPL | VERT: N/A | SHEET NO. 2 OF 2 SHEETS |

DR

Claremont Business Park Filing No. 2
Rational Method
Fully Developed Conditions
Final Drainage Report

| Sub-Basin Designation | Design Point | Sewer or Road | Sub-Basins | Comments | Total Area (ac.) | Weighted Coefficients | | CA | | Overland Time | | | Travel Time | | | | | T(c) check =d/180+10 | Final T(c) | Intensity | | Peak Runoff | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | C(5) | C(100) | CA(5) | CA(100) | Overland Length (ft) | Overland Slope (%) | T(initial) (min.) | Travel Length (ft) | Weighted Slope (%) | Velocity (fps) | T(travel) (min.) | Final T(c) | | | I(5) (in/hr) | I(100) (in/hr) | Q(5) (cfs) | Q(100) (cfs) |
| A1 | | | | | 3.35 | 0.80 | 0.90 | 2.68 | 3.02 | 60 | 2.0% | 9.8 | 475 | 2.0% | 2.6 | 3.0 | 12.8 | 13.0 | 12.8 | 3.71 | 6.60 | 9.9 | 19.9 |
| B1 | | | | | 7.35 | 0.80 | 0.90 | 5.88 | 6.62 | 50 | 2.0% | 8.9 | 1225 | 1.1% | 2.3 | 8.9 | 17.8 | 17.1 | 17.1 | 3.26 | 5.80 | 19.2 | 38.4 |
| B2 | | | | | 6.07 | 0.66 | 0.76 | 4.03 | 4.64 | 55 | 2.0% | 9.4 | 900 | 1.4% | 2.5 | 6.0 | 15.4 | 15.3 | 15.3 | 3.43 | 6.11 | 13.8 | 28.3 |
| B3 | | | | | 4.68 | 0.71 | 0.81 | 3.33 | 3.80 | 40 | 2.0% | 8.0 | 700 | 1.0% | 2.0 | 5.8 | 13.8 | 14.1 | 13.8 | 3.59 | 6.39 | 12.0 | 24.3 |
| B4 | | | | | 1.51 | 0.80 | 0.90 | 1.21 | 1.36 | 25 | 2.0% | 6.3 | 1650 | 1.3% | 2.3 | 12.0 | 18.3 | 19.3 | 18.3 | 3.15 | 5.61 | 3.8 | 7.6 |
| B5 | | | | | 4.00 | 0.80 | 0.90 | 3.20 | 3.60 | 60 | 2.0% | 9.8 | 500 | 3.0% | 3.2 | 2.6 | 12.4 | 13.1 | 12.4 | 3.76 | 6.70 | 12.0 | 24.1 |
| B6 | | | | | 0.79 | 0.80 | 0.90 | 0.63 | 0.71 | 50 | 2.0% | 8.9 | 175 | 2.0% | 2.6 | 1.1 | 10.1 | 11.3 | 10.1 | 4.09 | 7.28 | 2.6 | 5.2 |
| | 1 | Sewer | B1, B2 | | 13.42 | | | 9.91 | 11.25 | | | 17.1 | 275 | | 5.0 | 0.9 | 18.0 | | | 3.18 | 5.65 | 31.5 | 63.6 |
| | 2 | Sewer | 1, B3, OS1(flowby) | | 18.10 | | | 13.48 | 15.39 | | | 18.0 | 50 | | 5.0 | 0.2 | 18.2 | | | 3.16 | 5.63 | 42.6 | 86.6 |
| | 2 | Road | B3, OS1(flowby) | | 18.10 | | | 3.57 | 4.13 | | | 13.8 | 50 | | 5.0 | 0.2 | 14.0 | | | 3.57 | 6.36 | 12.8 | 26.3 |
| | 3 | Sewer | 2(sewer), B4(flow by) | | 19.61 | | | 13.73 | 15.78 | | | 18.3 | 50 | | 5.0 | 0.2 | 18.4 | | | 3.14 | 5.59 | 43.1 | 88.2 |
| | 4 | Sewer | 3, B6 | | 20.40 | | | 14.37 | 16.49 | | | 18.4 | 250 | | 5.0 | 0.8 | 19.3 | | | 3.07 | 5.47 | 44.1 | 90.1 |
| | 4A | Sewer | 4, B5 | | 24.40 | | | 17.57 | 20.09 | | | 19.3 | 250 | | 5.0 | 0.8 | 20.1 | | | 3.01 | 5.35 | 52.8 | 107.5 |
| C1 | | | | | 3.17 | 0.80 | 0.90 | 2.54 | 2.85 | 50 | 2.0% | 8.9 | 850 | 1.9% | 2.6 | 5.4 | 14.4 | 15.0 | 14.4 | 3.53 | 6.28 | 8.9 | 17.9 |
| C2 | | | | | 3.28 | 0.80 | 0.90 | 2.62 | 2.95 | 50 | 2.0% | 8.9 | 350 | 3.0% | 3.2 | 1.8 | 10.8 | 12.2 | 10.8 | 3.99 | 7.10 | 10.5 | 20.9 |
| D1 | | | | | 4.02 | 0.64 | 0.74 | 2.57 | 2.97 | 50 | 2.0% | 8.9 | 740 | 1.2% | 2.3 | 5.4 | 14.3 | 14.4 | 14.3 | 3.54 | 6.30 | 9.1 | 18.7 |
| D2 | | | | | 2.07 | 0.74 | 0.84 | 1.52 | 1.73 | 50 | 2.0% | 8.9 | 1500 | 1.4% | 2.4 | 10.4 | 19.4 | 18.6 | 18.6 | 3.12 | 5.56 | 4.8 | 9.6 |
| D3 | | | | | 2.02 | 0.73 | 0.83 | 1.48 | 1.69 | 50 | 2.0% | 8.9 | 540 | 1.1% | 2.0 | 4.5 | 13.4 | 13.3 | 13.3 | 3.66 | 6.51 | 5.4 | 11.0 |
| D4 | | | | | 2.06 | 0.73 | 0.83 | 1.51 | 1.72 | 50 | 2.0% | 8.9 | 675 | 1.0% | 2.1 | 5.4 | 14.3 | 14.0 | 14.0 | 3.57 | 6.35 | 5.4 | 10.9 |
| D5 | | | | | 7.69 | 0.80 | 0.90 | 6.15 | 6.92 | 40 | 2.0% | 8.0 | 1550 | 1.0% | 2.1 | 12.3 | 20.3 | 18.8 | 18.8 | 3.11 | 5.53 | 19.1 | 38.3 |
| D6 | | | | | 5.77 | 0.80 | 0.90 | 4.62 | 5.19 | 40 | 2.0% | 8.0 | 1200 | 1.2% | 2.3 | 8.7 | 16.7 | 16.9 | 16.7 | 3.29 | 5.86 | 15.2 | 30.4 |
| D7 | | | | | 4.99 | 0.80 | 0.90 | 3.99 | 4.49 | 75 | 2.0% | 11.0 | 1075 | 1.4% | 2.4 | 7.5 | 18.4 | 16.4 | 16.4 | 3.32 | 5.91 | 13.3 | 26.6 |
| | 5 | Sewer | D1 (flow by), D3 | | 6.04 | | | 3.84 | 4.14 | | | 14.3 | 50 | | 5.0 | 0.2 | 14.5 | | | 3.52 | 6.26 | 13.5 | 25.9 |
| | 6 | Sewer | 5, D4,OS2 | | 8.10 | | | 8.83 | 10.08 | | | 23.1 | 350 | | 5.0 | 1.2 | 24.2 | | | 2.73 | 4.85 | 24.1 | 48.9 |
| | 7 | Sewer | 6, D5 | | 15.79 | | | 14.98 | 17.00 | | | 24.2 | 10 | | 5.0 | 0.0 | 24.3 | | | 2.73 | 4.85 | 40.8 | 82.5 |
| | 8 | Sewer | 7, D6, B4(flow by) | | 23.07 | | | 19.85 | 22.59 | | | 24.3 | 250 | | 5.0 | 0.8 | 25.1 | | | 2.68 | 4.76 | 53.1 | 107.6 |
| | 9 | Sewer | 8, D7 | | 28.06 | | | 23.84 | 27.08 | | | 25.1 | 500 | | 5.0 | 1.7 | 26.8 | | | 2.58 | 4.60 | 61.6 | 124.5 |
| E1 | | | | | 5.83 | 0.77 | 0.87 | 4.49 | 5.08 | 55 | 2.0% | 9.4 | 800 | 1.9% | 2.6 | 5.1 | 14.5 | 14.8 | 14.5 | 3.52 | 6.26 | 15.8 | 31.8 |
| E2 | | | | | 3.86 | 0.80 | 0.90 | 3.09 | 3.47 | 50 | 2.0% | 8.9 | 400 | 1.3% | 2.3 | 2.9 | 11.8 | 12.5 | 11.8 | 3.84 | 6.83 | 11.8 | 23.7 |
| E4 | | | | | 3.87 | 0.80 | 0.90 | 3.10 | 3.48 | 55 | 2.0% | 9.4 | 750 | 1.6% | 2.5 | 5.0 | 14.4 | 14.5 | 14.4 | 3.53 | 6.28 | 10.9 | 21.9 |
| E5 | | | | | 4.00 | 0.80 | 0.90 | 3.20 | 3.60 | 55 | 2.0% | 9.4 | 800 | 1.6% | 2.5 | 5.3 | 14.7 | 14.8 | 14.7 | 3.49 | 6.22 | 11.2 | 22.4 |
| E6 | | | | | 2.74 | 0.80 | 0.90 | 2.19 | 2.47 | 55 | 2.0% | 9.4 | 800 | 1.6% | 2.5 | 5.3 | 14.7 | 14.8 | 14.7 | 3.49 | 6.22 | 7.7 | 15.3 |
| | 10 | Sewer | D1(flow by), D2, E1 | | 7.90 | | | 6.23 | 7.33 | | | 18.6 | 50 | | 5.0 | 0.2 | 18.8 | | | 3.11 | 5.54 | 19.4 | 40.6 |
| | 10 | Road | D1(flow by), D2 | | 7.90 | | | 1.74 | 2.26 | | | 18.6 | 10 | | 5.0 | 0.0 | 18.6 | | | | 5.56 | 5.4 | 12.5 |
| | 11 | Sewer | 10, E4 | | 11.77 | | | 9.33 | 10.82 | | | 18.8 | 550 | | 5.0 | 1.8 | 20.6 | | | | 5.28 | 27.7 | 57.1 |
| | 12 | Sewer | 11, E5 | | 15.77 | | | 12.53 | 14.42 | | | 20.6 | 300 | | 5.0 | 1.0 | 21.6 | | | | 5.16 | 36.3 | 74.3 |
| | 13 | Sewer | 12, E6 | | 18.51 | | | 14.72 | 16.88 | | | 21.6 | 250 | | 5.0 | 0.8 | 22.4 | | | | 5.06 | 41.8 | 85.3 |
| F1 | | | | | 7.72 | 0.80 | 0.90 | 6.18 | 6.95 | 50 | 2.0% | 8.9 | 900 | 1.9% | 2.6 | 5.8 | 14.7 | 15.3 | 14.7 | 3.49 | 6.22 | 21.6 | 43.2 |
| OS1 | | | | Marksheffel | 1.15 | 0.84 | 0.89 | 0.97 | 1.02 | 15 | 2.0% | 4.9 | 630 | 1.7% | 2.4 | 4.4 | 9.3 | 13.6 | 9.3 | 4.22 | 7.51 | 4.1 | 7.7 |
| OS2 | | | | Hwy 24 | 7.52 | 0.46 | 0.56 | 3.48 | 4.23 | 50 | 2.0% | 8.9 | 2300 | 2.0% | 2.6 | 14.7 | 23.7 | 23.1 | 23.1 | 2.80 | 4.98 | 9.7 | 21.1 |
| OS3 | | | | Hwy 24 | 0.79 | 0.50 | 0.60 | 0.40 | 0.47 | 25 | 2.0% | 6.3 | 300 | 3.0% | 3.2 | 1.6 | 7.9 | 11.8 | 7.9 | 4.47 | 7.95 | 1.8 | 3.8 |
| OS4 (Hist) | | | | Meadowbrook | 15.61 | 0.25 | 0.35 | 3.90 | 5.46 | 50 | 2.0% | 8.9 | 1500 | 3.8% | 4.0 | 6.3 | 15.2 | 18.6 | 15.2 | 3.44 | 6.13 | 13.4 | 33.5 |
| OS4 (Future) | | | | Meadowbrook | 15.61 | 0.90 | 0.90 | 14.05 | 14.05 | 50 | 2.0% | 8.9 | 1500 | 3.8% | 6.0 | 4.2 | 13.1 | 18.6 | 13.1 | 3.68 | 6.54 | 51.6 | 91.9 |

UPDATED CALCULATIONS




## STREET CAPACITIES FOR MEADOWBROOK PARKWAY

5-year flow (1/2 Street)
6.00% = 34.0cfs (max)
2.00% = 28.3cfs
1.20% = 21.9cfs
0.80% = 17.9cfs

100-year flow (1/2 Street)
6.00% = 235.2cfs
2.00% = 166.3cfs
1.20% = 128.8cfs
0.80% = 105.2cfs

## LEGEND

- DRAINAGE BASIN BOUNDARY
- DRAINAGE BASIN DESIGN POINT
- BASIN I.D. / ACREAGE / 5 YR/100 YR CFS
- DIRECTION OF FLOW
- EXISTING CONTOUR
- HP HIGH POINT
- LP LOW POINT
- GB GRADE BREAK
- TC TOP BACK OF CURB
- FL FLOWLINE
- TIME OF CONCENTRATION

CORE ENGINEERING GROUP
15004 1ST AVENUE S.
BURNSVILLE, MN 55306
PH: 719.570.1100
CONTACT: RICHARD L. SCHINDLER, P.E.
EMAIL: RichBeng.com

PREPARED FOR:
Circle K Stores, Inc.
1199 South Beltline Road, Suite 160
COPPELL, TEXAS 75019
(940) 453-0015
CONTACT: Tim Peters

PROJECT:
24/94 BUSINESS PARK
Meadowbrook Parkway & Platte Avenue
COLORADO SPRINGS, COLORADO

DESCRIPTION | DATE | NO.

DRAWN: LAB 4/7/15
DESIGNED: LAB 4/7/15
CHECKED: RLS 4/15/15

# EXISTING CONDITIONS DRAINAGE PLAN
## 24/94 BUSINESS PARK
## EL PASO COUNTY, CO

DATE: APRIL, 2016
PROJECT NO. 319.001
SHEET NUMBER 1
TOTAL SHEETS: 1

# EAST FORK SAND CREEK

FEMA 100 YEAR FLOODPLAIN

STREET CAPACITIES FOR MEADOWBROOK PARKWAY

5-year flow (1/2 Street)
6.00% = 34.0cfs (max)
2.00% = 28.3cfs
1.20% = 21.9cfs
0.80% = 17.9cfs

100-year flow (1/2 Street)
6.00% = 235.2cfs
2.00% = 166.3cfs
1.20% = 128.8cfs
0.80% = 105.2cfs

**DESIGN POINT SUMMARY TABLE**

| DESIGN POINT | RUNOFF 5 YR (cfs) | RUNOFF 100 YR (cfs) | COMMENTS |
|---|---|---|---|
| INLET #2 | 40.1 | 71.9 | FLOW IN PIPE |
| INLET #1 | 44.4 | 83.2 | FLOW IN PIPE |
| INLET #3 | 7.1 | 12.9 | ON-SITE INLET, 24" RCP TO WQ VAULT |
| STMH #6 | 7.5 | 15.1 | FROM BASIN EX-OS2 |

SCALE: 1"=40'

PROPOSED STORM SEWER PLAN
LOT 1 OF 24/94 BUSINESS PARK
Meadowbrook Pkwy, EL PASO COUNTY, CO

DATE: JULY 2016
PROJECT NO. 319.001
SHEET NUMBER 2
TOTAL SHEETS: 2

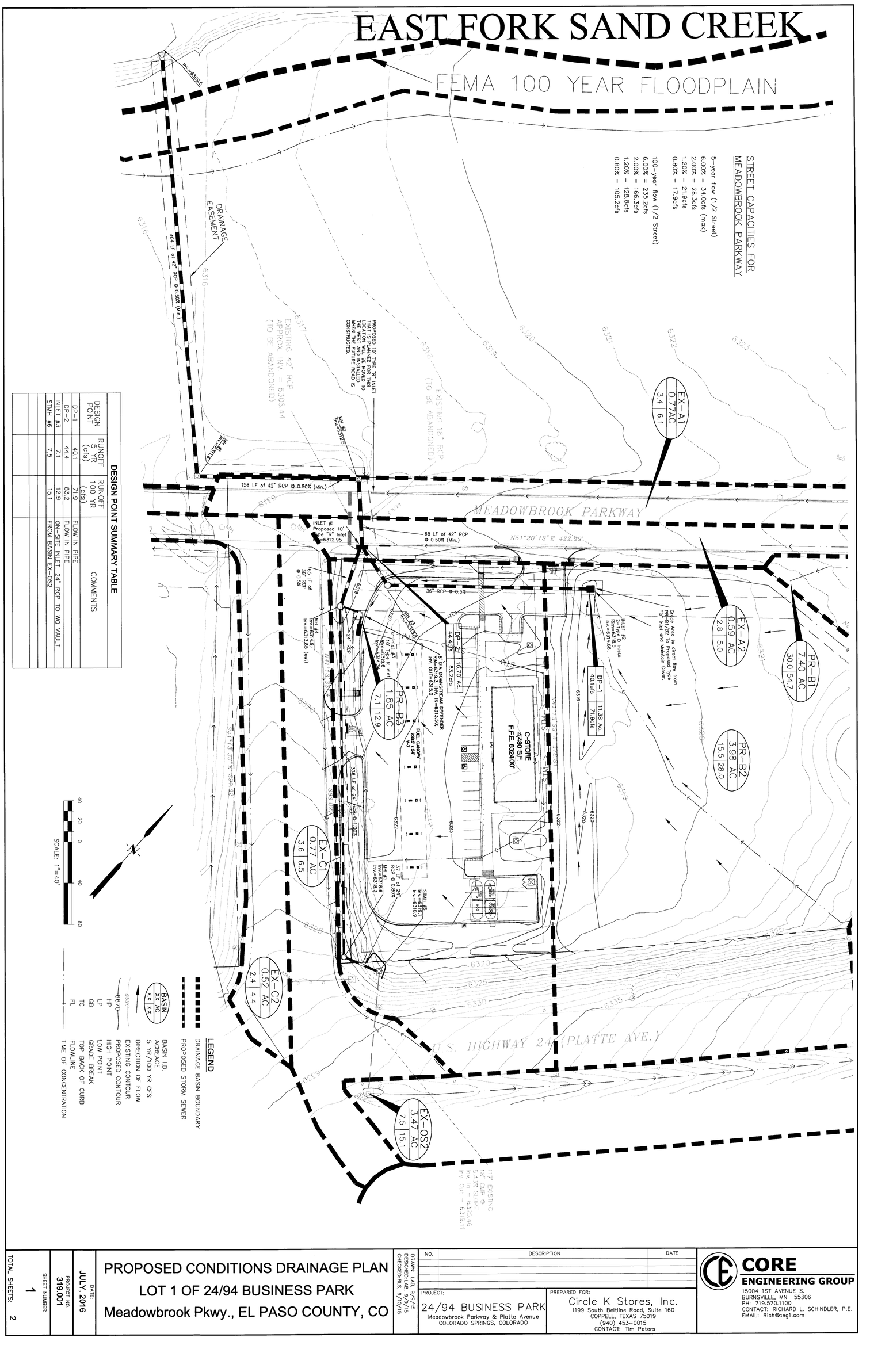
DRAWN: LAB 9/9/15
DESIGNED: RLS 9/9/15
CHECKED: RLS 9/10/15

| NO. | DESCRIPTION | DATE |
|---|---|---|
| | | |

PROJECT:
24/94 BUSINESS PARK
Meadowbrook Parkway & Platte Avenue
COLORADO SPRINGS, COLORADO

PREPARED FOR:
Circle K Stores, Inc.
1199 South Beltline Road, Suite 160
COPPELL, TEXAS 75019
(940) 453-0015
CONTACT: Tim Peters

CORE ENGINEERING GROUP
15004 1ST AVENUE S.
BURNSVILLE, MN 55306
PH: 719.570.1100
CONTACT: RICHARD L. SCHINDLER, P.E.
EMAIL: Rich@ceg1.com




**DESIGN POINT SUMMARY TABLE**

| DESIGN POINT | RUNOFF 5 YR (cfs) | RUNOFF 100 YR (cfs) | COMMENTS |
|---|---|---|---|
| DP-1 | 40.1 | 71.9 | FLOW IN PIPE |
| DP-2 | 44.4 | 83.2 | FLOW IN PIPE |
| INLET #3 | 7.1 | 12.9 | ON-SITE INLET, 24" RCP TO WQ VAULT |
| STMH #6 | 7.5 | 15.1 | FROM BASIN EX-OS2 |

# PROPOSED CONDITIONS DRAINAGE PLAN
## LOT 1 OF 24/94 BUSINESS PARK
## Meadowbrook Pkwy., EL PASO COUNTY, CO

DATE: JULY, 2016
PROJECT NO. 319.001
SHEET NUMBER 1
TOTAL SHEETS: 2

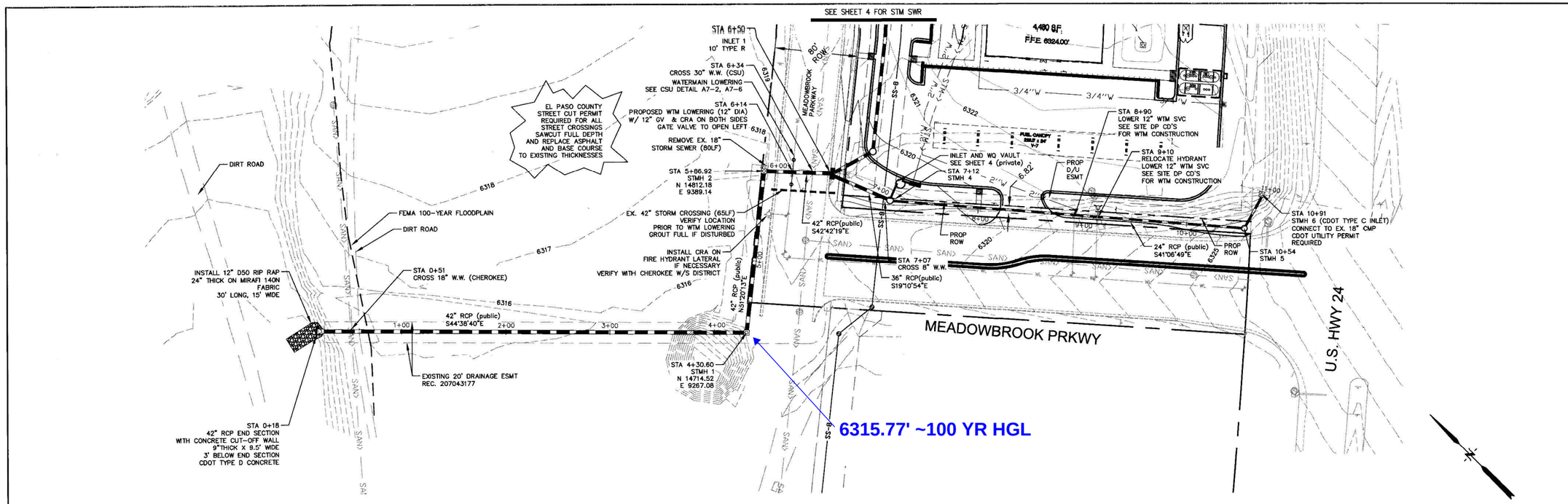
DRAWN: LAB, 9/9/15
DESIGNED: LAB, 9/9/15
CHECKED: KLS, 9/10/15

| NO. | DESCRIPTION | DATE |
|---|---|---|
| | | |

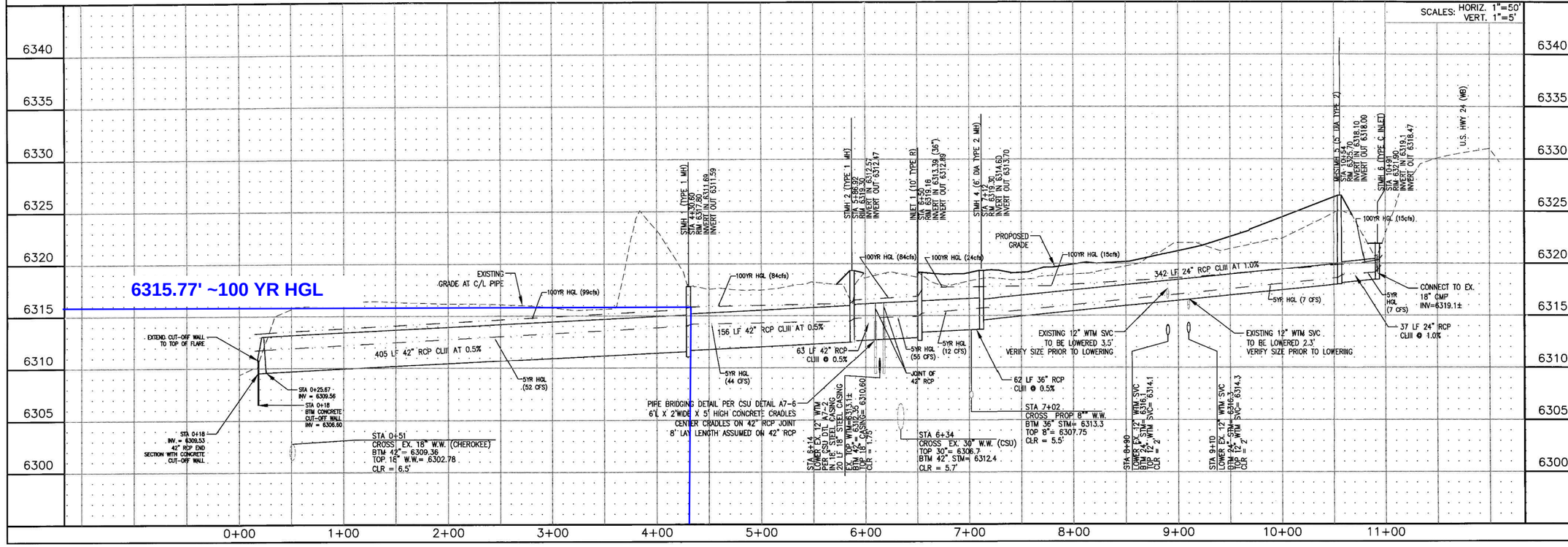
PROJECT:
24/94 BUSINESS PARK
Meadowbrook Parkway & Platte Avenue
COLORADO SPRINGS, COLORADO

PREPARED FOR:
Circle K Stores, Inc.
1199 South Beltline Road, Suite 160
COPPELL, TEXAS 75019
(940) 453-0015
CONTACT: Tim Peters

CORE ENGINEERING GROUP
15004 1ST AVENUE S.
BURNSVILLE, MN 55306
PH: 719.570.1100
CONTACT: RICHARD L. SCHINDLER, P.E.
EMAIL: Rich@ceg1.com




## STORM SEWER PLAN/PROFILE (PUBLIC)




CORE ENGINEERING GROUP
15004 1ST AVENUE S.
BURNSVILLE, MN 55306
PH: 719.570.1100
CONTACT: RICH D. L. SCHINDLER, P.E.
EMAIL: Rich@coreeng.com

NO. | DESCRIPTION | DATE

PREPARED FOR:
LOVE IN ACTION
525 N. WASHINGTON AVE. STE. 301
COLORADO SPRINGS, COLORADO 80903
(719) 635-3200
CONTACT: JEFF MARK

PROJECT:
24/94 BUSINESS PARK
MEADOWBROOK PARKWAY/HWY 24
COLORADO SPRINGS, COLORADO

DRAWN:
DESIGNED:
CHECKED:

# 24/94 BUSINESS PARK
## STORM SEWER PLAN AND PROFILE
## STA 0+18 TO 10+92

DATE:
11/21/2016

PROJECT NO.
319.001

SHEET NUMBER
3

TOTAL SHEETS: 8

CORE ENGINEERING GROUP

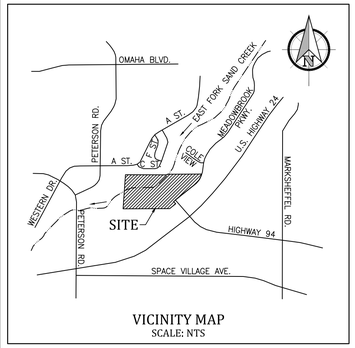
15004 1st Avenue South
Burnsville, MN 55306

PROJECT NAME: 24/94 Business Park
PROJECT NUMBER: 319 001
ENGINEER: LAB
DATE: September 4, 2015

Preliminary Drainage Plan
EXISTING CONDITIONS TRAVEL TIME CALCULATIONS

| BASIN | "P" or "K" (TR-55) | "n" "Cs" | "WP" or "Pipe Dia " | HIGH ELEV. | LOW ELEV. | LENGTH | HEIGHT | SLOPE | " V " | " TT " (min ) | COMMENTS |
|---|---|---|---|---|---|---|---|---|---|---|---|
| EX-A1 | | 0.90 | | 6340 7 | 6340.5 | 10 | 0 2 | 2.00% | | 0 92 | Overland Flow |
| | | | | 6340 0 | 6317.0 | 1250 | 23 0 | 1.84% | 4.00 | 5 21 | Street Flow |
| | | | | | | 1260 | | | | 6.1 | |
| EX-A2 | | 0.90 | | 6335 8 | 6335.6 | 10 | 0 2 | 2.00% | | 0 92 | Overland Flow |
| | | | | 6335.1 | 6318.1 | 916 | 17.0 | 1.86% | 4 02 | 3 80 | Street Flow |
| | | | | | | 926 | | | | 4.7 | |
| EX-B | | 0 08 | | 6380 0 | 6345.0 | 300 | 35 0 | 11.67% | | 14 28 | Overland Flow |
| | | | | 6345 0 | 6316.5 | 574 | 28.5 | 4.97% | 6 57 | 1.46 | |
| | | | | | | 874 | | | | 15.7 | |
| EX-C1 | | 0 90 | | 6332.1 | 6331.9 | 10 | 0.2 | 2.00% | | 0.92 | Overland Flow |
| | | | | 6331.9 | 6317.7 | 574 | 14.2 | 2.47% | 4 64 | 2 06 | Street Flow |
| | | | | | | 584 | | | | 3.0 | |
| EX-C2 | | 0.90 | | 6329.2 | 6329 0 | 10 | 0.2 | 2.00% | | 0 92 | Overland Flow |
| | | | | 6329 0 | 6317.0 | 530 | 12 0 | 2.26% | 4 44 | 1.99 | Street Flow |
| | | | | | | 540 | | | | 2.9 | |
| EX-O | | 0.08 | | 6325 0 | 6314.0 | 230 | 11.0 | 4.78% | | 16 83 | Overland Flow |
| | | | | 6314.0 | 6294 7 | 1585 | 19 3 | 1.22% | 2.13 | 12 41 | Swale Flow |
| | | | | | | 1815 | | | | 29.2 | |
| EX-E | | 0.08 | | 6321.5 | 6316 0 | 282 | 5 5 | 1.95% | | 25 13 | Overland Flow |
| | | | | 6316 0 | 6285.0 | 1364 | 31.0 | 2.27% | 2.62 | 8 69 | Swale Flow |
| | | | | | | 1646 | | | | 33.8 | |
| EX-F | | 0 08 | | 6318 0 | 6307.0 | 300 | 11.0 | 3 67% | | 21.00 | Overland Flow |
| | | | | 6307.0 | 6293 0 | 805 | 14.0 | 1.74% | 2.09 | 6 42 | Swale Flow |
| | | | | | | 1105 | | | | 27 4 | |
| EX-OS1 | | 0.08 | | 6316.0 | 6313 0 | 150 | 3.0 | 2.00% | | 18.18 | Overland Flow |
| | | | | 6313.0 | 6301.0 | 680 | 12.0 | 1.76% | 2.02 | 5 61 | Swale Flow |
| | | | | | | 830 | | | | 23 8 | |
| EX-OS2 | | 0.57 | | 6373.0 | 6371.0 | 35 | 2.0 | 5.71% | | 3.22 | Overland Flow |
| | | | | 6371.0 | 6329 4 | 1643 | 41 6 | 2.53% | 2 85 | 9 62 | Swale Flow |
| | | | | | | 1678 | | | | 12.8 | |
| DP-1 (EX-B & EX-OS2) | | 0.57 | | 6373.0 | 6371.0 | 35 | 2.0 | 5.71% | | 3 22 | Overland Flow |
| | | | | 6371.0 | 6329 4 | 1643 | 41.6 | 2.53% | 2.85 | 9 62 | Swale Flow |
| | | 0.024 | 1.5 | 6325.5 | 6319 1 | 117 | 6.4 | 5 43% | 7.50 | 0 26 | 18" Dia CMP |
| | | | | 6319.1 | 6316 5 | 400 | 2 6 | 0 65% | 1.22 | 5 47 | Swale Flow |
| | | | | | | 517 | | | | 18 6 | |

UPDATED CALCULATIONS




VICINITY MAP
SCALE: NTS


LEGEND

- A — PROPOSED BASIN DESIGNATION
- 1.84 AC — DRAINAGE BASIN ACRES
- C5 RUNOFF COEF 0.76 | 0.83 C100 RUNOFF COEFFICIENT
- DIRECTIONAL FLOW ARROW
- DRAINAGE BASIN BOUNDARY
- 10 — HYDRAULIC STRUCTURE IDENTIFIER
- S10 — STORM SEWER IDENTIFIER
- 1 — DESIGN POINT
- STORMWATER EMERGENCY OVERFLOW PATH
- R.O.W. / PROPERTY LINE
- EXISTING EASEMENT
- EX STM — EXISTING STORM SEWER
- (6020) — EXISTING CONTOURS
- 6020 — PROPOSED CONTOURS
- (2.3%) — EXISTING FLOW DIRECTION AND SLOPE
- 2.3% — PROPOSED FLOW DIRECTION AND SLOPE
- PROPOSED CURB AND GUTTER
- 100 — EXISTING 100 YEAR FLOODPLAIN
- 100 — PROPOSED 100 YEAR FLOODPLAIN
- PROPOSED 100 YEAR BASE FLOOD ELEVATION LINE
- PROPOSED STORM SEWER PIPE AND MANHOLE
- PROPOSED STORM CURB INLET
- PROPOSED OUTLET STRUCTURE

| WATER QUALITY BASIN | | | | |
|---|---|---|---|---|
| | WATER SURFACE ELEV. (FT) | REQUIRED STORAGE VOLUME | RELEASE RATE | PROVIDED STORAGE VOLUME |
| WQCV | 6303.62 | 0.36 AC-FT | | |
| SPILLWAY CREST ELEVATION: 6305.00 | | | | |
| TOP OF EMBANKMENT MINIMUM ELEVATION: 6306.50 | | | | |

NOTES:
1. AS-BUILT SURVEY AND VOLUME CERTIFICATION REQUIRED BY A LICENSED PROFESSIONAL LAND SURVEYOR, SEE GRADING NOTES.

| SUB-BASIN AND DESIGN POINT DISCHARGES | | | |
|---|---|---|---|
| BASIN & CONTRIBUTING BASINS | DESIGN POINT | 5-YR FLOW | 100-YR FLOW |
| OS-A | | 3.0 cfs | 5.9 cfs |
| OS-B | | 3.5 cfs | 6.9 cfs |
| OS-C | | 3.8 cfs | 6.7 cfs |
| OS-D | | 1.8 cfs | 3.4 cfs |
| OS-E | | 2.4 cfs | 5.0 cfs |
| A | | 3.4 cfs | 8.6 cfs |
| B | | 2.1 cfs | 5.3 cfs |
| C | | 1.8 cfs | 4.4 cfs |
| D | | 3.1 cfs | 7.9 cfs |
| E | | 2.2 cfs | 5.4 cfs |
| F | | 2.9 cfs | 7.3 cfs |
| G | | 3.1 cfs | 7.7 cfs |
| H | | 3.7 cfs | 9.3 cfs |
| I | | 0.8 cfs | 3.1 cfs |
| J | | 4.1 cfs | 23.4 cfs |
| F,G | DP 1 | 5.9 cfs | 15.0 cfs |
| E,H | DP 2 | 5.8 cfs | 14.6 cfs |
| A-H | DP 3 | 19.2 cfs | 48.4 cfs |
| A-I | DP 4 | 19.6 cfs | 50.2 cfs |
| A-I, OSB | DP 4A | 22.1 cfs | 55.1 cfs |
| A-J, OSB | DP 5 | 26.3 cfs | 77.7 cfs |
| OSC, OSD | DP 10 | 5.5 cfs | 10.1 cfs |

Kiowa Engineering Corporation
7175 West Jefferson Avenue, Suite 1300
Lakewood, Colorado 80235
(303) 692-0369

# MEADOWBROOK SUBDIVISION
## PRELIMINARY/FINAL DRAINAGE REPORT
## DRAINAGE PLAN - PROPOSED CONDITION
EL PASO COUNTY, COLORADO

| | |
|---|---|
| Project No.: | 16039 |
| Date: | July 25, 2017 |
| Design: | ELS |
| Drawn: | ELS |
| Check: | MWE |
| Revisions: | |

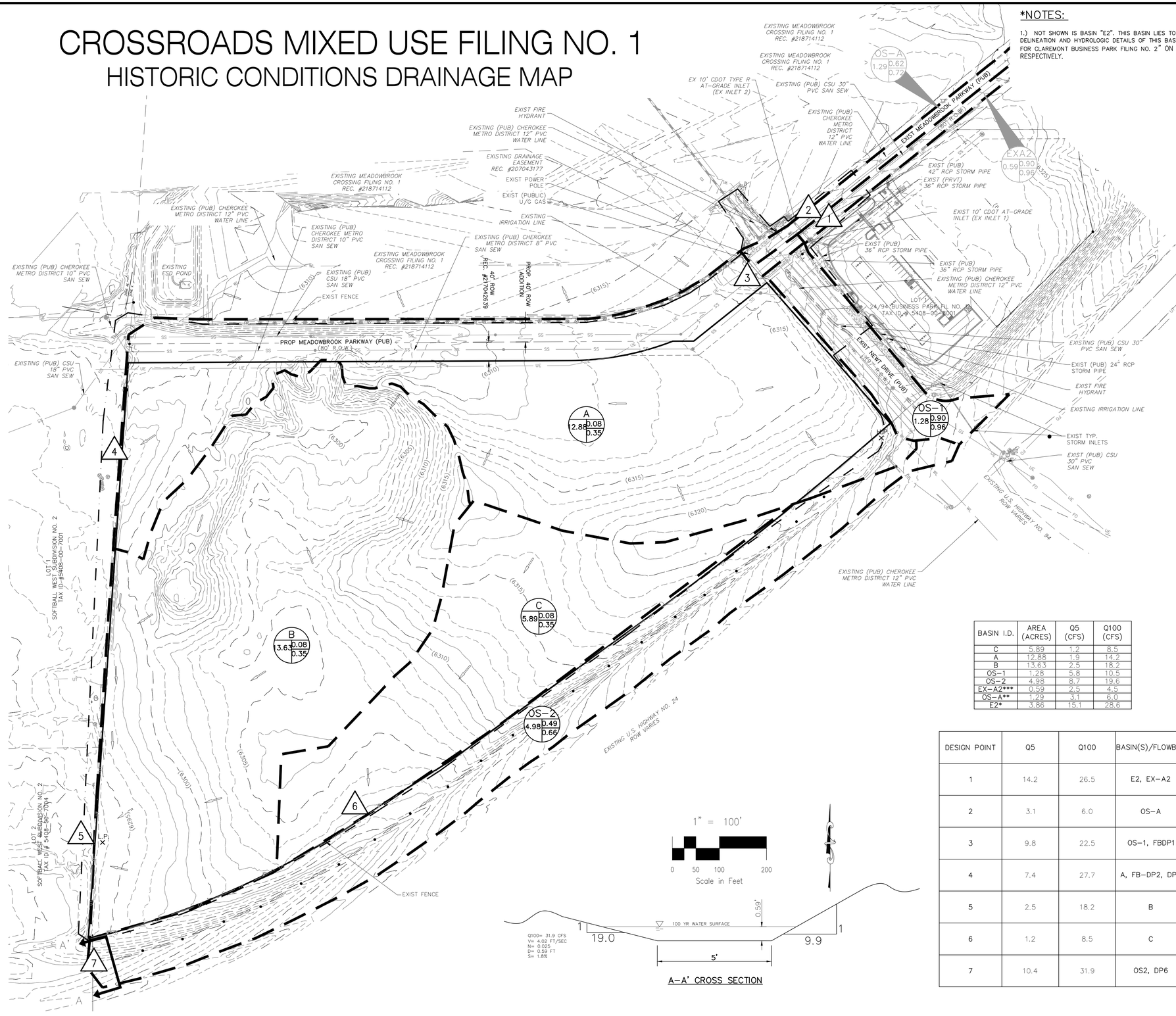
EXHIBIT

B

**DRAINAGE MAPS**

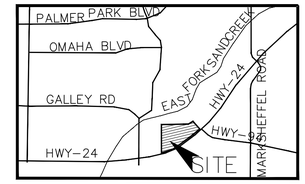
# CROSSROADS MIXED USE FILING NO. 1

## HISTORIC CONDITIONS DRAINAGE MAP




*NOTES:

1.) NOT SHOWN IS BASIN "E2". THIS BASIN LIES TO THE EAST OF BASIN "EX-A2". DELINEATION AND HYDROLOGIC DETAILS OF THIS BASIN CAN BE FOUND IN THE "FDR FOR CLAREMONT BUSINESS PARK FILING NO. 2" ON PAGES 39 AND 41, RESPECTIVELY.




VICINITY MAP
N.T.S.

LEGEND

BASIN DESIGNATION — Z — 25 — .25 / .35 — C5 / C100 — ACRES

- 1 — SURFACE DESIGN POINT
- BASIN BOUNDARY
- (7070) — EXIST MAJ CONT
- (7072) — EXIST MIN CONT
- EXISTING FLOW DIRECTION ARROW
- H.P. — HIGH POINT
- L.P. — LOW POINT
- PROPOSED SWALE
- EXISTING SWALE
- CONSTRUCTION/DISTURBANCE LIMITS
- SITE BOUNDARY
- R.O.W./EASEMENT
- LOT LINE
- ST — EX. STORM SEWER LINE
- UE — EX. UNDERGROUND ELECTRIC LINE
- SS — EX. SANITARY SEWER LINE
- WL — EX. WATER LINE
- ST — EX. STORM SEWER LINE
- 9 — LOT NUMBER
- IGV — EX. IRRIGATION VALVE
- ST — EX. STORM INLET
- G — EX. GAS TEST NODE
- T — EX. TELEPHONE PEDESTAL
- EVLT — EX. ELECTRIC VAULT
- SS — EX. SANITARY MANHOLE
- EX. WATER VALVE

| BASIN I.D. | AREA (ACRES) | Q5 (CFS) | Q100 (CFS) |
|---|---|---|---|
| C | 5.89 | 1.2 | 8.5 |
| A | 12.88 | 1.9 | 14.2 |
| B | 13.63 | 2.5 | 18.2 |
| OS-1 | 1.28 | 5.8 | 10.5 |
| OS-2 | 4.98 | 8.7 | 19.6 |
| EX-A2*** | 0.59 | 2.5 | 4.5 |
| OS-A** | 1.29 | 3.1 | 6.0 |
| E2* | 3.86 | 15.1 | 28.6 |

| DESIGN POINT | Q5 | Q100 | BASIN(S)/FLOWBYS | OUTFALL |
|---|---|---|---|---|
| 1 | 14.2 | 26.5 | E2, EX-A2 | EXIST 10' CDOT TYPE R AT GRADE INLET |
| 2 | 3.1 | 6.0 | OS-A | EXIST 10' CDOT TYPE R AT GRADE INLET |
| 3 | 9.8 | 22.5 | OS-1, FBDP1 | END OF PAVEMENT |
| 4 | 7.4 | 27.7 | A, FB-DP2, DP3 | DISCHARGE TO ADJACENT PARCEL (LOT 1) |
| 5 | 2.5 | 18.2 | B | DISCHARGE TO ADJACENT PARCEL (LOT2) |
| 6 | 1.2 | 8.5 | C | DISCHARGE TO CDOT ROW |
| 7 | 10.4 | 31.9 | OS2, DP6 | CDOT BARROW DITCH |




A-A' CROSS SECTION

HISTORIC DRAINAGE MAP
CROSSROADS MIXED USE
JOB NO. 18-003
DATE PREPARED: JANUARY 31, 2022
DATE REVISED:

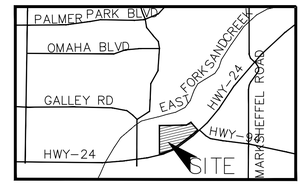

CIVIL CONSULTANTS, INC.
212 N. WAHSATCH AVE., STE 305
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485


PCD FILING NO: SF 21-029

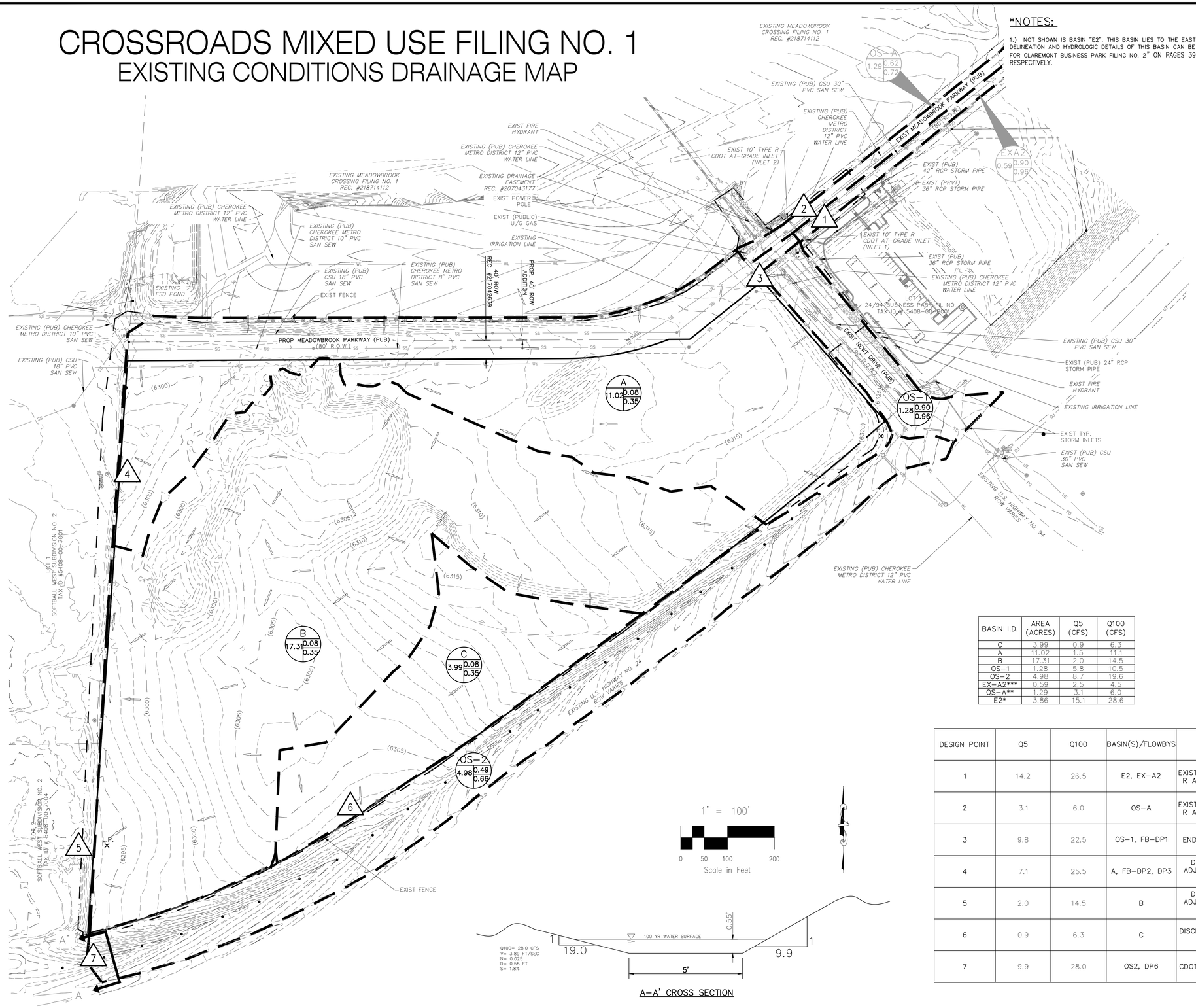
# CROSSROADS MIXED USE FILING NO. 1
## EXISTING CONDITIONS DRAINAGE MAP

*NOTES:

1.) NOT SHOWN IS BASIN "E2". THIS BASIN LIES TO THE EAST OF BASIN "EX-A2". DELINEATION AND HYDROLOGIC DETAILS OF THIS BASIN CAN BE FOUND IN THE "FDR FOR CLAREMONT BUSINESS PARK FILING NO. 2" ON PAGES 39 AND 41, RESPECTIVELY.




VICINITY MAP
N.T.S.




LEGEND

BASIN DESIGNATION — Z / 25 / .25 / .35 / C5 / C100 / ACRES

- SURFACE DESIGN POINT
- BASIN BOUNDARY
- (7070) EXIST MAJ CONT
- (7072) EXIST MIN CONT
- EXISTING FLOW DIRECTION ARROW
- H.P. HIGH POINT
- L.P. LOW POINT
- PROPOSED SWALE
- EXISTING SWALE
- CONSTRUCTION/DISTURBANCE LIMITS
- SITE BOUNDARY
- R.O.W./EASEMENT
- LOT LINE
- ST EX. STORM SEWER LINE
- UE EX. UNDERGROUND ELECTRIC LINE
- SS EX. SANITARY SEWER LINE
- WL EX. WATER LINE
- ST EX. STORM SEWER LINE
- 9 LOT NUMBER
- ICV EX. IRRIGATION VALVE
- ST EX. STORM INLET
- G EX. GAS TEST NODE
- T EX. TELEPHONE PEDESTAL
- ELEC EX. ELECTRIC VAULT
- SS EX. SANITARY MANHOLE
- EX. WATER VALVE

| BASIN I.D. | AREA (ACRES) | Q5 (CFS) | Q100 (CFS) |
|---|---|---|---|
| C | 3.99 | 0.9 | 6.3 |
| A | 11.02 | 1.5 | 11.1 |
| B | 17.31 | 2.0 | 14.5 |
| OS-1 | 1.28 | 5.8 | 10.5 |
| OS-2 | 4.98 | 8.7 | 19.6 |
| EX-A2*** | 0.59 | 2.5 | 4.5 |
| OS-A** | 1.29 | 3.1 | 6.0 |
| E2* | 3.86 | 15.1 | 28.6 |

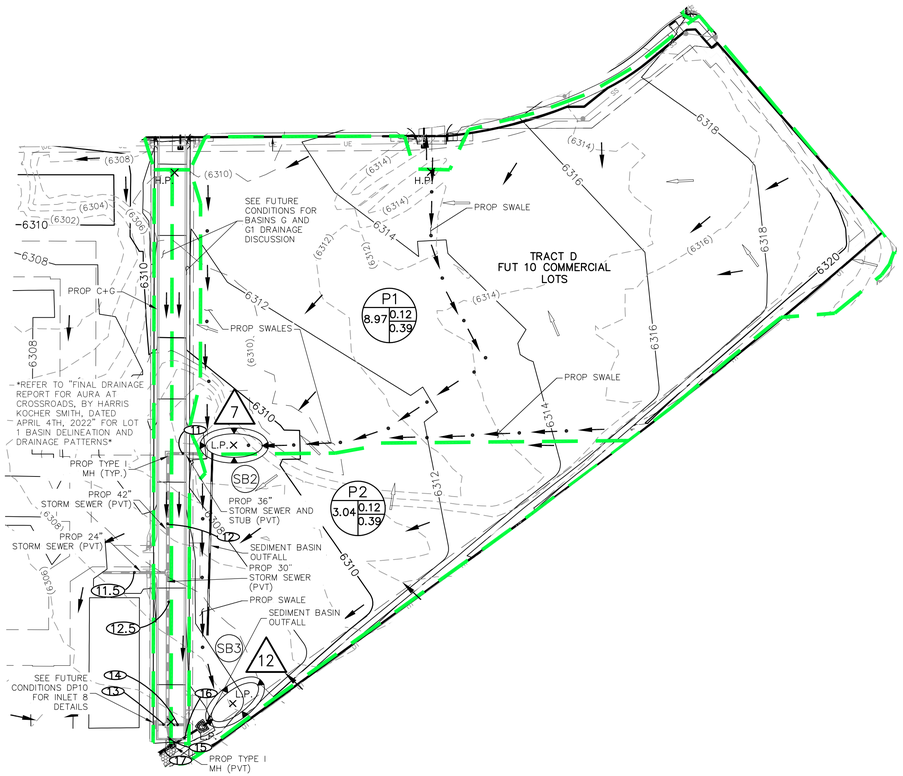
| DESIGN POINT | Q5 | Q100 | BASIN(S)/FLOWBYS | OUTFALL |
|---|---|---|---|---|
| 1 | 14.2 | 26.5 | E2, EX-A2 | EXIST 10' CDOT TYPE R AT GRADE INLET |
| 2 | 3.1 | 6.0 | OS-A | EXIST 10' CDOT TYPE R AT GRADE INLET |
| 3 | 9.8 | 22.5 | OS-1, FB-DP1 | END OF PAVEMENT |
| 4 | 7.1 | 25.5 | A, FB-DP2, DP3 | DISCHARGE TO ADJACENT PARCEL (LOT 1) |
| 5 | 2.0 | 14.5 | B | DISCHARGE TO ADJACENT PARCEL (LOT 2) |
| 6 | 0.9 | 6.3 | C | DISCHARGE TO CDOT ROW |
| 7 | 9.9 | 28.0 | OS2, DP6 | CDOT BARROW DITCH |

EXISTING DRAINAGE MAP
CROSSROADS MIXED USE
JOB NO. 18-003
DATE PREPARED: JANUARY 31, 2022
DATE REVISED:


CIVIL CONSULTANTS, INC.
212 N. WAHSATCH AVE., STE 305
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485


PCD FILING NO: SF 21-029

# CROSSROADS MIXED USE FILING NO. 1
## PROPOSED CONDITIONS DRAINAGE MAP




### SEDIMENT BASIN TABLE

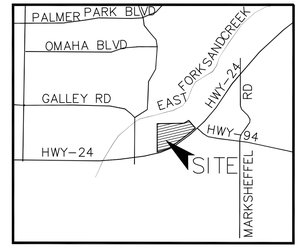
| SEDIMENT BASIN NO. | UPSTREAM DRAINAGE AREA AC. | BASIN WIDTH FT. | BASIN LENGTH FT. | ANTIC. MAX WATER HT FT. | REQ'D VOLUME C.F. | SPILLWAY LENGTH FT. | HOLE DIA. IN. | ROWS OF HOLES IN STANDPIPE |
|---|---|---|---|---|---|---|---|---|
| SB2 | 9 | 55 | 110 | 3 | 36,168 | 13 | 7/8 | 1 |
| SB3 | 4 | 33.5 | 67 | 3 | 16,818 | 6 | 9/16 | 1 |

| BASIN SUMMARY | | | |
|---|---|---|---|
| BASIN | AREA (ACRES) | $Q_5$ | $Q_{100}$ |
| P1 | 8.97 | 3.8 | 20.7 |
| P2 | 3.04 | 1.3 | 7.2 |

| DESIGN POINT SUMMARY | | | | |
|---|---|---|---|---|
| DESIGN POINT | $Q_5$ | $Q_{100}$ | BASIN | STRUCTURE |
| 7 | 3.8 | 20.7 | P1 | PROPOSED SEDIMENT BASIN (SB2) |
| 12 | 5.1 | 27.9 | P2, DP7 | PROPOSED SEDIMENT BASIN (SB3) |

| STORM SEWER SUMMARY | | | | |
|---|---|---|---|---|
| PIPE RUN | $Q_5$ | $Q_{100}$ | PIPE SIZE | CONTRIBUTING PIPES/DESIGN POINTS |
| 11 | 0.0 | 0.0 | 36" SD | N/A |
| 11.5* | 6.9 | 13.8 | 24" SD | SEE FDR FOR AURA AT CROSSROADS |
| 12 | 0.0 | 0.0 | 42" SD | PR11 |
| 12.5 | 6.9 | 13.8 | 48" SD | PR12, PR11.5 |
| 13 | 2.1 | 3.8 | 18" SD | DP10 (INLET 8) |
| 14 | 3.7 | 15.3 | 30" SD | DP11 (INLET 9) |
| 15 | 48.0 | 93.7 | 48" SD | PR12.5, PR13, PR14 |
| 16 | 10.8 | 19.6 | 24" SD | DP12 |
| 17 | 57.0 | 110.1 | 48" SD | PR15, PR16 |

SD = STORM DRAIN
* REFER TO "FDR FOR AURA AT CROSSROADS FOR CONTRIBUTING PIPE FLOW DETAILS




VICINITY MAP
N.T.S.

1" = 100'


Scale in Feet

LEGEND

- BASIN DESIGNATION (Z / 25 / .25 / .35 / C5 / C100; ACRES)
- SURFACE DESIGN POINT
- BASIN BOUNDARY
- PROP MAJ CONT
- PROP MIN CONT
- EXIST MAJ CONT
- EXIST MIN CONT
- PROPOSED STORM SEWER PIPE
- STORM SEWER PIPE BY OTHERS
- FUTURE STORM SEWER PIPE
- EXISTING FLOW DIRECTION ARROW
- H.P. HIGH POINT
- L.P. LOW POINT
- PROPOSED SWALE
- EXISTING SWALE
- SITE BOUNDARY
- UTILITY EASEMENT
- DRAINAGE EASEMENT
- LANDSCAPE EASEMENT
- LOT LINE
- ST STORM SEWER LINE
- UE EX. UNDERGROUND ELECTRIC LINE
- SS EX. SANITARY SEWER LINE
- WL EX. WATER LINE
- ST EX. STORM SEWER LINE
- 9 LOT NUMBER
- ICV EX. IRRIGATION VALVE
- ST EX. STORM INLET
- G EX. GAS TEST NODE
- T EX. TELEPHONE PEDESTAL
- EVL EX. ELECTRIC VAULT
- SS EX. SANITARY MANHOLE
- EX. WATER VALVE
- PROPOSED RIPRAP
- EMERGENCY OVERFLOW DIRECTION
- SB TEMPORARY SEDIMENT BASIN–INITIAL

### NOTES:

1.) REFER TO CROSSROADS MIXED USE FILING NO. 1 FUTURE CONDITIONS MAP FOR SURROUNDING BASIN DELINEATION AND DRAINAGE DETAILS

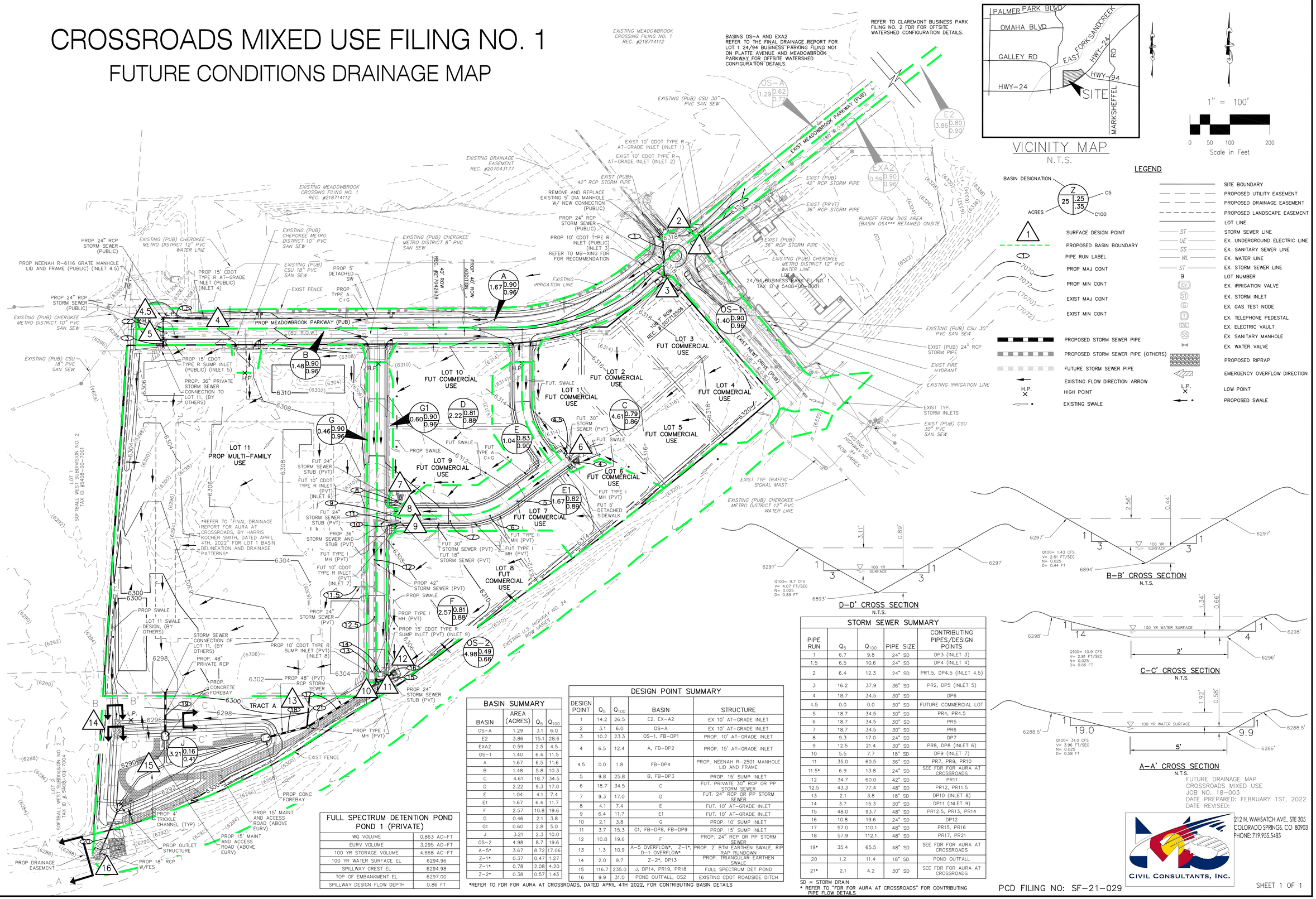
PROPOSED DRAINAGE MAP
CROSSROADS MIXED USE
JOB NO. 18–003
DATE PREPARED: FEBRUARY 7TH 2022
DATE REVISED:


CIVIL CONSULTANTS, INC.
212 N. WAHSATCH AVE., STE 305
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485


PCD FILING NO: SF–21–029

# CROSSROADS MIXED USE FILING NO. 1

## FUTURE CONDITIONS DRAINAGE MAP







VICINITY MAP
N.T.S.

1" = 100'
0 50 100 200
Scale in Feet

LEGEND

BASIN DESIGNATION: Z, 25, .25, .35, C5, C100, ACRES
SURFACE DESIGN POINT
PROPOSED BASIN BOUNDARY
PIPE RUN LABEL
PROP MAJ CONT
PROP MIN CONT
EXIST MAJ CONT
EXIST MIN CONT
PROPOSED STORM SEWER PIPE
PROPOSED STORM SEWER PIPE (OTHERS)
FUTURE STORM SEWER PIPE
EXISTING FLOW DIRECTION ARROW
HIGH POINT
EXISTING SWALE
SITE BOUNDARY
PROPOSED UTILITY EASEMENT
PROPOSED DRAINAGE EASEMENT
PROPOSED LANDSCAPE EASEMENT
LOT LINE
ST STORM SEWER LINE
UE EX. UNDERGROUND ELECTRIC LINE
SS EX. SANITARY SEWER LINE
WL EX. WATER LINE
ST EX. STORM SEWER LINE
9 LOT NUMBER
ICV EX. IRRIGATION VALVE
ST EX. STORM INLET
GS EX. GAS TEST NODE
T EX. TELEPHONE PEDESTAL
EVL EX. ELECTRIC VAULT
SS EX. SANITARY MANHOLE
EX. WATER VALVE
PROPOSED RIPRAP
EMERGENCY OVERFLOW DIRECTION
L.P. LOW POINT
PROPOSED SWALE




D-D' CROSS SECTION
N.T.S.




B-B' CROSS SECTION
N.T.S.




C-C' CROSS SECTION
N.T.S.




A-A' CROSS SECTION
N.T.S.

| BASIN SUMMARY | | | |
|---|---|---|---|
| BASIN | AREA (ACRES) | $Q_5$ | $Q_{100}$ |
| OS-A | 1.29 | 3.1 | 6.0 |
| E2 | 3.86 | 15.1 | 28.6 |
| EXA2 | 0.59 | 2.5 | 4.5 |
| OS-1 | 1.40 | 6.4 | 11.5 |
| A | 1.67 | 6.5 | 11.6 |
| B | 1.48 | 5.8 | 10.3 |
| C | 4.61 | 18.7 | 34.5 |
| D | 2.22 | 9.3 | 17.0 |
| E | 1.04 | 4.1 | 7.4 |
| E1 | 1.67 | 6.4 | 11.7 |
| F | 2.57 | 10.8 | 19.6 |
| G | 0.46 | 2.1 | 3.8 |
| G1 | 0.60 | 2.8 | 5.0 |
| J | 3.21 | 2.3 | 10.0 |
| OS-2 | 4.98 | 8.7 | 19.6 |
| A-5* | 3.67 | 8.72 | 17.06 |
| Z-1* | 0.37 | 0.47 | 1.27 |
| Z-1* | 0.78 | 2.08 | 4.20 |
| Z-2* | 0.38 | 0.57 | 1.43 |

*REFER TO FDR FOR AURA AT CROSSROADS, DATED APRIL 4TH 2022, FOR CONTRIBUTING BASIN DETAILS

| DESIGN POINT SUMMARY | | | | |
|---|---|---|---|---|
| DESIGN POINT | $Q_5$ | $Q_{100}$ | BASIN | STRUCTURE |
| 1 | 14.2 | 26.5 | E2, EX-A2 | EX 10' AT-GRADE INLET |
| 2 | 3.1 | 6.0 | OS-A | EX 10' AT-GRADE INLET |
| 3 | 10.2 | 23.3 | OS-1, FB-DP1 | PROP. 10' AT-GRADE INLET |
| 4 | 6.5 | 12.4 | A, FB-DP2 | PROP. 15' AT-GRADE INLET |
| 4.5 | 0.0 | 1.8 | FB-DP4 | PROP. NEENAH R-2501 MANHOLE LID AND FRAME |
| 5 | 9.8 | 25.8 | B, FB-DP3 | PROP. 15' SUMP INLET |
| 6 | 18.7 | 34.5 | C | FUT. PRIVATE 30" RCP OR PP STORM SEWER |
| 7 | 9.3 | 17.0 | D | FUT. 24" RCP OR PP STORM SEWER |
| 8 | 4.1 | 7.4 | E | FUT. 10' AT-GRADE INLET |
| 9 | 6.4 | 11.7 | E1 | FUT. 10' AT-GRADE INLET |
| 10 | 2.1 | 3.8 | G | PROP. 10' SUMP INLET |
| 11 | 3.7 | 15.3 | G1, FB-DP8, FB-DP9 | PROP. 15' SUMP INLET |
| 12 | 10.8 | 19.6 | F | PROP. 24" RCP OR PP STORM SEWER |
| 13 | 1.3 | 10.9 | A-5 OVERFLOW*, Z-1*, D-1 OVERFLOW* | PROP. 2' BTM EARTHEN SWALE, RIP RAP RUNDOWN |
| 14 | 2.0 | 9.7 | Z-2*, DP13 | PROP. TRIANGULAR EARTHEN SWALE |
| 15 | 116.7 | 235.0 | J, DP14, PR19, PR18 | FULL SPECTRUM DET POND |
| 16 | 9.9 | 31.0 | POND OUTFALL, OS2 | EXISTING CDOT ROADSIDE DITCH |

| FULL SPECTRUM DETENTION POND POND 1 (PRIVATE) | |
|---|---|
| WQ VOLUME | 0.863 AC-FT |
| EURV VOLUME | 3.295 AC-FT |
| 100 YR STORAGE VOLUME | 4.668 AC-FT |
| 100 YR WATER SURFACE EL | 6294.96 |
| SPILLWAY CREST EL | 6294.98 |
| TOP OF EMBANKMENT EL | 6297.00 |
| SPILLWAY DESIGN FLOW DEPTH | 0.86 FT |

| STORM SEWER SUMMARY | | | | |
|---|---|---|---|---|
| PIPE RUN | $Q_5$ | $Q_{100}$ | PIPE SIZE | CONTRIBUTING PIPES/DESIGN POINTS |
| 1 | 6.7 | 9.8 | 24" SD | DP3 (INLET 3) |
| 1.5 | 6.5 | 10.6 | 24" SD | DP4 (INLET 4) |
| 2 | 6.4 | 12.3 | 24" SD | PR1.5, DP4.5 (INLET 4.5) |
| 3 | 16.2 | 37.9 | 36" SD | PR2, DP5 (INLET 5) |
| 4 | 18.7 | 34.5 | 30" SD | DP6 |
| 4.5 | 0.0 | 0.0 | 30" SD | FUTURE COMMERCIAL LOT |
| 5 | 18.7 | 34.5 | 30" SD | PR4, PR4.5 |
| 6 | 18.7 | 34.5 | 30" SD | PR5 |
| 7 | 18.7 | 34.5 | 30" SD | PR6 |
| 8 | 9.3 | 17.0 | 24" SD | DP7 |
| 9 | 12.5 | 21.4 | 30" SD | PR8, DP8 (INLET 6) |
| 10 | 5.5 | 7.7 | 18" SD | DP9 (INLET 7) |
| 11 | 35.0 | 60.5 | 36" SD | PR7, PR9, PR10 |
| 11.5* | 6.9 | 13.8 | 24" SD | SEE FDR FOR AURA AT CROSSROADS |
| 12 | 34.7 | 60.0 | 42" SD | PR11 |
| 12.5 | 43.3 | 77.4 | 48" SD | PR12, PR11.5 |
| 13 | 2.1 | 3.8 | 18" SD | DP10 (INLET 8) |
| 14 | 3.7 | 15.3 | 30" SD | DP11 (INLET 9) |
| 15 | 48.0 | 93.7 | 48" SD | PR12.5, PR13, PR14 |
| 16 | 10.8 | 19.6 | 24" SD | DP12 |
| 17 | 57.0 | 110.1 | 48" SD | PR15, PR16 |
| 18 | 57.9 | 112.1 | 48" SD | PR17, PR21 |
| 19* | 35.4 | 65.5 | 48" SD | SEE FDR FOR AURA AT CROSSROADS |
| 20 | 1.2 | 11.4 | 18" SD | POND OUTFALL |
| 21* | 2.1 | 4.2 | 30" SD | SEE FDR FOR AURA AT CROSSROADS |

SD = STORM DRAIN
* REFER TO "FDR FOR AURA AT CROSSROADS" FOR CONTRIBUTING PIPE FLOW DETAILS

FUTURE DRAINAGE MAP
CROSSROADS MIXED USE
JOB NO. 18-003
DATE PREPARED: FEBRUARY 1ST, 2022
DATE REVISED:

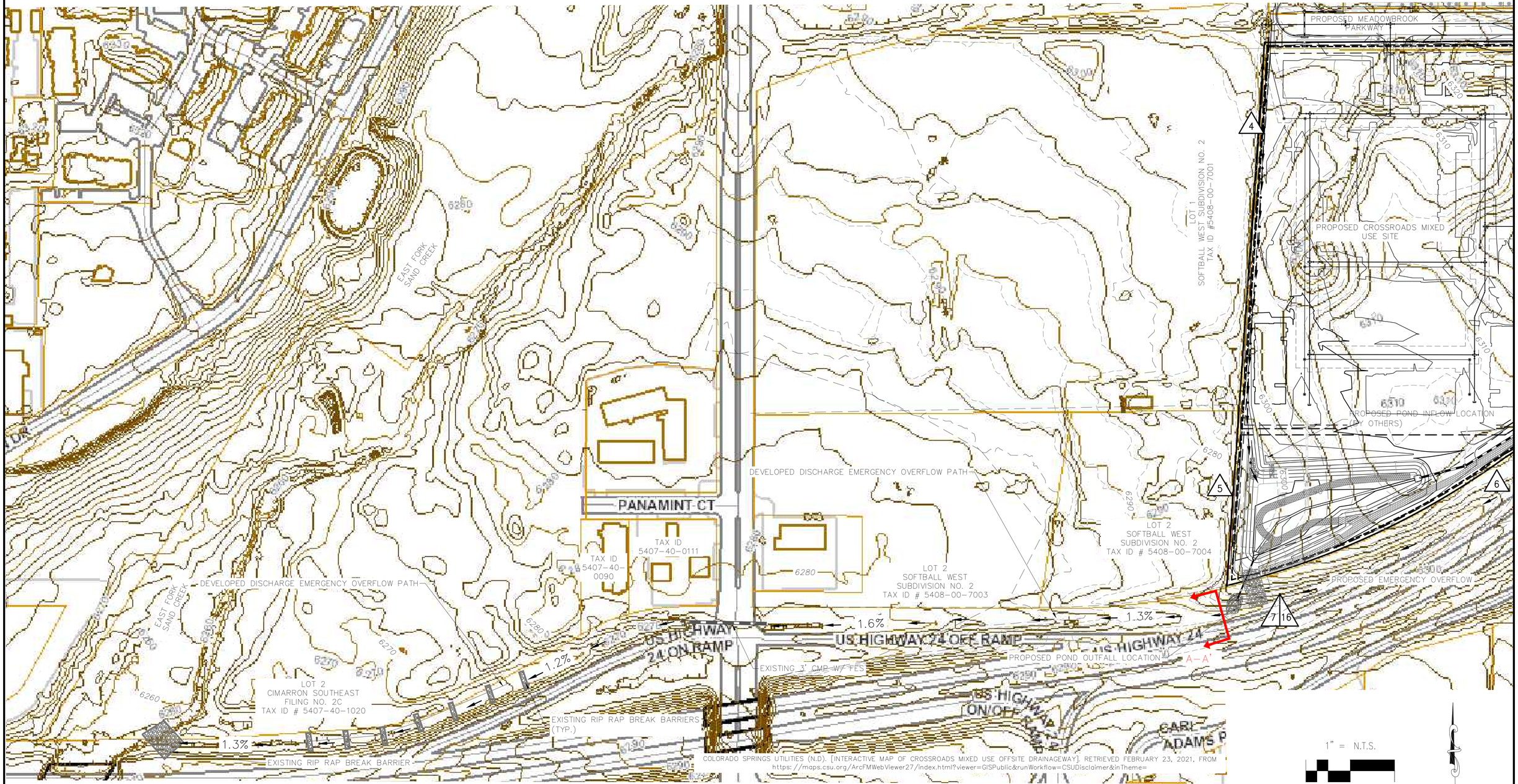

CIVIL CONSULTANTS, INC.
212 N. WAHSATCH AVE., STE 305
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485


PCD FILING NO: SF-21-029

SHEET 1 OF 1

# CROSSROADS MIXED USE

DRAINAGEWAY EXHIBIT
FEBRUARY 2021




COLORADO SPRINGS UTILITIES (N.D). [INTERACTIVE MAP OF CROSSROADS MIXED USE OFFSITE DRAINAGEWAY]. RETRIEVED FEBRUARY 23, 2021, FROM https://maps.csu.org/ArcFMWebViewer27/index.html?viewer=GISPublic&runWorkflow=CSUDisclaimer&inTheme=

## LEGEND

- SURFACE DESIGN POINT
- SITE BOUNDARY
- EXISTING RIP RAP BARRIERS
- PROPOSED CONTOUR (6280)
- EXISTING CONTOUR (6290)
- PROPOSED WATER FITTING
- EXISTING FLOW ARROW
- EXISTING DITCH SLOPE (1.3%)
- EXISTING STORM SEWER AND FES
- PROPOSED STORM SEWER
- R.O.W./EASEMENT
- PROPOSED SWALE
- PROPOSED SANITARY SEWER MANHOLE
- PROPOSED SEWER LINE
- PROPOSED WATER LINE
- PROPOSED EMERGENCY OVERFLOW DIRECTION

## HISTORIC DESIGN POINT SUMMARY

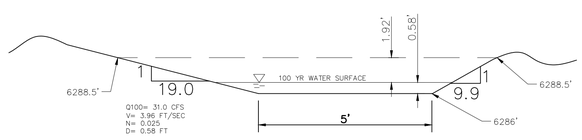
| DESIGN POINT | Q5 (CFS) | Q100 (CFS) |
|---|---|---|
| 4 | 7.4 | 27.7 |
| 5 | 8.3 | 18.2 |
| 6 | 1.2 | 8.5 |
| 7 | 10.4 | 31.9 |

## FUTURE DESIGN POINT SUMMARY

| DESIGN POINT | Q5 (CFS) | Q100 (CFS) |
|---|---|---|
| 4 | 0.0 | 0.0 |
| 5 | 0.0 | 0.0 |
| 6 | 0.0 | 0.0 |
| 16 | 9.9 | 31.0 |

## NOTES:

1.) REFER TO DRAINAGE MAPS IN FINAL DRAINAGE REPORT FOR CROSSROADS MIXED USE FOR DETAILED ON SITE FLOWS AS WELL AS HYDROLOGIC AND HYDRAULIC ANALYSES CORRESPONDING TO CHANGES FROM HISTORIC AND EXISTING CONDITIONS




A-A' CROSS SECTION
N.T.S.

1" = N.T.S.

0 N.T.S.
Scale in Feet

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