EPC STORMWATER REVIEW COMMENTS IN ORANGE BOXES WITH BLACK TEXT

Please remove "Preliminary" from the title.

Preliminary/ Final Drainage Report

for the

McGehee Tract
El Paso County, Colorado

December, 2020

For the next submittal, please upload this report as a non-scanned PDF so it is searchable. If the signature page needs to be scanned, just scan that one page and add it to the rest of the non-scanned PDF electronically, or e-sign that page.

Prepared for:
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Job No: 2020-104

Please add the following "PCD No. SF2011"

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APPENDIX

Certifications and Approvals

Engineer's Statement

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report had been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omission on my part in preparation this report

Signature _____ Seal

(Kenneth C. Harrison, P.E.)

Developer/Owner Statement

I, the developer/owner, _____, have read and will comply with all of the requirements specified in this drainage report and plan.

_____

(Business Name)

By: _____

(Signature) (Date)

Print Name and Title _____

Address: _____

El Paso County

Please revise to: Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

_____ _____

Jennifer Irvine, P.E.Date

County Engineer / ECM Administrator

Conditions:

Flood Plain Statement

See Section V of this report

Please update report contents to include a confirmation from the state engineers office regarding the use of the stock ponds, as well as a breach analysis for the dam upstream.

I. Report Purpose

The purpose of this report is to evaluate the existing and developed drainage characteristics for the McGehee Residence project site. This will include:

- The evaluation of offsite conditions both upstream and downstream of the project site.
- A description of the existing offsite and onsite drainage improvements.
- Hydrologic analysis for both the existing and developed conditions. The main purpose for the analysis of the developed conditions was to demonstrate the negligible increase in runoff as a result of development.
- A hydraulic analysis was performed for both the existing and proposed culverts, the onsite swales and the Thompson Road borrow ditch.
- Recommendations regarding onsite drainage improvements..
- Discussion regarding detention and storm water quality.
- General recommendations regarding erosion control.

II. General Description

Location

The site is a 10.5-acre tract is noted as Lot 3 (the site) of the Mountain Shadows Ranch Second Phase Subdivision which was platted December 12, 1999 *(Exhibit 1, Appendix)*. Mountain Shadows Ranch Second Phase Subdivision is located in the northerly section of El Paso County in the Section 19, Township 11 South, Range 65 West of the 6th Principal Meridian, El Paso County, Colorado.

Lot 3 of the Mountain Shadows Ranch Second Phase Subdivision consists of approximately 10.5-acres It is proposed to subdivide the site into 2 lots. The sizes of these lots are:

- Lot 1: 5.0-acres
- Lot 2: 5.5-acres

The property to the north of the site is unplatted. The property to the west and east of the site 3 is Mountain Shadows Ranch Second Phase Lots 1 and 2. The property to the east of the site is the Thompson Road right-of-way with a varying right-of-way width.

Topography

The topographic characteristics of the site were determined from GIS mapping provided by El Paso County. The majority of the runoff from the site is collected by a natural swale that traverses the site in a west to east to west direction. The swale is stable and vegetated with natural grasses and a few bushes and trees. There are several stock ponds located along the swale. The approximate locations of these ponds are shown on the Drainage Map included in the map pocket of this report.

S2 appears to be within the site boundaries as well (in Basin A). Revise text/map as necessary.

The high point of the swale (S1) located west and offsite of the site is located approximately 2,200 feet west of the site's westerly property line. The average slope of the swale west of the site is 4.6%. Despite the relatively steep slope the existing swale (S1) appears to be stable with only minimal signs of erosion. The average slope of the swale within the site boundaries (S3) is approximately 3.6%. This swale (S3) outfalls into a large stock pond which its embankment is located approximately 130 feet west of the easterly property line which is also the westerly right-of-way line of Thompson Road. It appears that the pond is typically dry since the vegetation throughout the bottom and sides of the pond is well established. The embankment of the stock pond is approximately 12 feet high. The outlet of the pond consists of a 24-inch corrugated metal pipe (STR1). The water from the pond is routed to the 30-inch culvert (STR2) under Thompson Road via Swale 4 (S4). Once under Thompson Road the water is routed in a northeasterly direction in another stable grass-lined swale (S6). A hydraulic analysis of swale east of the Thompson Road crossing is beyond the scope of this report.

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Chap 2.5.3

From the map, it appears this should be revised to "easterly"

Additional runoff from a small westerly portion of the site is collected by the borrow ditch (S7) located along the west side of Thompson Road. Offsite runoff from OS8 is collected by another section of the Thompson Road borrow ditch (S5) located south of the culvert at DP14.

Only preliminary hydraulic analyses for the swales and culverts discussed above was conducted to obtain a "preliminary" estimation of the suitability of each drainage facility. A complete analysis of each drainage facility is outside the scope of this report since the increase in the runoff, based on the developed conditions, is only negligible.

Structures

The only drainage structure that exists is the culvert (STR3) under the farm access road off of Thompson Road. This is located approximately 225-feet north of the southeasterly property corner. Another culvert (STR4) is proposed under the proposed driveway access to be constructed approximately 150-feet south of the northeasterly property corner.

According to the topography narrative there is a 30 inch culvert under Thompson Road that carries flow easterly at design point 14. Please update this narrative to include all existing structures.

III. Design Criteria and Methodology

a. Design Manuals

Applicable excerpts from the following manuals are included in the Appendix of this report (*Exhibit 4, Appendix*)

- El Paso County Drainage Criteria Manual (EPCDCM), dated September 30, 1990, Revised July, 2019
- Colorado Springs Drainage Criteria Manuals, Volume 1 and 2, dated May, 2014
- Urban Drainage and Flood Control Manual, Volumes 2 and 3, dated August 2018
- CDOT Erosion Control Field Handbook, dated April 20, 2017

Please revise reference manuals to include EPC ECM revision year 2020.

b. Specific Criteria

- Design storms

  The design storms are as follows:

  Minor storm: 5 year

  Major storm: 100 year

  Any recommended drainage facilities are sized for the 5-year storm event.

  Routing of the 100-storm event is analyzed and discussed regarding the safe conveyance to offsite facilities.

- Drainage Areas

  Areas for the offsite and onsite sub basins were delineated from available topographic GIS mapping obtained from El Paso County.

Please update list of references to include any material used in this drainage report.

- Runoff Estimation
  - Rational Method: This method was used to determine runoff estimates since the Offsite and onsite drainage basins are less than 130 acres.
  - Intensity-Duration-Frequency (IDF) curves were obtained from the CSDCM (*Appendix, Exhibit 4)*
  - Time of concentration was determined using the equations provided in the EPC Drainage Criteria Manual *(Appendix, Exhibit 4).* The time of concentration values shown in the "Area Drainage Summary" tables *(Appendix, Exhibit 7)* reflect the time it takes for all of the runoff from each individual sub basin to reach the DP for each sub basin. The time of concentration used to determine the total runoff for the entire site was determined from the cumulative time for water to travel from DP1 to DP7. This was based on the assumed velocity in the existing swale segments 1 through 4. This is summarized in the "Surface Routing Summary" table in *Exhibit 7 of the Appendix.*

- Onsite Storm Sewer and Inlets

  All onsite drainage facilities and structure are limited to the proposed driveway and existing farm road borrow ditch culverts under the driveway intersections with Thompson Road.

- Drainage swale and borrow ditch sizing

  The only swale that drains this site is located approximately in the middle and it drains the site in a west to east direction. This swale is broken into segments, S1 through S4. The swales drain all the offsite and onsite runoff to an existing stock pond. The swale was evaluated for both the

Revise to S3. Drainage Map shows S4 is downstream of the existing stock pond.

minor 5-year storm and the major 100-year storm events. The swale is a stable natural swale lined with native grasses with a few bushes and trees.

- The Froude Numbers are shown for both the 5-year and the 100-year storm events for information purp values under 1.0 indicate sub critical flow which is 0 indicate super critical flow which can cause a su sion. Values from 0.9 to 1.1 are considered to be ur nbers indicated in this report are determined from a ssumptions regarding the physical characteris se characteristics were determined based on the to e from El Paso County. Additional field informatic obtain a more accurate determination of the stal on. Based on visual observations, all swale sections appear to be relatively stable with only a minimal amount of erosion and down cutting.

The stock pond appears to straddle Lots 1 & *Lot 2 Mountain Shadow Ranch Second Phase. Identify who owns and maintains this facility.

Please update to include a confirmation from the state dam engineer that discusses continued use of ponds. Also determine if ponds are jurisdictional or non-jurisdictional. Prior to addressing this comment, contact the review engineer, Lupe Packman, lupepackman@elpasoco.com, 719-313-6215.

- **Stock Ponds**
  There is a total of four (4) stock ponds located along the existing swale. The furthest easterly one is the largest. The ponds located east of the largest pond are minor with embankment heights between 2 to 4 feet. The approximate location of these ponds is shown on the Drainage Plan included in the map pocket of this report. Evaluation of these ponds are outside the scope of this report. However, based on visual observations they appear to be stable with permanent stands of native vegetation.

- **Culverts**
  There is only one (1) existing culvert (STR #3) under an existing farm road that intersects with Thompson located approximately 200 feet north of the southeasterly property corner. A summary of the hydraulic characteristics of the culvert is presented in subsequent section of this report as well as in the Appendix *(Appendix, Exhibit 7*).

Please update this narrative to include information about STR2 if it is an existing culvert.

A proposed culvert (STR #4) is proposed in the westerly Thompson Road borrow ditch approximately 150-feet south of the northeasterly property corner' A summary of the hydraulic characteristics of the culvert is presented in subsequent section of this report as well as in the Appendix *(Appendix, Exhibit 7*).

Both culverts were sized and evaluated as follows:

- Headwater to Depth Ratio = 1.5 for the 5-year storm
- One lane open along Thompson Road for the 100-year storm. Since this criteria typically produces substantial erosion at the outlet the allowable velocity in the culvert was limited to no greater than 10 fps.

- Riprap Erosion Control at the outlet will be discussed. It is assumed that the Owner will not be required to install any erosion control improvements at this locations since the increase in runoff as a result of development is only negligible and therefore has little if any effect on the existing condition of the culvert..
- Flared End Sections at both the entrance and the outlet to the culvert.

- **Detention/ Water Quality Pond**
  A full spectrum detention pond is not required for this development. Reasons will be discussed in a subsequent section of this report. The reasons are based on El Paso County criteria as well as Colorado State criteria.

- **Erosion control**
  The following facilities are anticipated to be required along the proposed driveway located as shown on the Developed Conditions Drainage Plan:
  - Erosion Control Blankets
  - Silt fences
  - Staked hay bales
  - Erosion control fabric
  - Erosion control logs

IV. EXISTING REPORTS, MAPPING AND INFORMATION

No drainage reports have been prepared for the areas adjacent to the project site

V. FEMA FLOODPLAIN

The project site is located in FEMA map # 08041CO305G (eff 12/7/2018) (*Appendix, Exhibit 2).* The entire site is located outside the 100-year floodplain in Zone X which is an "Area of Minimal Disturbance" for which there are no special requirements for the construction of commercial or industrial structures.

VI. HYDROLOGIC SOILS INFORMATION

The hydrologic soils groups were obtained from the USDA National Resource Conservation Service website for soils types in El Paso County, Colorado (*Appendix, Exhibit 3*). The soils are identified as follows:

a. **Peyton Sandy Loam (Soil ID 67)** which have the following characteristics:
   - Well drained
   - Runoff Class; Medium
   - Depth to water table: >80 inches
   - Frequency of flooding: none
   - Frequency of ponding: none
   - Hydrologic Soil Group: B

   A detailed description of each of the type soil is included in *Appendix Exhibit 3.*

VII. EXISTING DRAINAGE CONDITIONS

a. Existing Drainage Reports

The drainage report for Mountain Shadows Ranch Second Phase Subdivision was not available through the EDARP service from El Paso County.

b. Offsite Drainage Areas for Existing Conditions

i. The hydrologic characteristics for both the existing and developed conditions of the site are included in the Appendix *(Exhibit 6)*. The hydraulic conditions for the existing swales and the existing and the proposed culverts are summarized in the tables included in the Appendix (*Exhibit 7*).

ii. **Design Point 1**

Description

DP1 is the collection point for runoff from Sub-basin OS1 (34.1 acres). This Sub-basin is vegetated with natural grasses with a few bushes and trees located along the flowline of the existing swale (S1). All the runoff is collected by an existing natural and stable swale (S1) that basically bisects the site in a west to east direction. The swale routes the water to DP1 located on the westerly property line 300-feet south of the northwesterly property corner of the study tract. The portion of the sub basin to the north of the swale slopes from north to south at an average slope between 4.0% and 7%.

Hydrologic Summary

The hydrologic characteristics of the runoff from OS1 at DP1 for the existing conditions is as follows:

- Drainage Area = 34.1 acres
- Runoff Coefficients: 5 year = 0.09, 100 year = 0.36
- Time of Concentration: 23.2 minutes
- Runoff: 5 year = 8.7 cfs, 100 year = 58.5 cfs

Hydraulic Summary for Offsite Swale 1 (S1) Calc Sheets CS1, CS2.

- Runoff: 5 year = 8.7 cfs, 100 year = 58.5 cfs
- Average Bottom width: 5 feet
- Average Side Slope ratio: 20 to 1
- Average Slope: 4.7%
- Velocity: 5-year = 2.9 fps, 100 year = 4.9 fps
- Depth of Flow = 5-year = 0.3 ft, 100 year = 0.7 ft.
- Froude No.= 5-year = 1.21 (Supercritical), 100 year = 1.38 (Super critical)

iii. **Design Point 2**

Description

DP2 is the collection point for runoff from Sub-basin OS3 (0.6 acres). Runoff at this DP flows to the south and enters swale 1 near DP1. The Sub-basin slopes from north to south at an average grade of 10%. Sub-basin OS3 is vegetated with natural grasses. All the runoff is collected by an existing natural and stable swale (S1) that basically bisects the Sub-basin in an east to west direction.

Hydrologic Summary

The hydrologic characteristics of the runoff from OS3 at DP1 for the existing conditions is as follows:

- Drainage Area = 0.6 acres
- Runoff Coefficients: 5 year = 0.08, 100 year = 0.35
- Time of Concentration: 15 minutes
- Runoff: 5 year = 0.2 cfs, 100 year = 1.2 cfs

iv. **Design Point 3**

Description

DP3 is the collection point for runoff from Sub-basin OS4 (0.6 acres). Runoff at this DP flows to the north and enters swale 1 near DP1. The Sub basin slopes from south to north at an average grade of 8%. Sub-basin OS4 is vegetated with natural grasses. All the runoff is collected by the existing natural and stable swale (S1).

Hydrologic Summary

The hydrologic characteristics of the runoff from OS4 at DP3 for the existing conditions is as follows:

- Drainage Area = 0.6 acres
- Runoff Coefficients: 5 year = 0.17, 100 year = 0.0.41
- Time of Concentration: 13.3 minutes
- Runoff: 5 year = 0.4 cfs, 100 year =1.5 cfs

v. **Design Point 4**

Description

Runoff from Sub-basin OS7 (3.6 acres) sheet flows onto onsite Sub-basin A. There is not a specific collection point. DP4 only represents the total amount of sheet flow that enters onsite Sub-basin A. This DP is also located at the entrance to the large stock pond. Runoff at this DP flows to the north and enters swale 2 (S2) at DP4. Sub basin OS7 slopes from south to north at an average grade of 8.3%. Sub basin OS7 is vegetated with natural grasses. All the runoff is collected by the existing natural and stable swale (S2) which outfalls into the stock pond.

Hydrologic Summary
The hydrologic characteristics of the runoff from OS7 at DP4 for the existing conditions is as follows:

- Drainage Area = 3.6 acres
- Runoff Coefficients: 5 year = 0.12, 100 year = 0.38
- Time of Concentration: 15.5 minutes
- Runoff: 5 year = 1.5 cfs, 100 year = 8.0 cfs

The hydraulic characteristics for the swale that drains this study site are summarized in the Onsite Drainage Areas for Existing Conditions section of this report in order to account for all of the upstream runoff that contributes storm water to swale 2.

v. Design Point 5 (see Onsite Drainage Areas for Existing Conditions)

vi. Design Point 6 (see Onsite Drainage Areas for Existing Conditions)

vii. Design Point 7 (see Onsite Drainage Areas for Existing Conditions)

viii. Design Point 8

Description
Runoff from sub basin OS5 (0.4 acres) sheet flows onto onsite Sub-basin A. There is not a specific collection point. DP8 only represents the total amount of sheet flow that enters onsite Sub- basin A. Sub-basin OS5 slopes from north to south to north at an average grade of 1.2%. Sub basin OS5 is vegetated with natural grasses.

Hydrologic Summary
The hydrologic characteristics of the runoff from OS5 at DP8 for the existing conditions is as follows:

- Drainage Area = 0.4 acres
- Runoff Coefficients: 5 year = 0.08, 100 year = 0.35
- Time of Concentration: 10.9 minutes
- Runoff: 5 year = 0.1 cfs, 100 year = 0.9 cfs

ix. Design Point 9

Description

Runoff from Sub-basin OS6 (0.9 acres) sheet flows onto onsite Sub-basin B. There is not a specific collection point for the runoff from Sub-basin OS6. DP9 only represents the total amount of sheet flow that enters onsite sub basin B. Sub-basin OS6 slopes from north to south to north at an average grade of 5.0%. Sub-basin OS6 is vegetated with natural grasses.

Hydrologic Summary

The hydrologic characteristics of the runoff from OS5 at DP8 for the existing conditions is as follows:

- Drainage Area = 0.9 acres
- Runoff Coefficients: 5 year = 0.08, 100 year = 0.35
- Time of Concentration: 15.2 minutes
- Runoff: 5 year = 0.3 cfs, 100 year = 1.9 cfs

x. Design Point 10

Description

Runoff from Sub-basin OS2 (10.3 acres) sheet flows into the stock pond from the south. There is no specific collection point. DP10 only represents the total amount of sheet flow that enters the stock pond. Sub-basin OS2 slopes from south to north at an average grade of 7.3%. Sub-basin OS2 is vegetated with natural grasses. All the runoff is collected by the existing stock pond.

Hydrologic Summary

The hydrologic characteristics of the runoff from OS2 at DP10 for the existing conditions is as follows:

- Drainage Area = 10.3 acres
- Runoff Coefficients: 5 year = 0.10, 100 year = 0.36
- Time of Concentration: 17.9 minutes
- Runoff: 5 year = 3.3 cfs, 100 year = 20.5 cfs

xi. Design Point 11

Description

Please revise. It appears that design point 11 drains to S4, and not S5, per the existing drainage map.

Runoff from Sub-basin OS8 (1.4 acres) sheet flows into an existing grass lined swale (S5) that routes the storm water from the outlet of the stock pond to the existing 30-inch culvert under Thompson Road. There is no specific collection point from OS8. DP11 only represents the total amount of sheet flow that enters S4. Sub-basin OS8 slopes from south to north at an average grade of 5.5%. Sub-basin OS8 is

vegetated with natural grasses. All the runoff is collected by the existing swale (S4).

Hydrologic Summary
The hydrologic characteristics of the runoff from OS8 at DP11 for the existing conditions is as follows:

- Drainage Area = 1.4 acres
- Runoff Coefficients: 5 year = 0.08, 100 year = 0.35
- Time of Concentration: 14.1 minutes
- Runoff: 5 year =0.4 cfs, 100 year = 3.0 cfs

xii. Design Point 12

Description
Runoff from Sub-basin OS10 (0.5 acres) is collected by the westerly borrow ditch (S5) along Thompson Road. The high point of the borrow ditch is located approximately 500-feet south of the culvert under Thompson Road at DP14. The water flows from south to north and joins the water flowing from the north from Sub-basin OS9. The water is then routed under Thompson Road via a 30-inch corrugated metal culvert at DP14.

Sub-basin OS10 is composed of native vegetation and asphalt roadway for Thompson Road. All the runoff is collected by the existing borrow ditch (S5) which routes the water under Thompson Road via a 30-inch CMP culvert.

Hydrologic Summary
The hydrologic characteristics of the runoff from OS10 at DP12 for the existing conditions is as follows:

- Drainage Area = 0.5 acres
- Runoff Coefficients: 5 year = 0.57, 100 year = 0.72
- Time of Concentration: 5.3 minutes
- Runoff: 5 year = 1.5 cfs, 100 year = 3.1 cfs

Hydraulic Summary for Offsite Swale 5 (S5) Calc Sheets 3-4,

- Runoff: 5 year = 1.5 cfs, 100 year = 3.1 cfs
- Average Bottom width: 2 feet
- Average Side Slope ratio: 3 to 1
- Average Slope: 5%
- Velocity: 5-year = 2.7 fps, 100 year = 3.5 fps
- Depth of Flow = 5-year = 0.2 ft, 100 year = 0.3 ft.
- Froude No.= 5-year = 1.2 (Supercritical), 100 year = 1.28 (Super critical)

xiii. Design Point 13

Description

Runoff from Sub-basin OS9 (0.4 acres) is collected by the westerly borrow ditch (S6) for Thompson Road. The high point of the borrow ditch is located approximately 550-feet north of the culvert under Thompson Road at DP14. The water flows from north to the south and joins the water flowing from Sub-basin OS10. The water then is routed under Thompson Road via a 30-inch corrugated metal culvert at DP14.

Sub-basin OS9 is composed of native vegetation and asphalt roadway for Thompson Road. All the runoff is collected by the existing borrow ditch (S6) which routes the water under Thompson Road via a 30" CMP culvert

Hydrologic Summary

The hydrologic characteristics of the runoff from OS9 at DP13 for the existing conditions is as follows:

- Drainage Area = 0.4 acres
- Runoff Coefficients: 5 year = 0.70, 100 year = 0.81
- Time of Concentration: 6.1 minutes
- Runoff: 5 year = 1.4 cfs, 100 year = 2.6 cfs

Hydraulic Summary for Offsite Swale 6 (S6) Calc Sheets CS13, 14.

- Runoff: 5 year = 1.4 cfs, 100 year = 2.6 cfs
- Average Bottom width: 2 feet
- Average Side Slope ratio: 3 to 1
- Average Slope: 5.5%
- Velocity: 5-year = 2.8 fps, 100 year = 3.4 fps
- Depth of Flow = 5-year = 0.2 ft, 100 year = 0.3 ft.
- Froude No.= 5-year = 1.27 (Supercritical), 100 year = 1.32 (Super critical)

Preliminary Hydraulic Summary for Proposed Driveway Culvert 150 south of NE Property Corner (Calc Sheet 9)

- Runoff: 5 year = 1.4 cfs, 100 year = 2.6 cfs
- Culvert Size: 18- inches
- Conditions: inlet control
  Headwater to Depth Ratio: 5 year = neg, 100 year = 0.6
- Depth at inlet
  5 year = neg, 100 year = 0.9 ft

xiv. Design Point 14

Description

DP 14 is located at the entrance to the existing 30-inch culvert under Thompson Road. The hydraulic characteristics are discussed in the "Onsite Drainage Areas for Existing Conditions" in the following section of this report.

Several hydraulic summaries do not meet criteria. Per EPCDCM Vol.1 6.5.2 grass lined channels shall not be used when froude number is greater than 0.9 for the 100 year storm. Please revise drainage report to include recommendations for channel linings where channel froude numbers exceed 0.9 and permissible velocities, per table 10-3 and 10-4.

c. Onsite Drainage Areas for Existing Conditions

i. Design Point 5

Description

Runoff from Sub-basins OS1 (34.1 acres), OS3 (0.6 acres), OS4 (0.6 acres), OS5 (0.4 acres), OS7 (3.6 acres) and onsite Sub-basin A (6.3 acres), resulting in a total drainage are of 45.6 acres, is collected at DP5 which is located at the entrance to the existing stock pond. The DP is also located where the existing swale (S2) routes the water outside the site along the southerly property line. The swale (S2) is grass lined and stable with only a minimal amount of erosion.

Hydrologic Summary

The hydrologic characteristics of the runoff DP1 for the existing conditions is as follows:

- Drainage Area = 45.6 acres
- CA: 5 year = 4.16, 100 year = 16.31
- Time of Concentration: 27 minutes
- Runoff: 5 year = 11.0 cfs, 100 year = 72.2 cfs

Hydraulic Summary for Onsite Swale 2 (S2) (Calc Sheets 5 and 6)

- Runoff: 5 year = 11.0 cfs, 100 year = 72.2 cfs
- Average Bottom width: 5 feet
- Average Side Slope ratio: 18
- Average Slope: 5.5%
- Velocity: 5-year = 3.4 fps, 100 year = 5.5 fps
- Depth of Flow = 5-year = 0.3 ft, 100 year = 0.7 ft.
- Froude No.= 5-year = 1.32 (Supercritical), 100 year = 1.5 (Supercritical)

ii. Design Point 6

Description

DP6 is located at the 24-inch outlet for the stock pond. Runoff from Sub-basins B (3.5 acres) and OS2 (10.3 acres) and OS6 (0.9 acres combines with runoff at DP 5 for a total contributing drainage acreage of 60.3 acres. The water from this acreage is collected in the stock pond and is routed under the pond embankment via a 24-inch CMP. The water in Swale 2 is routed to the south of the site's southerly property line along the bottom of the stock pond. Swale 2 swale routes the natural "low flow" section when there is no water in the pond. This pond may provide some type of "detention" should the flow into it become substantial. The structural and hydraulic evaluation of the stock pond is beyond the scope of this report.

Hydrologic Summary
The hydrologic characteristics of the runoff at DP6 for the existing conditions is as follows:

- Drainage Area = 60.3 acres
- CA: 5 year = 5.47, 100 year = 21.32
- Time of Concentration: 28.4 minutes
- Runoff: 5 year = 14.0 cfs, 100 year = 91.7 cfs

Hydraulic Summary for Offsite Swale 3 (S3) (Calc Sheets 7 and 8)
- Runoff: 5 year = 14 cfs, 100 year = 91.7 cfs
- Average Bottom width: 5 feet
- Average Side Slope ratio: 15
- Average Slope: 8.3%
- Velocity: 5-year = 4.4 fps, 100 year = 7.2 fps
- Depth of Flow = 5-year = 0.3 ft, 100 year = 0.8 ft.
- Froude No.= 5-year = 1.66 (super critical), 100 year = 1.89 (super critical)

Preliminary Hydraulic Summary for Existing culvert at DP6 (Calc Sheet 9)
- Runoff: 5 year = 14 cfs, 100 year = 91.7 cfs
- Culvert Size: 24- inches
- Conditions: inlet control
- Headwater to Depth Ratio: 5 year = 1.1, 100 year = out of range and roadway overtopping.

iii. <u>Design Point 7</u>
Description
DP7 is located on the easterly property line between the pond outlet and the culvert under Thompson Road. DP7 is located approximately 530-feet north of the southeast property corner. Runoff from Sub-basins C (0.8 acres) and OS8 (1.4 acres) join with runoff at DP6 (60.3 acres) resulting in a total drainage area of 62.5 acres.

Hydrologic Summary
The hydrologic characteristics of the runoff at DP7 for the existing conditions is as follows:

- Drainage Area = 62.5 acres
- CA: 5 year = 5.71, 100 year = 22.37
- Time of Concentration: 29 minutes
- Runoff: 5 year = 14.5 cfs, 100 year = 95 cfs

Hydraulic Summary for Offsite Swale 3 (S4) (Calc Sheets 11 and 12)

- Runoff: 5 year = 14.5 cfs, 100 year = 95 cfs
- Average Bottom width: 5 feet
- Average Side Slope ratio: 10
- Average Slope: 8.3%
- Velocity: 5-year = 4.8 fps, 100 year = 8.0 fps
- Depth of Flow = 5-year = 0.4 ft, 100 year = 0.9 ft.
- Froude No.= 5-year = 1.69 (super critical), 100 year = 1.92 (super critical)

iv. **Design Point 14**

Description

DP 14 is located at the upstream end of the 30" CMP culvert under Thompson Road. Runoff from OS9 (0.4 acres) and OS10 (0.5 acres) join the runoff at DP7 (62.5 acres) resulting in a total acreage of 63.4 acres.

Hydrologic Summary

The hydrologic characteristics of the runoff at DP14 for the existing condition is as follows:

- Drainage Area = 63.4 acres
- CA: 5 year = 6.28, 100 year = 23.05
- Time of Concentration: 29 minutes
- Runoff: 5 year = 15.9 cfs, 100 year = 97.9 cfs

Preliminary Hydraulic Summary for Existing culvert at DP14 (Calc Sheet 10)

- Runoff: 5 year = 15.9 cfs, 100 year = 97.9 cfs
- Culvert Size: 30-inches
- Conditions: inlet control
- Headwater to Depth Ratio: 5 year = 0.76, 100 year = >6.0
- Upstream Depth: 5 year = 1.9 ft, 100 year = roadway overtopping

VIII. DEVELOPED ONSITE DRAINAGE CONDITIONS AND IMPROVEMENTS (EXHIBIT 6, APPENDIX)

I. Offsite Sub-basin Characteristics for Developed Conditions

There are no plans to develop the tracts located upstream and/or adjacent to the project site. Therefore, the hydrologic conditions for the offsite sub basins will remain the same, as described *Section VII.*

ii. Onsite Sub-basin Characteristics for Developed Conditions

The development of this site consists of the subdivision of the entire 10.495-acre into one 5-acre parcel (Lot 2) and a 5.5-acre parcel (Lot 1). Lot 1 is located in Sub-basin B and Lot 2 if located in Sub-basin A. The development includes a single-family residence, area landscaping, and a gravel driveway. The hydrologic parameters used to estimate runoff were determined based on the following parameters;

- Drainage Area Sub basins:
  - The only sub basins that will change to reflect proposed development will be onsite Subbasins B (Lot 1) and A (Lot 2).

- Runoff Coefficients
  - Gravel Driveway: Lot 1= 250 ft. Lot 2 = 420 feet; C5 = 0.59, C100 = 0.70
  - The driveways were conservatively sized at 20 feet wide.
  - The roof of each house was conservatively estimated to be 3,000 sf each with the following coefficients: C5 = 0.90, C100 = 0.96
  - It was conservatively assumed that ½ acre would be landscaped resulting in the following coefficients; C5 = 0.12, C100 = 0.39
  - The remaining area of each lot would remain as the native vegetation resulting in the following coefficients: C5 = 0.08, C100 = 0.35.

- Time of Concentration
  - The time of concentration for each sub-basin remains the same despite development since the time of concentration is defined as the time it takes for runoff from the farthest "corner" of the contributing drainage sub-basin to reach the design point.

- Rainfall Intensity
  - The rainfall intensity for each sub-basin remains the same since the time of concentration remains the same.

- Estimated Runoff
  - Based on the above assumptions, runoff for the minor (5 year)

and major (100 year) storms were estimated for each sub-basin.

iii. **Runoff Comparison Summary**

a. **Drainage Area**

i. The areas are as follows:

- Lot 1: 5.000 acres (portion of sub basin A)
- Lot 2: 5.495 acres (portion of sub basin B)

b. **Composite Runoff Coefficients**

i. Exist Conditions

1. Lot 1(sub basin B): C5 = 0.08, C100 = 0.35
2. Lot 2 (sub basin A): C5 = 0.08, C100 = 0.35

ii. Developed Conditions

1. Lot 1(sub basin B): C5 = 0.11, C100 = 0.37
2. Lot 2 (sub basin A: C5 = 0.11, C100 = 0.37

c. **Rainfall Intensity**

i. The Rainfall Intensity is the same for both the existing and proposed conditions.

d. **Estimate Runoff for each Developed Lot**

Runoff from each lot will sheet flow to swales (S2, S3, S4) and the stock pond. The runoff for the existing and developed conditions are summarized as follows:

i. Lot 1 (sub basin B)

1. Existing Conditions: Q5 = 1.8 cfs, Q100 = 13.3 cfs
2. Developed Conditions: Q5 = 2.5 cfs, Q100 = 14.1 cfs

ii. Lot 2 (sub basin A)

1. Existing Conditions: Q5 = 1.2 cfs, Q100 = 8.9 cfs
2. Developed Conditions: Q5 = 1.9 cfs, Q100 = 11.1 cfs

e. **Total Discharge at Thompson Road Culvert**

Refer to Surface Routing Summary for table using the Times of Concentration applicable to determining cumulation runoff in the channel located in the center of the site.

i. Existing Conditions: Q5 = 15.9 cfs, Q100 = 97.9 cfs
ii. Developed Conditions: Q5 = 16.4 cfs, Q100 = 98.5 cfs

As demonstrated, the increase in runoff, as a result of development, is negligible and therefore has little, if any, impact on the existing facilities.

IX. FULL SPECTRUM DETENTION POND (EXHIBIT 7, APPENDIX)

Criteria

Reference is made to El Paso County Engineering Criteria Manual, Appendix 1, Page 1.18-19. According to El Paso County criteria a Water Quality Capture

Please include the estimated area of disturbance

Volume (WQCV) pond is not required for lots 2.5 acres or larger. Also, since the area of disturbance is less than 1- acre a WQCV pond is not required.

X. FOUR STEP PROCESS

Please include the four step process in this section and also include ECM reference to Appendix I.7.1.B.5.

In accordance with section 4.0 of chapter 1 of the El Paso County ECM Appendix 1.7.1, the four-step process applies to "projects with construction activities that disturb 1 acre or greater or that disturb less than 1 acre but are part of a larger plan of development or sale". Therefore, the four-step process does not apply to this development.

EROSION CONTROL

The following erosion control measures are recommended for the proposed private driveway:

- Silt fence along the southerly side of the proposed driveway
- Erosion control fabric on all disturbed surfaces
- Buried riprap at the outlet end of the proposed culverts installed under the proposed driveway.

XII. STORMWATER MANAGEMENT PLAN (SWMP)

A **SWMP** will not be required for this site since the improvements are limited to a shared private gravel driveway with a shared access off of Thompson Road.

XIII. DRAINAGE/ BRIDGE FEES

The site is located in the **Cherry Creek Drainage Basin** for which there are no established fees.

XIV. SUMMARY

This report provides a thorough analysis of the historic and developed drainage conditions for the proposed McGehee Subdivision. The property is comprised of 10.5 acres and is located north of Hodgen Road and west of Thompson Road. The subdivision is to be subdivided into two (2) consisting of a 5-acre lot and a 5.5-acre lot.

The vegetation consists of primarily prairie grass with no trees. There is a main natural drainage way that is located in the approximate middle of the site.

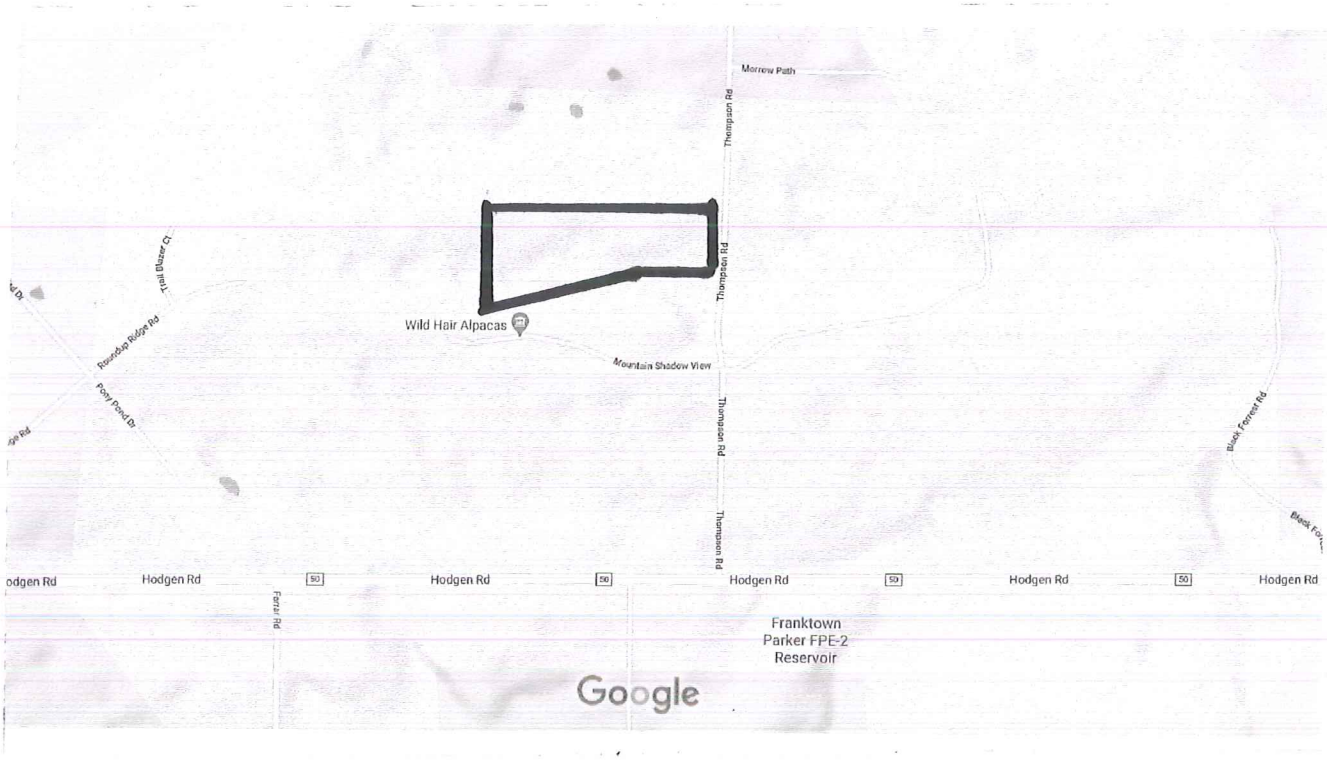
It has been demonstrated that there is only a negligible increase in runoff as a result of development. Also, based on the present engineering criteria for El Paso County a full spectrum detention pond is not required. Improvements are to be limited to two (2) residential homes, a common gravel driveway with a common access to Thompson Road, and a driveway culvert located approximately 150-feet south of the northeast property corner.

Included in the map pocket are drainage maps for the **Existing Drainage Conditions** and the **Developed Drainage Conditions**. No storm water structures are proposed for this subdivision.

APPENDIX

Exhibit 1

Location Map



Vicinity Map

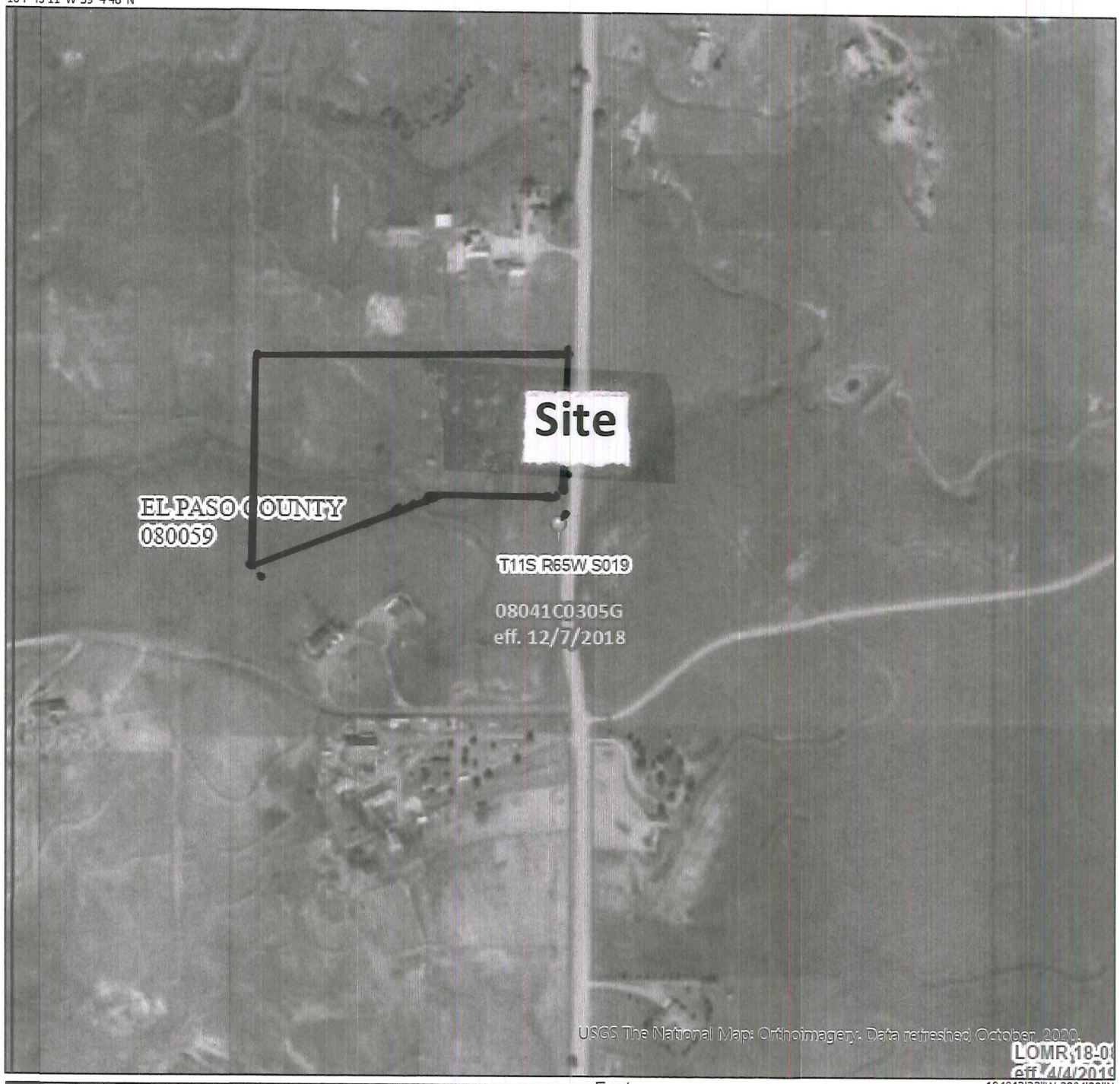
Exhibit 2

FEMA FIRM Map

National Flood Hazard Layer FIRMette

FEMA

104°43'11"W 39°4'48"N



104°42'33"W 39°4'20"N

0 250 500 1,000 1,500 2,000 Feet

1:6,000

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- B 20.2 17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
- 8 Coastal Transect
- 513 Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

The pin displayed on the map is an approximate point selected by the user and does not represen an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 11/9/2020 at 8:17 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, City of Fountain, Bureau of Land Management, National Oceanic and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

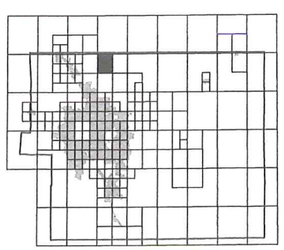
Contact **FEMA Map Service Center** (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table

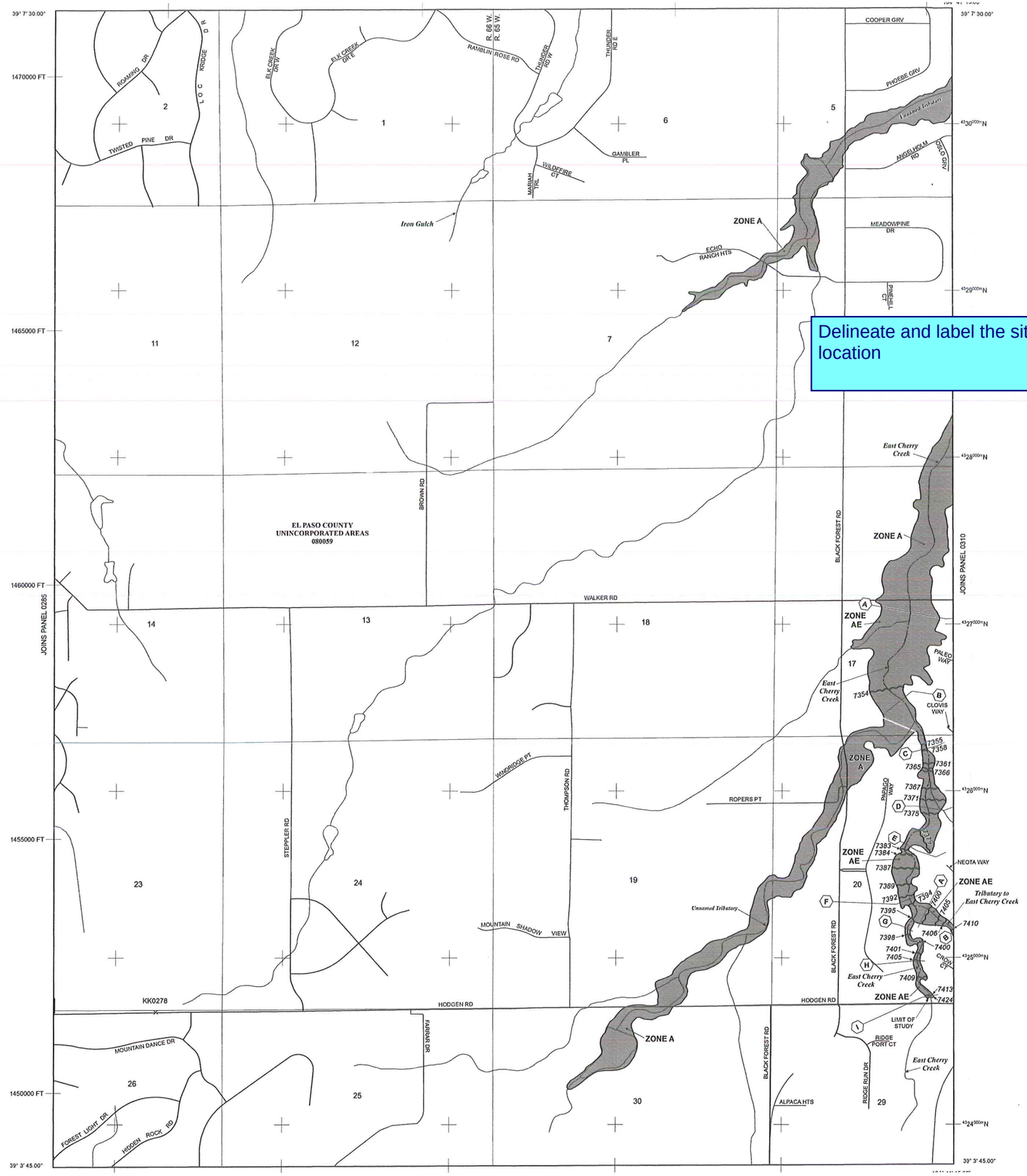
Flooding Source	Vertical Datum Offset (ft)
REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION	

Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D	Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

	Floodplain boundary
	Floodway boundary
	Zone D Boundary
	CBRS and OPA boundary
	Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
513	Base Flood Elevation line and value; elevation in feet*
(EL 987)	Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

A — A	Cross section line
23 - - - - 23	Transect line
97° 07' 30.00" 32° 22' 30.00"	Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
4375000mN	1000-meter Universal Transverse Mercator grid ticks, zone 13
6000000 FT	5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
DX5510 ×	Bench mark (see explanation in Notes to Users section of this FIRM panel)
• M1.5	River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE
FLOOD INSURANCE RATE MAP
MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 1000'

500 0 1000 2000 FEET

300 0 300 600 METERS

NFIP

NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0305G

FIRM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY, COLORADO
AND INCORPORATED AREAS

PANEL 305 OF 1300

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
EL PASO COUNTY	080059	0305	G

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
08041C0305G

Exhibit 3

SCS Soils Map and Data

United States Department of Agriculture

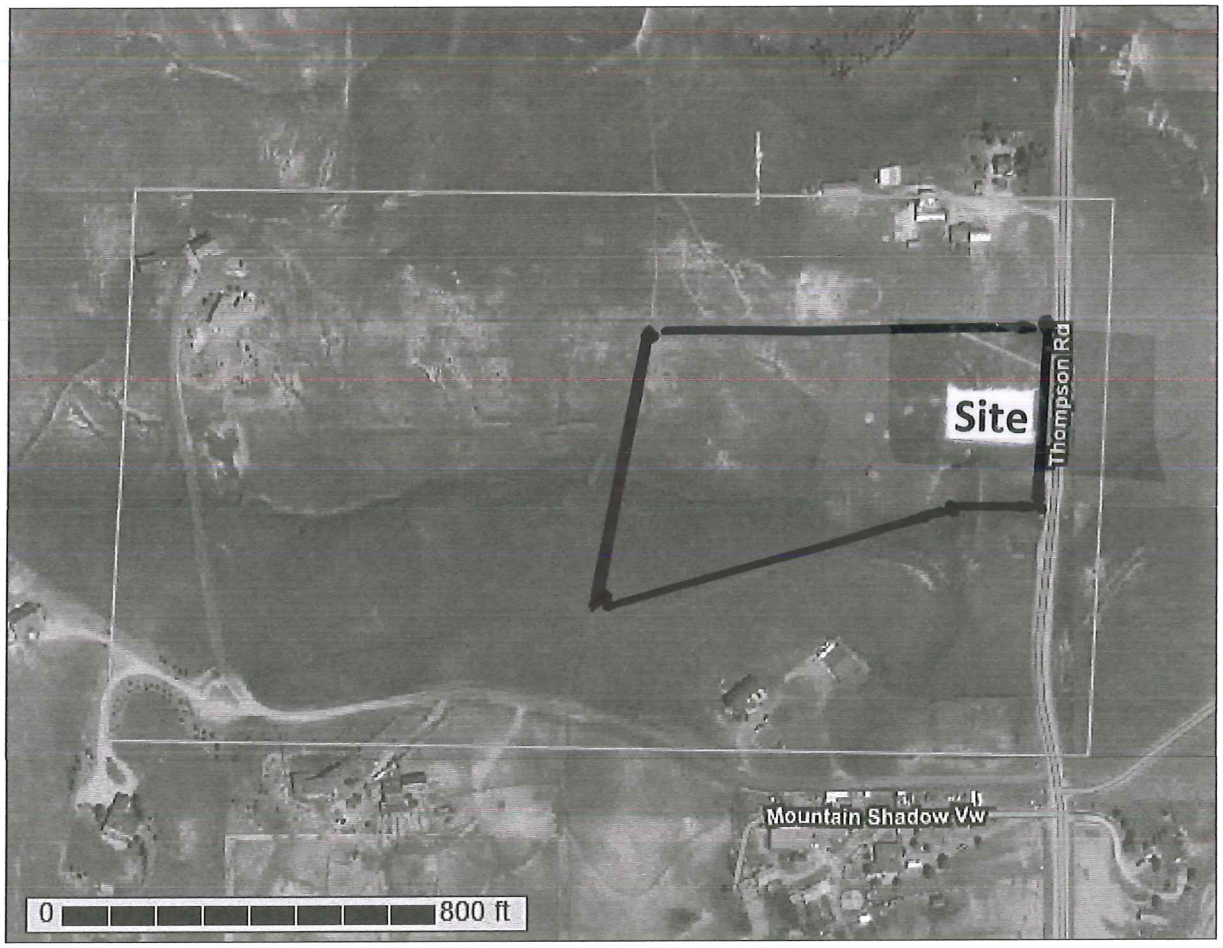
NRCS

Natural Resources Conservation Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for

El Paso County Area, Colorado



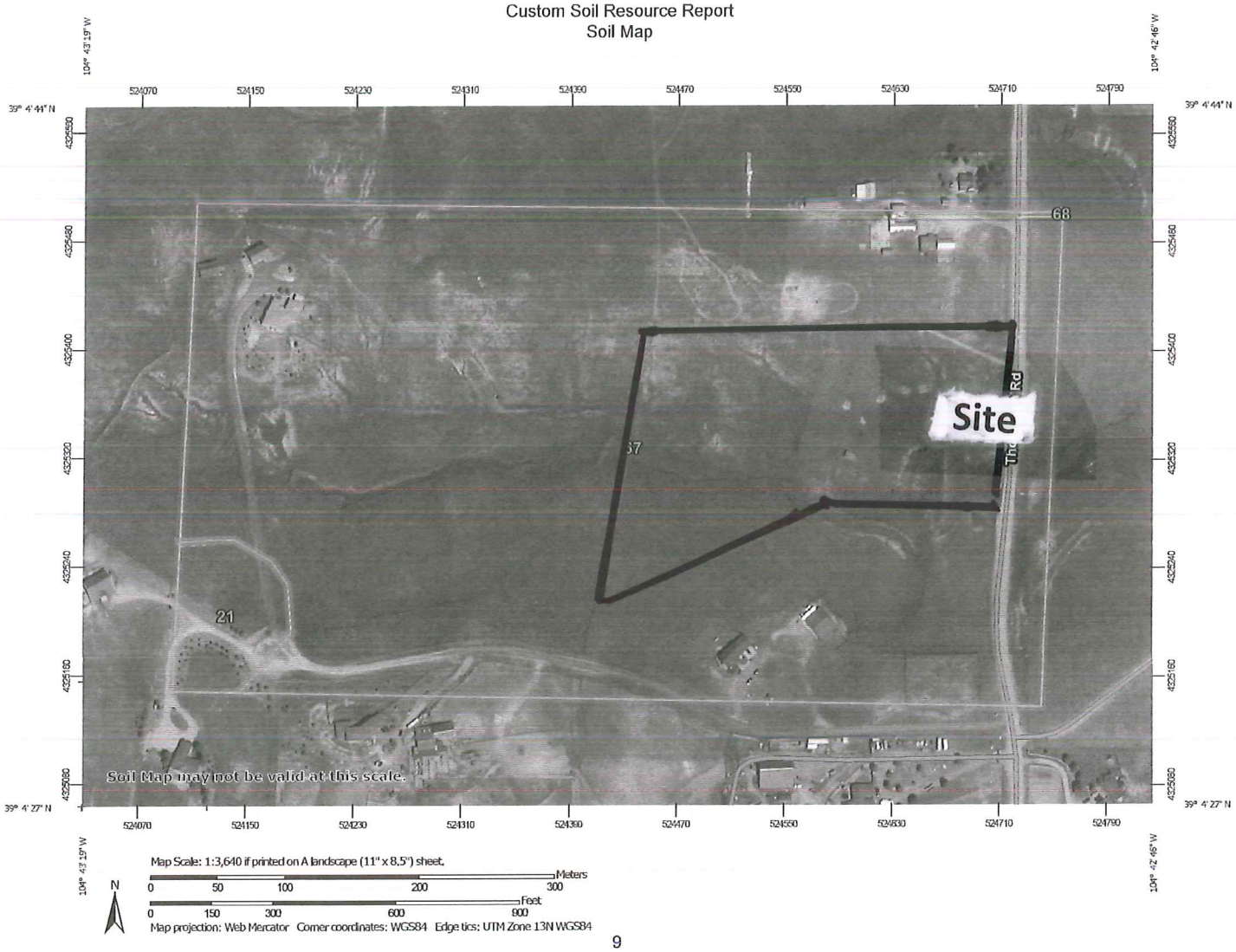
October 24, 2020

Contents

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report
Soil Map



MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features

- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 18, Jun 5, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 8, 2018—May 26, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
21	Cruckton sandy loam, 1 to 9 percent slopes	2.0	3.5%
67	Peyton sandy loam, 5 to 9 percent slopes	55.9	96.5%
68	Peyton-Pring complex, 3 to 8 percent slopes	0.0	0.0%
Totals for Area of Interest		**57.9**	**100.0%**

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas.* Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

21—Cruckton sandy loam, 1 to 9 percent slopes

Map Unit Setting

National map unit symbol: 367s
Elevation: 7,200 to 7,600 feet
Mean annual precipitation: 16 to 18 inches
Mean annual air temperature: 42 to 46 degrees F
Frost-free period: 110 to 120 days
Farmland classification: Not prime farmland

Map Unit Composition

Cruckton and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Cruckton

Setting

Landform: Flats, hills
Landform position (three-dimensional): Side slope, talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium derived from arkose

Typical profile

A - 0 to 11 inches: sandy loam
Bt - 11 to 28 inches: sandy loam
C - 28 to 60 inches: loamy coarse sand

Properties and qualities

Slope: 1 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: B
Ecological site: R049XB216CO - Sandy Divide
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:
Hydric soil rating: No

67—Peyton sandy loam, 5 to 9 percent slopes

Map Unit Setting

National map unit symbol: 369d
Elevation: 6,800 to 7,600 feet
Mean annual air temperature: 43 to 45 degrees F
Frost-free period: 115 to 125 days
Farmland classification: Not prime farmland

Map Unit Composition

Peyton and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Peyton

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam
Bt - 12 to 25 inches: sandy clay loam
BC - 25 to 35 inches: sandy loam
C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 5 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: B
Ecological site: R049XB216CO - Sandy Divide
Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

68—Peyton-Pring complex, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 369f
Elevation: 6,800 to 7,600 feet
Farmland classification: Not prime farmland

Map Unit Composition

Peyton and similar soils: 40 percent
Pring and similar soils: 30 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Peyton

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam
Bt - 12 to 25 inches: sandy clay loam
BC - 25 to 35 inches: sandy loam
C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 3 to 5 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4c
Hydrologic Soil Group: B
Ecological site: R049XB216CO - Sandy Divide
Hydric soil rating: No

Description of Pring

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Arkosic alluvium derived from sedimentary rock

Typical profile

A - 0 to 14 inches: coarse sandy loam
C - 14 to 60 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: B
Ecological site: R048AY222CO
Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

Exhibit 4

Charts and Tables

Table 6-6. Runoff Coefficients for Rational Method
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis-- Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

3.2 Time of Concentration

One of the basic assumptions underlying the Rational Method is that runoff is a function of the average rainfall rate during the time required for water to flow from the hydraulically most remote part of the drainage area under consideration to the design point. However, in practice, the time of concentration can be an empirical value that results in reasonable and acceptable peak flow calculations.

For urban areas, the time of concentration (t_c) consists of an initial time or overland flow time (t_i) plus the travel time (t_t) in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For non-urban areas, the time of concentration consists of an overland flow time (t_i) plus the time of travel in a concentrated form, such as a swale or drainageway. The travel portion (t_t) of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or drainageway. Initial time, on the other hand, will vary with surface slope, depression storage, surface cover, antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. The time of concentration is represented by Equation 6-7 for both urban and non-urban areas.

$$t_c = t_i + t_t \qquad \text{(Eq. 6-7)}$$

Where:

t_c = time of concentration (min)

t_i = overland (initial) flow time (min)

t_t = travel time in the ditch, channel, gutter, storm sewer, etc. (min)

3.2.1 Overland (Initial) Flow Time

The overland flow time, t_i, may be calculated using Equation 6-8.

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \qquad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \qquad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)

C_v = conveyance coefficient (from Table 6-7)

S_w = watercourse slope (ft/ft)

concentration, all other factors being equal. Although it is possible to calculate a longer time of concentration for a post-development condition versus a pre-development condition by increasing the length of the flow path, this is often a result of selecting unrealistic flow path lengths. As a matter of practice and for the sake of conservative design, it is required that the post-development time of concentration be less than or equal to the pre-development time of concentration. As a general rule and when sufficiently detailed development plans are not available, the post-development time of concentration can be estimated to be about 75% of the pre-development value.

3.2.6 Common Error in Calculating Time of Concentration

A common error in estimating the time of concentration occurs when a designer does not check the peak runoff generated from smaller portions of the catchment that may have a significantly shorter time of concentration (and, therefore, a higher rainfall intensity) than the drainage basin as a whole. Sometimes calculations using the Rational Method for a lower, urbanized portion of a watershed will produce a higher peak runoff than the calculations for the drainage basin as a whole, especially if the drainage basin is long or the upper portion has little or no impervious cover.

3.3 Rainfall Intensity (I)

The average rainfall intensity (I), in inches per hour, by recurrence interval, can be found from the Intensity-Duration-Frequency curves provided in Figure 6-5. The value for I is based on the assumption that the peak runoff will occur when the duration of the rainfall is equal to the time of concentration. For example, Figure 6-5 indicates a rainfall intensity of approximately 5.00 inches/hour for the 100-year event for a catchment with a time of concentration of 20 minutes. These curves are based on the rainfall depths for an elevation of 6,840 feet in the Colorado Springs area. IDF curves for other elevations or locations can be created using the UD-Rain spreadsheet based on 6-hour and 24-hour rainfall depths for each recurrence interval needed. The Z-1 (Zone 1) tab should be used for Arkansas River basin locations.

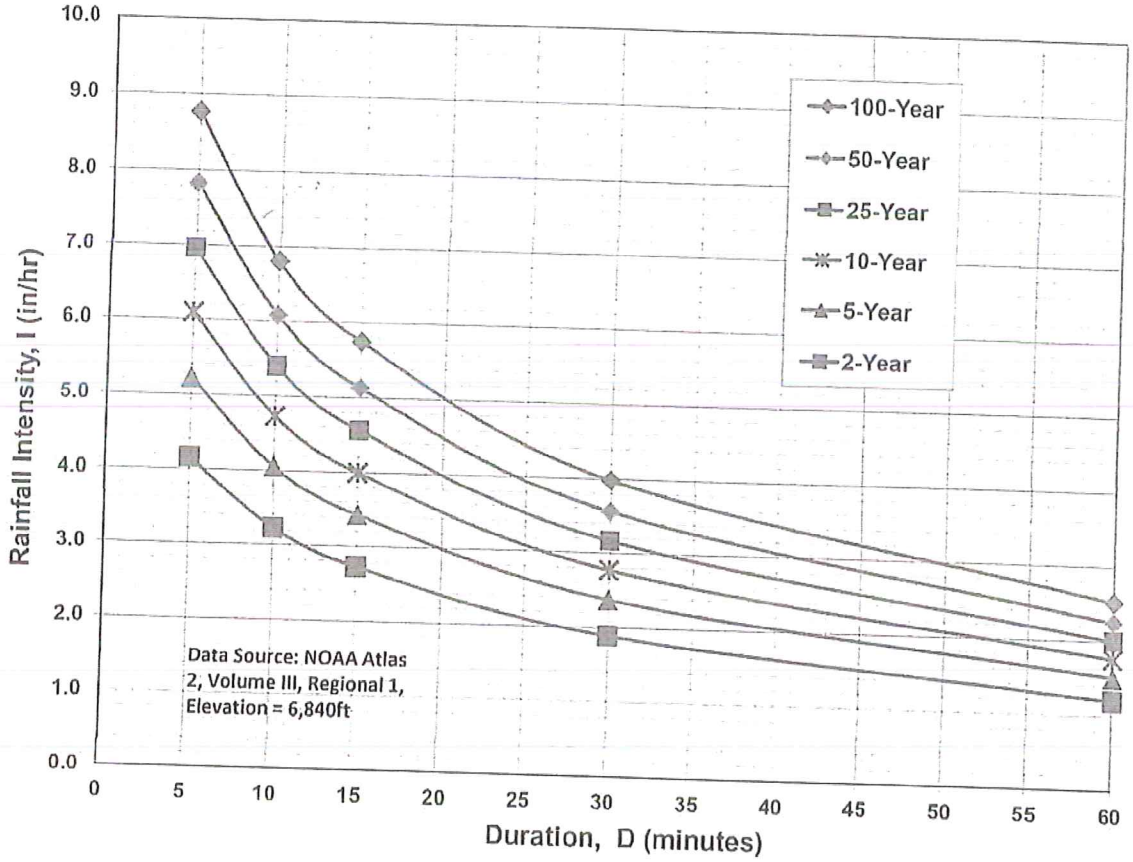
3.4 Drainage Basin Area (A)

The size of a drainage basin contributing runoff to a design point, in acres, is used to calculate peak runoff in the Rational Method. Accurately delineating the area contributing to each design point is one of the most important tasks for hydrologic analyses since the estimated runoff is directly proportional to the basin area. The area may be determined through the use of planimetric-topographic maps, supplemented by field surveys where topographic data has changed or where the contour interval is too great to distinguish the direction of flow. The drainage basin lines are determined by the natural topography, pavement slopes, locations of downspouts and inlets, paved and unpaved yards, grading of lawns, and many other features found on the urban landscape. In areas where there are storm drains, the entire contributing drainage area can sometimes be greater than the drainage area determined by topographic analysis of the ground surface, due to storm drains collecting runoff from areas that lie outside of the surface topographic extent of the basin.

4.0 NRCS Curve Number Loss and Dimensionless Unit Hydrograph Method

The NRCS curve number loss and dimensionless unit hydrograph method has used been the most widely used method in the region. It can be applied for drainage basins as small as 10 acres and is the only method that should be applied for drainage basins larger than 640 acres. This method can be used to estimate peak flows or to produce a runoff hydrograph and also provides estimates of runoff volume.

Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



IDF Equations

$I_{100} = -2.52 \ln(D) + 12.735$

$I_{50} = -2.25 \ln(D) + 11.375$

$I_{25} = -2.00 \ln(D) + 10.111$

$I_{10} = -1.75 \ln(D) + 8.847$

$I_{5} = -1.50 \ln(D) + 7.583$

$I_{2} = -1.19 \ln(D) + 6.035$

Note: Values calculated by equations may not precisely duplicate values read from figure.

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

*For buried riprap, select C_v value based on type of vegetative cover.

The travel time is calculated by dividing the flow distance (in feet) by the velocity calculated using Equation 6-9 and converting units to minutes.

The time of concentration (t_c) is then the sum of the overland flow time (t_i) and the travel time (t_t) per Equation 6-7.

3.2.3 First Design Point Time of Concentration in Urban Catchments

Using this procedure, the time of concentration at the first design point (typically the first inlet in the system) in an urbanized catchment should not exceed the time of concentration calculated using Equation 6-10. The first design point is defined as the point where runoff first enters the storm sewer system.

$$t_c = \frac{L}{180} + 10 \qquad \text{(Eq. 6-10)}$$

Where:

t_c = maximum time of concentration at the first design point in an urban watershed (min)

L = waterway length (ft)

Equation 6-10 was developed using the rainfall-runoff data collected in the Denver region and, in essence, represents regional "calibration" of the Rational Method. Normally, Equation 6-10 will result in a lesser time of concentration at the first design point and will govern in an urbanized watershed. For subsequent design points, the time of concentration is calculated by accumulating the travel times in downstream drainageway reaches.

3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

3.2.5 Post-Development Time of Concentration

As Equation 6-8 indicates, the time of concentration is a function of the 5-year runoff coefficient for a drainage basin. Typically, higher levels of imperviousness (higher 5-year runoff coefficients) correspond to shorter times of concentration, and lower levels of imperviousness correspond to longer times of

trol were developed. These nomographs give headwater-discharge relationships for most conventional culverts flowing with inlet control through a range of headwater depths or discharges. An example of these nomographs is shown in *Figure 3.25*.

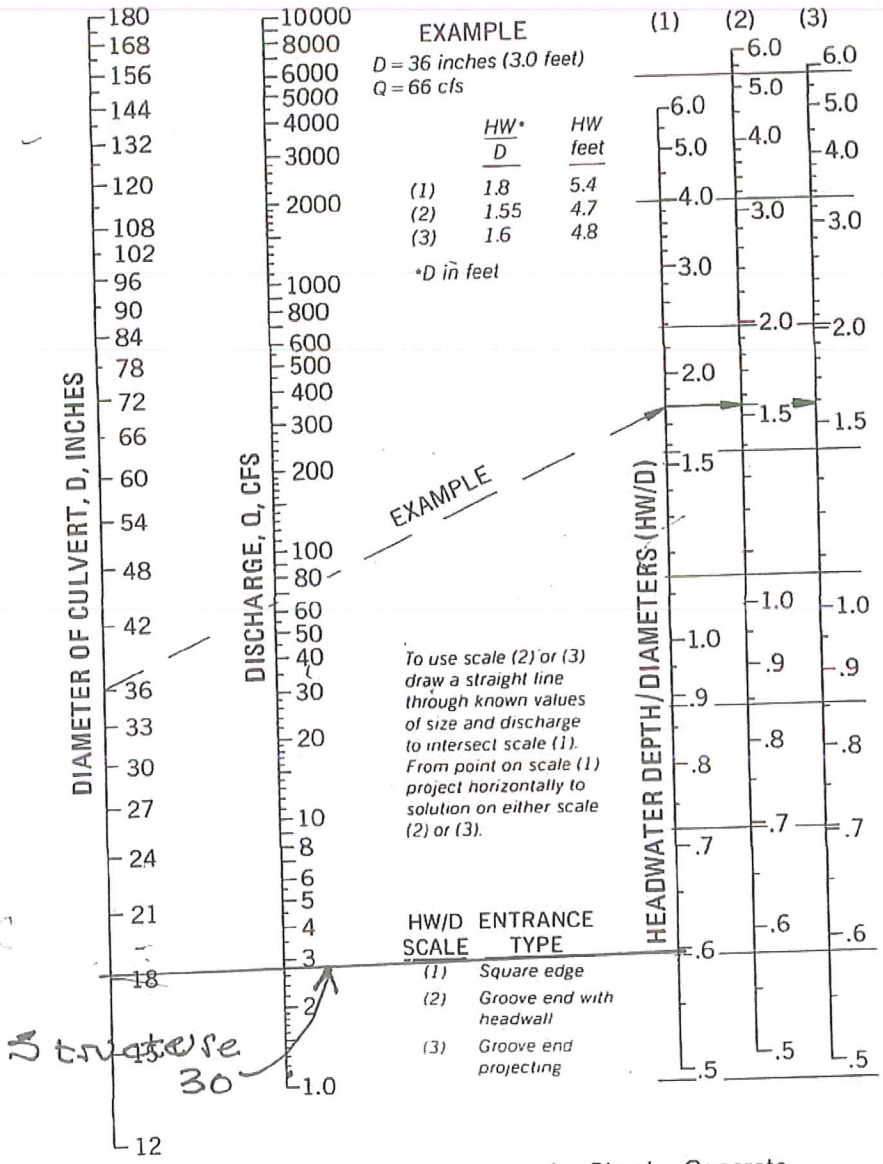


Figure 3.25. Headwater Depth for Circular Concrete Pipe Culverts with Inlet Control.

Exhibit 5

Calculation Sheets

KCH Engineering Solutions
5228 Cracker Barrel Circle
Colorado Springs, CO 80917
(719) 246-4471

JOB Security Fire Station
SHEET NO. 4 OF
CALCULATED BY K. Harrison DATE 10-6-20
CHECKED BY DATE
SCALE

Inlet (cont)

1. Location: STR 6 (DP 17)

2. Calc Sheet: #27 & #28a,b,c

3 Condition: on grade 4%, cross slope, assume 2%

4. Design Flow

Bypass @ STR 25 = 0.6 cfs
Runoff from Sub basin K $Q_5 = 0.7$ $Q_{100} = 1.3$
Design Discharge = 1.3 + 0.6 = 1.9 cfs

5. Inlet Type
Nyoplast inlet: Double 2' x 3' Curb Inlet Diagonal
Interceptor: 1.1 cfs
Bypass 0.8 cfs

6. Ck. 10' DIOR option
Parameters: See CS 28b

Depth @ Curb = max 6"
Ht of Curb 6"
Distance to Crown = 12' (assume)
Gutter width = use 24"
Transverse slope = 2% (assume)
Gutter cross slope = 8.3% (Typical, standard)
Long Slope = 4.0%
Allowable spread = 12 ft for both minor & major
Max Allow depth
12' * 0.02 = 0.24' or 2.9"
Interception = 1.9 cfs
Bypass = 0 cfs

KCH Engineering Solutions
5228 Cracker Barrel Circle
Colorado Springs, CO 80917
(719) 246-4471

JOB
SHEET NO. OF
CALCULATED BY DATE
CHECKED BY DATE
SCALE

Inlet Design

Assumptions (in report)

1. Cross slope = 2%
2. Allowable Depth = 6"
3. Design Flow = includes 25% clogging, 100 year storm event
4. Design Storm = 100 year
5. Pavement Cross Section = Uniform
6. Safety factor = 25% clogging

CS #24
JTR #1 (DP3)

Nyloplast
A division of ADS

Nyloplast Inlet Capacity Table

DISCLAIMER: SAFETY FACTORS ARE NOT INCLUDED IN THESE CALCULATIONS. ACTUAL CALCULATIONS SHOULD BE CARRIED OUT AND VERIFIED BY THE DESIGN ENGINEER TAKING INTO ACCOUNT ALL LOCAL CONDITIONS. NYLOPLAST RECOMMENDS USING A MINIMUM SAFETY FACTOR OF 1.25 FOR PAVED AREAS AND 2.0 FOR TURF AREAS. ADS/NYLOPLAST IS NOT RESPONSIBLE FOR MISUSE OF THIS TOOL.

12' Sump Inlet, 100yr = 0.3 cfs, 50% clogging
Clogging = 0.3 + 1.5(0.3) = 0.45
use 0.5

Input

Type of Grate	12" Standard
Head (ft)	0.2

Properties

Orifice Flow Area (in)	60.62
Orifice Flow Area (ft)	0.42
Weir Flow Perimeter (in)	43.75
Weir Flow Perimeter (ft)	3.65

Solution

Capacity (cfs)	0.90
Capacity (gpm)	404.07

$$Q_{weir} = CLH^{3/2}$$

$C = 3.33$ *Weir Discharge Coefficient*

$L =$ *Perimeter of Grate Opening (ft)*

$H =$ *Flow Height of Water Surface Above Weir (ft)*

$$Q_{orifice} = CA\sqrt{2gh}$$

$C = 0.60$ *Orifice Discharge Coefficient*

$A =$ *Area of the Orifice* (ft^2)

$g =$ *Gravitational Constant* $\left(32.2\frac{ft}{s^2}\right)$

$H =$ *Depth of Water Above Center of Orifice (ft)*

REV 5.4.12

KCH Engineering Solutions
5228 Cracker Barrel Circle
Colorado Springs, CO 80917
(719) 246-4471

JOB: Security Fire Station
SHEET NO. 2 OF
CALCULATED BY: K. Hanson DATE: 10-6-20
CHECKED BY: DATE:
SCALE:

Inlet Sizing (cont)

1. Location: STR 2, DP 4

2. Calc Sheet: #25

3. Condition: Sump, curb & gutter (6" curb)

4. Input
Based on depth @ building
Slope 0.7%, distance = 20', Allowable depth = 0.14 ft (1.7 inches)
Cross Slope 0.7% (No cross flow permitted)

5. Inlet Size & Type

2' x 2' Curb Inlet Standard

6. Design Flow = 0.2 cfs or 0.3 cfs w/ 50% clogging

7. Headwater = 0.1 ft, 0.4 cfs intercept

CS #25
STR 2 (DP4)

Nyloplast Inlet Capacity Table

DISCLAIMER: SAFETY FACTORS ARE NOT INCLUDED IN THESE CALCULATIONS. ACTUAL CALCULATIONS SHOULD BE CARRIED OUT AND VERIFIED BY THE DESIGN ENGINEER TAKING INTO ACCOUNT ALL LOCAL CONDITIONS. NYLOPLAST RECOMMENDS USING A MINIMUM SAFETY FACTOR OF 1.25 FOR PAVED AREAS AND 2.0 FOR TURF AREAS. ADS/NYLOPLAST IS NOT RESPONSIBLE FOR MISUSE OF THIS TOOL.

2' by 2' Curb Inlet (standard), Q_{100} = 0.2 cfs w/ 50% clogging

Input

Type of Grate	12" Standard
Head (ft)	0.1

Properties

Orifice Flow Area (in)	60.62
Orifice Flow Area (ft)	0.42
Weir Flow Perimeter (in)	43.75
Weir Flow Perimeter (ft)	3.65

Solution

Capacity (cfs)	0.38
Capacity (gpm)	172.30

$$Q_{weir} = CLH^{3/2}$$

$C = 3.33\ Weir\ Discharge\ Coefficient$

$L = Perimeter\ of\ Grate\ Opening\ (ft)$

$H = Flow\ Height\ of\ Water\ Surface\ Above\ Weir\ (ft)$

$$Q_{orifice} = CA\sqrt{2gh}$$

$C = 0.60\ Orifice\ Discharge\ Coefficient$

$A = Area\ of\ the\ Orifice\ (ft^2)$

$g = Gravitational\ Constant\ \left(32.2\frac{ft}{s^2}\right)$

$H = Depth\ of\ Water\ Above\ Center\ of\ Orifice\ (ft)$

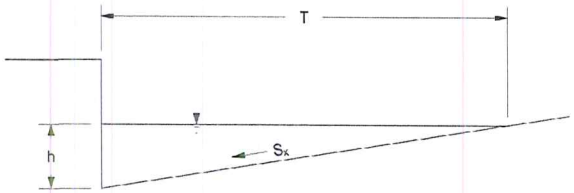
REV 5.4.12

Nyloplast Curb & Gutter Inlet Capacity Calculator - Uniform Cross Section

EQUATIONS AND CALCULATIONS ARE BASED OFF USDOT/FHWA URBAN DESIGN MANUAL, HYDRAULIC ENGINEERING CIRCULAR NO. 22, THIRD EDITION, PUBLICATION NO. FHWA-NHI-10-009.

Curb & Gutter Design Inputs

Surface Type	Concrete pavement, broom finish	Drop Down
Mannings Coeffiecient for Street & Pavement Gutters	0.016	
T (ft)	7.1	Enter Units
S_x (ft/ft)	0.020	Enter Units
S_L (ft/ft)	0.040	Enter Units
h (in)	1.70	
Gutter Flow (cfs)	1.91	
Gutter Flow (gpm)	856.57	
Gutter Velocity (fps)	3.75	



UNIFORM GUTTER CROSS SECTION

T - SPREAD
S_x - TRAVERSE SLOPE
S_L - LONGITUDINAL SLOPE
h - DEPTH OF WATER AT GUTTER

Output

Grate Style	Double 2'x3' Curb Inlet Diagonal	Drop Down
Intercepted Flow (cfs)	1.149	
Intercepted Flow (gpm)	515.52	
Carryover Flow (cfs)	0.760	
Carryover Flow (gpm)	341.047	

DISCLAIMER: SAFETY FACTORS ARE NOT INCLUDED. ACTUAL CALCULATIONS SHOULD BE CARRIED OUT AND VERIFIED BY DESIGN ENGINEER TAKING INTO ACCOUNT ALL LOCAL CONDITIONS. FAA RECOMMENDS USING A SAFETY FACTOR OF 1.25 FOR PAVED AREAS AND 2.0 FOR TURF AREAS. ADS/NYLOPLAST IS NOT RESPONSIBLE FOR MISUSE OF THIS TOOL.

CS #27
At Grade Inlet Sta 8
Q_{100} = 1.9cfs
Intercept: 1.2cfs
Bypass: 0.6cfs
Not Selected
See 10' DIOR
Calc Sheets

KCH Engineering Solutions
5228 Cracker Barrel Circle
Colorado Springs, CO 80917
(719) 246-4471

JOB
SHEET NO. OF
CALCULATED BY DATE
CHECKED BY DATE
SCALE

Developed Runoff Coefficients

Gravel Drive
Lot 1 - 250ft * 20ft = 0.11 Acres (Lot B)
Lot 2 : 420ft * 20ft = 0.19 Acres (Lot A)
$C_5 = 0.59$ $C_{100} = 0.70$

Roof : 3000 ft² / 43560 = 0.07 Acres per both lot 1 & 2

$C_5 = 0.90$ $C_{100} = 0.96$

Landscape
Each lot 1/2 acre $C_5 = 0.12$ $C_{100} = 0.39$

Cummulative C
Lot A (5 acres)
5yr (0.11*0.59+0.07*(0.90) + 0.5*0.12)+4.3(0.08)/5.0 = 0.11

100yr (0.11*0.59)+0.07(0.96)+0.5(0.39)+4.3(0.35)/5.0 = 0.37

Lot B (5.5 Acres)
5yr: (0.19)(0.59+0.07(0.90) + 0.5(0.12) + 4.74(0.08)/5.5 = 0.11 5y

100yr: 0.19*0.70 + 0.07*0.96 + 0.5*0.39 + 4.74*0.35/5.5 = 0.37

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JOB McGehee Track
SHEET NO. 1 OF 1
CALCULATED BY K. Harrison DATE 10/28/20
CHECKED BY _____ DATE _____
SCALE _____

Commulative Runoff

A. DP 1
1. Areas: OS1, OS3, OS4 = 35.3 acres
2. Runoff Coef: see excel spreadsheet
3. Time of concentration = 23.2 min

B DP 5
1. Areas: DP1 + OS5 + OS7, + A
35.3 + 0.4 + 3.6 = 39.3 acres
2. Runoff Coef: see excel spreadsheet
3. T_c: DP1 + channel from DP1 to DP5

$V = C_v S_w^{0.5}$ where $C_v = 15$ (grassed waterway)
$S = 20ft / 600 = 3.3\%$

$V = 15 (0.033)^{0.5} = 2.6$ fps ok
$T_c = 600/2.6 * 60 = 3.8$ min

Total T_c @ DP5 = 23.2 + 3.8 = $\boxed{27.0 \text{ min}}$

PRODUCT 204-1 (Single Sheets) 205-1 (Padded)

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JOB McGehee Tract
SHEET NO. 2 OF -
CALCULATED BY K. Harrison DATE 10/28/20
CHECKED BY DATE
SCALE

C. DP 6 (Pond Embankment Outlet pipe

A Area: DP 5 + B + OS.2
= 39.3 + 3.5 + 10.3 = 53.1 acres

B T_c
DP 6 + swale (DP 5 to DP6)

Swale Travel Time
$C_v = 15$ (grassed lined swale)
$S_w = 5/200 = 2.5\%$
$T_T = 15 * (0.025)^{1/2}$
$= 2.4$ fpm
Travel Time = 200/2.4*60 = 1.4 min

$\boxed{\text{Total } T_c \text{ @ DP 6} = 27.0 + 1.4 = 28.4 \text{ min}}$

D. DP 7 (Thompson Road culvert)

1. Area + DP 6 + OS6 + D + OS9 + OS10 + C
= 53.1 + 0.9 + 1.4 + 0.4 + 0.5 + 0.8
= 57.1 Acres

2. Travel Time DP6 to DP7
Channel Time $C_v * (S)^{1/2}$
$C_v = 15$ L = 100 Fall = 4'
$S_w = 4/100 = 4.0\%$

$V = 15 * (0.04)^{1/2} = 3.0$ fpm ok

$T_T = 100/3.0*60 = 0.6$ min

Time of Concentration @ DP 7 =
$\boxed{28.4 + 0.6 = 29.0 \text{ min}}$

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JOB Security Fire Station
SHEET NO. 1 OF
CALCULATED BY K. Hamson DATE 10-6-20
CHECKED BY DATE
SCALE

Inlet Sizing

1 Location = STR 1 (DP3)

2 Calc Sheet = #24

3. Condition = Sump

4 Allowable depth ≈ 0.5 ft (6")

5 Inlet Size & type 12" standard

6 Design Flow (100 year) = 0.3 cfs

7. % clogging = 50%,

8 Updated Design flow 0.75 cfs or 0.8 cfs

9. Headwater = 0.90 cfs

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JOB: Security Fire Station
SHEET NO. 3 OF
CALCULATED BY: K. Harrison DATE: 12-16-20
CHECKED BY: DATE:
SCALE:

Inlets (cont)

1. Location: Str #19 DP 6 Subbasin E

2. Design Flow: $Q_{5yr} = 1.0$ cfs $Q_{100yr} = 1.8$ cfs use 100 year for Subbasin E
w/ clogging factor of 25% = 1.8 + 0.25(1.8) = 2.25 use 2.3 cfs

3. Design Values
Cross Slope = 2%
Max Depth @ inlet = 0.5'
Slope = 1.6%
T = 0.5'/0.02 = 25 ft use 1/2 of roadway width
1/2 (22') = 11 ft

4. Type Inlet = 2' x 3' Wyoplate Curb Inlet Diagonal
Design Flow = 2.3 cfs
Interception = 1.7 cfs
Bypass = 0.6 cfs

CS #26
At grade inlet STR 19
Q_{100} = 2.3 cfs (includes 25%)
Intercept 1.7 cfs Bypass = 0.6 cfs

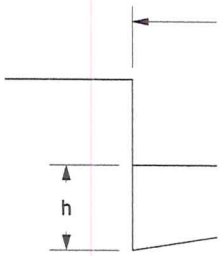
Nyloplast
A division of ADS

Nyloplast Curb & Gutter Inlet Capacity Calculator - Uniform Cross Section

EQUATIONS AND CALCULATIONS ARE BASED OFF USDOT/FHWA URBAN DESIGN MANUAL, HYDRAULIC ENGINEERING CIRCULAR NO. 22, THIRD EDITION, PUBLICATION NO. FHWA-NHI-10-009.

Curb & Gutter Design Inputs

Surface Type	Asphalt Pavement, rough texture	Drop Down
Mannings Coeffiecient for Street & Pavement Gutters	0.016	
T (ft)	11	Enter Units
S_x (ft/ft)	0.020	Enter Units
S_L (ft/ft)	0.016	Enter Units
h (in)	2.64	
Gutter Flow (cfs)	3.88	
Gutter Flow (gpm)	1743.62	
Gutter Velocity (fps)	3.18	



UNIF

Output

Grate Style	2'x3' Curb Inlet Diagonal	Drop Down
Intercepted Flow (cfs)	1.706	
Intercepted Flow (gpm)	765.79	
Carryover Flow (cfs)	2.179	
Carryover Flow (gpm)	977.825	

DISCLAIMER: SAFETY FACTORS ARE NOT INCLUDED. ACTUAL CALCULATIONS SHOULD BE CARRIED OUT AND VERIFIED BY DESIGN ENGINEER TAKING INTO ACCOUNT ALL LOCAL CONDITIONS. FAA RECOMMENDS USING A SAFETY FACTOR OF 1.25 FOR PAVED AREAS AND 2.0 FOR TURF AREAS. ADS/NYLOPLAST IS NOT RESPONSIBLE FOR MISUSE OF THIS TOOL.

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JOB: Security Fire Station
SHEET NO. 24 OF
CALCULATED BY: K. Harrison DATE: 12-16-20
CHECKED BY: DATE:
SCALE:

Inlets (Cont)

1 Location: STR 6, Subbasin J, DP 13

2. Design Flow: $Q_5 = 0.7$ $Q_{100} = 1.3$
25% clogging factor $Q_{100} = 1.6$ cfs
Bypass STR 19 $Q_{100} = 0.6$
Total Design Flow $Q_{100} = 2.2$ cfs

3. Design Values
Cross slope = 2%
Max Depth @ inlet = 0.5'
Slope = 4.8%
T = assume 11 ft

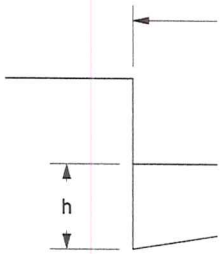
4. Type Inlet = 2' x 3' Curb Inlet Diagonal
Interception = 2.5 cfs
Bypass = 0 cfs

Nyloplast Curb & Gutter Inlet Capacity Calculator - Uniform Cross Section

EQUATIONS AND CALCULATIONS ARE BASED OFF USDOT/FHWA URBAN DESIGN MANUAL, HYDRAULIC ENGINEERING CIRCULAR NO. 22, THIRD EDITION, PUBLICATION NO. FHWA-NHI-10-009.

Curb & Gutter Design Inputs

Surface Type	Asphalt Pavement, rough texture	Drop Down
Mannings Coeffiecient for Street & Pavement Gutters	0.016	
T (ft)	11	Enter Units
S_x (ft/ft)	0.020	Enter Units
S_L (ft/ft)	0.048	Enter Units
h (in)	2.64	
Gutter Flow (cfs)	6.73	
Gutter Flow (gpm)	3020.03	
Gutter Velocity (fps)	5.51	



UNIF

Output

Grate Style	2'x3' Curb Inlet Diagonal	Drop Down
Intercepted Flow (cfs)	2.532	
Intercepted Flow (gpm)	1136.26	
Carryover Flow (cfs)	4.197	
Carryover Flow (gpm)	1883.771	

DISCLAIMER: SAFETY FACTORS ARE NOT INCLUDED. ACTUAL CALCULATIONS SHOULD BE CARRIED OUT AND VERIFIED BY DESIGN ENGINEER TAKING INTO ACCOUNT ALL LOCAL CONDITIONS. FAA RECOMMENDS USING A SAFETY FACTOR OF 1.25 FOR PAVED AREAS AND 2.0 FOR TURF AREAS. ADS/NYLOPLAST IS NOT RESPONSIBLE FOR MISUSE OF THIS TOOL.

C5 #27
Inlet STR6, DD13
Intercepted = 2.53cfs
Bypass = 0cfs

Exhibit 6

Hydrologic Calculations

Design Point Summary

Existing Conditions

DP	Contributing Sub basins	Area	Tc	C5	C100	CA5	CA100	Q5	Q100
1	OS1	34.1	23.2	0.09	0.36			8.7	58.5
2	OS3	0.6	15	0.08	0.35			0.2	1.2
3	OS4	0.6	13.3	0.17	0.41			0.4	1.5
4	OS7	3.6	15.5	0.12	0.38			1.5	8.0
5	OS1,OS3, OS4, OS5,OS7,A	45.6	27			4.16	16.31	11..0	72.2
6	DP5,OS6,B,OS2	60.3	28.4			5.47	21.28	14.0	91.5
7	DP6, C, OS8	62.5	29			5.71	22.37	14.5	95.0
8	OS5	0.4	10.9	0.08	0.35			0.1	0.9
9	OS6	0.9	15.2	0.08	0.35			0.3	1.9
10	OS2	10.3	17.9	0.10	0.36			3.3	20.5
11	OS8	1.4	14.1	0.08	0.35			0.4	3.0
12	OS10	0.5	5.3	0.57	0.72			1.5	3.1
13	OS9	0.4	6.1	0.70	0.81			1.4	2.6
14	DP7, OS9, OS10	61.5	29			6.28	23.05	15.9	97.9
	Lot 1 located in sub basin B	5.5	12.9	0.08	0.35			1.8	13.3
	Lot 2 Located in sub basin A	5.0	14.8	0.08	0.35			1.2	8.9

Design Point Summary

Developed Conditions

DP	Contributing Sub basins	Area	Tc	C5	C100	CA5	CA100	Q5	Q100
1	OS1	34.1	23.2	0.09	0.36			8.7	58.5
2	OS3	0.6	15	0.08	0.35			0.2	1.2
3	OS4	0.6	13.5	0.17	0.41			0.3	1.3
4	OS7	3.6	15.5	0.12	0.38			1.5	8.0
5	OS1,OS3, OS4, OS5,OS7,A	45.6	27			4.28	16.39	11.3	72.6
6	DP5,OS6,B,OS2	60.3	28.4			5.66	21.42	14.5	92.1
7	DP6, C, OS8	62.5	29			5.71	22.37	14.5	95.0
8	OS5	0.4	10.9	0.08	0.35			0.1	0.9
9	OS6	0.9	15.2	0.08	0.35			0.3	1.9
10	OS2	10.3	17.9	0.10	0.36			3.3	20.5
11	OS8	1.4	14.1	0.08	0.35			0.4	3.0
12	OS10	0.5	5.3	0.57	0.72			1.5	3.1
13	OS9	0.4	6.1	0.70	0.81			1.4	2.6
14	DP7, OS9, OS10	63.4	29			6.47	23.18	16.4	98.5
	Lot 1 located in sub basin B	5.5	12.9	0.11	0.37			1.7	9.4
	Lot 2 Located in sub basin A	5.0	14.8	0.08	0.35			2.5	14.1

Cummulative Acreage at Design Points

DP	Sub Basin ID	Sub basin Area	Sub total
1	OS1	34.1	34.1
5	OS1	34.1	
	OS3	0.6	
	OS4	0.6	
	OS5	0.4	
	A	6.3	
5	OS7	3.6	45.6
6	DP5	45.6	
	OS6	0.9	
	B	3.5	
6	OS2	10.3	60.3
7	DP6	60.3	
	C	0.8	
7	OS8	1.4	62.5
14	DP7	62.5	
	OS9	0.4	
14	OS10	0.5	63.4

Stormwater Runoff Summary Comparison

Existing Conditions

DP	Contributing Sub basins	Area	Tc	C5	C100	CA5	CA100	Q5	Q100
14	DP7, OS9, OS10	63.4	29			6.28	23.05	15.9	97.9
	Lot 1 (sub basin B)	5.5	12.9	0.11	0.37			1.8	13.3
	Lot 2 (Sub basin A)	5.0	14.8	0.11	0.37			1.2	8.9

Developed Conditions

			Rational						
DP	Contributing Sub basins	Area	Tc	C5	C100	CA5	CA100	Q5	Q100
14	DP7, OS9, OS10	63.4	29			6.47	23.18	16.4	98.5
	Lot 1 (sub basin B)	5.5	12.9	0.11	0.37			2.5	14.1
	Lot 2 (Sub basin A)	5.0	14.8	0.11	0.37			1.7	9.4

Notes

1. Design Point 14 is located at the upstream end of the culvert under Thompson Road
2. The majority of Lot 2 is located in sub basin A.
3. The majority of Lot 1 is located in sub basin B.
4. All of the runoff from the sub basins not shown in the above table remains The same as shown on the Existing Conditions Drainage Plan
5. CA values are used in order to accurately reflect controlling Tc

MCGEHEE TRACT
FINAL DRAINAGE REPORT
(Area Drainage Summary)
Existing Conditions

See surface flow routing table for total combined flow at culvert under Thompson Road

From Area Runoff Coefficient Summary				OVERLAND				CHANNEL FLOW				Time of Travel (T_t)		INTENSITY *		TOTAL FLOWS	
BASIN	AREA TOTAL	C_5	C_{100}	C_5	Length	Height	T_C	Length	Slope	Velocity	T_t	TOTAL	CHECK	I_5	I_{100}	Q_5	Q_{100}
	(Acres)	From DCM Table 5-1			(ft)	(ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
OS1	34.10	0.09	0.36	0.09	300	40	13.4	1900	4.7%	3.3	9.7	23.2	22.2	2.9	4.8	*8.7*	*58.5*
OS2	10.30	0.10	0.36	0.10	300	20	16.7	300	8.3%	4.3	1.2	17.9	13.3	3.3	5.5	*3.3*	*20.5*
OS3	0.60	0.08	0.35	0.08	300	30	14.9	1	0.1%	0.5	0.0	15.0	11.7	3.5	5.9	*0.2*	*1.2*
OS4	0.60	0.17	0.41	0.17	300	35	13.0	100	10.0%	4.7	0.4	13.3	12.2	3.7	6.2	*0.4*	*1.5*
OS5	0.40	0.08	0.35	0.08	100	5	10.8	1	0.1%	0.5	0.0	10.9	10.6	4.0	6.7	*0.1*	*0.9*
OS6	0.90	0.08	0.35	0.08	150	5	15.2	1	0.1%	0.5	0.0	15.2	10.8	3.5	5.9	*0.3*	*1.9*
OS7	3.60	0.12	0.38	0.12	300	25	15.2	100	15.0%	5.8	0.3	15.5	12.2	3.5	5.8	*1.5*	*8.0*
OS8	1.40	0.08	0.35	0.08	50	0.5	13.0	250	7.3%	4.1	1.0	14.1	11.7	3.6	6.1	*0.4*	*3.0*
OS9	0.40	0.70	0.81	0.70	20	0.5	2.4	550	2.7%	2.5	3.7	6.1	13.2	4.9	8.2	*1.4*	*2.6*
OS10	0.50	0.57	0.72	0.57	20	0.5	3.2	550	8.2%	4.3	2.1	5.3	13.2	5.1	8.5	*1.5*	*3.1*
A	6.30	0.08	0.35	0.08	200	20	12.2	550	5.5%	3.5	2.6	14.8	14.2	3.5	5.9	*1.8*	*13.1*
B	3.50	0.08	0.35	0.08	150	10	12.1	175	5.7%	3.6	0.8	12.9	11.8	3.8	6.3	*1.1*	*7.7*
C	0.80	0.08	0.35	0.08	150	10	12.1	250	10.0%	4.7	0.9	12.9	12.2	3.7	6.3	*0.2*	*1.8*
Sub Total	63.40																
Lots to be developed																	
Lot 2 located in Sub basin A	5.00	0.08	0.35									14.8	10.0	3.1	5.1	*1.2*	*8.9*
Lot 1 located in Sub basin B	5.50	0.08	0.35									12.9	10.0	4.1	6.9	*1.8*	*13.3*

MCGEHEE TRACT
FINAL DRAINAGE REPORT
(Surface Routing Summary- surface runoff)
Existing Conditions

From Area Runoff Coefficient Summary					Time of Travel (T_t)	INTENSITY *		TOTAL FLOWS		
DESIGN POINT	Swale ID	CONTRIBUTING BASINS	CA_5	CA_{100}	TOTAL (min)	I_5 (in/hr)	I_{100} (in/hr)	Q_5 (c.f.s.)	Q_{100} (c.f.s.)	COMMENTS
DP1	*S1*	OS1, OS3, OS4	3.18	12.60	23.2	2.9	4.8	*9.1*	*60.6*	west property line
DP5	*S2*	DP1,OS5,OS7,A	4.16	16.31	27.0	2.6	4.4	*11.0*	*72.2*	entrance to the stock pond
DP6	*S3*	DP5, B, OS2	5.47	21.28	28.4	2.6	4.3	*14.0*	*91.5*	outlet to the pond
DP7	*S4*	DP6,OS6,C,OS8	5.71	22.37	29.0	2.5	4.2	*14.5*	*95.0*	Thompson Road culvert
DP14	*S4*	DP7,OS9,OS10	6.28	23.05	29.0	2.5	4.2	*15.9*	*97.9*	Thompson Road culvert
minimal change from DP7										

MCGEHEE TRACT
DRAINAGE CALCULATIONS
(Area Runoff Coefficient Summary)
Existing Conditions

See surface flow routing table for total combined flow at culvert under Thompson Road

		GRAVEL/ ASPHALT ROADS			*DEVELOPED LOTS*			*NATURAL*			*RUNOFF COEFFICIENT*	
BASIN	TOTAL AREA *(Acres)*	AREA *(Acres)*	C_5	C_{100}	AREA *(Acres)*	C_5	C_{100}	AREA *(Acres)*	C_5	C_{100}	C_5	C_{100}
OS1	34.10	0.60	0.59	0.70	0.00	0.38	0.57	33.50	0.08	0.35	*0.09*	*0.36*
OS2	10.30	0.40	0.59	0.70	0.00	0.38	0.57	9.90	0.08	0.35	*0.10*	*0.36*
OS3	0.60	0.00	0.59	0.70	0.00	0.38	0.57	0.60	0.08	0.35	*0.08*	*0.35*
OS4	0.60	0.10	0.59	0.70	0.00	0.38	0.57	0.50	0.08	0.35	*0.17*	*0.41*
OS5	0.40	0.00	0.59	0.70	0.00	0.38	0.57	0.40	0.08	0.35	*0.08*	*0.35*
OS6	0.90	0.00	0.59	0.70	0.00	0.38	0.57	0.90	0.08	0.35	*0.08*	*0.35*
OS7	3.60	0.30	0.59	0.70	0.00	0.38	0.57	3.30	0.08	0.35	*0.12*	*0.38*
OS8	1.40	0.00	0.59	0.70	0.00	0.38	0.57	1.40	0.08	0.35	*0.08*	*0.35*
OS9	0.40	0.30	0.90	0.96	0.00	0.38	0.57	0.10	0.08	0.35	*0.70*	*0.81*
OS10	0.50	0.30	0.90	0.96	0.00	0.38	0.57	0.20	0.08	0.35	*0.57*	*0.72*
A	6.30	0.00	0.59	0.70	0.00	0.38	0.57	6.30	0.08	0.35	*0.08*	*0.35*
B	3.50	0.00	0.59	0.70	0.00	0.38	0.57	3.50	0.08	0.35	*0.08*	*0.35*
C	0.80	0.00	0.59	0.70	0.00	0.38	0.57	0.80	0.08	0.35	*0.08*	*0.35*
Sub Total	63.40											
Lots to be developed												
Lot 2 located in Sub basin A	5.00										*0.08*	*0.35*
Lot 1 located in Sub basin B	5.50										*0.08*	*0.35*

MCGEHEE TRACT
DRAINAGE CALCULATIONS
(Sub basin Runoff Coefficient Summary)
Developed Conditions

See surface flow routing table for total combined flow at culvert under Thompson Road

		GRAVEL/ ASPHALT ROADS			*ROOFS*			*NATURAL*			*RUNOFF COEFFICIENT*	
BASIN	TOTAL AREA *(Acres)*	AREA *(Acres)*	C_5	C_{100}	AREA *(Acres)*	C_5	C_{100}	AREA *(Acres)*	C_5	C_{100}	C_5	C_{100}
OS1	34.10	0.60	0.59	0.70	0.00	0.38	0.57	33.50	0.08	0.35	*0.09*	*0.36*
OS2	10.30	0.40	0.59	0.70	0.00	0.38	0.57	9.90	0.08	0.35	*0.10*	*0.36*
OS3	0.60	0.00	0.59	0.70	0.00	0.38	0.57	0.60	0.08	0.35	*0.08*	*0.35*
OS4	0.60	0.10	0.59	0.70	0.00	0.38	0.57	0.40	0.08	0.35	*0.15*	*0.35*
OS5	0.40	0.00	0.59	0.70	0.00	0.38	0.57	0.50	0.08	0.35	*0.10*	*0.44*
OS6	0.90	0.00	0.59	0.70	0.00	0.38	0.57	0.90	0.08	0.35	*0.08*	*0.35*
OS7	3.60	0.30	0.59	0.70	0.00	0.38	0.57	3.30	0.08	0.35	*0.12*	*0.38*
OS8	1.40	0.00	0.59	0.70	0.00	0.38	0.57	1.40	0.08	0.35	*0.08*	*0.35*
OS9	0.40	0.30	0.90	0.96	0.00	0.38	0.57	0.10	0.08	0.35	*0.70*	*0.81*
OS10	0.50	0.30	0.90	0.96	0.00	0.38	0.57	0.20	0.08	0.35	*0.57*	*0.72*
A	6.30	0.19	0.59	0.70	0.07	0.38	0.57	6.04	0.08	0.35	*0.10*	*0.36*
B	3.50	0.11	0.59	0.70	0.07	0.38	0.57	3.32	0.08	0.35	*0.10*	*0.37*
C	0.80	0.00	0.59	0.70	0.00	0.38	0.57	0.80	0.08	0.35	*0.08*	*0.35*
Sub Total	63.40											
Lots to be developed												
Lot 2 located in Sub Basin A	5.00										*0.11*	*0.37*
Lot 1 located in Sub Basin B	5.50										*0.11*	*0.37*

MCGEHEE TRACT
FINAL DRAINAGE REPORT
(Area Drainage Summary)
Developed Conditions

See surface flow routing table for total combined flow at culvert under Thompson Road

From Area Runoff Coefficient Summary				*OVERLAND*				*CHANNEL FLOW*				*Time of Travel (T_t)*	*INTENSITY* [a]		*TOTAL FLOWS*	
BASIN	AREA TOTAL	C_5	C_{100}	C_5	Length	Height	T_C	Length	Slope	Velocity	T_t	TOTAL	I_5	I_{100}	Q_5	Q_{100}
	(Acres)	From DCM Table 5-1			(ft)	(ft)	(min)	(ft)	(%)	(fps)	(min)	(min)	(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
OS1	34.10	0.09	0.36	0.09	300	40	13.4	1900	4.7%	3.3	9.7	23.2	2.9	4.8	*8.7*	*58.5*
OS2	10.30	0.10	0.36	0.10	300	20	16.7	300	8.3%	4.3	1.2	17.9	3.3	5.5	*3.3*	*20.5*
OS3	0.60	0.08	0.35	0.08	300	30	14.9	1	0.1%	0.5	0.0	15.0	3.5	5.9	*0.2*	*1.2*
OS4	0.60	0.15	0.35	0.15	300	35	13.2	100	10.0%	4.7	0.4	13.5	3.7	6.2	*0.3*	*1.3*
OS5	0.40	0.10	0.44	0.10	100	5	10.6	1	0.1%	0.5	0.0	10.7	4.0	6.8	*0.2*	*1.2*
OS6	0.90	0.08	0.35	0.08	150	5	15.2	1	0.1%	0.5	0.0	15.2	3.5	5.9	*0.3*	*1.9*
OS7	3.60	0.12	0.38	0.12	300	25	15.2	100	15.0%	5.8	0.3	15.5	3.5	5.8	*1.5*	*8.0*
OS8	1.40	0.08	0.35	0.08	50	0.5	13.0	250	7.3%	4.1	1.0	14.1	3.6	6.1	*0.4*	*3.0*
OS9	0.40	0.70	0.81	0.70	20	0.5	2.4	550	2.7%	2.5	3.7	6.1	4.9	8.2	*1.4*	*2.6*
OS10	0.50	0.57	0.72	0.57	20	0.5	3.2	550	8.2%	4.3	2.1	5.3	5.1	8.5	*1.5*	*3.1*
A	6.30	0.10	0.36	0.10	200	20	12.0	550	5.5%	3.5	2.6	14.8	3.5	5.9	*2.2*	*13.6*
B	3.50	0.10	0.37	0.10	150	10	11.8	175	5.7%	3.6	0.8	12.9	3.7	6.3	*1.3*	*8.0*
C	0.80	0.08	0.35	0.08	150	10	12.1	250	10.0%	4.7	0.9	12.9	3.7	6.3	*0.2*	*1.8*
Sub Totals	63.40															
Lots to be developed																
Lot 2 located in Sub Basin A	5.00	0.11	0.37									14.8	3.1	5.1	*1.7*	*9.4*
Lot 1 located in Sub Basin B	5.50	0.11	0.37									12.9	4.1	6.9	*2.5*	*14.1*

3

Date: 12/11/2019

Checked by: _____

MCGEHEE TRACT
FINAL DRAINAGE REPORT
(Surface Routing Summary- surface runoff)
Developed Conditions

From Area Runoff Coefficient Summary					Time of Travel (T_t)	INTENSITY [a]		TOTAL FLOWS		
DESIGN POINT	Swale ID	CONTRIBUTING BASINS	CA_5	CA_{100}	TOTAL (min)	I_5 (in/hr)	I_{100} (in/hr)	Q_5 (c.f.s.)	Q_{100} (c.f.s.)	COMMENTS
DP1	*S1*	OS1, OS3, OS4	3.17	12.57	23.2	2.9	4.8	*9.1*	*60.5*	west property line
DP5	*S2*	DP1,OS5,OS7,A	4.28	16.39	27.0	2.6	4.4	*11.3*	*72.6*	entrance to the stock pond
DP6	*S3*	DP5, B, OS2	5.66	21.42	28.4	2.6	4.3	*14.5*	*92.1*	outlet to the pond
DP7	*S4*	DP6,C,,OS8	5.90	22.53	29.0	2.5	4.2	*14.9*	*95.7*	
DP14	*S5*	DP7,OS9,OS10	6.47	23.18	29.0	2.5	4.2	*16.4*	*98.5*	Thompson Road culvert

Exhibit 7

Hydraulic Calculations

Swale Summary

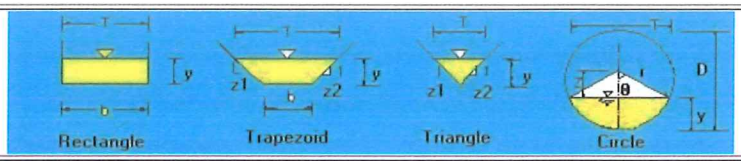
Existing Conditions (Negligible changes for developed conditions)

Swale #	Design Points	Contributing Subbasins	Slope	Design Flow		Depth of Flow		Velocity		Froude Number (5 year)	Flow Regime
				Q5	Q100	Q5	Q100	Q5	Q100		
			%	cfs	cfs	ft	ft	fps	fps		
1	1	OS1	4.7	8.7	58.5	0.3	0.7	2.9	4.8	1.21	super
2	1 to 5	OS1,OS3, OS4, A, OS7	5.5	11	72.2	0.3	0.7	3.4	5.5	1.32	super
3	5 to 6	DP5, B, OS2	8.3	14	91.7	0.3	0.8	4.4	7.2	1.66	super
4	6 to7	DP10,C, OS8	8.3	14.5	95	0.4	0.9	4.8	7.9	1.7	super
5	12	OS10	5.0	1.5	3.1	0.2	0.3	2.8	3.5	1.2	super
6	13	OS9	5.5	1.4	2.6	0.2	0.3	2.8	3.4	1.27	super

CS 1
5 year
Swale 1

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

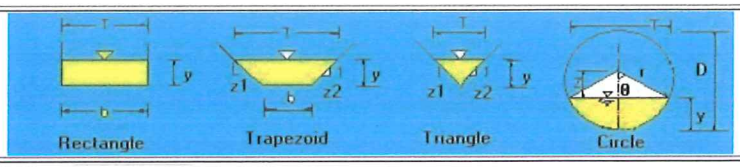
Select unit system: Feet(ft)

Channel slope: .047 ft/ft	Water depth(y): 0.28 ft	Bottom width(b) 5 ft
Flow velocity 2.9425 ft/s	LeftSlope (Z1): 20 to 1 (H:V)	RightSlope (Z2): 20 to 1 (H:V)
Flow discharge 8.7 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 16.19 ft	Flow area 2.96 ft^2	Top width(T) 16.17 ft
Specific energy 0.41 ft	Froude number 1.21	Flow status Supercritical flow
Critical depth 0.31 ft	Critical slope 0.0304 ft/ft	Velocity head 0.13 ft

CS 2
100 year
Swale 1

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

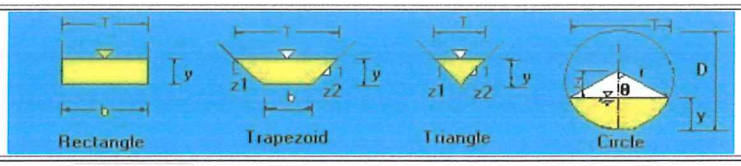
Select unit system: Feet(ft)

Channel slope: .047 ft/ft	Water depth(y): 0.66 ft	Bottom width(b) 5 ft
Flow velocity 4.851 ft/s	LeftSlope (Z1): 20 to 1 (H:V)	RightSlope (Z2): 20 to 1 (H:V)
Flow discharge 58.5 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 31.49 ft	Flow area 12.06 ft^2	Top width(T) 31.46 ft
Specific energy 1.03 ft	Froude number 1.38	Flow status Supercritical flow
Critical depth 0.77 ft	Critical slope 0.0232 ft/ft	Velocity head 0.37 ft

CS#3
5 yest, ~~[illegible]~~
swale 5

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

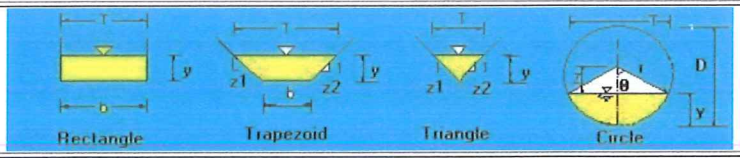
Select unit system: Feet(ft)

Channel slope: .05 ft/ft	Water depth(y): 0.21 ft	Bottom width(b) 2 ft
Flow velocity 2.784683 ft/s	LeftSlope (Z1): 3 to 1 (H:V)	RightSlope (Z2): 3 to 1 (H:V)
Flow discharge 1.5 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 3.3 ft	Flow area 0.54 ft^2	Top width(T) 3.23 ft
Specific energy 0.33 ft	Froude number 1.2	Flow status Supercritical flow
Critical depth 0.23 ft	Critical slope 0.0321 ft/ft	Velocity head 0.12 ft

C5 24
100 yr
Solve #5

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

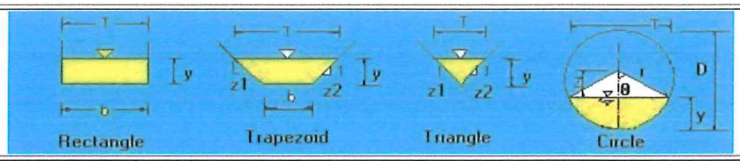
Select unit system: Feet(ft)

Channel slope: .05 ft/ft	Water depth(y): 0.3 ft	Bottom width(b) 2 ft
Flow velocity 3.504871 ft/s	LeftSlope (Z1): 3 to 1 (H:V)	RightSlope (Z2): 3 to 1 (H:V)
Flow discharge 3.1 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 3.92 ft	Flow area 0.88 ft^2	Top width(T) 3.82 ft
Specific energy 0.49 ft	Froude number 1.28	Flow status Supercritical flow
Critical depth 0.35 ft	Critical slope 0.0283 ft/ft	Velocity head 0.19 ft

CS 5
5 year
Swale 2

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

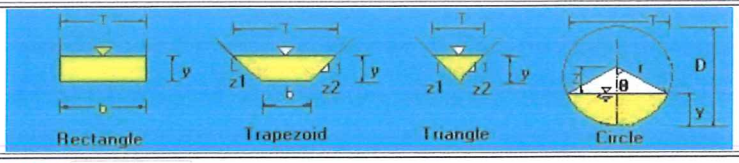
Select unit system: Feet(ft)

Channel slope: .055 ft/ft	Water depth(y): 0.31 ft	Bottom width(b) 5 ft
Flow velocity 3.375452 ft/s	LeftSlope (Z1): 18 to 1 (H:V)	RightSlope (Z2): 18 to 1 (H:V)
Flow discharge 11 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 16.13 ft	Flow area 3.26 ft^2	Top width(T) 16.11 ft
Specific energy 0.49 ft	Froude number 1.32	Flow status Supercritical flow
Critical depth 0.36 ft	Critical slope 0.0287 ft/ft	Velocity head 0.18 ft

CS #6
100 year
Swale 2

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

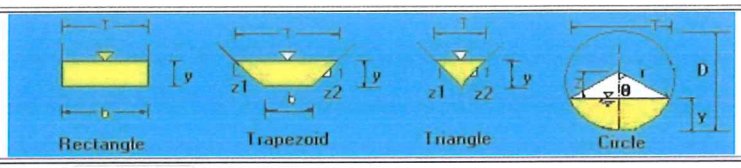
Select unit system: Feet(ft)

Channel slope: .055 ft/ft	Water depth(y): 0.73 ft	Bottom width(b) 5 ft
Flow velocity 5.515 ft/s	LeftSlope (Z1): 18 to 1 (H:V)	RightSlope (Z2): 18 to 1 (H:V)
Flow discharge 72.2 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 31.15 ft	Flow area 13.09 ft^2	Top width(T) 31.11 ft
Specific energy 1.2 ft	Froude number 1.5	Flow status Supercritical flow
Critical depth 0.88 ft	Critical slope 0.022 ft/ft	Velocity head 0.47 ft

CS 7
3 yr
Swale 3

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

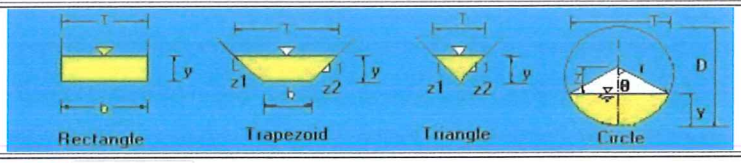
Select unit system: Feet(ft)

Channel slope: .083 ft/ft	Water depth(y): 0.32 ft	Bottom width(b) 5 ft
Flow velocity 4.394482 ft/s	LeftSlope (Z1): 15 to 1 (H:V)	RightSlope (Z2): 15 to 1 (H:V)
Flow discharge 14 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 14.72 ft	Flow area 3.19 ft^2	Top width(T) 14.7 ft
Specific energy 0.62 ft	Froude number 1.66	Flow status Supercritical flow
Critical depth 0.42 ft	Critical slope 0.0274 ft/ft	Velocity head 0.3 ft

CS 8
100 yeat
Swale 3

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

Select unit system: Feet(ft)

Channel slope: .083 ft/ft	Water depth(y): 0.77 ft	Bottom width(b) 5 ft
Flow velocity 7.207 ft/s	LeftSlope (Z1): 15 to 1 (H:V)	RightSlope (Z2): 15 to 1 (H:V)
Flow discharge 91.7 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 28.13 ft	Flow area 12.72 ft^2	Top width(T) 28.08 ft
Specific energy 1.58 ft	Froude number 1.89	Flow status Supercritical flow
Critical depth 1.03 ft	Critical slope 0.0211 ft/ft	Velocity head 0.81 ft

C39

Improved Inlets

Culvert capacity may be increased through the use of special inlet designs. The Federal Highway Administration has developed extensive data[19,20] on these. While these designs increase the flow, their use has not been as expected. The increased costs of the special treatments is apparently responsible.

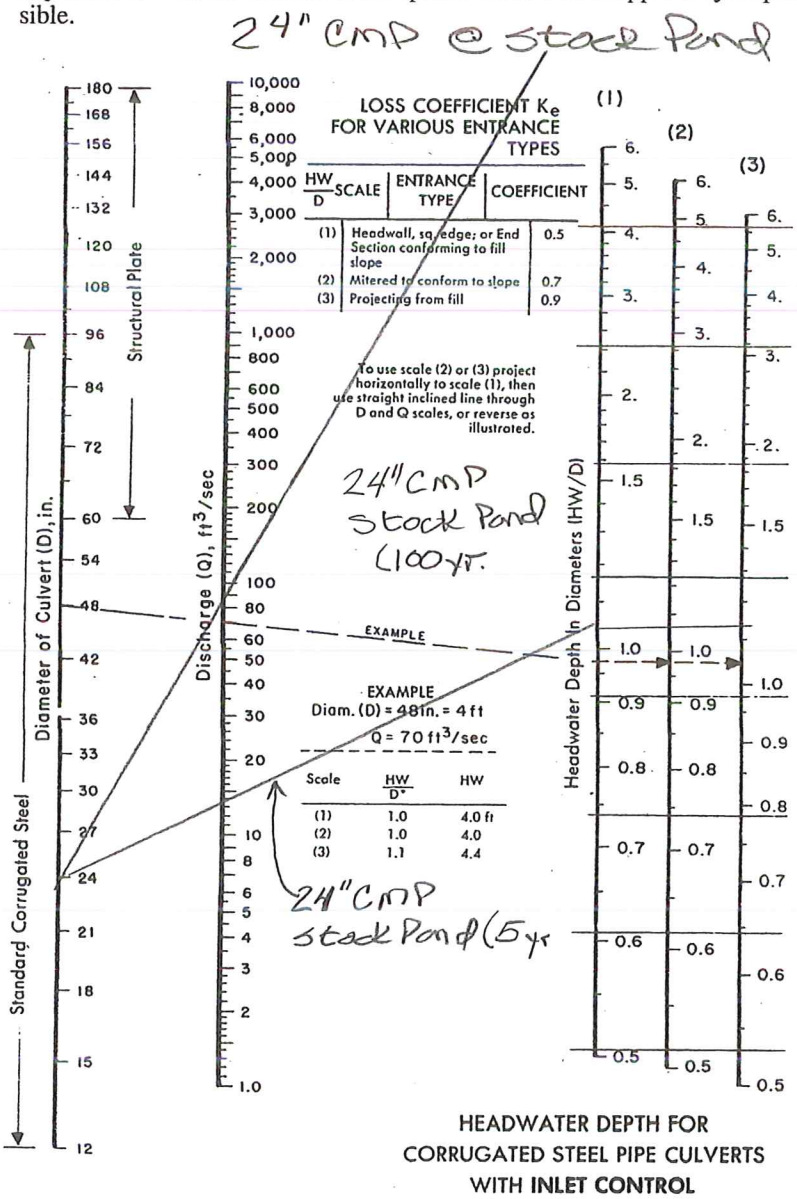


Figure 3.28 *Inlet control* nomograph for corrugated steel *pipe* culverts.[13] The manufacturers recommend keeping *HW/D* to a maximum of 1.5 and preferably to no more than 1.0 for diameters greater than 4 to 5 feet.

30" Culvert under Thompson Road / 18" Driveway Culvert

CS 10

Improved Inlets

Culvert capacity may be increased through the use of special inlet designs. The Federal Highway Administration has developed extensive data[19,20] on these. While these designs increase the flow, their use has not been as expected. The increased costs of the special treatments is apparently responsible.

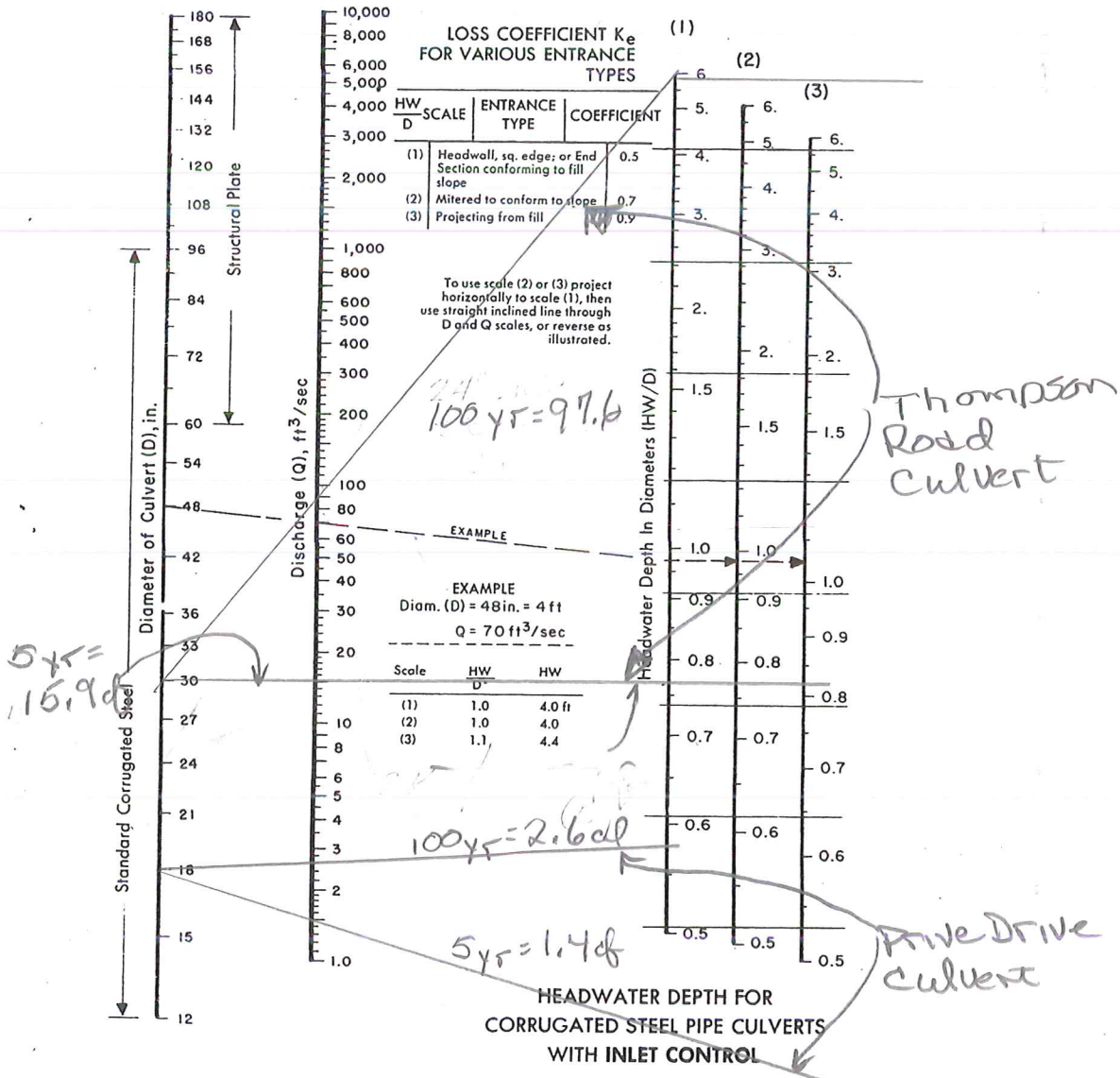
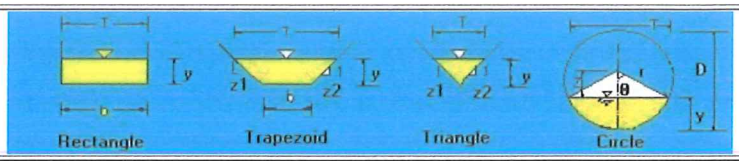


Figure 3.28 *Inlet control* nomograph for corrugated steel *pipe* culverts.[13] The manufacturers recommend keeping *HW/D* to a maximum of 1.5 and preferably to no more than 1.0 for diameters greater than 4 to 5 feet.

CS11
5 year
Swale

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

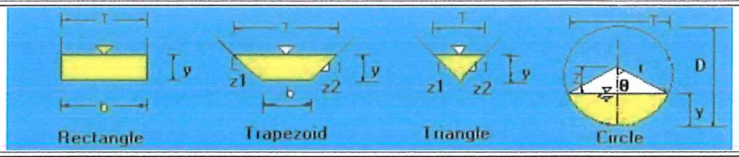
Select unit system: Feet(ft)

Channel slope: .083 ft/ft	Water depth(y): 0.35 ft	Bottom width(b) 5 ft
Flow velocity 4.819392 ft/s	LeftSlope (Z1): 10 to 1 (H:V)	RightSlope (Z2): 10. to 1 (H:V)
Flow discharge 14.5 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 12.09 ft	Flow area 3.01 ft^2	Top width(T) 12.06 ft
Specific energy 0.71 ft	Froude number 1.7	Flow status Supercritical flow
Critical depth 0.47 ft	Critical slope 0.026 ft/ft	Velocity head 0.36 ft

CS 12
100 year
Swale #4

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

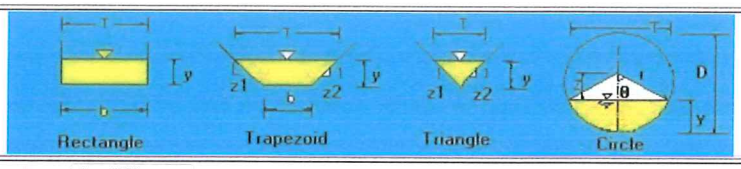
Select unit system: Feet(ft)

Channel slope: .083 ft/ft	Water depth(y): 0.87 ft	Bottom width(b) 5 ft
Flow velocity 7.938 ft/s	LeftSlope (Z1): 10 to 1 (H:V)	RightSlope (Z2): 10. to 1 (H:V)
Flow discharge 95 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 22.53 ft	Flow area 11.97 ft^2	Top width(T) 22.44 ft
Specific energy 1.85 ft	Froude number 1.92	Flow status Supercritical flow
Critical depth 1.19 ft	Critical slope 0.02 ft/ft	Velocity head 0.98 ft

CS13
5 year
Swale 6

The open channel flow calculator

Select Channel Type: Trapezoid



Depth from Q

Select unit system: Feet(ft)

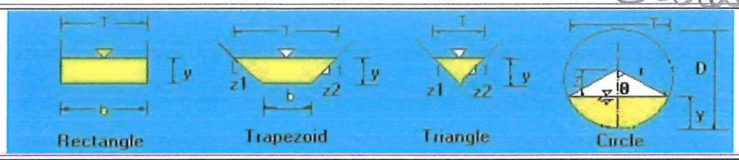
Channel slope: .055 ft/ft	Water depth(y): 0.19 ft	Bottom width(b) 2 ft
Flow velocity 2.846931 ft/s	LeftSlope (Z1): 3 to 1 (H:V)	RightSlope (Z2): 3 to 1 (H:V)
Flow discharge 1.4 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 3.21 ft	Flow area 0.49 ft^2	Top width(T) 3.15 ft
Specific energy 0.32 ft	Froude number 1.27	Flow status Supercritical flow
Critical depth 0.23 ft	Critical slope 0.0303 ft/ft	Velocity head 0.13 ft

CS 14

The open channel flow calculator

100 year Swale 6

Select Channel Type: Trapezoid



Depth from Q

Select unit system: Feet(ft)

Channel slope: .055 ft/ft	Water depth(y): 0.27 ft	Bottom width(b) 2 ft
Flow velocity 3.435106 ft/s	LeftSlope (Z1): 3 to 1 (H:V)	RightSlope (Z2): 3 to 1 (H:V)
Flow discharge 2.6 ft^3/s	Input n value .035 or select n	
Calculate!	Status: Calculation finished	Reset
Wetted perimeter 3.7 ft	Flow area 0.76 ft^2	Top width(T) 3.62 ft
Specific energy 0.45 ft	Froude number 1.32	Flow status Supercritical flow
Critical depth 0.32 ft	Critical slope 0.0293 ft/ft	Velocity head 0.18 ft

Exhibit 8

Historic Drainage Conditions

N

150' 0 150' 300'

SCALE: 1" = 150'

LEGEND:

- XXX – INDEX CONTOURS
- XXX – INTERMEDIATE CONTOURS
- SUBDIVISION BOUNDRY
- SUBBASIN BOUNDRY
- DIRECTION OF FLOW
- EXISTING SWALE CENTERLINE
- X – DESIGN POINT
- X – SWALE NUMBER
- AREA (ACRES) – X / XX – SUBBASIN I.D.
- X – DRAINAGE STRUCTURE NIUMBER
- STRX – DRAINAGE STRUCTURE
- EXISTING BUILDING
- NO BUILD AREA BOUNDARY
- EXSITING STOCK POND

CUMMULATIVE ACREAGE AT DESIGN POINTS

SUB BASIN ID	SUB BASIN AREA	SUB TOTAL
DP1		
OS1	34.1	34.1
DP5		
OS1	34.1	
OS3	0.6	
OS4	0.6	
OS5	0.4	
A	6.3	
OS7	3.6	45.6
DP6		
DP5	45.6	
OS6	0.9	
B	3.5	
OS2	10.3	60.3
DP7		
DP6	60.3	
C	0.8	
OS8	1.4	62.5
DP14		
DP7	62.5	
OS9	0.4	
OS10	0.5	63.4

DESIGN POINT SUMMARY

DESIGN POINT	CONTRIB SUB BASINS	AREA (ACRES)	Tc (CFS)	C5 (CFS)	C100 (CFS)	CA5 (CFS)	CA100 (CFS)	Q5 (CFS)	Q100 (CFS)
1	OS1	34.1	23.2	0.09	0.36			8.7	58.5
2	OS3	0.6	15	0.08	0.35			0.2	1.2
3	OS4	0.6	13.3	0.17	0.41			0.4	1.5
4	OS7	3.6	15.5	0.12	0.38			1.5	8.0
5	OS1,OS3,OS4,OS5,OS7,A	45.6	27			4.16	16.31	11.0	72.2
6	DP5,OS6,B,OS2	60.3	28.4			5.47	21.28	14.0	91.5
7	DP6,C,OS8	62.5	29			5.71	22.37	14.5	95.0
8	OS5	0.4	10.9	0.08	0.35			0.1	0.9
9	OS6	0.9	15.2	0.08	0.35			0.3	1.9
10	OS2	10.3	17.9	0.10	0.36			3.3	20.5
11	OS8	1.4	14.1	0.08	0.35			0.4	3.0
12	OS10	0.5	5.3	0.57	0.72			1.5	3.1
13	OS9	0.4	6.1	0.70	0.81			1.4	2.6
14	DP7,OS9,OS10	61.5	29			6.28	23.05	15.9	97.9
	Lot 1 located in sub basin B	5.5	12.9	0.08	0.35			1.8	13.3
	Lot 2 located in sub basin A	5.0	14.8	0.08	0.35			1.2	8.9

SWALE SUMMARY

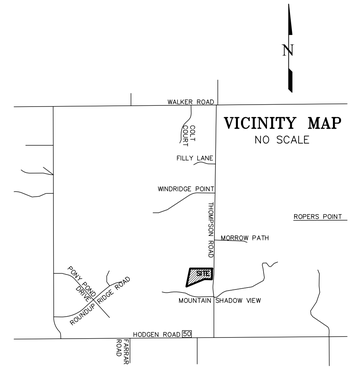
SWALE #	DESIGN POINTS	CONTRIBUTING SUBBASINS	SLOPE %	DESIGN FLOW Q5 cfs	DESIGN FLOW Q100 cfs	DEPTH OF FLOW Q5 ft	DEPTH OF FLOW Q100 ft	VELOCITY Q5 fps	VELOCITY Q100 fps	FROUDE NUMBER (5 YEAR)	FLOW REGIME
1	1	OS1	4.7	8.7	58.5	0.3	0.7	2.9	4.8	1.21	SUPER
2	1 TO 5	OS1,OS3, OS4,A,OS7	5.5	11	72.2	0.3	0.7	3.4	5.5	1.32	SUPER
3	5 TO 6	DP5,B,OS2	8.3	14	91.7	0.3	0.8	4.4	7.2	1.66	SUPER
4	6 TO 7	DP10,C,OS8	8.3	14.5	95	0.4	0.9	4.8	7.9	1.7	SUPER
5	12	OS10	5.0	1.5	3.1	0.2	0.3	2.8	3.5	1.2	SUPER
6	13	OS9	5.5	1.4	2.6	0.2	0.3	2.8	3.4	1.27	SUPER

STORMWATER RUNOFF SUMMARY

EXISTING CONDITIONS

(NEGLIGIBLE CHANGES FOR DEVELOPED CONDITIONS)

DP	CONTRIBUTING SUB BASINS	AREA	Tc	C5	C100	CA5	CA100	Q5	Q100
14	DP7,OS9,OS10	63.4	29			6.28	23.05	15.9	97.9
	Lot 1 (sub basin B)	5.5	12.9	0.08	0.35			1.8	13.3
	Lot 2 (sub basin A)	5.0	14.8	0.08	0.35			1.2	8.9

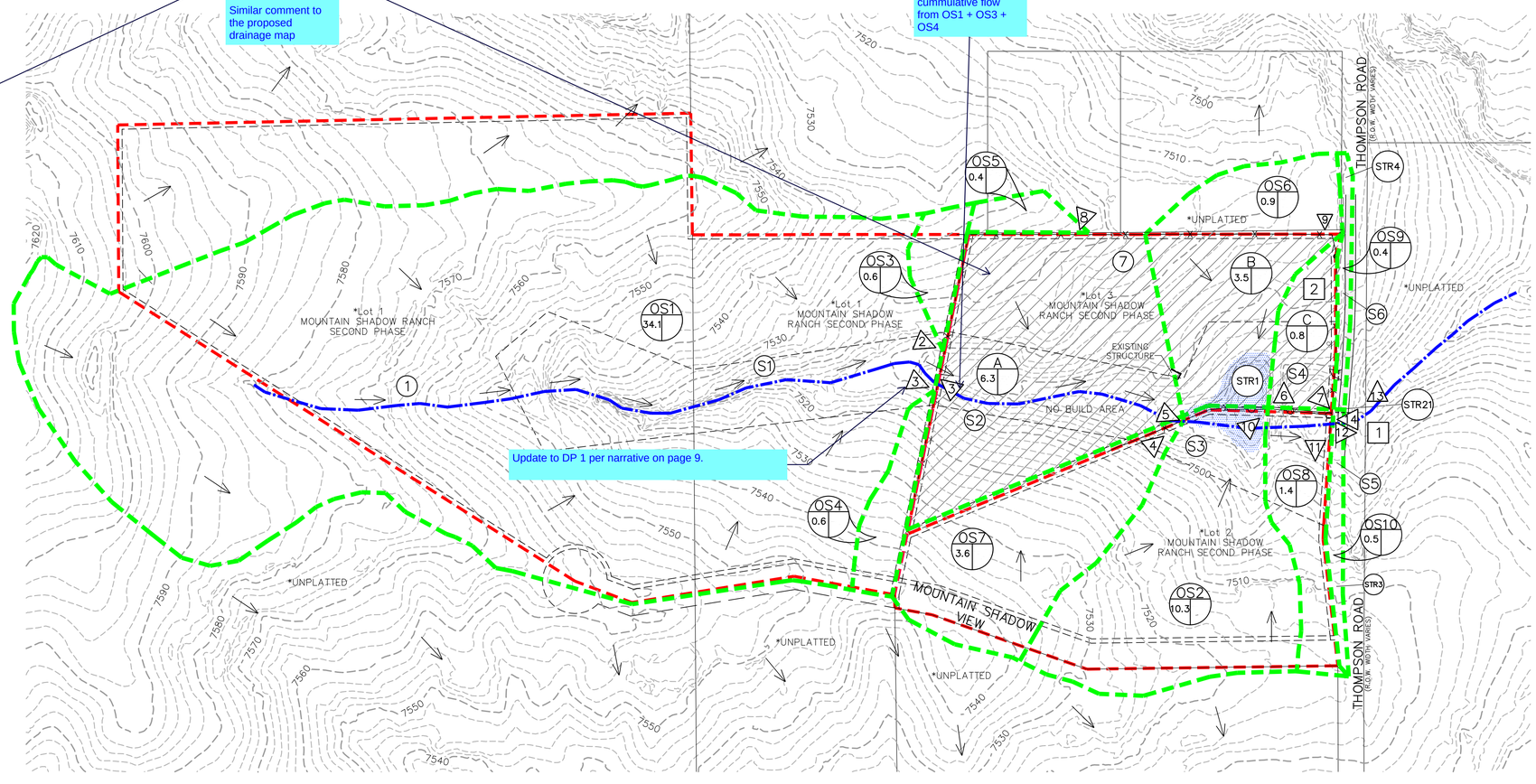


Update hatching. The as platted does not identify the entire Lot 3 as "No Build" per the legend.

Similar comment to the proposed drainage map

Update design point three to be the cummulative flow from OS1 + OS3 + OS4

Update to DP 1 per narrative on page 9.



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CALL BEFORE YOU DIG . . .

811

DIAL 811

48 HOURS BEFORE YOU DIG, CALL UTILITY LOCATORS FOR LOCATING AND MARKING GAS, ELECTRIC, WATER AND WASTEWATER

REVISIONS

No.	Description	By	Date

H Scale: 1"=100'
V Scale: N/A
Designed By: KH
Drawn By: TLC
Checked By: XXX
Date: 11/12/2020

Land Development Consultants, Inc.
PLANNING SURVEYING
www.ldc-inc.com TEL: (719) 528-6133 FAX: (719) 528-6848
3898 MAIZELAND ROAD COLORADO SPRINGS, CO 80909

MCGEHEE SUBDIVISION

DRAINAGE PLAN

EXISTING CONDITIONS

Project No.: 20018

Sheet: 1 of 1

F:\20018\20018 - McGehee Subdivision\Engineering\on Harrison Thompson\Construction\Harrison\2018 - DRAINAGE PLAN.dwg

Exhibit 9

Developed Drainage Conditions

N

150' 0 150' 300'

SCALE: 1" = 150'

LEGEND:

- INDEX CONTOURS
- INTERMEDIATE CONTOURS
- SUBDIVISION BOUNDRY
- SUBBASIN BOUNDRY
- DIRECTION OF FLOW
- EXISTING SWALE CENTERLINE
- DESIGN POINT
- SWALE NUMBER
- SUBBASIN I.D. (AREA (ACRES))
- DRAINAGE STRUCTURE NIUMBER
- DRAINAGE STRUCTURE
- EXISTING BUILDING
- NO BUILD AREA BOUNDARY
- EXSITING STOCK POND

CUMMULATIVE ACREAGE AT DESIGN POINTS

SUB BASIN ID	SUB BASIN AREA	SUB TOTAL
DP1		
OS1	34.1	34.1
DP5		
OS1	34.1	
OS3	0.6	
OS4	0.6	
OS5	0.4	
A	6.3	
OS7	3.6	45.6
DP6		
DP5	45.6	
OS6	0.9	
B	3.5	
OS2	10.3	60.3
DP7		
DP6	60.3	
C	0.8	
OS8	1.4	62.5
DP14		
DP7	62.5	
OS9	0.4	
OS10	0.5	63.4

DESIGN POINT SUMMARY

DESIGN POINT	CONTRIB SUB BASINS	AREA (ACRES)	Tc (CFS)	C5 (CFS)	C100 (CFS)	CA5 (CFS)	CA100 (CFS)	Q5 (CFS)	Q100 (CFS)
1	OS1	34.1	23.2	0.09	0.36			8.7	58.5
2	OS3	0.6	15	0.08	0.35			0.2	1.2
3	OS4	0.6	13.5	0.17	0.41			0.3	1.3
4	OS7	3.6	15.5	0.12	0.38			1.5	8.0
5	OS1,OS3,OS4,OS5,OS7,A	45.6	27			4.28	16.39	11.3	72.6
6	DP5,OS6,B,OS2	60.3	28.4			5.66	21.42	14.5	92.1
7	DP6,C,OS8	62.5	29			5.71	22.37	14.5	95.0
8	OS5	0.4	10.9	0.08	0.35			0.1	0.9
9	OS6	0.9	15.2	0.08	0.35			0.3	1.9
10	OS2	10.3	17.9	0.10	0.36			3.3	20.5
11	OS8	1.4	14.1	0.08	0.35			0.4	3.0
12	OS10	0.5	5.3	0.57	0.72			1.5	3.1
13	OS9	0.4	6.1	0.70	0.81			1.4	2.6
14	DP7,OS9,OS10	63.4	29			6.47	23.18	16.4	98.5
	Lot 1 located in sub basin B	5.5	12.9	0.11	0.37			1.7	9.4
	Lot 2 located in sub basin A	5.0	14.8	0.08	0.35			2.5	14.1

SWALE SUMMARY

SWALE #	DESIGN POINTS	CONTRIBUTING SUBBASINS	SLOPE %	DESIGN FLOW Q5 cfs	DESIGN FLOW Q100 cfs	DEPTH OF FLOW Q5 ft	DEPTH OF FLOW Q100 ft	VELOCITY Q5 fps	VELOCITY Q100 fps	FROUDE NUMBER (5 YEAR)	FLOW REGIM e
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3	5 TO 6	DP5,B,OS2	8.3	14	91.7	0.3	0.8	4.4	7.2	1.66	SUPER
4	6 TO 7	DP10,C,OS8	8.3	14.5	95	0.4	0.9	4.8	7.9	1.7	SUPER
5	12	OS10	5.0	1.5	3.1	0.2	0.3	2.8	3.5	1.2	SUPER
6	13	OS9	5.5	1.4	2.6	0.2	0.3	2.8	3.4	1.27	SUPER

Add a row for S7

Revise to be in S1, S2... format for clarity (so it matches map below)

STORMWATER RUNOFF SUMMARY

EXISTING CONDITIONS

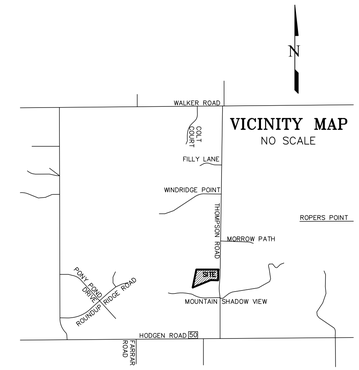
DP	CONTRIBUTING SUB BASINS	AREA	Tc	C5	C100	CA5	CA100	Q5	Q100
14	DP7,OS9,OS10	63.4	29			6.28	23.05	15.9	97.9
	Lot 1 (sub basin B)	5.5	12.9	0.08	0.35			1.8	13.3
	Lot 2 (sub basin A)	5.0	14.8	0.08	0.35			1.2	8.9

DEVELOPED CONDITIONS

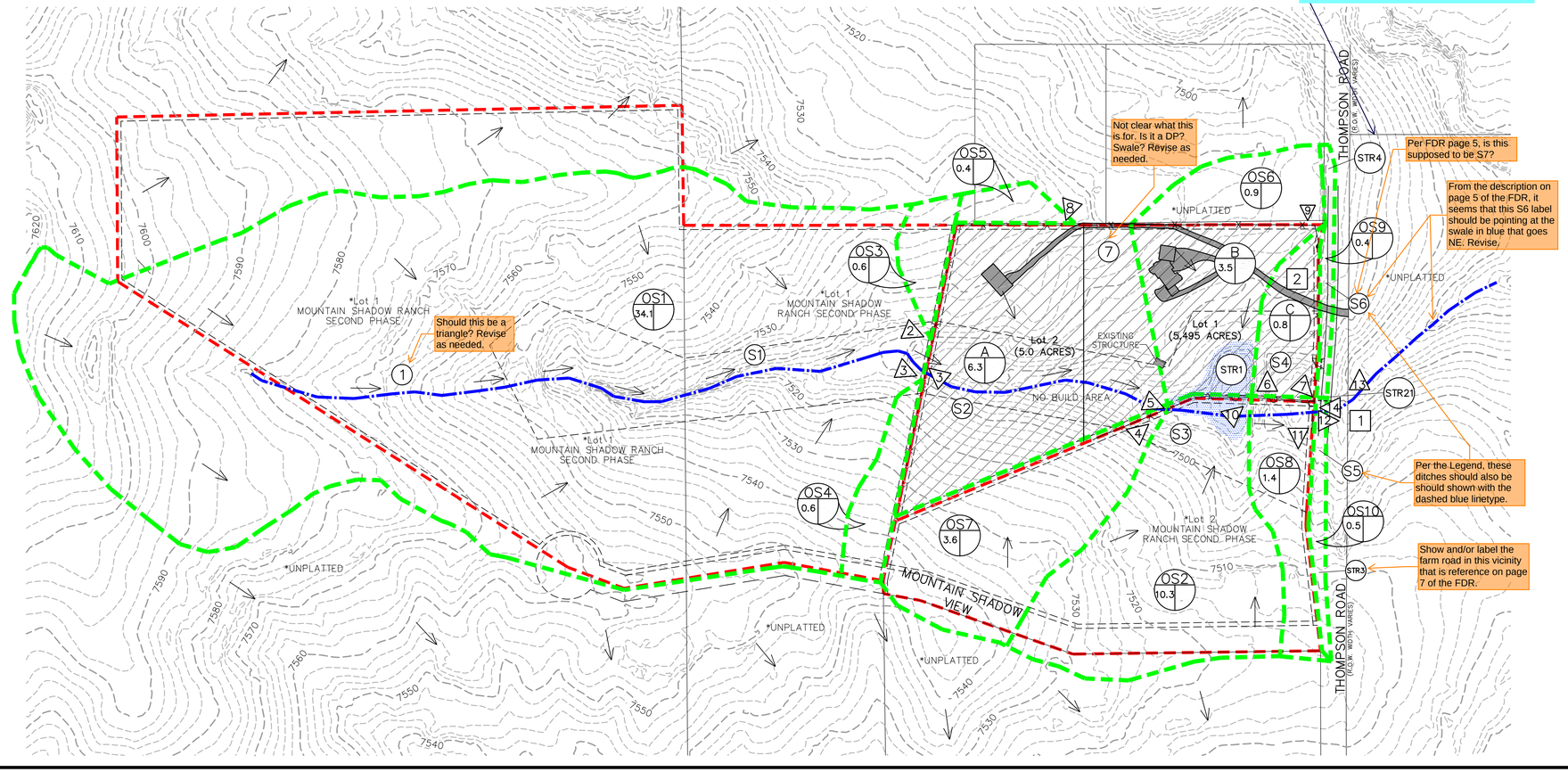
DP	CONTRIBUTING SUB BASINS	AREA	Tc	C5	C100	CA5	CA100	Q5	Q100
14	DP7,OS9,OS10	63.4	29			6.47	23.18	16.4	98.5
	Lot 1 (sub basin B)	5.5	12.9	0.11	0.37			2.5	14.1
	Lot 2 (sub basin A)	5.0	14.8	0.11	0.37			1.7	9.4

NOTES:

1. DESIGN POINT 14 IS LOCATED AT THE UPSTREAM END OF THE CULVERT UNDER THOMPSON ROAD.
2. THE MAJORITY OF LOT 2 IS LOCATED IN BASIN A.
3. THE MAJORITY OF LOT 1 IS LOCATED IN BASIN B.
4. ALL OF THE OFFSITE SUB BASINS ARE UNCHANGED FROM EXSITING CONDITIONS.
5. CA VALUES ARE USED IN ORDER TO ACCURATELY REFLECT CONTROLLING Tc.



STR4 is shown offsite. Update location. The narrative on page 7 noted this is 150 south of the NE property corner.



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MCGEHEE SUBDIVISION
DRAINAGE PLAN
DEVELOPED CONDITIONS

Project No.: 20018

Sheet: 1 of 1

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