PRELIMINARY/FINAL DRAINAGE REPORT

SPACE VILLAGE FILING NO. 3

SOUTHWEST CORNER OF PETERSON BOULEVARD & SPACE VILLAGE AVENUE

EL PASO COUNTY, COLORADO

PREPARED FOR:

Space Village Retail, LLC 90 South Cascade Avenue, Suite 1500 Colorado Springs, CO 80903 (719) 448-4034 Contact: Danny Mientka

PREPARED BY:

Olsson Associates 1880 Fall River Drive, Suite 200 Loveland, CO 80538 (970) 461-7733 Contact: Josh Erramouspe

October 2, 2018 PCD Project No. SP-17-009 & SF-18-016 Olsson Project No. 017-1754



ENGINEER'S STATEMENT

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability cause by any negligent acts, errors, or omissions on my part in preparing this report.



Josh Erramouspe Colorado Licensed Professional Engineer No. 42141

DEVELOPER'S STATEMENT

I, the developer, have read and will comply with all the requirements specified in this drainage report and plan.

Space Village Retail, LLC hereby certifies that the drainage facilities for Space Village Filing No. 3 shall be constructed according to the design presented in this report. I understand that El Paso County does not and will not assume liability for the drainage facilities designed and/or certified by my engineer and that El Paso County reviews drainage plans pursuant to Colorado Revised Statutes, Title 30, Article 28; but cannot, on behalf of Space Village Filing No. 3 guarantee that final drainage design review will absolve Space Village Retail, LLC and/or their successors and/or assigns of future liability for improper design. I further understand that approval of the final plat does not imply approval of my engineer's drainage design.

Space Village Retail, LLC

Ву: _____

Title: _____

Address:

EL PASO COUNTY

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual, and Land Development Code as amended.

Jenifer Irvine, PE Count Engineer/ECM Administrator Date

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Provide a explanation regarding the purpose of this report. ie. It should state that no development or redevelopment are proposed at this time. This FDR is written for the benefit of the plat.
 The SINE is a 1.132-acre parcel situated in the northwest quarter of Section 17, Township 14 South, Range Also you need to state that redevelopment of any lots State of Colorado. The SITE is be will require compliance with current water quality and Lot 1, Space Village

Filing No. 2 an south by Lot 1 Flood Zone "X Map 08041C0

2.0 GENERAL EXISTING DRAINAGE CHARACTERISTICS

2.1 Soils Condition

Existing soils within the SITE consist entirely of Truckton sandy loam. The NRCS hydrologic soil classification assigned to this type of soil is Type A. Refer to Appendix A for NRCS web soil survey mapping.

2.2 Existing Site Conditions

The SITE, which lies within the Sand Creek Drainage Basin, contains strip retail and undeveloped ground. Portions of the property are also being used as a parking lot for the surrounding developments. The existing strip retail, with its associated drives/parking, occupies 2.36 acres. The remaining 1.77 acres has been reserved for future commercial development. Asphalt paving covers approximately 50% of the SITE. The remaining area is covered by buildings, grass and landscape gravel.

2.3 Existing Drainage Conditions

The existing drainage on the site generally flows from northeast to southwest with slopes ranging from 1%-10%. Refer to the Existing Drainage Basin Map in Appendix C.

Basin B-1 encompasses approximately 3.07 acres. The basin is comprised of approximately 30% grassy landscape and 70% asphalt pavement. Runoff (Q_5 =9.12 cfs, Q_{100} =18.07 cfs) flows into the existing 24" RCP culvert located on the west side of the SITE. The basin runoff is discharged from the 24" culvert on the west side of the site to the west side of Peterson Boulevard.

Basin B-2 encompasses approximately 1.12 acres south of the proposed Lot 1. The basin is comprised of mostly asphalt pavement, with small portions of roof & landscape. This basin was assumed to be 95% impervious per the El Paso County Drainage Criteria Manual. Runoff (Q_5 =4.39 cfs, Q_{100} =7.98 cfs) flows into the existing 24" RCP culvert located on the west side of the SITE. The basin runoff is discharged from the 24" culvert on the west side of the site to the west side of Peterson Boulevard. The drainage patterns within the basin will not be altered after the development of Lot 1 and Lot 3; however, the basin has been analyzed here since the runoff from this basin will be routed through the 24" RCP culvert.

Basin B-3 encompasses an additional 1.51 acres of land to the north & west of the SITE. Currently stormwater surface runoff (Q_5 =4.17 cfs, Q_{100} =8.40 cfs) discharges to the existing 24" RCP culvert located on the west side of the SITE. Again, the drainage patterns associated with this additional 1.51 acres will not be altered after Lot 1 and Lot 3 are developed. We have included the runoff from this acreage in our sizing calculations for the proposed grated inlet that will replace the existing 24" RCP flared end section located on the west side of the SITE.

Design Point 4 is located at the inlet side of the existing 24" culvert that runs below Peterson Boulevard and outfalls on the west side of that road to property owned by Peterson AFB. Runoff generated within Basins B-1, B-2 and B-3 converges at this design point. In the existing



condition the total flows at this design point are Q_5 =17.25 cfs, Q_{100} =26.37 cfs. It should be noted that this culvert will experience inundation during the 100 YR event since the calculated runoff is greater than the full-flow capacity of the culvert.

3.0 PROPOSED DRAINAGE CONDITIONS

3.1 Proposed Basin Description

Upon future development of Lot 1 and Lot 3, developed runoff from Basin B-1 will drain through a private storm sewer system to a private on-site detention facility. The detention facility will discharge to the existing 24" RCP pipe at the west side of the SITE and will ultimately outfall to the land on the west side of Peterson Boulevard. Refer to the Drainage Basin Map for more detail on basin delineation. A detailed breakdown of the runoff generated on-site is described as follows:

Basin B-1 is the on-site portion of flow that will be routed to a detention facility. This basin encompasses approximately 3.07 acres and is assumed to be commercial development. The El Paso County Drainage Criteria Manual states that commercial development shall be assumed to be 95% impervious. Runoff generated in Basin B-1 in the proposed condition (Q_5 =12.08 cfs, Q_{100} =22.04 cfs) was calculated using the rational method and will be routed to a detention facility via overland flow, curb and gutter, and private storm sewer if necessary.

See Section 2.3 for descriptions of Basin B-2 and B-3 as these basins will not change with the development of Lot 1 and Lot 3. Both basins will bypass the future detention facility and will discharge runoff directly to the existing 24" culvert under Peterson Boulevard.

Design Point 4 will see significantly less flow in the proposed condition compared to the existing condition because a detention facility will be designed to capture and attenuate the flows from Basin B-1 prior to discharging to the existing 24" culvert. The 24" RCP has a maximum capacity of 22.62 cfs (See appendix for Flowmaster calculation) without headwater, and the routed flow from basins B-2 and B-3 is Q_5 =8.34 cfs, Q_{100} =15.97 cfs. To that end, the culvert has capacity for an additional 6.65 cfs in the 100 YR event before it becomes inundated. Therefore, Basin B-1 could feasibly contribute 6.65 cfs to the culvert in the 100 YR event, however, a full spectrum detention pond will capture runoff from Basin B-1 and the resultant pond release rate will be less than 6.65 cfs in the 100 YR event, the ultimate discharge to the land owned by Peterson AFB (on the west side of Peterson Boulevard) will be less than what is currently being discharged to that same land today.

3.2 Detention Facility

Upon the development of either Lot 1 or Lot 3, a privately-owned detention facility will be constructed on Lot 1 to provide water quality, attenuation of the EURV, and also attenuation of the 100 YR runoff rate associated with Basin B-1. The pond should be designed and constructed a single time to handle developed flows from the entirety of basin B-1, rather than being designed/constructed in a phased approach. The water quality capture volume (WQCV), EURV release rate, and 100 YR release rate will be sized using the latest version of UD-Detention from Urban Drainage for full spectrum detention facilities. The detention facility and site design for development within Basin B-1 shall adhere to the four-step process as detailed in Appendix I Section 1.7.2 of the ECM. While the pond will be owned by the owner of Lot 1, the pond will be granted around the pond to allow the owner of Lot 3 to construct the pond improvements (in the case that Lot 3 develops before Lot 1).



4.0 DRAINAGE FEES/DRAINAGE BASIN PLANNING STUDY

This project is located within the Sand Creek Drainage Basin and is in general conformance with the Sand Creek DPBS. Future development of Lot 1 and Lot 3 will need to comply with the DBPS as well. There do not appear to be any drainage improvements identified in the DBPS associated with this land. The development's drainage fees are as follows and will be paid at the time of platting:

Drainage Fee: \$17,197 per impervious acre x (0.95 x 4.13ac) = \$67,472.43

Bridge Fee: \$5,210 per impervious acre x (0.95 x 4.13ac) = \$20,441.44

* These fees are based on the 2018 fee schedule and are due prior to recordation of the plat.

5.0 SUMMARY

In summary, assuming vacant portions of the SITE will be developed with commercial uses, the developed drainage patterns within the SITE will not be altered compared to existing drainage patterns. The ultimate discharge point will remain the same for this site (the 24" RCP culvert crossing Peterson Boulevard). As a part of any future development within Basin B-1, runoff generated within this basin will be detained in a full-spectrum detention facility and released at a controlled rate to the aforementioned existing 24" RCP culvert. If future development within Basin B-1 adheres to the recommendations presented in this drainage report, the runoff discharged to the existing 24" culvert under Peterson Boulevard will be less that what is discharged to the same culvert in the existing condition. A full, site-specific drainage report will need to be submitted to El Paso County for review and approval in conjunction with a Site Development Plan for either Lot 1 or Lot 3.

6.0 REFERENCES

"Drainage Criteria Manual Volume 1." Colorado Springs, CO (1994)

"Urban Storm Drainage." Criteria Manual Volume 1 (2017)

"Urban Storm Drainage." Criteria Manual Volume 2 (2017)

"Urban Storm Drainage." Criteria Manual Volume 3 (2010)

Sand Creek Drainage Basin Planning Study



Basin Name	Basin Description	Paved 100% (acres)	Building 90% (acres)	Gravel 40% (acres)	Landscape 2% (acres)	Total Area (ac)	C5	C100	Percent Imperviousness
B-1	Flows to Detention Pond	2.20	-	-	0.87	3.07	0.67	0.79	72.1%
B-2	Flows Around Detention Pond	1.06	-	-	0.05	1.12	0.86	0.93	95.2%
B-3	Within ROW	1.02	-	-	0.48	1.50	0.64	0.76	67.9%
					TOTAL	5.69	0.70	0.81	76%

		OVERLAN	ID FLO	W	GUTTER FLOW 1			GUTTER FLOW 2			Total T	Check T	Final T
BASIN	L1	S1	C E	Ti	L2	V	T2	L3	V	Т3	(min)		(min)
	(ft)	(%)	05	(min)	(ft)	(ft/s)	(min)	(ft)	(ft/s)	(min)	(mm)	Ed 0-9	(mm)
B-1	100.0	2.30%	0.67	6.01	357.0	2.9	2.1				8.06	12.54	8.06
B-2	100.0	1.00%	0.86	4.40	399.0	2.2	3.0				7.40	12.77	7.40
B-3	64.0	2.81%	0.64	4.81	579.0	2.5	3.8				8.63	13.57	8.63

	Basin Characteristics Intensities												
BASIN NAME	Description	AREA (acres)	C5	C100	Tc* (min)	l5 (in/hr)	l 100 (in/hr)	Q 5-yr (cfs)	Q 100-yr (cfs)				
B-1	Flows to Detention Pond	3.07	0.67	0.79	8.1	4.45	7.47	9.12	18.07				
B-2	Flows Around Detention Pond	1.12	0.86	0.93	7.4	4.58	7.69	4.39	7.98				
B-3	Within ROW	1.50	0.64	0.76	8.6	4.35	7.30	4.17	8.40				

KG 692

EXISTING HYDROLOGIC CALCULATIONS



1880 Fall River Drive Suite 200 Loveland, CO 80538 TEL 970.461.7733

Basin Name	Basin Description	Paved 100% (acres)	Building 90% (acres)	Gravel 40% (acres)	Landscape 2% (acres)	Total Area (ac)	C5	C100	Percent Imperviousness
B-1	Flows to Detention Pond					3.07	0.81	0.88	95.0%
B-2	Flows Around Detention Pond	1.06	-	-	0.05	1.12	0.86	0.93	95.2%
B-3	Within ROW	1.02	-	-	0.48	1.50	0.64	0.76	67.9%
					TOTAL	5.69	0.77	0.86	88%

BASIN		OVERLAN	W	GUTTER FLOW 1			GUTTER FLOW 2			Total T	Chock T	Final T	
	L1	S1	C5	Ti	L2	V	T2	L3	V	Т3	(min)		(min)
	(ft)	(%)	03	(min)	(ft) (ft/s)	(min)	(ft)	(ft/s)	(min)	(11111)	Eq 0-5	(11111)	
B-1	100.0	1.93%	0.81	4.26	354.0	3.1	1.9				6.17	12.52	6.17
B-2	100.0	1.00%	0.86	4.40	399.0	2.2	3.0				7.40	12.77	7.40
B-3	64.0	2.81%	0.64	4.81	579.0	2.5	3.8				8.63	13.57	8.63

	Basin Characteris	stics				Inten	sities	Sub-basin	
Basin Name	Description	Area (acres)	C5	C100	Tc* (min)	l5 (in/hr)	l 100 (in/hr)	Q 5-yr (cfs)	Q 100-yr (cfs)
B-1	Flows to Detention Pond	3.07	0.81	0.88	6.17	4.85	8.15	12.08	22.04
B-2	Flows Around Detention Pond	1.12	0.86	0.93	7.40	4.58	7.69	4.39	7.98
B-3	1.50	0.64	0.76	8.63	4.35	7.30	4.17	8.40	



PROPOSED HYDROLOGIC CALCULATIONS



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Land Har and Surface	Demonst						Runoff Co	efficients					
Characteristics	Impervious	2-y	ear	5-y	ear	10-1	year	25-	/ear	50-1	year	100-	year
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Historic Flow Analysis													
Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when	45												
landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

Table 6-6. Runoff Coefficients for Rational Method (Source: UDFCD 2001)

3.2 Time of Concentration

One of the basic assumptions underlying the Rational Method is that runoff is a function of the average rainfall rate during the time required for water to flow from the hydraulically most remote part of the drainage area under consideration to the design point. However, in practice, the time of concentration can be an empirical value that results in reasonable and acceptable peak flow calculations.

For urban areas, the time of concentration (t_c) consists of an initial time or overland flow time (t_i) plus the travel time (t_i) in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For nonurban areas, the time of concentration consists of an overland flow time (t_i) plus the time of travel in a concentrated form, such as a swale or drainageway. The travel portion (t_i) of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or drainageway. Initial time, on the other hand, will vary with surface slope, depression storage, surface cover, antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. The time of concentration is represented by Equation 6-7 for both urban and non-urban areas.



	MAP L	EGEND		MAP INFORMATION
Area of Int	terest (AOI)	300	Spoil Area	The soil surveys that comprise your AOI were mapped at
	Area of Interest (AOI)	۵	Stony Spot	1:24,000.
Soils		0	Very Stony Spot	Warning: Soil Man may not be valid at this scale
	Soil Map Unit Polygons	w.	Wet Spot	Warning. Our map may not be valid at this searc.
~	Soil Map Unit Lines	~	Other	Enlargement of maps beyond the scale of mapping can cause
	Soil Map Unit Points		Special Line Features	line placement. The maps do not show the small areas of
Special	Point Features	Water Fea	itures	contrasting soils that could have been shown at a more detailed
<u></u>	Biowout	~	Streams and Canals	Scale.
×	Borrow Pit	Transport	ation	Please rely on the bar scale on each map sheet for map
×	X Clay Spot HI Rails Image: Closed Depression Intersta Gravel Pit Image: Closed Depression Image: Closed Depression Image: Closed Depression		Rails	measurements.
\diamond			Interstate Highways	Source of Map: Natural Resources Conservation Service
X			US Routes	Web Soil Survey URL:
0 0 0			Major Roads	Coordinate System: Web Mercator (EPSG:3857)
0			Local Roads	Maps from the Web Soil Survey are based on the Web Mercator
Α.	Lava Flow	Backgrou	nd	projection, which preserves direction and shape but distorts
عليه	Marsh or swamp	No.	Aerial Photography	Albers equal-area conic projection, should be used if more
奈	Mine or Quarry			accurate calculations of distance or area are required.
0	Miscellaneous Water			This product is generated from the USDA-NRCS certified data as
0	Perennial Water			of the version date(s) listed below.
\sim	Rock Outcrop			Soil Survey Area: El Paso County Area. Colorado
+	Saline Spot			Survey Area Data: Version 14, Sep 23, 2016
°°.	Sandy Spot			Soil map units are labeled (as space allows) for map scales
-	 Sandy Spot Severely Eroded Spot Sinkhole 			1:50,000 or larger.
۵				Data(e) agrial images were photographed: Jun 3, 2014 Jun 17
s.	 Sinkhole Slide or Slip Sodic Spot 			2014 2014 2014 2014 2014 2014 2014 2014
Ŕ				
				compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

	El Paso County Area, Colorado (CO625)											
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI									
96	Truckton sandy loam, 0 to 3 percent slopes	2.2	100.0%									
Totals for Area of Interest	•	2.2	100.0%									

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

96—Truckton sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 36bf
Elevation: 6,000 to 7,000 feet
Mean annual precipitation: 14 to 15 inches
Mean annual air temperature: 46 to 50 degrees F
Frost-free period: 125 to 145 days
Farmland classification: Prime farmland if irrigated and the product of I (soil erodibility) x C (climate factor) does not exceed 60

Map Unit Composition

Truckton and similar soils: 85 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Truckton

Setting

Landform: Flats Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 8 inches: sandy loam Bt - 8 to 24 inches: sandy loam C - 24 to 60 inches: coarse sandy loam

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 5.7 inches)

Interpretive groups

Land capability classification (irrigated): 2e Land capability classification (nonirrigated): 3e Hydrologic Soil Group: A Ecological site: Sandy Foothill (R049BY210CO) Hydric soil rating: No

Minor Components

Other soils

Percent of map unit: Hydric soil rating: No

Pleasant

Percent of map unit: Landform: Depressions Hydric soil rating: Yes

	worksneet for 24	RCP	Cuivert
Project Description			
Friction Method	Manning Formula		
Solve For	Full Flow Capacity		
Input Data			
Roughness Coefficient		0.013	
Channel Slope	1.	00000	%
Normal Depth		24	in
Diameter		24	in
Discharge		22.62	ft³/s
Results			
Discharge		22.62	ft³/s
Normal Depth		24	in
Flow Area		3.14	ft²
Wetted Perimeter		6.28	ft
Hydraulic Radius		6	in
Top Width		0.00	ft
Critical Depth		1.69	ft
Percent Full		100.0	%
Critical Slope	0.	.00946	ft/ft
Velocity		7.20	ft/s
Velocity Head		0.81	ft
Specific Energy		2.81	ft
Froude Number		0.00	
Maximum Discharge		24.33	ft³/s
Discharge Full		22.62	ft³/s
Slope Full	0.	.01000	ft/ft
Flow Type	SubCritical		
GVF Input Data			
Downstream Depth		0	in
Length		0.00	ft
Number Of Steps		0	
GVF Output Data			
Upstream Depth		0	in
Profile Description			
Profile Headloss		0.00	ft
Average End Depth Over Rise		0.00	%

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 Bentley Systems, Inc.
 Haestad Methods SoluBitontl@efitervMaster V8i (SELECTseries 1) [08.11.01.03]

 27 Siemons Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666
 Page 1 of 2

Worksheet for 24" RCP Culvert

GVF Output Data

Normal Depth Over Rise	100.00	%
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	24	in
Critical Depth	1.69	ft
Channel Slope	1.00000	%
Critical Slope	0.00946	ft/ft



DWG: F:\2017\1501-2000\017-1754\40-Design\Reports\GNCV\Drainage Memo\AutoCAD\C_XBSN_71754.dwg USER: jerra DATE: Aug 29, 2018 6:21pm XREFS: C_XBASE_71754 C_PCONT_71754



DWG: F:\2017\1501-2000\017-1754\40-Design\Reports\GNCV\Drainage Memo\AutoCAD\C_PBSN_71754.dwg USER: jerra DATE: Aug 29, 2018 6:20pm XREFS: C_PBASE_71754 C_XBASE_71754

Markup Summary

dsdlaforce (2)

Subject: Callout Page Label: 3 Lock: Locked Author: dsdlaforce Date: 10/25/2018 10:09:23 AM Color:

Provide a explanation regarding the purpose of this report. ie. It should state that no development or redevelopment are proposed at this time. This FDR is written for the benefit of the plat.

Also you need to state that redevelopment of any lots will require compliance with current water quality and full spectrum criteria. This is in the event lot 2 ever redevelops (basin B2) then the redevelopment will have to provide water quality.

Update. The Drainage Fee in Sand Creek is \$17,751.

Subject: Callout Page Label: 5 Lock: Locked Author: dsdlaforce Date: 10/25/2018 10:09:25 AM Color:
