# MEADOWBROOK PARK

## MEADOWBROOK PARKWAY EL PASO COUNTY, COLORADO

WATER RESOURCE REPORT

JULY 22<sup>ND</sup>, 2021

Prepared by:

Kimley »Horn

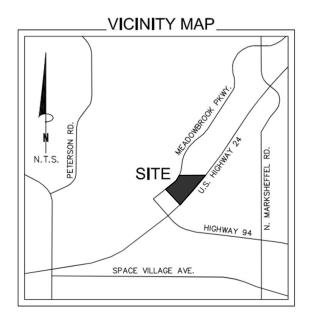
## TABLE OF CONTENTS

| SUMMARY OF THE PROPOSED SUBDIVISION                            | 3 |
|--|---|
| SITE LOCATION  | 3 |
| DESCRIPTION OF PROPERTY  | 3 |
| INFROMATION REGARDING SUFFICEINT QUANITY OF WATER              | 4 |
| CALCULATION OF WATER DEMAND                                    | 4 |
| CALCULATION OF QUANITY OF WATER AVAILABLE                      | 5 |
| INFORMATION REGUARDING SUFFICIENT DEPENDABLITY OF WATER SUPPLY | 5 |
| INFORMATION REGUARDING SUFFICIENT QUALITITY                    | 6 |
| PUBLIC AND RPIVATE COMMERICAL WATER PROVIDERS                  | 6 |
| REFERENCES   | 6 |

## SUMMARY OF THE PROPOSED SUBDIVISION

#### SITE LOCATION

The Site is located at the northeast corner of Meadowbrook Parkway and CO-24 and currently consists of Tract A of the 94/24 Business Park Filing No. 1 (the "Site"). More specifically, the Site is located in the southeast quarter of Section 8, Township 14 South, Range 65 West of the Sixth Principal Meridian, County of El Paso, State of Colorado. The Site is bounded by Meadowbrook Pkwy to the west, U.S. Highway 24 to the east, and existing commercial lots to the east and west. A vicinity map is provided below for reference:



#### DESCRIPTION OF PROPERTY

The overall site is approximately 8.010 acres of partially undeveloped land. The site development is anticipated to consist of 67 single family homes. Roadway infrastructure proposed within the site will provide access from the Project to adjacent right-of-way and access roadways. Project access will be obtained through Meadowbrook Pkwy.

A proposed street will run through the middle of the Project with two streets connecting to Meadowbrook Pkwy to provide a loop through the site for proficient emergency access.

The existing topography generally drains from east to west. The overall site varies in elevation from a low of approximately 6322 feet to a high of approximately 6353.

There are two points of connection for proposed water service to the Site. Both connections will be made off the 12-inch line within Meadowbrook Pkwy from the northeastern and northwestern corners of the Project. Refer to **Appendix A** for an overview of the water system and points of connection.

The water design presented herein will focus on the water demands anticipated with development of the Site.

## INFROMATION REGARDING SUFFICEINT QUANITY OF WATER

#### CALCULATION OF WATER DEMAND

The water system demands were based on a formal letter of Commitment sent by the CMD for Meadowbrook Park dated May 10<sup>th</sup>, 2021. See **Appendix A** for reference.

Demand Factors/Allowed Flows:

- Domestic Annual Water Demand
  - o 14.7 AF/yr
- Irrigation Annual Water Demand
  - o 3.3 AF/yr
- Average Day Demand
  - 0.159 GPM per Unit
  - Maximum Day Demand
    - 0.318 GPM per Unit

Based on this information, the domestic water and irrigation demand was calculated as follows:

| DOMESTIC WATER DEMAND CALCULATIONS |                  |           |                                  |                       |                          |                   |                   |
|------------------------------------|------------------|-----------|----------------------------------|-----------------------|--------------------------|-------------------|-------------------|
| Parcel                             | Building<br>Type | Lot Count | Estimated<br>Gross Floor<br>Area | Average Day<br>Demand | Average<br>Day<br>Demand | Max Day<br>Demand | Max Day<br>Demand |
|                                    | 51               | EA        | SF                               | GPM/Unit              | GPM                      | GPM/Unit          | GPM               |
| Area A                             | Residential      | 10        | 10,643                           | 0.159                 | 1.6                      | 0.318             | 3.2               |
| Area B                             | Residential      | 5         | 5,321                            | 0.159                 | 0.8                      | 0.318             | 1.6               |
| Area C                             | Residential      | 8         | 8,514                            | 0.159                 | 1.3                      | 0.318             | 2.5               |
| Area D                             | Residential      | 15        | 15,964                           | 0.159                 | 2.4                      | 0.318             | 4.8               |
| Area E                             | Residential      | 13        | 13,835                           | 0.159                 | 2.1                      | 0.318             | 4.1               |
| Area F                             | Residential      | 16        | 17,028                           | 0.159                 | 2.5                      | 0.318             | 5.1               |
|                                    |                  | Total FI  | ow Rates:                        | Average Day           | 10.7                     | Max Day           | 21.3              |

Section 2.6 of the Colorado Springs Utilities (CSU) standards was used to analyze the proposed water system. CSU standards and Water distribution systems design scenarios is as follows:

- o Static Scenario
  - No demands on the system. Maximum pressure = 170 psi.
- o Average Day Scenario
  - Average demands on the system based on conversion listed above. Minimum pressure = 50 psi.
- Maximum Day + Fire Flow Scenario
  - fire flow demand of 2,500 gpm at each hydrant. Minimum pressure = 20 psi

Pipe Sizing Calculations:

- WaterCAD was used to size water mains.
- Minimum Diameter = 8 inches for water mains, 6 inches for hydrant laterals

The proposed water main will be tapping into existing water lines in two (2) locations. Both connections will be made off the 12-inch line within Meadowbrook Pkwy from the northeastern and northwestern corners of the Project.

The site falls within the CMD which uses groundwater for the water system. An HGL of 6505 was used to model the connections to the existing system based off the fire flow test provided in May 2021. The fire flow results can be reviewed in **Appendix A** The high and low proposed finish grades for the site are approximately 6342 and 6321. The full hydraulic analysis using WaterCAD can be reviewed in **Appendix A** of this report.

The system will have an average day demand of 11.1 GPM and a max day demand of 22.3 GPM based on the Criteria. Based on the results of the WaterCAD analysis, it is anticipated that the existing system has capacity for the proposed development.

The buildings within the development shall be constructed per the 2018 International Fire Code (IFC) and 2018 International Building Code (IBC), or most current code. The proposed buildings will require fire flows per the International Fire Code.

Water main design calculations and the WaterCAD pipe network Model are provided in Appendix A.

#### CALCULATION OF QUANITY OF WATER AVAILABLE

Cherokee Metro District has a "Water Provider Supplement to Water Resource Report for Meadowbrook Park" attached as a part of the Appendix. The supplemental information confirms the availability of water to service this project. This project will be served by Cherokee Metro District water mains only.

With 4,443.0 AFY of exportable supply and 4,211.3 AFY of commitments, CMD has a water balance of 231.7 AFY before the subject development. After commitment of 18.0 AFY to this development, the District will have 213.7 AFY remaining for additional commitments. Below is a table showing the district's water balance with the new development.

| Water Balance Before New Commitment | 231.7 AFY |
|-------------------------------------|-----------|
| New Commitment: Meadowbrook Park    | 18.0 AFY  |
| Water Balance Remaining             | 213.7 AFY |

# INFORMATION REGUARDING SUFFICIENT DEPENDABLITY OF WATER SUPPLY

Currently Cherokee Metro District serves approximately 7000 residential taps and 600 commercial taps in addition to bulk users in eastern El Paso County including Schriever Air Force Base and several small developments located along State Highway 94.

Cherokee Metro District water is sourced entirely from groundwater in two regions. The majority is recovered from the alluvial Upper Black Squirrel (UBS) Aquifer in eastern El Paso County via 20 wells. The remainder is sourced from two wells in deep bedrock aquifers in the northern part of the county on the "Sundance Ranch" property. Water from eight of the 20 wells in the eastern part of the county can only be used to serve a fixed list of customers. Water for the main service area of CMD comes only from the remaining 12 wells in UBS along with the two wells at the Sundance Ranch.

The supplement to the Water Resource Report provided by Cherokee Metro District provides a description of the water supply, calculations demonstrating quantity, and evidence of water system source.

#### INFORMATION REGUARDING SUFFICIENT WATER QUALITITY

Cherokee Metro District uses a water system based on groundwater sources and water is treated to conform to all Federal and State regulatory requirements. Additional information is provided in the providers supplement to this Report.

#### PUBLIC AND PRIVATE COMMERICAL WATER PROVIDERS

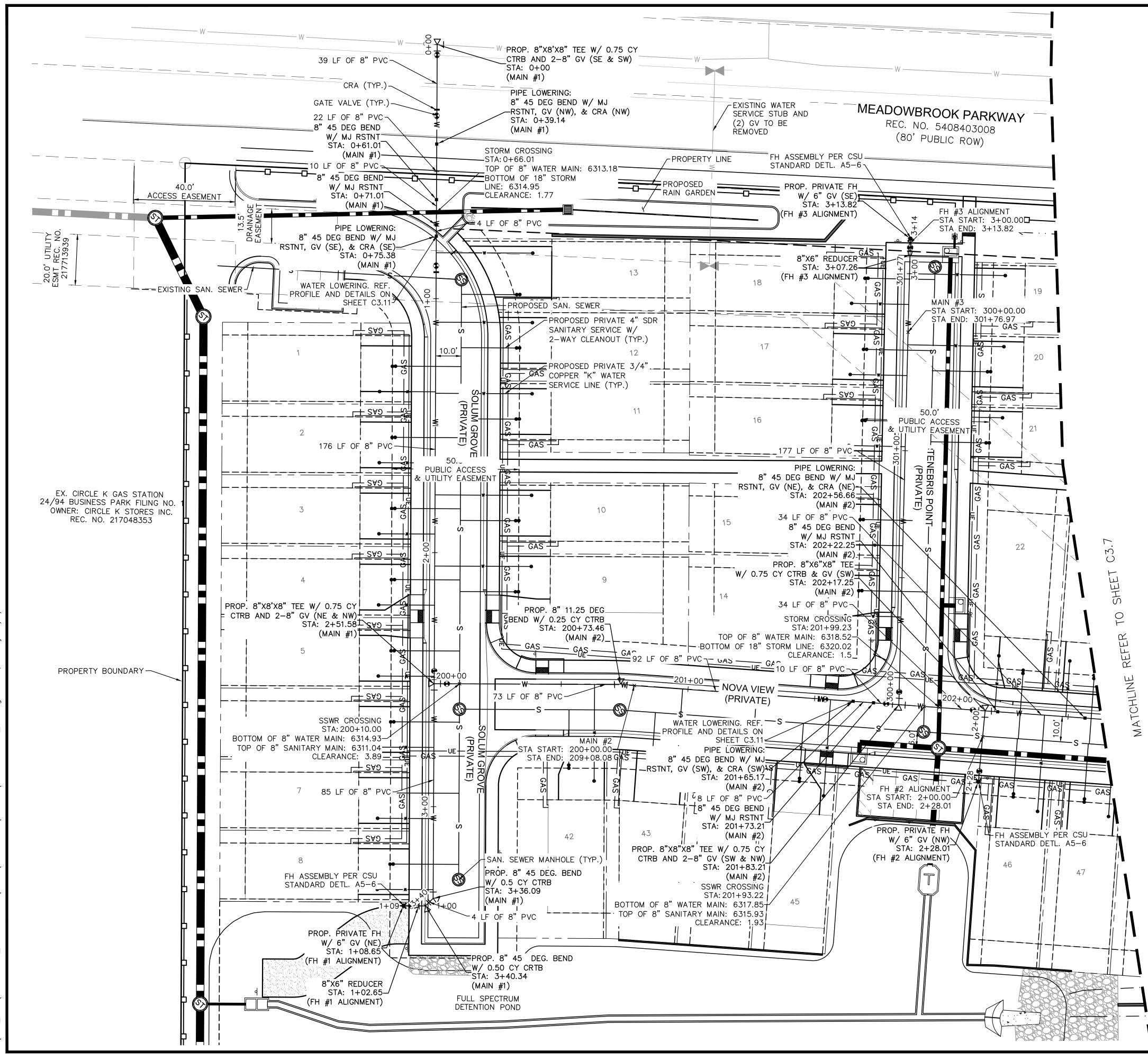
Cherokee Metro District has a "Water Provider Supplement to Water Resource Report for Meadowbrook Park" attached as a part of the Appendix. This supplement provides content that meets or exceeds the provided content provided in this Water Resource Report.

## REFERENCES

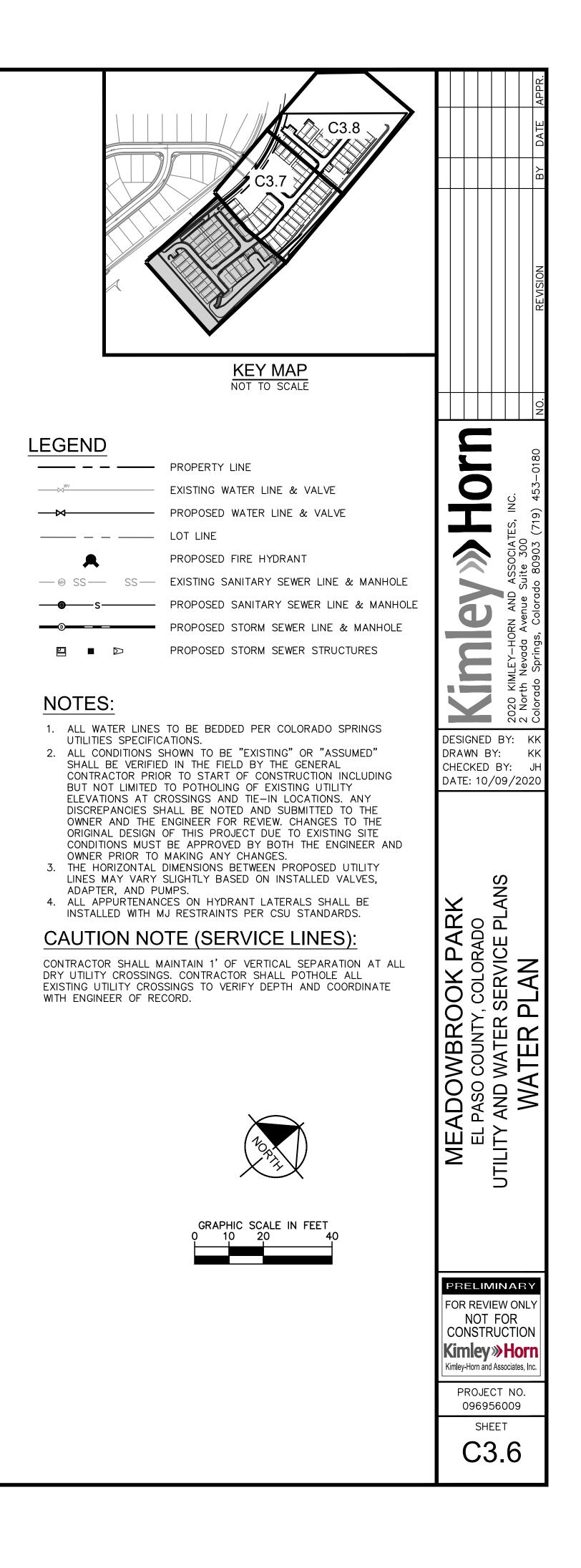
<u>Colorado Springs Utilities Water Line Extension & Service Standards 2019</u>, City of Colorado Springs; July 1, 2019.

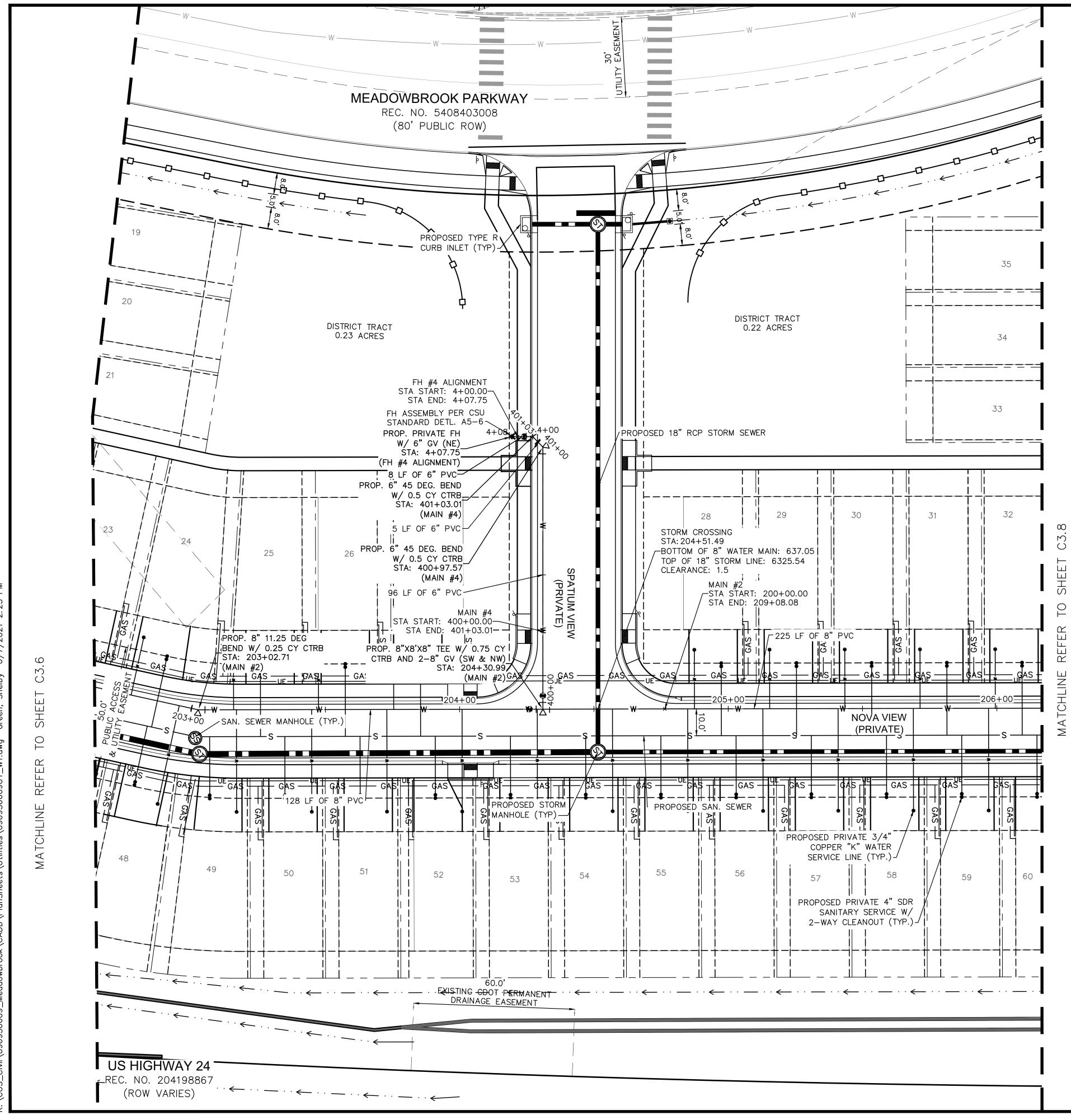
Cherokee Metropolitan District. "Water and Sewer Service to Meadowbrook Park. Commitment Letter No. 2021-07 (Revision of 2020-11)." Letter to Kevin Kofford. 6 May 2021.

APPENDIX A – WATER SCHEMATIC & COMPUTATIONS

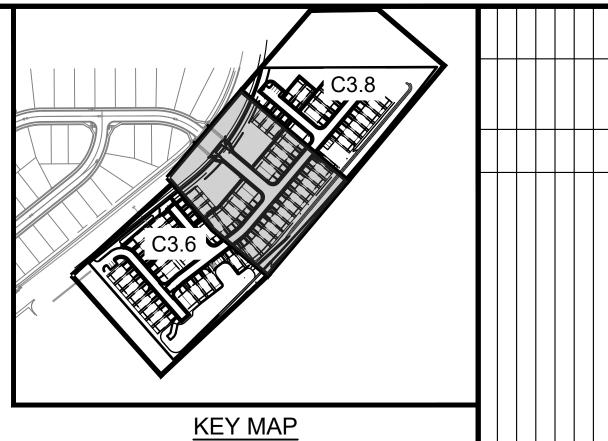


K: \COS\_Civil\096956009\_Meadowbrook\CADD\PlanSheets\Utilities\096956009UT\_WT.dwg Green, Shelby 6/7/2021 2:25









NOT TO SCALE

## LEGEND

| <br>⋈         |  |
|---------------|--|
| ♣<br>— ⊕ SS — |  |
| S−S−          |  |
|               |  |

PROPERTY LINE EXISTING WATER LINE & VALVE PROPOSED WATER LINE & VALVE PROPOSED FIRE HYDRANT EXISTING SANITARY SEWER LINE & MANHOLE PROPOSED SANITARY SEWER LINE & MANHOLE PROPOSED STORM SEWER STRUCTURES

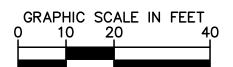
## NOTES:

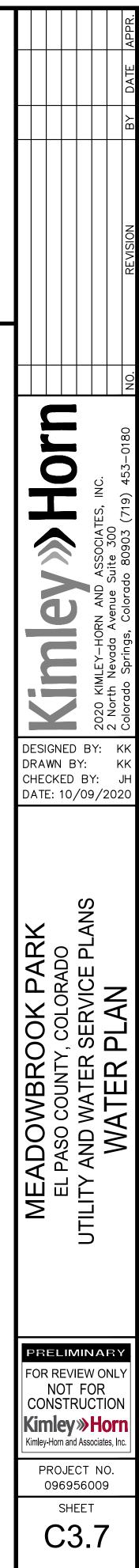
- 1. ALL WATER LINES TO BE BEDDED PER COLORADO SPRINGS UTILITIES SPECIFICATIONS.
- 2. ALL CONDITIONS SHOWN TO BE "EXISTING" OR "ASSUMED" SHALL BE VERIFIED IN THE FIELD BY THE GENERAL CONTRACTOR PRIOR TO START OF CONSTRUCTION INCLUDING BUT NOT LIMITED TO POTHOLING OF EXISTING UTILITY ELEVATIONS AT CROSSINGS AND TIE-IN LOCATIONS. ANY DISCREPANCIES SHALL BE NOTED AND SUBMITTED TO THE OWNER AND THE ENGINEER FOR REVIEW. CHANGES TO THE ORIGINAL DESIGN OF THIS PROJECT DUE TO EXISTING SITE CONDITIONS MUST BE APPROVED BY BOTH THE ENGINEER AND OWNER PRIOR TO MAKING ANY CHANGES.
- 3. THE HORIZONTAL DIMENSIONS BETWEEN PROPOSED UTILITY LINES MAY VARY SLIGHTLY BASED ON INSTALLED VALVES, ADAPTER, AND PUMPS.
- 4. ALL APPURTENANCES ON HYDRANT LATERALS SHALL BE INSTALLED WITH MJ RESTRAINTS PER CSU STANDARDS.

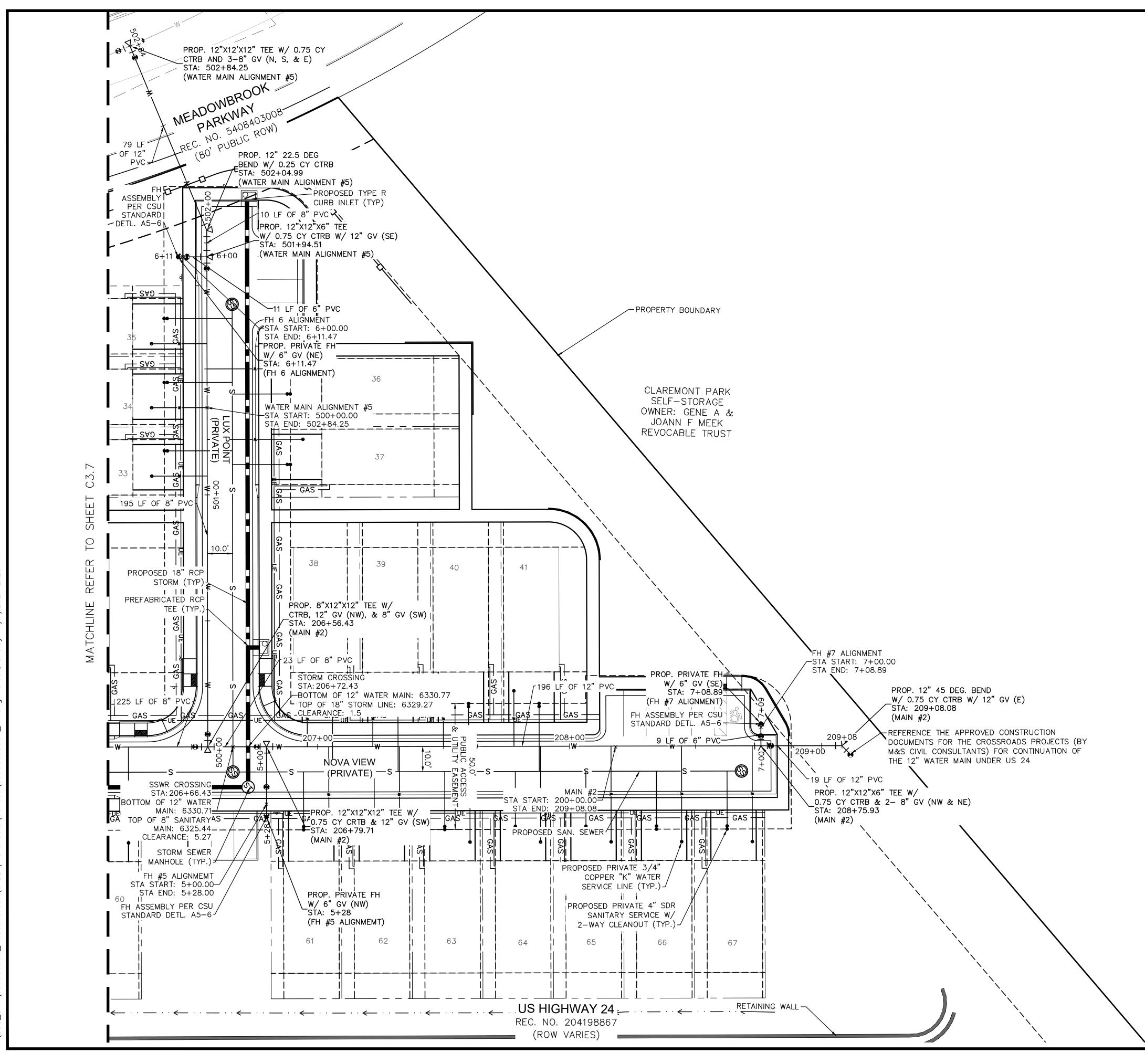
## CAUTION NOTE (SERVICE LINES):

CONTRACTOR SHALL MAINTAIN 1' OF VERTICAL SEPARATION AT ALL DRY UTILITY CROSSINGS. CONTRACTOR SHALL POTHOLE ALL EXISTING UTILITY CROSSINGS TO VERIFY DEPTH AND COORDINATE WITH ENGINEER OF RECORD.

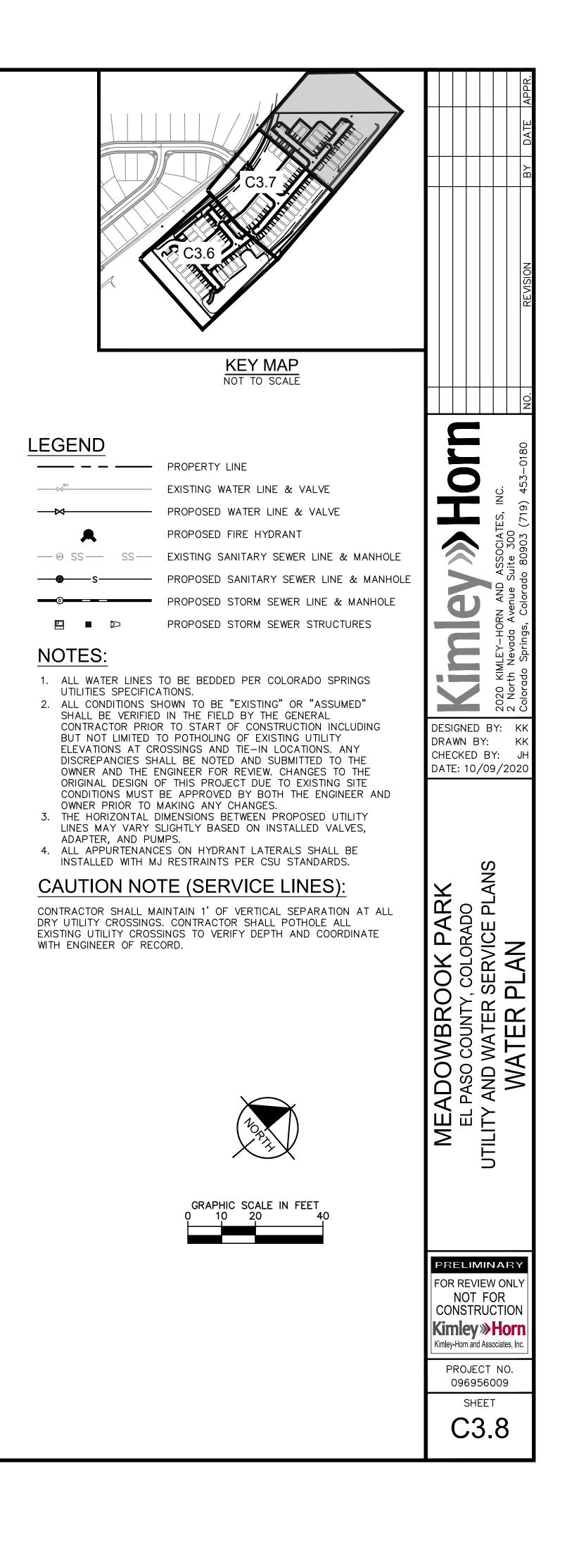








(:) COS Civil\096956009 Meadowbrook\CADD\PlanSheets\Utilities\096956009UT WT.dwa Green. Shelbv 6/7/2021 2:2



|        | DOMESTIC WATER DEMAND CALCULATIONS |                   |                               |                       |                       |                   |                   |  |
|--------|------------------------------------|-------------------|-------------------------------|-----------------------|-----------------------|-------------------|-------------------|--|
| Parcel | Building Type                      | Lot Count         | Estimated Gross<br>Floor Area | Average Day<br>Demand | Average Day<br>Demand | Max Day<br>Demand | Max Day<br>Demand |  |
|        | EA                                 | SF                | GPM/Unit                      | GPM                   | GPM/Unit              | GPM               |                   |  |
| Area A | Residential                        | 10                | 10,643                        | 0.159                 | 1.6                   | 0.318             | 3.2               |  |
| Area B | Residential                        | 5                 | 5,321                         | 0.159                 | 0.8                   | 0.318             | 1.6               |  |
| Area C | Residential                        | 8                 | 8,514                         | 0.159                 | 1.3                   | 0.318             | 2.5               |  |
| Area D | Residential                        | 15                | 15,964                        | 0.159                 | 2.4                   | 0.318             | 4.8               |  |
| Area E | Residential                        | 13                | 13,835                        | 0.159                 | 2.1                   | 0.318             | 4.1               |  |
| Area F | Residential                        | 16                | 17,028                        | 0.159                 | 2.5                   | 0.318             | 5.1               |  |
|        |                                    | Total Flow Rates: |                               | Average Day           | 10.7                  | Max Day           | 21.3              |  |

### **Static Scenario**

| 1 - 1 - 1    |                  | Matailal     |                          | <b>V</b> 1 1       |                  |
|--------------|------------------|--------------|--------------------------|--------------------|------------------|
| Label        | Diameter<br>(in) | Material     | Flow (Absolute)<br>(gpm) | Velocity<br>(ft/s) | Headloss<br>(ft) |
| P-15         | 6.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-15<br>P-21 | 6.0              | Ductile Iron | 0.0                      | 0.00               | 0.00             |
| P-11         | 6.0              | Ductile Iron | 0.0                      | 0.00               | 0.00             |
| P-23         | 6.0              | Ductile Iron | 0.0                      | 0.00               | 0.00             |
| P-24         | 6.0              | Ductile Iron | 0.0                      | 0.00               | 0.00             |
| P-25         | 6.0              | Ductile Iron | 0.0                      | 0.00               | 0.00             |
| P-26         | 6.0              | Ductile Iron | 0.0                      | 0.00               | 0.00             |
| P-29         | 6.0              | Ductile Iron | 0.0                      | 0.00               | 0.00             |
| P-28         | 6.0              | Ductile Iron | 0.0                      | 0.00               | 0.00             |
| P-5          | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-6          | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-7          | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-8          | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-9          | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-10         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-12         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-13         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-14         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-16         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-17         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-18         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-19         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-20         | 8.0              | PVC          | 0.0                      | 0.00               | 0.00             |
| P-1          | 12.0             | PVC          | 0.0                      | 0.00               | 0.00             |
| P-2          | 12.0             | PVC          | 0.0                      | 0.00               | 0.00             |
| P-3          | 12.0             | PVC          | 0.0                      | 0.00               | 0.00             |
| P-27         | 12.0             | PVC          | 0.0                      | 0.00               | 0.00             |
| P-28         | 12.0             | PVC          | 0.0                      | 0.00               | 0.00             |
| P-29         | 24.0             | PVC          | 0.0                      | 0.00               | 0.00             |
| P-30         | 24.0             | PVC          | 0.0                      | 0.00               | 0.00             |

#### Pipe Table - Time: 0.00 hours

Meadowbrook 5/18/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 WaterCAD [10.03.01.08] Page 1 of 2

### **Static Scenario**

| Label  | Elevation<br>(ft) | Demand<br>(gpm) | Hydraulic<br>Grade<br>(ft) | Pressure<br>(psi) | Concentration<br>(Calculated)<br>(mg/L) |
|--------|-------------------|-----------------|----------------------------|-------------------|---|
| J-8    | 6,314.47          | 0.0             | 6,505.00                   | 82                | (N/A)                                   |
| J-5    | 6,315.20          | 0.0             | 6,505.00                   | 82                | (N/A)                                   |
| J-7    | 6,315.30          | 0.0             | 6,505.00                   | 82                | (N/A)                                   |
| J-10   | 6,315.91          | 0.0             | 6,505.00                   | 82                | (N/A)                                   |
| J-1    | 6,316.00          | 0.0             | 6,505.00                   | 82                | (N/A)                                   |
| AREA A | 6,316.84          | 0.0             | 6,505.00                   | 81                | (N/A)                                   |
| J-6    | 6,317.07          | 0.0             | 6,505.00                   | 81                | (N/A)                                   |
| J-2    | 6,317.88          | 0.0             | 6,505.00                   | 81                | (N/A)                                   |
| J-3    | 6,318.02          | 0.0             | 6,505.00                   | 81                | (N/A)                                   |
| AREA B | 6,319.76          | 0.0             | 6,505.00                   | 80                | (N/A)                                   |
| AREA C | 6,321.79          | 0.0             | 6,505.00                   | 79                | (N/A)                                   |
| AREA D | 6,323.42          | 0.0             | 6,505.00                   | 79                | (N/A)                                   |
| J-16   | 6,325.49          | 0.0             | 6,505.00                   | 78                | (N/A)                                   |
| J-15   | 6,327.22          | 0.0             | 6,505.00                   | 77                | (N/A)                                   |
| J-4    | 6,329.91          | 0.0             | 6,505.00                   | 76                | (N/A)                                   |
| J-20   | 6,330.00          | 0.0             | 6,505.00                   | 76                | (N/A)                                   |
| J-21   | 6,330.35          | 0.0             | 6,505.00                   | 76                | (N/A)                                   |
| AREA E | 6,331.01          | 0.0             | 6,505.00                   | 75                | (N/A)                                   |
| J-19   | 6,331.22          | 0.0             | 6,505.00                   | 75                | (N/A)                                   |
| AREA F | 6,337.12          | 0.0             | 6,505.00                   | 73                | (N/A)                                   |

#### Junction Table - Time: 0.00 hours

#### Reservoir Table - Time: 0.00 hours

| Label | Elevation<br>(ft) | Flow (Out net)<br>(gpm) | Hydraulic Grade<br>(ft) |
|-------|-------------------|-------------------------|-------------------------|
| R-1   | 6,505.00          | 0.0                     | 6,505.00                |
| R-2   | 6,505.00          | 0.0                     | 6,505.00                |

### **Average Day Scenario**

| Label  | Elevation<br>(ft) | Demand<br>(gpm) | Hydraulic<br>Grade<br>(ft) | Pressure<br>(psi) |
|--------|-------------------|-----------------|----------------------------|-------------------|
| AREA F | 6,337.12          | 2.5             | 6,505.00                   | 73                |
| J-19   | 6,331.22          | 0.0             | 6,505.00                   | 75                |
| AREA E | 6,331.01          | 2.1             | 6,505.00                   | 75                |
| J-21   | 6,330.35          | 0.0             | 6,505.00                   | 76                |
| J-20   | 6,330.00          | 0.0             | 6,505.00                   | 76                |
| J-4    | 6,329.91          | 0.0             | 6,505.00                   | 76                |
| J-15   | 6,327.22          | 0.0             | 6,505.00                   | 77                |
| J-16   | 6,325.49          | 0.0             | 6,505.00                   | 78                |
| AREA D | 6,323.42          | 2.4             | 6,505.00                   | 79                |
| AREA C | 6,321.79          | 1.3             | 6,505.00                   | 79                |
| AREA B | 6,319.76          | 0.8             | 6,505.00                   | 80                |
| J-3    | 6,318.02          | 0.0             | 6,505.00                   | 81                |
| J-2    | 6,317.88          | 0.0             | 6,505.00                   | 81                |
| J-6    | 6,317.07          | 0.0             | 6,505.00                   | 81                |
| AREA A | 6,316.84          | 1.6             | 6,505.00                   | 81                |
| J-1    | 6,316.00          | 0.0             | 6,505.00                   | 82                |
| J-10   | 6,315.91          | 0.0             | 6,505.00                   | 82                |
| J-7    | 6,315.30          | 0.0             | 6,505.00                   | 82                |
| J-5    | 6,315.20          | 0.0             | 6,505.00                   | 82                |
| J-8    | 6,314.47          | 0.0             | 6,505.00                   | 82                |

#### Junction Table - Time: 0.00 hours

Meadowbrook 7/2/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 WaterCAD [10.03.01.08] Page 1 of 2

## Average Day Scenario

| Label | Diameter | Material     | Flow (Absolute) | Velocity | Headloss |
|-------|----------|--------------|-----------------|----------|----------|
|       | (in)     |              | (gpm)           | (ft/s)   | (ft)     |
| P-15  | 6.0      | PVC          | 0.0             | 0.00     | 0.00     |
| P-21  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-11  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-23  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-24  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-25  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-26  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-29  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-28  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-5   | 8.0      | PVC          | 4.8             | 0.03     | 0.00     |
| P-6   | 8.0      | PVC          | 4.8             | 0.03     | 0.00     |
| P-7   | 8.0      | PVC          | 4.8             | 0.03     | 0.00     |
| P-8   | 8.0      | PVC          | 4.8             | 0.03     | 0.00     |
| P-9   | 8.0      | PVC          | 1.6             | 0.01     | 0.00     |
| P-10  | 8.0      | PVC          | 3.2             | 0.02     | 0.00     |
| P-12  | 8.0      | PVC          | 1.3             | 0.01     | 0.00     |
| P-13  | 8.0      | PVC          | 1.1             | 0.01     | 0.00     |
| P-14  | 8.0      | PVC          | 1.3             | 0.01     | 0.00     |
| P-16  | 8.0      | PVC          | 1.3             | 0.01     | 0.00     |
| P-17  | 8.0      | PVC          | 3.4             | 0.02     | 0.00     |
| P-18  | 8.0      | PVC          | 2.5             | 0.02     | 0.00     |
| P-19  | 8.0      | PVC          | 5.9             | 0.04     | 0.00     |
| P-20  | 8.0      | PVC          | 5.9             | 0.04     | 0.00     |
| P-1   | 12.0     | PVC          | 4.0             | 0.01     | 0.00     |
| P-2   | 12.0     | PVC          | 0.8             | 0.00     | 0.00     |
| P-3   | 12.0     | PVC          | 0.8             | 0.00     | 0.00     |
| P-27  | 12.0     | PVC          | 0.8             | 0.00     | 0.00     |
| P-28  | 12.0     | PVC          | 6.7             | 0.02     | 0.00     |
| P-29  | 24.0     | PVC          | 6.7             | 0.00     | 0.00     |
| P-30  | 24.0     | PVC          | 4.0             | 0.00     | 0.00     |

#### Pipe Table - Time: 0.00 hours

#### **Reservoir Table - Time: 0.00 hours**

| Label | Elevation<br>(ft) | Flow (Out net)<br>(gpm) | Hydraulic Grade<br>(ft) |
|-------|-------------------|-------------------------|-------------------------|
| R-1   | 6,505.00          | 4.0                     | 6,505.00                |
| R-2   | 6,505.00          | 6.7                     | 6,505.00                |

### **Max Day Scenario**

| Label  | Elevation<br>(ft) | Demand<br>(gpm) | Hydraulic<br>Grade<br>(ft) | Pressure<br>(psi) |
|--------|-------------------|-----------------|----------------------------|-------------------|
| AREA F | 6,337.12          | 5.1             | 6,505.00                   | 73                |
| J-19   | 6,331.22          | 0.0             | 6,505.00                   | 75                |
| AREA E | 6,331.01          | 4.1             | 6,505.00                   | 75                |
| J-21   | 6,330.35          | 0.0             | 6,505.00                   | 76                |
| J-20   | 6,330.00          | 0.0             | 6,505.00                   | 76                |
| J-4    | 6,329.91          | 0.0             | 6,505.00                   | 76                |
| J-15   | 6,327.22          | 0.0             | 6,505.00                   | 77                |
| J-16   | 6,325.49          | 0.0             | 6,505.00                   | 78                |
| AREA D | 6,323.42          | 4.8             | 6,505.00                   | 79                |
| AREA C | 6,321.79          | 2.5             | 6,505.00                   | 79                |
| AREA B | 6,319.76          | 1.6             | 6,505.00                   | 80                |
| J-3    | 6,318.02          | 0.0             | 6,505.00                   | 81                |
| J-2    | 6,317.88          | 0.0             | 6,505.00                   | 81                |
| J-6    | 6,317.07          | 0.0             | 6,505.00                   | 81                |
| AREA A | 6,316.84          | 3.2             | 6,505.00                   | 81                |
| J-1    | 6,316.00          | 0.0             | 6,505.00                   | 82                |
| J-10   | 6,315.91          | 0.0             | 6,505.00                   | 82                |
| J-7    | 6,315.30          | 0.0             | 6,505.00                   | 82                |
| J-5    | 6,315.20          | 0.0             | 6,505.00                   | 82                |
| J-8    | 6,314.47          | 0.0             | 6,505.00                   | 82                |

Junction Table - Time: 0.00 hours

Meadowbrook 7/2/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 WaterCAD [10.03.01.08] Page 1 of 2

## Max Day Scenario

| Label | Diameter | Material     | Flow (Absolute) | Velocity | Headloss |
|-------|----------|--------------|-----------------|----------|----------|
| Label | (in)     | Material     | (gpm)           | (ft/s)   | (ft)     |
| P-15  | 6.0      | PVC          | 0.0             | 0.00     | 0.00     |
| P-21  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-11  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-23  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-24  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-25  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-26  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-29  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-28  | 6.0      | Ductile Iron | 0.0             | 0.00     | 0.00     |
| P-5   | 8.0      | PVC          | 9.6             | 0.06     | 0.00     |
| P-6   | 8.0      | PVC          | 9.6             | 0.06     | 0.00     |
| P-7   | 8.0      | PVC          | 9.6             | 0.06     | 0.00     |
| P-8   | 8.0      | PVC          | 9.6             | 0.06     | 0.00     |
| P-9   | 8.0      | PVC          | 3.2             | 0.02     | 0.00     |
| P-10  | 8.0      | PVC          | 6.4             | 0.04     | 0.00     |
| P-12  | 8.0      | PVC          | 2.5             | 0.02     | 0.00     |
| P-13  | 8.0      | PVC          | 2.3             | 0.01     | 0.00     |
| P-14  | 8.0      | PVC          | 2.5             | 0.02     | 0.00     |
| P-16  | 8.0      | PVC          | 2.5             | 0.02     | 0.00     |
| P-17  | 8.0      | PVC          | 6.6             | 0.04     | 0.00     |
| P-18  | 8.0      | PVC          | 5.1             | 0.03     | 0.00     |
| P-19  | 8.0      | PVC          | 11.7            | 0.07     | 0.00     |
| P-20  | 8.0      | PVC          | 11.7            | 0.07     | 0.00     |
| P-1   | 12.0     | PVC          | 7.9             | 0.02     | 0.00     |
| P-2   | 12.0     | PVC          | 1.7             | 0.00     | 0.00     |
| P-3   | 12.0     | PVC          | 1.7             | 0.00     | 0.00     |
| P-27  | 12.0     | PVC          | 1.7             | 0.00     | 0.00     |
| P-28  | 12.0     | PVC          | 13.4            | 0.04     | 0.00     |
| P-29  | 24.0     | PVC          | 13.4            | 0.01     | 0.00     |
| P-30  | 24.0     | PVC          | 7.9             | 0.01     | 0.00     |

#### Pipe Table - Time: 0.00 hours

#### **Reservoir Table - Time: 0.00 hours**

| Label | Elevation<br>(ft) | Flow (Out net)<br>(gpm) | Hydraulic Grade<br>(ft) |
|-------|-------------------|-------------------------|-------------------------|
| R-1   | 6,505.00          | 7.9                     | 6,505.00                |
| R-2   | 6,505.00          | 13.4                    | 6,505.00                |

## Fire Flow Results

| Label | Demand<br>(gpm) | Satisfies Fire<br>Flow<br>Constraints? | Fire Flow<br>(Needed)<br>(gpm) | Fire Flow<br>(Available)<br>(gpm) | Pressure<br>(Calculated<br>Residual)<br>(psi) | Junction w/<br>Minimum<br>Pressure<br>(Zone) |
|-------|-----------------|--|--------------------------------|-----------------------------------|---|--|
| H-1   | 0.0             | True                                   | 0.0                            | 2,501.0                           | 74  | H-5  |
| H-2   | 0.0             | True                                   | 0.0                            | 2,501.0                           | 77  | H-5  |
| H-3   | 0.0             | True                                   | 0.0                            | 2,501.0                           | 56  | J-16   |
| H-4   | 0.0             | True                                   | 0.0                            | 2,501.0                           | 65  | H-5  |
| H-5   | 0.0             | True                                   | 0.0                            | 2,501.0                           | 59  | AREA F                                       |
| H-6   | 0.0             | True                                   | 0.0                            | 2,501.0                           | 72  | H-5  |
| H-7   | 0.0             | True                                   | 0.0                            | 2,501.0                           | 73  | H-5  |
| H-8   | 0.0             | True                                   | 0.0                            | 2,501.0                           | 69  | H-5  |

#### Fire Flow Report - Time: 0.00 hours

Meadowbrook 7/2/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 WaterCAD [10.03.01.08] Page 1 of 1



## **CHEROKEE METROPOLITAN DISTRICT** 6250 Palmer Park Blvd., Colorado Springs, CO 80915-2842 Telephone: (719) 597-5080 Fax: (719) 597-5145

May 6<sup>th</sup>, 2021 Danny Mientka The Equity Group 90 S Cascade Avenue, Suite 1500 Colorado Springs, CO 80903

Sent via email: kevin.kofford@kimley-horn.com

Re: Water and Sewer Service to Meadowbrook Park Commitment Letter No. 2021-07 (Revision of 2020-11)

Dear Danny Mientka,

As requested, this document will serve is as a formal Letter of Commitment from the Cherokee Metropolitan District to provide municipal water and sewer services Meadowbrook Park located at the North corner of U.S. Highway 24 and State Highway 94. The location for this proposed development is located within the District's established boundaries and therefore is eligible for service connections from the District.

Cherokee Metropolitan District staff, along with the developer, have determined that the following will be the total water demand required by this development:

| Type of Use | Demand (AF/yr) |
|-------------|----------------|
| Domestic    | 14.7           |
| Irrigation  | 3.3            |
| Total       | 18.0           |

Based on a conservatively low 0% consumptive use of domestic water, the development is expected to produce 13,160 gallons of wastewater per day, representing 0.5% of CMD's wastewater capacity. This is in line with anticipated wastewater demand for this area of the District.

This water and wastewater commitment is hereby made exclusively for this specific development project at this site within the District. To confirm this commitment you must provide the District with a copy of the final plat approval from El Paso County Development Services within 12 months of the date of this letter. Otherwise, the District may use this allocation for other developments requesting a water commitment. If the subject project is re-platted, you must submit a new commitment request

prior to submitting the re-plat to El Paso County, which may result in a recalculation of the water demand for the project.

If I may be of further assistance please contact me at your convenience.

Sincerely,

Amy Lather General Manager

Cc: Peter Johnson; Water Counsel w/ encl: sent via email Steve Hasbrouck; Board President w/ encl: sent via email Jeff Munger, Water Resource Engineer Kevin Brown; Jr. Water Resource Engineer

### **FIRE FLOW TEST**

DATE <u>5/12/21</u> TIME <u>1400</u>

LOCATION Meadowbrook Pkwy

TANK LEVEL 19.75 ft

BOOSTER RUNNING <u>1825</u> GPM

RESIDUAL HYDRANT NUMBER: N/A LOCATION: Newt Dr. & Meadowbrook Pkwy 38.844477, -104.692405

STATIC LINE PRESSURE <u>110</u> PSI

DYNAMIC LINE PRESSURE 82 PSI

FLOW HYDRANT NUMBER: 2234-3 LOCATION: Meadowbrook Pkwy 38.845232, -104.691159

Flow/Pressure READING: GPM 2040 PSI 29.5

Results taken from gauge on the Big Boy Pitot-less Hose Monster

TAKEN BY: A. Wenger; K. Love

**REVIEWED BY: Nicholas Griffin** 

RESULTS SENT TO: Kevin.Kofford@kimley-horn.com



## **CHEROKEE METROPOLITAN DISTRICT**

6250 Palmer Park Blvd., Colorado Springs, CO 80915-2842 Telephone: (719) 597-5080 Fax: (719) 597-5145

## Water Provider Supplement to Water Resource Report for Meadowbrook park

May 10<sup>th</sup>, 2021

Commitment 2021-07

This document has been prepared to satisfy El Paso County's requirement of a Water Provider's Supplement in support of Meadowbrook Park at the north corner of U.S. Highway 24 and State Highway 94

### Introduction

Cherokee Metropolitan District (CMD) is a Title 32 special District which provides water and wastewater to a 5000-acre enclave of unincorporated El Paso county surrounded by the City of Colorado Springs. Currently CMD serves approximately 7000 residential taps and 600 commercial taps in addition to bulk users in eastern El Paso County including Schriever Air Force Base and several small developments located along State Highway 94.

CMD water is sourced entirely from groundwater in two regions. The majority is recovered from the alluvial Upper Black Squirrel (UBS) Aquifer in eastern El Paso County via 20 wells. The remainder is sourced from two wells in deep bedrock aquifers in the northern part of the county on the "Sundance Ranch" property. Water from eight of the 20 wells in the eastern part of the county can only be used to serve a fixed list of customers. Water for the main service area of CMD comes only from the remaining 12 wells in UBS along with the two wells at the Sundance Ranch.

## **Calculation of Anticipated Water Demand**

The subdivision will include 67 residential lots with 0.88 acres of fully irrigated common space and 1.07 acres of 60% reduced watering native grass. This development will have 3000 square foot lots with limited opportunities for on-lot landscaping. Based on billing records of similar small lot developments in the District and El Paso County, a lower presumptive water use value is applicable than with traditional single family subdivisions.

The District estimates a presumptive use value of 0.22 Acre-Feet per Year (AFY) per Single Family Equivalent (SFE) as a conservative estimate for actual water use in these small lot developments. Actual small lot single family use is close to 0.20 AFY even in low precipitation years and this higher value accounts for distribution system losses. 67 lots at 0.22 AFY per lot yields 14.74 AFY for domestic and inside lot irrigation use.

Water demand for the 1.95 acres of common space was calculated using El Paso County's fully irrigated landscaping estimate of 2.43 feet of water per year. Applying this to the 0.88 acres of fully irrigated common space and the 1.07 acres of 60% reduced watering common space yields an irrigation use estimate of 3.18 AFY. The total for irrigation and domestic use across the development is projected to be 18.0 AFY.

## Water Supplies

Cherokee has eight wells (numbered 1-8) that are restricted to serving a maximum of 653 AFY to a fixed list of customers within the Upper Black Squirrel Creek Designated Basin (the Basin). Excess allocation for these wells is unavailable for new developments, even if those developments are located inside the Basin, so this water is tracked separately from CMD's general exportable supply portfolio. Water from CMD's other alluvial wells is exported for use

outside the UBS basin. The total annual volume available to CMD from these exportable supplies is 3,985 Acre-Feet per Year (AFY) (Table 1). The physical yield of these wells is significantly higher than their annual appropriation, allowing for flexibility in satisfying irrigation season demand.

| Well<br>Number | Water Right<br>(AFY) | 2019 Use<br>(AFY) | Permit Number | Aquifer          | Aquifer Status |
|----------------|----------------------|-------------------|---------------|------------------|----------------|
| Well 9         | 176                  | 132               | 14145-FP-R    | UBS Alluvium     | Tributary      |
| Well 10        | 176                  | 108               | 14146-FP-R    | UBS Alluvium     | Tributary      |
| Well 11        | 244                  | 161               | 6821-FP-R     | UBS Alluvium     | Tributary      |
| Well 12        | 244                  | 149               | 11198-FP      | UBS Alluvium     | Tributary      |
| Well 13        | 1268                 | 975               | 49988-F       | UBS Alluvium     | Tributary      |
| Well 14        | 0                    | 0                 | 52429-F       | UBS Alluvium     | Tributary      |
| Well 15*       | 281                  | 145               | 54070-F       | UBS Alluvium     | Tributary      |
| Well 16*       | 219                  | 123               | 54069-F       | UBS Alluvium     | Tributary      |
| Well 17*       | 175                  | 151               | 63094-F       | UBS Alluvium     | Tributary      |
| Well 18        | 225                  | 138               | 16253-RFP-R   | UBS Alluvium     | Tributary      |
| Well 19        | 95                   | 79                | 20567-RFP-R   | UBS Alluvium     | Tributary      |
| Well 20        | 400                  | 38                | 4332-RFP      | UBS Alluvium     | Tributary      |
| Well 21        | 290                  | 0                 | 81782-F       | UBS Alluvium     | Tributary      |
| DN-4**         | 110                  | 110               | 78315-F       | Denver Aquifer   | Non-Tributary  |
| AR-1***        | 147.7                | 155               | 75881-F       | Arapahoe Aquifer | Non-Tributary  |
| Total          | 3984.7               | 2464              |               |                  |                |

**Table 1:** Water rights and tributary status of Exportable Wells

\*Wells 15, 16, and 17 can produce a combined 609 AFY despite their total individual allocations equaling 675 AFY. This reduction is reflected in the total.

\*\*CMD holds additional water rights in the Denver Aquifer associated with the Sundance Ranch property but this particular well has a maximum annual recorded yield of 110 AFY

\*\*\*As of December 2019 AR-1 has 2040 AF of banked water which allows actual pumping to exceed allocation on a limited basis

CMD is developing owned water supplies to increase available water and improve flexibility in provision of summer flows. By the end of 2021, these new wells will contribute 458 AFY of capacity to the CMD system (Table 2) for a total of 4,443.0 AFY. Since 2011, actual demand from CMD customers has fallen 30-35% below commitments, partially due to some committed developments being incomplete but largely due to water saving measures undertaken by CMD customers.

| Well Number    | Water Right | Permit   | Aquifer      | Aquifer Status    |
|----------------|-------------|----------|--------------|-------------------|
|                | (AFY)       | Number   |              |                   |
| Albrecht Well  | 153.5       | 27571-FP | UBS Alluvium | Tributary         |
| DA-1           | 40.3        | 83604-F  | Dawson       | Not Non-Tributary |
| DA-4           | 64.5        | 83603-F  | Dawson       | Not Non-Tributary |
| AR-1 Expansion | 200         | 75881-F  | Arapahoe     | Non-Tributary     |
| Total          | 458.3       |          |              |                   |

**Table 2:** New water supplies slated for completion in 2021

By the end of 2021, CMD will have at total of 4,443 AFY of exportable water supplies sourced from alluvial and deep bedrock aquifers. Further development in the Denver Basin is not planned at this time and instead CMD is focusing on acquiring new renewable supplies proximate to existing infrastructure.

## Water Commitments

CMD's water commitments stand at 4,211.3 AFY before the addition of the proposed development. These commitments are broken down below in Table 3. The Tipton and Kane commitments are related to an arrangement from the mid-2000's where developers reserved commitments on two new wells. The water from these wells is considered fully committed to these developers even if they have not yet begun the projects associated with the reserved commitments. Due to a complex legal history, the "Kane" water right was not tied to a specific physical water well but instead operates as a commitment served from CMD's general supply portfolio. The "Tipton" water right corresponds to CMD's Well 18.

| Table 3: CMD Commitme | ents before additio | n of new development |
|-----------------------|---------------------|----------------------|
|-----------------------|---------------------|----------------------|

| Commitments              | AFY    |
|--------------------------|--------|
| In-District (2015)       | 2693   |
| Committed Since 2015     | 506.3  |
| Schriever Air Force Base | 537    |
| Kane                     | 200    |
| Tipton                   | 225    |
| Construction             | 25     |
| Parks                    | 25     |
| Total                    | 4211.3 |

### **Water Balance**

With 4,443.0 AFY of exportable supply and 4,211.3 AFY of commitments, CMD has a water balance of 231.7 AFY before the subject development. After commitment of 18.0 AFY to this development, the District will have 213.7 AFY remaining for additional commitments.

**Table 4:** Water balance with new development

| Water Balance Before New Commitment    | 231.7 AFY |
|--|-----------|
| New Commitment: Windermere Subdivision | 18.0      |
| Water Balance Remaining                | 213.7 AFY |

### **Other Relevant District Information**

#### **Recent Water Acquisitions/Losses**

CMD has not acquired any new water rights since 2015 but has been developing owned water rights into production wells. CMD has not engaged in any water trades nor lost any water rights in the last year. The District is not currently under contract to purchase new water rights although CMD is investigating purchases of renewable water rights proximate to its existing infrastructure on an ongoing basis.

#### **New Augmentation Plans**

CMD is currently pursuing a replacement plan in partnership with Meridian Service Metropolitan District (MSMD) in order to maximize the efficiency of its water supplies.

#### **Major Water System Capital Improvements**

CMD has been actualizing owned water by drilling wells and beginning production on several well sites. In February 2020 CMD brought the Sweetwater 5 well (81782-F) online as well 21. In April 2021 CMD brought the Albrecht well (27571-FP-R) online as well 22. It is not yet connected to the overall system pending CDPHE approval.

CMD recently expanded production from well AR-1 (75881-F), its only well in the Arapahoe aquifer, and will soon install pumps in two existing wells in the Dawson Aquifer (83603-F & 83604-F). Beyond these projects, additional well construction in the Denver Basin is not anticipated at this time, although CMD has a substantial amount of undeveloped water rights in the Denver Basin Aquifers.

Existing CMD wells have had a series of upgrades to improve quality and efficiency within in the last year. More incremental improvements in the distribution system to improve

reliability and resiliency include deeper computer integration, well rehabilitations, upgrades to treatment systems, and emergency generator refurbishment.