## Kimley »Horn

May 28, 2021

Ms. Kelly Nelson Pikes Peak Investments LLC c/o The Equity Group 90 South Cascade Avenue Suite 1500 Colorado Springs, Colorado 80903

Re: Meadowbrook Park Traffic Study Letter El Paso County, Colorado

Dear Ms. Nelson:

This traffic study letter has been prepared for a proposed residential development, Meadowbrook Park, to be located north of Newt Drive between Meadowbrook Parkway and US-24 in El Paso County, Colorado. This letter is an addendum to the *Crossroads-Meadowbrook-Reagan Ranch Master Traffic Impact Study* (MTIS) completed by Kimley-Horn and Associates in April 2021. This supplement to the MTIS is to provide a site-specific analysis addressing comments from master traffic impact study for the Meadowbrook Park. For the purposes of this analysis, full buildout of Meadowbrook Park is expected to include 67 single family residences using private internal roads. A conceptual site plan of the project is attached.

A vicinity map illustrating the location of the property is attached as **Figure 1**. The surrounding area primarily consists of vacant land, industrial uses, and residential use. The existing site is comprised of undeveloped land while residential and industrial uses are located north and northeast of the project site, respectively. The site area is shown in the aerial of attached **Figure 2**.

The purpose of this study is to identify project traffic generation characteristics and to develop an internal roadway circulation plan for the project based on daily traffic volumes projections, as well as to address comments specific to Meadowbrook Park from the MTIS. The existing private driveway along Meadowbrook Parkway with access to the Circle K gas station development and the proposed private access intersection of Spatium View and Meadowbrook Parkway were evaluated. A shared access easement with the Circle K driveway along Meadowbrook Parkway is being proposed as part of this project. This shared access would connect with the internal local streets of Meadowbrook Parkway.

As requested by El Paso County, it should be noted that all known development traffic studies have been included in this study in the last five years and this includes the *Crossroads-Meadowbrook-Reagan Ranch Master Traffic Impact Study* (MTIS) completed by Kimley-Horn and Associates in April 2021. Applicable documents from this master traffic impact study are attached.

#### **Existing Roadway Network**

Regional access to the project is provided by Interstate 25 (I-25) and US-24 while primary access to the project will be provided by Meadowbrook Parkway, State Highway 94 (SH-94), and Newt Drive. Direct access will be provided by two private accesses located along the south side of Meadowbrook Parkway.

Meadowbrook Parkway is an El Paso County Urban Non-Residential Collector roadway that provides one lane of travel in each direction with a 35 mile per hour speed limit through the study area. Newt Drive extends northwest and southeast with one through lane of travel in each direction.

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#### **Trip Generation**

Site-generated traffic estimates are determined through a process known as trip generation. Rates and equations are applied to the proposed land use to estimate traffic generated by the development during a specific time interval. The acknowledged source for trip generation rates is the *Trip Generation Manual*<sup>1</sup> published by the Institute of Transportation Engineers (ITE). ITE has established trip rates in nationwide studies of similar land uses. Project generated traffic volumes are identified on a weekday daily as well as on a morning peak hour and afternoon peak hour basis. The morning peak hour is the highest one-hour time period of adjacent street traffic during four consecutive 15-minute intervals during the morning peak hour, between 7:00 am and 9:00 am. The afternoon peak hour is the highest one-hour time period of four consecutive 15-minute intervals between the hours of 4:00 pm and 6:00 pm representing the afternoon peak hour.

For this study, Kimley-Horn used the ITE Trip Generation Manual fitted curve equations that apply to Single-Family Detached Housing (ITE 210) for traffic associated with the Meadowbrook Park development.

Meadowbrook Park is expected to generate approximately 720 daily weekday trips with 52 of these trips occurring during the morning peak hour and 69 trips occurring during the afternoon peak hour. Calculations were based on the procedure and information provided in the ITE Trip Generation Manual, 10th Edition – Volume 1: User's Guide and Handbook, 2017. **Table 1** provides the estimated trip generation for the project with calculation worksheets attached.

				We	ekday V	ehicle T	rips	
			AM	Peak H	our	PM	Peak H	our
Use	Quantity	Daily	In	Out	Total	In	Out	Total
	Меа	adowbroo	ok Park					
Single Family Housing (ITE 210)	67 Units	720	13	39	52	43	26	69

#### Table 1 – Meadowbrook Parkway Project Traffic Generation

#### Project Access Spacing Requirements and Internal Roadway Classifications

The existing west private street access along Meadowbrook Parkway provides full turning movements for the Circle K development. The project is proposing a shared access easement with the existing access to the Circle K gas station along Meadowbrook Parkway. This west access along Meadowbrook Parkway is currently located approximately 250 feet northeast of the Newt Drive (measured centerline to centerline). The proposed access intersection of Spatium View and Meadowbrook Parkway is proposed to align with Preble Drive and is located approximately 500 feet northeast of the Circle K access and 480 feet south of the Cole View and Meadowbrook Parkway intersection.

The intersection offsets surrounding the proposed access intersection of Spatium View and Meadowbrook Parkway meets the El Paso County spacing standards of 330 feet along collector roadways with access to local streets.

As the project is only anticipated to generate 720 daily vehicle trips and the internal streets to the project will not have any cut through traffic, all internal streets to the project meet El Paso County average daily traffic threshold standard of 3,000 vehicles per day for an Urban Local street.

All roadways proposed with the exception of Spatium View have a paved cross section that is less than the County urban local low volume roadway. The report should also compare the proposed roadways to the County Urban local low volume(threshold of 300 ADT) as all roadways except Spatium are most like this standard cross section. Per figure 3 it appears that the roadways (except spatium view) meet the 300 ADT threshold of the Urban local low volume. This should be stated in the text.



Please also indicate whether Circle K access point will operate acceptably (LOS D) with the added traffic of this development. Is the intent for the Circle K access/Solum Grove access to have a gate for emergency access only as inferred in the Cimarron Hills Fire Department letter attached to the deviation request form? Please address:<sup>a</sup> 3 illustrates the circulation plan and street classification map for roadways

> Project Access: Lane Configurations, Operational Analysis, and Turn Lane Requirements The future access intersection of Spatium View and Meadowbrook Parkway will align with Preble Drive. Left turn movements for entering this project access will be provided from an existing two-way left turn lane along Meadowbrook Parkway. The westbound exiting approach of this driveway should provide stop control with installation of a R1-1 "STOP" sign. It is recommended that the existing access to Circle K along Meadowbrook Parkway remain with same lane configurations and stop control. Based on the original master traffic impact study, all movements at the intersection of Spatium View and Meadowbrook Parkway are expected to operate acceptably during the peak hours throughout the 2040 horizon. Applicable documents from the master traffic impact study including intersection operational outputs, traffic volume projections, and recommended lane configurations and control are attached. Please indicate whether or not the length of the existing two way left turn lane is sufficient for each of the access points

> The El Paso County ECM was used to dated which is part and any left with any left with the project access. El Paso County classifies Meadowbrook Parkway as an Urban Non-Residential Collector. According to El Paso County classifies for Minor Arterials and Lower Classifications, a right turn lane is required for any access with a projected peak hour right turning volume of 50 vehicles per hour or greater.

Based on 2040 traffic volume projections, right turn lane requirements at the intersection of Spatium View and Meadowbrook Parkway are as follows:

• A northeastbound right turn lane is not warranted for the intersection of Spatium View and Meadowbrook Parkway based on projected 2040 total traffic volumes being 30 northbound right turns during the peak hour and the threshold being 50 vehicles per hour. It should be noted that the master traffic study conservatively did not evaluate the Circle K driveway access and these 30 northbound turn movements are expected to be less when some traffic is distributed to second access location.

#### Sight Distance Evaluation

It is recommended that appropriate sight distance triangles be provided at the future access intersection of Spatium View and Meadowbrook Parkway to give drivers exiting the development areas a clear view of oncoming traffic. Landscaping and objects within sight triangles must not obstruct drivers' views of the adjacent travel lanes. Intersection sight distances for left turn from stop and right turn from stop were analyzed for the proposed project access along Meadowbrook Parkway.

With El Paso County standards and a design speed of 40 miles per hour along Meadowbrook Parkway, the intersection sight distance for a vehicle turning from stop is 445 feet. Therefore, all obstructions for turning vehicles from stop should be clear to the right and left within the triangle created with a vertex point located 13 feet (10 feet from local roads) from the edge of the major road traveled way (typical position of the minor road driver's eye when stopped) and a line of sight distance of 445 feet located in the middle of the northeastbound and southwestbound through lanes along Meadowbrook Parkway. It is believed that the project access is appropriately located to provide the necessary sight distance needed but verification should be provided with sight distance triangles incorporated within the design plans.

#### **Conclusions and Recommendations**

In summary, this traffic study provides project traffic generation estimates to identify potential project traffic related impacts on the local street system with the proposed Meadowbrook Park project. Based on the analysis presented in this study, Kimley-Horn believes the proposed Meadowbrook Park development will be successfully incorporated into the existing and future roadway network. It is not



A deviation request for the proposed private roadways not meeting ECM standards has been submitted. Please revise<sup>Meadowbrook Park Traffic Letter</sup> 096956015 the text accordingly. Page 4

anticipated that any deviations from El Paso County ECM criteria will be needed with development of Meadowbrook Park. El Paso County road impact fees will be coordinated with the County through the development process; however, it should be noted that reimbursement may not be available for certain improvements.

The intersection offsets surrounding the proposed access intersection of Spatium View and Meadowbrook Parkway meets the El Paso County spacing standards of 330 feet along collector roadways with access to local streets.

The future access intersection of Spatium View and Meadowbrook Parkway will align with Preble Drive. Left turn movements for entering this project access will be provided from an existing two-way left turn lane along Meadowbrook Parkway. The westbound exiting approach of this driveway should provide stop control with installation of a R1-1 "STOP" sign.

It is believed that the project access along Meadowbrook Parkway is appropriately located to provide the necessary intersection sight distance set forth by El Paso County.

If you have any questions or require anything further, please feel free to call me at (720) 943-9962.

Sincerely,

KIMLEY-HORN AND ASSOCIATES, INC.

Drey R. Hanck

Jeffrey R. Planck, P.E. Project Manager Please identify the Road impact fees for the development.



Please identify any offsite improvements that will be triggered by this project should this be developed first before any of the other projects included in the master TIS. If there is none, then please state that in your narrative.



Meadowbrook Park Traffic Letter 096956015 Page 5

Traffic Engineer's Statement

The attached traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.

frey R. Hanck

Jeffrey R. Planck, P.E., PE #53006

<u>May 28, 2021</u> Date

Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

Ms. Kelly Nelson Pikes Peak Investments LLC c/o The Equity Group 90 South Cascade Avenue Suite 1500 Colorado Springs, Colorado 80903 Date

Figures









Trip Generation Worksheets

## Meadowbrook Park Trip Generation Summary

				We	ekday V	ehicle Ti	rips	
			AM	Peak H	our	PM	Peak H	our
Use	Quantity	Daily	In	Out	Total	In	Out	Total
	Меа	adowbroo	ok Park					
Single Family Housing (ITE 210)	67 Units	720	13	39	52	43	26	69

# Kimley **»Horn**

Project Crossroads-Meadwok	prook-Reagan Ra	nch (M	eadowb	rook P	ark)				
Subject I rip Generation for Si	ngle-Family Deta	Chea H Februa	ousing arv 08. 2	2021	J	nh No.	0969560	)15	
Checked by	Date	1 00.0.0	(1) CC, _		She	et No.	0000000	of	
TRIP GENERATION MANUAL TEC   TE Trip Generation Manual 10th Ec   _and Use Code - Single-Family Det   ndependant Variable - Dwelling Un   X = 67	<u>HNIQUES</u> dition, Fitted Curv ached Housing ( its (X)	'e Equa 210)	tions						
T = Average Vehicle Trip Ends	; ffic. One Hour B	etweer	7 and	9 a.m.	(200 S	eries P	age 3)		
Average Weekday (T) = 0.71 (X) + 4.80 (T) = 0.71 * (67) + 4.80	D T 4.80	irection = 13	al Distri 52 enterin	ibution: Avei	rage Ve 39	25% ehicle T exitir	ent. 7: rip Ends	5%	exit.
Peak Hour of Adjacent Street Tra	<u>ffic, One Hour B</u>	13 <u>etweer</u>	+ <u>1 4 and</u>	39 <u>6 p.m.</u>	= (200 S	52 Series P	<u>age 4)</u>		
Average Weekday _n(T) = 0.96 Ln(X) + 0.20 _n(T) = 0.96 * Ln(67) + 0	D T 0.20	rirection = 43	al Distri 69 enterin	ibution: Avei g	: rage Ve 26	63% ehicle T exitir	ent. 3 <sup>-</sup> rip Ends ng	7% (	exit.
		43	+	26	=	69			
Peak Hour of Generator, Saturday	y (200 Series Pag	<u>ge 8)</u>							
Average Saturday T) = 0.84 (X) + 17.99 T) = 0.84 * (67) +	D T 17.99	irection = 40	al Distri 74 enterin	ibution: Avei g	: rage Ve 34	54% ehicle T exitir	ent. 40 rip Ends ng	6% (	exit.
		40	+	34	=	74			
<u>Neekday (200 Series Page 2)</u>									
Average Weekday _n(T) = 0.92 Ln(X) + 2.71 _n(T) = 0.92 * Ln(67) + 2	D T 2.71	virection = 360	al Distri 720 enterin	ibution: Avei g	: 50% rage Ve 360	enterinç ehicle T exitir	g, 50% e rip Ends ng	xiting	

Original Traffic Study Documents

### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		1	et 👘		1	et 👘	
Traffic Vol, veh/h	5	0	10	30	0	15	5	280	10	5	365	5
Future Vol, veh/h	5	0	10	30	0	15	5	280	10	5	365	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage	,# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	6	2	2	6	2
Mvmt Flow	5	0	11	33	0	16	5	304	11	5	397	5

Major/Minor	Minor2		ļ	Minor1			Major1		Ν	/lajor2			
Conflicting Flow All	738	735	400	735	732	310	402	0	0	315	0	0	
Stage 1	410	410	-	320	320	-	-	-	-	-	-	-	
Stage 2	328	325	-	415	412	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	334	347	650	335	348	730	1157	-	-	1245	-	-	
Stage 1	619	595	-	692	652	-	-	-	-	-	-	-	
Stage 2	685	649	-	615	594	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	324	344	650	327	345	730	1157	-	-	1245	-	-	
Mov Cap-2 Maneuver	439	438	-	440	438	-	-	-	-	-	-	-	
Stage 1	617	593	-	689	649	-	-	-	-	-	-	-	
Stage 2	667	646	-	602	592	-	-	-	-	-	-	-	
Annroach	ГD						ND			CD			

Approach	EB	WB	NB	SB	
HCM Control Delay, s	11.6	12.9	0.1	0.1	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1157	-	-	560	507	1245	-	-	
HCM Lane V/C Ratio	0.005	-	-	0.029	0.096	0.004	-	-	
HCM Control Delay (s)	8.1	-	-	11.6	12.9	7.9	-	-	
HCM Lane LOS	А	-	-	В	В	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	0.1	0.3	0	-	-	

## Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		1	et 👘		1	et 👘	
Traffic Vol, veh/h	5	0	10	20	0	10	10	210	30	15	405	5
Future Vol, veh/h	5	0	10	20	0	10	10	210	30	15	405	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage	,# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	5	0	11	22	0	11	11	228	33	16	440	5

Major/Minor	Minor2			Vinor1			Major1			Ν	/lajor2			
Conflicting Flow All	747	758	443	747	744	245	445	C	)	0	261	0	0	
Stage 1	475	475	-	267	267	-	-			-	-	-	-	
Stage 2	272	283	-	480	477	-	-			-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12			-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-			-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-			-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218			-	2.218	-	-	
Pot Cap-1 Maneuver	329	336	615	329	343	794	1115			-	1303	-	-	
Stage 1	570	557	-	738	688	-	-			-	-	-	-	
Stage 2	734	677	-	567	556	-	-			-	-	-	-	
Platoon blocked, %										-		-	-	
Mov Cap-1 Maneuver	319	329	615	318	335	794	1115			-	1303	-	-	
Mov Cap-2 Maneuver	427	420	-	422	423	-	-			-	-	-	-	
Stage 1	564	550	-	731	681	-	-			-	-	-	-	
Stage 2	717	670	-	550	549	-	-			-	-	-	-	
Approach	FB			WB			NB				SB			

Approach	FR	VVB	NR	SB	
HCM Control Delay, s	11.9	12.7	0.3	0.3	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR E	BLn1	WBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1115	-	-	536	500	1303	-	-	
HCM Lane V/C Ratio	0.01	-	-	0.03	0.065	0.013	-	-	
HCM Control Delay (s)	8.3	-	-	11.9	12.7	7.8	-	-	
HCM Lane LOS	А	-	-	В	В	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	0.1	0.2	0	-	-	

### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		1	et 👘		1	et 👘	
Traffic Vol, veh/h	5	0	10	30	0	15	5	315	10	5	420	5
Future Vol, veh/h	5	0	10	30	0	15	5	315	10	5	420	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage	,# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	6	2	2	6	2
Mvmt Flow	5	0	11	33	0	16	5	342	11	5	457	5

Major/Minor	Minor2			Minor1		[	Major1		[	Vajor2			
Conflicting Flow All	836	833	460	833	830	348	462	0	0	353	0	0	
Stage 1	470	470	-	358	358	-	-	-	-	-	-	-	
Stage 2	366	363	-	475	472	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	287	304	601	288	306	695	1099	-	-	1206	-	-	
Stage 1	574	560	-	660	628	-	-	-	-	-	-	-	
Stage 2	653	625	-	570	559	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	278	301	601	281	303	695	1099	-	-	1206	-	-	
Mov Cap-2 Maneuver	400	404	-	400	405	-	-	-	-	-	-	-	
Stage 1	571	558	-	657	625	-	-	-	-	-	-	-	
Stage 2	635	622	-	557	557	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	12.2			13.6			0.1			0.1			

HCM Control Delay, s 12.2 13.6 HCM LOS B B

Minor Long/Major Mumt	MDI	NDT				CDI	СРТ	CDD
ivinor Lane/iviajor ivivmi	INBL	INRI	INRK	ERFUI	VBLUI	SRF	SRI	SBK
Capacity (veh/h)	1099	-	-	515	466	1206	-	-
HCM Lane V/C Ratio	0.005	-	-	0.032	0.105	0.005	-	-
HCM Control Delay (s)	8.3	-	-	12.2	13.6	8	-	-
HCM Lane LOS	А	-	-	В	В	А	-	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.3	0	-	-

## Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			÷		1	et P		1	el el	
Traffic Vol, veh/h	5	0	10	20	0	10	10	255	30	15	480	5
Future Vol, veh/h	5	0	10	20	0	10	10	255	30	15	480	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage,	,# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	5	0	11	22	0	11	11	277	33	16	522	5

Major/Minor	Minor2			Minor1			Major1			N	lajor2			
Conflicting Flow All	878	889	525	878	875	294	527	C	)	0	310	0	0	
Stage 1	557	557	-	316	316	-	-		-	-	-	-	-	
Stage 2	321	332	-	562	559	-	-		-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12		-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-		-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-		-	-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218		-	-	2.218	-	-	
Pot Cap-1 Maneuver	268	282	552	268	288	745	1040		-	-	1250	-	-	
Stage 1	515	512	-	695	655	-	-		-	-	-	-	-	
Stage 2	691	644	-	512	511	-	-		-	-	-	-	-	
Platoon blocked, %									-	-		-	-	
Mov Cap-1 Maneuver	259	275	552	258	281	745	1040		-	-	1250	-	-	
Mov Cap-2 Maneuver	377	377	-	371	379	-	-		-	-	-	-	-	
Stage 1	509	505	-	687	648	-	-		-	-	-	-	-	
Stage 2	674	637	-	495	504	-	-		-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	12.8	13.7	0.3	0.2	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1040	-	-	478	446	1250	-	-	
HCM Lane V/C Ratio	0.01	-	-	0.034	0.073	0.013	-	-	
HCM Control Delay (s)	8.5	-	-	12.8	13.7	7.9	-	-	
HCM Lane LOS	А	-	-	В	В	А	-	-	
HCM 95th %tile Q(veh)	0	-	-	0.1	0.2	0	-	-	









**Conceptual Site Plans** 



