

FINAL DRAINAGE LETTER for CROSSROADS MIXED USE FILING NO. 1 UNDERGROUND DETENTION

EL PASO COUNTY, COLORADO

DECEMBER 2022

Prepared for:

Crossroads Metropolitan District No. 1
Mr. Danny Mientka
90 South Cascade Avenue, Suite 1500
Colorado Springs, Colorado Springs 80903

Prepared by:



CIVIL CONSULTANTS, INC.

212 N. Wahsatch Avenue, Suite 305
Colorado Springs, CO 80903
(719) 955-5485

Project #18-003
D File No.

Please add "PCD File No. CDR-23-002"

actually revise to "CDR232"
(with no dashes or extras
zeros or extra spaces in the
file number)

**FINAL DRAINAGE LETTER FOR CROSSROADS MIXED
USE FILING NO.1 UNDERGROUND DETENTION
EL PASO COUNTY COLORADO**

DRAINAGE PLAN STATEMENTS

ENGINEERS STATEMENT

The attached drainage plan and report was prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Virgil A. Sanchez, P.E. #37160
For and on Behalf of M&S Civil Consultants, Inc

DEVELOPER'S STATEMENT

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

BY: _____
Danny Mientka –Owner

DATE: _____

ADDRESS: Crossroads Metropolitan District No. 1
 90 South Cascade Avenue, Suite 1500
 Colorado Springs, Colorado Springs 80903

EL PASO COUNTY'S STATEMENT

Filed in accordance with the requirements of El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Manual, as amended.

BY: _____ DATE: _____
 Joshua Palmer, P.E.
 County Engineer / ECM Administrator

CONDITIONS:



CIVIL CONSULTANTS, INC.

December 22, 2022

El Paso County Planning & Community Development
2880 International Circle Suite 110
Colorado Springs, Colorado 80910
Attn: Joshua Palmer, P.E./County Engineer

RE: Drainage Letter for Crossroads Mixed Use Filing No.1 Underground Detention

Dear Mr. Palmer,

The following is the Drainage Letter for Crossroads Mixed Use Filing No.1 Underground Detention. The purpose of this letter is to show general conformance with the drainage patterns established by the **Final Drainage Report for Crossroads Mixed Use Filing No.1** (herein referenced as **FDR-CMU**) and to revise drainage patterns within the pond site to accommodate the underground detention (**Tract A**). **Tract A** contains 3.257 acres and is located at 0 Meadowbrook Parkway in the southwestern quarter of Section 8, Township 14 South, Range 65 West of the 6th P.M. in El Paso County, Colorado.

Soils in the project area have been determined to be Blakeland Loamy Sand (8) and Blendon Sandy Loam (10), which are characterized to be part of Hydrologic Soil Types "A" & "B" as determined from the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS) "Web Soils Survey". A soils map illustrating the site location and soil types is provided in the appendix of this report.

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Nos. 08041C0754 G & 08041C0752 G, effective date December 7th, 2018, none of the site lies within a designated floodplain. A copy of these annotated maps can be found in the appendix. The Sand Creek East Fork Channel is located to the northwest of the adjacent Meadowbrook Crossing subdivision.

The **Final Drainage Report for Crossroads Mixed Use Filing No.1 (FDR-CMU)**, prepared by M&S Civil Consultants, Inc. and approved 06/09/2022. **Tract A** is identified as **Basin J** and consists of the proposed EDB pond. The majority of the surface flow (**Basin J**) was routed into the pond and will now be routed as surface flow, over the underground detention facility, to a proposed 5' sump inlet and 2-2' x 2' area inlets. The commercial and multi-family development surrounding the site will still route flows to the underground detention at the locations specified by the **FDR-CMU** and the **Final Drainage Report for Aura at Crossroads** (herein referenced as **FDR-AC**), prepared by Harris Kocher Smith, approved 06/23/2022.

Design Point 1 (Q5=1.1 cfs, Q100=4.7cfs) total runoff generated by **Basin A** and adjacent planned **Lot 11** runoff. **Basin A** (Q5=0.6 cfs, Q100=3.3cfs) contains 1.62 acres of park/playground area and access road and adjacent planned landscaped area of **Lot 11 DP16 (FDR-AC, Q5=0.57 cfs, Q100=1.43cfs)**. The combined surface runoff sheet flows to **Design Point 1** (Q5=1.1 cfs, Q100=4.7 cfs) and will captured by a proposed 2'x2' ADS area inlet. The captured flow shall be routed via a

Expand this paragraph to include analysis of suitability of soils specific to the underground detention system. Examples:
1) what soils type and infiltration rate does ADS recommend vs what is onsite.
2) structural stability of subgrade for ADS system
3) is any subgrade prep necessary?
4) is there any need for fill to be imported?
5) any concerns with groundwater?

If any of this is already stated in the I&M Manual (Appendix C of O&M Manual), reference that here so it is known where that info can be found.

There should also be a statement that says that there is no change in the amount of flow from basins as discussed in the previous FDR (ie: all areas tributary to the pond and excluded areas remain the same). Also state that the UGD system will essentially function the same as the originally designed above ground in terms of capture and release rates

Provide an engineering cost estimate for the underground detention.

proposed 18" RCP storm pipe and a proposed Type 1 manhole. See proposed drainage map in the appendix.

Design Point 2 (Q5=0.8 cfs, Q100=3.2cfs) total runoff generated by **Basin B** and adjacent planned **Lot 11** runoff. **Basin B** (Q5=0.4 cfs, Q100=2.3cfs) contains 1.13 acres of park/playground area and access road and adjacent planned landscaped area of **Lot 11 DP15 (FDR-AC, Q5=0.47 cfs, Q100=1.27cfs)**. The combined surface runoff sheet flows to **Design Point 2** (Q5=0.8 cfs, Q100=3.2 cfs) and will be captured by a proposed 2'x2' ADS area inlet. The captured flow shall be routed via a proposed 18" RCP storm pipe **PR26** (Q5=0.8 cfs, Q100=3.2 cfs). These flows are routed to a proposed Type 1 manhole. The cumulative flows from **PR26, PR25 and PR19 (48" RCP FDR-AC, Q5=35.4 cfs, Q100=65.5 cfs)** are routed via a proposed 54" RCP **PR27** (Q5=37.5 cfs, Q100=74.6 cfs) to an XK Baysaver vault which is part of the underground detention infrastructure. See proposed drainage map and underground detention details in the appendix.

Design Point 3 (Q5=1.3 cfs, Q100=3.0 cfs) total runoff generated by **Basin C. Basin C** (Q5=1.3 cfs, Q100=3.0 cfs) contains 0.55 acres of park/playground area, access road and future parking lot. The surface runoff sheet flows to **Design Point 3** (Q5=1.3 cfs, Q100=3.0 cfs) and will be captured by a proposed 5' CDOT Type R sump inlet. The captured flow shall be routed via a proposed 15" RCP storm pipe **PR23** (Q5=1.3 cfs, Q100=3.0 cfs). These flows are routed to a proposed Type 1 manhole. The cumulative flows from **PR15 (48" RCP FDR-AC, Q5=10.8 cfs, Q100=19.7 cfs)** and **PR16 (24" RCP FDR-CMU, Q5=10.8 cfs, Q100=19.7 cfs)** are routed via a proposed 54" RCP **PR17** (Q5=57.0 cfs, Q100=110.1 cfs) to a Type 1 manhole. The cumulative flows from **DP14 PR21 (30" RCP FDR-AC, Q5=2.1 cfs, Q100=4.2 cfs)** and **PR22 (54" RCP PR22 (Q5=56.5 cfs, Q100=109.4 cfs)** to a Type 1 manhole. The cumulative flows from **PR23 and PR22** are routed via a proposed 54" RCP **PR24** (Q5=56.5 cfs, Q100=109.4 cfs) to an XK Baysaver vault which is part of the underground detention infrastructure. See proposed drainage map and underground detention details in the appendix.

Clarify. This project is replacing the EDB with underground detention w/ underground water quality. This does not seem to apply.

Add a paragraph discussing the capabilities of the Baysaver (what it's designed to do, how it works, its capacity, etc)

All storm and **WQ pond** improvements shall be installed per the Crossroads Mixed Use Filing No.1 Storm Underground Detention construction plans. No additional storm sewer improvements are proposed for this site. This drainage letter includes the previously approved Proposed Drainage Map for **Drainage Report for Crossroads Mixed Use Filing No.1 (FDR-CMU)** and **Final Drainage Report for Aura at Crossroads (FDR-AC)**.

This final drainage letter for Tract A and underground detention is in compliance with the design as proposed within the **Final Drainage Report for Crossroads Mixed Use Filing No.1 (FDR-CMU)**; therefore no negative impacts are anticipated to the downstream improvements or facilities with the approval of this drainage letter.

This site is in the Sand Creek Drainage Basin. Drainage fees were paid at the time of platting as Tract A of Crossroads Mixed Use Filing No. 1 (Reception No. 222714975), therefore no additional Drainage Bridge and/or Pond fees are not required. See Final Drainage Report for Aura at Crossroads (FDR-AC) dated June 22, 2022, by MS Civil Consultants.

Respectfully,

Discuss how much extra capacity above the 100-yr storm has been provided in the UGD system.

Virgil A. Sanchez, P.E.
M&S Civil Consultants, Inc.



Include the four step process based on proposed design. What was described in the Fil 1 FDR is no longer applicable.

Reminder:
a. For step 3 provide clarification regarding water quality implementation. Pg 29 identifies the WQCV storage while the deviation also identifies TSS removal. Section I.7.1.C allows for both. Clarify if both WQCV and TSS is being implemented. For TSS identify the provided rate vs criteria.
b. Per step 4 description it seems the appropriate narrative is to describe the specialized bmp you are proposing. Also provide a summary regarding the approved deviation for the underground detention/WQ. [Include the reference PCD File No. DEV221]

Discuss sizing of ADS system. ADS told us that they count the stone as part of the volume at 40%. Discuss design parameters and output of ADS sizing spreadsheet (pg 29 of 44 below). About 8 inches of capacity is provided above the 100-yr storm. State the sediment depth that will cause the 100-yr event to cause the system to be at 100% capacity. That will give us knowledge of the absolute highest level of sediment (without allowing for any freeboard or factor of safety). Discuss factor of safety.

REFERENCES

- 1.) "El Paso County and City of Colorado Springs Drainage Criteria Manual".
- 2.) "Mile High Flood District Storm Drainage Criteria Manual"
- 3.) SCS Soils Map for El Paso County.
- 4.) Flood Insurance Rate Map (FIRM), Federal Emergency Management Agency, Revised date December 7th, 2018.
- 5.) " Final Drainage Report for Crossroads Mixed Use Filing No.1", prepared by M&S Civil Consultants, Inc. and approved 06/09/2022Final Drainage Report for Claremont Business Park 2 Filing No.1", dated December, 2020, by M&S Civil Consultants, Inc.
- 6.) " Final Drainage Report for Aura at Crossroads", prepared by Harris Kocher Smith, approved 06/23/2022

We want to include an outline of some inspection requirements during construction:

- Set-up a pre-con with manufacturer/vendor and EPC staff.
- EPC – DPW – Stormwater staff need to be onsite to perform QC/testing for the following milestones. Contractor to alert EPC prior to commencing any of these items.
 - Installation of fabric
 - Placement of underground structures (like ADS arch pipe, BaySeparator, outlet structure, etc)
 - Placement of stone
- Provide EPC staff with lab results, spec sheets, and delivery receipts for:
 - Subgrade compaction per ADS req's
 - Stone (size, washed type, etc)
 - Fabric

Also state as a condition of deviation request DEV221, this system will follow the post-construction requirements of the Pilot Program outlined by EPC on a separate document.

ATTACHMENTS:

Vicinity map

Crossroads Mixed Use Filing No. 1 - Final Plat

Final Drainage Report for Crossroads Mixed Use Filing No.1- Proposed Drainage Map

Final Drainage Report for Aura at Crossroads - Proposed Drainage Map

Hydrologic and Hydraulic Calculations

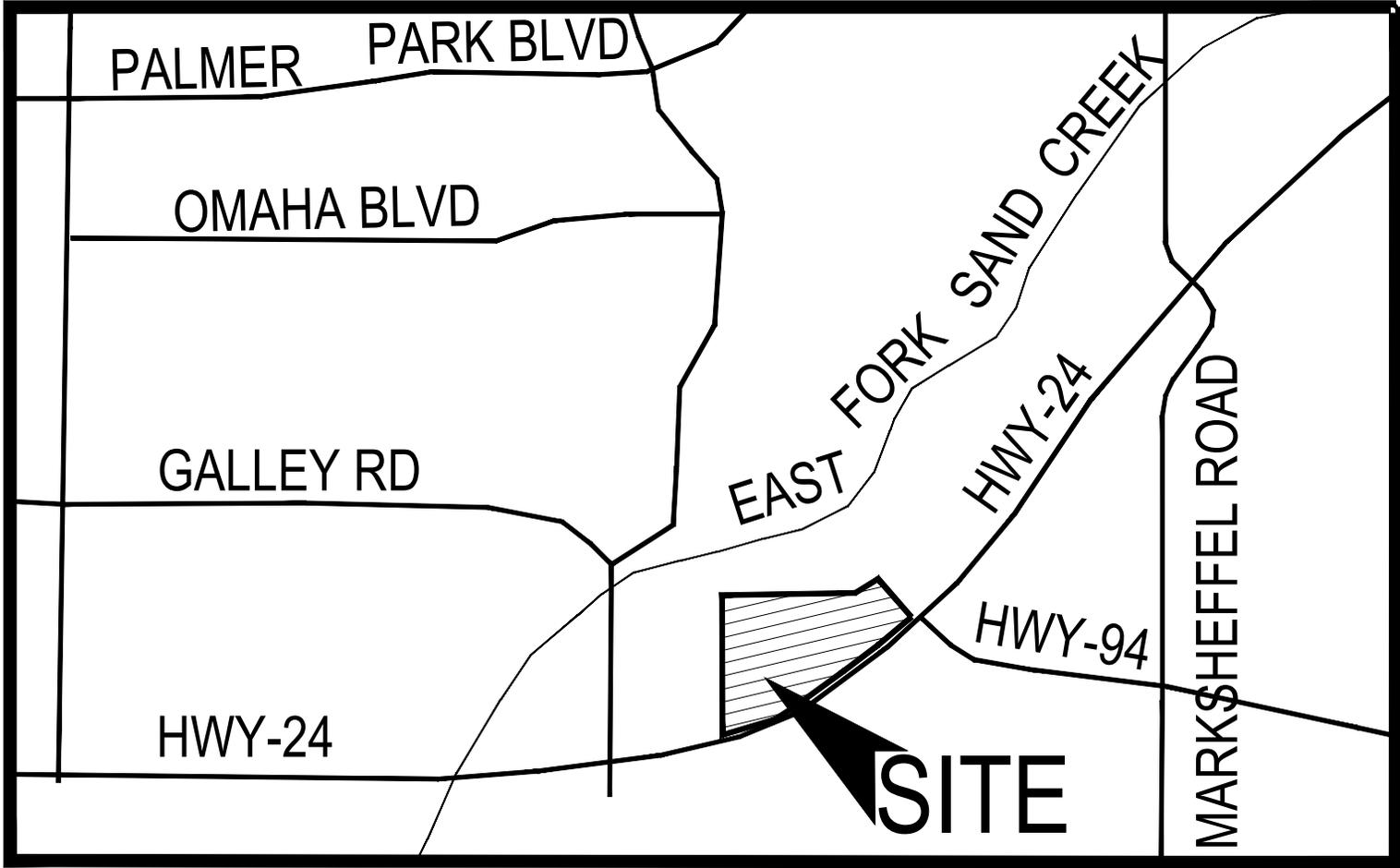
Underground Detention Details

Final Drainage Letter Crossroads Mixed Use Filing No.1 Underground Detention

– Proposed Drainage Map

ATTACHMENTS

VICINITY MAP



VICINITY MAP

N.T.S.

Remove Plat document.

**CROSSROADS MIXED USE FILING NO. 1
-PLAT**

CROSSROADS MIXED USE FILING NO. 1

A REPLAT OF TRACT B "24/94 BUSINESS PARK FILING NO. 1", BEING A TRACT OF LAND IN THE SOUTH HALF (S 1/2) OF SECTION 8, T14S, R65W, OF THE 6TH P.M., EL PASO COUNTY, COLORADO

14975

BE IT KNOWN BY THESE PRESENTS:

THAT COLORADO SPRINGS EQUITIES, LLC, A COLORADO LIMITED LIABILITY COMPANY, BEING THE OWNERS OF THE FOLLOWING DESCRIBED TRACT OF LAND:

LEGAL DESCRIPTION:

A PARCEL OF LAND IN THE SOUTH HALF OF SECTION 8, T14S, R65W OF THE 6TH P.M., EL PASO COUNTY, COLORADO BEING TRACT B "24/94 BUSINESS PARK FILING NO. 1" AS RECORDED UNDER RECEPTION NO. 217713939 OF THE RECORDS OF EL PASO COUNTY, COLORADO, MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT THE SOUTHEAST CORNER OF "SOFTBALL WEST SUBDIVISION NO 2"; THENCE N03°58'20"E ALONG THE EASTERLY LINE THEREOF, 1,170.16 FEET; THENCE N03°56'37"E ALONG THE EASTERLY LINE THEREOF, 57.75 FEET TO THE SOUTHWEST CORNER OF MEADOWBROOK PARKWAY RIGHT-OF-WAY; THENCE ALONG THE SOUTHERLY LINE THEREOF THE FOLLOWING FIVE (5) COURSES:

1. THENCE N89°43'00"E A DISTANCE OF 940.70 FEET TO A POINT OF CURVE;
2. THENCE 221.10 FEET ALONG THE ARC OF A CURVE TO THE LEFT, SAID CURVE HAVING A RADIUS OF 605.00 FEET, A CENTRAL ANGLE OF 20°56'21", THE CHORD OF 219.87 FEET WHICH BEARS N79°14'49"E;
3. THENCE N89°43'06"E, NON-TANGENT TO THE PREVIOUS COURSE, 44.80 FEET;
4. THENCE N51°19'02"E A DISTANCE OF 198.81 FEET;
5. THENCE S41°14'31"E A DISTANCE OF 397.89 FEET TO THE NORTHERLY RIGHT-OF-WAY LINE OF U.S. HIGHWAY 24;

THENCE ALONG THE NORTHERLY LINE THEREOF THE FOLLOWING FOUR (4) COURSES:

1. THENCE 682.61 FEET ALONG THE ARC OF A CURVE TO THE RIGHT, SAID CURVE HAVING A RADIUS OF 7,514.00 FEET, A CENTRAL ANGLE OF 5°12'18", THE CHORD OF 682.38 FEET WHICH BEARS S51°24'05"W TO A POINT OF TANGENT;
2. THENCE S54°01'07"W A DISTANCE OF 497.15 FEET;
3. THENCE S57°40'23"W A DISTANCE OF 163.43 FEET;
4. THENCE S98.63 FEET ALONG THE ARC OF A CURVE TO THE RIGHT, SAID CURVE HAVING A RADIUS OF 1,780.00 FEET, A CENTRAL ANGLE OF 22°29'17", THE CHORD OF 694.16 FEET WHICH BEARS S65°14'17"W TO THE POINT OF BEGINNING.

SAID PARCEL CONTAINS A CALCULATED AREA OF 1,265,357 SQUARE FEET (29.049 ACRES MORE OR LESS).

SEE GENERAL PLAT NOTE 1 FOR BASIS OF BEARING.

DEDICATION:

THE UNDERSIGNED, BEING ALL THE OWNERS, MORTGAGEES, BENEFICIARIES OF DEEDS OF TRUST AND HOLDERS OF OTHER INTERESTS IN THE LAND DESCRIBED HEREIN, HAVE LAID OUT, SUBDIVIDED, AND PLATTED SAID LANDS INTO LOTS, TRACTS, STREETS, AND EASEMENTS (USE WHICH ARE APPLICABLE) AS SHOWN HEREON UNDER THE NAME AND SUBDIVISION OF "CROSSROADS MIXED USE FILING NO. 1". ALL PUBLIC IMPROVEMENTS SO PLATTED ARE HEREBY DEDICATED TO PUBLIC USE AND SAID OWNER DOES HEREBY COVENANT AND AGREE THAT THE PUBLIC IMPROVEMENTS WILL BE CONSTRUCTED TO EL PASO COUNTY STANDARDS AND THAT PROPER DRAINAGE AND EROSION CONTROL FOR SAME WILL BE PROVIDED AT SAID OWNER'S EXPENSE, ALL TO THE SATISFACTION OF THE BOARD OF COUNTY COMMISSIONERS OF EL PASO COUNTY, COLORADO. UPON ACCEPTANCE BY RESOLUTION, ALL PUBLIC IMPROVEMENTS SO DEDICATED WILL BECOME MATTERS OF MAINTENANCE BY EL PASO COUNTY, COLORADO. THE UTILITY EASEMENTS SHOWN HEREON ARE HEREBY DEDICATED FOR PUBLIC UTILITIES AND COMMUNICATION SYSTEMS AND OTHER PURPOSES AS SHOWN HEREON. THE ENTITIES RESPONSIBLE FOR PROVIDING THE SERVICES FOR WHICH THE EASEMENTS ARE ESTABLISHED ARE HEREBY GRANTED THE PERPETUAL RIGHT OF INGRESS AND EGRESS FROM AND TO ADJACENT PROPERTIES FOR INSTALLATION, MAINTENANCE, AND REPLACEMENT OF UTILITY LINES AND RELATED FACILITIES.

BY: Danny Mientka
MANAGER, COLORADO SPRINGS EQUITIES, LLC, A COLORADO LIMITED LIABILITY COMPANY

NOTARIAL:

STATE OF COLORADO }
COUNTY OF EL PASO } SS

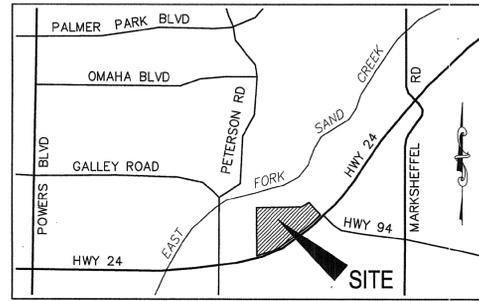
ACKNOWLEDGED BEFORE ME THIS 21st DAY OF June, 2022 BY
DANNY MIENTKA, AS MANAGER, COLORADO SPRINGS EQUITIES, LLC, A COLORADO
LIMITED LIABILITY COMPANY

WITNESS MY HAND AND OFFICIAL SEAL:

MY COMMISSION EXPIRES: December 3, 2025

NOTARY PUBLIC: Sybil Skull

SYBLAR SHULL
NOTARY PUBLIC
STATE OF COLORADO
NOTARY ID 20214047126
MY COMMISSION EXPIRES DECEMBER 03, 2025



VICINITY MAP
N.T.S.

LIEN HOLDER STATEMENT:

FRANK W. HOWARD #2 LIMITED PARTNERSHIP, L.L.P., A COLORADO LIMITED LIABILITY LIMITED PARTNERSHIP, OWNER AND HOLDER OF A LIEN AGAINST THE PROPERTY DESCRIBED IN THE PLAT KNOWN AS "CROSSROADS MIXED USE FILING NO. 1", SAID LIEN BEING EVIDENCED BY A DEED OF TRUST OF RECORD UNDER RECEPTION NO. 22118423 OF THE REAL PROPERTY RECORDS OF EL PASO COUNTY, COLORADO, DO HEREBY RATIFY AND CONFIRM SAID SUBDIVISION AND DEDICATION, AND DO HEREBY IN ALL THINGS SUBJECT TO SAID PLAT SAID LIENS. I HEREBY CONFIRM THAT I AM THE PRESENT OWNER OF SAID LIENS AND HAVE NOT ASSIGNED THE SAME NOR ANY PART THEREOF.

BY: Kevin Howard, AS Co-Partner, FRANK W. HOWARD #2 LIMITED PARTNERSHIP, L.L.P., A COLORADO LIMITED LIABILITY LIMITED PARTNERSHIP

NOTARIAL:

STATE OF COLORADO }
COUNTY OF EL PASO } SS

THE ABOVE AND AFOREMENTIONED INSTRUMENT WAS ACKNOWLEDGED BEFORE ME THIS 22nd DAY OF June, 2022, A.D. BY Kevin Howard, AS Co-Partner OF FRANK W. HOWARD #2 LIMITED PARTNERSHIP, L.L.P., A COLORADO LIMITED LIABILITY LIMITED PARTNERSHIP

WITNESS MY HAND AND OFFICIAL SEAL:

MY COMMISSION EXPIRES: December 3, 2025

NOTARY PUBLIC: Sybil Skull

SYBLAR SHULL
NOTARY PUBLIC
STATE OF COLORADO
NOTARY ID 20214047126
MY COMMISSION EXPIRES DECEMBER 03, 2025

LIEN HOLDER STATEMENT:

LEGACY BANK, ORGANIZED AND EXISTING UNDER THE LAWS OF COLORADO, OWNER AND HOLDER OF A LIEN AGAINST THE PROPERTY DESCRIBED IN THE PLAT KNOWN AS "CROSSROADS MIXED USE FILING NO. 1", SAID LIEN BEING EVIDENCED BY A DEED OF TRUST OF RECORD UNDER RECEPTION NO. 219089188, PARTIAL RELEASE OF DEED OF TRUST OF RECORD UNDER RECEPTION NUMBER 221158823, AND MODIFICATION OF DEED OF TRUST OF RECORD UNDER RECEPTION NUMBER 222015688 OF THE REAL PROPERTY RECORDS OF EL PASO COUNTY, COLORADO, DO HEREBY RATIFY AND CONFIRM SAID SUBDIVISION AND DEDICATION, AND DO HEREBY IN ALL THINGS SUBJECT TO SAID PLAT SAID LIENS. I HEREBY CONFIRM THAT I AM THE PRESENT OWNER OF SAID LIENS AND HAVE NOT ASSIGNED THE SAME NOR ANY PART THEREOF.

BY: Josh Stensrud, AS SA. Vice President, LEGACY BANK, ORGANIZED AND EXISTING UNDER THE LAWS OF COLORADO

NOTARIAL:

STATE OF COLORADO }
COUNTY OF EL PASO } SS

THE ABOVE AND AFOREMENTIONED INSTRUMENT WAS ACKNOWLEDGED BEFORE ME THIS 21st DAY OF June, 2022, A.D. BY Josh Stensrud AS SVP OF LEGACY BANK, ORGANIZED AND EXISTING UNDER THE LAWS OF COLORADO

WITNESS MY HAND AND OFFICIAL SEAL:

MY COMMISSION EXPIRES: 8-13-2024

NOTARY PUBLIC: Darlene J. Robinson

DARLENE J. ROBINSON
NOTARY PUBLIC
STATE OF COLORADO
NOTARY ID 19664013962
MY COMMISSION EXPIRES AUGUST 13, 2024

EASEMENTS:

UNLESS OTHERWISE INDICATED, ALL SIDE, FRONT, AND REAR LOT LINES ARE HEREBY PLATTED ON EITHER SIDE WITH A 10 FOOT PUBLIC UTILITY AND DRAINAGE EASEMENT UNLESS OTHERWISE INDICATED. ALL EXTERIOR SUBDIVISION BOUNDARIES ARE HEREBY PLATTED WITH A 20 FOOT PUBLIC UTILITY AND DRAINAGE EASEMENT. THE SOLE RESPONSIBILITY FOR MAINTENANCE OF THESE EASEMENTS IS HEREBY VESTED WITH THE INDIVIDUAL PROPERTY OWNERS.

EASEMENTS ARE AS SHOWN ON SHEETS 4 AND 5 OF THIS PLAT.

SURVEYORS CERTIFICATE

I, VERNON P. TAYLOR, A DULY REGISTERED PROFESSIONAL LAND SURVEYOR IN THE STATE OF COLORADO, DO HEREBY CERTIFY THAT THIS PLAT TRULY AND CORRECTLY REPRESENTS THE RESULTS OF A SURVEY MADE APRIL 2021, BY ME OR UNDER MY DIRECT SUPERVISION AND THAT ALL MONUMENTS EXIST AS SHOWN HEREON; THAT MATHEMATICAL CLOSURE ERRORS ARE LESS THAN 1:10,000; AND THAT SAID PLAT HAS BEEN PREPARED IN FULL COMPLIANCE WITH ALL APPLICABLE LAWS OF THE STATE OF COLORADO DEALING WITH MONUMENTS, SUBDIVISION, OR SURVEYING OF LAND AND ALL APPLICABLE PROVISIONS OF THE EL PASO COUNTY LAND DEVELOPMENT CODE.

I ATTEST THE ABOVE ON THIS 20th DAY OF JUNE, 2022.

Vernon P. Taylor
VERNON P. TAYLOR DATE
COLORADO PLS NO. 259966,
FOR AND ON BEHALF OF
M&S CIVIL CONSULTANTS, INC



NOTICE:

ACCORDING TO COLORADO LAW, YOU MUST COMMENCE ANY LEGAL ACTION BASED UPON ANY DEFECT IN THIS SURVEY WITHIN THREE YEARS AFTER YOU FIRST DISCOVER SUCH DEFECT. IN NO EVENT, MAY ANY ACTION BASED UPON ANY DEFECT IN THIS SURVEY BE COMMENCED MORE THAN TEN YEARS FROM THE DATE OF THE CERTIFICATION SHOWN HEREON.

PLANNING AND COMMUNITY DEVELOPMENT DEPARTMENT EXECUTIVE DIRECTOR CERTIFICATE:

THIS PLAT FOR "CROSSROADS MIXED USE FILING NO. 1" WAS APPROVED FOR FILING BY THE EL PASO COUNTY, COLORADO PLANNING AND COMMUNITY DEVELOPMENT DIRECTOR ON THE 20th DAY OF JUNE, 2022, SUBJECT TO ANY NOTES SPECIFIED HEREON AND ANY CONDITIONS INCLUDED IN THE RESOLUTION OF APPROVAL. THE DEDICATIONS OF LAND TO THE PUBLIC (STREETS, TRACTS, AND EASEMENTS) ARE ACCEPTED, BUT PUBLIC IMPROVEMENTS THEREON WILL NOT BECOME THE MAINTENANCE RESPONSIBILITY OF EL PASO COUNTY UNTIL PRELIMINARY ACCEPTANCE OF THE PUBLIC IMPROVEMENTS IN ACCORDANCE WITH THE REQUIREMENTS OF THE LAND DEVELOPMENT CODE AND ENGINEERING CRITERIA MANUAL, AND THE SUBDIVISION IMPROVEMENTS AGREEMENT.

Kevin M. Stensrud 6/22/22
INTERIM EXECUTIVE DIRECTOR, PLANNING AND
COMMUNITY DEVELOPMENT DEPARTMENT DATE

CLERK AND RECORDER:

STATE OF COLORADO }
COUNTY OF EL PASO } SS

I HEREBY CERTIFY THAT THIS INSTRUMENT WAS FILED FOR RECORD IN MY OFFICE AT 4:04 O'CLOCK pm THIS 22nd DAY OF June, 2022, A.D., AND DULY RECORDED UNDER RECEPTION NO. 222114975 OF THE RECORDS OF EL PASO COUNTY, COLORADO.

FEES: \$50.00 CHUCK BROERMAN, RECORDER

SURCHARGE: \$3.00 BY: Cayla Young
DEPUTY

FEES:

| | |
|--------------------|---------------|
| DRAINAGE FEE: | \$ 292,304.51 |
| BRIDGE FEE: | \$ 119,566.96 |
| SCHOOL FEE: | \$ 31,212.00 |
| URBAN PARK FEE: | \$ 88,740.00 |
| REGIONAL PARK FEE: | \$ 140,760.00 |

SUMMARY:

| | | |
|---------------|--------------|---------|
| 1 LOT | 12.703 ACRES | 43.73% |
| 4 TRACTS | 16.292 ACRES | 56.08% |
| RIGHTS-OF-WAY | 0.054 ACRES | 0.19% |
| TOTAL | 29.049 ACRES | 100.00% |

COLORADO SPRINGS EQUITIES, LLC
90 S. CASCADE AVE., SUITE 1500
COLORADO SPRINGS, CO 80903
PHONE: 719-475-7621

PREPARED BY:
ERIC L. YOKOM
FOR AND ON
BEHALF OF
212 N. WAHSATCH AVE., STE 305
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

FINAL PLAT
CROSSROADS MIXED USE
FILING NO. 1
JOB NO. 18-003
DATE PREPARED: 06/23/2021
DATE REVISED: 06/20/2022
ISSUED FOR MYLAR



PCD FIL. NO. SF-21-029

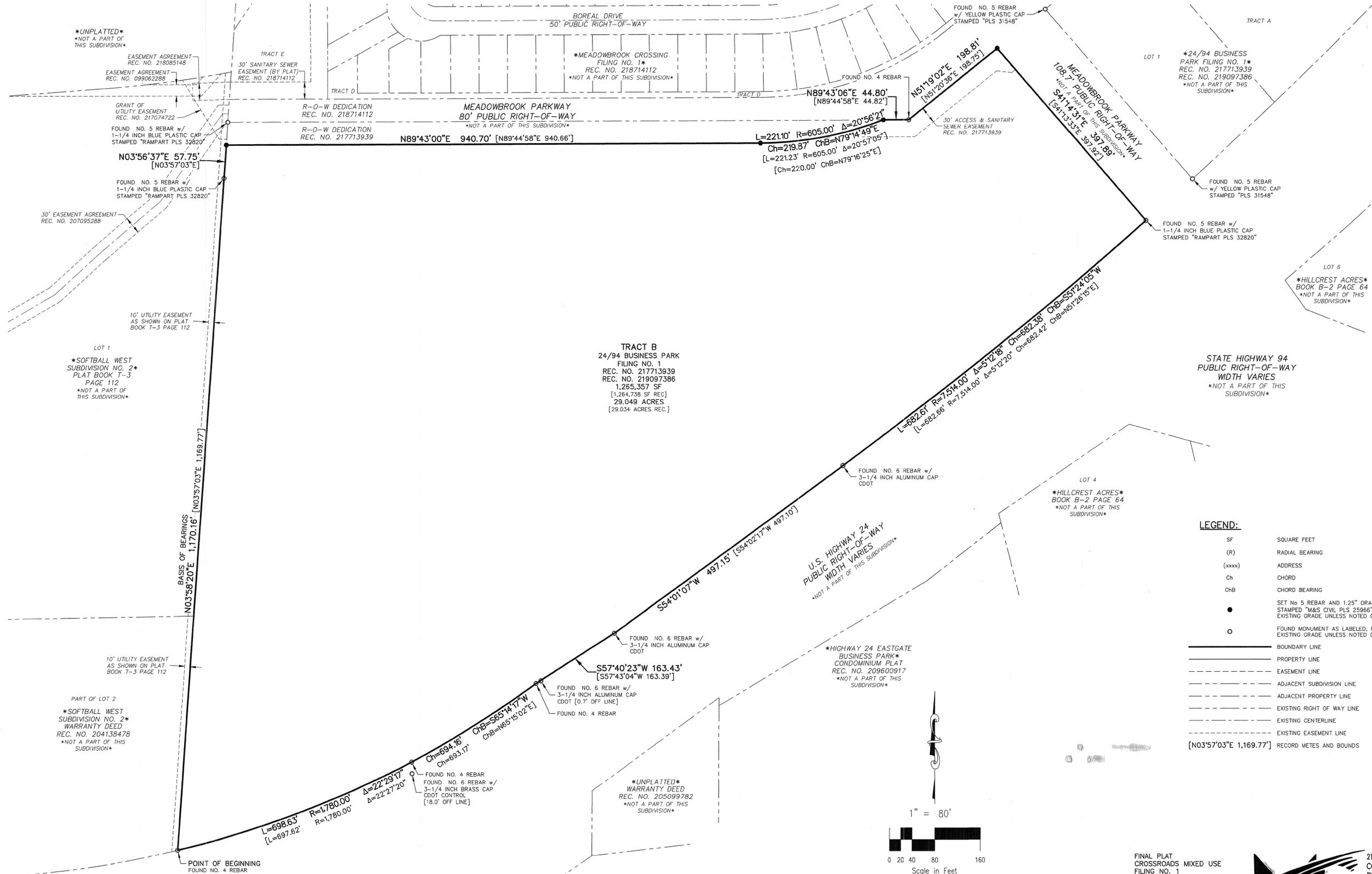
CIVIL CONSULTANTS, INC.

SHEET 1 OF 5

CROSSROADS MIXED USE FILING NO. 1

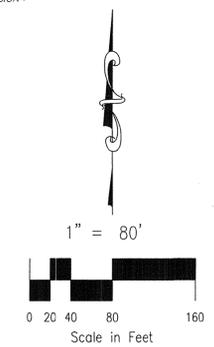
14975

A REPLAT OF TRACT B "24/94 BUSINESS PARK FILING NO. 1", BEING A TRACT OF LAND IN THE SOUTH HALF (S 1/2) OF SECTION 8, T14S, R65W, OF THE 6TH P.M., EL PASO COUNTY, COLORADO



TRACT B
 24/94 BUSINESS PARK
 FILING NO. 1
 REC. NO. 217713939
 REC. NO. 219097386
 1,265,357 SF
 [1,264,738 SF REC.]
 28.049 ACRES
 [29.034 ACRES REC.]

- LEGEND:**
- SF SQUARE FEET
 - (R) RADIAL BEARING
 - (xxxx) ADDRESS
 - Ch CHORD
 - ChB CHORD BEARING
 - SET NO. 5 REBAR AND 1.25" ORANGE CAP STAMPED "M&S CIVIL PLS 25966" FLUSH W/ EXISTING GRADE UNLESS NOTED OTHERWISE
 - FOUND MONUMENT AS LABELED, FLUSH W/ EXISTING GRADE UNLESS NOTED OTHERWISE
 - BOUNDARY LINE
 - - - PROPERTY LINE
 - - - EASEMENT LINE
 - - - ADJACENT SUBDIVISION LINE
 - - - ADJACENT PROPERTY LINE
 - - - EXISTING RIGHT OF WAY LINE
 - - - EXISTING CENTERLINE
 - - - EXISTING EASEMENT LINE
 - [N03°57'03"E 1,169.77'] RECORD METES AND BOUNDS



AS PLATTED

FINAL PLAT
 CROSSROADS MIXED USE
 FILING NO. 1
 JOB NO. 18-003
 DATE PREPARED: 06/23/2021
 DATE REVISED: 06/20/2022
 ISSUED BY: MYLAR



212 N. WAHSATCH AVE., STE 305
 COLORADO SPRINGS, CO 80903
 PHONE: 719.955.5465

PCD FIL. NO. SF-21-029

CIVIL CONSULTANTS, INC.

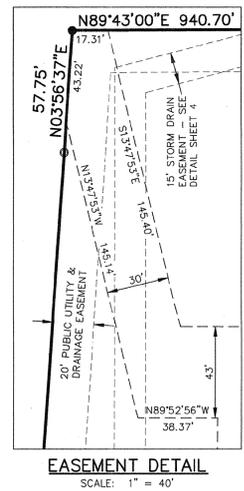
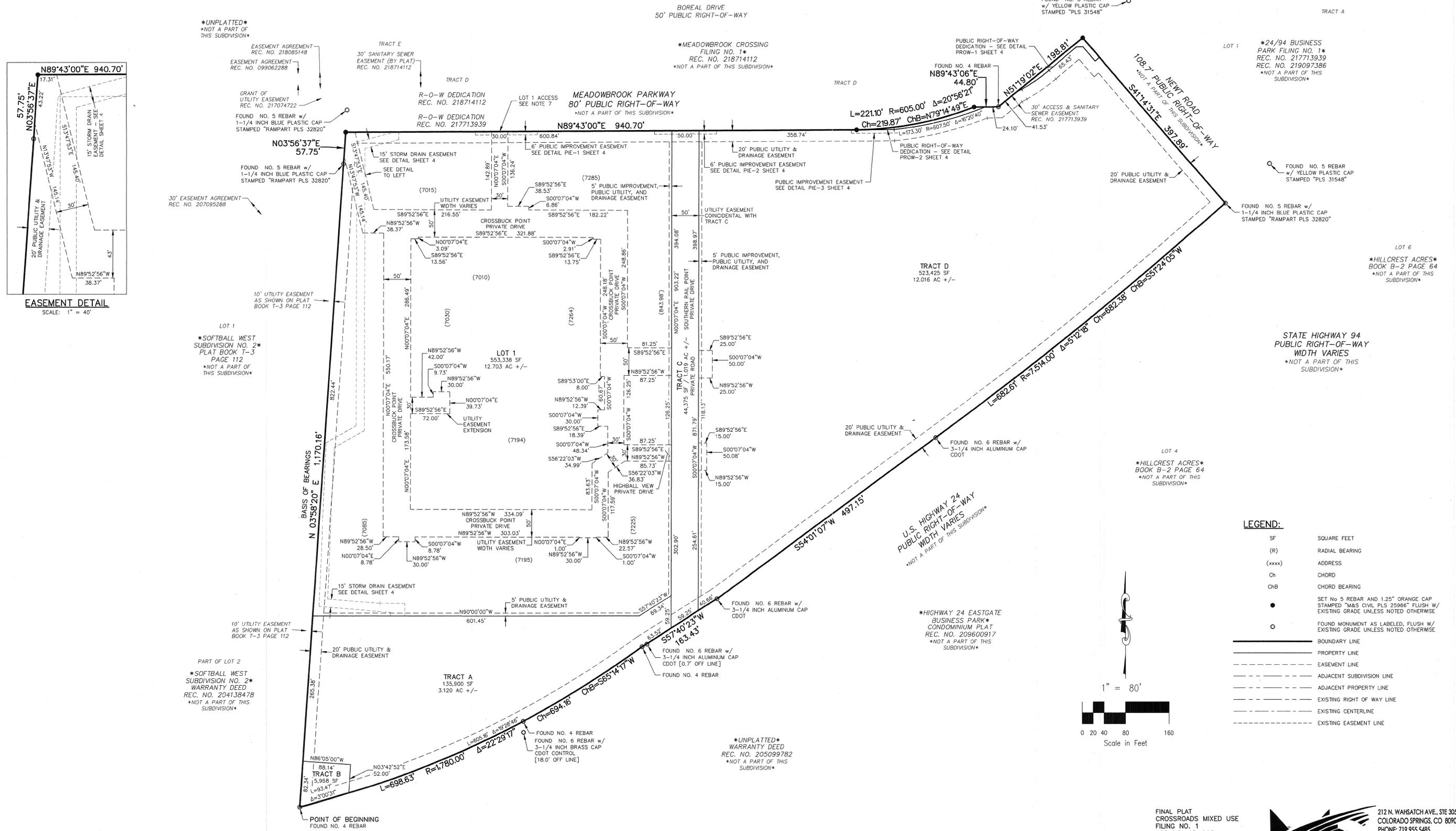
SHEET 3 OF 5

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CROSSROADS MIXED USE FILING NO. 1

14975

A REPLAT OF TRACT B "24/94 BUSINESS PARK FILING NO. 1", BEING A TRACT OF LAND IN THE SOUTH HALF (S 1/2) OF SECTION 8, T14S, R65W, OF THE 6TH P.M., EL PASO COUNTY, COLORADO



UNPLATTED
NOT A PART OF THIS SUBDIVISION

LOT 1
SOFTBALL WEST SUBDIVISION NO. 2
PLAT BOOK T-3 PAGE 112
NOT A PART OF THIS SUBDIVISION

PART OF LOT 2
SOFTBALL WEST SUBDIVISION NO. 2
WARRANTY DEED
REC. NO. 204138478
NOT A PART OF THIS SUBDIVISION

MEADOWBROOK CROSSING FILING NO. 1
REC. NO. 218714112
NOT A PART OF THIS SUBDIVISION

LOT 1
24/94 BUSINESS PARK FILING NO. 1
REC. NO. 217713939
REC. NO. 219097386
NOT A PART OF THIS SUBDIVISION

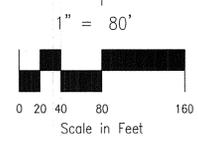
LOT 6
HILLCREST ACRES
BOOK B-2 PAGE 64
NOT A PART OF THIS SUBDIVISION

STATE HIGHWAY 94
PUBLIC RIGHT-OF-WAY
WIDTH VARIES
NOT A PART OF THIS SUBDIVISION

LOT 4
HILLCREST ACRES
BOOK B-2 PAGE 64
NOT A PART OF THIS SUBDIVISION

LEGEND:

- SF SQUARE FEET
- (R) RADIAL BEARING
- (xxxx) ADDRESS
- Ch CHORD
- ChB CHORD BEARING
- SET NO. 5 REBAR AND 1.25" ORANGE CAP STAMPED "M&S CIVIL PLS 25966" FLUSH W/ EXISTING GRADE UNLESS NOTED OTHERWISE
- FOUND MONUMENT AS LABELED, FLUSH W/ EXISTING GRADE UNLESS NOTED OTHERWISE
- BOUNDARY LINE
- - - PROPERTY LINE
- - - EASEMENT LINE
- - - ADJACENT SUBDIVISION LINE
- - - ADJACENT PROPERTY LINE
- - - EXISTING RIGHT OF WAY LINE
- - - EXISTING CENTERLINE
- - - EXISTING EASEMENT LINE



AS REPLATTED

RBD

FINAL PLAT
CROSSROADS MIXED USE
FILING NO. 1
JOB NO. 18-003
DATE PREPARED: 06/23/2021
DATE REVISED: 06/20/2022
ISSUED FOR MYLAR



212 N. WAHSATCH AVE., STE 305
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5465

PCD FIL. NO. SF-21-029

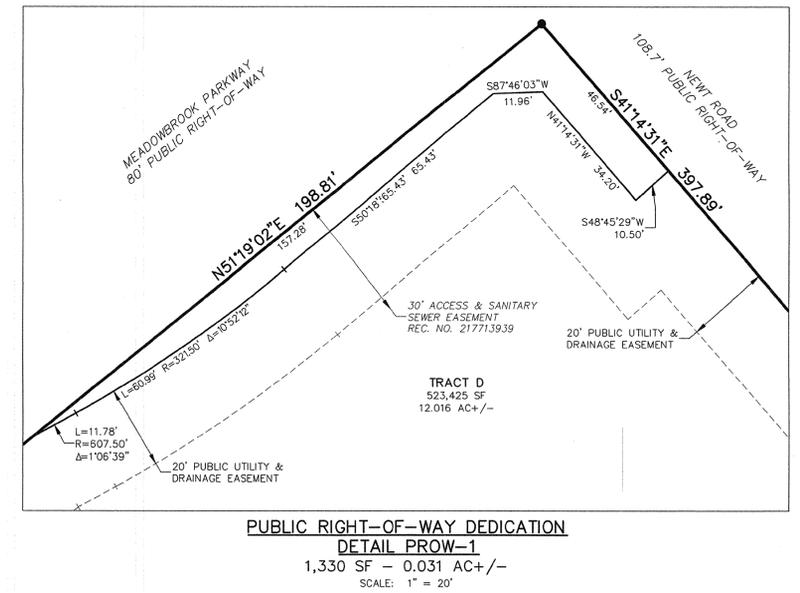
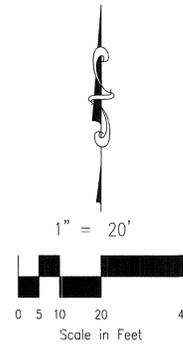
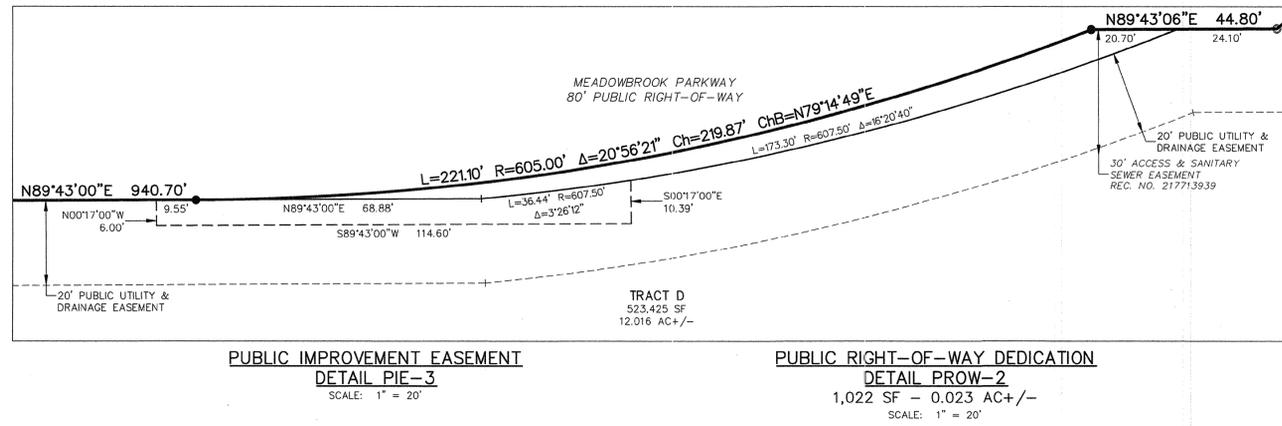
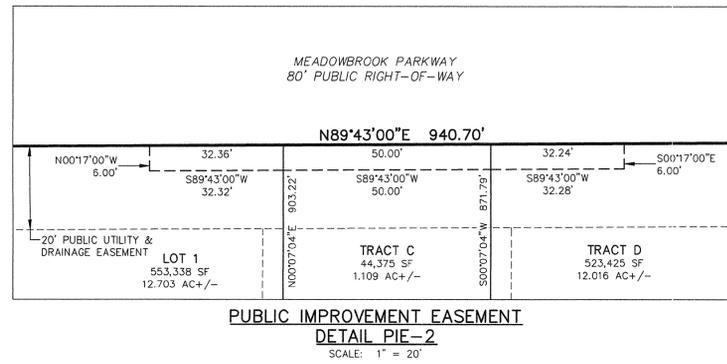
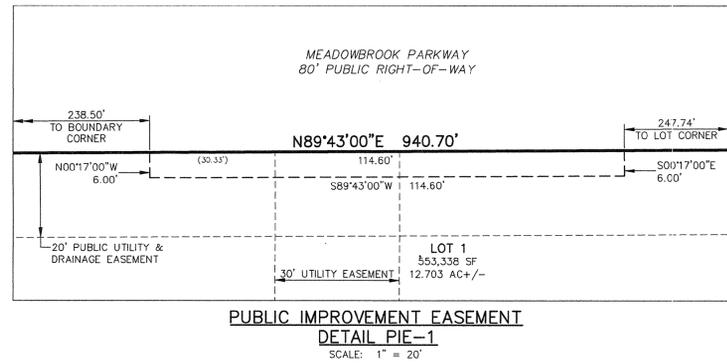
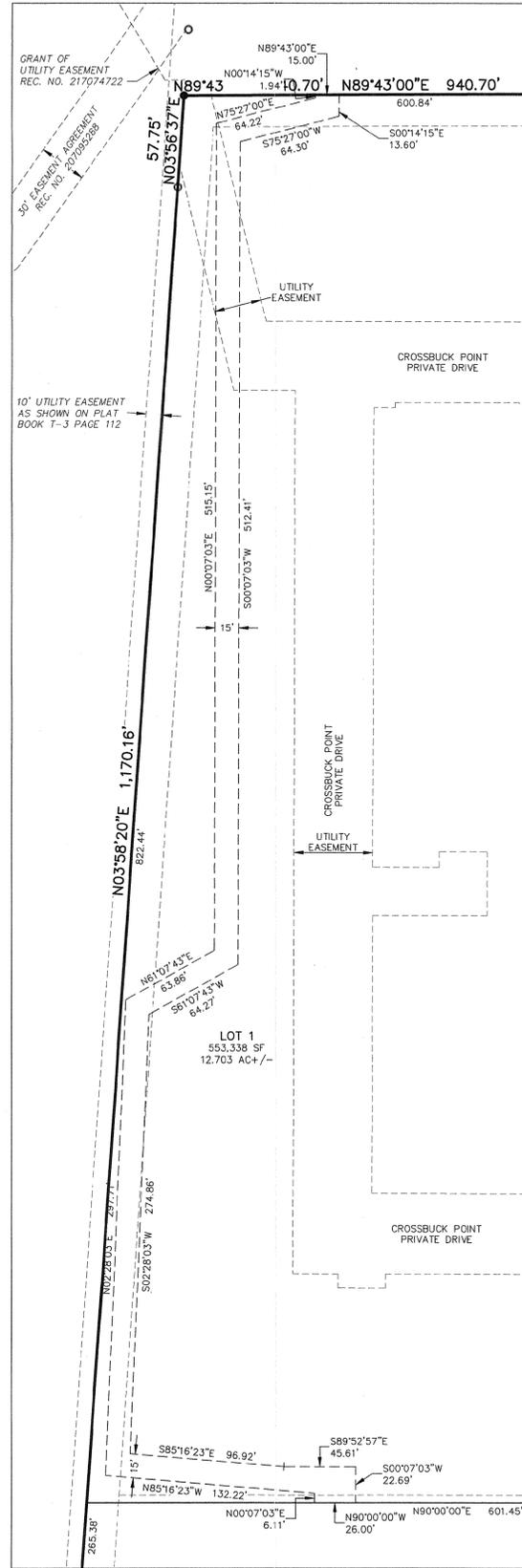
SHEET 4 OF 5

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CROSSROADS MIXED USE FILING NO. 1

14975

A REPLAT OF TRACT B "24/94 BUSINESS PARK FILING NO. 1", BEING A TRACT OF LAND IN THE SOUTH HALF (S 1/2) OF SECTION 8, T14S, R65W, OF THE 6TH P.M., EL PASO COUNTY, COLORADO



| TRACT TABLE | | | | |
|-------------|--------------|--------------------------|--------|-------|
| TRACT | SIZE (ACRES) | USE | MAINT. | OWNER |
| A | 3.120 | PRIVATE DETENTION POND | CMD1 | CMD1 |
| B | 0.137 | SIGNAGE | CMD1 | CSE |
| C | 1.019 | ACCESS, PUBLIC UTILITIES | CMD1 | CMD1 |
| D | 12.016 | FUTURE DEVELOPMENT | CSE | CSE |
| TOTAL | 16.292 | | | |

CMD1 - CROSSROADS METROPOLITAN DISTRICT NO. 1
 CSE - COLORADO SPRINGS EQUITIES, LLC

FINAL PLAT
 CROSSROADS MIXED USE
 FILING NO. 1
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212 N. WAHSATCH AVE., STE 305
 COLORADO SPRINGS, CO 80903
 PHONE: 719.955.5465

PCD FIL. NO. SF-21-029

SHEET 5 OF 5

**FINAL DRAINAGE REPORT
CROSSROAD MIXED USE FILING NO. 1
- PROPOSED DRAINAGE MAP**

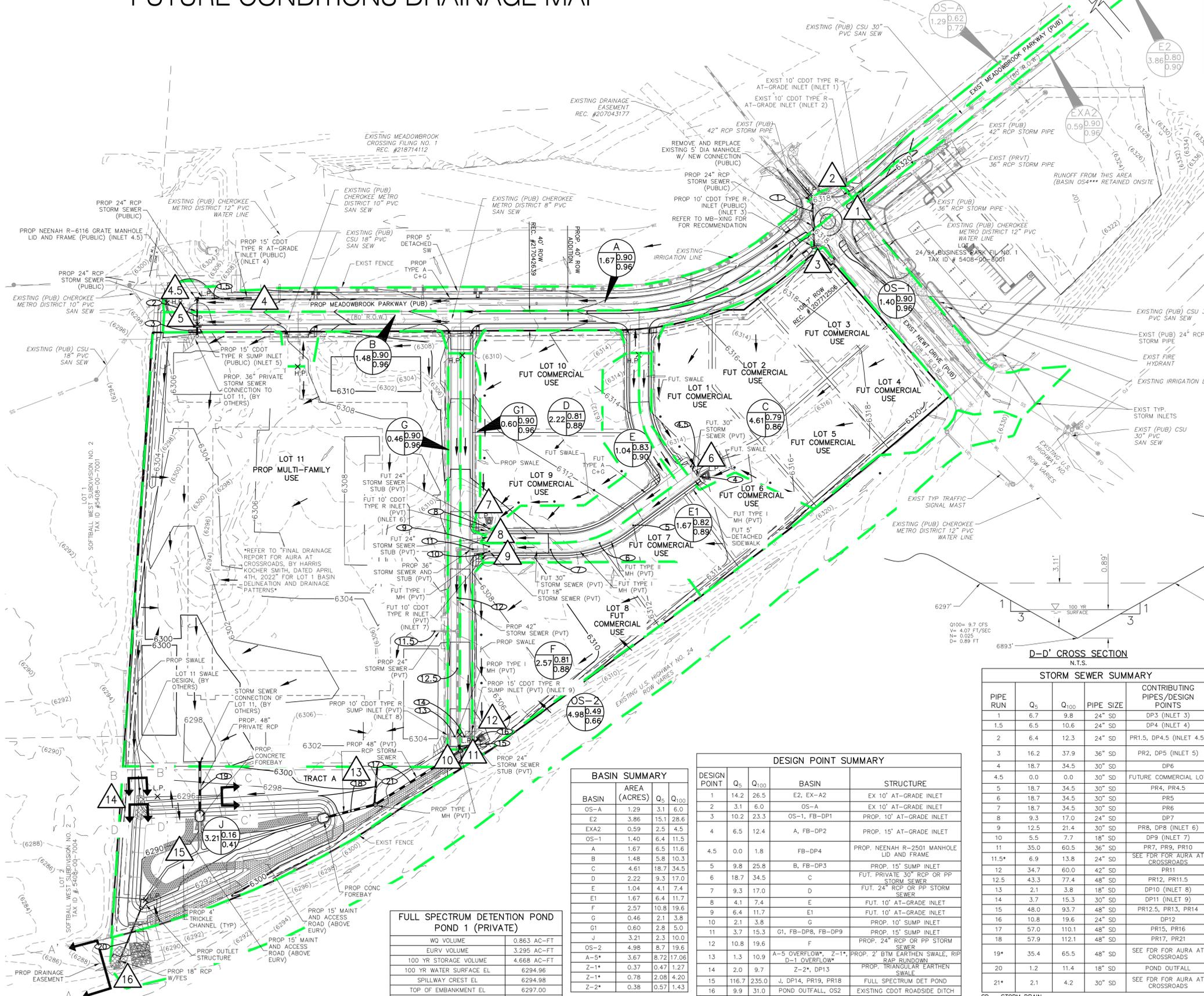
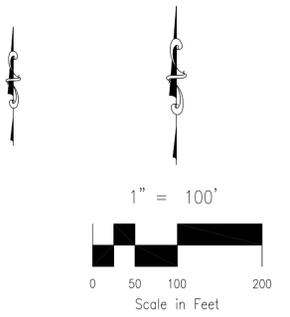
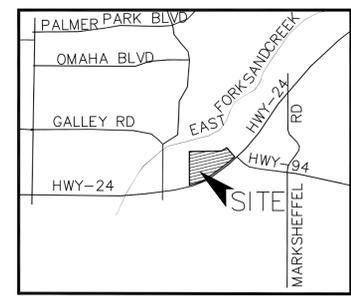
CROSSROADS MIXED USE FILING NO. 1

FUTURE CONDITIONS DRAINAGE MAP

EXISTING MEADOWBROOK CROSSING FILING NO. 1 REC. #21874112

BASINS OS-A AND EXA2 REFER TO THE FINAL DRAINAGE REPORT FOR LOT 1 24/94 BUSINESS PARKING FILING NO.1 ON PLATTE AVENUE AND MEADOWBROOK PARKWAY FOR OFFSITE WATERSHED CONFIGURATION DETAILS.

REFER TO CLAREMONT BUSINESS PARK FILING NO. 2 FOR OFFSITE WATERSHED CONFIGURATION DETAILS.

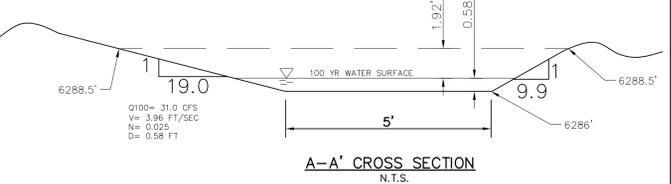
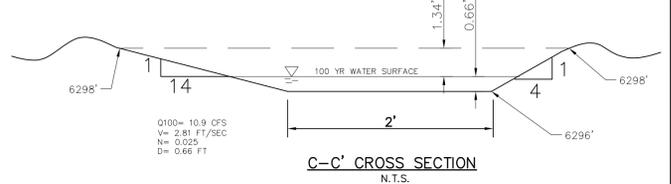
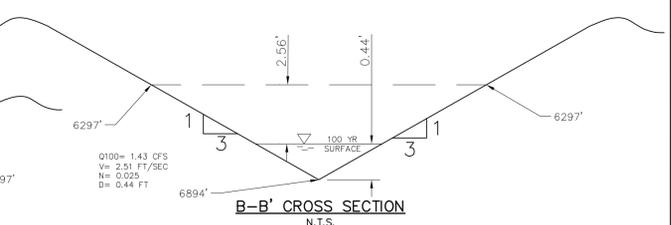


VICINITY MAP
N.T.S.

BASIN DESIGNATION
Z
25 1.25
35 C5
ACRES

- 1 SURFACE DESIGN POINT
- PROPOSED BASIN BOUNDARY
- PIPE RUN LABEL
- PROP MAJ CONT
- PROP MIN CONT
- EXIST MAJ CONT
- EXIST MIN CONT
- PROPOSED STORM SEWER PIPE
- PROPOSED STORM SEWER PIPE (OTHERS)
- FUTURE STORM SEWER PIPE
- EXISTING FLOW DIRECTION ARROW
- H.P. X HIGH POINT
- EXISTING SWALE

- LEGEND**
- SITE BOUNDARY
 - PROPOSED UTILITY EASEMENT
 - PROPOSED DRAINAGE EASEMENT
 - PROPOSED LANDSCAPE EASEMENT
 - LOT LINE
 - ST STORM SEWER LINE
 - UE EX. UNDERGROUND ELECTRIC LINE
 - SS EX. SANITARY SEWER LINE
 - WL EX. WATER LINE
 - ST EX. STORM SEWER LINE
 - 9 LOT NUMBER
 - EX. IRRIGATION VALVE
 - EX. STORM INLET
 - EX. GAS TEST NODE
 - EX. TELEPHONE PEDESTAL
 - EX. ELECTRIC VAULT
 - EX. SANITARY MANHOLE
 - EX. WATER VALVE
 - PROPOSED RIPRAP
 - EMERGENCY OVERTFLOW DIRECTION
 - LOW POINT
 - PROPOSED SWALE



STORM SEWER SUMMARY

| PIPE RUN | Q _s | Q ₁₀₀ | PIPE SIZE | CONTRIBUTING PIPES/DESIGN POINTS |
|----------|----------------|------------------|-----------|----------------------------------|
| 1 | 6.7 | 9.8 | 24" SD | DP3 (INLET 3) |
| 1.5 | 6.5 | 10.6 | 24" SD | DP4 (INLET 4) |
| 2 | 6.4 | 12.3 | 24" SD | PR1.5, DP4.5 (INLET 4.5) |
| 3 | 16.2 | 37.9 | 36" SD | PR2, DP5 (INLET 5) |
| 4 | 18.7 | 34.5 | 30" SD | DP6 |
| 4.5 | 0.0 | 0.0 | 30" SD | FUTURE COMMERCIAL LOT |
| 5 | 18.7 | 34.5 | 30" SD | PR4, PR4.5 |
| 6 | 18.7 | 34.5 | 30" SD | PR5 |
| 7 | 18.7 | 34.5 | 30" SD | PR6 |
| 8 | 9.3 | 17.0 | 24" SD | DP7 |
| 9 | 12.5 | 21.4 | 30" SD | PR8, DP8 (INLET 6) |
| 10 | 5.5 | 7.7 | 18" SD | DP9 (INLET 7) |
| 11 | 35.0 | 60.5 | 36" SD | PR7, PR9, PR10 |
| 11.5* | 6.9 | 13.8 | 24" SD | SEE FOR FOR AURA AT CROSSROADS |
| 12 | 34.7 | 60.0 | 42" SD | PR11 |
| 12.5 | 43.3 | 77.4 | 48" SD | PR12, PR11.5 |
| 13 | 2.1 | 3.8 | 18" SD | DP10 (INLET 8) |
| 14 | 3.7 | 15.3 | 30" SD | DP11 (INLET 9) |
| 15 | 48.0 | 93.7 | 48" SD | PR12.5, PR13, PR14 |
| 16 | 10.8 | 19.6 | 24" SD | DP12 |
| 17 | 57.0 | 110.1 | 48" SD | PR15, PR16 |
| 18 | 57.9 | 112.1 | 48" SD | PR17, PR21 |
| 19* | 35.4 | 65.5 | 48" SD | SEE FOR FOR AURA AT CROSSROADS |
| 20 | 1.2 | 11.4 | 18" SD | POND OUTFALL |
| 21* | 2.1 | 4.2 | 30" SD | SEE FOR FOR AURA AT CROSSROADS |

BASIN SUMMARY

| BASIN | AREA (ACRES) | Q _s | Q ₁₀₀ |
|-------|--------------|----------------|------------------|
| OS-A | 1.29 | 3.1 | 6.0 |
| E2 | 3.86 | 15.1 | 28.6 |
| EXA2 | 0.59 | 2.5 | 4.5 |
| OS-1 | 1.40 | 6.4 | 11.5 |
| A | 1.67 | 6.5 | 11.6 |
| B | 1.48 | 5.8 | 10.3 |
| C | 4.61 | 18.7 | 34.5 |
| D | 2.22 | 9.3 | 17.0 |
| E | 1.04 | 4.1 | 7.4 |
| E1 | 1.67 | 6.4 | 11.7 |
| F | 2.57 | 10.8 | 19.6 |
| G | 0.46 | 2.1 | 3.8 |
| G1 | 0.60 | 2.8 | 5.0 |
| J | 3.21 | 2.3 | 10.0 |
| OS-2 | 4.98 | 8.7 | 19.6 |
| A-5* | 3.67 | 8.72 | 17.06 |
| Z-1* | 0.37 | 0.47 | 1.27 |
| Z-1* | 0.78 | 2.08 | 4.20 |
| Z-2* | 0.38 | 0.57 | 1.43 |

DESIGN POINT SUMMARY

| DESIGN POINT | Q _s | Q ₁₀₀ | BASIN | STRUCTURE |
|--------------|----------------|------------------|----------------------------------|---|
| 1 | 14.2 | 26.5 | E2, EX-A2 | EX 10" AT-GRADE INLET |
| 2 | 3.1 | 6.0 | OS-A | EX 10" AT-GRADE INLET |
| 3 | 10.2 | 23.3 | OS-1, FB-DP1 | PROP. 10" AT-GRADE INLET |
| 4 | 6.5 | 12.4 | A, FB-DP2 | PROP. 15" AT-GRADE INLET |
| 4.5 | 0.0 | 1.8 | FB-DP4 | PROP. NEENAH R-2501 MANHOLE LID AND FRAME |
| 5 | 9.8 | 25.8 | B, FB-DP3 | PROP. 15" SUMP INLET |
| 6 | 18.7 | 34.5 | C | FUT. PRIVATE 30" RCP OR PP STORM SEWER |
| 7 | 9.3 | 17.0 | D | FUT. 24" RCP OR PP STORM SEWER |
| 8 | 4.1 | 7.4 | E | FUT. 10" AT-GRADE INLET |
| 9 | 6.4 | 11.7 | E1 | FUT. 10" AT-GRADE INLET |
| 10 | 2.1 | 3.8 | G | PROP. 10" SUMP INLET |
| 11 | 3.7 | 15.3 | G1, FB-DP8, FB-DP9 | PROP. 15" SUMP INLET |
| 12 | 10.8 | 19.6 | F | PROP. 24" RCP OR PP STORM SEWER |
| 13 | 1.3 | 10.9 | A-5 OVERFLOW, Z-1*, D-1 OVERFLOW | PROP. 2" BTM EARTHEN SWALE, RIP RAP RUNDOWN |
| 14 | 2.0 | 9.7 | Z-2*, DP13 | PROP. TRIANGULAR EARTHEN SWALE |
| 15 | 116.7 | 235.0 | J, DP14, PR19, PR18 | FULL SPECTRUM DET POND |
| 16 | 9.9 | 31.0 | POND OUTFALL, OS2 | EXISTING CDDOT ROADSIDE DITCH |

FULL SPECTRUM DETENTION POND POND 1 (PRIVATE)

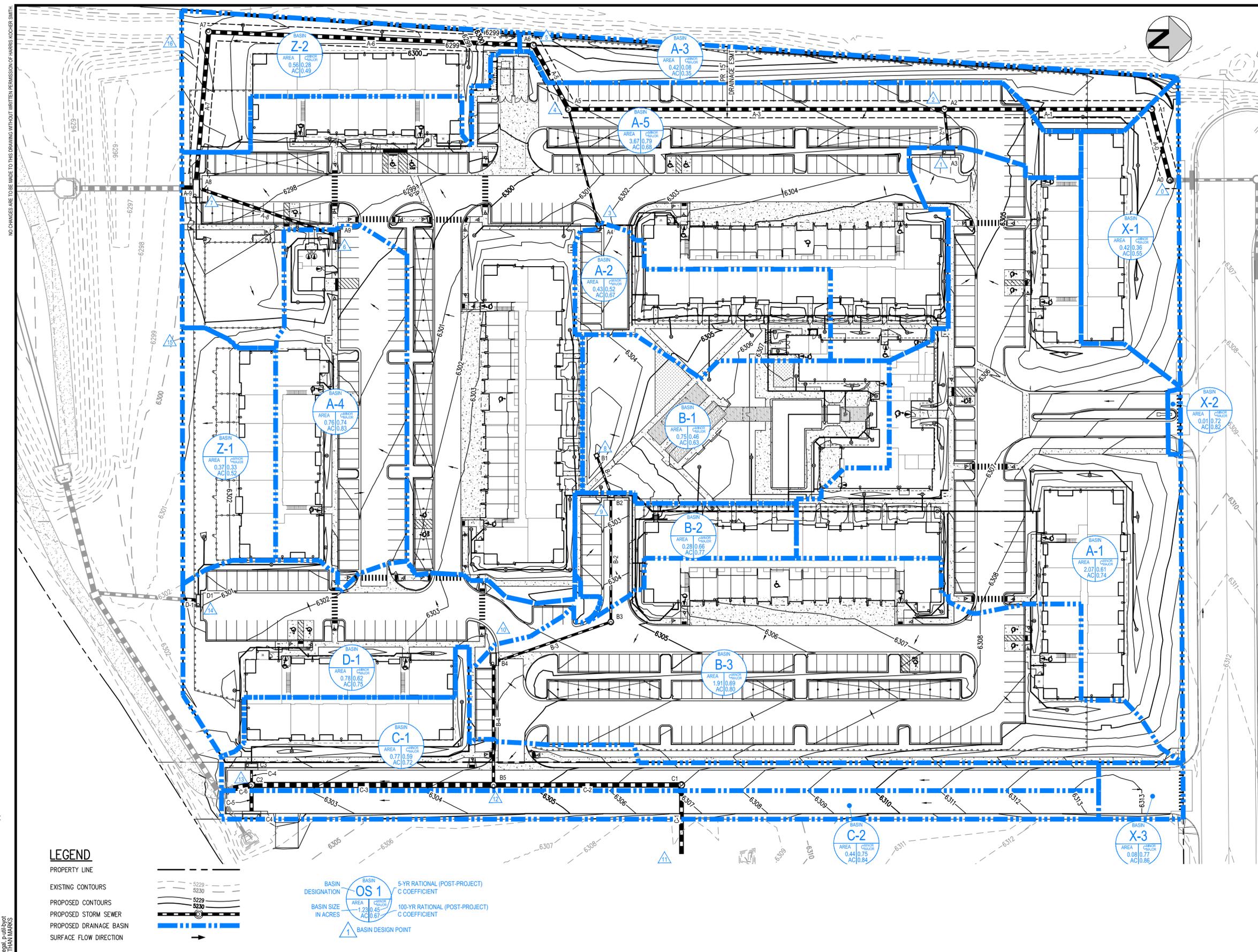
| | |
|----------------------------|-------------|
| WQ VOLUME | 0.863 AC-FT |
| EURV VOLUME | 3.295 AC-FT |
| 100 YR STORAGE VOLUME | 4.668 AC-FT |
| 100 YR WATER SURFACE EL | 6294.96 |
| SPILLWAY CREST EL | 6294.98 |
| TOP OF EMBANKMENT EL | 6297.00 |
| SPILLWAY DESIGN FLOW DEPTH | 0.86 FT |

*REFER TO FDR FOR AURA AT CROSSROADS, DATED APRIL 4TH 2022, FOR CONTRIBUTING BASIN DETAILS

SD = STORM DRAIN
REFER TO FDR FOR AURA AT CROSSROADS FOR CONTRIBUTING PIPE FLOW DETAILS

212 N. WAHSATCH AVE., STE 305
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

**FINAL DRAINAGE REPORT
FOR AURA AT CROSSROADS
- PROPOSED DRAINAGE MAP**



| STRUCTURE TABLE | | STRUCTURE TABLE | |
|-----------------|----------------------|-----------------|------------------|
| STRUCTURE ID | DESCRIPTION | STRUCTURE ID | DESCRIPTION |
| A0 | TYPE I MANHOLE | B2 | INLET TYPE R 5' |
| A1 | TYPE I MANHOLE | B3 | TYPE II MANHOLE |
| A2 | TYPE I MANHOLE | B4 | INLET TYPE R 10' |
| A3 | INLET TYPE R 10' | B5 | TYPE I MANHOLE |
| A4 | INLET TYPE R 5' | C1 | TYPE I MANHOLE |
| A5 | TYPE I MANHOLE | C2 | TYPE I MANHOLE |
| A6 | TYPE I MANHOLE | C3 | INLET TYPE R 10' |
| A7 | TYPE I MANHOLE | C4 | INLET TYPE R 15' |
| A8 | INLET TYPE R 15' MOD | D1 | INLET TYPE R 10' |
| A9 | INLET TYPE R 10' | | |
| B1 | INLET TYPE C | | |

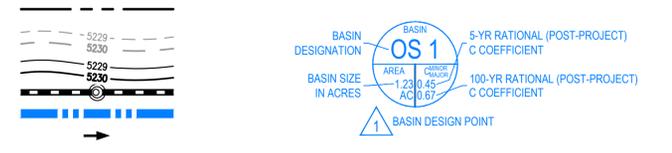
| PIPE TABLE | | | | | | |
|------------|--------------------|----------------------|------|---------|-------|----------|
| NAME | UPSTREAM STRUCTURE | DOWNSTREAM STRUCTURE | SIZE | LENGTH | SLOPE | MATERIAL |
| A-0 | A0 | A1 | 36" | 64.26' | 0.50% | RCP |
| A-1 | A1 | A2 | 36" | 182.49' | 0.50% | RCP |
| A-2 | A3 | A2 | 18" | 36.82' | 4.50% | RCP |
| A-3 | A2 | A5 | 36" | 331.29' | 0.50% | RCP |
| A-4 | A4 | A5 | 15" | 102.94' | 1.50% | RCP |
| A-5 | A5 | A6 | 36" | 64.06' | 2.03% | RCP |
| A-6 | A6 | A7 | 36" | 286.28' | 0.58% | RCP |
| A-7 | A7 | A8 | 36" | 130.14' | 0.50% | RCP |
| A-8 | A9 | A8 | 24" | 125.80' | 1.00% | RCP |
| A-9 | A8 | A8 | 48" | 10.05' | 0.50% | RCP |
| B-1 | B1 | B3 | 15" | 35.69' | 0.50% | RCP |
| B-2 | B2 | B3 | 18" | 109.40' | 0.50% | RCP |
| B-3 | B3 | B4 | 18" | 107.86' | 0.50% | RCP |
| B-4 | B4 | B5 | 24" | 110.33' | 0.50% | RCP |
| C-1 | C1 | C1 | 36" | 60.28' | 2.00% | RCP |
| C-2 | C1 | B5 | 42" | 165.64' | 1.30% | RCP |
| C-3 | B5 | C2 | 48" | 213.24' | 0.60% | RCP |
| C-4 | C3 | C2 | 18" | 15.67' | 5.00% | RCP |
| C-5 | C4 | C2 | 30" | 25.67' | 5.00% | RCP |
| C-6 | C2 | C2 | 48" | 16.52' | 0.80% | RCP |

| DESIGN POINT SUMMARY | | | |
|----------------------|----------|------------|--|
| DESIGN POINT | Q5 (CFS) | Q100 (CFS) | |
| 0 | 16.48 | 38.58 | |
| 1 | 4.61 | 9.36 | |
| 2 | 19.17 | 43.45 | |
| 3 | 0.88 | 1.90 | |
| 4 | 19.60 | 44.42 | |
| 5 | 19.55 | 45.20 | |
| 6 | 2.76 | 5.24 | |
| 7 | 30.16 | 65.43 | |
| 8 | 1.38 | 3.17 | |
| 9 | 2.06 | 4.57 | |
| 10 | 6.85 | 13.80 | |
| 11 | 35.00 | 60.50 | |
| 12 | 41.65 | 73.75 | |
| 13 | 44.47 | 79.25 | |
| 14 | 2.08 | 4.20 | |
| 15 | 0.47 | 1.27 | |
| 16 | 0.57 | 1.43 | |

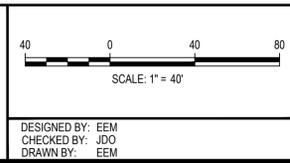
| DIRECT RUNOFF SUMMARY | | | |
|-----------------------|-----------|----------|------------|
| SUBBASIN | AREA (AC) | Q5 (CFS) | Q100 (CFS) |
| X-1 | 0.42 | 0.58 | 1.50 |
| X-2 | 0.01 | 0.05 | 0.10 |
| X-3 | 0.08 | 0.26 | 0.50 |
| A-1 | 2.07 | 4.61 | 9.36 |
| A-2 | 0.43 | 0.88 | 1.90 |
| A-3 | 0.42 | 0.13 | 0.94 |
| A-4 | 0.76 | 2.76 | 5.24 |
| A-5 | 3.67 | 8.72 | 17.06 |
| B-1 | 0.75 | 1.38 | 3.17 |
| B-2 | 0.28 | 0.74 | 1.45 |
| B-3 | 1.91 | 4.89 | 9.52 |
| C-1 | 0.77 | 1.86 | 3.84 |
| C-2 | 0.44 | 1.39 | 2.64 |
| D-1 | 0.78 | 2.08 | 4.20 |
| Z-1 | 0.37 | 0.47 | 1.27 |
| Z-2 | 0.38 | 0.57 | 1.43 |

LEGEND

- PROPERTY LINE
- EXISTING CONTOURS
- PROPOSED CONTOURS
- PROPOSED STORM SEWER
- PROPOSED DRAINAGE BASIN
- SURFACE FLOW DIRECTION



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| ISSUE DATE: 08-06-2021 | |
|------------------------|---------------------|
| DATE | REVISION COMMENTS |
| 10-29-2021 | PER COUNTY COMMENTS |
| 01-13-2022 | PER COUNTY COMMENTS |

1120 Lincoln Street, Suite 1000
 Denver, Colorado 80203
 P: 303.623.6300 F: 303.623.6311
 HarrisKocherSmith.com

TRINISIC ACQUISITION COMPANY, LLC

AURA AT CROSSROADS
 DRAINAGE PLAN

PROJECT #: 200823
 SHEET NUMBER
1
 1 OF 1

HYDROLIC AND HYDRALIC CALCULATIONS

CROSSROADS MIXED USE FIL. NO.1 FOR UNDERGROUND DETENTION
PRELIMINARY/FINAL DRAINAGE CALCULATIONS
(Area Runoff Coefficient Summary)

| BASIN | TOTAL AREA (Sq Ft) | TOTAL AREA (Acres) | STREETS / COMMERC. | | | MULTI-FAMILY/PARKS | | | OVERLAND / UNDEVELOPED | | | WEIGHTED | |
|------------------------|-----------------------|-----------------------|--------------------|----------------|------------------|--------------------|----------------|------------------|------------------------|----------------|------------------|----------------|------------------|
| | | | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | AREA (Acres) | C ₅ | C ₁₀₀ | C ₅ | C ₁₀₀ |
| PROPOSED BASINS | | | | | | | | | | | | | |
| <i>A</i> | 70707 | 1.62 | 0.00 | 0.90 | 0.96 | 1.62 | 0.12 | 0.39 | 0.00 | 0.08 | 0.35 | <i>0.12</i> | <i>0.39</i> |
| <i>B</i> | 49122 | 1.13 | 0.00 | 0.90 | 0.96 | 1.13 | 0.12 | 0.39 | 0.00 | 0.08 | 0.35 | <i>0.12</i> | <i>0.39</i> |
| <i>C</i> | 23922 | 0.55 | 0.26 | 0.90 | 0.96 | 0.29 | 0.12 | 0.39 | 0.00 | 0.08 | 0.35 | <i>0.49</i> | <i>0.66</i> |

Calculated by: GT
Date: 12/10/2022
Checked by: DLM

**CROSSROADS MIXED USE FIL. NO.1 FOR UNDERGROUND DETENTION
PRELIMINARY/FINAL DRAINAGE CALCULATIONS
(Area Drainage Summary)**

| <i>From Area Runoff Coefficient Summary</i> | | | | OVERLAND | | | | STREET / CHANNEL FLOW | | | | Time of Travel (T_t) | | INTENSITY # | | TOTAL FLOWS | |
|---|-----------------------|-----------------------------------|------------------|-----------------|----------------|----------------|-------------------------|------------------------------|--------------|-------------------|-------------------------|---------------------------------------|----------------|---------------------------|-----------------------------|----------------------------|------------------------------|
| BASIN | AREA TOTAL (Acres) | C ₅ | C ₁₀₀ | C ₅ | Length (ft) | Height (ft) | T _C (min) | Length (ft) | Slope (%) | Velocity (fps) | T _t (min) | TOTAL (min) | CHECK (min) | I ₅ (in/hr) | I ₁₀₀ (in/hr) | Q ₅ (c.f.s.) | Q ₁₀₀ (c.f.s.) |
| | | <small>From DCM Table S-1</small> | | | | | | | | | | | | | | | |
| Proposed Area Drainage Summary | | | | | | | | | | | | | | | | | |
| A | 1.62 | 0.12 | 0.39 | 0.12 | 50 | 0.25 | 15.7 | 261 | 1.9% | 1.0 | 4.5 | 20.2 | 11.7 | 3.1 | 5.2 | 0.6 | 3.3 |
| B | 1.13 | 0.12 | 0.39 | 0.12 | 50 | 0.25 | 15.7 | 134 | 0.5% | 0.5 | 4.5 | 20.2 | 11.0 | 3.1 | 5.2 | 0.4 | 2.3 |
| C | 0.55 | 0.49 | 0.66 | 0.49 | 25 | 0.5 | 4.4 | 273 | 2.0% | 2.8 | 1.6 | 6.0 | 11.7 | 4.9 | 8.2 | 1.3 | 3.0 |

Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: GT
Date: 12/10/2022
Checked by: DLM

**CROSSROADS MIXED USE FIL. NO.1 FOR UNDERGROUND DETENTION
PRELIMINARY/FINAL DRAINAGE CALCULATIONS
(Basin Routing Summary)**

| From Area Runoff Coefficient Summary | | | | OVERLAND | | | | PIPE / CHANNEL FLOW | | | | Time of Travel (T _t) | INTENSITY * | | TOTAL FLOWS | | COMMENTS | |
|--|---------------------|-----------------|-------------------|---------------------------------|----------------|----------------|-------------------------|---------------------|--------------|-------------------|-------------------------|----------------------------------|----------------|------------------|----------------|------------------|------------------------------------|--|
| DESIGN POINT | CONTRIBUTING BASINS | CA ₅ | CA ₁₀₀ | C _s | Length (ft) | Height (ft) | T _c (min) | Length (ft) | Slope (%) | Velocity (fps) | T _t (min) | TOTAL (min) | I ₅ | I ₁₀₀ | Q ₅ | Q ₁₀₀ | | |
| | | (in/hr) | (in/hr) | | | | | | | | | | (c.f.s.) | (c.f.s.) | | | | |
| PROPOSED DRAINAGE BASIN ROUTING SUMMARY | | | | | | | | | | | | | | | | | | |
| 1 | A, OFFSITE DP16* | 0.35 | 0.90 | | | | | | | | | 20.2 | 3.1 | 5.2 | 1.1 | 4.7 | Prop 2' x 2' Area Inlet | |
| | | | | T _c for A Used | | | | | | | | | | | | | | |
| 2 | B, OFFSITE DP15* | 0.26 | 0.63 | | | | | | | | | 20.2 | 3.1 | 5.2 | 0.8 | 3.2 | Prop 2' x 2' Area Inlet | |
| | | | | T _c for Basin B used | | | | | | | | | | | | | | |
| 3 | C | 0.27 | 0.36 | | | | | | | | | 6.0 | 4.9 | 8.2 | 1.3 | 3.0 | Proposed 5' CDOT Type R Sump Inlet | |
| | | | | T _c for C Used | | | | | | | | | | | | | | |

Intensity equations assume a minimum travel time of 5 minutes.

*FDR for AURA at Crossroads, prepared by HKS, dated April 4, 2022

**FDR for Crossroads Mixed Use Filing No.1, prepared by MS Civil Consultants, Inc., dated April 2022

GT _____
Date: 12/10/2022
Checked by: DLM _____

**CROSSROADS MIXED USE FIL. NO.1 FOR UNDERGROUND DETENTION
PRELIMINARY/FINAL DRAINAGE CALCULATIONS
(Storm Sewer Routing Summary)**

| PIPE RUN | Contributing Pipes/Design Points | Equivalent CA ₅ | Equivalent CA ₁₀₀ | Maximum T _c | Intensity* | | Flow | | PIPE SIZE |
|----------|----------------------------------|----------------------------|------------------------------|------------------------|----------------|------------------|----------------|------------------|-----------|
| | | | | | I ₅ | I ₁₀₀ | Q ₅ | Q ₁₀₀ | |
| 15** | PR12.5, PR13, PR14 | 10.52 | 12.24 | 7.5 | 4.6 | 7.7 | 48.0 | 93.7 | 48" RCP |
| 16** | DP12 | 2.08 | 2.26 | 5.0 | 5.2 | 8.7 | 10.8 | 19.6 | 24" RCP |
| 17 | PR15**, PR16** | 12.60 | 14.50 | 7.7 | 4.5 | 7.6 | 57.0 | 110.1 | 54" RCP |
| 19* | SEE FDR FOR AURA AT CROSSROADS | 10.05 | 11.09 | 15.0 | 3.5 | 5.9 | 35.4 | 65.5 | 48" RCP |
| 21* | SEE FDR FOR AURA AT CROSSROADS | 0.48 | 0.58 | 8.8 | 4.3 | 7.3 | 2.1 | 4.2 | 30" RCP |
| 22 | PR17, PR21* | 13.08 | 15.08 | 8.8 | 4.3 | 7.3 | 56.5 | 109.4 | 54" RCP |
| 23 | DP3 | 0.27 | 0.36 | 6.0 | 4.9 | 8.2 | 1.3 | 3.0 | 15" RCP |
| 24 | PR22, PR23 | 13.35 | 15.45 | 8.8 | 4.3 | 7.3 | 57.7 | 112.0 | 54" RCP |
| 25 | DP1 | 0.35 | 0.90 | 20.2 | 3.1 | 5.2 | 1.1 | 4.7 | 18" RCP |
| 26 | DP2 | 0.26 | 0.63 | 20.2 | 3.1 | 5.2 | 0.8 | 3.2 | 18" RCP |
| 27 | PR19*, PR25, PR26 | 10.66 | 12.62 | 15.0 | 3.5 | 5.9 | 37.5 | 74.6 | 54" RCP |
| 20 | POND OUTFALL | PER | MHFD | WKSHT | | | 1.2 | 11.4 | 18" RCP |

*FDR for AURA at Crossroads, prepared by HKS, dated April 4, 2022

Calculated by: GT

**FDR for Crossroads Mixed Use Filing No.1, prepared by MS Civil Consultants, Inc., dated April 2022

DP - Design Point

FB- Flow By from Design Point

Checked by: DLM

EX - Existing Design Point

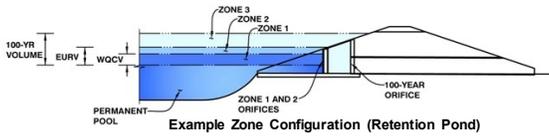
INT- Intercepted Flow from Design Point

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: **CROSSROADS MIXED USE**

Basin ID: **POND 1 UNDERGROUND DETENTION**



Example Zone Configuration (Retention Pond)

Watershed Information

| | | |
|---|------------|---------|
| Selected BMP Type = | SF | |
| Watershed Area = | 32.20 | acres |
| Watershed Length = | 1,725 | ft |
| Watershed Length to Centroid = | 1,000 | ft |
| Watershed Slope = | 0.006 | ft/ft |
| Watershed Imperviousness = | 78.67% | percent |
| Percentage Hydrologic Soil Group A = | 95.4% | percent |
| Percentage Hydrologic Soil Group B = | 4.6% | percent |
| Percentage Hydrologic Soil Groups C/D = | 0.0% | percent |
| Target WQCV Drain Time = | 12.0 | hours |
| Location for 1-hr Rainfall Depths = | User Input | |

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Update the percent imperviousness to account for converting Tract A from open space to a park & for the future overflow parking within basin C.

Optional User Overrides

| | | | | |
|--|-------|-----------|------|-----------|
| Water Quality Capture Volume (WQCV) = | 0.687 | acre-feet | | acre-feet |
| Excess Urban Runoff Volume (EURV) = | 3.293 | acre-feet | | acre-feet |
| 2-yr Runoff Volume (P1 = 1.19 in.) = | 2.407 | acre-feet | 1.19 | inches |
| 5-yr Runoff Volume (P1 = 1.5 in.) = | 3.122 | acre-feet | 1.50 | inches |
| 10-yr Runoff Volume (P1 = 1.75 in.) = | 3.696 | acre-feet | 1.75 | inches |
| 25-yr Runoff Volume (P1 = 2 in.) = | 4.394 | acre-feet | 2.00 | inches |
| 50-yr Runoff Volume (P1 = 2.25 in.) = | 5.058 | acre-feet | 2.25 | inches |
| 100-yr Runoff Volume (P1 = 2.52 in.) = | 5.833 | acre-feet | 2.52 | inches |
| 500-yr Runoff Volume (P1 = 3.14 in.) = | 7.551 | acre-feet | | inches |
| Approximate 2-yr Detention Volume = | 2.178 | acre-feet | | |
| Approximate 5-yr Detention Volume = | 2.835 | acre-feet | | |
| Approximate 10-yr Detention Volume = | 3.393 | acre-feet | | |
| Approximate 25-yr Detention Volume = | 4.014 | acre-feet | | |
| Approximate 50-yr Detention Volume = | 4.379 | acre-feet | | |
| Approximate 100-yr Detention Volume = | 4.723 | acre-feet | | |

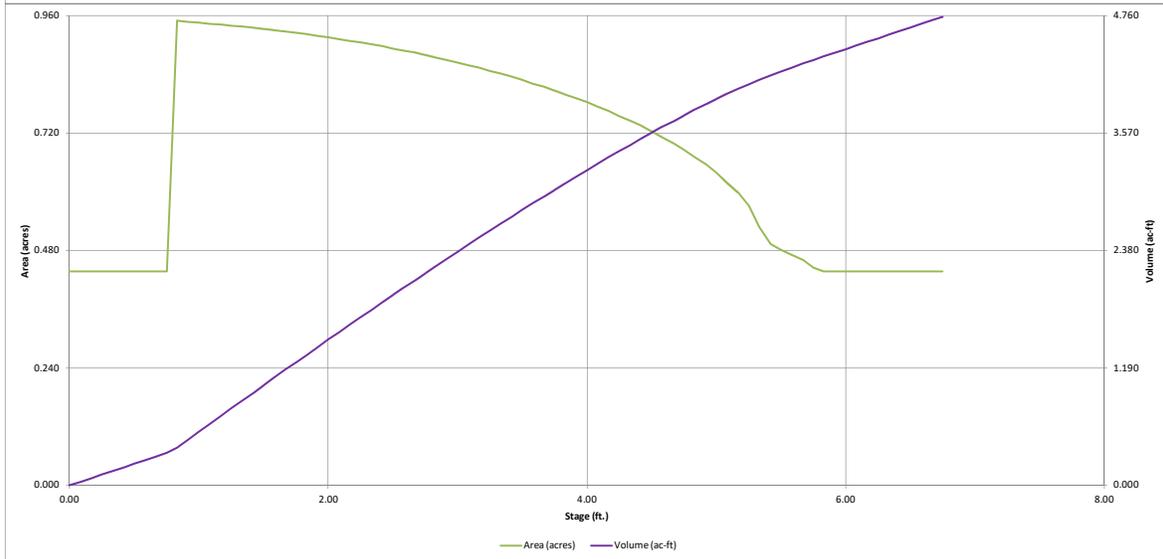
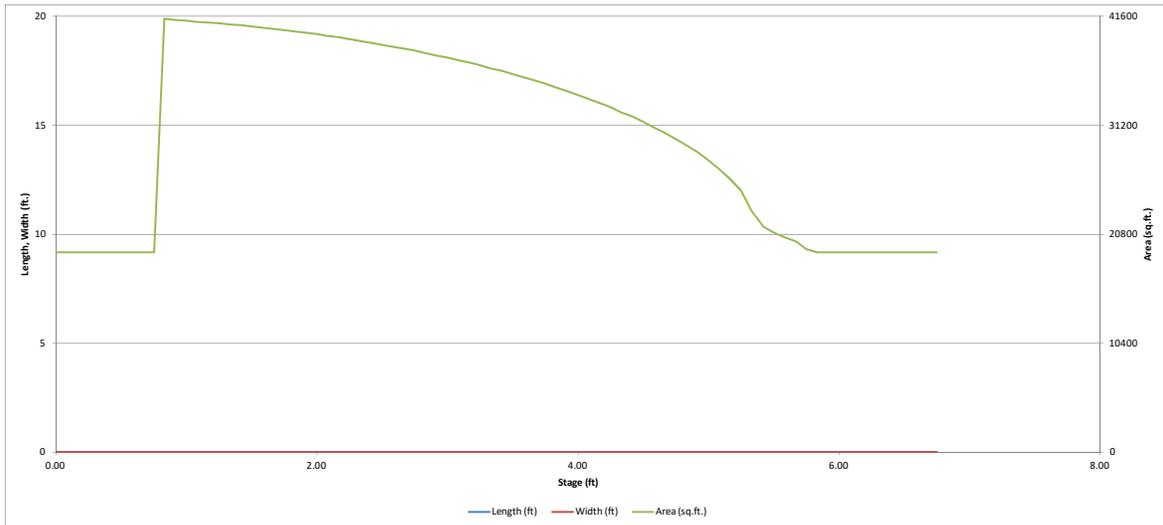
Define Zones and Basin Geometry

| | | |
|---|-------|-----------------|
| Zone 1 Volume (WQCV) = | 0.687 | acre-feet |
| Zone 2 Volume (EURV - Zone 1) = | 2.605 | acre-feet |
| Zone 3 Volume (100-year - Zones 1 & 2) = | 1.430 | acre-feet |
| Total Detention Basin Volume = | 4.723 | acre-feet |
| Initial Surcharge Volume (ISV) = | N/A | ft ³ |
| Initial Surcharge Depth (ISD) = | N/A | ft |
| Total Available Detention Depth (H _{total}) = | user | ft |
| Depth of Trickle Channel (H _{TC}) = | N/A | ft |
| Slope of Trickle Channel (S _{TC}) = | N/A | ft/ft |
| Slopes of Main Basin Sides (S _{main}) = | user | H:V |
| Basin Length-to-Width Ratio (R _{LW}) = | user | |
| Initial Surcharge Area (A _{ISV}) = | user | ft ² |
| Surcharge Volume Length (L _{ISV}) = | user | ft |
| Surcharge Volume Width (W _{ISV}) = | user | ft |
| Depth of Basin Floor (H _{FLOOR}) = | user | ft |
| Length of Basin Floor (L _{FLOOR}) = | user | ft |
| Width of Basin Floor (W _{FLOOR}) = | user | ft |
| Area of Basin Floor (A _{FLOOR}) = | user | ft ² |
| Volume of Basin Floor (V _{FLOOR}) = | user | ft ³ |
| Depth of Main Basin (H _{MAIN}) = | user | ft |
| Length of Main Basin (L _{MAIN}) = | user | ft |
| Width of Main Basin (W _{MAIN}) = | user | ft |
| Area of Main Basin (A _{MAIN}) = | user | ft ² |
| Volume of Main Basin (V _{MAIN}) = | user | ft ³ |
| Calculated Total Basin Volume (V _{total}) = | user | acre-feet |

| Depth Increment = 0.50 ft | | | | | | | | | |
|-----------------------------|------------|------------------------------|-------------|------------|-------------------------|---|-------------|---------------------------|----------------|
| Stage - Storage Description | Stage (ft) | Optional Override Stage (ft) | Length (ft) | Width (ft) | Area (ft ²) | Optional Override Area (ft ²) | Area (acre) | Volume (ft ³) | Volume (ac-ft) |
| Media Surface | -- | 0.00 | -- | -- | -- | 19,068 | 0.438 | | |
| | -- | 0.08 | -- | -- | -- | 19,068 | 0.438 | 1,525 | 0.035 |
| | -- | 0.17 | -- | -- | -- | 19,068 | 0.438 | 3,242 | 0.074 |
| | -- | | -- | -- | -- | 19,068 | 0.438 | 4,767 | 0.109 |
| | -- | | -- | -- | -- | 19,068 | 0.438 | 6,292 | 0.144 |
| | -- | | -- | -- | -- | 19,068 | 0.438 | 8,009 | 0.184 |
| | -- | | -- | -- | -- | 19,068 | 0.438 | 9,534 | 0.219 |
| | -- | | -- | -- | -- | 19,068 | 0.438 | 11,059 | 0.254 |
| | -- | | -- | -- | -- | 19,068 | 0.438 | 12,776 | 0.293 |
| | -- | | -- | -- | -- | 19,068 | 0.438 | 14,301 | 0.328 |
| | -- | | -- | -- | -- | 41,371 | 0.950 | 16,718 | 0.384 |
| | -- | | -- | -- | -- | 41,255 | 0.947 | 20,436 | 0.469 |
| | -- | | -- | -- | -- | 41,182 | 0.945 | 23,733 | 0.545 |
| | -- | 1.00 | -- | -- | -- | 41,104 | 0.944 | 27,025 | 0.620 |
| | -- | 1.08 | -- | -- | -- | 41,104 | 0.944 | 27,025 | 0.620 |
| | -- | 1.17 | -- | -- | -- | 41,017 | 0.942 | 30,720 | 0.705 |
| | -- | 1.25 | -- | -- | -- | 40,930 | 0.940 | 33,998 | 0.780 |
| | -- | 1.33 | -- | -- | -- | 40,842 | 0.938 | 37,269 | 0.856 |
| | -- | 1.42 | -- | -- | -- | 40,744 | 0.935 | 40,940 | 0.940 |
| | -- | 1.50 | -- | -- | -- | 40,640 | 0.933 | 44,196 | 1.015 |
| | -- | 1.58 | -- | -- | -- | 40,531 | 0.930 | 47,443 | 1.089 |
| | -- | 1.67 | -- | -- | -- | 40,415 | 0.928 | 51,085 | 1.173 |
| | -- | 1.75 | -- | -- | -- | 40,293 | 0.925 | 54,314 | 1.247 |
| | -- | 1.83 | -- | -- | -- | 40,166 | 0.922 | 57,532 | 1.321 |
| | -- | 1.92 | -- | -- | -- | 40,026 | 0.919 | 61,141 | 1.404 |
| | -- | 2.00 | -- | -- | -- | 39,892 | 0.916 | 64,337 | 1.477 |
| | -- | 2.08 | -- | -- | -- | 39,744 | 0.912 | 67,523 | 1.550 |
| | -- | 2.17 | -- | -- | -- | 39,591 | 0.909 | 71,093 | 1.632 |
| | -- | 2.25 | -- | -- | -- | 39,430 | 0.905 | 74,254 | 1.705 |
| | -- | 2.33 | -- | -- | -- | 39,260 | 0.901 | 77,401 | 1.777 |
| | -- | 2.42 | -- | -- | -- | 39,089 | 0.897 | 80,927 | 1.858 |
| | -- | 2.50 | -- | -- | -- | 38,907 | 0.893 | 84,047 | 1.929 |
| | -- | 2.58 | -- | -- | -- | 38,718 | 0.889 | 87,152 | 2.001 |
| | -- | 2.67 | -- | -- | -- | 38,517 | 0.884 | 90,227 | 2.081 |
| | -- | 2.75 | -- | -- | -- | 38,315 | 0.880 | 93,701 | 2.151 |
| | -- | 2.83 | -- | -- | -- | 38,103 | 0.875 | 96,757 | 2.221 |
| | -- | 2.92 | -- | -- | -- | 37,882 | 0.870 | 100,177 | 2.300 |
| | -- | 3.00 | -- | -- | -- | 37,653 | 0.864 | 103,198 | 2.369 |
| | -- | 3.08 | -- | -- | -- | 37,416 | 0.859 | 106,201 | 2.438 |
| | -- | 3.17 | -- | -- | -- | 37,169 | 0.853 | 109,557 | 2.515 |
| | -- | 3.25 | -- | -- | -- | 36,912 | 0.847 | 112,520 | 2.583 |
| | -- | 3.33 | -- | -- | -- | 36,646 | 0.841 | 115,463 | 2.651 |
| | -- | 3.42 | -- | -- | -- | 36,369 | 0.835 | 118,748 | 2.726 |
| | -- | 3.50 | -- | -- | -- | 36,081 | 0.828 | 121,646 | 2.793 |
| | -- | 3.58 | -- | -- | -- | 35,779 | 0.821 | 124,521 | 2.859 |
| | -- | 3.67 | -- | -- | -- | 35,472 | 0.814 | 127,727 | 2.932 |
| | -- | 3.75 | -- | -- | -- | 35,150 | 0.807 | 130,552 | 2.997 |
| | -- | 3.83 | -- | -- | -- | 34,814 | 0.799 | 133,350 | 3.061 |
| | -- | 3.92 | -- | -- | -- | 34,466 | 0.791 | 136,468 | 3.133 |
| | -- | 4.00 | -- | -- | -- | 34,101 | 0.783 | 139,211 | 3.196 |
| | -- | 4.08 | -- | -- | -- | 33,720 | 0.774 | 141,924 | 3.258 |
| | -- | 4.17 | -- | -- | -- | 33,322 | 0.765 | 144,940 | 3.327 |
| | -- | 4.25 | -- | -- | -- | 32,904 | 0.755 | 147,590 | 3.388 |
| | -- | 4.33 | -- | -- | -- | 32,466 | 0.745 | 150,204 | 3.448 |
| | -- | 4.42 | -- | -- | -- | 32,005 | 0.735 | 153,106 | 3.515 |
| | -- | 4.50 | -- | -- | -- | 31,522 | 0.724 | 155,647 | 3.573 |
| | -- | 4.58 | -- | -- | -- | 31,010 | 0.712 | 158,148 | 3.631 |
| | -- | 4.67 | -- | -- | -- | 30,463 | 0.699 | 160,914 | 3.694 |
| | -- | 4.75 | -- | -- | -- | 29,887 | 0.686 | 163,328 | 3.749 |
| | -- | 4.83 | -- | -- | -- | 29,259 | 0.672 | 165,694 | 3.804 |
| | -- | 4.92 | -- | -- | -- | 28,588 | 0.656 | 168,297 | 3.864 |
| | -- | 5.00 | -- | -- | -- | 27,845 | 0.639 | 170,554 | 3.915 |
| | -- | 5.08 | -- | -- | -- | 27,013 | 0.620 | 172,749 | 3.966 |
| | -- | 5.17 | -- | -- | -- | 26,058 | 0.598 | 175,137 | 4.021 |
| | -- | 5.25 | -- | -- | -- | 24,880 | 0.571 | 177,175 | 4.067 |
| | -- | 5.33 | -- | -- | -- | 23,005 | 0.528 | 179,090 | 4.111 |
| | -- | 5.42 | -- | -- | -- | 21,507 | 0.494 | 181,093 | 4.157 |
| | -- | 5.50 | -- | -- | -- | 20,969 | 0.481 | 182,792 | 4.196 |
| | -- | 5.58 | -- | -- | -- | 20,533 | 0.471 | 184,452 | 4.234 |
| | -- | 5.67 | -- | -- | -- | 20,080 | 0.461 | 186,280 | 4.276 |
| | -- | 5.75 | -- | -- | -- | 19,385 | 0.445 | 187,858 | 4.313 |
| | -- | 5.83 | -- | -- | -- | 19,068 | 0.438 | 189,396 | 4.348 |
| | -- | 5.92 | -- | -- | -- | 19,068 | 0.438 | 191,113 | 4.387 |
| | -- | 6.00 | -- | -- | -- | 19,068 | 0.438 | 192,638 | 4.422 |
| | -- | 6.08 | -- | -- | -- | 19,068 | 0.438 | 194,163 | 4.457 |
| | -- | 6.17 | -- | -- | -- | 19,068 | 0.438 | 195,880 | 4.497 |
| | -- | 6.25 | -- | -- | -- | 19,068 | 0.438 | 197,405 | 4.532 |
| | -- | 6.33 | -- | -- | -- | 19,068 | 0.438 | 198,930 | 4.567 |
| | -- | 6.42 | -- | -- | -- | 19,068 | 0.438 | 200,647 | 4.606 |
| | -- | 6.50 | -- | -- | -- | 19,068 | 0.438 | 202,172 | 4.641 |
| | -- | 6.58 | -- | -- | -- | 19,068 | 0.438 | 203,697 | 4.676 |
| | -- | 6.67 | -- | -- | -- | 19,068 | 0.438 | 205,414 | 4.716 |
| | -- | 6.75 | -- | -- | -- | 19,068 | 0.438 | 206,939 | 4.751 |

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

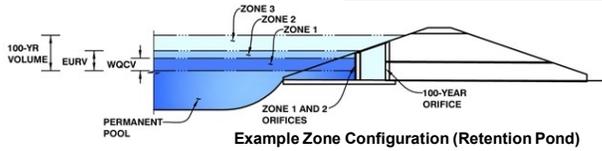


Internal note: review this with next submittal once outlet structure details are included in CDs

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD- Detention, Version 4.06 (July 2022)

Project: CROSSROADS MIXED USE
Basin ID: POND 1 UNDERGROUND DETENTION



| | Estimated Stage (ft) | Estimated Volume (ac-ft) | Outlet Type |
|--------------------------|----------------------|--------------------------|----------------------|
| Zone 1 (WQCV) | 1.16 | 0.687 | Filtration Media |
| Zone 2 (EURV) | 4.13 | 2.605 | Circular Orifice |
| Zone 3 (100-year) | 6.69 | 1.430 | Weir&Pipe (Restrict) |
| Total (all zones) | | 4.723 | |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = 0.75 ft (distance below the filtration media surface)
 Underdrain Orifice Diameter = 4.93 inches

Calculated Parameters for Underdrain
 Underdrain Orifice Area = 0.1 ft²
 Underdrain Orifice Centroid = 0.21 feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)
 Depth at top of Zone using Orifice Plate = N/A ft (relative to basin bottom at Stage = 0 ft)
 Orifice Plate: Orifice Vertical Spacing = N/A inches
 Orifice Plate: Orifice Area per Row = N/A sq. inches

Calculated Parameters for Plate
 WQ Orifice Area per Row = N/A ft²
 Elliptical Half-Width = N/A feet
 Elliptical Slot Centroid = N/A feet
 Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (optional) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|--------------------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|
| Stage of Orifice Centroid (ft) | N/A |
| Orifice Area (sq. inches) | N/A |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|--------------------------------|------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Stage of Orifice Centroid (ft) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Orifice Area (sq. inches) | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |

User Input: Vertical Orifice (Circular or Rectangular)

| | Zone 2 Circular | Not Selected | |
|---|-----------------|--------------|---|
| Invert of Vertical Orifice = | 1.17 | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Depth at top of Zone using Vertical Orifice = | 4.13 | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Vertical Orifice Diameter = | 0.38 | N/A | inches |

Calculated Parameters for Vertical Orif
 Vertical Orifice Area = 0.00 ft²
 Vertical Orifice Centroid = 0.02 feet

| Zone 2 Circular | Not Selected |
|-----------------|--------------|
| N/A | N/A |
| 0.02 | N/A |

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

| | Zone 3 Weir | Not Selected | |
|---------------------------------------|-------------|--------------|---|
| Overflow Weir Front Edge Height, Ho = | 4.14 | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Overflow Weir Front Edge Length = | 5.70 | N/A | feet |
| Overflow Weir Gate Slope = | 0.00 | N/A | H:V |
| Horiz. Length of Weir Sides = | 2.91 | N/A | feet |
| Overflow Gate Type = | Type C Gate | N/A | |
| Debris Clogging % = | 50% | N/A | % |

Calculated Parameters for Overflow W
 Height of Gate Upper Edge, H_t = 4.14 feet
 Overflow Weir Slope Length = 2.91 feet
 Grate Open Area / 100-yr Orifice Area = 9.68
 Overflow Gate Open Area w/o Debris = 11.54
 Overflow Gate Open Area w/ Debris = 5.77

| Zone 3 Weir | Not Selected |
|-------------|--------------|
| 4.14 | N/A |
| 2.91 | N/A |
| 9.68 | N/A |
| 11.54 | N/A |
| 5.77 | N/A |

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

| | Zone 3 Restrictor | Not Selected | |
|---|-------------------|--------------|--|
| Depth to Invert of Outlet Pipe = | 1.00 | N/A | ft (distance below basin bottom at Stage = 0 ft) |
| Outlet Pipe Diameter = | 18.00 | N/A | inches |
| Restrictor Plate Height Above Pipe Invert = | 11.50 | N/A | inches |

Calculated Parameters for Outlet Pipe w/ Flow Restriction Pl
 Outlet Orifice Area = 1.19 ft²
 Outlet Orifice Centroid = 0.54 feet
 Half-Central Angle of Restrictor Plate on Pipe = 1.85

| Zone 3 Restrictor | Not Selected |
|-------------------|--------------|
| 1.19 | N/A |
| 0.54 | N/A |
| 1.85 | N/A |

User Input: Emergency Spillway (Rectangular or Trapezoidal)

| Spillway Invert Stage = | 6.16 | ft (relative to basin bottom at Stage = 0 ft) |
|-------------------------------------|-------|---|
| Spillway Crest Length = | 33.00 | feet |
| Spillway End Slopes = | 4.00 | H:V |
| Freeboard above Max Water Surface = | 1.00 | feet |

Calculated Parameters for Spillway
 Spillway Design Flow Depth = 0.84 feet
 Stage at Top of Freeboard = 8.00 feet
 Basin Area at Top of Freeboard = 0.44 acres
 Basin Volume at Top of Freeboard = 4.75 acre-ft

| Spillway Design Flow Depth = | 0.84 feet |
|------------------------------------|--------------|
| Stage at Top of Freeboard = | 8.00 feet |
| Basin Area at Top of Freeboard = | 0.44 acres |
| Basin Volume at Top of Freeboard = | 4.75 acre-ft |

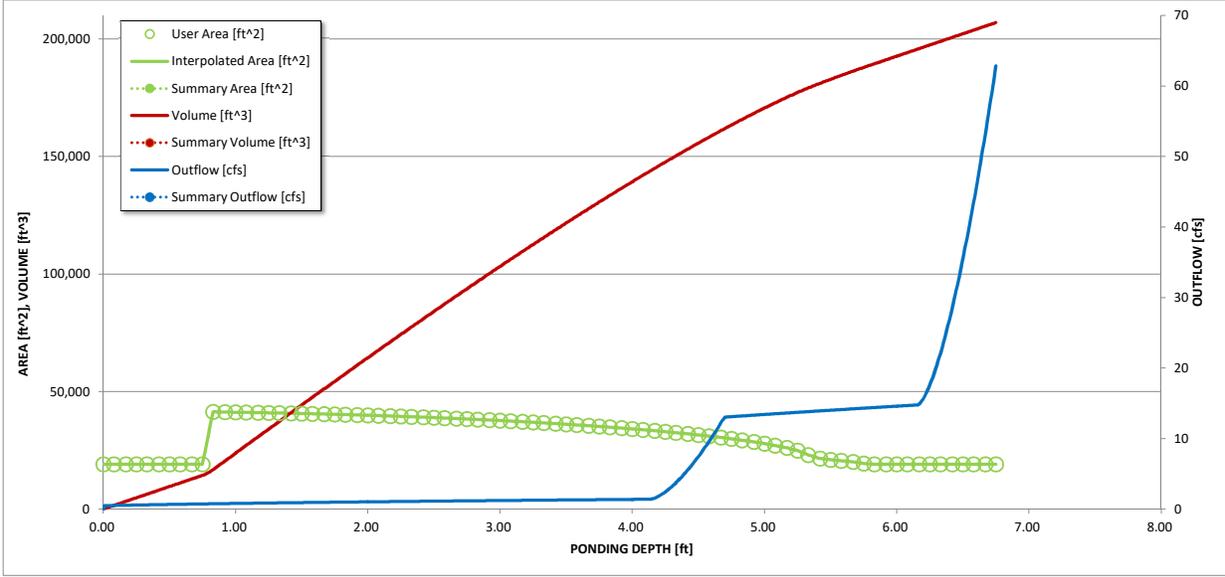
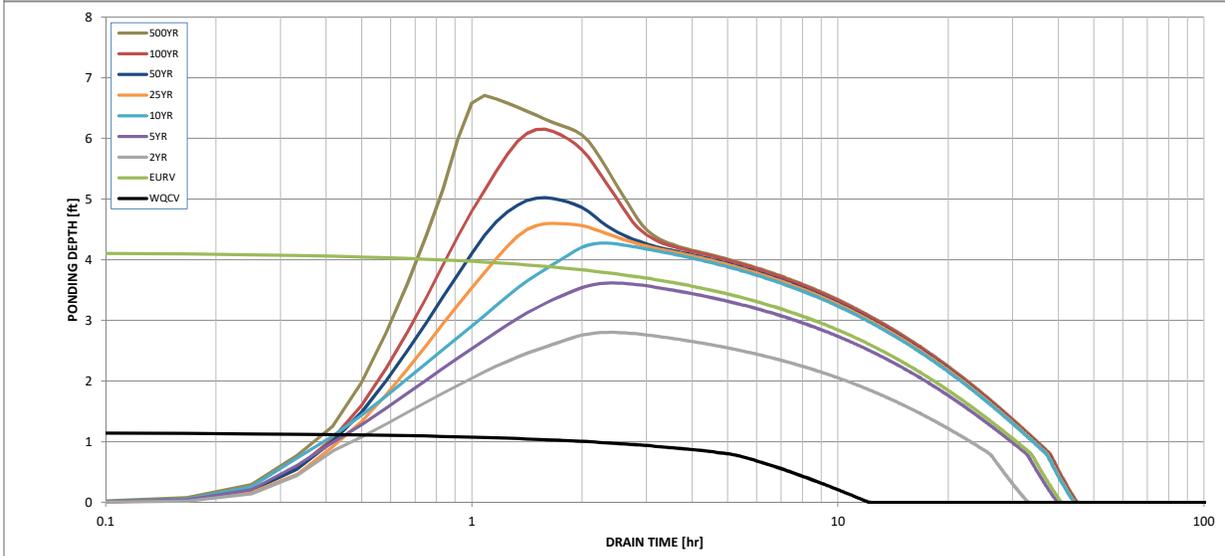
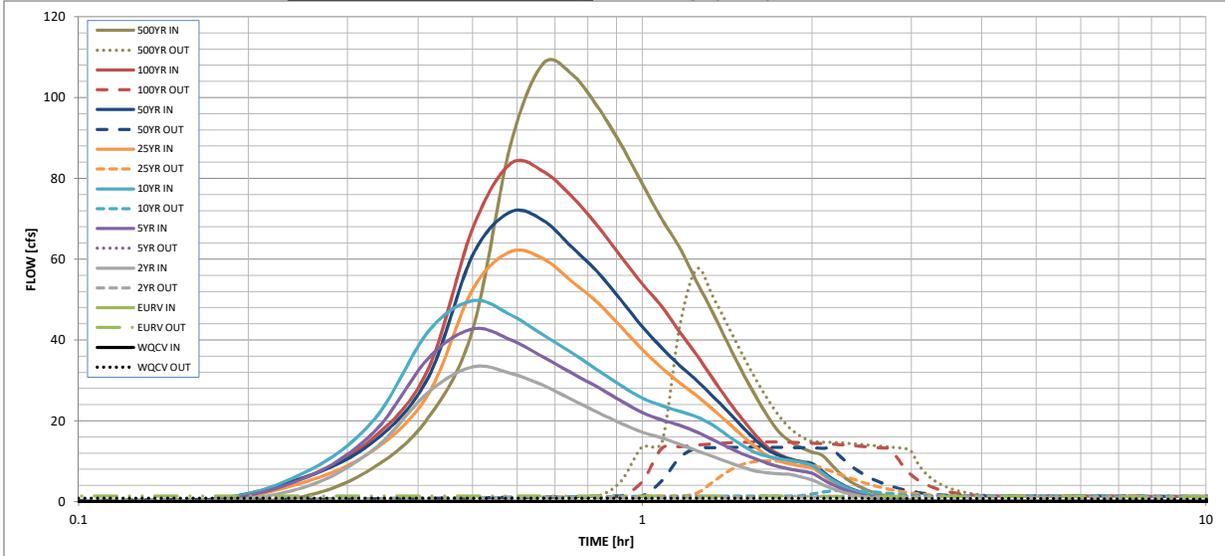
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AI)

| | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year |
|---|------------------|--------------------|--------------------|--------------------|-----------------|-----------------|----------------|----------------|
| Design Storm Return Period = | N/A | N/A | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 |
| One-Hour Rainfall Depth (in) = | 0.687 | 3.293 | 2.407 | 3.122 | 3.696 | 4.394 | 5.058 | 5.833 |
| CUHP Runoff Volume (acre-ft) = | N/A | N/A | 2.407 | 3.122 | 3.696 | 4.394 | 5.058 | 5.833 |
| Inflow Hydrograph Volume (acre-ft) = | N/A | N/A | 0.2 | 0.3 | 0.5 | 5.1 | 9.2 | 14.8 |
| CUHP Predevelopment Peak Q (cfs) = | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| OPTIONAL Override Predevelopment Peak Q (cfs) = | N/A | N/A | 0.01 | 0.01 | 0.01 | 0.16 | 0.29 | 0.46 |
| Predevelopment Unit Peak Flow, q (cfs/acre) = | N/A | N/A | 33.3 | 42.7 | 49.7 | 61.8 | 71.7 | 83.5 |
| Peak Inflow Q (cfs) = | 0.8 | 1.4 | 1.2 | 1.3 | 2.8 | 10.1 | 13.4 | 14.8 |
| Peak Outflow Q (cfs) = | N/A | N/A | N/A | 3.9 | 5.9 | 2.0 | 1.5 | 1.0 |
| Ratio Peak Outflow to Predevelopment Q = | N/A | N/A | N/A | 3.9 | 5.9 | 2.0 | 1.5 | 1.0 |
| Structure Controlling Flow = | Filtration Media | Vertical Orifice 1 | Vertical Orifice 1 | Vertical Orifice 1 | Overflow Weir 1 | Overflow Weir 1 | Outlet Plate 1 | Outlet Plate 1 |
| Max Velocity through Gate 1 (fps) = | N/A | N/A | N/A | N/A | 0.1 | 0.7 | 1.0 | 1.1 |
| Max Velocity through Gate 2 (fps) = | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Time to Drain 97% of Inflow Volume (hours) = | 12 | 39 | 31 | 38 | 42 | 41 | 41 | 41 |
| Time to Drain 99% of Inflow Volume (hours) = | 12 | 40 | 33 | 39 | 43 | 43 | 43 | 44 |
| Maximum Ponding Depth (ft) = | 1.16 | 4.13 | 2.80 | 3.62 | 4.27 | 4.60 | 5.02 | 6.15 |
| Area at Maximum Ponding Depth (acres) = | 0.94 | 0.77 | 0.88 | 0.82 | 0.75 | 0.71 | 0.63 | 0.44 |
| Maximum Volume Stored (acre-ft) = | 0.696 | 3.297 | 2.195 | 2.883 | 3.403 | 3.638 | 3.928 | 4.484 |

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)



| S-A-V-D Chart Axis Override | X-axis | Left Y-Axis | Right Y-Axis |
|-----------------------------|--------|-------------|--------------|
| minimum bound | | | |
| maximum bound | | | |

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

| Time Interval | SOURCE | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP |
|---------------|---------|------------|------------|--------------|--------------|---------------|---------------|---------------|----------------|----------------|
| | TIME | WQCV [cfs] | EURV [cfs] | 2 Year [cfs] | 5 Year [cfs] | 10 Year [cfs] | 25 Year [cfs] | 50 Year [cfs] | 100 Year [cfs] | 500 Year [cfs] |
| 5.00 min | 0:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 0:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 0:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.39 | 0.04 | 1.25 |
| | 0:15:00 | 0.00 | 0.00 | 3.46 | 5.62 | 6.96 | 4.67 | 5.89 | 5.69 | 8.36 |
| | 0:20:00 | 0.00 | 0.00 | 12.83 | 16.99 | 20.01 | 12.66 | 14.80 | 15.76 | 20.62 |
| | 0:25:00 | 0.00 | 0.00 | 26.94 | 35.34 | 41.95 | 26.44 | 30.57 | 32.65 | 42.44 |
| | 0:30:00 | 0.00 | 0.00 | 33.30 | 42.69 | 49.70 | 52.46 | 60.93 | 67.51 | 88.01 |
| | 0:35:00 | 0.00 | 0.00 | 31.75 | 40.04 | 46.24 | 61.83 | 71.65 | 83.54 | 108.32 |
| | 0:40:00 | 0.00 | 0.00 | 28.76 | 35.78 | 41.26 | 60.15 | 69.62 | 81.73 | 105.76 |
| | 0:45:00 | 0.00 | 0.00 | 25.24 | 31.76 | 36.81 | 54.48 | 62.96 | 75.48 | 97.70 |
| | 0:50:00 | 0.00 | 0.00 | 22.08 | 28.32 | 32.53 | 49.25 | 56.81 | 68.18 | 88.41 |
| | 0:55:00 | 0.00 | 0.00 | 19.37 | 24.91 | 28.73 | 43.27 | 49.85 | 60.62 | 78.63 |
| | 1:00:00 | 0.00 | 0.00 | 17.17 | 22.01 | 25.61 | 37.64 | 43.31 | 53.86 | 69.88 |
| | 1:05:00 | 0.00 | 0.00 | 15.76 | 20.18 | 23.73 | 33.08 | 38.03 | 48.24 | 62.68 |
| | 1:10:00 | 0.00 | 0.00 | 14.15 | 18.82 | 22.31 | 29.32 | 33.64 | 41.85 | 54.29 |
| | 1:15:00 | 0.00 | 0.00 | 12.58 | 17.18 | 20.93 | 26.15 | 29.93 | 36.21 | 46.84 |
| | 1:20:00 | 0.00 | 0.00 | 11.18 | 15.31 | 18.94 | 22.83 | 26.07 | 30.49 | 39.31 |
| | 1:25:00 | 0.00 | 0.00 | 9.82 | 13.47 | 16.35 | 19.64 | 22.38 | 25.23 | 32.44 |
| | 1:30:00 | 0.00 | 0.00 | 8.59 | 11.88 | 14.02 | 16.49 | 18.74 | 20.67 | 26.49 |
| | 1:35:00 | 0.00 | 0.00 | 7.63 | 10.64 | 12.22 | 13.63 | 15.45 | 16.69 | 21.29 |
| | 1:40:00 | 0.00 | 0.00 | 7.11 | 9.47 | 11.22 | 11.37 | 12.85 | 13.51 | 17.17 |
| | 1:45:00 | 0.00 | 0.00 | 6.88 | 8.60 | 10.60 | 10.09 | 11.39 | 11.68 | 14.80 |
| | 1:50:00 | 0.00 | 0.00 | 6.73 | 7.98 | 10.16 | 9.28 | 10.46 | 10.53 | 13.28 |
| | 1:55:00 | 0.00 | 0.00 | 6.06 | 7.51 | 9.68 | 8.74 | 9.85 | 9.74 | 12.24 |
| | 2:00:00 | 0.00 | 0.00 | 5.38 | 7.00 | 8.94 | 8.35 | 9.41 | 9.17 | 11.49 |
| | 2:05:00 | 0.00 | 0.00 | 4.29 | 5.61 | 7.16 | 6.74 | 7.58 | 7.28 | 9.11 |
| | 2:10:00 | 0.00 | 0.00 | 3.29 | 4.28 | 5.47 | 5.12 | 5.76 | 5.45 | 6.80 |
| | 2:15:00 | 0.00 | 0.00 | 2.52 | 3.28 | 4.17 | 3.90 | 4.38 | 4.10 | 5.11 |
| | 2:20:00 | 0.00 | 0.00 | 1.92 | 2.49 | 3.15 | 2.95 | 3.32 | 3.11 | 3.88 |
| | 2:25:00 | 0.00 | 0.00 | 1.45 | 1.87 | 2.35 | 2.21 | 2.49 | 2.35 | 2.92 |
| | 2:30:00 | 0.00 | 0.00 | 1.08 | 1.37 | 1.74 | 1.63 | 1.84 | 1.75 | 2.17 |
| | 2:35:00 | 0.00 | 0.00 | 0.79 | 0.99 | 1.28 | 1.20 | 1.34 | 1.29 | 1.60 |
| | 2:40:00 | 0.00 | 0.00 | 0.57 | 0.72 | 0.95 | 0.90 | 1.01 | 0.96 | 1.20 |
| | 2:45:00 | 0.00 | 0.00 | 0.39 | 0.50 | 0.66 | 0.64 | 0.72 | 0.69 | 0.85 |
| | 2:50:00 | 0.00 | 0.00 | 0.24 | 0.33 | 0.43 | 0.43 | 0.48 | 0.46 | 0.57 |
| | 2:55:00 | 0.00 | 0.00 | 0.13 | 0.20 | 0.25 | 0.26 | 0.29 | 0.27 | 0.34 |
| | 3:00:00 | 0.00 | 0.00 | 0.06 | 0.10 | 0.12 | 0.13 | 0.14 | 0.14 | 0.17 |
| | 3:05:00 | 0.00 | 0.00 | 0.02 | 0.03 | 0.04 | 0.04 | 0.05 | 0.04 | 0.05 |
| | 3:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 5:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 6:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |

Project: Crossroads Mixed Use

Chamber Model -
 Units -
 Number of Chambers -
 Number of End Caps -
 Voids in the stone (porosity) -
 Base of Stone Elevation -
 Amount of Stone Above Chambers -
 Amount of Stone Below Chambers -

| |
|----------|
| MC-7200 |
| Imperial |
| 731 |
| 44 |
| 40 |
| 6287.47 |
| 12 |
| 9 |



Include Perimeter Stone in Calculations

Click for Stage Area Data

[Click Here for Metric](#)

Area of system - 47670 sf Min. Area - 45260 sf min. area

StormTech MC-7200 Cumulative Storage Volumes

| Height of System (inches) | Incremental Single Chamber (cubic feet) | Incremental Single End Cap (cubic feet) | Incremental Chambers (cubic feet) | Incremental End Cap (cubic feet) | Incremental Stone (cubic feet) | Incremental Ch. EC and Stone (cubic feet) | Cumulative System (cubic feet) | Elevation (feet) |
|---------------------------|---|---|-----------------------------------|----------------------------------|--------------------------------|---|--------------------------------|------------------|
| 81 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 206903.58 | 6294.22 |
| 80 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 205314.58 | 6294.14 |
| 79 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 203725.58 | 6294.05 |
| 78 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 202136.58 | 6293.97 |
| 77 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 200547.58 | 6293.89 |
| 76 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 198958.58 | 6293.80 |
| 75 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 197369.58 | 6293.72 |
| 74 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 195780.58 | 6293.64 |
| 100-YR | | | | | | | | |
| 73 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 194191.58 | 6293.55 |
| 72 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 192602.58 | 6293.47 |
| 71 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 191013.58 | 6293.39 |
| 70 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 189424.58 | 6293.30 |
| 69 | 0.06 | 0.01 | 43.40 | 0.57 | 1571.41 | 1615.39 | 187835.58 | 6293.22 |
| 68 | 0.19 | 0.03 | 139.02 | 1.49 | 1532.79 | 1673.31 | 186246.58 | 6293.14 |
| 67 | 0.28 | 0.05 | 201.16 | 2.28 | 1507.63 | 1711.06 | 184657.58 | 6293.05 |
| 66 | 0.36 | 0.07 | 261.15 | 2.90 | 1483.38 | 1747.43 | 183068.58 | 6292.97 |
| 65 | 0.46 | 0.08 | 335.09 | 3.66 | 1453.50 | 1792.25 | 181479.58 | 6292.89 |
| 64 | 0.74 | 0.11 | 542.14 | 4.63 | 1370.29 | 1917.06 | 179890.58 | 6292.80 |
| 63 | 1.10 | 0.13 | 801.45 | 5.83 | 1266.09 | 2073.36 | 178301.58 | 6292.72 |
| 62 | 1.32 | 0.16 | 963.81 | 7.08 | 1200.64 | 2171.54 | 176712.58 | 6292.64 |
| 61 | 1.50 | 0.19 | 1095.21 | 8.30 | 1147.60 | 2251.11 | 175123.58 | 6292.55 |
| 60 | 1.65 | 0.22 | 1209.43 | 9.62 | 1101.38 | 2320.43 | 173534.58 | 6292.47 |
| 59 | 1.79 | 0.25 | 1311.39 | 10.87 | 1060.10 | 2382.35 | 171945.58 | 6292.39 |
| 58 | 1.92 | 0.28 | 1403.27 | 12.11 | 1022.85 | 2438.23 | 170356.58 | 6292.30 |
| 57 | 2.04 | 0.30 | 1489.37 | 13.28 | 987.94 | 2490.59 | 168767.58 | 6292.22 |
| 56 | 2.15 | 0.33 | 1568.23 | 14.41 | 955.94 | 2538.59 | 167178.58 | 6292.14 |
| 55 | 2.25 | 0.35 | 1643.07 | 15.60 | 925.53 | 2584.20 | 165589.58 | 6292.05 |
| 54 | 2.34 | 0.38 | 1712.84 | 16.88 | 897.11 | 2626.83 | 164000.58 | 6291.97 |
| 53 | 2.43 | 0.41 | 1778.85 | 18.00 | 870.26 | 2667.11 | 162411.58 | 6291.89 |
| 52 | 2.52 | 0.44 | 1841.43 | 19.40 | 844.67 | 2705.50 | 160822.58 | 6291.80 |
| 51 | 2.60 | 0.47 | 1901.02 | 20.63 | 820.34 | 2741.99 | 159233.58 | 6291.72 |
| 50 | 2.68 | 0.50 | 1957.92 | 21.79 | 797.12 | 2776.83 | 157644.58 | 6291.64 |
| EURV | | | | | | | | |
| 49 | 2.75 | 0.52 | 2012.13 | 22.91 | 774.98 | 2810.02 | 146055.58 | 6291.55 |
| 48 | 2.82 | 0.54 | 2063.93 | 23.96 | 753.84 | 2841.73 | 139800.42 | 6291.47 |
| 47 | 2.89 | 0.57 | 2113.62 | 24.94 | 733.58 | 2872.14 | 136958.68 | 6291.39 |
| 46 | 2.96 | 0.59 | 2161.10 | 25.89 | 714.20 | 2901.20 | 134086.55 | 6291.30 |
| 45 | 3.02 | 0.61 | 2206.72 | 26.84 | 695.57 | 2929.14 | 131185.35 | 6291.22 |
| 44 | 3.08 | 0.63 | 2250.48 | 27.81 | 677.68 | 2955.98 | 128256.21 | 6291.14 |
| 5-YR | | | | | | | | |
| 43 | 3.14 | 0.64 | 2292.62 | 28.30 | 660.63 | 2981.55 | 125300.24 | 6291.05 |
| 42 | 3.19 | 0.68 | 2333.18 | 29.80 | 643.81 | 3006.79 | 122318.69 | 6290.97 |
| 41 | 3.25 | 0.70 | 2372.11 | 30.79 | 627.84 | 3030.74 | 119311.89 | 6290.89 |
| 40 | 3.30 | 0.72 | 2409.61 | 31.79 | 612.44 | 3053.84 | 116281.15 | 6290.80 |
| 39 | 3.35 | 0.74 | 2445.65 | 32.72 | 597.65 | 3076.02 | 113227.32 | 6290.72 |
| 38 | 3.39 | 0.76 | 2480.36 | 33.64 | 583.40 | 3097.40 | 110151.30 | 6290.64 |
| 37 | 3.44 | 0.79 | 2513.80 | 34.56 | 569.65 | 3118.02 | 107053.90 | 6290.55 |
| 36 | 3.48 | 0.80 | 2545.99 | 35.32 | 556.48 | 3137.78 | 103935.88 | 6290.47 |
| 35 | 3.53 | 0.82 | 2577.04 | 36.08 | 543.75 | 3156.87 | 100798.10 | 6290.39 |
| 34 | 3.57 | 0.84 | 2606.87 | 36.89 | 531.49 | 3175.26 | 97641.23 | 6290.30 |
| 33 | 3.61 | 0.85 | 2635.68 | 37.46 | 519.75 | 3192.88 | 94465.97 | 6290.22 |
| 32 | 3.64 | 0.86 | 2663.38 | 37.82 | 508.52 | 3209.72 | 91273.09 | 6290.14 |
| 31 | 3.68 | 0.89 | 2690.04 | 39.14 | 497.33 | 3226.50 | 88063.36 | 6290.05 |
| 30 | 3.71 | 0.90 | 2715.66 | 39.79 | 486.82 | 3242.27 | 84836.86 | 6289.97 |
| 29 | 3.75 | 0.92 | 2740.29 | 40.36 | 476.74 | 3257.39 | 81594.59 | 6289.89 |
| 28 | 3.78 | 0.92 | 2763.91 | 40.47 | 467.25 | 3271.63 | 78337.20 | 6289.80 |
| 27 | 3.81 | 0.94 | 2786.61 | 41.51 | 457.75 | 3285.87 | 75065.57 | 6289.72 |
| 26 | 3.84 | 0.96 | 2808.28 | 42.08 | 448.86 | 3299.21 | 71779.70 | 6289.64 |
| 25 | 3.87 | 0.97 | 2829.09 | 42.62 | 440.32 | 3312.03 | 68480.49 | 6289.55 |
| 24 | 3.90 | 0.98 | 2849.00 | 43.18 | 432.13 | 3324.31 | 65168.46 | 6289.47 |
| 23 | 3.92 | 0.97 | 2868.04 | 42.73 | 424.69 | 3335.46 | 61844.15 | 6289.39 |
| 22 | 3.95 | 1.00 | 2886.19 | 44.14 | 416.87 | 3347.20 | 58508.69 | 6289.30 |
| 21 | 3.97 | 1.01 | 2903.49 | 44.49 | 409.81 | 3357.79 | 55161.49 | 6289.22 |
| 20 | 3.99 | 1.02 | 2920.00 | 44.90 | 403.04 | 3367.94 | 51803.70 | 6289.14 |
| 19 | 4.02 | 1.03 | 2935.64 | 45.33 | 396.61 | 3377.59 | 48435.76 | 6289.05 |
| 18 | 4.04 | 1.04 | 2950.47 | 45.70 | 390.53 | 3386.70 | 45058.18 | 6288.97 |
| 17 | 4.06 | 1.05 | 2964.52 | 46.04 | 384.78 | 3395.33 | 41671.47 | 6288.89 |
| 16 | 4.07 | 1.05 | 2977.77 | 46.38 | 379.34 | 3403.49 | 38276.14 | 6288.80 |
| 15 | 4.09 | 1.05 | 2990.16 | 46.22 | 374.45 | 3410.83 | 34872.65 | 6288.72 |
| 14 | 4.11 | 1.06 | 3001.99 | 46.49 | 369.61 | 3418.09 | 31461.83 | 6288.64 |
| WQCV | | | | | | | | |
| 13 | 4.12 | 1.08 | 3013.25 | 47.32 | 364.77 | 3425.34 | 28043.73 | 6288.55 |
| 12 | 4.14 | 1.08 | 3023.81 | 47.64 | 360.42 | 3431.87 | 24618.39 | 6288.47 |
| 11 | 4.15 | 1.09 | 3033.63 | 47.88 | 356.40 | 3437.90 | 21186.52 | 6288.39 |
| 10 | 4.17 | 1.11 | 3049.01 | 48.68 | 349.92 | 3447.62 | 17748.62 | 6288.30 |
| 9 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 14301.00 | 6288.22 |
| 8 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 12712.00 | 6288.14 |
| 7 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 11123.00 | 6288.05 |
| 6 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 9534.00 | 6287.97 |
| 5 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 7945.00 | 6287.89 |
| 4 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 6356.00 | 6287.80 |
| 3 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 4767.00 | 6287.72 |
| 2 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 3178.00 | 6287.64 |
| 1 | 0.00 | 0.00 | 0.00 | 0.00 | 1589.00 | 1589.00 | 1589.00 | 6287.55 |

Stage Area Data

| Depth (feet) | Elevation (feet) | Area (ft2) | Area (acres) |
|--------------|------------------|------------|--------------|
| 0.00 | 6287.47000 | 19068.00 | 0.4377 |
| 0.08 | 6287.55333 | 19068.00 | 0.4377 |
| 0.17 | 6287.63667 | 19068.00 | 0.4377 |
| 0.25 | 6287.72000 | 19068.00 | 0.4377 |
| 0.33 | 6287.80333 | 19068.00 | 0.4377 |
| 0.42 | 6287.88667 | 19068.00 | 0.4377 |
| 0.50 | 6287.97000 | 19068.00 | 0.4377 |
| 0.58 | 6288.05333 | 19068.00 | 0.4377 |
| 0.67 | 6288.13667 | 19068.00 | 0.4377 |
| 0.75 | 6288.22000 | 19068.00 | 0.4377 |
| 0.83 | 6288.30333 | 41371.38 | 0.9498 |
| 0.92 | 6288.38667 | 41254.86 | 0.9471 |
| 1.00 | 6288.47000 | 41182.44 | 0.9454 |
| 1.08 | 6288.55333 | 41104.14 | 0.9436 |
| 1.17 | 6288.63667 | 41017.10 | 0.9416 |
| 1.25 | 6288.72000 | 40929.93 | 0.9396 |
| 1.33 | 6288.80333 | 40841.83 | 0.9376 |
| 1.42 | 6288.88667 | 40744.01 | 0.9354 |
| 1.50 | 6288.97000 | 40640.42 | 0.9330 |
| 1.58 | 6289.05333 | 40531.05 | 0.9305 |
| 1.67 | 6289.13667 | 40415.23 | 0.9278 |
| 1.75 | 6289.22000 | 40293.48 | 0.9250 |
| 1.83 | 6289.30333 | 40166.39 | 0.9221 |
| 1.92 | 6289.38667 | 40025.56 | 0.9189 |
| 2.00 | 6289.47000 | 39891.71 | 0.9158 |
| 2.08 | 6289.55333 | 39744.30 | 0.9124 |
| 2.17 | 6289.63667 | 39590.57 | 0.9089 |
| 2.25 | 6289.72000 | 39430.43 | 0.9052 |
| 2.33 | 6289.80333 | 39259.55 | 0.9013 |
| 2.42 | 6289.88667 | 39088.69 | 0.8974 |
| 2.50 | 6289.97000 | 38907.23 | 0.8932 |
| 2.58 | 6290.05333 | 38718.06 | 0.8888 |
| 2.67 | 6290.13667 | 38516.65 | 0.8842 |
| 2.75 | 6290.22000 | 38314.58 | 0.8796 |
| 2.83 | 6290.30333 | 38103.12 | 0.8747 |
| 2.92 | 6290.38667 | 37882.42 | 0.8697 |
| 3.00 | 6290.47000 | 37653.39 | 0.8644 |
| 3.08 | 6290.55333 | 37416.22 | 0.8590 |
| 3.17 | 6290.63667 | 37168.80 | 0.8533 |
| 3.25 | 6290.72000 | 36912.26 | 0.8474 |
| 3.33 | 6290.80333 | 36646.04 | 0.8413 |
| 3.42 | 6290.88667 | 36368.88 | 0.8349 |
| 3.50 | 6290.97000 | 36081.49 | 0.8283 |
| 3.58 | 6291.05333 | 35778.60 | 0.8214 |
| 3.67 | 6291.13667 | 35471.74 | 0.8143 |
| 3.75 | 6291.22000 | 35149.65 | 0.8069 |
| 3.83 | 6291.30333 | 34814.35 | 0.7992 |
| 3.92 | 6291.38667 | 34465.62 | 0.7912 |
| 4.00 | 6291.47000 | 34100.80 | 0.7828 |
| 4.08 | 6291.55333 | 33720.27 | 0.7741 |
| 4.17 | 6291.63667 | 33321.92 | 0.7650 |
| 4.25 | 6291.72000 | 32903.83 | 0.7554 |
| 4.33 | 6291.80333 | 32466.00 | 0.7453 |
| 4.42 | 6291.88667 | 32005.36 | 0.7347 |
| 4.50 | 6291.97000 | 31521.99 | 0.7236 |
| 4.58 | 6292.05333 | 31010.39 | 0.7119 |
| 4.67 | 6292.13667 | 30463.04 | 0.6993 |
| 4.75 | 6292.22000 | 29887.06 | 0.6861 |
| 4.83 | 6292.30333 | 29258.73 | 0.6717 |
| 4.92 | 6292.38667 | 28588.23 | 0.6563 |

INLET MANAGEMENT

Worksheet Protected

| | |
|------------------------------------|--------------------------|
| INLET NAME | DP3 |
| Site Type (Urban or Rural) | URBAN |
| Inlet Application (Street or Area) | STREET |
| Hydraulic Condition | In Sump |
| Inlet Type | CDOT Type R Curb Opening |

USER-DEFINED INPUT**User-Defined Design Flows**

| | |
|--------------------------------|-----|
| Minor Q_{known} (cfs) | 1.3 |
| Major Q_{known} (cfs) | 3.0 |

Bypass (Carry-Over) Flow from Upstream

| | |
|---|-------------------------|
| Receive Bypass Flow from: | No Bypass Flow Received |
| Minor Bypass Flow Received, Q_b (cfs) | 0.0 |
| Major Bypass Flow Received, Q_b (cfs) | 0.0 |

Watershed Characteristics

| | |
|---------------------------|--|
| Subcatchment Area (acres) | |
| Percent Impervious | |
| NRCS Soil Type | |

Watershed Profile

| | |
|------------------------|--|
| Overland Slope (ft/ft) | |
| Overland Length (ft) | |
| Channel Slope (ft/ft) | |
| Channel Length (ft) | |

Minor Storm Rainfall Input

| | |
|---|--|
| Design Storm Return Period, T_r (years) | |
| One-Hour Precipitation, P_1 (inches) | |

Major Storm Rainfall Input

| | |
|---|--|
| Design Storm Return Period, T_r (years) | |
| One-Hour Precipitation, P_1 (inches) | |

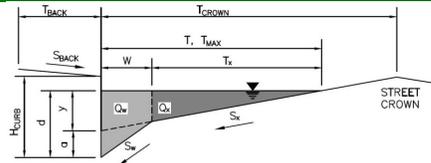
CALCULATED OUTPUT

| | |
|--|------------|
| Minor Total Design Peak Flow, Q (cfs) | 1.3 |
| Major Total Design Peak Flow, Q (cfs) | 3.0 |
| Minor Flow Bypassed Downstream, Q_b (cfs) | N/A |
| Major Flow Bypassed Downstream, Q_b (cfs) | N/A |

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

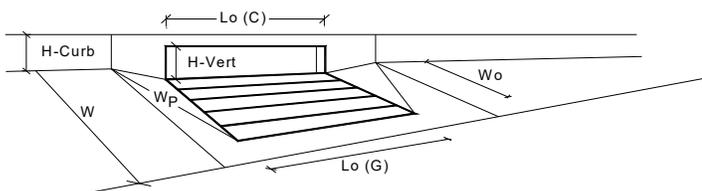
Project: _____
 Inlet ID: _____ **DP3**



| Gutter Geometry (Enter data in the blue cells) | | | | | | | |
|--|--|-------------|-------------|--------|---------------------------|---------------------------|--|
| Maximum Allowable Width for Spread Behind Curb | $T_{BACK} = 6.5$ ft | | | | | | |
| Side Slope Behind Curb (leave blank for no conveyance credit behind curb) | $S_{BACK} = 0.020$ ft/ft | | | | | | |
| Manning's Roughness Behind Curb (typically between 0.012 and 0.020) | $n_{BACK} = 0.020$ | | | | | | |
| Height of Curb at Gutter Flow Line | $H_{CURB} = 6.00$ inches | | | | | | |
| Distance from Curb Face to Street Crown | $T_{CROWN} = 18.0$ ft | | | | | | |
| Gutter Width | $W = 1.00$ ft | | | | | | |
| Street Transverse Slope | $S_x = 0.020$ ft/ft | | | | | | |
| Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft) | $S_w = 0.083$ ft/ft | | | | | | |
| Street Longitudinal Slope - Enter 0 for sump condition | $S_o = 0.000$ ft/ft | | | | | | |
| Manning's Roughness for Street Section (typically between 0.012 and 0.020) | $n_{STREET} = 0.016$ | | | | | | |
| Max. Allowable Spread for Minor & Major Storm | <table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>ft</th> </tr> <tr> <td>$T_{MAX} = 18.0$</td> <td>$T_{MAX} = 18.0$</td> <td></td> </tr> </table> | Minor Storm | Major Storm | ft | $T_{MAX} = 18.0$ | $T_{MAX} = 18.0$ | |
| Minor Storm | Major Storm | ft | | | | | |
| $T_{MAX} = 18.0$ | $T_{MAX} = 18.0$ | | | | | | |
| Max. Allowable Depth at Gutter Flowline for Minor & Major Storm | <table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>inches</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 7.5$</td> <td></td> </tr> </table> | Minor Storm | Major Storm | inches | $d_{MAX} = 6.0$ | $d_{MAX} = 7.5$ | |
| Minor Storm | Major Storm | inches | | | | | |
| $d_{MAX} = 6.0$ | $d_{MAX} = 7.5$ | | | | | | |
| Check boxes are not applicable in SUMP conditions | <input type="checkbox"/> <input type="checkbox"/> | | | | | | |
| MINOR STORM Allowable Capacity is based on Depth Criterion | | | | | | | |
| MAJOR STORM Allowable Capacity is based on Depth Criterion | | | | | | | |
| | <table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>cfs</th> </tr> <tr> <td>$Q_{allow} = \text{SUMP}$</td> <td>$Q_{allow} = \text{SUMP}$</td> <td></td> </tr> </table> | Minor Storm | Major Storm | cfs | $Q_{allow} = \text{SUMP}$ | $Q_{allow} = \text{SUMP}$ | |
| Minor Storm | Major Storm | cfs | | | | | |
| $Q_{allow} = \text{SUMP}$ | $Q_{allow} = \text{SUMP}$ | | | | | | |

INLET IN A SUMP OR SAG LOCATION

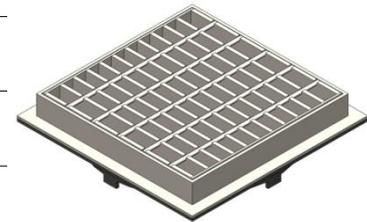
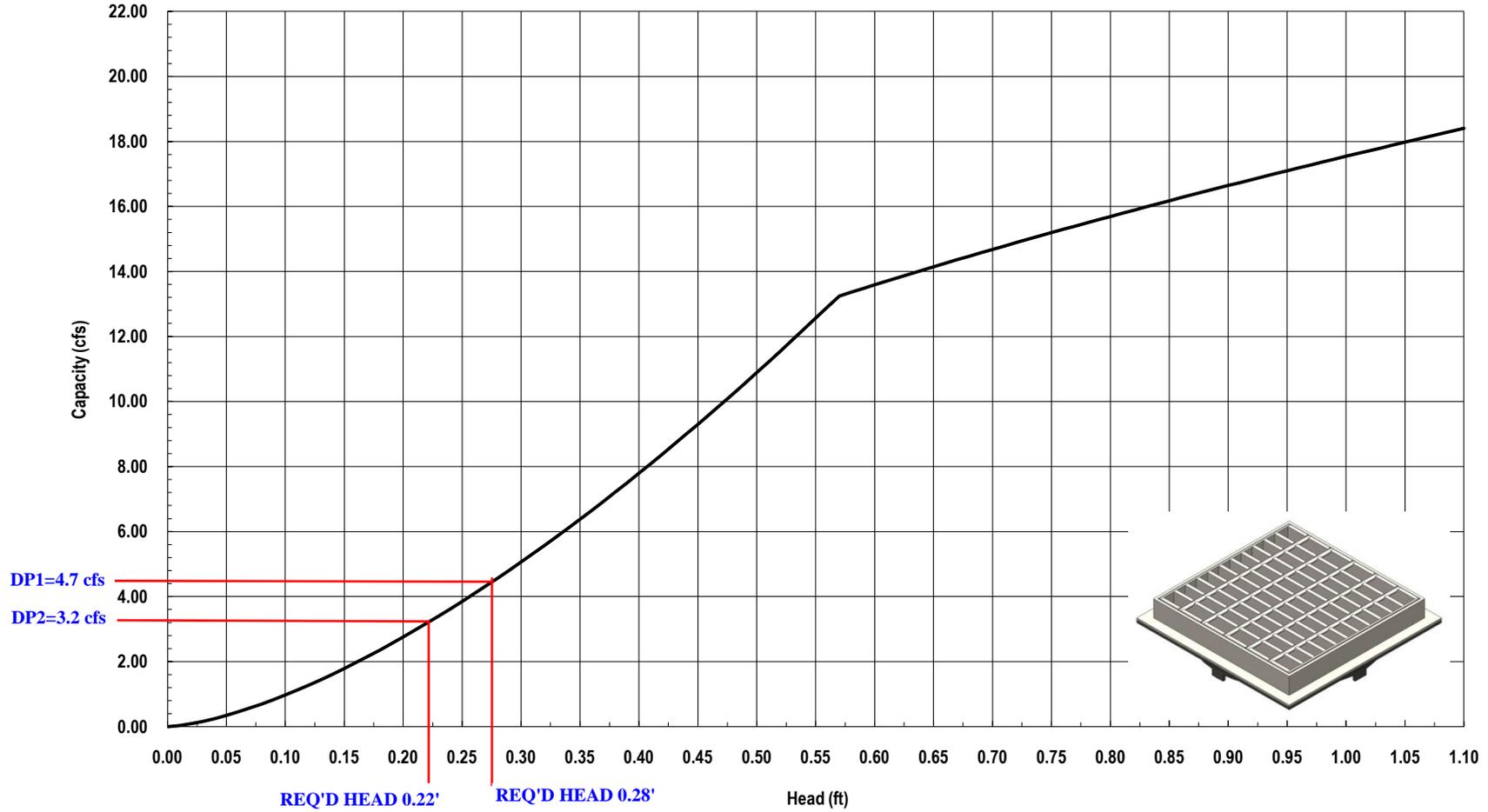
Version 4.06 Released August 2018



| Design Information (Input) | MINOR | MAJOR | |
|--|--------------------------|------------|--|
| Type of Inlet | CDOT Type R Curb Opening | | |
| Local Depression (additional to continuous gutter depression 'a' from above) | 3.00 | 3.00 | inches |
| Number of Unit Inlets (Grate or Curb Opening) | 1 | 1 | |
| Water Depth at Flowline (outside of local depression) | 5.1 | 5.1 | inches |
| Grate Information | MINOR | MAJOR | <input type="checkbox"/> Override Depths |
| Length of a Unit Grate | N/A | N/A | feet |
| Width of a Unit Grate | N/A | N/A | feet |
| Area Opening Ratio for a Grate (typical values 0.15-0.90) | N/A | N/A | |
| Clogging Factor for a Single Grate (typical value 0.50 - 0.70) | N/A | N/A | |
| Grate Weir Coefficient (typical value 2.15 - 3.60) | N/A | N/A | |
| Grate Orifice Coefficient (typical value 0.60 - 0.80) | N/A | N/A | |
| Curb Opening Information | MINOR | MAJOR | |
| Length of a Unit Curb Opening | 5.00 | 5.00 | feet |
| Height of Vertical Curb Opening in Inches | 6.00 | 6.00 | inches |
| Height of Curb Orifice Throat in Inches | 6.00 | 6.00 | inches |
| Angle of Throat (see USDCM Figure ST-5) | 63.40 | 63.40 | degrees |
| Side Width for Depression Pan (typically the gutter width of 2 feet) | 1.00 | 1.00 | feet |
| Clogging Factor for a Single Curb Opening (typical value 0.10) | 0.10 | 0.10 | |
| Curb Opening Weir Coefficient (typical value 2.3-3.7) | 3.60 | 3.60 | |
| Curb Opening Orifice Coefficient (typical value 0.60 - 0.70) | 0.67 | 0.67 | |
| Low Head Performance Reduction (Calculated) | MINOR | MAJOR | |
| Depth for Grate Midwidth | N/A | N/A | ft |
| Depth for Curb Opening Weir Equation | 0.34 | 0.34 | ft |
| Combination Inlet Performance Reduction Factor for Long Inlets | 0.65 | 0.65 | |
| Curb Opening Performance Reduction Factor for Long Inlets | 1.00 | 1.00 | |
| Grated Inlet Performance Reduction Factor for Long Inlets | N/A | N/A | |
| Total Inlet Interception Capacity (assumes clogged condition) | MINOR | MAJOR | |
| Q_a | 4.4 | 4.4 | cfs |
| Q _{PEAK REQUIRED} | 1.3 | 3.0 | cfs |

Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)

Nyloplast 2' x 2' Steel Bar / MAG Grate Inlet Capacity Chart



3130 Verona Avenue • Buford, GA 30518
 (866) 888-8479 / (770) 932-2443 • Fax: (770) 932-2490
 © Nyloplast Inlet Capacity Charts June 2012

UNDERGROUND DETENTION DETAILS

| PROJECT INFORMATION | |
|-----------------------------|--|
| ENGINEERED PRODUCT MANAGER: | JEROME MAGSINO 303-349-7555 JEROME.MAGSINO@ADSPIPE.COM |
| ADS SALES REP: | AARON ZEE 303-548-3479 AARON.ZEE@ADSPIPE.COM |
| PROJECT NO: | S295850 |



CROSSROADS MIXED USE FILING NO. 1

COLORADO SPRINGS, CO

MC-7200 STORMTECH CHAMBER SPECIFICATIONS

- CHAMBERS SHALL BE STORMTECH MC-7200.
- CHAMBERS SHALL BE ARCH-SHAPED AND SHALL BE MANUFACTURED FROM VIRGIN, IMPACT-MODIFIED POLYPROPYLENE COPOLYMERS.
- CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS" CHAMBER CLASSIFICATION 60x101.
- CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORTS THAT WOULD IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.
- THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.
- CHAMBERS SHALL BE DESIGNED, TESTED AND ALLOWABLE LOAD CONFIGURATIONS DETERMINED IN ACCORDANCE WITH ASTM F2787, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". LOAD CONFIGURATIONS SHALL INCLUDE: 1) INSTANTANEOUS (<1 MIN) AASHTO DESIGN TRUCK LIVE LOAD ON MINIMUM COVER 2) MAXIMUM PERMANENT (75-YR) COVER LOAD AND 3) ALLOWABLE COVER WITH PARKED (1-WEEK) AASHTO DESIGN TRUCK.
- REQUIREMENTS FOR HANDLING AND INSTALLATION:
 - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
 - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 3".
 - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT SHALL BE GREATER THAN OR EQUAL TO 450 LBS/FT/%. THE ASC IS DEFINED IN SECTION 6.2.8 OF ASTM F2418. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.
- ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. UPON REQUEST BY THE SITE DESIGN ENGINEER OR OWNER, THE CHAMBER MANUFACTURER SHALL SUBMIT A STRUCTURAL EVALUATION FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE AS FOLLOWS:
 - THE STRUCTURAL EVALUATION SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER.
 - THE STRUCTURAL EVALUATION SHALL DEMONSTRATE THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY SECTIONS 3 AND 12.12 OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR THERMOPLASTIC PIPE.
 - THE TEST DERIVED CREEP MODULUS AS SPECIFIED IN ASTM F2418 SHALL BE USED FOR PERMANENT DEAD LOAD DESIGN EXCEPT THAT IT SHALL BE THE 75-YEAR MODULUS USED FOR DESIGN.
- CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.

IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF MC-7200 CHAMBER SYSTEM

- STORMTECH MC-7200 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS.
- STORMTECH MC-7200 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH MC-7200 CONSTRUCTION GUIDE".
- CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR EXCAVATOR SITUATED OVER THE CHAMBERS. STORMTECH RECOMMENDS 3 BACKFILL METHODS:
 - STONESHOOTER LOCATED OFF THE CHAMBER BED.
 - BACKFILL AS ROWS ARE BUILT USING AN EXCAVATOR ON THE FOUNDATION STONE OR SUBGRADE.
 - BACKFILL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR.
- THE FOUNDATION STONE SHALL BE LEVELED AND COMPACTED PRIOR TO PLACING CHAMBERS.
- JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE.
- MAINTAIN MINIMUM - 9" (230 mm) SPACING BETWEEN THE CHAMBER ROWS.
- INLET AND OUTLET MANIFOLDS MUST BE INSERTED A MINIMUM OF 12" (300 mm) INTO CHAMBER END CAPS.
- EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE MEETING THE AASHTO M43 DESIGNATION OF #3 OR #4.
- STONE SHALL BE BROUGHT UP EVENLY AROUND CHAMBERS SO AS NOT TO DISTORT THE CHAMBER SHAPE. STONE DEPTHS SHOULD NEVER DIFFER BY MORE THAN 12" (300 mm) BETWEEN ADJACENT CHAMBER ROWS.
- STONE MUST BE PLACED ON THE TOP CENTER OF THE CHAMBER TO ANCHOR THE CHAMBERS IN PLACE AND PRESERVE ROW SPACING.
- THE CONTRACTOR MUST REPORT ANY DISCREPANCIES WITH CHAMBER FOUNDATION MATERIAL BEARING CAPACITIES TO THE SITE DESIGN ENGINEER.
- ADS RECOMMENDS THE USE OF "FLEXSTORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE RUNOFF.

NOTES FOR CONSTRUCTION EQUIPMENT

- STORMTECH MC-7200 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH MC-7200 CONSTRUCTION GUIDE".
- THE USE OF EQUIPMENT OVER MC-7200 CHAMBERS IS LIMITED:
 - NO EQUIPMENT IS ALLOWED ON BARE CHAMBERS.
 - NO RUBBER TIRE LOADER, DUMP TRUCK, OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE WITH THE "STORMTECH MC-7200 CONSTRUCTION GUIDE".
 - WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH MC-7200 CONSTRUCTION GUIDE".
- FULL 36" (900 mm) OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.

USE OF A DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO CHAMBERS AND IS NOT AN ACCEPTABLE BACKFILL METHOD. ANY CHAMBERS DAMAGED BY USING THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH STANDARD WARRANTY.

CONTACT STORMTECH AT 1-888-892-2694 WITH ANY QUESTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT.

CONCEPTUAL LAYOUT

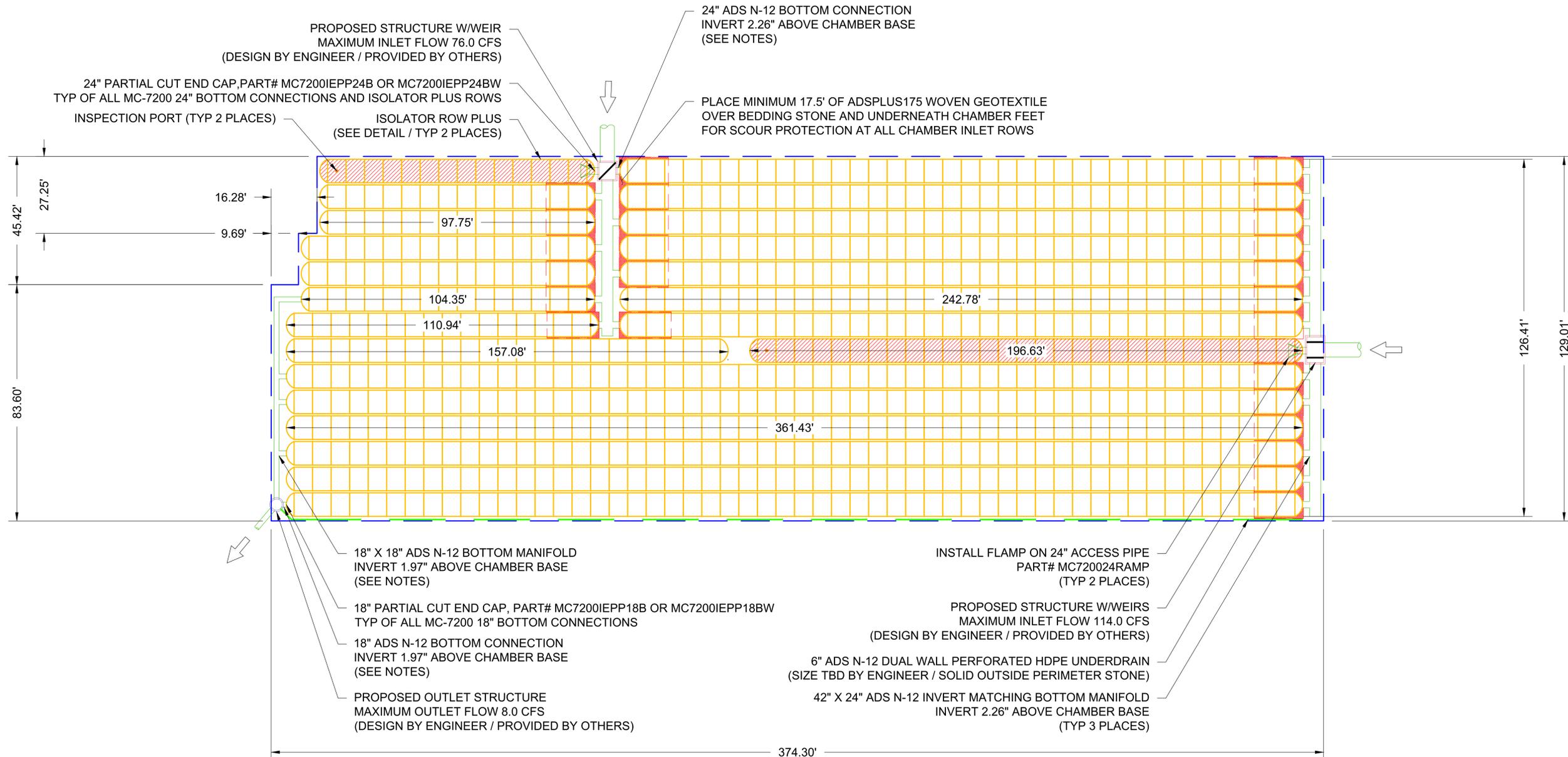
| | |
|----------------|--|
| 731 | STORMTECH MC-7200 CHAMBERS |
| 44 | STORMTECH MC-7200 END CAPS |
| 12 | STONE ABOVE (in) |
| 9 | STONE BELOW (in) |
| 40 | % STONE VOID |
| 206,903 | INSTALLED SYSTEM VOLUME (CF) (PERIMETER STONE INCLUDED) |
| 47670 | SYSTEM AREA (ft²) |
| 1006 | SYSTEM PERIMETER (ft) |

CONCEPTUAL ELEVATIONS

| | |
|---------|---|
| 6300.22 | MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT/UNPAVED) |
| 6295.72 | MINIMUM ALLOWABLE GRADE (UNPAVED WITH TRAFFIC) |
| 6295.22 | MINIMUM ALLOWABLE GRADE (UNPAVED NO TRAFFIC) |
| 6295.22 | MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT) |
| 6295.22 | MINIMUM ALLOWABLE GRADE (TOP OF RIGID PAVEMENT) |
| 6294.22 | TOP OF STONE |
| 6293.22 | TOP OF MC-7200 CHAMBER |
| 6288.41 | 42" X 24" MANIFOLD INVERT |
| 6288.41 | 24" ISOLATOR ROW PLUS CONNECTION INVERT |
| 6288.41 | 24" BOTTOM CONNECTION |
| 6288.38 | 18" BOTTOM MANIFOLD / CONNECTION INVERT |
| 6288.22 | BOTTOM OF MC-7200 CHAMBER |
| 6287.47 | UNDERDRAIN INVERT |
| 6287.47 | BOTTOM OF STONE |

NOTES

- MANIFOLD SIZE TO BE DETERMINED BY SITE DESIGN ENGINEER. SEE TECHNICAL NOTE 6.32 FOR MANIFOLD SIZING GUIDANCE.
 - DUE TO THE ADAPTATION OF THIS CHAMBER SYSTEM TO SPECIFIC SITE AND DESIGN CONSTRAINTS, IT MAY BE NECESSARY TO CUT AND COUPLE ADDITIONAL PIPE TO STANDARD MANIFOLD COMPONENTS IN THE FIELD.
 - THE SITE DESIGN ENGINEER MUST REVIEW ELEVATIONS AND IF NECESSARY ADJUST GRADING TO ENSURE THE CHAMBER COVER REQUIREMENTS ARE MET.
 - THIS CHAMBER SYSTEM WAS DESIGNED WITHOUT SITE-SPECIFIC INFORMATION ON SOIL CONDITIONS OR BEARING CAPACITY. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SOIL AND PROVIDING THE BEARING CAPACITY OF THE INSITU SOILS. THE BASE STONE DEPTH MAY BE INCREASED OR DECREASED ONCE THIS INFORMATION IS PROVIDED.
- **NOT FOR CONSTRUCTION:** THIS LAYOUT IS FOR DIMENSIONAL PURPOSES ONLY TO PROVE CONCEPT & THE REQUIRED STORAGE VOLUME CAN BE ACHIEVED ON SITE.



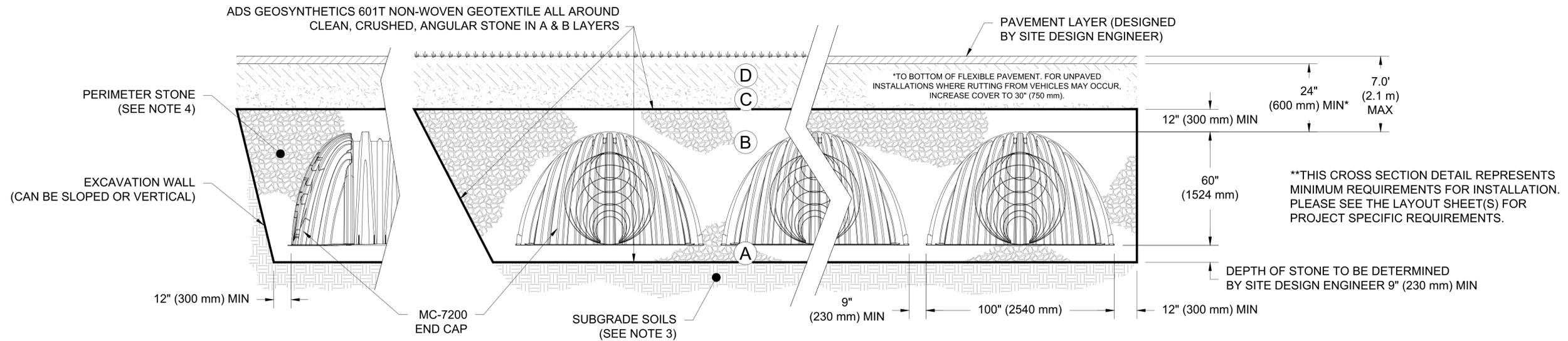
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|--|--|--|-------------|--|--|
| <p>CROSSROADS MIXED USE FILING NO. 1 COLORADO SPRINGS, CO</p> | | <p>DATE: 05-05-22 DRAWN: TSG PROJECT #: S295850 CHECKED: CTS</p> | | | |
| | <p>12-18-22 RKC</p> | <p>12/13/22 BMW</p> | <p>DATE</p> | <p>DESCRIPTION</p> | |
| <p>StormTech® Chamber System</p> | | <p>888-892-2694 WWW.STORMTECH.COM</p> | | <p>4640 TRUEMAN BLVD HILLIARD, OH 43026</p> | |
| <p>2 SHEET OF 5</p> | | <p>THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS UNDER THE DIRECTION OF THE SITE DESIGN ENGINEER OR OTHER PROJECT REPRESENTATIVE. THE SITE DESIGN ENGINEER SHALL REVIEW THIS DRAWING PRIOR TO CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE SITE DESIGN ENGINEER TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE LAWS, REGULATIONS, AND PROJECT REQUIREMENTS.</p> | | | |

ACCEPTABLE FILL MATERIALS: STORMTECH MC-7200 CHAMBER SYSTEMS

| MATERIAL LOCATION | | DESCRIPTION | AASHTO MATERIAL CLASSIFICATIONS | COMPACTION / DENSITY REQUIREMENT |
|-------------------|--|--|---|---|
| D | FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER | ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS. | N/A | PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS. |
| C | INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 24" (600 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER. | GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE. MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER. | AASHTO M145 ¹ A-1, A-2-4, A-3 OR AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10 | BEGIN COMPACTIONS AFTER 24" (600 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 12" (300 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. |
| B | EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE. | CLEAN, CRUSHED, ANGULAR STONE | AASHTO M43 ¹ 3, 4 | NO COMPACTION REQUIRED. |
| A | FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER. | CLEAN, CRUSHED, ANGULAR STONE | AASHTO M43 ¹ 3, 4 | PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. ^{2,3} |

PLEASE NOTE:

- THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
- STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 9" (230 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
- WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.
- ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.

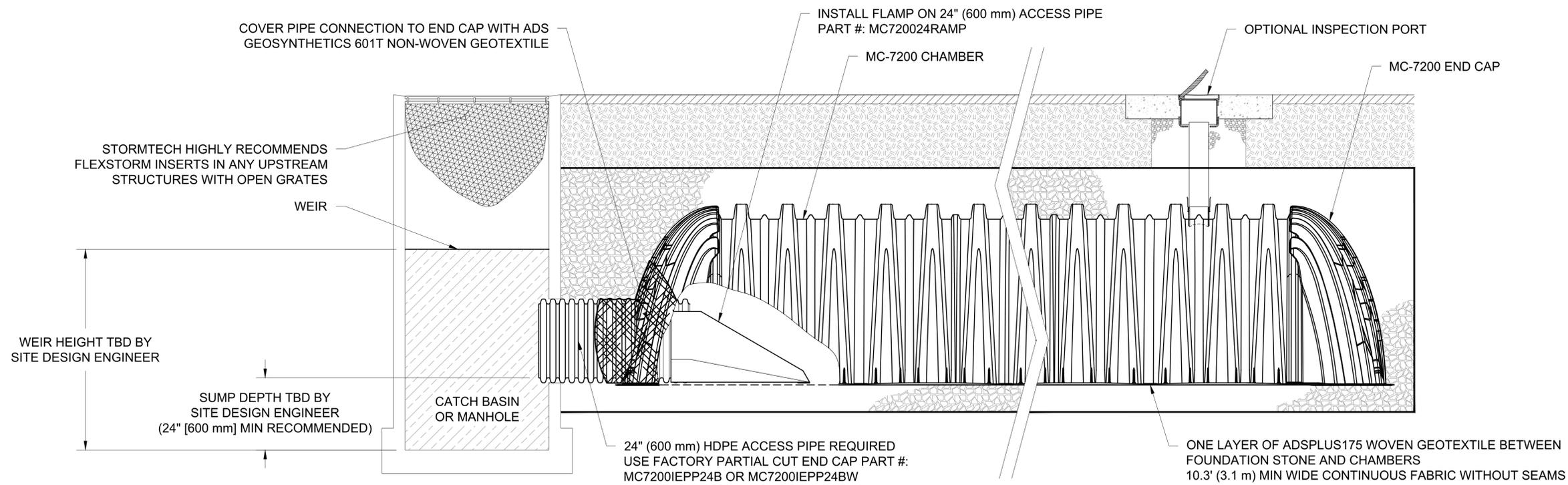


NOTES:

- CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS" CHAMBER CLASSIFICATION 60x101
- MC-7200 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
- THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
- PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
- REQUIREMENTS FOR HANDLING AND INSTALLATION:
 - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
 - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 3".
 - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 500 LBS/FT/%. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.

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| CROSSROADS MIXED USE FILING NO. 1 COLORADO SPRINGS, CO | | DATE: 05-05-22 DRAWN: TSG PROJECT #: S295850 CHECKED: CTS |
| 12-18-22 RKC JPR BMW DRWN CHKD | UPDATED ELEVATIONS ELEVATION AND LAYOUT ADJUSTMENTS DESCRIPTION | |
| | | 888-892-2694 WWW.STORMTECH.COM |
| 4640 TRUEMAN BLVD HILLIARD, OH 43026 | | |
| 3 SHEET OF 5 | | |

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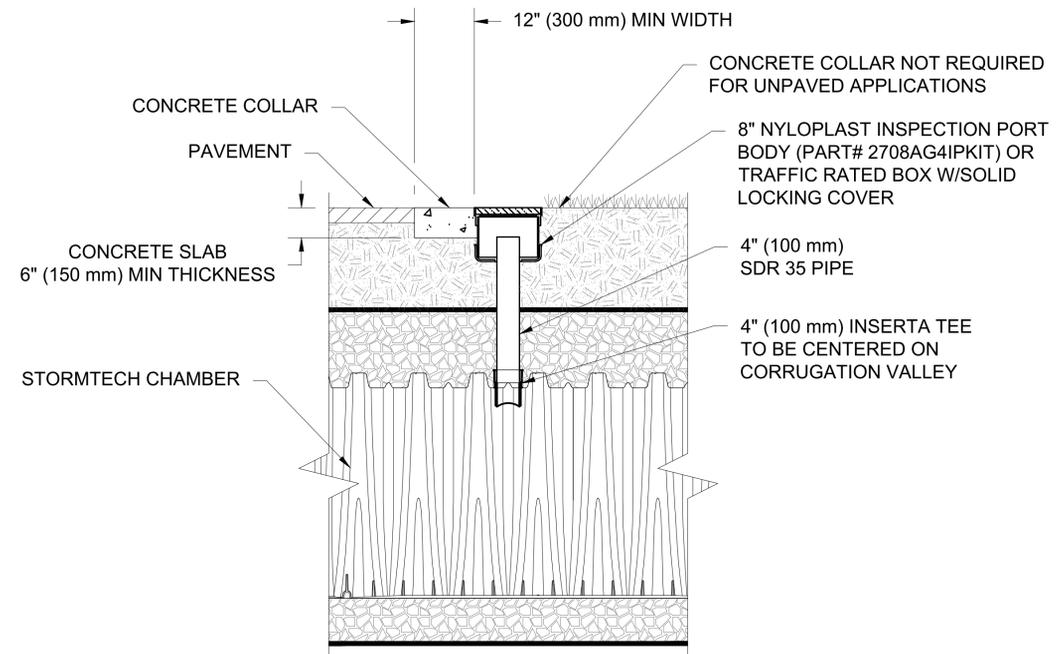
MC-7200 ISOLATOR ROW PLUS DETAIL
NTS

INSPECTION & MAINTENANCE

- STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
 - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
 - A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
 - A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
 - A.4. LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
 - A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
 - B. ALL ISOLATOR PLUS ROWS
 - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
 - B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
 - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
 - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
 - B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
 - B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
 - C. VACUUM STRUCTURE SUMP AS REQUIRED
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.



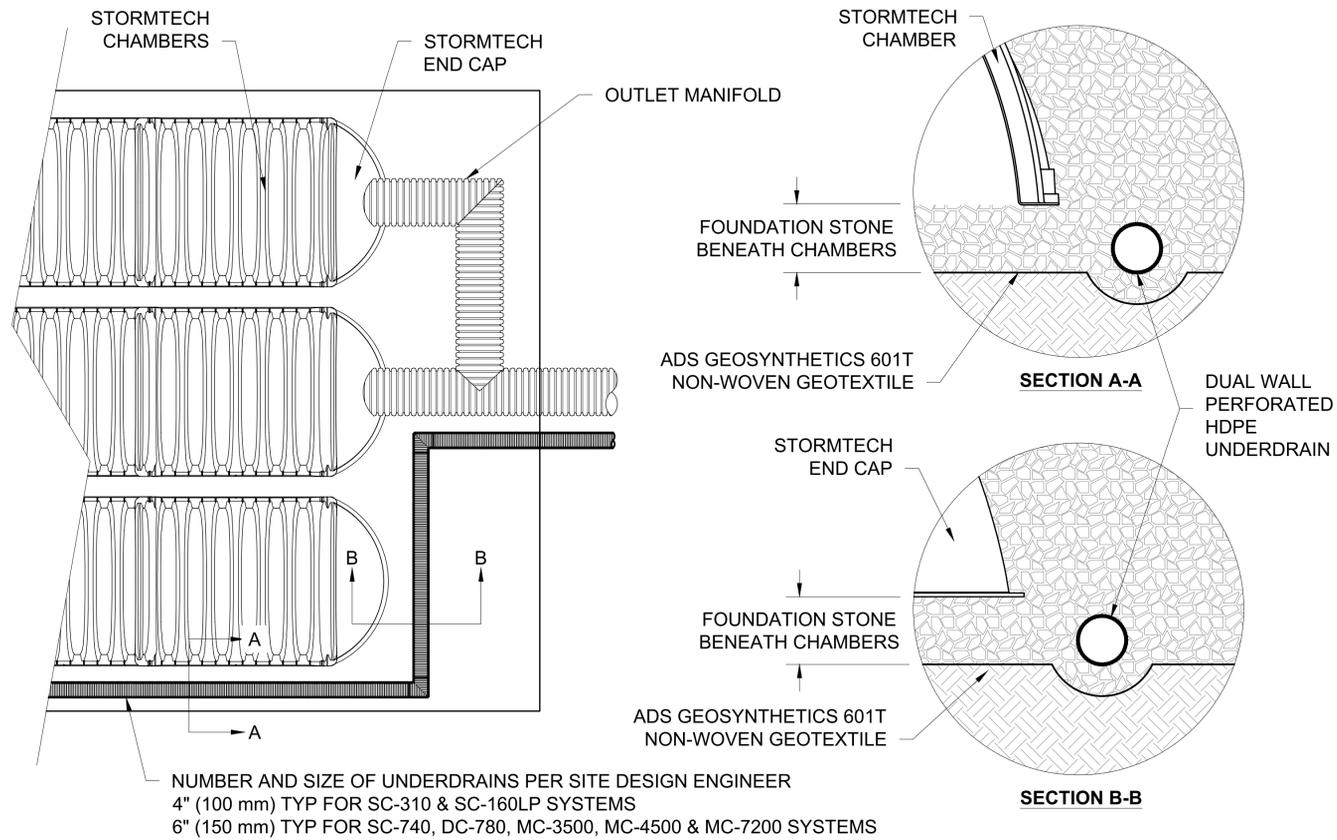
NOTE:
INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER CORRUGATION VALLEY.

4\"/>NTS

| | | | | | |
|--|-----|--|-----|---------------------|-------------|
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| | | | | | |
| 12-18-22 | RKC | BMW | JPR | DATE | DESCRIPTION |
| | | | | | |
| <p>StormTech® Chamber System 888-892-2694 WWW.STORMTECH.COM</p> | | | | | |
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| <p>4 OF 5</p> | | | | <p>SHEET OF</p> | |

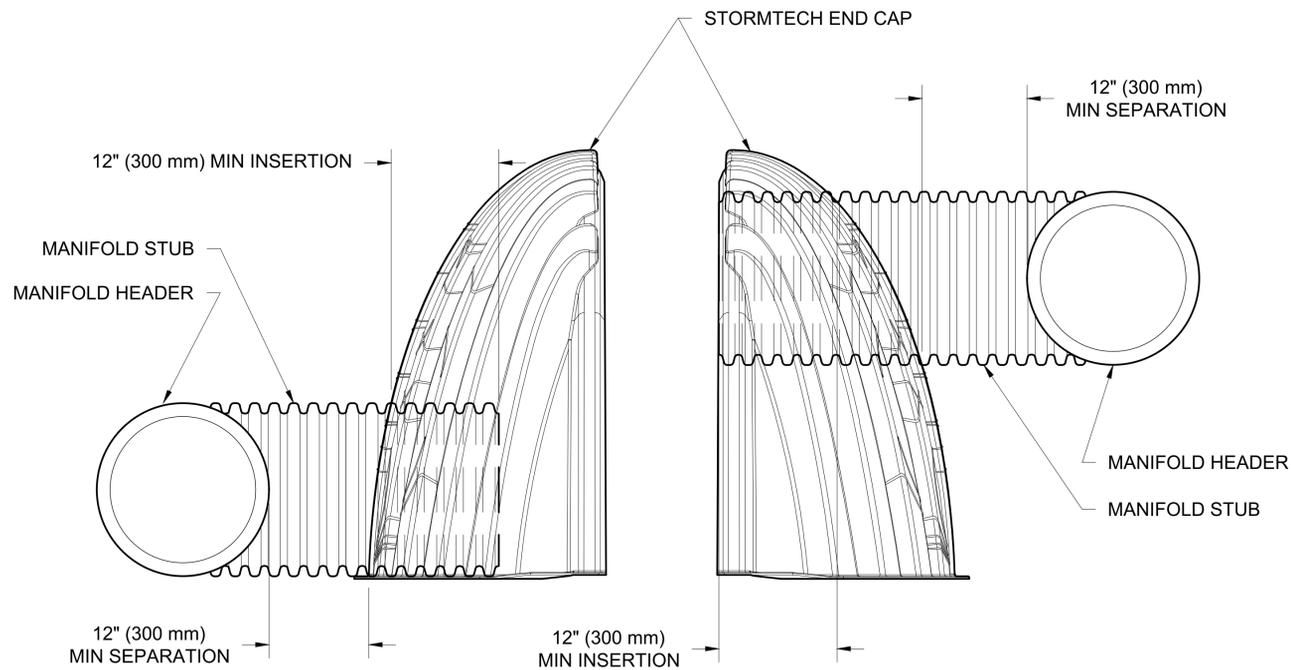
UNDERDRAIN DETAIL

NTS



MC-SERIES END CAP INSERTION DETAIL

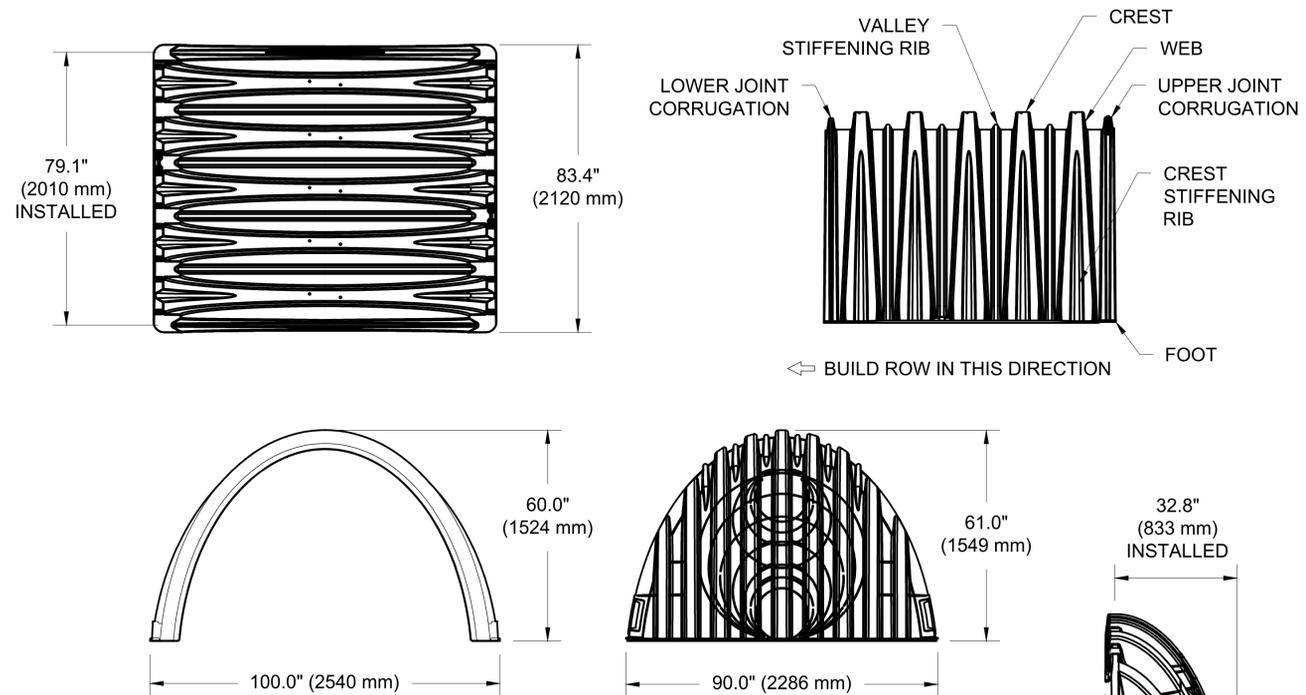
NTS



NOTE: MANIFOLD STUB MUST BE LAID HORIZONTAL FOR A PROPER FIT IN END CAP OPENING.

MC-7200 TECHNICAL SPECIFICATION

NTS



NOMINAL CHAMBER SPECIFICATIONS

| | | |
|---------------------------------|------------------------|-------------------------------|
| SIZE (W X H X INSTALLED LENGTH) | 100.0" X 60.0" X 79.1" | (2540 mm X 1524 mm X 2010 mm) |
| CHAMBER STORAGE | 175.9 CUBIC FEET | (4.98 m ³) |
| MINIMUM INSTALLED STORAGE* | 267.3 CUBIC FEET | (7.56 m ³) |
| WEIGHT (NOMINAL) | 205 lbs. | (92.9 kg) |

NOMINAL END CAP SPECIFICATIONS

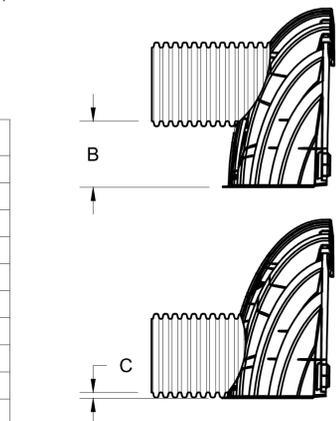
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|---------------------------------|-----------------------|------------------------------|
| SIZE (W X H X INSTALLED LENGTH) | 90.0" X 61.0" X 32.8" | (2286 mm X 1549 mm X 833 mm) |
| END CAP STORAGE | 39.5 CUBIC FEET | (1.12 m ³) |
| MINIMUM INSTALLED STORAGE* | 115.3 CUBIC FEET | (3.26 m ³) |
| WEIGHT (NOMINAL) | 90 lbs. | (40.8 kg) |

*ASSUMES 12" (305 mm) STONE ABOVE, 9" (229 mm) STONE FOUNDATION AND BETWEEN CHAMBERS, 12" (305 mm) STONE PERIMETER IN FRONT OF END CAPS AND 40% STONE POROSITY.

PARTIAL CUT HOLES AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "B"
 PARTIAL CUT HOLES AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "T"
 END CAPS WITH A PREFABRICATED WELDED STUB END WITH "W"

| PART # | STUB | B | C |
|----------------|---------------|------------------|---------------|
| MC7200IEPP06T | 6" (150 mm) | 42.54" (1081 mm) | --- |
| MC7200IEPP06B | --- | --- | 0.86" (22 mm) |
| MC7200IEPP08T | 8" (200 mm) | 40.50" (1029 mm) | --- |
| MC7200IEPP08B | --- | --- | 1.01" (26 mm) |
| MC7200IEPP10T | 10" (250 mm) | 38.37" (975 mm) | --- |
| MC7200IEPP10B | --- | --- | 1.33" (34 mm) |
| MC7200IEPP12T | 12" (300 mm) | 35.69" (907 mm) | --- |
| MC7200IEPP12B | --- | --- | 1.55" (39 mm) |
| MC7200IEPP15T | 15" (375 mm) | 32.72" (831 mm) | --- |
| MC7200IEPP15B | --- | --- | 1.70" (43 mm) |
| MC7200IEPP18T | --- | 29.36" (746 mm) | --- |
| MC7200IEPP18TW | 18" (450 mm) | --- | --- |
| MC7200IEPP18B | --- | --- | 1.97" (50 mm) |
| MC7200IEPP18BW | --- | --- | --- |
| MC7200IEPP24T | --- | 23.05" (585 mm) | --- |
| MC7200IEPP24TW | 24" (600 mm) | --- | --- |
| MC7200IEPP24B | --- | --- | 2.26" (57 mm) |
| MC7200IEPP24BW | --- | --- | --- |
| MC7200IEPP30BW | 30" (750 mm) | --- | 2.95" (75 mm) |
| MC7200IEPP36BW | 36" (900 mm) | --- | 3.25" (83 mm) |
| MC7200IEPP42BW | 42" (1050 mm) | --- | 3.55" (90 mm) |

NOTE: ALL DIMENSIONS ARE NOMINAL



CUSTOM PREFABRICATED INVERTS ARE AVAILABLE UPON REQUEST. INVENTORIED MANIFOLDS INCLUDE 12-24" (300-600 mm) SIZE ON SIZE AND 15-48" (375-1200 mm) ECCENTRIC MANIFOLDS. CUSTOM INVERT LOCATIONS ON THE MC-7200 END CAP CUT IN THE FIELD ARE NOT RECOMMENDED FOR PIPE SIZES GREATER THAN 10" (250 mm). THE INVERT LOCATION IN COLUMN 'B' ARE THE HIGHEST POSSIBLE FOR THE PIPE SIZE.

CROSSROADS MIXED USE
 FILING NO. 1
 COLORADO SPRINGS, CO
 DATE: 05-05-22
 PROJECT #: S295850
 DRAWN: TSG
 CHECKED: CTS

| DATE | DESCRIPTION |
|----------|--------------------------------------|
| 12-18-22 | UPDATED ELEVATIONS |
| 12/13/22 | JPR ELEVATION AND LAYOUT ADJUSTMENTS |
| | DATE |
| | DRWN |
| | CHKD |
| | DATE |

StormTech®
 Chamber System
 888-892-2694 | WWW.STORMTECH.COM

4640 TRUEMAN BLVD
 HILLIARD, OH 43026

ADS

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| PROJECT INFORMATION | |
|-----------------------------|--|
| ENGINEERED PRODUCT MANAGER: | JEROME MAGSINO 303-349-7555 JEROME.MAGSINO@ADSPIPE.COM |
| ADS SALES REP: | AARON ZEE ---- AARON.ZEE@ADSPIPE.COM |
| PROJECT NO: | S295850 |



CROSSROADS MIXED USE FILING NO. 1

COLORADO SPRINGS, CO

BAYSAVER BAYSEPARATOR SPECIFICATIONS

MATERIALS AND DESIGN

- A. CONCRETE STRUCTURES SHALL BE DESIGNED FOR H-20 TRAFFIC LOADING AND APPLICABLE SOIL LOADS OR AS OTHERWISE DETERMINED BY A LICENSED PROFESSIONAL ENGINEER. THE MATERIALS AND STRUCTURAL DESIGN OF THE DEVICES SHALL BE PER ASTM C857 AND ASTM C858.
 1. THE MINIMUM COMPRESSIVE STRENGTH OF THE CONCRETE IN THE MANHOLE BASE, RISER, AND TOP SECTIONS SHALL BE 4000 PSI.
 2. THE MINIMUM WALL THICKNESS SHALL BE ONE TWELFTH OF THE INTERNAL DIAMETER OF THE RISER OF LARGEST CONE DIAMETER.
 3. CEMENT SHALL CONFORM TO THE REQUIREMENTS FOR PORTLAND CEMENT OF SPECIFICATION C150.
 4. AGGREGATES SHALL CONFORM TO SPECIFICATION C33, EXCEPT THAT THE REQUIREMENT FOR GRADATION SHALL NOT APPLY.
 5. REINFORCEMENT SHALL CONSIST OF WIRE CONFORMING TO SPECIFICATION A82 OR SPECIFICATION A496, OF WIRE FABRIC CONFORMING TO SPECIFICATION A185 OR SPECIFICATION A497, OR OF BARS OF GRADE 40 STEEL CONFORMING TO SPECIFICATION A615/A615M.
 6. THE ACCESS COVER SHALL BE DESIGNED FOR HS20-44 TRAFFIC LOADING AND SHALL PROVIDE A MINIMUM 30 INCH CLEAR OPENING.
 7. ALL JOINTS SHALL BE WATERPROOF WITH WRAPPED GASKETS OR SEALED WITH A MASTIC TREATMENT.
 8. ANY GROUT USED WITHIN THE SYSTEM SHALL MEET THE ASTM C 1107 "STANDARD SPECIFICATION FOR PACKAGED DRY, HYDRAULIC-CEMENT GROUT (NON-SHRINK)". GRADES A, B AND C AT A POURABLE AND PLASTIC CONSISTENCY AT 70°F. CRD C 621 "CORPS OF ENGINEERS SPECIFICATION FOR NON-SHRINK GROUT."
 9. STORAGE MANHOLE CONNECTOR PIPES SHALL BE EQUIPPED WITH A SEAL GASKET THAT MEETS OR EXCEEDS MATERIAL SPECIFICATIONS OF ASTM C-923 OR OTHER LOCALLY APPROVED METHODS.
- B. THE SEPARATOR STRUCTURE SHALL BE SUBSTANTIALLY CONSTRUCTED OF HDPE OR EQUIVALENT CORROSION RESISTANT MATERIAL MEETING ASTM D330, ASTM F412, AND ASTM C-425.
- C. PIPES WITHIN THE UNIT, (I.E., TEE PIPES, CONNECTOR PIPES AND DOWN PIPES) SHALL BE CONSTRUCTED OF AT LEAST SDR 32.5 HDPE PIPE OF STANDARD ASTM F412.
- D. PIPE AND FITTING MATERIAL SHALL BE HIGH DENSITY POLYETHYLENE MEETING ASTM D330 MINIMUM CELL CLASSIFICATION 335400C FOR 24-INCH THROUGH 60-INCH DIAMETERS. THE 24- THROUGH 60-INCH PIPE MATERIAL SHALL BE SLOW CRACK RESISTANT HDPE MATERIAL, EVALUATED USING THE SINGLE POINT NOTCHED CONSTANT TENSILE LOAD (SP-NCTL) TEST.

PERFORMANCE

- A. THE STORMWATER TREATMENT UNIT SHALL BE AN ONLINE UNIT CAPABLE OF CONVEYING 100% OF THE DESIGN PEAK FLOW.
- B. THE BAYSEPARATOR UNIT SHALL BE DESIGNED TO REMOVE AT LEAST 80% OF THE SUSPENDED SOLIDS LOAD ON AN ANNUAL AGGREGATE REMOVAL BASIS. SAID REMOVAL SHALL BE BASED ON FULL-SCALE THIRD PARTY TESTING USING F-95 MEDIA GRADATION (MANUFACTURED BY US SILICA) OR EQUIVALENT. SAID FULL SCALE TESTING SHALL HAVE INCLUDED SEDIMENT CAPTURE BASED ON ACTUAL TOTAL MASS COLLECTED BY THE STORMWATER TREATMENT UNIT(S).
- C. THE STORMWATER TREATMENT UNIT SHALL CONSIST OF ONE (1) PREFABRICATED SEPARATOR STRUCTURE, ONE (1) ONLINE COARSE SEDIMENT CAPTURE STRUCTURE, AND ONE (1) OFFLINE SEDIMENT AND FLOATABLE CAPTURE STRUCTURE. THE SEPARATOR STRUCTURE SHALL BE SUBSTANTIALLY CONSTRUCTED OF HDPE OR EQUIVALENT CORROSION RESISTANT MATERIAL. THE OFFLINE SEDIMENT STORAGE STRUCTURE MUST PROVIDE FOR OFFLINE SEDIMENT STORAGE OF SEDIMENTS AND FLOATABLES THAT ARE ISOLATED FROM HIGH INTENSITY STORMS.
- D. THE STORMWATER TREATMENT UNIT(S) HEAD LOSS AT THE PEAK DESIGN FLOW RATE SHALL NOT EXCEED THE HEAD LOSS SPECIFIED BY THE ENGINEER.
- E. THE UNIT SHALL BE DESIGNED TO REMOVE SEDIMENT PARTICLES AS WELL AS FLOATING OILS AND DEBRIS.

MANUFACTURER

- A. THE STORMWATER TREATMENT UNIT(S) SHALL BE OF A BASIC DESIGN THAT HAS BEEN INSTALLED AND USED SUCCESSFULLY FOR A MINIMUM OF 5 YEARS.
- B. EACH STORMWATER TREATMENT SYSTEM SHALL BE A BAYSEPARATOR SYSTEM AS MANUFACTURED BY BAYSAVER, LLC, 1030 DEER HOLLOW DR., MOUNT AIRY, MD 21771, PHONE (301) 829-6470, FAX (301)-829-3747, TOLL FREE 1-800-229-7283 (1-800-BAYSAVER), EMAIL INFO@BAYSAVER.COM PROTECTED UNDER US PATENT NUMBER 5746911

BAYSEPARATOR MAINTENANCE

BAYSEPARATOR SYSTEMS MUST BE INSPECTED AND MAINTAINED PERIODICALLY. INSPECTION IS MADE BY CHECKING THE DEPTH OF SEDIMENT IN EACH MANHOLE WITH A GRADE STICK OR SIMILAR DEVICE. MAINTENANCE IS REQUIRED WHEN THE SEDIMENT DEPTH IN EITHER MANHOLE EXCEEDS 2 FEET. MINIMUM INSPECTION IS RECOMMENDED TWICE A YEAR TO MAINTAIN OPERATION AND FUNCTION OF BAYSAVER.

MAINTENANCE CONSISTS OF THE FOLLOWING:

A. STORAGE MANHOLE

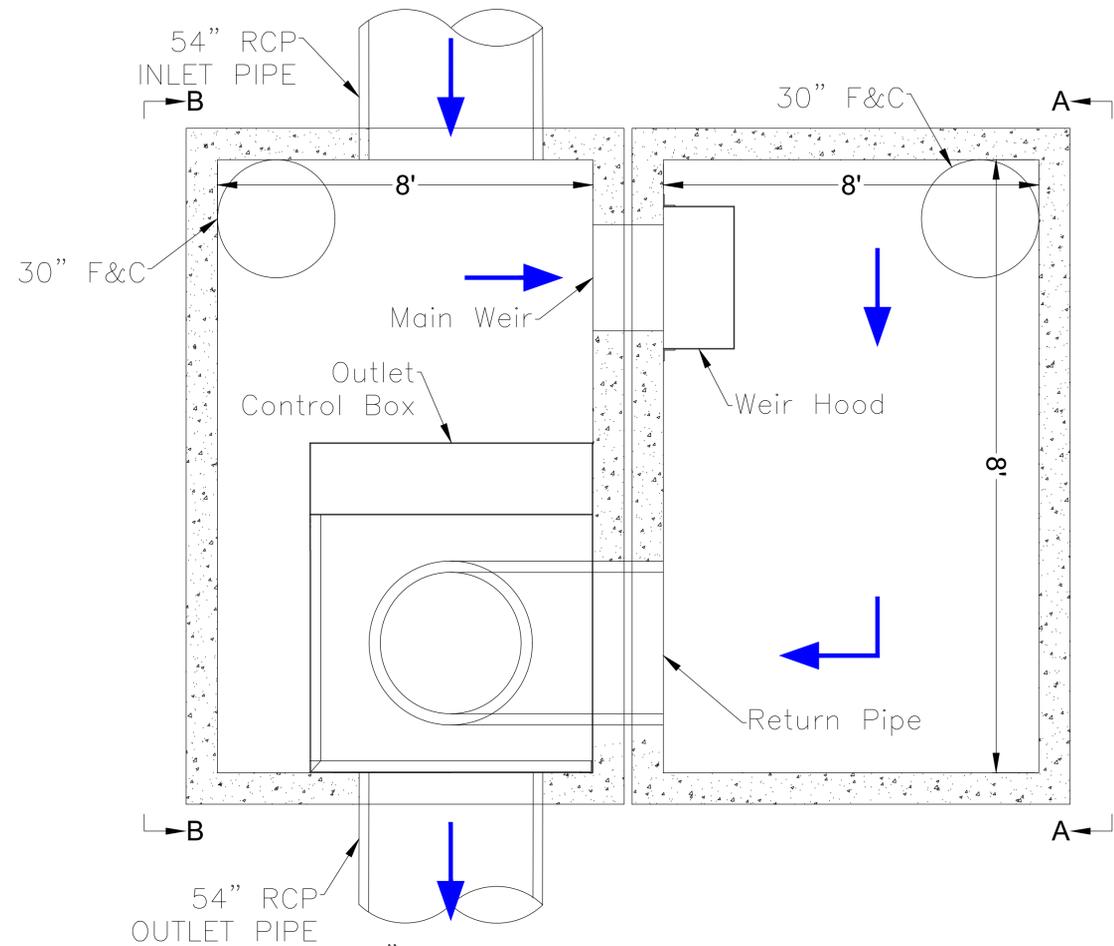
1. REMOVE THE ENTIRE VOLUME OF THE CONTAMINATED WATER BY VACUUM TRUCK.
2. CLEAN THE MANHOLE WALLS AND FLUSH OUT THE MANHOLE USING A HIGH PRESSURE HOSE AND REMOVE FLUSHING WATER BY VACUUM TRUCK. MAKE CERTAIN MANHOLE IS CLEAN.

B. PRIMARY MANHOLE

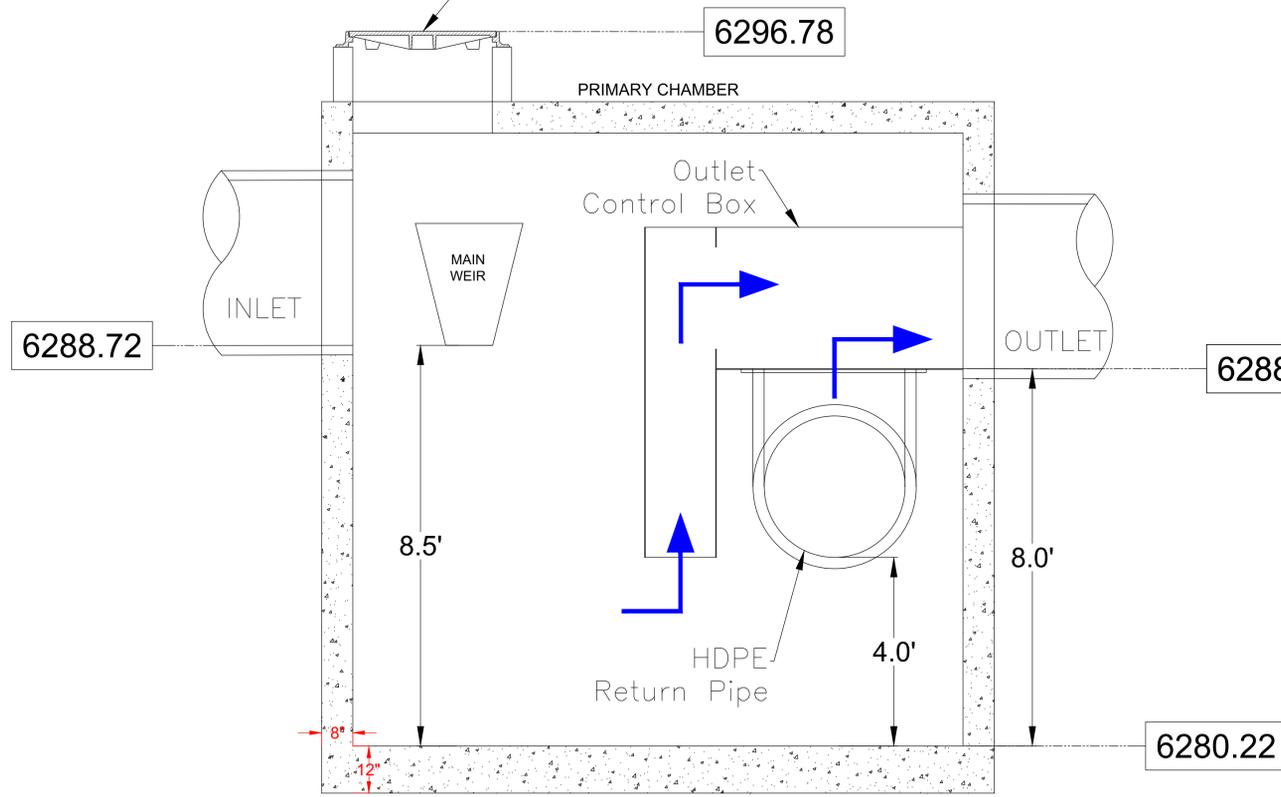
1. USING A SUBMERSIBLE PUMP, PUMP THE CLEAN WATER FROM THE CENTER OF THE MANHOLE DIRECTLY INTO THE EMPTY STORAGE MANHOLE UNTIL THE WATER LEVEL FALLS TO 1 FOOT ABOVE THE SEDIMENT LAYER.
2. REMOVE THE SETTLED SEDIMENT AND REMAINING WATER BY VACUUM TRUCK.
3. CLEAN THE MANHOLE WALLS AND FLUSH OUT THE MANHOLE USING A HIGH PRESSURE HOSE AND REMOVE FLUSHING WATER BY VACUUM TRUCK. MAKE CERTAIN MANHOLE IS CLEAN.
4. CONTAMINATED MATERIAL REMOVED FROM THE MANHOLES MUST BE DISPOSED OF RESPONSIBLY AND LEGALLY BY THE OPERATOR OF THE VACUUM TRUCK.

BAYSEPARATOR INSTALLATION NOTES

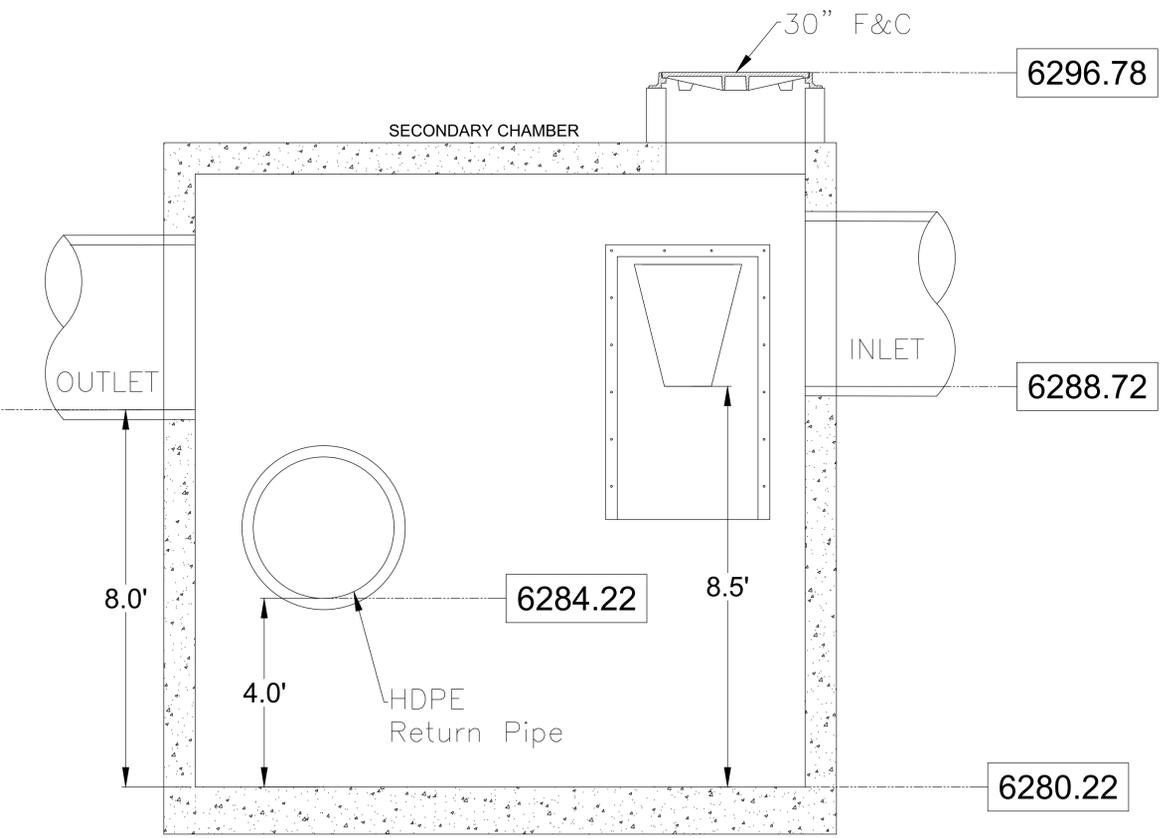
1. EXCAVATION MUST PROVIDE ADEQUATE SPACE TO CONNECT INLET AND OUTLET PIPES TO STORAGE MANHOLE AND BAYSEPARATOR UNIT. INSTALL PRECAST DROP STRUCTURES ON SOLID GROUND AS VERIFIED BY A GEOTECHNICAL ENGINEER.
2. VERIFY THE SUBGRADE ELEVATION AGAINST THE MANHOLE DIMENSIONS AND CONNECTING STORM DRAIN INVERTS.
3. MAKING SURE THE BASES ARE LEVEL AND THE STORAGE MANHOLE OPENINGS ARE ALIGNED WITH THE SEPARATOR UNIT, INSTALL PRIMARY AND STORAGE MANHOLES. INSTALL WATERTIGHT GASKETS ON BASE UNITS AND COAT WITH LUBRICATING GREASE (IF REQUIRED). INSTALL ADDITIONAL MANHOLE SECTIONS AS REQUIRED. SEAL LIFT HOLES WITH NON-SHRINK GROUT.
4. BACKFILL BASE SECTIONS OF MANHOLES TO INVERT OF STORAGE MANHOLE CONNECTING PIPES. USING APPROVED BACKFILL MATERIAL, BACKFILL AND COMPACT IN 8 INCH LIFTS. BACKFILL AND COMPACTION SHOULD BE MONITORED BY A GEOTECHNICAL ENGINEER.
5. INSTALL BAYSEPARATOR UNIT AND CONNECTING PIPES. SEAL ALL CONNECTING JOINTS AND INSTALL SEPARATOR HDPE REDUCER/ADAPTER. CUT EXCESS LENGTH OFF CONNECTING PIPES INSIDE STORAGE MANHOLE.
6. BACKFILL SEPARATOR UNIT AND MANHOLES. AREAS NOT ACCESSIBLE TO COMPACTION EQUIPMENT MUST BE BACKFILLED WITH #57, #7, OR PEA GRAVEL.
7. INSTALL AND SET MANHOLE COVER GRADE ADJUSTMENT RINGS AS NECESSARY.
8. INSTALL AND SET MANHOLE FRAME AND COVER UNITS.



| SYSTEM | NORTHSIDE |
|----------------------|-----------|
| WQ FLOW RATE (CFS) | 24 |
| PEAK FLOW RATE (CFS) | 65.5 |
| INLET PIPE | 54" |
| INLET INVERT | 6288.72 |
| OUTLET PIPE | 54" |
| OUTLET INVERT | 6288.22 |
| RIM ELEVATION | 6296.78 |
| WIDTH (FT) | 8' |
| LENGTH (FT) | 8' |
| INSIDE HEIGHT (FT) | 16' |



SECTION B-B



SECTION A-A

CROSSROADS MIXED USE FILING NO. 1

COLORADO SPRINGS, CO

DATE: 12/21/22 DRAWN: PR PROJECT #: S295850 CHECKED: PR

| DATE | DRWN | CHKD | DESCRIPTION |
|------|------|------|-------------|
| | | | |

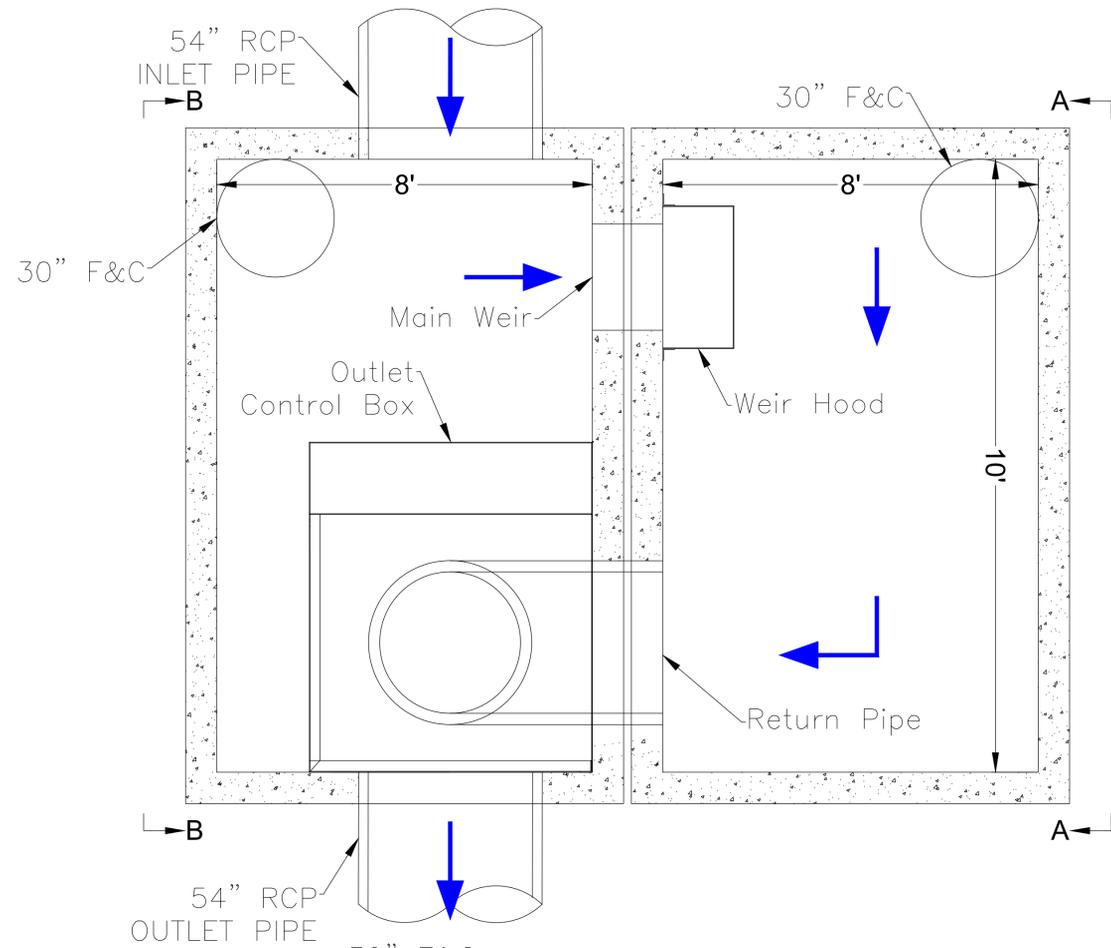
BaySeparator
Stormwater Treatment System

4640 TRUEMAN BLVD
HILLIARD, OH 43026

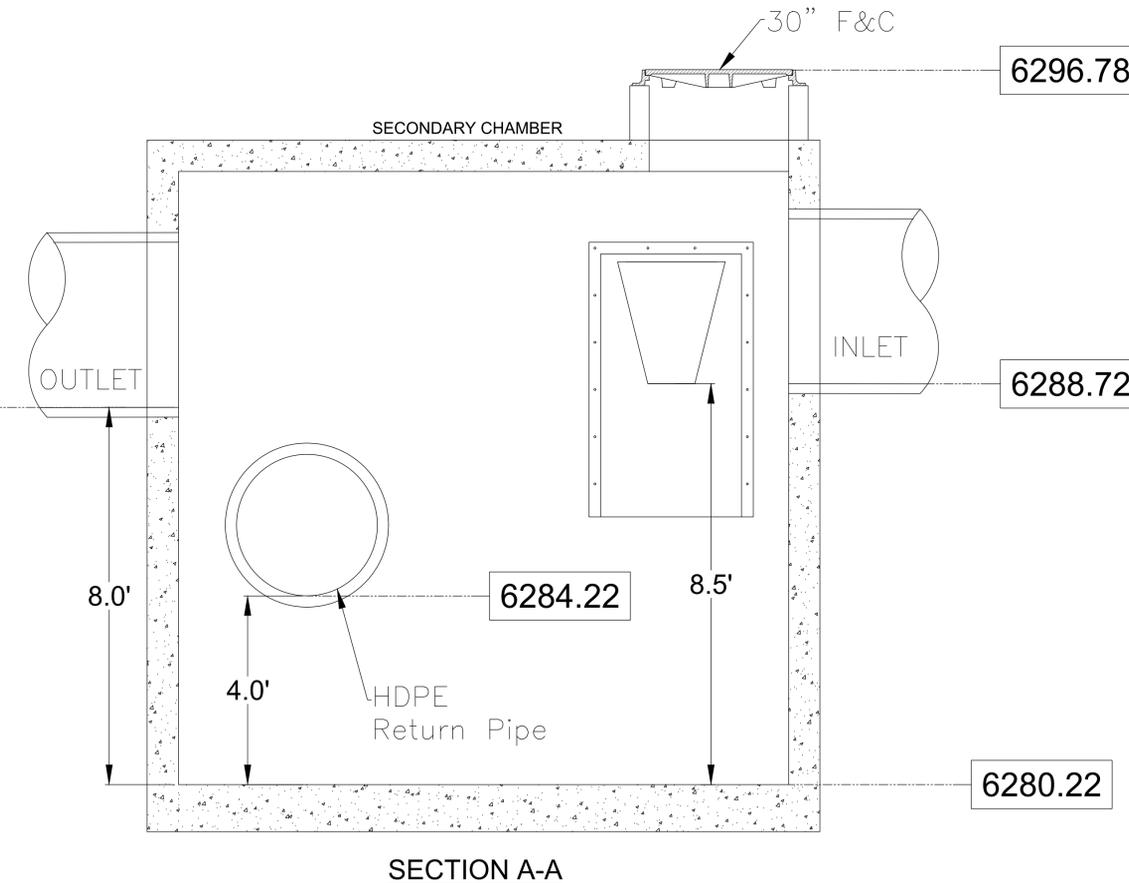
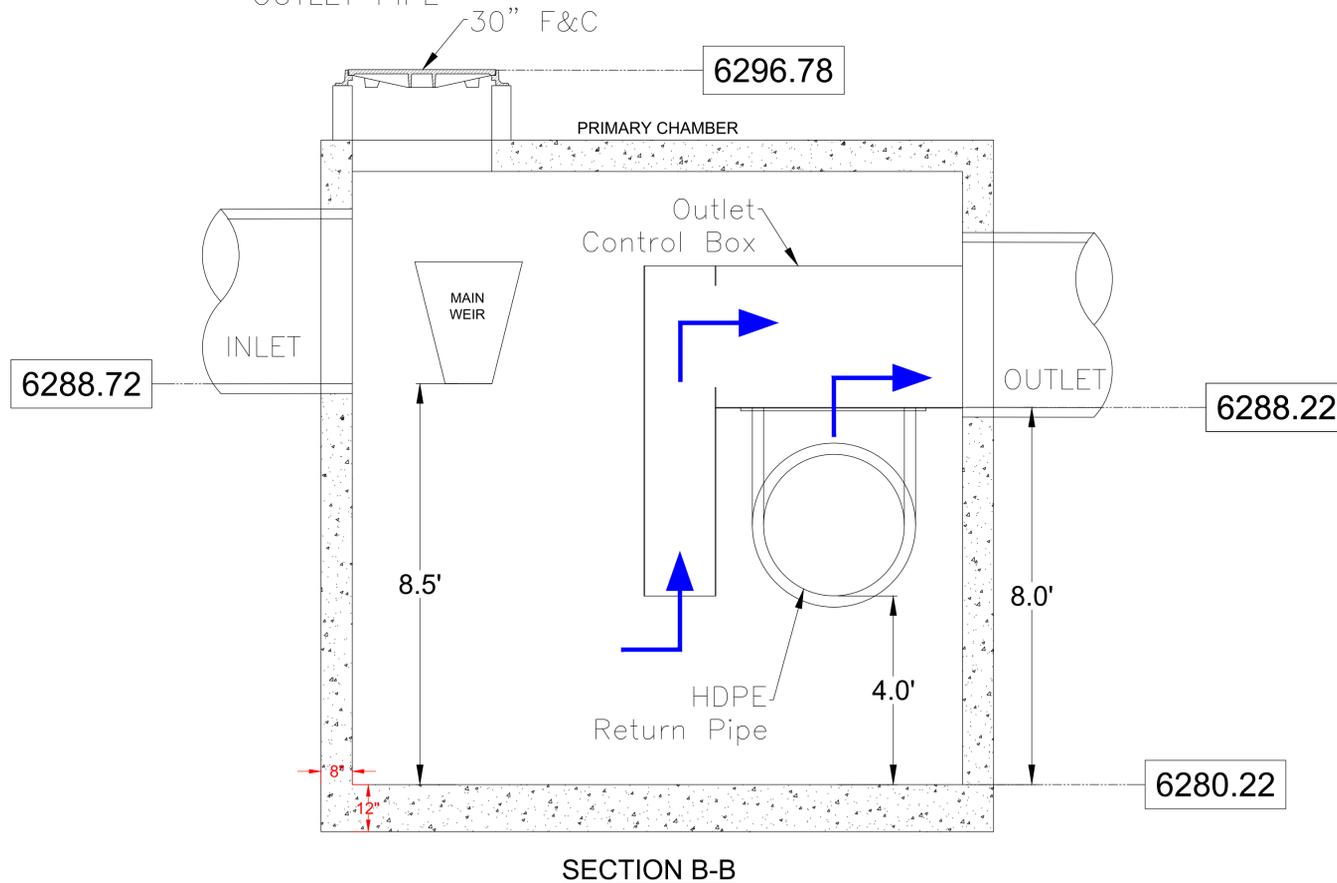
NOT TO SCALE

2 SHEET
OF 3

THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS UNDER THE DIRECTION OF THE SITE DESIGN ENGINEER OR OTHER PROJECT REPRESENTATIVE. THE SITE DESIGN ENGINEER SHALL REVIEW THIS DRAWING PRIOR TO CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE SITE DESIGN ENGINEER TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE LAWS, REGULATIONS, AND PROJECT REQUIREMENTS.



| SYSTEM | EASTSIDE |
|----------------------|----------|
| WQ FLOW RATE (CFS) | 32 |
| PEAK FLOW RATE (CFS) | 112 |
| INLET PIPE | 54" |
| INLET INVERT | 6288.72 |
| OUTLET PIPE | 54" |
| OUTLET INVERT | 6288.22 |
| RIM ELEVATION | 6296.78 |
| WIDTH (FT) | 8' |
| LENGTH (FT) | 10' |
| INSIDE HEIGHT (FT) | 16' |



CROSSROADS MIXED USE FILING NO. 1

COLORADO SPRINGS, CO

DATE: 12/21/22 DRAWN: PR

PROJECT #: S295850 CHECKED: PR

| DATE | DRWN | CHKD | DESCRIPTION |
|------|------|------|-------------|
| | | | |

BaySeparator
Stormwater Treatment System

4640 TRUEMAN BLVD
HILLIARD, OH 43026

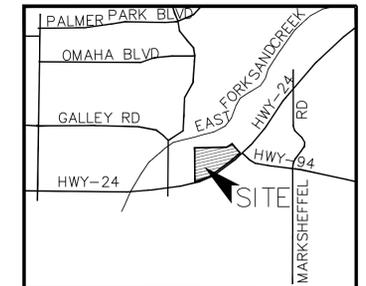
ADS

NOT TO SCALE

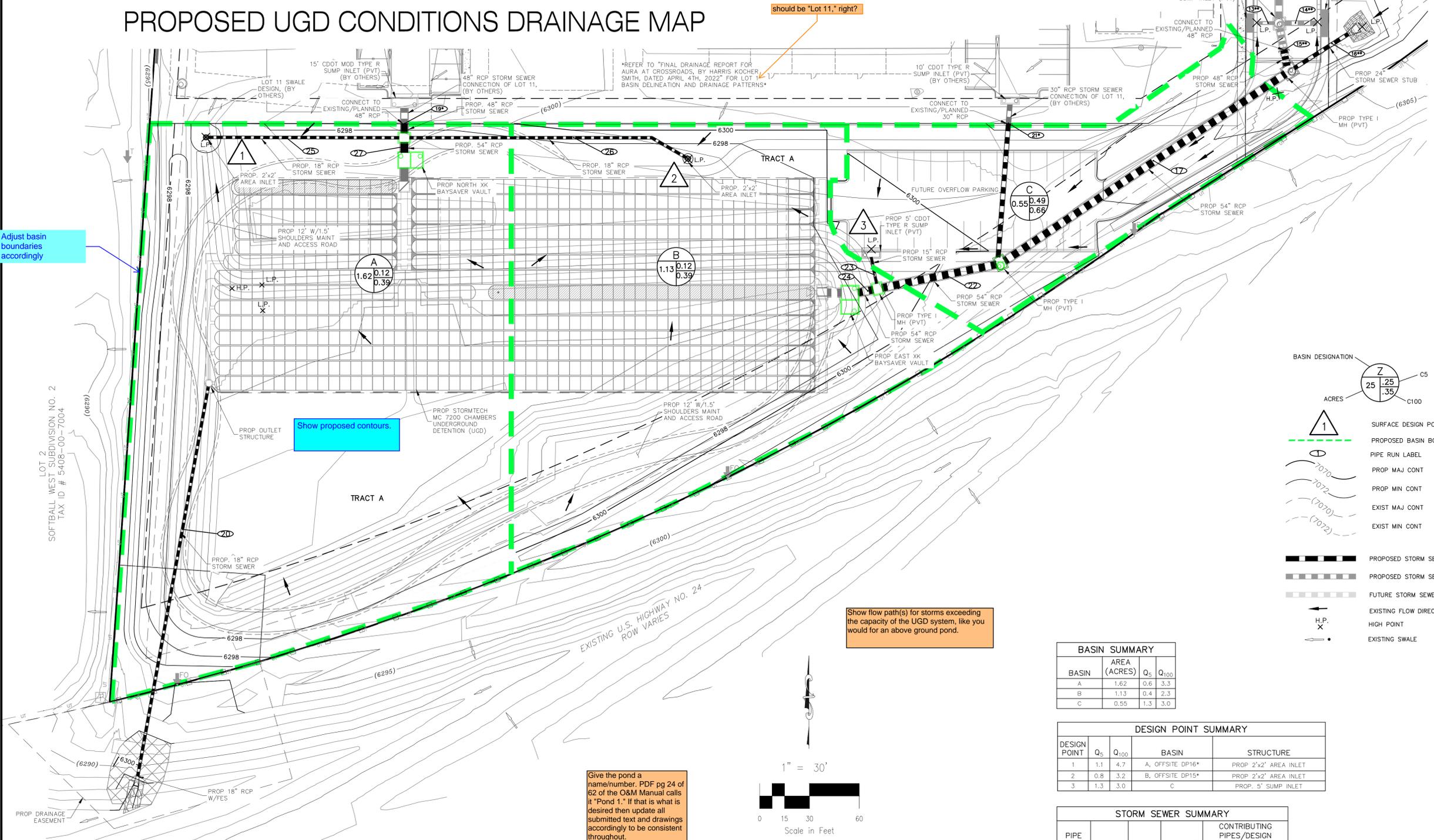
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**FINAL DRAINAGE REPORT
CROSSROADS MIXED USE FILING NO. 1
UNDERGROUND DETENTION
– PROPOSED DRAINAGE MAP**

CROSSROADS MIXED USE FILING NO. 1 FOR UNDERGROUND DETENTION (UGD) PROPOSED UGD CONDITIONS DRAINAGE MAP



VICINITY MAP
N.T.S.



LEGEND

- SITE BOUNDARY
- PROPOSED UTILITY EASEMENT
- PROPOSED DRAINAGE EASEMENT
- PROPOSED LANDSCAPE EASEMENT
- LOT LINE
- ST - STORM SEWER LINE
- UE - EX. UNDERGROUND ELECTRIC LINE
- SS - EX. SANITARY SEWER LINE
- WL - EX. WATER LINE
- ST - EX. STORM SEWER LINE
- 9 - LOT NUMBER
- CV - EX. IRRIGATION VALVE
- ST - EX. STORM INLET
- G - EX. GAS TEST NODE
- D - EX. TELEPHONE PEDESTAL
- EV - EX. ELECTRIC VAULT
- SM - EX. SANITARY MANHOLE
- WV - EX. WATER VALVE
- PROPOSED RIPRAP
- EMERGENCY OVERFLOW DIRECTION
- L.P. X - LOW POINT
- PROPOSED SWALE

BASIN DESIGNATION

ACRES

| | |
|----|------|
| Z | C5 |
| 25 | 0.25 |
| | 0.35 |

DESIGN POINT

1 - SURFACE DESIGN POINT

2 - PROPOSED BASIN BOUNDARY

3 - PIPE RUN LABEL

4 - PROP MAJ CONT

5 - PROP MIN CONT

6 - EXIST MAJ CONT

7 - EXIST MIN CONT

PROPOSED STORM SEWER PIPE

8 - PROPOSED STORM SEWER PIPE (OTHERS)

9 - FUTURE STORM SEWER PIPE

EXISTING FLOW DIRECTION ARROW

H.P. X - HIGH POINT

EXISTING SWALE

BASIN SUMMARY

| BASIN | AREA (ACRES) | Q ₅ | Q ₁₀₀ |
|-------|--------------|----------------|------------------|
| A | 1.62 | 0.6 | 3.3 |
| B | 1.13 | 0.4 | 2.3 |
| C | 0.55 | 1.3 | 3.0 |

DESIGN POINT SUMMARY

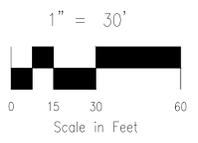
| DESIGN POINT | Q ₅ | Q ₁₀₀ | BASIN | STRUCTURE |
|--------------|----------------|------------------|------------------|-----------------------|
| 1 | 1.1 | 4.7 | A, OFFSITE DP16* | PROP 2'x2' AREA INLET |
| 2 | 0.8 | 3.2 | B, OFFSITE DP15* | PROP 2'x2' AREA INLET |
| 3 | 1.3 | 3.0 | C | PROP. 5' SUMP INLET |

STORM SEWER SUMMARY

| PIPE RUN | Q ₅ | Q ₁₀₀ | PIPE SIZE | CONTRIBUTING PIPES/DESIGN POINTS |
|----------|----------------|------------------|-----------|----------------------------------|
| 15** | 48.0 | 93.7 | 48" RCP | **FDR |
| 16** | 10.8 | 19.6 | 24" RCP | **FDR |
| 17 | 57.0 | 110.1 | 54" RCP | PR15**, PR16** |
| 19* | 35.4 | 65.5 | 48" RCP | *FDR |
| 21* | 2.1 | 4.2 | 30" RCP | *FDR |
| 22 | 56.5 | 109.4 | 54" RCP | PR17, PR21* |
| 23 | 1.3 | 3.0 | 15" RCP | DP3 |
| 24 | 57.7 | 112.0 | 54" RCP | PR22, PR23 |
| 25 | 1.1 | 4.7 | 18" RCP | DP1 |
| 26 | 0.8 | 3.2 | 18" RCP | DP2 |
| 27 | 37.5 | 74.6 | 54" RCP | PR19*, PR25, PR26 |
| 20 | 1.2 | 11.4 | 18" RCP | DETENTION POND OUTFALL |

FULL SPECTRUM DETENTION POND 1 (PRIVATE)

| | |
|-------------------------|-------------|
| WQ VOLUME | 0.863 AC-FT |
| EURV VOLUME | 3.295 AC-FT |
| 100 YR STORAGE VOLUME | 4.668 AC-FT |
| 100 YR WATER SURFACE EL | 6294.96 |



**FDR FOR AURA AT CROSSROADS, PREPARED BY HKS, DATED APRIL 4, 2022
**FDR CROSSROADS MIXED USE FILING NO.1, PREPARED BY MS CIVIL CONSULTANTS INC., DATED APRIL 2022 PCD FILING NO:

PROPOSED DRAINAGE MAP
CROSSROADS MIXED USE FILING NO.1 FOR UGD
JOB NO. 18-003
DATE PREPARED: DECEMBER 20, 2022
DATE REVISED:

212 N. WAHSATCH AVE., STE 305
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485