DRAINAGE LETTER
FOR
STERLING RANCH ROAD AND BRIARGATE PARKWAY INTERIM PLAN

Prepared For:

SR Land, LLC
20 Boulder Crescent, Suite 200
Colorado Springs, CO 80903
(719) 491-3024

December 31, 2021
Project No. 25188.03

Note: This report is associated with CDR-221 and being updated to address the February comments received for that project.

Prepared By:

JR Engineering, LLC
5475 Tech Center Drive, Suite 235
Colorado Springs, CO 80919
719-593-2593

J·R ENGINEERING

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

_____

Mike Bramlett, Colorado P.E. 32314
For and On Behalf of JR Engineering, LLC

DEVELOPER'S STATEMENT:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: SR Land, LLC

By: _____

Title: _____

Address: 20 Boulder Crescent, Suite 200
Colorado Springs, CO 80903

El Paso County:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2 and Engineering Criteria Manual, as amended.

_____ _____

Jennifer Irvine, P.E. Date
County Engineer/ ECM Administrator

Conditions:

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APPENDIX

PURPOSE

This document is the Drainage Letter for Sterling Ranch Road and Briargate Parkway Interim Plan. The purpose of this report is to identify on-site and off-site drainage patterns, storm sewer, culvert and inlet locations, areas tributary to the site, and to safely route developed storm water to adequate outfall facilities during the interim condition of development and the construction of Sterling Ranch Road and Briargate Parkway.

GENERAL SITE DESCRIPTION

GENERAL LOCATION

Sterling Ranch and Briargate Parkway Interim Plan (hereby referred to as the "site") is a proposed development within the Sterling Ranch master planned community with a total area of approximately 376 acres that are presently undeveloped.

The site is located in portions of Section 33 & 34, Township 12 South, Range 65 West of the Sixth Principal Meridian in El Paso County, State of Colorado. The site is bounded by Sand Creek to the west, Sterling Ranch Road cuts through the site, and future development land borders the site to the south, north and east. Refer to the vicinity map in Appendix A for additional information.

DESCRIPTION OF PROPERTY

In the interim condition, the property will be roadway (approximately 17 acres), open space and drainage tracts (approximately 359 acres). The site is comprised of variable sloping grasslands that generally slope(s) downward to the southwest at 1 to 6% towards the Sand Creek tributary basin.

Soils for this project are classified as Blakeland Loamy Sand (8) and Gravelly Sandy Loam (19). These soils are characterized as hydrologic soil types Type A. Group A soils exhibit high infiltration rates when thoroughly wet, and consist mainly of deep, well drained to excessively drained sands or gravelly sands. Pring Coarse Sandy Loam (71) is characterized as Hydrologic Soil Types "B". Group B soils exhibit moderate infiltration rate when thoroughly wet, and consist primarily of deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. Refer to the soil survey map in Appendix A for additional information.

There are no major drainage ways running through the site, although the site is tributary to Sand Creek located immediately to the west of the site. Currently, Kiowa Engineering Corp. is performing studies and plans to address Sand Creek stabilization.

There are no known irrigation facilities located on the project site.

FLOODPLAIN STATEMENT

Based on the FEMA FIRM Maps number 08041C0533G, dated December 7, 2018, the far western portion of the project site that is adjacent to the existing drainage way lies within Zone AE. Zone AE is defined as area subject to inundation by the 1-percent-annual-chance flood event. All of the proposed development lies within Zone X. Zone X is defined as area outside the Special Flood Hazard Area (SFHA) and higher than the elevation of the 0.2-percent-annual-chance (or 500-year) flood. No grading operations are proposed within the Zone AE at this time. FIRM Maps have been presented in Appendix A.

EXISTING DRAINAGE CONDITIONS

MAJOR BASIN DESCRIPTIONS

The site lies within the upper Sand Creek Drainage Basin based **o**n the "Sand Creek Drainage Basin Planning Study" (DBPS) completed by Kiowa Engineering Corporation in January 1993, revised March 1996. The Sand Creek Drainage Basin covers approximately 54 square miles and is divided into 7 major sub-basins. The site is within the respective upper basin Sand Creek sub-basin as shown in Appendix C. The Sand Creek DBPS assumed the Sterling Ranch East of Sand Creek property to have a "single family residential" use for the majority of the site.

The site was also previously studied in the Master Development Drainage Plan (MDDP) for Sterling Ranch prepared by M&S Civil Consultants, INC in October 2018. Excerpts from this report can be found in Appendix C. The Sterling Ranch MDDP assumed a mix of low density to medium density and single family residential lots ranging in size from 0.1 to 1.0 acres for the Sterling Ranch Phase 3 site. The proposed Sterling Ranch master plan is a mix of; school, multi-family, single-family, and parks and open space. The proposed drainage on the site closely follows the approved "Master Development Drainage Plan for Sterling Ranch", (MMDP). The site is tributary to Pond FSD14A, and Pond FSD11B as well as future ponds FSD16A, FSD16B, AND FSD14B.

The site generally drains from north to southwest. Currently, the site is used as pasture land for cattle. Sand Creek is located west of the site running north to south. This reach of drainage conveyance is not currently improved. Currently, Kiowa is performing studies and plans to address Sand Creek stabilization adjacent to the site.

EXISTING SUB-BASIN DRAINAGE

The existing / predeveloped condition of the site was broken into five major basins. The basin and sub-basin delineation is shown in the existing drainage map in Appendix E and is described as follows:

Sub-basin EX1 (Q_5= 24.0cfs, Q_{100}=176.3cfs) is 178.68 acres and 2 percent impervious consists of the northern portion of Sterling Ranch Phase 3. Runoff from this basin sheet flows from the north to south to design point EX1 at the northern edge of future Briargate Parkway.

Sub-basin EX2 (Q_5= 2.8cfs, Q_{100}=20.6cfs) is 14.67 acres and 2 percent impervious and consists the northeast portion of Sterling Ranch Phase 3. Runoff from this basin sheet flows south to design point EX2 located just north of future Briargate Parkway.

Sub-basin EX3 (Q_5= 21.7cfs, Q_{100}=159.2cfs) is 160.58 acres and 2 percent impervious and is located onsite in the central part of the site. Runoff from this basin drains southwest to design point EX3 along the eastern edge of Sand Creek.

Sub-basin EX4 (Q_5= 6.0cfs, Q_{100}=44.3cfs) is 36.46 and is 2 percent impervious and is located on the eastern portion of the site. Runoff from this basin sheet flows southwest to design point EX4 located just east of future Sterling Ranch Road.

PROPOSED DRAINAGE CONDITIONS

PROPOSED SUB-BASIN DRAINAGE

The proposed site was broken into two major basins: Basin A (Briargate Parkway), Basin B (Sterling Ranch Road), and two offsite basins. The proposed basin (and sub-basin) delineation is shown on the drainage basin map within Appendix E and is described as follows.

Basin A1 (Q_5= 10.5 cfs, Q_{100}=21.2cfs) is 4.95 acres and 67 percent impervious and is comprised of Briargate Parkway. Runoff from this basin drains to design point 1, an on grade inlet at the northeast corner of the basin. Collected runoff is piped east to pond FSD16 and will outfall to Sand Creek.

Basin A2 (Q_5= 10.4 cfs, Q_{100}=21.1 cfs) is 4.97 acres and 68 percent impervious is comprised of Briargate Parkway. Runoff from this basin drains to design point 2, an on grade inlet on the southeast corner of the basin. Collected runoff is piped east to pond FSD16 and will outfall to Sand Creek.

Basin A3 (Q_5= 5.1 cfs, Q_{100}=10.5 cfs) is 2.01 acres and 62 percent impervious is comprised of Briargate Parkway. Runoff from this basin drains to an on grade inlet located at design point 6 in confluence with uncaptured upstream flows from basin A2. The flow will be piped to northeast pond FSD16 and will outfall to Sand Creek.

Basin A4 (Q_5= 4.1 cfs, Q_{100}=8.5 cfs) is 1.63 acres and 66 percent impervious is comprised of Briargate Parkway. Runoff from this basin drains to an on grade inlet located at design point 4 in

confluence with uncaptured upstream flows from basin A1. The flow will be piped to future detention pond FSD16 and will outfall to Sand Creek.

Basin B1 (Q_5= 5.2 cfs, Q_{100}=10.7 cfs) is 1.9 acres and 65 percent impervious is comprised of Sterling Ranch Road. Runoff from this basin drains to an on grade inlet at design point 8 in confluence with uncaptured upstream flows from basin A4. Collected runoff is piped southwest then conveyed via a proposed swale to pond FSD14A and will outfall to Sand Creek.

Basin B2 (Q_5= 5.1 cfs, Q_{100}=10.9 cfs) is 2.06 acres and 60 percent impervious is comprised of Sterling Ranch Road. Runoff from this basin drains to an on grade inlet at design point 7. Collected runoff is piped southwest then conveyed via a proposed swale to pond FSD14A and will outfall to Sand Creek.

Basin B3 (Q_5= 3.3 cfs, Q_{100}=6.8 cfs) is 1.27 acres and 64 percent impervious is comprised of Sterling Ranch Road. The runoff from this basin drains to an on grade inlet located at design point 11 in confluence with uncaptured upstream flows from basin B1. The flow will be piped northwest then conveyed via a proposed swale to detention pond FSD14A and will outfall to Sand Creek.

Basin B4 (Q_5= 3.3 cfs, Q_{100}=6.8 cfs) 1.33 acres and 61 percent impervious is comprised of Sterling Ranch Road. The runoff from this basin drains to an on grade inlet located at design point 10 in confluence with uncaptured upstream flows from basin B2. The flow will be piped northwest then conveyed via the proposed swale to detention pond FSD14A and will outfall to Sand Creek.

Basin B5 (Q_5= 2.3cfs, Q_{100}=4.9 cfs) 0.89 acres and 61 percent impervious is comprised of Sterling Ranch Road. The runoff from this basin drains to an on grade inlet located at design point 15 in confluence with uncaptured upstream flows from basin B3. The flow will be piped northwest then conveyed via a proposed swale to detention pond FSD14A and will outfall to Sand Creek.

Basin B6 (Q_5= 2.5 cfs, Q_{100}=5.2 cfs) 0.91 acres and 63 percent impervious is comprised of a Sterling Ranch Road. The runoff from this basin drains to an on grade inlet located at design point 14 in confluence with uncaptured upstream flows from basin B4. The flow will be piped northwest then conveyed via a proposed swale to detention pond FSD14A and will outfall to Sand Creek.

Basin B7 (Q_5= 2.4 cfs, Q_{100}=5.3 cfs) is 1.08 acres and 52 percent impervious is comprised of Sterling Ranch Road. The runoff from basin B7 drains to an on grade inlet located at design point 19 in confluence with uncaptured upstream flows from basin B5. The flow will be piped northwest then conveyed via a proposed swale to detention pond FSD14A and will outfall to Sand Creek.

Basin B8 (Q_5= 2.9 cfs, Q_{100}=6.2 cfs) is 1.16 acres and 58 percent impervious is comprised of Sterling Ranch Road. Runoff from basin B8 drains to an on grade inlet located at design point 18 in

confluence with uncaptured upstream flows from basin B6. The flow will be piped north then conveyed via a proposed swale to detention pond FSD14A and will outfall to Sand Creek.

Basin B9 (Q_5= 4.3 cfs, Q_{100}=9.7 cfs) is 1.98 acres and 51 percent impervious is comprised of Sterling Ranch Road. Runoff from basin B9 drains to a sump inlet located at design point 23 in confluence with uncaptured upstream flows from basin B7. The flow will be piped to detention pond FSD14A and will outfall to Sand Creek.

Basin B10 (Q_5= 5.0 cfs, Q_{100}=11.0 cfs) is 2.19 acres and 53 percent impervious is comprised of Sterling Ranch Road. Runoff from basin B10 drains to a sump inlet located at design point 22 in confluence with uncaptured upstream flows from basin B8. The flow will be piped to detention pond FSD14A and will outfall to Sand Creek.

Basin B11 (Q_5=16.6cfs, Q_{100}=121.6cfs) is 126.23 acres and 2 percent impervious is comprised of open space. Runoff from basin B11 drains to a detention pond at design point 25. Runoff from Basin B11 sheet flow southwest and will be conveyed via a proposed swale to detention pond FSD14A and will outfall to Sand Creek.

Basin C1 (Q_5=1.6 cfs, Q_{100}=11.8 cfs) is 5.87 acres and 2 percent impervious is comprised of open space. Runoff from basin C1 will be conveyed via a proposed swale then piped southwest to detention pond FSD14A.

Basin OS1 (Q_5= 23.8 cfs, Q_{100}=174.5 cfs) is 176.86 acres and 2 percent impervious and is comprised of future development including open space area, single family residential lots and local roads. Basin OS1 is located north of Briargate Parkway. Runoff from this basin drains southeast and is conveyed via a proposed swale to Pond FSD16.

Basin OS2 (Q_5= 7.7cfs, Q_{100}=56.7 cfs) is 39.27 acres and 2 percent impervious and is comprised of open space area. Basin OS2 is located northeast of the intersection of Briargate Parkway and Sterling Ranch Road. Runoff from this basin drains southwest to Pond FSD16.

DRAINAGE DESIGN CRITERIA

DEVELOPMENT CRITERIA REFERENCE

Storm drainage analysis and design criteria for this project were taken from the "*City of Colorado Springs/El Paso County Drainage Criteria Manual*" Volumes 1 and 2 (EPCDCM), dated October 12, 1994, the "*Urban Storm Drainage Criteria Manual*" Volumes 1 to 3 (USDCM) and Chapter 6 and

Section 3.2.1 of Chapter 13 of the "*Colorado Springs Drainage Criteria Manual*" (CSDCM), dated May 2014, as adopted by El Paso County.

HYDROLOGIC CRITERIA

All hydrologic data was obtained from the "*El Paso Drainage Criteria Manual*" Volumes 1 and 2, and the "*Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual*" Volumes 1, 2, and 3. Onsite drainage improvements were designed based on the 5 year (minor) storm event and the 100-year (major) storm event. Runoff was calculated using the Rational Method, and rainfall intensities for the 5-year and the 100-year storm return frequencies were obtained from Table 6-2 of the CSDCM. One hour point rainfall data for the storm events is identified in the chart below. Runoff coefficients were determined based on proposed land use and from data in Table 6-6 from the CSDCM. Time of concentrations were developed using equations from CSDCM. All runoff calculations and applicable charts and graphs are included in the Appendices.

Table 2 - 1-hr Point Rainfall Data

Storm	Rainfall (in.)
5-year	1.50
100-year	2.52

HYDRAULIC CRITERIA

The Rational Method and USDCM's SF-2 and SF-3 forms were used to determine the runoff from the minor and major storms on the site. Sumps and on-grade inlets were sized using UDFCD UD-Inlet v4.06 as shown in Appendix C. Manning's equation was used to size the proposed pipes in this report and StormCAD was used to model the proposed storm sewer system and to analyze the proposed HGL calculations for the Construction Drawings.

Table 2 - StormCAD Standard Method Conversions

StormCAD Conversion Table			
Bend Loss	Bend Angle	K coefficient Conversion	
	0	0.05	
	22.5	0.1	
	45	0.4	
	60	0.64	
	90	1.32	
Lateral Loss	1 Lateral K coefficient Conversion		
	Bend Angle	Non Surcharged	Surcharged
	45	0.27	0.47
	60	0.52	0.9
	90	1.02	1.77
	2 Laterals K coefficient Conversion		
	45	0.96	
	60	1.16	
	90	1.52	

DRAINAGE FACILITY DESIGN

GENERAL CONCEPT

The proposed stormwater conveyance system was designed to convey the developed Sterling Ranch – East of Sand Creek roadway runoff and treat the interim condition in the interim, full spectrum water quality and detention ponds via storm sewer. The proposed interim ponds were designed to release at less than historic rates to minimize adverse impacts downstream. Treated water will outfall directly into the Sand Creek drainage way, where it will eventually outfall into Fountain Creek. A proposed drainage map is presented in Appendix D showing locations of the ponds.

FOUR STEP PROCESS TO MINIMIZE ADVERSE IMPACTS OF URBANIZATION

In accordance with the El Paso County Drainage Criteria Manual Volume 2, this site has implemented the four step process to minimize adverse impacts of urbanization. The four step process includes reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainage ways, and implementing long-term source controls.

Step 1 – Reducing Runoff Volumes: The Sterling Ranch East of Sand Creek development project consists of Sterling Ranch Road and Briargate Parkway with open spaces and lawn areas interspersed within the development which helps disconnect impervious areas and reduce runoff volumes.

Step 2 – Stabilize Drainageways: The site lies within the Sand Creek Drainage Basin. Basin and bridge fees will be due at time of platting. These funds will be used for the channel stabilization being designed by Kiowa adjacent to the site and on future projects within the basin to stabilize

drainageways. The site does not discharge directly into the open drainageway of Sand Creek, therefore no downstream stabilization will be accomplished with this project.

Step 3 – Treat the WQCV: Water Quality treatment for this site is provided in the proposed full spectrum water quality detention ponds. The runoff from this site will be collected within inlets and conveyed to the proposed ponds via storm sewer and/ or swales. The outlet structure will be designed to detain the water quality capture volume (WQCV) for 40 hours, and the extended urban runoff volume (EURV) for 72 hours. All flows released from the ponds will be reduced to less than historic rates.

Step 4 –BMPs will be utilized to minimize off-site contaminants and to protect the downstream receiving waters. The permanent erosion control BMPs include asphalt drives, storm inlets and storm pipe, four full spectrum water quality and detention ponds, and permanent vegetation.

WATER QUALITY

In accordance with Section 13.3.2.1 of the CCS/EPCDCM, full spectrum water quality and detention are provided for all developed basins. This site will drain into two Full Spectrum Drainage Interim Ponds FSD14A and FSD16. Further details as well as all pond volume, water quality, and outfall calculations are included in the Appendix C of this report. A summary of Interim Pond FSD14A and FSD16 has been included below for reference. Ponds FSD14A and FSD16 will be sized per the MDDP and will be designed with the corresponding future development.

Table 3. Pond Volumes & Release Rates

	REQUIRED VOLUME (AC-FT)	VOLUME PROVIDED (AC-FT)	WQCV (AC-FT)	EURV (AC-FT)	5-YEAR RELEASE (CFS)	ALLOWABLE 5-YEAR RELEASE (CFS)	100-YEAR RELEASE (CFS)	ALLOWABLE 100-YEAR RELEASE (CFS)
POND FSD14A	4.67	9.98	0.64	N/A	6.5	7.5	126.3	142.2
POND FSD16	6.56	13.29	0.78	N/A	16.4	9.2	150.6	156.6

OPERATION & MAINTENANCE

In order to ensure the function and effectiveness of the stormwater infrastructure, maintenance activities such as inspection, routine maintenance, restorative maintenance, rehabilitation and repair, are required. The district shall be responsible for the inspection, maintenance, rehabilitation and repair of stormwater and erosion control facilities located on the property unless another party accepts such responsibility in writing and responsibility is properly assigned through legal documentation. Access is provided from onsite facilities and easements for proposed infrastructure

located offsite. We respectfully request that the Operation & Maintenance Manual be submitted in conjunction with the construction documents, prior to obtaining a grading permit.

DRAINAGE AND BRIDGE FEES

The site lies within the Sand Creek Drainage Basin. Anticipated drainage and bridge fees are presented below and will be due at time of platting (depending on date of plat submittal):.

2020 DRAINAGE AND BRIDGE FEES – STERLING RANCH PHASE 3				
Impervious Acres (ac)	Drainage Fee (Per Imp. Acre)	Bridge Fee (Per Imp. Acre)	Sterling Ranch Drainage Fee	Sterling Ranch Bridge Fee
17.38	$20,387	$8,339	$354,326	$6,158,187

SUMMARY

The proposed Sterling Ranch East of Sand Creek drainage improvements were designed to meet or exceed the El Paso County Drainage Criteria. The proposed development will not adversely affect the offsite drainageways or surrounding development. This report is in conformance and meets the latest El Paso County Storm Drainage Criteria requirements for this site.

REFERENCES

1. "El Paso County and City of Colorado Springs Drainage Criteria Manual, Vol I & II".

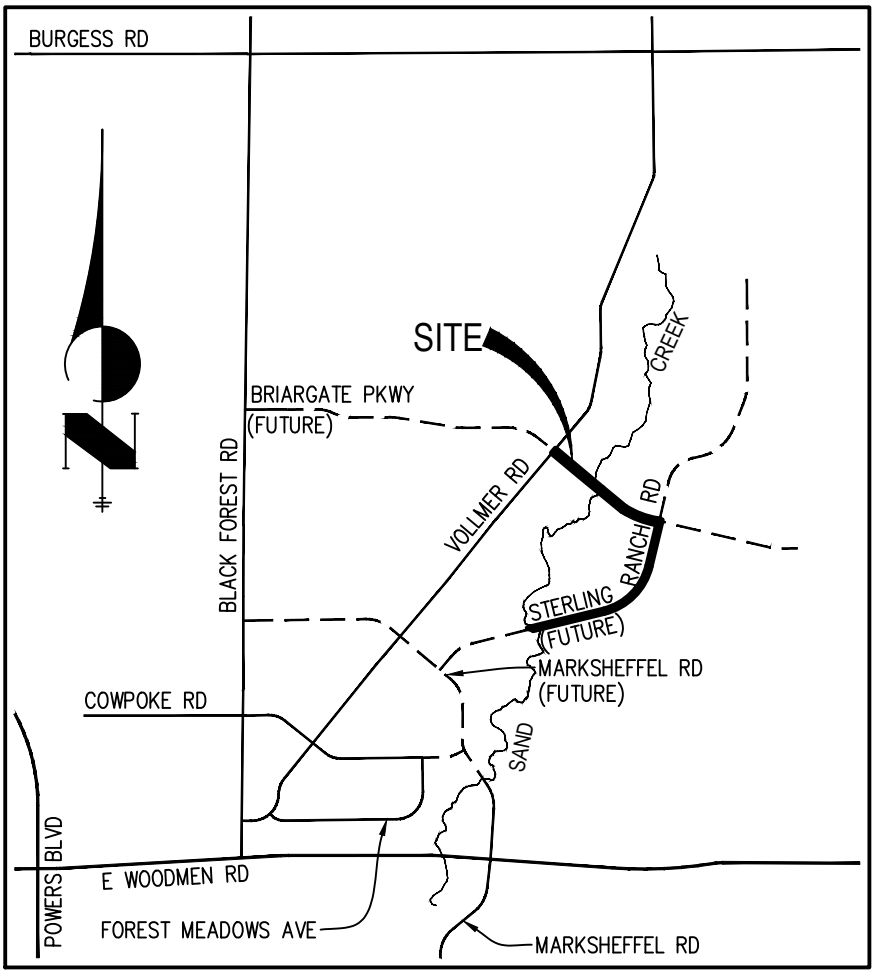
2. Final Bridge and Channel Design Report, prepared by Kiowa Engineering Corporation, May 19, 2020 (not yet approved)

3. "Master Development Drainage Plan for Sterling Ranch", (MMDP) prepared by M&S Civil Consultants, Inc., dated October 24, 2018.

4. Sand Creek Drainage Basin Planning Study, prepared Kiowa Engineering Corporation, January 1993, revised March 1996.

5. Urban Storm Drainage Criteria Manual (Volumes 1, 2, and 3), Urban Drainage and Flood Control District, June 2001.

Appendix A
Vicinity Map, Soil Descriptions, FEMA Floodplain Map



VICINITY MAP

N.T.S.

VICINITY MAP
STERLING RANCH ROAD
JOB NO. 25188.03
12/17/2021
SHEET 1 OF 1

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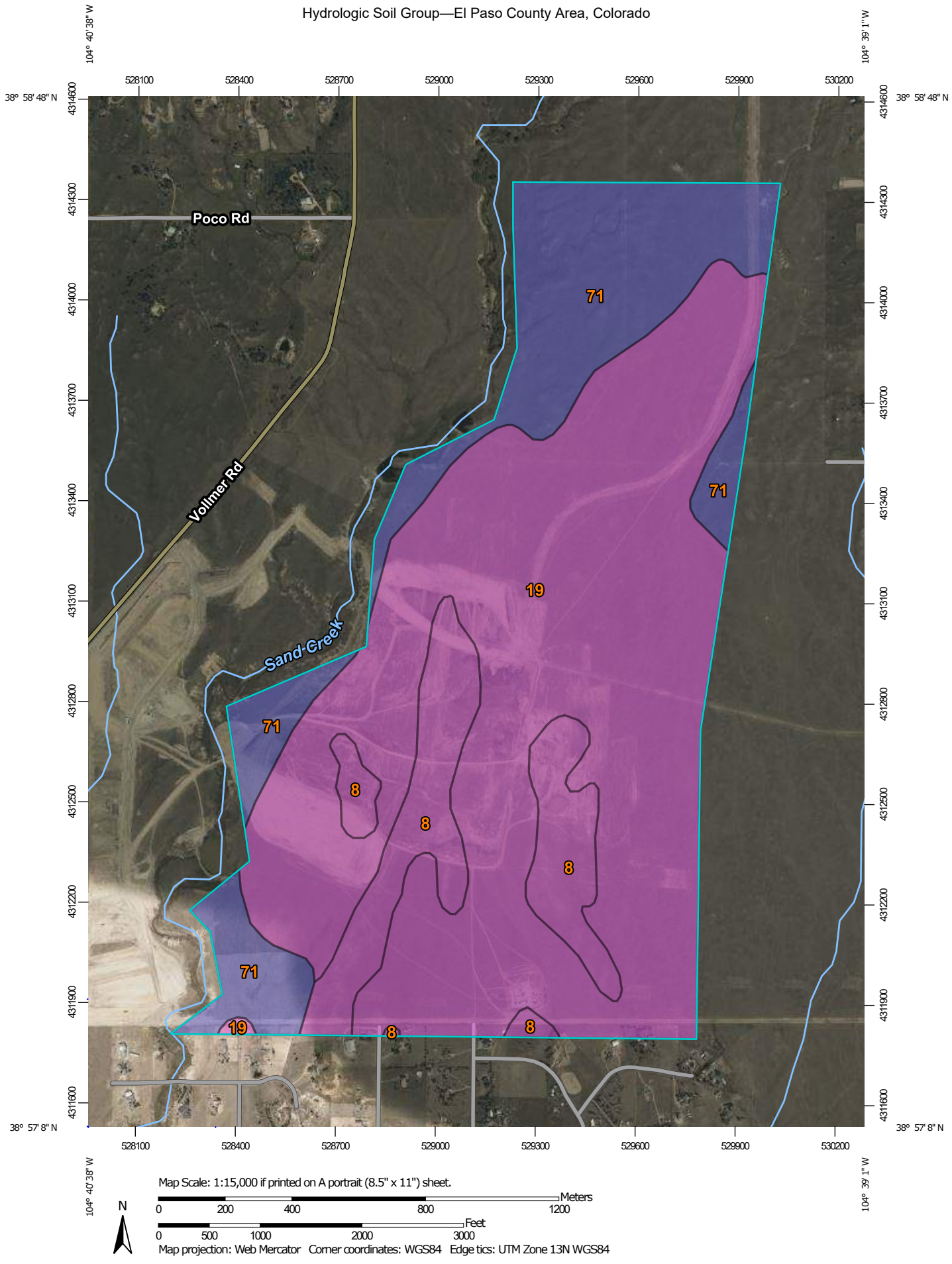

104° 40' 38" W
104° 39' 1" W
38° 58' 48" N
38° 57' 8" N
528100
528400
528700
529000
529300
529600
529900
530200
4314600
4314300
4314000
4313700
4313400
4313100
4312800
4312500
4312200
4311900
4311600
Poco Rd
Vollmer Rd
Sand Creek
71
19
8
Map Scale: 1:15,000 if printed on A portrait (8.5" x 11") sheet.
N
Meters
0 200 400 800 1200
Feet
0 500 1000 2000 3000
Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 18, Jun 5, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 19, 2018—May 26, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	89.8	12.7%
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	464.8	65.6%
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	153.8	21.7%
Totals for Area of Interest			**708.4**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or Floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross-sections and interpolated between cross-sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD88). These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, NNGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Andersen Consulting Engineers, Inc. These data are current as of 2005.

This map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which reflects authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new blue line stream representations and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact FEMA Map Service Center (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/

If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table

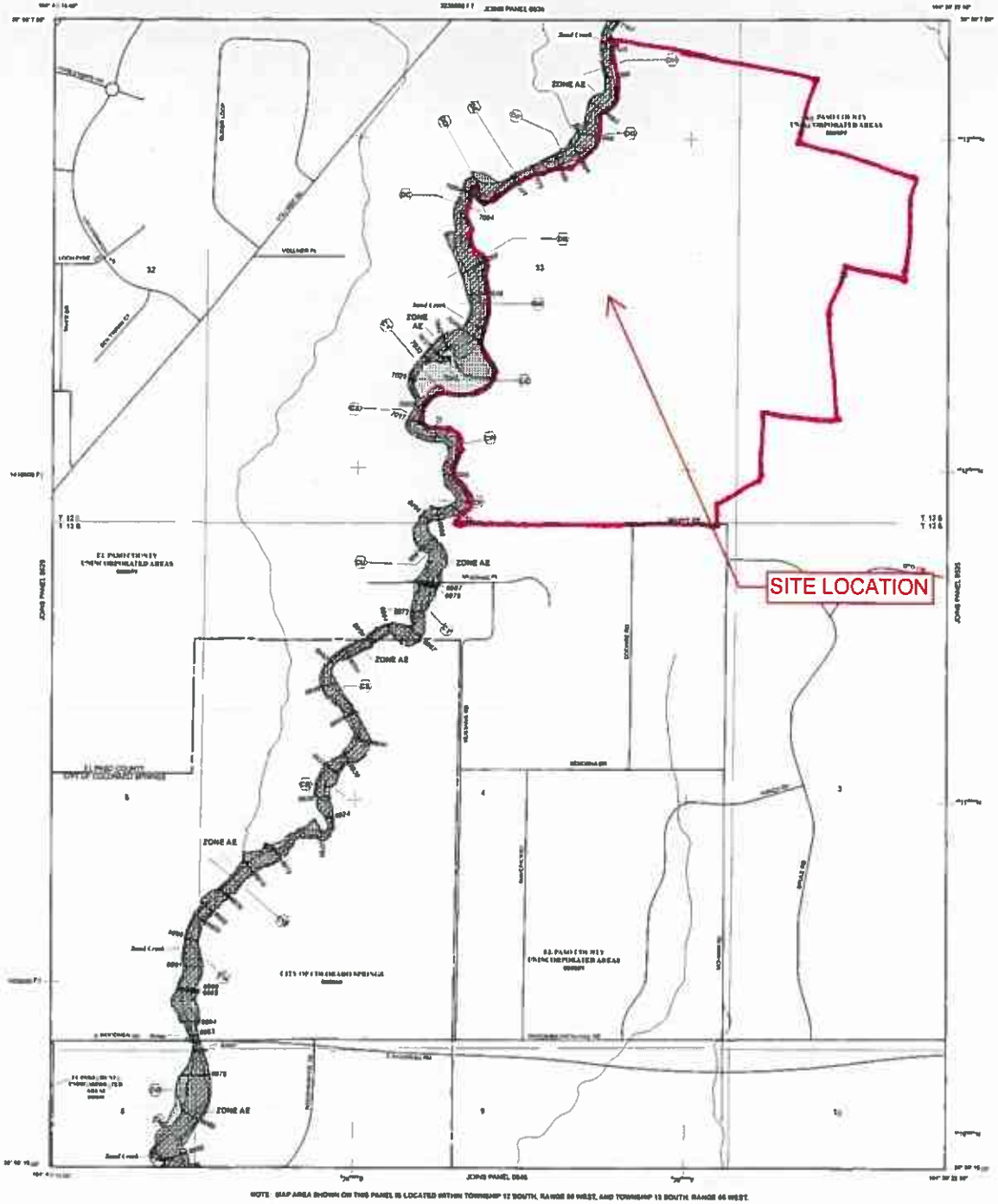
Flooding Source	Vertical Datum Offset (ft)

SPECIFIC INFORMATION FOR EL PASO COUNTY IS SHOWN IN THE FLOOD INSURANCE STUDY REPORT FOR EL PASO COUNTY IN THE VERTICAL DATUM CONVERSION INFORMATION

Panel Location Map

The Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partners (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

Additional Flood Hazard information and floodplain are available from local communities and the Colorado Water Conservation Board



NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 66 WEST, AND TOWNSHIP 13 SOUTH, RANGE 66 WEST.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Areas are the areas subject to flooding by the 1% annual chance flood. Special Flood Hazard Areas include Zones A, AE, AH, AO, AR, A1-A30, and VE. The 1% annual chance flood is also referred to as the base flood or 100-year flood.

ZONE A — No Base Flood Elevations determined.

ZONE AE — Base Flood Elevations determined.

ZONE AH — Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.

ZONE AO — Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.

ZONE AR — Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.

ZONE A99 — Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.

ZONE V — Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.

ZONE VE — Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREA IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X — Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X — Areas determined to be outside the 0.2% annual chance floodplain.

ZONE D — Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- Floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Flood Elevations, flood depths or flood velocities.
- 513 — Base Flood Elevation line and value; elevation in feet*
- (EL 987) — Base Flood Elevation value where uniform within zone; elevation in feet*
- * Referenced to the North American Vertical Datum of 1988 (NAVD 88)
- (A)—(A) — Cross section line
- 23—23 — Transect line
- 97° 07' 30" 32° 22' 30" — Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
- 4275000 mN — Universal Transverse Mercator grid ticks, zone 13
- 3000 FT — 1000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
- DX5510 — Bench mark (see explanation in notes to users)
- M1.5 — River Mile

MAP REPOSITORY
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2012 - to update corporate limits, to change Base Flood Elevations, to change Special Flood Hazard Areas, to incorporate previously issued Letters of Map Revision, to reflect new topographic information, and to incorporate previously issued Letters of Map Revision

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 500'

PANEL 0833G

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RA

EL PASO COUNTY
COLORADO
AND INCORPORATED

PANEL 533 OF 1300

(SEE MAP INDEX FOR FIRM PAN

CONTAINS:
COMMUNITY — NUMBER — PAN

MAP N
08041

MAP R
DECEMBE

Federal Emergency Managem

Appendix B
Hydrologic Calculations

COMPOSITE % IMPERVIOUS & COMPOSITE EXISTING RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 10/8/20

Basin ID	Total Area (ac)	Streets (100% Impervious)				Residential (65% Impervious) Neighborhood Area (70% Impervious)				1 Acre lot Rersidential (20% Impervious) Light Commercial (80% Impervious)				Historical Analysis (2% Impervious)				Basins Total Weighted C Values		Basins Total Weighted % Imp.
		C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	
EX1	178.68	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	178.68	2.0%	0.08	0.35	2.0%
EX2	14.67	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	14.67	2.0%	0.08	0.35	2.0%
EX4	36.46	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	36.46	2.0%	0.08	0.35	2.0%
EX3	160.58	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.59	0.70	0.00	0.0%	0.08	0.35	160.58	2.0%	0.08	0.35	2.0%
TOTAL (EX1-EX4)	**390.39**																			**2.0%**
TOTAL	**390.39**																			**2.0%**

COMPOSITE RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 10/8/20

Basin ID	Total Area (ac)	Basins Total Weighted % Imp.	Hydrologic Soil Group			Hydrologic Soil Group			Minor Coefficients			Major Coefficients			Basins Total Weighted C_5	Basins Total Weighted C_{100}
			Area A (ac)	Area B (ac)	Area C/D (ac)	% A (ac)	% B (ac)	% C/D (ac)	$C_{5,A}$	$C_{5,B}$	$C_{5,C/D}$	$C_{100,A}$	$C_{100,B}$	$C_{100,C/D}$		
EX1	178.68	2.0%	0.00	178.68	0.00	0%	100%	0%	0.01	0.01	0.05	0.13	0.44	0.49	0.01	0.44
EX2	14.67	2.0%	0.00	14.67	0.00	0%	100%	0%	0.02	0.02	0.07	0.17	0.46	0.52	0.02	0.46
EX3	160.58	2.0%	0.00	160.58	0.00	0%	100%	0%	0.02	0.02	0.07	0.17	0.46	0.52	0.02	0.46
EX4	36.46	2.0%	0.00	36.46	0.00	0%	100%	0%	0.02	0.02	0.07	0.17	0.46	0.52	0.02	0.46
TOTAL	**390.39**	**2.0%**	**0.00**	**390.39**	**0.00**	**0%**	**100%**	**0%**	---	---	---	---	---	---	**0.02**	**0.45**

Table 6-4. Runoff coefficient equations based on NRCS soil group and storm return period

NRCS Soil Group	Storm Return Period						
	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year	500-Year
A	$C_A = 0.84i^{1.302}$	$C_A = 0.86i^{1.276}$	$C_A = 0.87i^{1.232}$	$C_A = 0.84i^{1.124}$	$C_A = 0.85i+0.025$	$C_A = 0.78i+0.110$	$C_A = 0.65i+0.254$
B	$C_B = 0.84i^{1.169}$	$C_B = 0.86i^{1.088}$	$C_B = 0.81i+0.057$	$C_B = 0.63i+0.249$	$C_B = 0.56i+0.328$	$C_B = 0.47i+0.426$	$C_B = 0.37i+0.536$
C/D	$C_{C/D} = 0.83i^{1.122}$	$C_{C/D} = 0.82i+0.035$	$C_{C/D} = 0.74i+0.132$	$C_{C/D} = 0.56i+0.319$	$C_{C/D} = 0.49i+0.393$	$C_{C/D} = 0.41i+0.484$	$C_{C/D} = 0.32i+0.588$

Where:

i = % imperviousness (expressed as a decimal)

C_A = Runoff coefficient for Natural Resources Conservation Service (NRCS) HSG A soils

C_B = Runoff coefficient for NRCS HSG B soils

$C_{C/D}$ = Runoff coefficient for NRCS HSG C and D soils.

EXISTING
STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 10/8/20

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					*tc* CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (ft)	S_o (%)	t_i (min)	L_t (ft)	S_t (%)	K	VEL. (ft/s)	t_t (min)	COMP. t_c (min)	TOTAL LENGTH (ft)	Urbanized t_c (min)	t_c (min)
EX1	178.68	A	2%	0.08	0.35	1275	2.5%	48.7	2184	2.4%	10.0	1.5	23.7	72.4	3459.0	51.2	51.2
EX2	14.67	A	2%	0.08	0.35	1101	2.7%	43.9	551	2.5%	10.0	1.6	5.8	49.7	1652.0	31.9	31.9
EX4	36.46	A	2%	0.08	0.35	1037.38	2.4%	44.4	941	1.5%	10.0	1.2	12.9	57.2	1978.3	39.5	39.5
EX3	160.58	A	2%	0.08	0.35	974	2.1%	44.9	1892	1.8%	10.0	1.3	23.4	68.3	2866.0	50.9	50.9

NOTES:

$$t_c = t_i + t_t$$ Equation 6-2

Where:

t_c = computed time of concentration (minutes)
t_i = overland (initial) flow time (minutes)
t_t = channelized flow time (minutes).

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L_i}}{S_o^{0.33}}$$ Equation 6-3

Where:

t_i = overland (initial) flow time (minutes)
C_5 = runoff coefficient for 5-year frequency (from Table 6-4)
L_i = length of overland flow (ft)
S_o = average slope along the overland flow path (ft/ft).

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$ Equation 6-4

Where:

t_t = channelized flow time (travel time, min)
L_t = waterway length (ft)
S_o = waterway slope (ft/ft)
V_t = travel time velocity (ft/sec) = $K\sqrt{S_o}$
K = NRCS conveyance factor (see Table 6-2).

$$t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$ Equation 6-5

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.
L_t = length of channelized flow path (ft)
i = imperviousness (expressed as a decimal)
S_t = slope of the channelized flow path (ft/ft).

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

STANDARD FORM SF-3 - EXISTING

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 10/8/20

		DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	EX1	EX1	178.68	0.08	51.2	14.29	1.68	24.0															
	EX2	EX2	14.67	0.08	31.9	1.17	2.39	2.8															
	EX3	EX3	160.58	0.08	50.9	12.85	1.69	21.7															
	EX4	EX4	36.46	0.08	39.5	2.92	2.07	6.0															

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

STANDARD FORM SF-3 - EXISTING

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Subdivision- Existing
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 10/8/20

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	EX1	EX1	178.68	0.35	51.2	62.54	2.82	176.3															
	EX2	EX2	14.67	0.35	31.9	5.13	4.01	20.6															
	EX3	EX3	160.58	0.35	50.9	56.20	2.83	159.2															
	EX4	EX4	36.46	0.35	39.5	12.76	3.47	44.3															

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

COMPOSITE % IMPERVIOUS & COMPOSITE PROPOSED RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Sterling Ranch Rd & Briargate Pkwy
Location: El Paso County

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 12/2/21

Basin ID	Total Area (ac)	Streets/ Walks (100% Impervious)				Lawn/ Historic (2% Impervious)				Basins Total Weighted C Values		Basins Total Weighted % Imp.
		C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	
A1	4.95	0.90	0.96	3.28	66.3%	0.08	0.35	1.67	0.7%	0.62	0.75	66.9%
A2	4.97	0.90	0.96	3.33	67.0%	0.08	0.35	1.64	0.7%	0.63	0.76	67.7%
A3	2.01	0.90	0.96	1.23	61.2%	0.08	0.35	0.78	0.8%	0.58	0.72	62.0%
A4	1.63	0.90	0.96	1.06	65.0%	0.08	0.35	0.57	0.7%	0.61	0.75	65.7%
B1	1.90	0.90	0.96	1.23	64.7%	0.08	0.35	0.67	0.7%	0.61	0.74	65.4%
B2	2.06	0.90	0.96	1.21	58.7%	0.08	0.35	0.85	0.8%	0.56	0.71	59.6%
B3	1.27	0.90	0.96	0.80	63.0%	0.08	0.35	0.47	0.7%	0.60	0.73	63.7%
B4	1.33	0.90	0.96	0.80	60.2%	0.08	0.35	0.53	0.8%	0.57	0.72	60.9%
B5	0.89	0.90	0.96	0.54	60.7%	0.08	0.35	0.35	0.8%	0.58	0.72	61.5%
B6	0.91	0.90	0.96	0.57	62.6%	0.08	0.35	0.34	0.7%	0.59	0.73	63.4%
B7	1.08	0.90	0.96	0.55	50.9%	0.08	0.35	0.53	1.0%	0.50	0.66	51.9%
B8	1.16	0.90	0.96	0.66	56.9%	0.08	0.35	0.50	0.9%	0.55	0.70	57.8%
B9	1.98	0.90	0.96	0.98	49.5%	0.08	0.35	1.00	1.0%	0.49	0.65	50.5%
B10	2.19	0.90	0.96	1.14	52.1%	0.08	0.35	1.05	1.0%	0.51	0.67	53.0%
B11	126.23	0.90	0.96	0.00	0.0%	0.08	0.35	126.23	2.0%	0.08	0.35	2.0%
C1	5.87	0.90	0.96	0.00	0.0%	0.08	0.35	5.87	2.0%	0.08	0.35	2.0%
OS1	176.86	0.90	0.96	0.00	0.0%	0.08	0.35	176.86	2.0%	0.08	0.35	2.0%
OS2	39.27	0.90	0.96	0.00	0.0%	0.08	0.35	39.27	2.0%	0.08	0.35	2.0%

Basin ID	Total Area (ac)	Streets/ Walks (100% Impervious)				Lawn/ Historic (2% Impervious)				Basins Total Weighted C Values		Basins Total Weighted % Imp.
		C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	Area (ac)	Weighted % Imp.	C_5	C_{100}	
Pond FSD 16 (Total of A and OS)	229.69											5.8%
Pond FSD 14A (Total of B)	141.00											7.9%
Pond FSD 14B (Total of C)	5.87											2.0%
TOTAL	376.56											5.4%

PROPOSED
STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Sterling Ranch Rd & Briargate Pkwy
Location: El Paso County

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 12/2/21

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					*tc* CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (ft)	S_o (%)	t_i (min)	L_t (ft)	S_t (%)	K	VEL. (ft/s)	t_t (min)	COMP. t_c (min)	TOTAL LENGTH (ft)	Urbanized t_c (min)	t_c (min)
A1	4.95	A	66.9%	0.62	0.75	37	2.5%	3.9	1330	0.8%	20.0	1.8	12.5	16.4	1367.0	28.3	16.4
A2	4.97	A	67.7%	0.63	0.76	37	2.5%	3.8	1332	0.7%	20.0	1.7	13.2	17.0	1369.0	28.8	17.0
A3	2.01	A	62.0%	0.58	0.72	50	4.0%	4.2	552	1.0%	20.0	2.0	4.6	8.8	602.0	20.7	8.8
A4	1.63	A	65.7%	0.61	0.75	50	2.3%	4.7	590	0.9%	20.0	1.9	5.3	10.0	640.0	20.6	10.0
B1	1.90	A	65.4%	0.61	0.74	30	2.7%	3.5	745	2.1%	20.0	2.9	4.3	7.8	775.0	19.6	7.8
B2	2.06	A	59.6%	0.56	0.71	30	2.7%	3.8	757	2.1%	20.0	2.9	4.4	8.2	787.0	20.9	8.2
B3	1.27	A	63.7%	0.60	0.73	30	2.3%	3.8	714	1.5%	20.0	2.4	4.9	8.6	744.0	20.6	8.6
B4	1.33	A	60.9%	0.57	0.72	30	2.3%	4.0	760	1.5%	20.0	2.5	5.1	9.1	790.0	21.5	9.1
B5	0.89	A	61.5%	0.58	0.72	30	2.5%	3.8	559	1.5%	20.0	2.4	3.8	7.6	589.0	19.9	7.6
B6	0.91	A	63.4%	0.59	0.73	30	2.5%	3.7	495	1.5%	20.0	2.4	3.4	7.1	525.0	19.0	7.1
B7	1.08	A	51.9%	0.50	0.66	30	2.5%	4.4	531	1.5%	20.0	2.4	3.6	8.0	561.0	21.6	8.0
B8	1.16	A	57.8%	0.55	0.70	30	2.5%	4.0	526	1.5%	20.0	2.4	3.6	7.6	556.0	20.4	7.6
B9	1.98	A	50.5%	0.49	0.65	30	2.5%	4.5	628	2.3%	20.0	3.0	3.4	7.9	658.0	21.7	7.9
B10	2.19	A	53.0%	0.51	0.67	30	2.5%	4.3	645	2.3%	20.0	3.0	3.5	7.9	675.0	21.3	7.9
B11	126.23	A	2.0%	0.08	0.35	700	2.0%	38.8	2340	2.0%	20.0	2.8	13.8	52.6	3040.0	55.4	52.6
C1	5.87	A	2.0%	0.08	0.35	150	20.0%	8.4	1300	2.0%	20.0	2.8	7.7	16.1	1450.0	42.2	16.1
OS1	176.86	A	2.0%	0.08	0.35	1275	2.0%	52.3	2184	2.4%	20.0	3.1	11.8	64.1	3459.0	51.2	51.2
OS2	39.27	A	2.0%	0.08	0.35	786	1.8%	42.5	403	2.3%	20.0	3.0	2.2	44.7	1189.0	30.4	30.4

NOTES:

$$t_c = t_i + t_t$$ Equation 6-2

Where:

t_c = computed time of concentration (minutes)
t_i = overland (initial) flow time (minutes)
t_t = channelized flow time (minutes).

$$t_t = \frac{0.395(1.1 - C_5)\sqrt{L_t}}{S_t^{0.33}}$$ Equation 6-3

Where:

t_t = overland (initial) flow time (minutes)
C_5 = runoff coefficient for 5-year frequency (from Table 6-4)
L_t = length of overland flow (ft)
S_t = average slope along the overland flow path (ft/ft)

Use a minimum t_c value of 5 minutes for urbanized areas and a minimum t_c value of 10 minutes for areas that are not considered urban. Use minimum values even when calculations result in a lesser time of concentration.

$$t_t = \frac{L_c}{60K\sqrt{S_c}} = \frac{L_c}{60V_c}$$ Equation 6-4

Where:

t_t = channelized flow time (travel time, min)
L_c = waterway length (ft)
S_c = waterway slope (ft/ft)
V_c = travel time velocity (ft/sec) = $K\sqrt{S_c}$
K = NRCS conveyance factor (see Table 6-2)

$$t_c = (26 - 17i) + \frac{L_c}{60(14i + 9)\sqrt{S_c}}$$ Equation 6-5

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1
L_c = length of channelized flow path (ft)
i = imperviousness (expressed as a decimal)
S_c = slope of the channelized flow path (ft/ft)

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

STANDARD FORM SF-3 - PROPOSED

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Rd & Briargate Pkwy
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 12/2/21

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	OS1	OS1	176.86	0.08	51.2	14.15	1.68	23.8								23.8	14.15	0.9	60	800	1.9	7.0	Future Development to Pond FSD16
	2	A2	4.97	0.63	17.0	3.13	3.33	10.4					0.1	0.03	0.9	10.3	3.10	0.5	18	65	1.4	0.8	On-grade inlet, bypass to DP6 Capture to DP1
	1	A1	4.95	0.62	16.4	3.09	3.39	10.5	17.8	6.19	3.27	20.2	0.0	0	0.9	20.2	6.19	0.5	24	860	1.4	10.1	On-grade inlet, bypass to DP4 Capture to DP5
	4	A4	1.63	0.61	10.0	1.00	4.13	4.1	27.9	1.00	2.59	2.6	0.0	0	0.9	2.6	1.00	0.5	18	25	1.4	0.3	On-grade inlet, bypass to future Briargate Pkwy Capture to DP5
	5								27.9	7.19	2.59	18.6				18.6	7.19	0.5	24	96	1.4	1.1	Manhole Piped to DP6
	6	A3	2.01	0.58	8.8	1.17	4.32	5.1	29.0	8.39	2.53	21.2	0.0	0	0.9	21.2	8.39	0.3	36	333	1.1	5.1	On-grade inlet, bypass to future Briargate Pkwy Capture to DP6.1
	OS2	OS2	39.27	0.08	30.4	3.14	2.46	7.7	58.2	25.68	1.49	38.2											Future Development to Interim Pond FSD16
	OS2.1															16.4	3.06	1.5	48	2740	2.4	18.6	Pond FSD16 Outfall Piped to DP26.A
	7	B2	2.06	0.56	8.2	1.16	4.43	5.1					0.0	0	0.9	5.1	1.16	0.5	18	59	1.4	0.7	On-grade inlet, bypass to DP10 Capture to DP8
	8	B1	1.90	0.61	7.8	1.16	4.51	5.2	10.3	2.32	4.09	9.5	0.0	0	0.9	9.5	2.32	2.2	18	1000	3.0	5.6	On-grade inlet, bypass to DP11 Capture conveyed via swale to DP13
	10	B4	1.33	0.57	9.1	0.76	4.28	3.3	9.1	0.76	4.28	3.3	0.0	0	0.9	3.3	0.76	0.5	18	60	1.4	0.7	On-grade inlet, bypass to DP14 Capture to DP12
	11	B3	1.27	0.60	8.6	0.76	4.35	3.3	13.4	1.52	3.69	5.6	0.0	0	0.9	5.6	1.52	4.0	18	200	4.0	0.8	On-grade inlet, bypass to DP15 Capture to DP12
	13								15.9	3.84	3.43	13.2				13.2	3.84	2.0	60	560	2.8	3.3	Inlet outflow Converyed via swale to Pond FSD14A
	14	B6	0.91	0.59	7.1	0.54	4.65	2.5	9.8	0.54	4.16	2.2	1.0	0.24	0.9	1.2	0.30	0.5	18	60	1.4	0.7	On-grade inlet, bypass to DP18 Capture to DP16
	15	B5	0.89	0.58	7.6	0.51	4.54	2.3	14.2	0.81	3.60	2.9	1.0	0.28	0.9	1.9	0.53	7.0	18	170	5.3	0.5	On-grade inlet, bypass to DP19 Capture to DP17

STANDARD FORM SF-3 - PROPOSED

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Rd & Briargate Pkwy
Location: El Paso County
Design Storm: 5-Year

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 12/2/21

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t_c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	17								19.2	4.37	3.15	13.8				13.8	4.37	2.0	60	700	2.8	4.1	Outlet structure Captured conveyed via a swale to Pond Fsd14a
	18	B8	1.16	0.55	7.6	0.63	4.54	2.9	10.5	0.87	4.06	3.5	1.0	0.25	0.9	2.5	0.62	5.0	18	54	4.5	0.2	On-grade inlet, bypass to DP22 Capture to DP20
	19	B7	1.08	0.50	8.0	0.54	4.46	2.4	14.8	1.69	3.55	6.0	1.0	0.28	0.9	5.0	1.41	5.0	18	103	4.5	0.4	On-grade inlet, bypass to DP23 Capture to DP20
	20								19.2	5.25	3.15	16.5				16.5	5.25	2.5	60	300	3.2	1.6	Inlet outflow Captured conveyed via a swale to Pond FSD14A
	22	B10	2.19	0.51	7.9	1.11	4.49	5.0	10.7	1.36	4.03	5.5				5.5	1.36	0.5	18	53	1.4	0.6	Sump Inlet Piped to DP23
	23	B9	1.98	0.49	7.9	0.96	4.48	4.3	15.1	2.60	3.51	9.1				9.1	2.60	0.5	18	328	1.4	3.9	Sump Inlet Piped to Pond FSD14A
	25	B11	126.23	0.08	52.6	10.10	1.64	16.6	52.6	17.94	1.64	29.4											Pond FSD14A
	26	C1	5.87	0.08	16.1	0.47	3.42	1.6								1.6	0.47						To Pond FSD 14B
	26.1								16.1	0.47	3.42	1.6				0.3	0.02	4.0	18	84	4.0	0.4	Outflow from future pond FSD14B Piped to DP26A
	26A								18.6	3.53	3.19	11.3				11.3	3.53	1.8	54	2280	2.7	14.2	Manhole. Flow from MDDP DP21 Piped to DP27
	25.1															6.5	0.59	0.5	48	168	1.4	2.0	Outflow from Pond FSD14B Piped to DP27
	27								32.8	4.12	2.35	9.7											Manhole Confluenced flow from Pond FSD14A and DP26A

Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

STANDARD FORM SF-3 - PROPOSED

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Rd & Briargate Pkwy
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 12/2/21

		DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			
Description	Design Point	Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	REMARKS
	OS1	OS1	176.86	0.35	51.2	61.90	2.82	174.5								174.5	61.90	0.9	60	800	1.9	7.0	Future Development to Pond FSD16
	2	A2	4.97	0.76	17.0	3.77	5.60	21.1					3.3	0.59	0.9	17.8	3.18	0.5	18	65	1.4	0.8	On-grade inlet, bypass to DP6 Capture to DP1
	1	A1	4.95	0.75	16.4	3.73	5.68	21.2	17.8	6.91	5.49	37.9	1.3	0.237	0.9	36.6	6.67	0.5	24	860	1.4	10.1	On-grade inlet, bypass to DP4 Capture to DP5
	4	A4	1.63	0.75	10.0	1.22	6.93	8.5	27.9	1.46	4.35	6.3	2.0	0.46	0.9	4.3	1.00	0.5	18	25	1.4	0.3	On-grade inlet, bypass to future Briargate Pkwy Capture to DP5
	5								26.6	7.67	4.47	34.3				34.3	7.67	0.5	24	96	1.4	1.1	Manhole Piped to DP6
	6	A3	2.01	0.72	8.8	1.45	7.26	10.5	26.6	9.71	4.47	43.4	2.5	0.559	0.9	40.9	9.15	0.3	36	333	1.1	5.1	On-grade inlet, bypass to future Briargate Pkwy Capture to DP6.1
	OS2	OS2	39.27	0.35	30.4	13.74	4.13	56.7	58.2	84.79	2.49	211.5											Future Development to Interim Pond FSD16
	OS2.1															147.9	27.58	1.5	48	2740	2.4	18.6	Pond FSD16 Outfall Piped to DP26.A
	7	B2	2.06	0.71	8.2	1.46	7.44	10.9					1.3	0.175	0.9	9.6	1.29	0.5	18	59	1.4	0.7	On-grade inlet, bypass to DP10 Capture to DP8
	8	B1	1.90	0.74	7.8	1.42	7.57	10.7	10.3	3.34	6.86	22.9	2.3	0.335	0.9	20.6	3.00	2.2	18	1000	3.0	5.6	On-grade inlet, bypass to DP11 Capture conveyed via swale to DP13
	10	B4	1.33	0.72	9.1	0.95	7.18	6.8	9.1	1.12	7.18	8.1	2.7	0.376	0.9	5.4	0.75	0.5	18	60	1.4	0.7	On-grade inlet, bypass to DP14 Capture to DP12
	11	B3	1.27	0.73	8.6	0.93	7.30	6.8	13.4	2.01	6.20	12.5	1.2	0.194	0.9	11.3	1.82	4.0	18	200	4.0	0.8	On-grade inlet, bypass to DP15 Capture to DP12
	13								15.9	4.83	5.76	27.8				27.8	4.83	2.0	60	560	2.8	3.3	Inlet outflow Converyed via swale to Pond FSD14A
	14	B6	0.91	0.73	7.1	0.67	7.80	5.2	9.8	1.05	6.99	7.3	1.0	0.143	0.9	6.3	0.90	0.5	18	60	1.4	0.7	On-grade inlet, bypass to DP18 Capture to DP16
	15	B5	0.89	0.72	7.6	0.64	7.62	4.9	14.2	1.74	6.05	10.5	1.0	0.165	0.9	9.5	1.57	7.0	18	170	5.3	0.5	On-grade inlet, bypass to DP19 Capture to DP17

STANDARD FORM SF-3 - PROPOSED

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision: Sterling Ranch Rd & Briargate Pkwy
Location: El Paso County
Design Storm: 100-Year

Project Name: Sterling Ranch Phase 3
Project No.: 25188.03
Calculated By: CGV
Checked By: RAB
Date: 12/2/21

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t_c (min)	C*A (ac)	I (in/hr)	Q (cfs)	tc (min)	C*A (ac)	I (in/hr)	Q (cfs)	$Q_{street/swale}$ (cfs)	C*A (ac)	Slope (%)	Q_{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t_t (min)	
	17								19.2	6.40	5.29	33.8				33.8	6.40	2.0	60	700	2.8	4.1	Outlet structure Captured conveyed via a swale to Pond Fsd14a
	18	B8	1.16	0.70	7.6	0.81	7.62	6.2	10.5	0.95	6.82	6.5	1.0	0.147	0.9	5.5	0.81	5.0	18	54	4.5	0.2	On-grade inlet, bypass to DP22 Capture to DP20
	19	B7	1.08	0.66	8.0	0.71	7.49	5.3	14.8	1.68	5.95	10.0	1.0	0.168	0.9	9.0	1.51	5.0	18	103	4.5	0.4	On-grade inlet, bypass to DP23 Capture to DP20
	20								23.3	7.91	4.80	38.0				38.0	7.91	2.5	60	300	3.2	1.6	Inlet outflow Captured conveyed via a swale to Pond FSD14A
	22	B10	2.19	0.67	7.9	1.46	7.53	11.0	10.7	1.61	6.77	10.9				10.9	1.61	0.5	18	53	1.4	0.6	Sump Inlet Piped to DP23
	23	B9	1.98	0.65	7.9	1.29	7.52	9.7	15.1	3.06	5.89	18.0				18.0	3.06	0.5	18	328	1.4	3.9	Sump Inlet Piped to Pond FSD14A
	25	B11	126.23	0.35	52.6	44.18	2.75	121.6	52.6	55.15	2.75	151.7				151.7	55.15						Pond FSD14A
	26	C1	5.87	0.35	16.1	2.05	5.74	11.8								11.8	2.05						To Pond FSD 14B
	26.1								16.1	2.05	5.74	11.8				11.8	0.77	4.0	18	84	4.0	0.4	Outflow from future pond FSD14B Piped to DP26A
	26A								18.6	29.63	5.36	158.9				158.9	29.63	1.8	54	2280	2.7	14.2	Manhole. Flow from MDDP DP21 Piped to DP27
	25.1															126.3	11.47	0.5	24	168	1.4	2.0	Outflow from Pond FSD14B Piped to DP27
	27								32.8	41.10	3.94	161.9											Manhole Confluenced flow from Pond FSD14A and DP26A

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

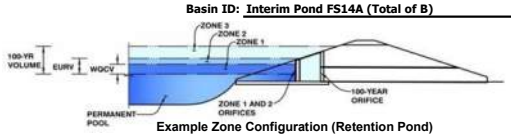
Appendix C
Hydraulic Calculations

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

Project: Sterling Ranch - East of Sand Creek

Basin ID: Interim Pond FS14A (Total of B)



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	EDB	
Watershed Area =	141.00	acres
Watershed Length =	3,300	ft
Watershed Length to Centroid =	1,350	ft
Watershed Slope =	0.034	ft/ft
Watershed Imperviousness =	7.90%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

			Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.642	acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	1.027	acre-feet		acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	1.663	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	3.877	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	6.117	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	10.204	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	12.947	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	16.941	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 4 in.) =	34.465	acre-feet	4.00	inches
Approximate 2-yr Detention Volume =	0.640	acre-feet		
Approximate 5-yr Detention Volume =	1.024	acre-feet		
Approximate 10-yr Detention Volume =	2.372	acre-feet		
Approximate 25-yr Detention Volume =	3.434	acre-feet		
Approximate 50-yr Detention Volume =	3.565	acre-feet		
Approximate 100-yr Detention Volume =	4.668	acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.642	acre-feet
Zone 2 Volume (100-year - Zone 1) =	4.026	acre-feet
Select Zone 3 Storage Volume (Optional) =		acre-feet
Total Detention Basin Volume =	4.668	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

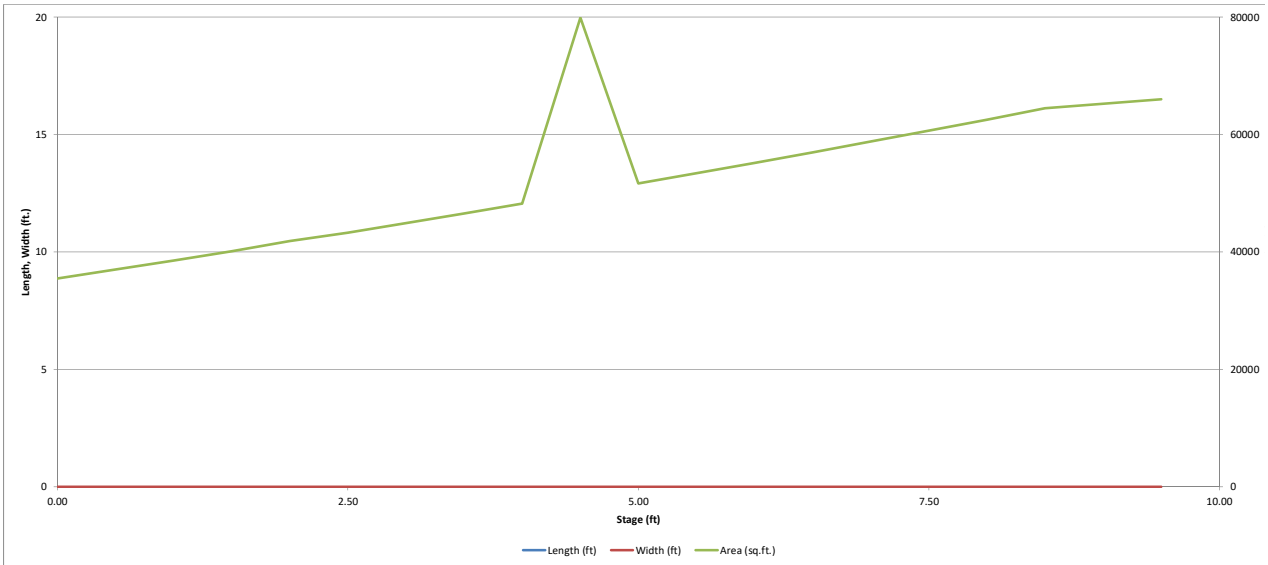
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

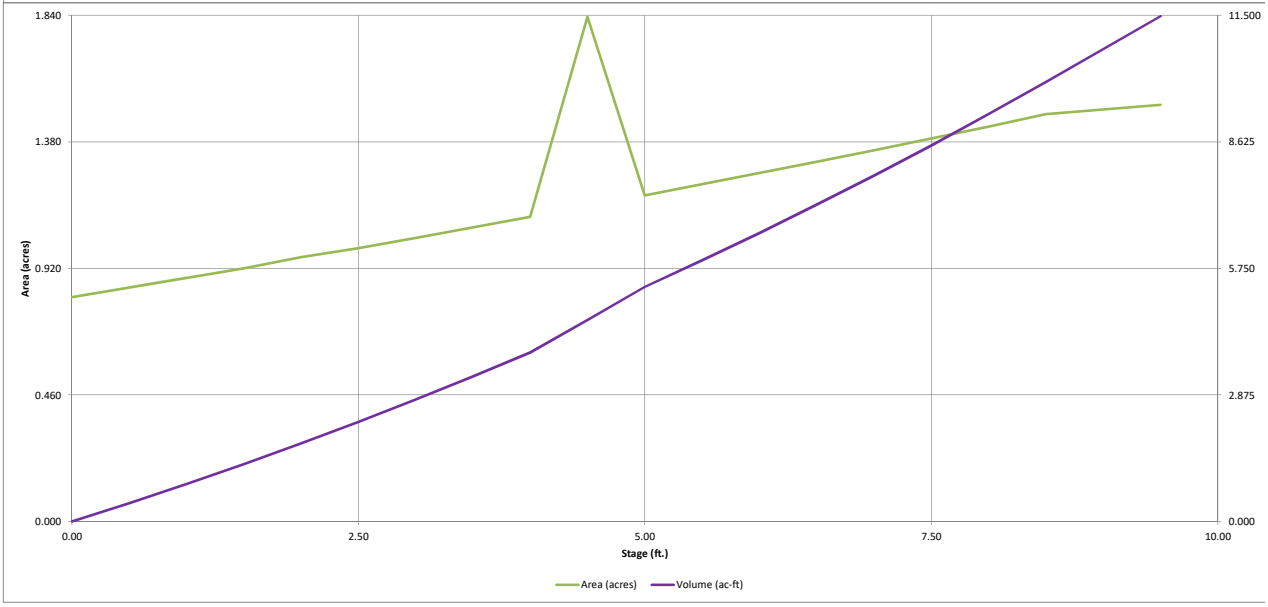
Depth Increment = 0.50 ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	35,513	0.815		
7010.5	--	0.50	--	--	--	37,022	0.850	18,134	0.416
7011	--	1.00	--	--	--	38,551	0.885	37,027	0.850
7011.5	--	1.50	--	--	--	40,104	0.921	56,691	1.301
7012	--	2.00	--	--	--	41,871	0.961	77,184	1.772
7012.5	--	2.50	--	--	--	43,283	0.994	98,473	2.261
7013	--	3.00	--	--	--	44,909	1.031	120,521	2.767
7013.5	--	3.50	--	--	--	46,560	1.069	143,388	3.292
7014	--	4.00	--	--	--	48,236	1.107	167,087	3.836
7014.5	--	4.50	--	--	--	79,936	1.835	199,130	4.571
7015	--	5.00	--	--	--	51,660	1.186	232,029	5.327
7015.5	--	5.50	--	--	--	53,409	1.226	258,296	5.930
7016	--	6.00	--	--	--	55,182	1.267	285,444	6.553
7016.5	--	6.50	--	--	--	56,980	1.308	313,485	7.197
7017	--	7.00	--	--	--	58,803	1.350	342,430	7.861
7017.5	--	7.50	--	--	--	60,650	1.392	372,293	8.547
7018	--	8.00	--	--	--	62,521	1.435	403,086	9.254
7018.5	--	8.50	--	--	--	64,486	1.480	434,838	9.983
7019.5	--	9.50	--	--	--	66,000	1.515	500,081	11.480

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)



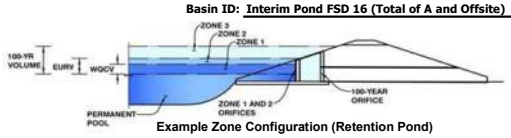


DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

Project: Sterling Ranch - East of Sand Creek

Basin ID: Interim Pond FSD 16 (Total of A and Offsite)



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	**EDB**	
Watershed Area =	226.94	acres
Watershed Length =	4,600	ft
Watershed Length to Centroid =	3,200	ft
Watershed Slope =	0.034	ft/ft
Watershed Imperviousness =	5.80%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

			Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.783	acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	1.184	acre-feet		acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	2.329	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	5.787	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	9.335	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	15.975	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	20.358	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	26.812	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 4 in.) =	54.919	acre-feet	4.00	inches
Approximate 2-yr Detention Volume =	0.714	acre-feet		
Approximate 5-yr Detention Volume =	1.173	acre-feet		
Approximate 10-yr Detention Volume =	3.205	acre-feet		
Approximate 25-yr Detention Volume =	4.820	acre-feet		
Approximate 50-yr Detention Volume =	4.937	acre-feet		
Approximate 100-yr Detention Volume =	6.557	acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.783	acre-feet
Zone 2 Volume (100-year - Zone 1) =	5.774	acre-feet
Select Zone 3 Storage Volume (Optional) =		acre-feet
Total Detention Basin Volume =	6.557	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

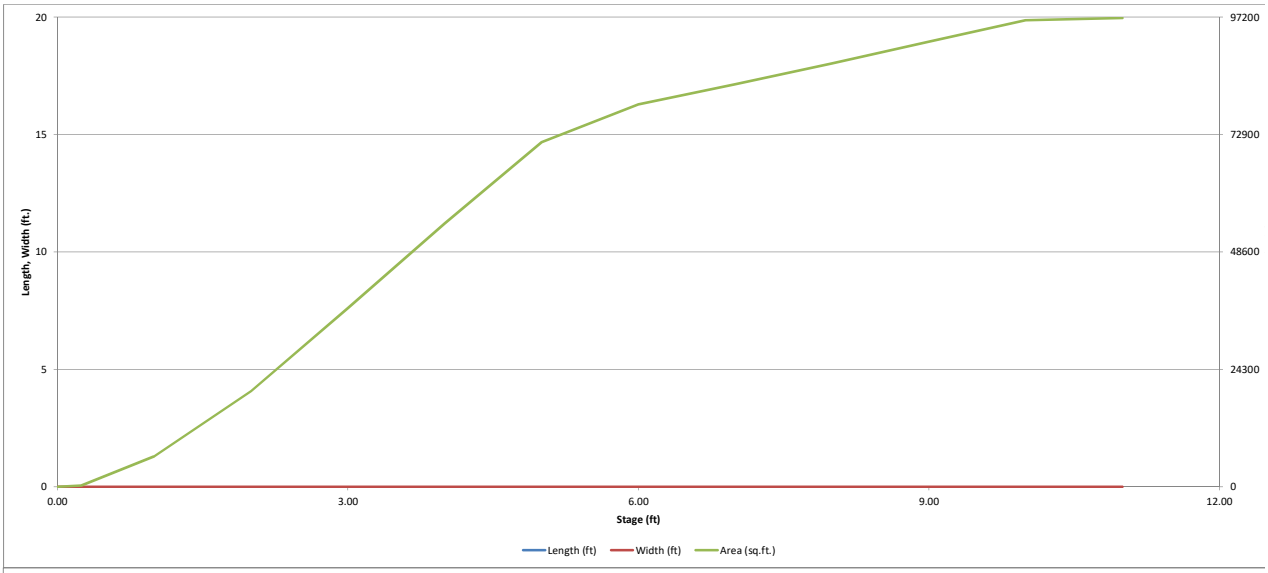
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

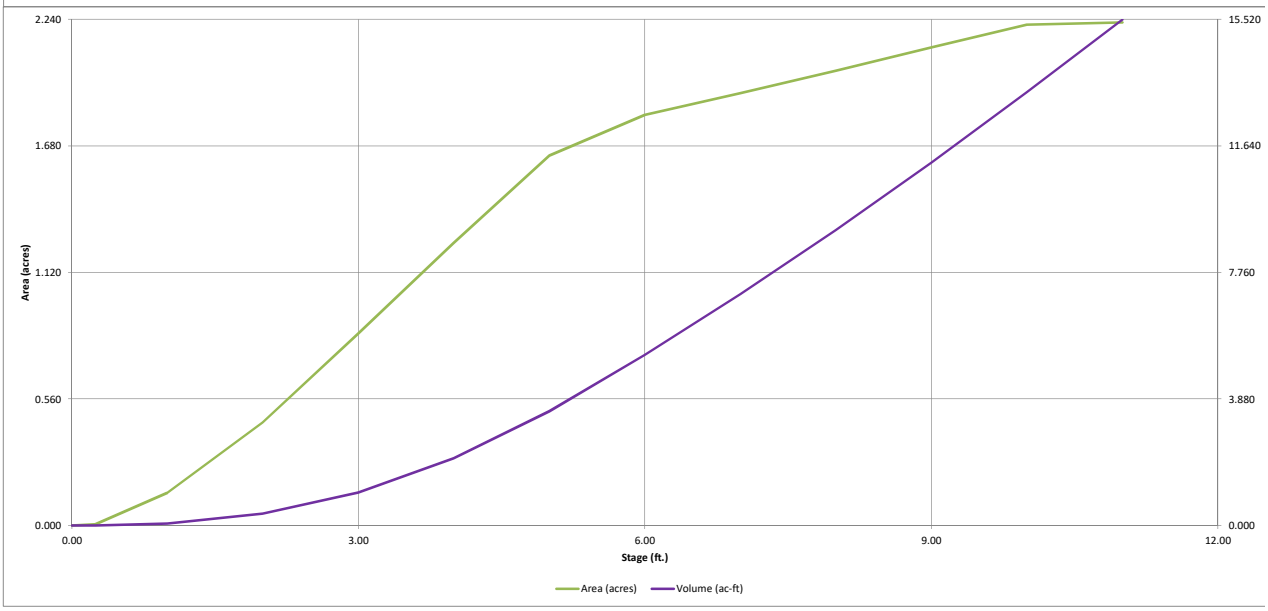
Depth Increment = 1.00 ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	0	0.000		
7093	--	0.25	--	--	--	247	0.006	31	0.001
7094	--	1.00	--	--	--	6,288	0.144	2,481	0.057
7095	--	2.00	--	--	--	19,829	0.455	15,540	0.357
7096	--	3.00	--	--	--	36,992	0.849	43,951	1.009
7097	--	4.00	--	--	--	54,484	1.251	89,689	2.059
7098	--	5.00	--	--	--	71,338	1.638	152,600	3.503
7099	--	6.00	--	--	--	79,154	1.817	227,846	5.231
7100	--	7.00	--	--	--	83,341	1.913	309,093	7.096
7101	--	8.00	--	--	--	87,657	2.012	394,592	9.059
7102	--	9.00	--	--	--	92,163	2.116	484,502	11.123
7103	--	10.00	--	--	--	96,555	2.217	578,861	13.289
7104	--	11.00	--	--	--	97,000	2.227	675,638	15.511

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)



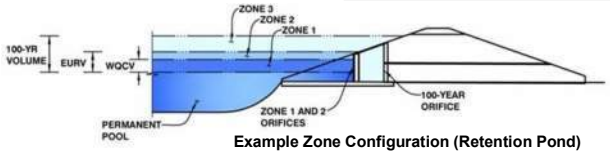


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: **Sterling Ranch - East of Sand Creek**

Basin ID: **Interim Pond FS14A (Total of B)**



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	0.77	0.642	Orifice Plate
Zone 2 (100-year)	4.56	4.026	Weir&Pipe (Restrict)
Zone 3			
Total (all zones)		4.668	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)

Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = ft²

Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)

Depth at top of Zone using Orifice Plate = 4.56 ft (relative to basin bottom at Stage = 0 ft)

Orifice Plate: Orifice Vertical Spacing = N/A inches

Orifice Plate: Orifice Area per Row = 12.00 sq. inches (use rectangular openings)

Calculated Parameters for Plate

WQ Orifice Area per Row = 8.333E-02 ft²

Elliptical Half-Width = N/A feet

Elliptical Slot Centroid = N/A feet

Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.75	1.50	2.25				
Orifice Area (sq. inches)	12.00	12.00	12.00	12.00				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =			ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =			ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =			inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =			ft²
Vertical Orifice Centroid =			feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe)

	Zone 2 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	2.75		ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	7.00		feet
Overflow Weir Grate Slope =	4.00		H:V
Horiz. Length of Weir Sides =	12.42		feet
Overflow Grate Type =	Close Mesh Grate	N/A	
Debris Clogging % =	75%		%

Calculated Parameters for Overflow Weir

	Zone 2 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	5.86		feet
Overflow Weir Slope Length =	12.80		feet
Grate Open Area / 100-yr Orifice Area =	6.33	N/A	
Overflow Grate Open Area w/o Debris =	70.89	N/A	ft²
Overflow Grate Open Area w/ Debris =	17.72	N/A	ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 2 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	3.35		ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	48.00		inches
Restrictor Plate Height Above Pipe Invert =	40.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 2 Restrictor	Not Selected	
Outlet Orifice Area =	11.19		ft²
Outlet Orifice Centroid =	1.80		feet
Half-Central Angle of Restrictor Plate on Pipe =	2.30	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage= 7.50 ft (relative to basin bottom at Stage = 0 ft)

Spillway Crest Length = 120.00 feet

Spillway End Slopes = 4.00 H:V

Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway

Spillway Design Flow Depth= 0.71 feet

Stage at Top of Freeboard = 9.21 feet

Basin Area at Top of Freeboard = 1.51 acres

Basin Volume at Top of Freeboard = 11.04 acre-ft

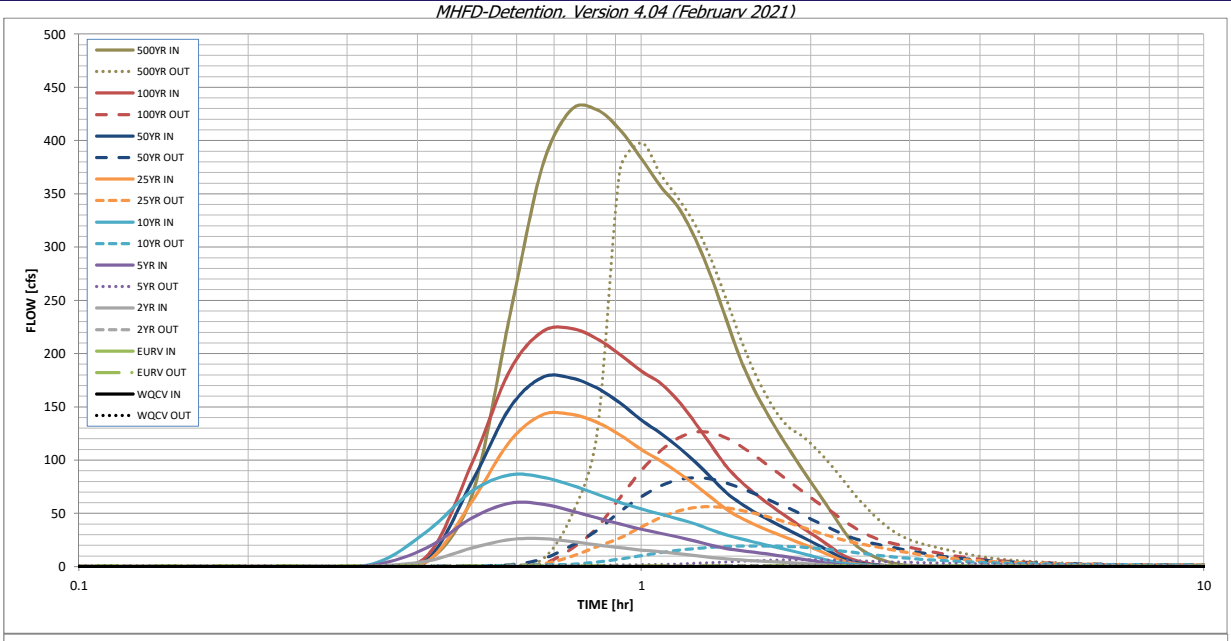
Routed Hydrograph Results

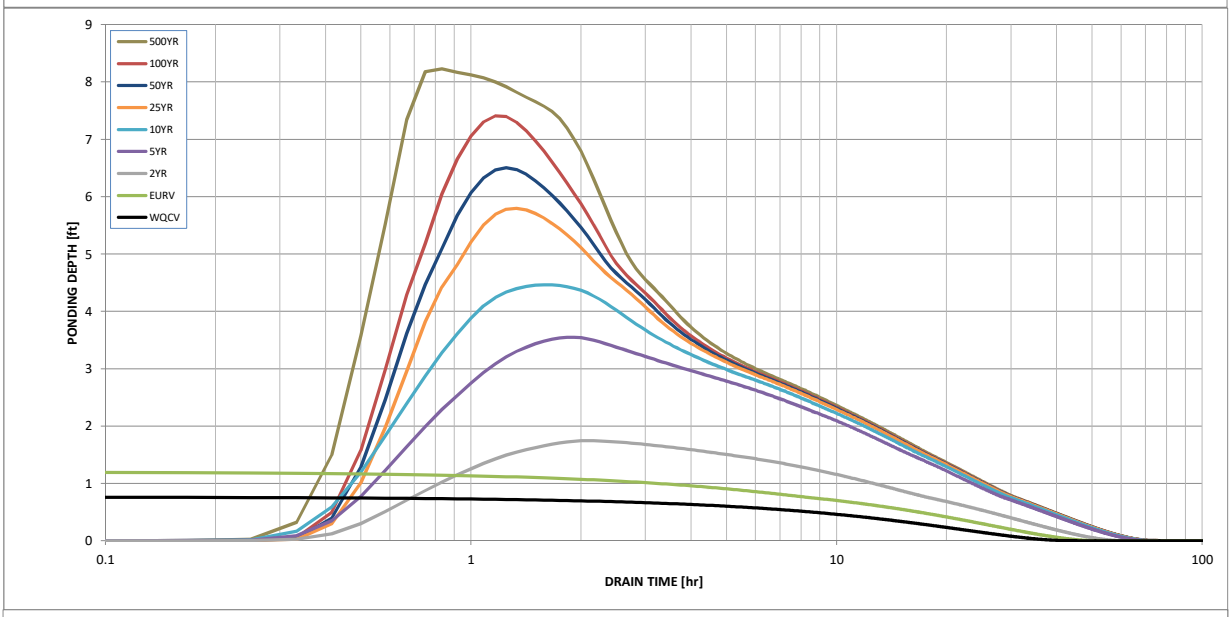
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

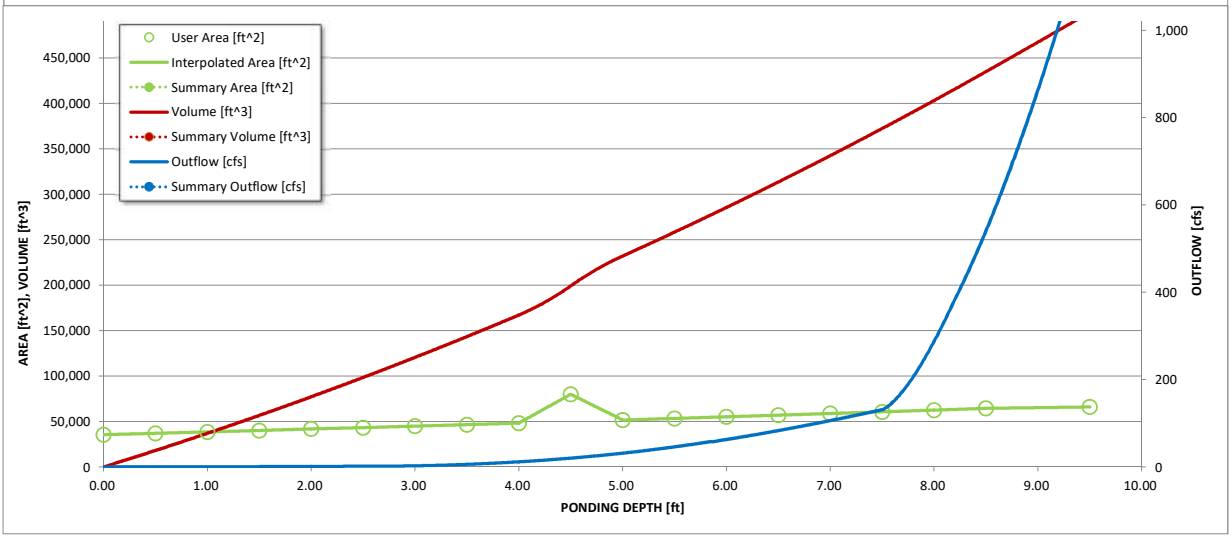
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	4.00
CUHP Runoff Volume (acre-ft) =	0.642	1.027	1.663	3.877	6.117	10.204	12.947	16.941	34.465
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	1.663	3.877	6.117	10.204	12.947	16.941	34.465
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	18.5	50.9	76.5	136.4	170.9	217.2	423.7
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.13	0.36	0.54	0.97	1.21	1.54	3.00
Peak Inflow Q (cfs) =	N/A	N/A	26.2	59.5	86.1	143.0	177.6	223.7	429.0
Peak Outflow Q (cfs) =	0.4	0.7	1.1	6.5	19.5	56.3	83.5	126.3	398.1
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.1	0.3	0.4	0.5	0.6	0.9
Structure Controlling Flow =	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	0.1	0.2	0.7	1.1	1.7	2.3
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	36	43	49	54	50	44	40	35	22
Time to Drain 99% of Inflow Volume (hours) =	**40**	47	54	62	60	57	55	52	42
Maximum Ponding Depth (ft) =	0.77	1.20	1.75	3.55	4.46	5.80	6.50	7.41	8.23
Area at Maximum Ponding Depth (acres) =	0.87	0.90	0.94	1.07	1.78	1.25	1.31	1.38	1.46
Maximum Volume Stored (acre-ft) =	0.648	1.028	1.525	3.335	4.499	6.289	7.197	8.408	9.586

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.10
	0:15:00	0.00	0.00	0.12	0.19	0.24	0.16	0.21	0.20	0.48
	0:20:00	0.00	0.00	0.51	1.43	2.58	0.52	0.62	0.71	4.92
	0:25:00	0.00	0.00	5.32	17.97	33.38	5.07	6.66	10.47	67.02
	0:30:00	0.00	0.00	17.50	45.69	70.88	61.21	79.65	96.91	234.48
	0:35:00	0.00	0.00	25.36	59.46	86.12	117.24	148.69	183.69	375.32
	0:40:00	0.00	0.00	26.24	58.76	84.03	142.45	177.58	220.49	428.69
	0:45:00	0.00	0.00	23.64	52.42	76.64	142.99	177.11	223.69	428.99
	0:50:00	0.00	0.00	20.47	45.90	68.23	135.59	167.85	214.02	410.08
	0:55:00	0.00	0.00	17.75	40.02	60.23	123.59	153.61	199.13	383.11
	1:00:00	0.00	0.00	15.55	35.15	54.13	109.96	137.63	183.60	356.78
	1:05:00	0.00	0.00	13.91	31.26	49.22	99.12	125.10	172.00	336.35
	1:10:00	0.00	0.00	12.21	27.59	44.56	87.89	111.75	154.97	306.83
	1:15:00	0.00	0.00	10.44	23.85	39.94	76.28	97.76	134.25	271.18
	1:20:00	0.00	0.00	8.67	19.98	34.46	64.69	83.28	113.37	231.73
	1:25:00	0.00	0.00	7.20	17.02	29.84	53.79	69.40	94.14	195.21
	1:30:00	0.00	0.00	6.33	15.18	26.26	46.08	59.65	80.41	167.61
	1:35:00	0.00	0.00	5.64	13.61	23.14	39.96	51.83	69.70	145.54
	1:40:00	0.00	0.00	5.01	12.03	20.35	34.88	45.28	60.65	126.71
	1:45:00	0.00	0.00	4.39	10.42	17.78	30.25	39.33	52.51	109.79
	1:50:00	0.00	0.00	3.79	8.85	15.33	26.09	33.97	45.05	94.32
	1:55:00	0.00	0.00	3.17	7.32	12.86	22.07	28.81	38.02	79.76
	2:00:00	0.00	0.00	2.55	5.82	10.35	18.20	23.87	31.45	66.13
	2:05:00	0.00	0.00	1.93	4.33	7.83	14.40	18.98	25.19	52.89
	2:10:00	0.00	0.00	1.30	2.86	5.42	10.59	14.06	18.90	39.69
	2:15:00	0.00	0.00	0.75	1.69	3.64	6.88	9.32	12.81	27.90
	2:20:00	0.00	0.00	0.44	1.06	2.63	4.18	5.90	8.29	19.36
	2:25:00	0.00	0.00	0.30	0.77	2.03	2.63	3.91	5.53	13.76
	2:30:00	0.00	0.00	0.23	0.59	1.58	1.69	2.65	3.68	9.76
	2:35:00	0.00	0.00	0.18	0.46	1.23	1.08	1.78	2.37	6.77
	2:40:00	0.00	0.00	0.13	0.36	0.93	0.70	1.20	1.45	4.54
	2:45:00	0.00	0.00	0.10	0.27	0.69	0.45	0.80	0.81	2.88
	2:50:00	0.00	0.00	0.08	0.20	0.49	0.27	0.51	0.40	1.74
	2:55:00	0.00	0.00	0.06	0.15	0.34	0.18	0.34	0.24	1.10
	3:00:00	0.00	0.00	0.05	0.11	0.23	0.13	0.25	0.18	0.77
	3:05:00	0.00	0.00	0.04	0.08	0.17	0.09	0.18	0.14	0.58
	3:10:00	0.00	0.00	0.03	0.05	0.13	0.07	0.14	0.11	0.46
	3:15:00	0.00	0.00	0.02	0.03	0.09	0.05	0.10	0.08	0.35
	3:20:00	0.00	0.00	0.02	0.02	0.06	0.04	0.08	0.06	0.26
	3:25:00	0.00	0.00	0.01	0.01	0.04	0.03	0.05	0.04	0.18
	3:30:00	0.00	0.00	0.01	0.01	0.02	0.02	0.03	0.03	0.11
	3:35:00	0.00	0.00	0.00	0.00	0.01	0.01	0.02	0.02	0.06
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.03
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

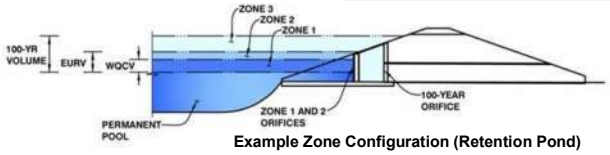
Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: Sterling Ranch - East of Sand Creek

Basin ID: Interim Pond FSD 16 (Total of A and Offsite)



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.72	0.783	Orifice Plate
Zone 2 (100-year)	6.72	5.774	Weir&Pipe (Restrict)
Zone 3			
Total (all zones)		6.557	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =		ft (distance below the filtration media surface)	Underdrain Orifice Area =		ft²
Underdrain Orifice Diameter =		inches	Underdrain Orifice Centroid =		feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	2.146E-02	ft²
Depth at top of Zone using Orifice Plate =	2.72	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	N/A	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	3.09	sq. inches (diameter = 2 inches)	Elliptical Slot Area =	N/A	ft²

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.00	2.00					
Orifice Area (sq. inches)	3.09	3.09	3.09					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected			Not Selected	Not Selected	
Invert of Vertical Orifice =			ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =			ft²
Depth at top of Zone using Vertical Orifice =			ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =			feet
Vertical Orifice Diameter =			inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe)

	Zone 2 Weir	Not Selected			Zone 2 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	3.50		ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	6.00		feet
Overflow Weir Front Edge Length =	6.00		feet	Overflow Weir Slope Length =	10.31		feet
Overflow Weir Grate Slope =	4.00		H:V	Grate Open Area / 100-yr Orifice Area =	5.31		
Horiz. Length of Weir Sides =	10.00		feet	Overflow Grate Open Area w/o Debris =	48.92		ft²
Overflow Grate Type =	Close Mesh Grate			Overflow Grate Open Area w/ Debris =	12.23		ft²
Debris Clogging % =	75%		%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 2 Restrictor	Not Selected			Zone 2 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	4.00		ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	9.21		ft²
Outlet Pipe Diameter =	48.00		inches	Outlet Orifice Centroid =	1.54		feet
Restrictor Plate Height Above Pipe Invert =	33.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	1.96	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	9.07	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.79 feet
Spillway Crest Length =	120.00	feet	Stage at Top of Freeboard =	10.86 feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	2.23 acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	15.20 acre-ft

Calculated Parameters for Spillway

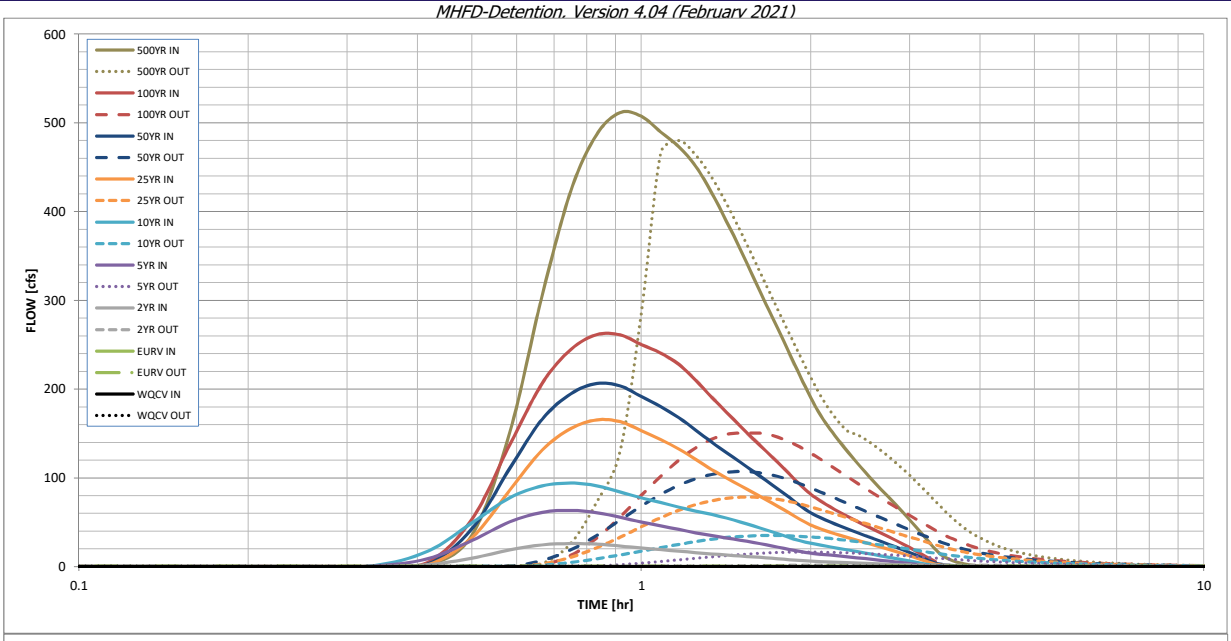
Routed Hydrograph Results

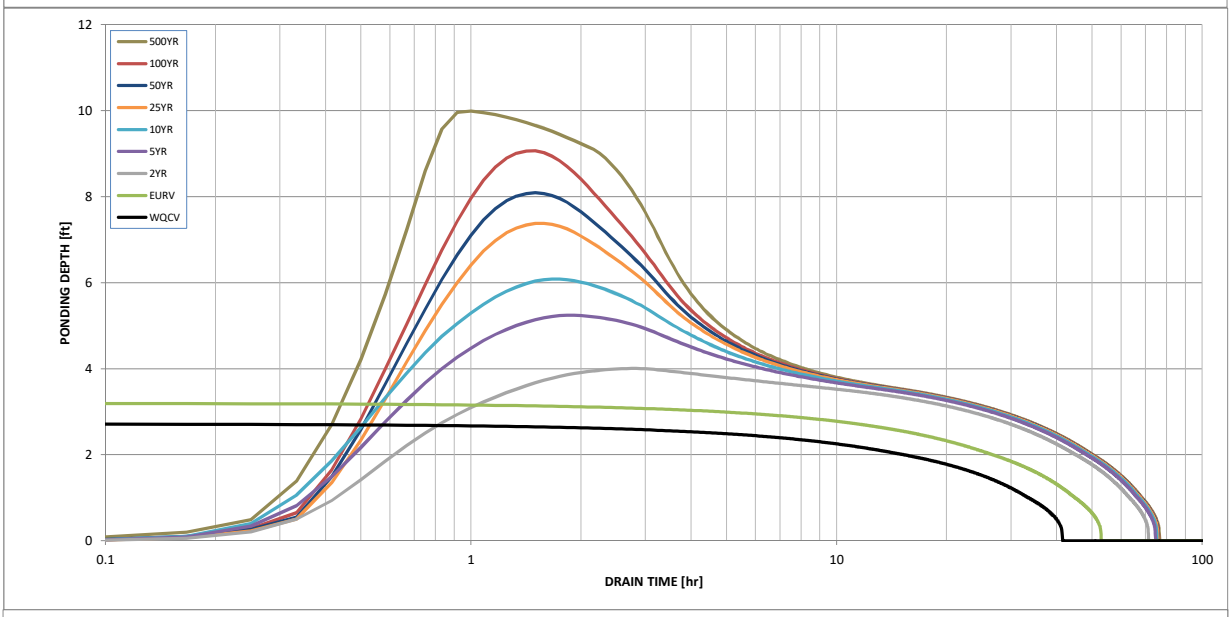
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

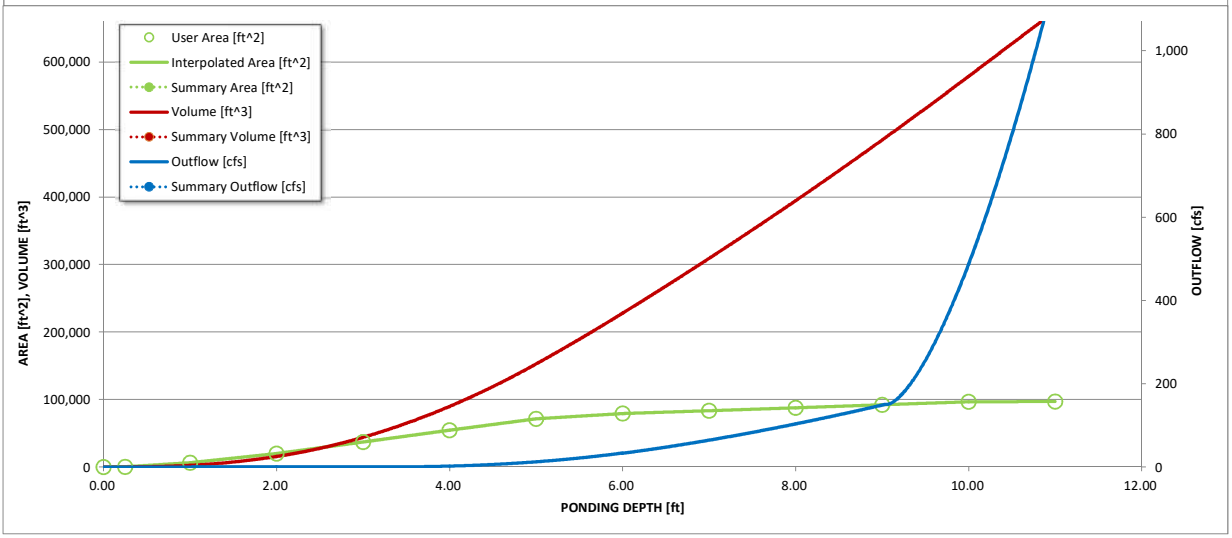
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	4.00
CUHP Runoff Volume (acre-ft) =	0.783	1.184	2.329	5.787	9.335	15.975	20.358	26.812	54.919
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	2.329	5.787	9.335	15.975	20.358	26.812	54.919
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	20.0	56.8	87.5	158.6	199.3	256.2	504.0
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.09	0.25	0.39	0.70	0.88	1.13	2.22
Peak Inflow Q (cfs) =	N/A	N/A	26.1	63.3	94.2	165.3	206.3	261.3	511.4
Peak Outflow Q (cfs) =	0.4	0.5	2.2	16.4	35.3	78.4	107.2	150.6	479.9
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.3	0.4	0.5	0.5	0.6	1.0
Structure Controlling Flow =	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	0.03	0.3	0.7	1.6	2.2	3.1	3.2
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	48	62	58	53	44	40	34	12
Time to Drain 99% of Inflow Volume (hours) =	**40**	51	68	66	64	60	58	54	43
Maximum Ponding Depth (ft) =	2.72	3.20	4.00	5.24	6.09	7.38	8.09	9.06	9.99
Area at Maximum Ponding Depth (acres) =	0.74	0.93	1.25	1.68	1.82	1.95	2.02	2.12	2.21
Maximum Volume Stored (acre-ft) =	0.787	1.187	2.059	3.901	5.376	7.810	9.220	11.250	13.245

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.03
	0:15:00	0.00	0.00	0.03	0.06	0.07	0.05	0.06	0.06	0.16
	0:20:00	0.00	0.00	0.17	0.57	1.03	0.19	0.23	0.28	1.99
	0:25:00	0.00	0.00	2.16	8.87	17.26	2.07	2.71	4.74	35.65
	0:30:00	0.00	0.00	9.57	29.49	49.33	32.64	42.76	53.48	150.60
	0:35:00	0.00	0.00	18.81	50.34	77.41	85.94	110.46	136.90	305.66
	0:40:00	0.00	0.00	24.50	61.18	90.91	131.37	166.09	206.58	423.88
	0:45:00	0.00	0.00	26.05	63.30	94.19	155.64	194.88	244.74	486.82
	0:50:00	0.00	0.00	25.31	60.82	91.12	165.32	206.27	261.29	511.39
	0:55:00	0.00	0.00	23.25	55.61	84.04	163.34	203.71	261.04	507.38
	1:00:00	0.00	0.00	20.88	50.27	77.52	153.21	191.62	250.26	489.51
	1:05:00	0.00	0.00	19.11	45.97	72.14	143.11	180.13	240.58	472.92
	1:10:00	0.00	0.00	17.42	41.89	66.98	132.43	167.72	227.77	450.92
	1:15:00	0.00	0.00	15.76	38.15	62.56	120.71	153.83	209.81	421.43
	1:20:00	0.00	0.00	14.32	35.02	58.79	109.46	140.28	190.93	389.32
	1:25:00	0.00	0.00	13.14	32.36	54.76	100.05	128.60	174.02	357.44
	1:30:00	0.00	0.00	12.05	29.78	50.36	91.37	117.55	158.24	325.87
	1:35:00	0.00	0.00	10.98	27.23	45.88	83.19	107.04	143.61	295.85
	1:40:00	0.00	0.00	9.93	24.64	41.42	75.42	97.06	130.11	267.72
	1:45:00	0.00	0.00	8.88	22.00	37.04	67.86	87.37	117.06	240.63
	1:50:00	0.00	0.00	7.85	19.35	32.80	60.42	77.88	104.37	214.63
	1:55:00	0.00	0.00	6.87	16.96	29.12	53.15	68.63	92.18	190.48
	2:00:00	0.00	0.00	6.13	15.23	26.42	46.95	60.80	81.78	170.52
	2:05:00	0.00	0.00	5.64	14.03	24.29	42.41	55.07	73.92	154.79
	2:10:00	0.00	0.00	5.21	12.96	22.33	38.71	50.29	67.33	141.14
	2:15:00	0.00	0.00	4.81	11.93	20.48	35.46	46.04	61.45	128.71
	2:20:00	0.00	0.00	4.42	10.94	18.71	32.51	42.15	56.10	117.26
	2:25:00	0.00	0.00	4.03	9.97	16.98	29.76	38.53	51.14	106.59
	2:30:00	0.00	0.00	3.65	9.02	15.32	27.11	35.05	46.48	96.59
	2:35:00	0.00	0.00	3.29	8.08	13.71	24.55	31.72	42.14	87.29
	2:40:00	0.00	0.00	2.92	7.15	12.15	22.06	28.50	37.99	78.44
	2:45:00	0.00	0.00	2.56	6.24	10.65	19.59	25.32	33.85	69.80
	2:50:00	0.00	0.00	2.20	5.34	9.19	17.12	22.17	29.73	61.28
	2:55:00	0.00	0.00	1.84	4.44	7.73	14.67	19.03	25.62	52.78
	3:00:00	0.00	0.00	1.48	3.55	6.28	12.22	15.90	21.50	44.30
	3:05:00	0.00	0.00	1.12	2.66	4.84	9.78	12.77	17.39	35.83
	3:10:00	0.00	0.00	0.76	1.78	3.41	7.34	9.65	13.29	27.41
	3:15:00	0.00	0.00	0.43	1.00	2.18	4.92	6.56	9.25	19.45
	3:20:00	0.00	0.00	0.20	0.54	1.49	2.89	3.99	5.86	13.23
	3:25:00	0.00	0.00	0.12	0.38	1.14	1.76	2.57	3.84	9.27
	3:30:00	0.00	0.00	0.09	0.28	0.88	1.09	1.69	2.53	6.54
	3:35:00	0.00	0.00	0.07	0.22	0.69	0.68	1.12	1.62	4.51
	3:40:00	0.00	0.00	0.05	0.17	0.53	0.41	0.73	0.98	3.01
	3:45:00	0.00	0.00	0.04	0.13	0.40	0.26	0.48	0.54	1.90
	3:50:00	0.00	0.00	0.03	0.10	0.28	0.15	0.30	0.26	1.12
	3:55:00	0.00	0.00	0.02	0.07	0.19	0.09	0.19	0.12	0.65
	4:00:00	0.00	0.00	0.02	0.05	0.13	0.07	0.13	0.09	0.43
	4:05:00	0.00	0.00	0.02	0.04	0.09	0.05	0.09	0.07	0.31
	4:10:00	0.00	0.00	0.01	0.02	0.06	0.03	0.07	0.06	0.24
	4:15:00	0.00	0.00	0.01	0.02	0.05	0.03	0.05	0.04	0.19
	4:20:00	0.00	0.00	0.01	0.01	0.03	0.02	0.04	0.03	0.14
	4:25:00	0.00	0.00	0.00	0.01	0.02	0.01	0.03	0.02	0.10
	4:30:00	0.00	0.00	0.00	0.00	0.01	0.01	0.02	0.01	0.06
	4:35:00	0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.01	0.04
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.02
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

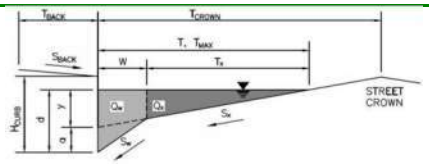
Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP1



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	17.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	39.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.009	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	27.0	27.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	8.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

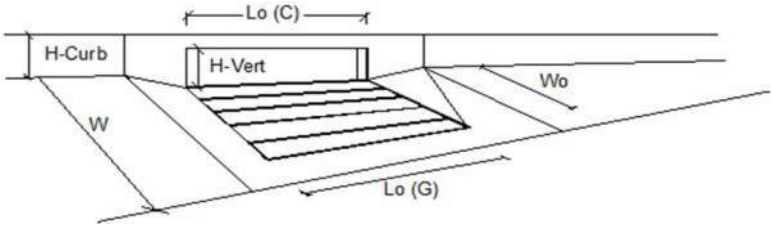
		Minor Storm	Major Storm	
	Q_{allow} =	16.1	42.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



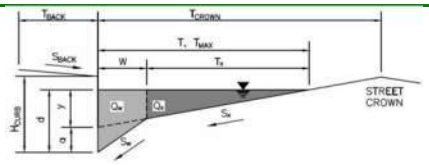
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	4	4	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	10.4	16.9	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.1	4.3	cfs
Capture Percentage = Q_a/Q_o =	C% =	99	80	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP2



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	17.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	39.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.009	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	27.0	27.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	8.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

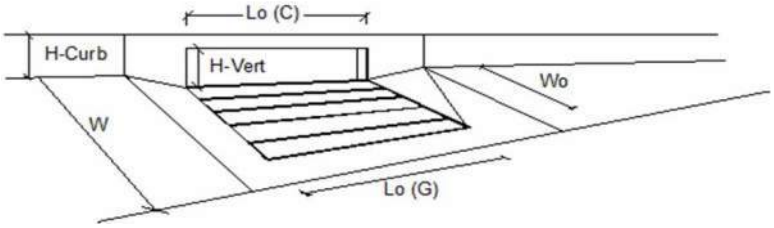
		Minor Storm	Major Storm	
	Q_{allow} =	16.1	42.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	4	4	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

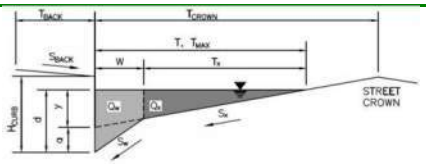
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	10.4	16.0	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.1	3.3	cfs
Capture Percentage = Q_a/Q_o =	C% =	99	83	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP4



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	17.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	39.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.009	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	27.0	27.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	8.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

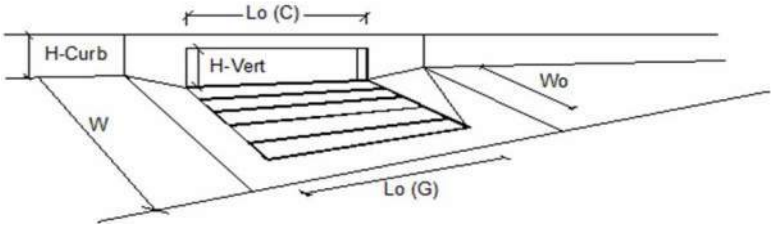
		Minor Storm	Major Storm	
	Q_{allow} =	16.1	42.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



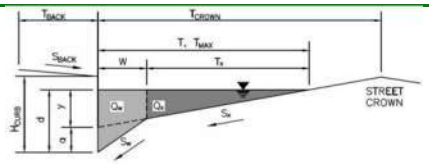
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	4.5	10.2	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	2.0	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	83	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP6



Gutter Geometry:			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	17.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	39.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.009	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	27.0	27.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	8.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

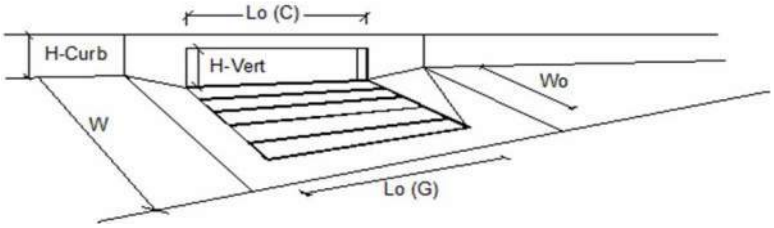
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Spread Criterion	Q_{allow} =	16.1	42.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



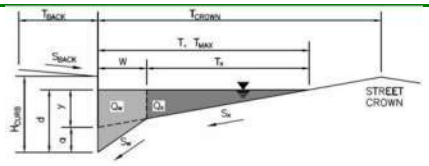
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	5.5	10.5	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	2.5	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	81	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP7



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.022	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	8.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

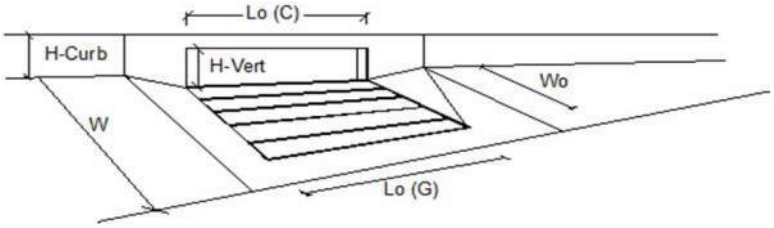
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	24.0	51.9	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

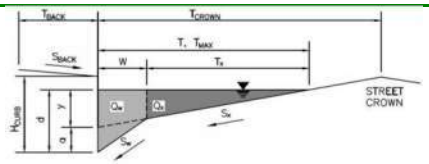
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	5.1	9.6	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	1.3	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	88	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP8



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Depth Criterion

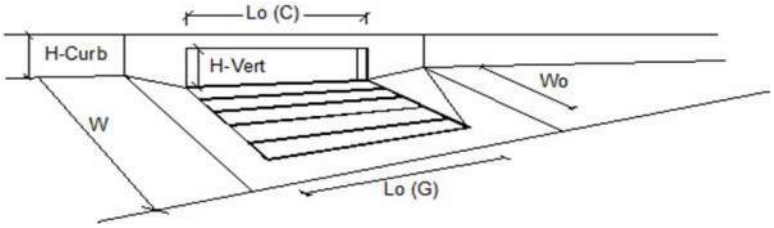
		Minor Storm	Major Storm	
	Q_{allow} =	20.8	20.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



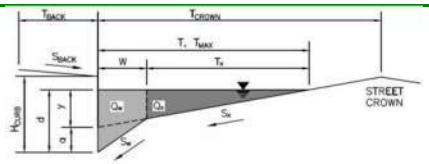
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	5.2	10.5	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	2.3	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	82	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP10



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	29.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.020	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Depth Criterion

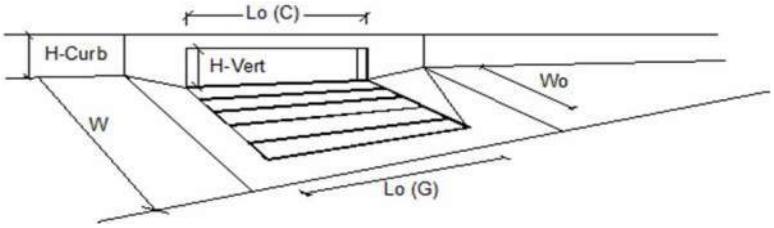
		Minor Storm	Major Storm	
	Q_{allow} =	24.0	24.0	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	

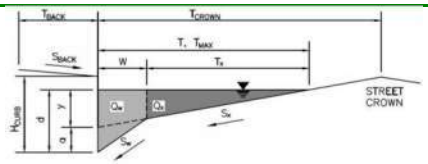
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	3.3	6.2	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	2.0	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	76	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP11



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	29.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.020	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Depth Criterion

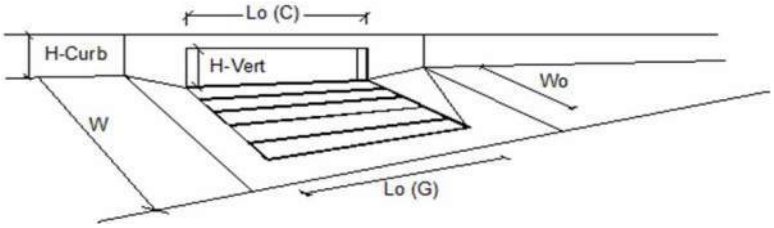
	Minor Storm	Major Storm	
Q_{allow} =	24.0	24.0	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



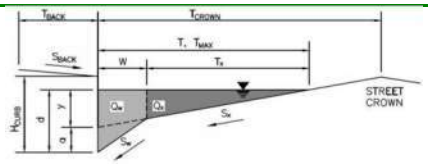
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	3.3	5.6	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	1.2	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	82	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP14



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	29.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Depth Criterion

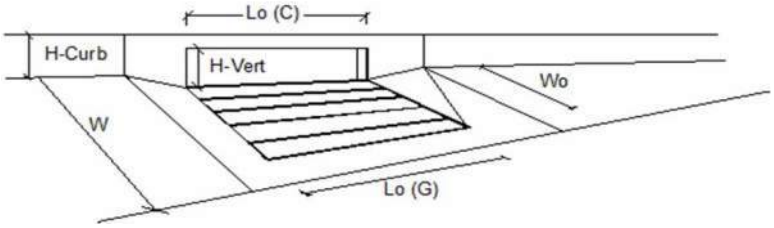
		Minor Storm	Major Storm	
	Q_{allow} =	20.8	20.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



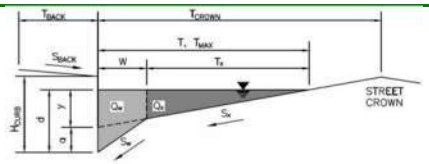
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	2.5	5.7	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	1.4	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	80	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP15



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	29.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Depth Criterion

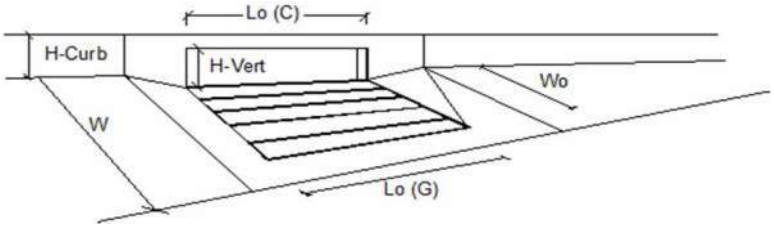
		Minor Storm	Major Storm	
	Q_{allow} =	20.8	20.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



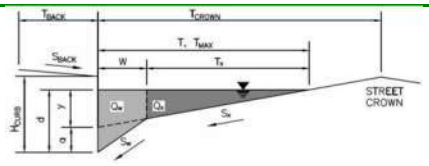
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	2.7	5.2	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	0.9	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	85	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP18



Gutter Geometry:			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

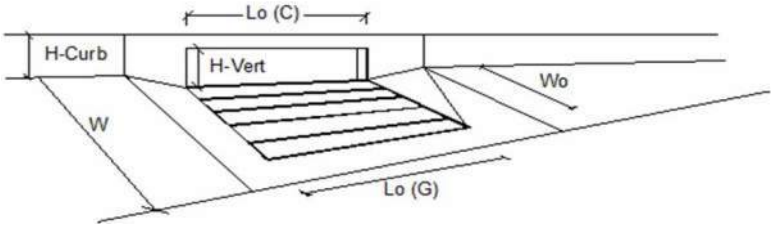
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	20.8	20.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



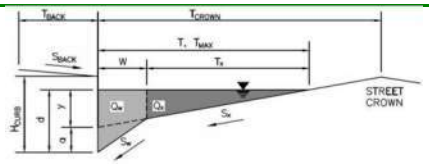
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	2.9	5.9	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	1.7	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	78	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP19



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

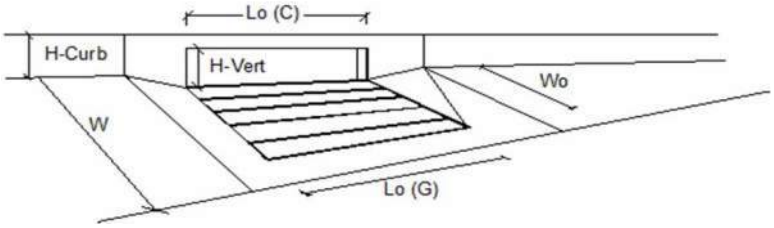
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	20.8	20.8	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.01 (April 2021)



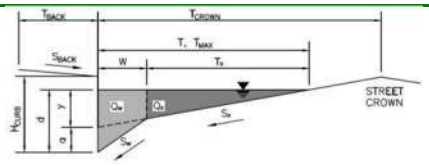
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	2	2	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f-G =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f-C =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Total Inlet Interception Capacity	Q =	2.4	4.8	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	Q_b =	0.0	0.5	cfs
Capture Percentage = Q_a/Q_o =	C% =	100	90	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP22



Gutter Geometry:

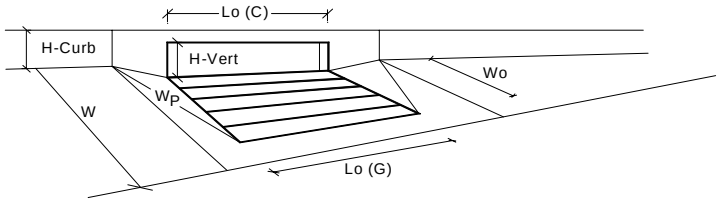
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.025	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	SUMP	SUMP	cfs

INLET IN A SUMP OR SAG LOCATION

MHFD-Inlet, Version 5.01 (April 2021)



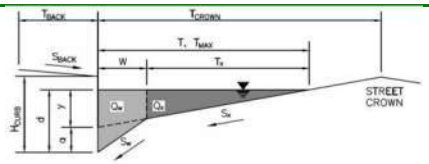
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	3	3	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	6.0	inches
Grate Information		MINOR	MAJOR	☐ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.33	0.33	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.57	0.57	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.79	0.79	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	Q_a =	13.5	13.5	cfs
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	4.7	12.7	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: Sterling Ranch Road & Briargate Parkway - East of Sand Creek

Inlet ID: DP23



Gutter Geometry:

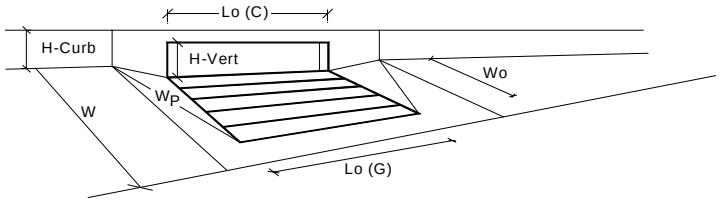
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	39.0	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.013	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	26.0	ft
Gutter Width	W =	2.00	ft
Street Transverse Slope	S_X =	0.025	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.013	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	26.0	26.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	6.0	6.0	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

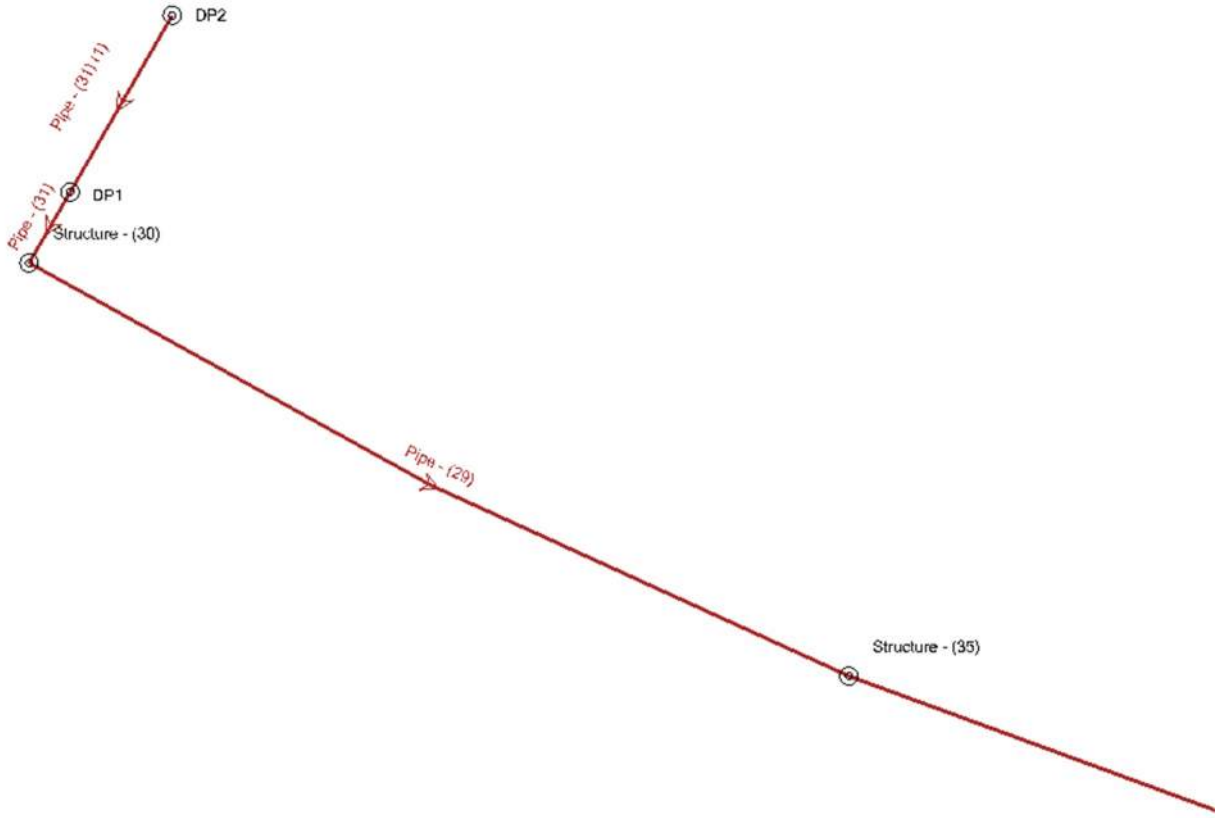
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	Q_{allow} =	SUMP	SUMP	cfs

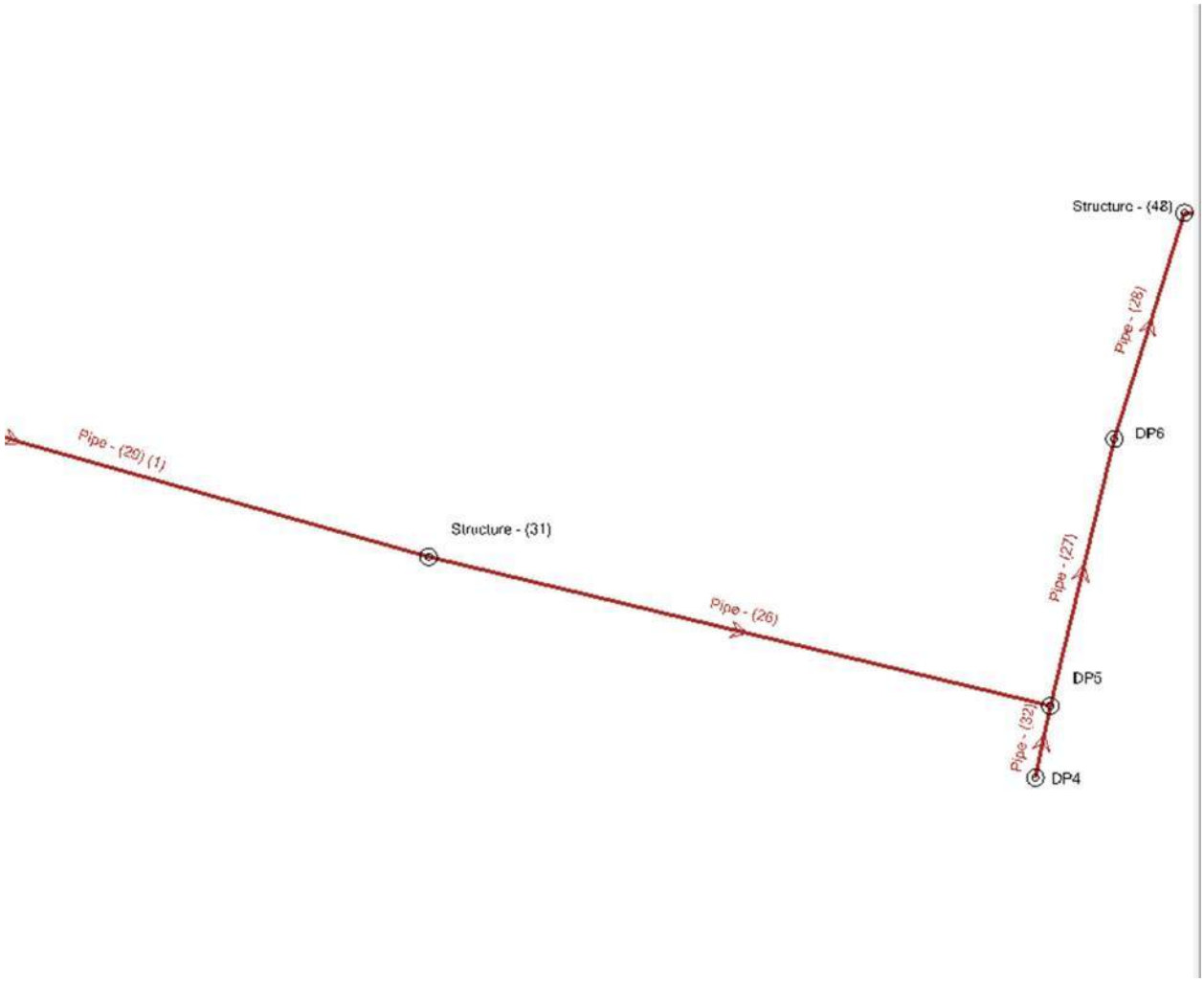
INLET IN A SUMP OR SAG LOCATION

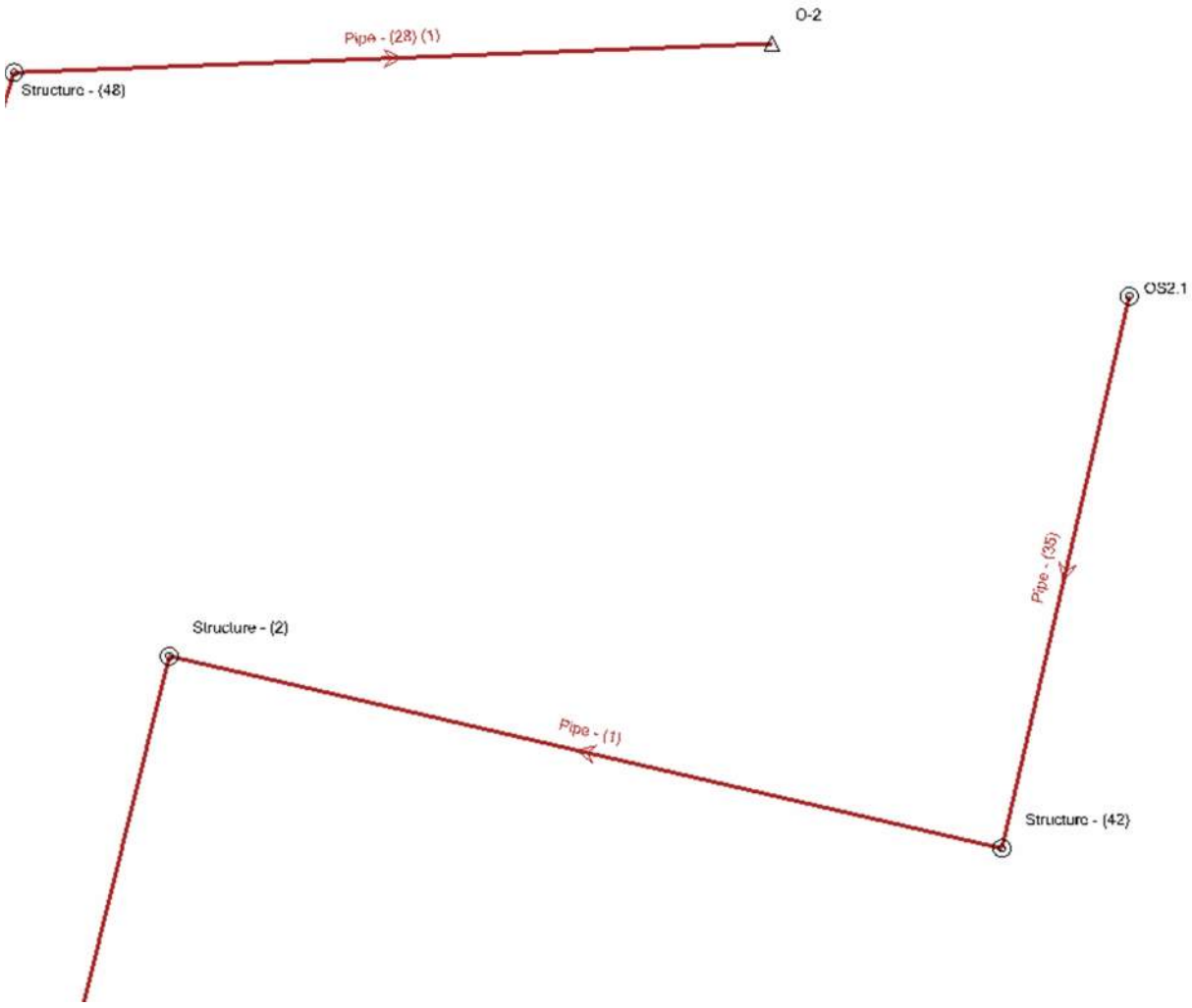
MHFD-Inlet, Version 5.01 (April 2021)



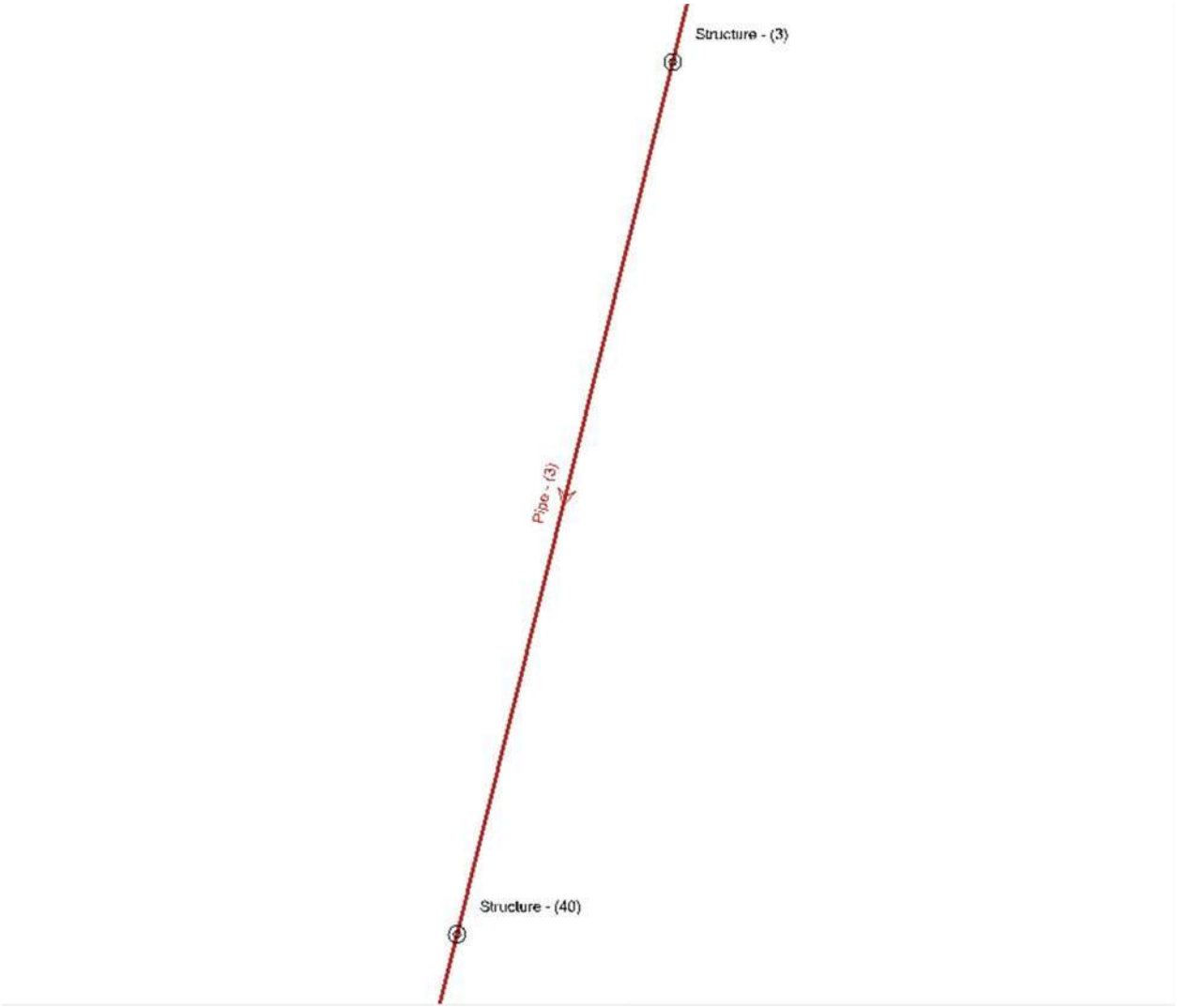
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	2	2	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	6.0	6.0	inches
Grate Information		MINOR	MAJOR	☐ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.33	0.33	ft
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	0.57	0.57	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.93	0.93	
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	Q_a =	10.5	10.5	cfs
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)	$Q_{PEAK\ REQUIRED}$ =	4.3	10.2	cfs

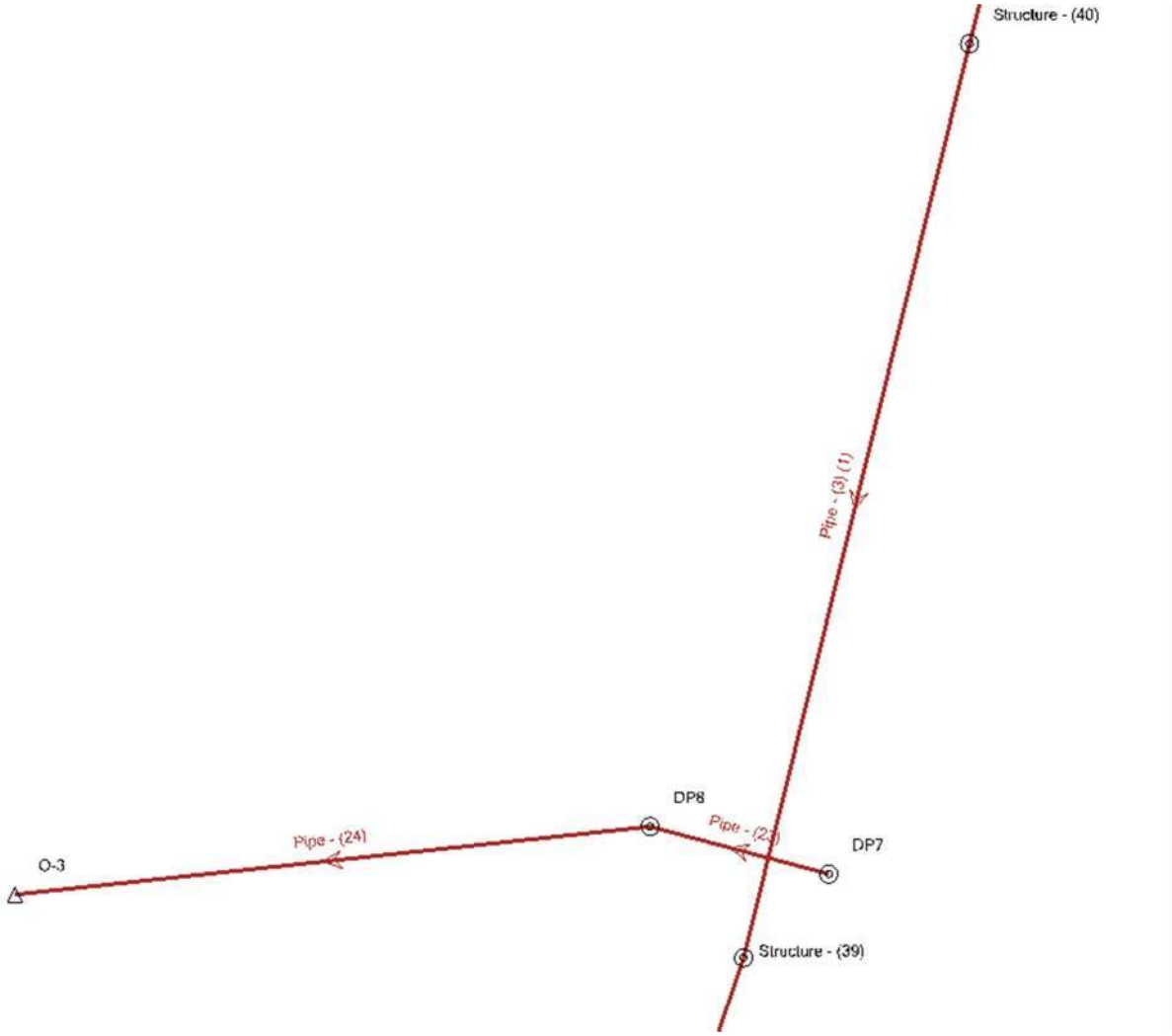

DP2
Pipe - (31) (1)
DP1
Pipe - (31)
Structure - (30)
Pipe - (29)
Structure - (35)

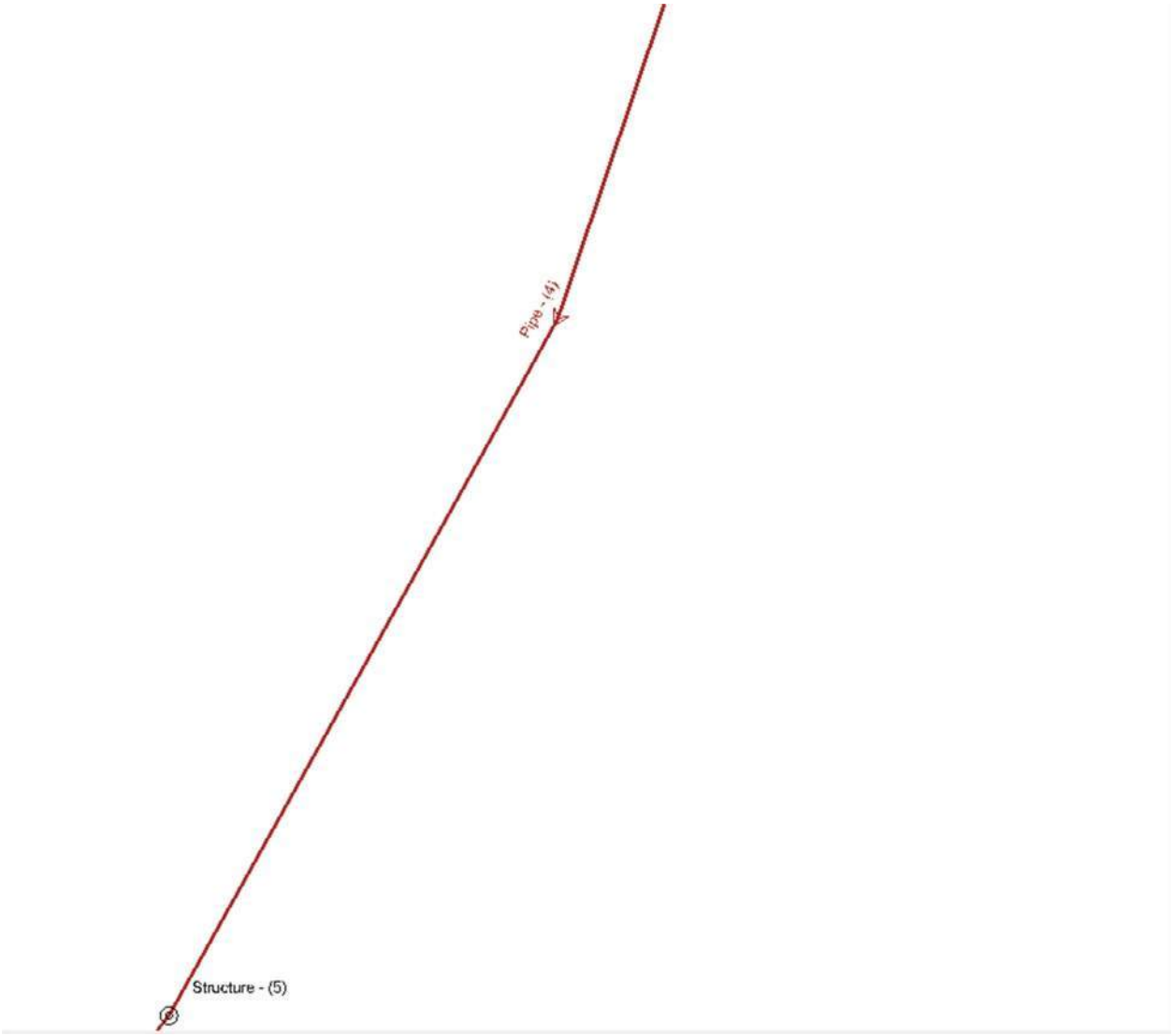

Structure - (48)
Pipe - (28)
DP6
Pipe - (20) (1)
Structure - (31)
Pipe - (27)
Pipe - (26)
DP5
Pipe - (32)
DP4

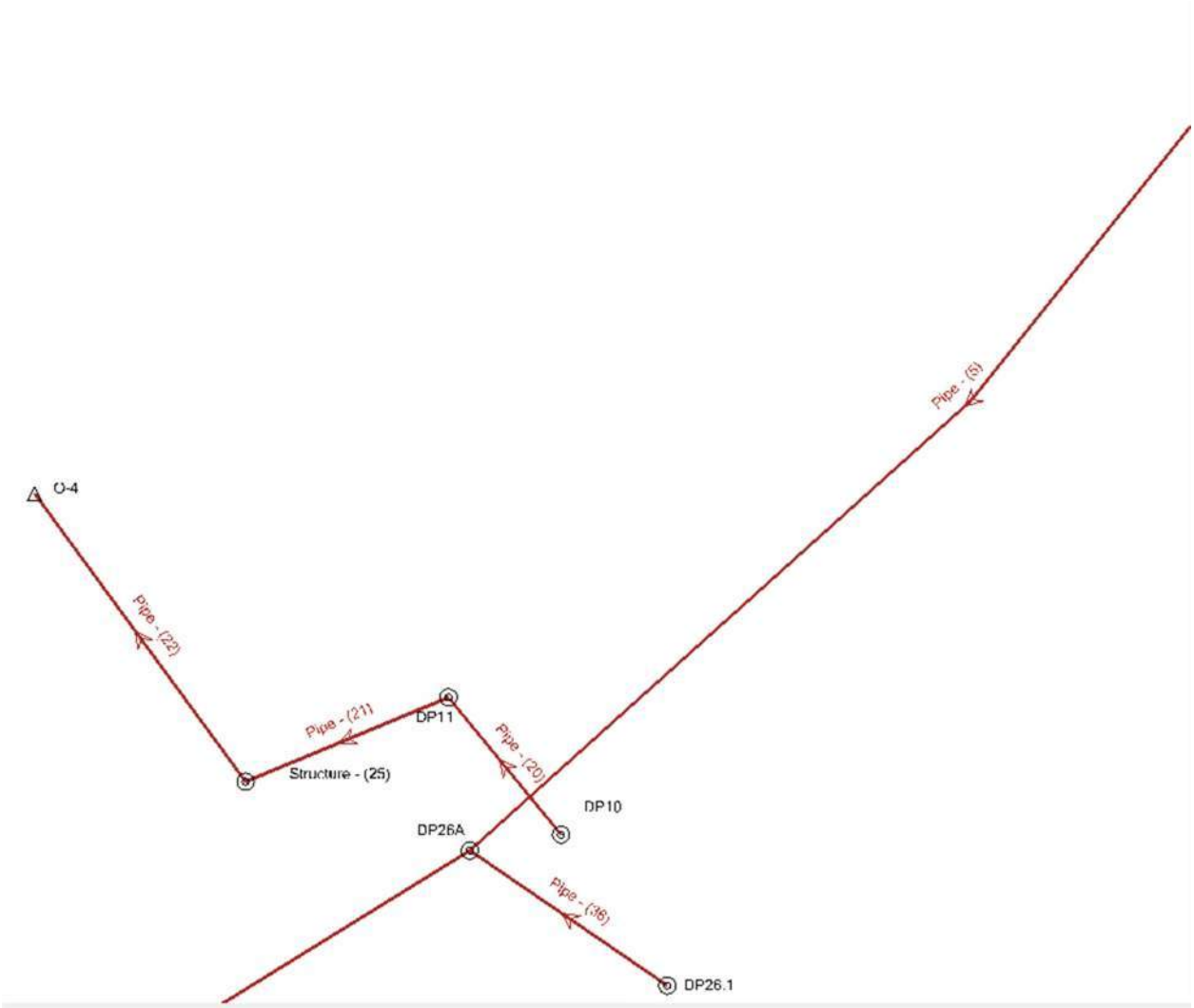

O-2
Pipe - (28) (1)
Structure - (48)
OS2.1
Pipe - (35)
Structure - (2)
Pipe - (1)
Structure - (42)

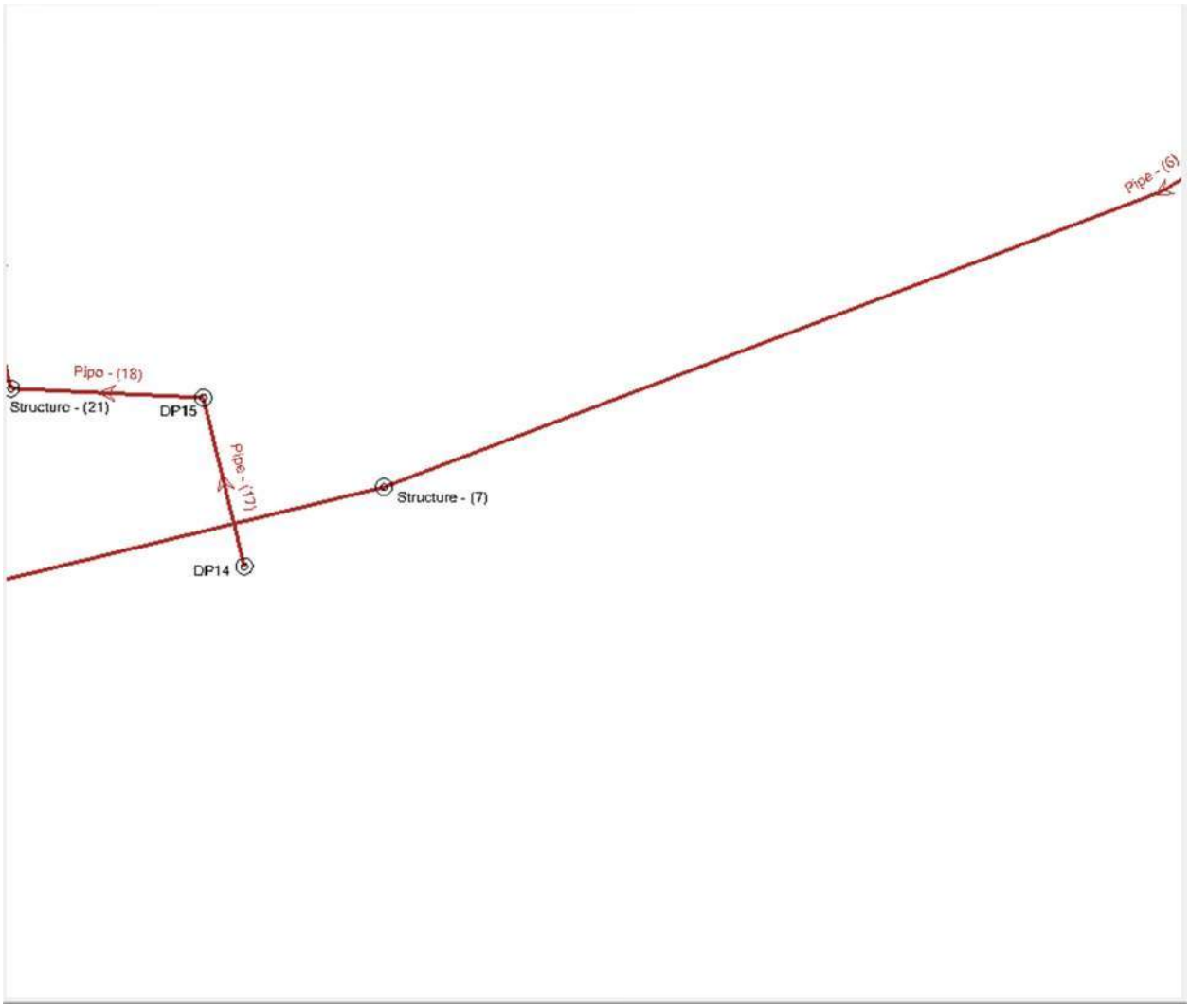

Pipe - (2)
Structure - (3)

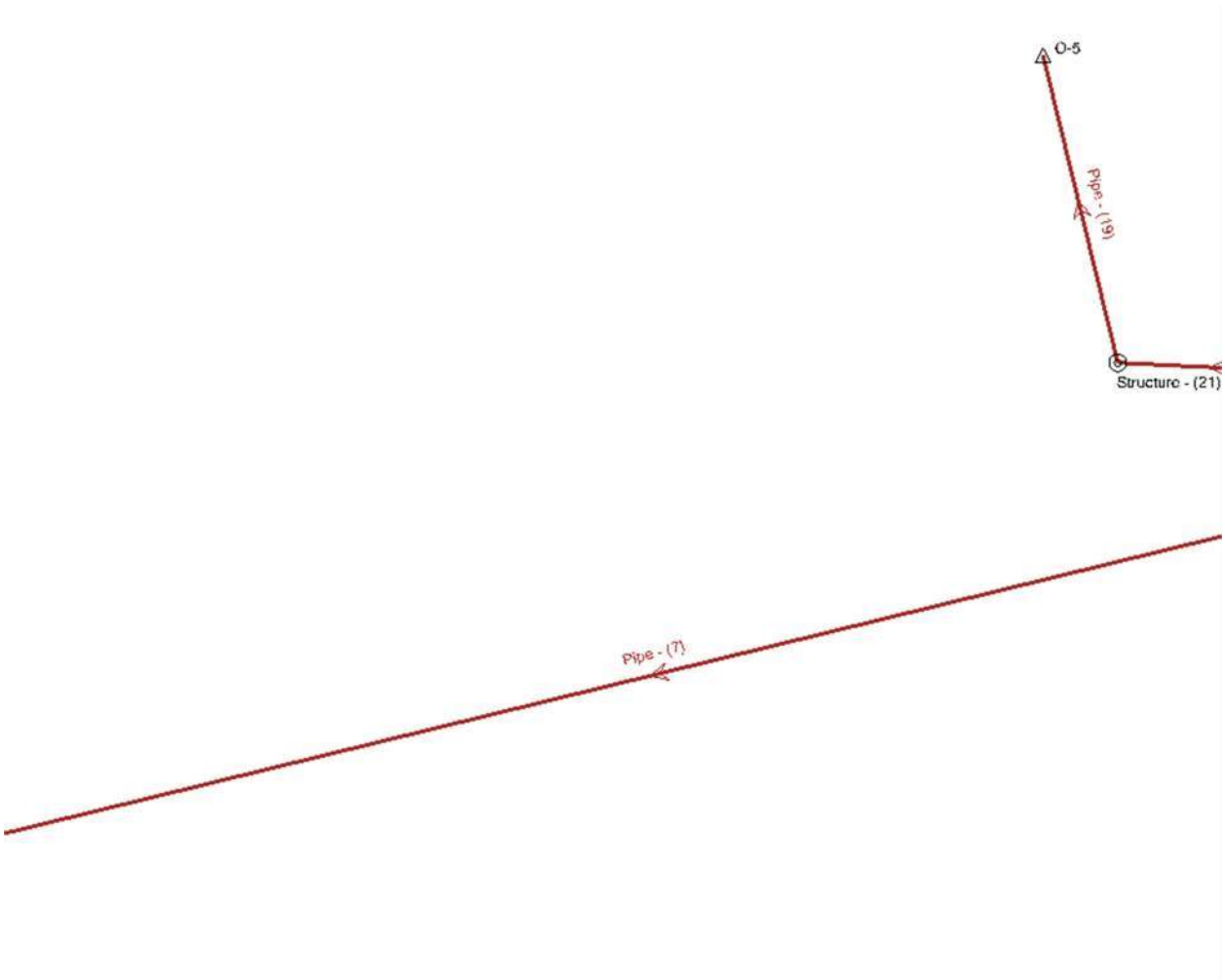

Structure - (3)
Pipe - (3)
Structure - (40)

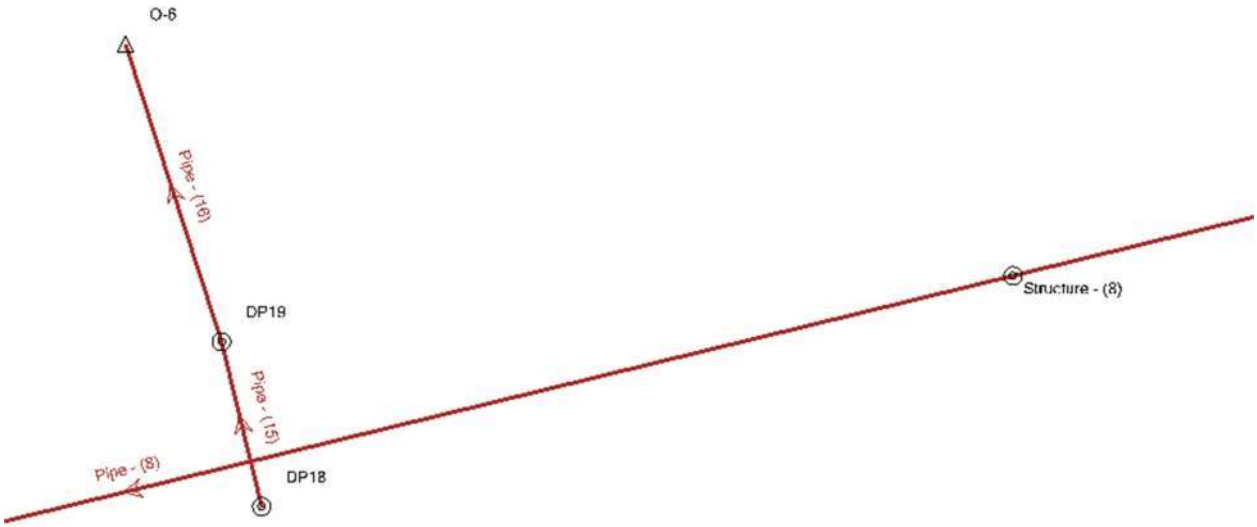

Structure - (40)
Pipe - (3) (1)
DP8
Pipe - (24)
Pipe - (23)
DP7
O-3
Structure - (39)

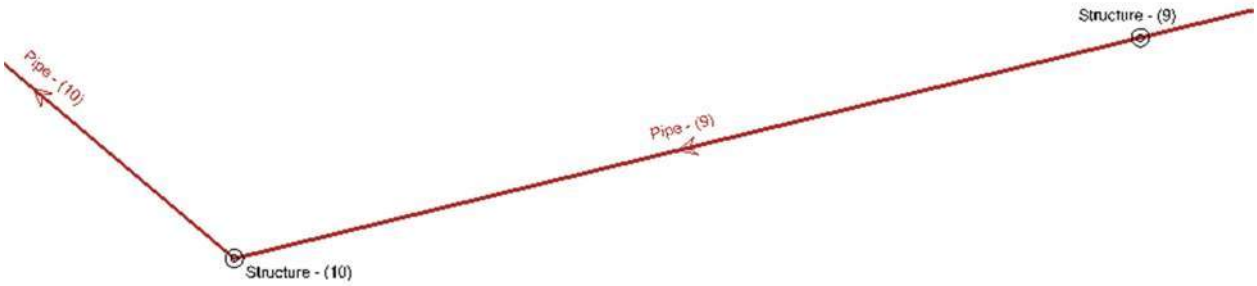

Pipe - (4)
Structure - (5)

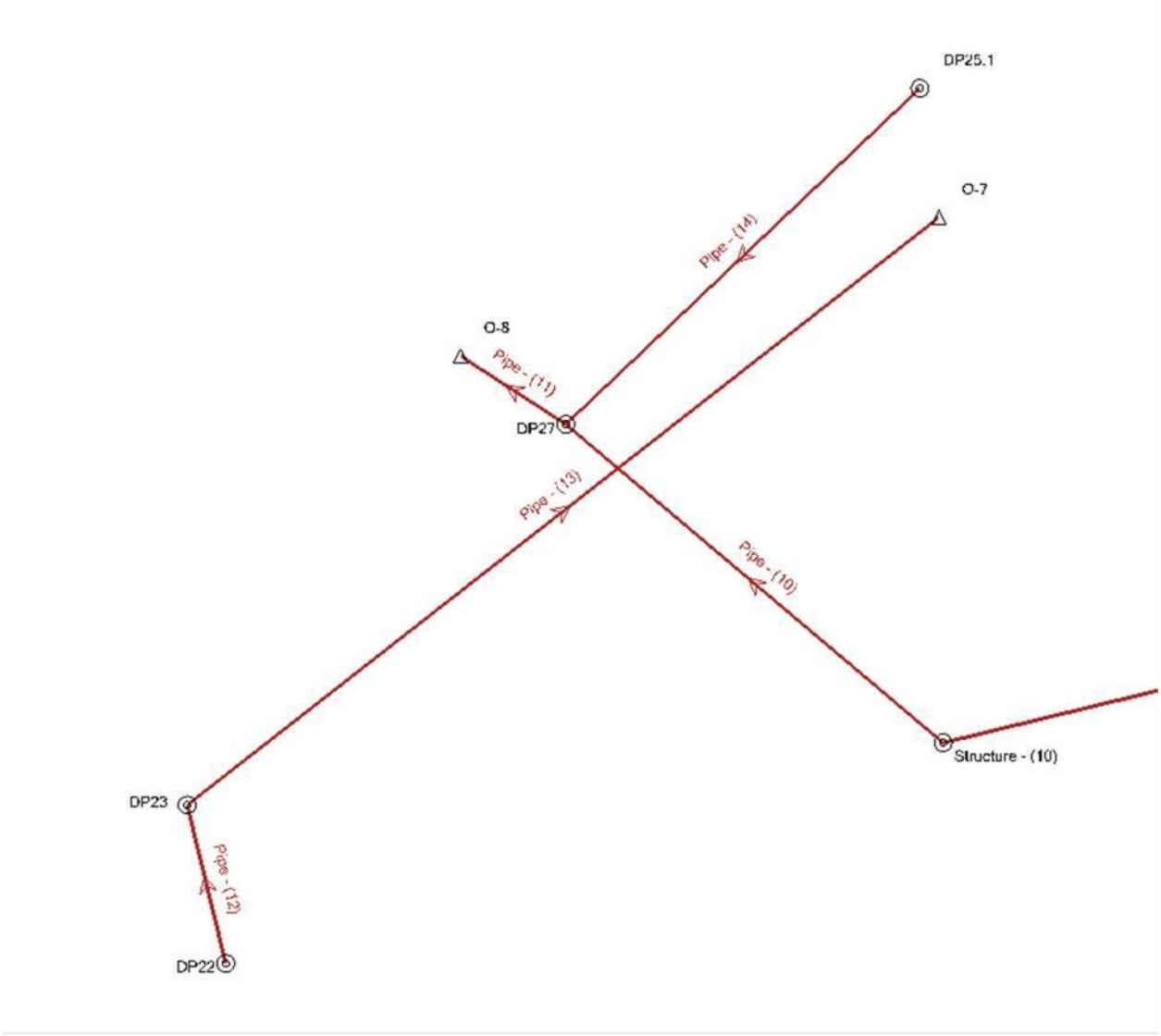

O-4
Pipe - (22)
Pipe - (21)
DP11
Structure - (25)
Pipe - (20)
DP10
DP26A
Pipe - (5)
Pipe - (36)
DP26.1


Pipe - (6)
Pipo - (18)
Structure - (21)
DP15
Pipe - (17)
Structure - (7)
DP14


O-5
Pipe - (6)
Structure - (21)
Pipe - (7)


O-6
Pipe - (16)
DP19
Structure - (8)
Pipe - (15)
Pipe - (8)
DP18


Structure - (9)
Pipe - (10)
Pipe - (9)
Structure - (10)

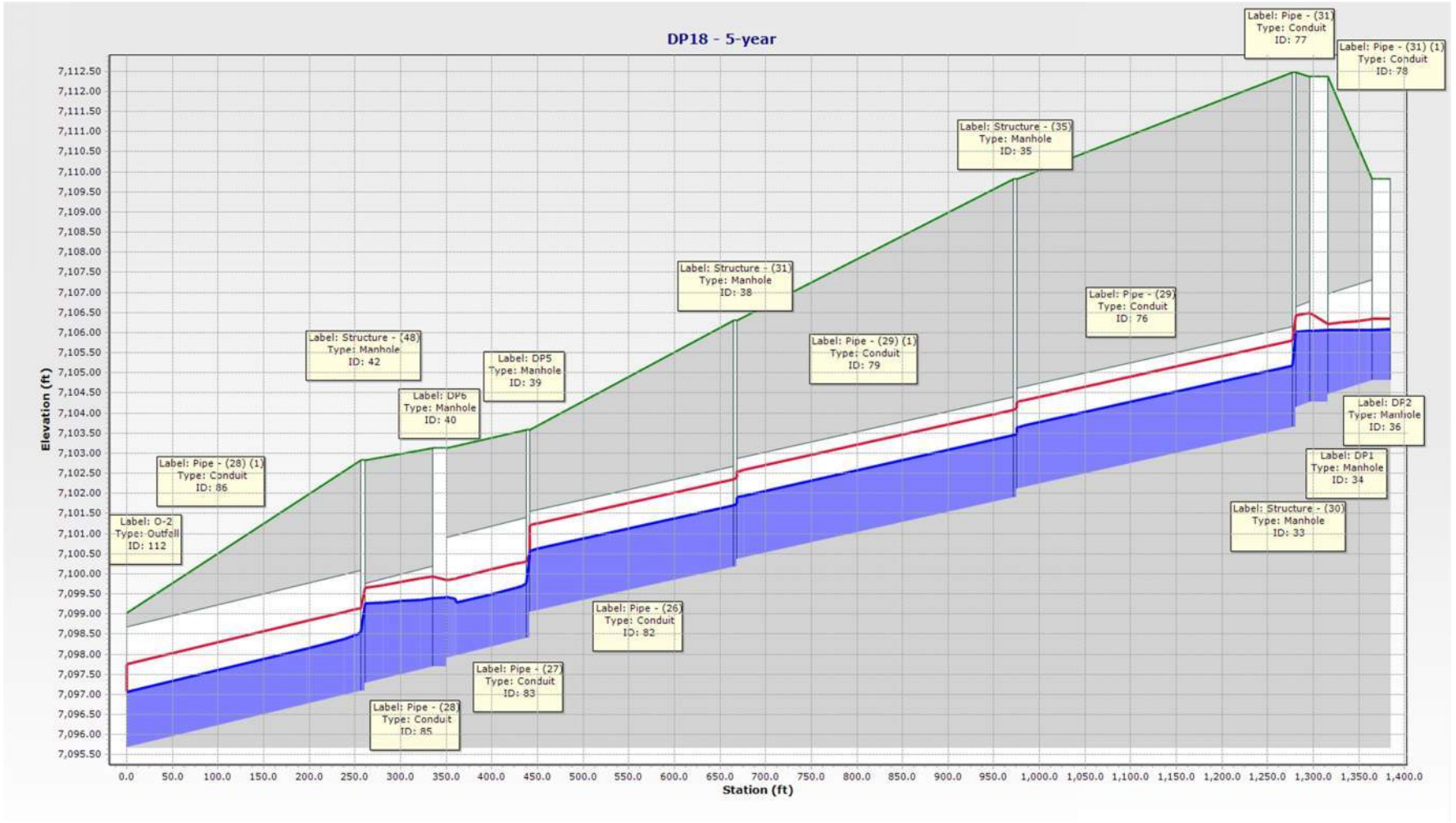

DP25.1
O-7
Pipe - (14)
O-8
Pipe - (11)
DP27
Pipe - (13)
Pipe - (10)
Structure - (10)
DP23
Pipe - (12)
DP22

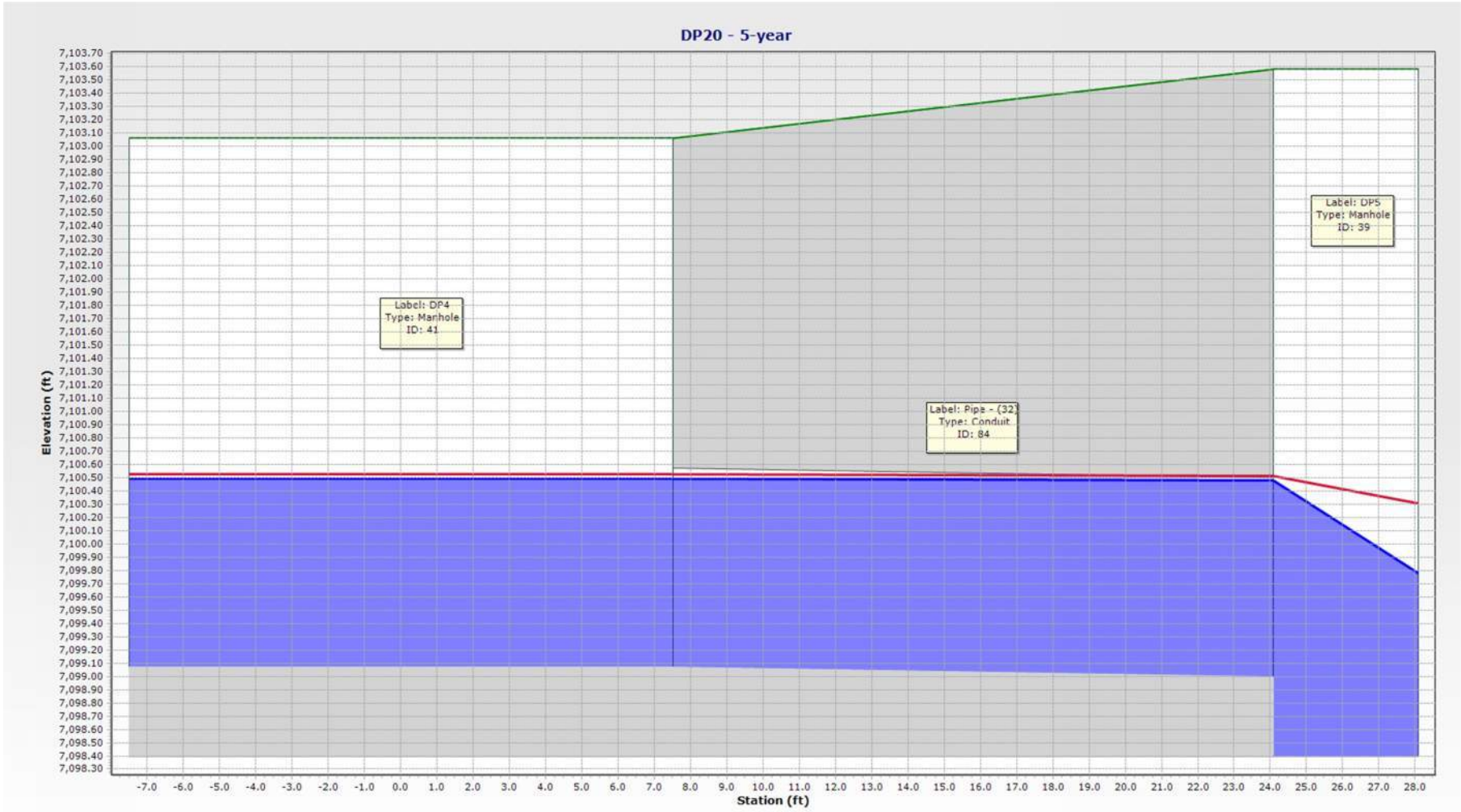
Scenario: 5-year
Current Time Step: 0.000 h
FlexTable: Conduit Table

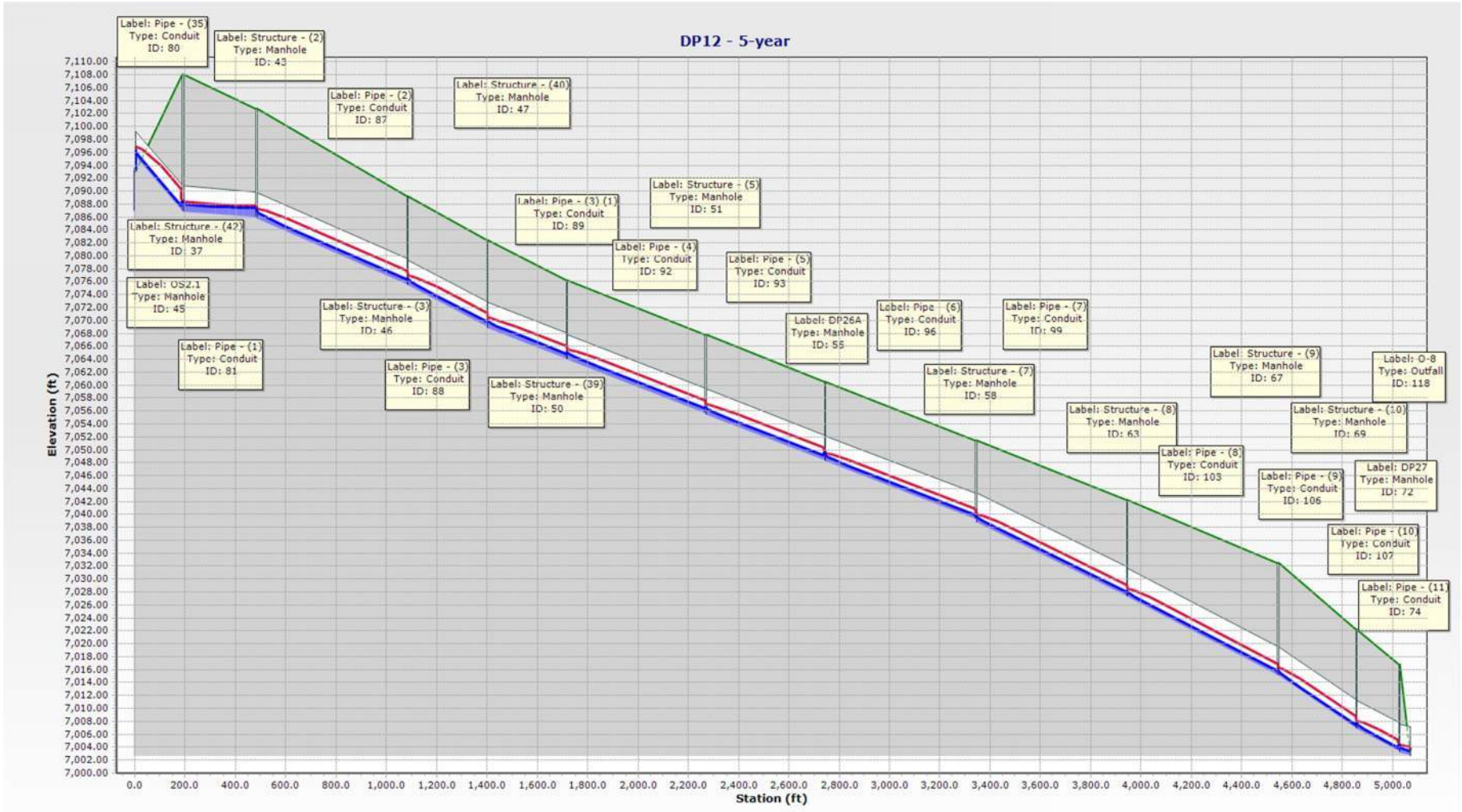
Label	Diameter (in)	Manning's n	Invert (Start) (ft)	Invert (Stop) (ft)	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Flow (cfs)	Velocity (ft/s)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
Pipe - (1)	48.0	0.013	7,086.75	7,085.88	291.3	0.003	16.40	4.95	78.67	7,087.99	7,087.44
Pipe - (2)	48.0	0.013	7,085.68	7,075.54	599.8	0.017	16.40	9.15	186.75	7,086.87	7,076.35
Pipe - (3)	48.0	0.013	7,075.34	7,069.01	316.6	0.020	16.40	9.71	203.11	7,076.53	7,069.78
Pipe - (3) (1)	48.0	0.013	7,068.81	7,064.05	317.1	0.015	16.40	8.77	175.91	7,070.00	7,064.88
Pipe - (4)	48.0	0.013	7,063.85	7,055.58	551.6	0.015	16.40	8.77	175.87	7,065.04	7,056.41
Pipe - (5)	48.0	0.013	7,055.38	7,048.25	475.2	0.015	16.40	8.77	175.91	7,056.57	7,049.65
Pipe - (6)	48.0	0.013	7,048.16	7,039.26	599.8	0.015	11.10	7.78	174.89	7,049.13	7,039.95
Pipe - (7)	54.0	0.013	7,038.76	7,027.36	600.1	0.019	11.10	8.37	271.02	7,039.70	7,027.99
Pipe - (8)	54.0	0.013	7,027.16	7,015.16	600.1	0.020	11.10	8.52	278.06	7,028.10	7,015.78
Pipe - (9)	54.0	0.013	7,013.15	7,006.91	312.2	0.020	11.10	8.52	277.99	7,014.09	7,007.88
Pipe - (10)	54.0	0.013	7,006.73	7,003.32	170.2	0.020	11.10	8.52	278.06	7,007.67	7,004.40
Pipe - (11)	54.0	0.013	7,003.13	7,002.69	43.2	0.010	9.60	6.40	196.63	7,004.00	7,003.39
Pipe - (12)	21.0	0.013	7,012.44	7,012.16	56.2	0.005	5.50	4.64	11.20	7,013.35	7,013.32
Pipe - (13)	21.0	0.013	7,011.66	7,010.01	328.8	0.005	9.10	5.19	11.20	7,012.86	7,011.14
Pipe - (14)	48.0	0.013	7,006.65	7,003.61	168.2	0.018	6.50	7.12	193.10	7,007.39	7,004.40
Pipe - (15)	18.0	0.013	7,030.14	7,027.33	56.2	0.050	2.50	8.65	23.47	7,030.74	7,027.66
Pipe - (16)	18.0	0.013	7,026.19	7,021.01	103.6	0.050	5.00	10.56	23.47	7,027.05	7,021.48
Pipe - (17)	18.0	0.013	7,044.83	7,044.52	62.3	0.005	1.20	3.09	7.43	7,045.24	7,045.05
Pipe - (18)	18.0	0.013	7,044.32	7,039.48	69.4	0.070	1.90	8.99	27.76	7,044.84	7,040.00
Pipe - (19)	18.0	0.013	7,039.27	7,034.92	108.6	0.040	1.90	7.39	21.02	7,039.79	7,035.23
Pipe - (20)	18.0	0.013	7,055.26	7,054.95	62.5	0.005	3.30	4.08	7.43	7,056.12	7,056.09
Pipe - (21)	18.0	0.013	7,054.75	7,050.89	77.5	0.050	5.60	10.89	23.47	7,055.67	7,052.02
Pipe - (22)	18.0	0.013	7,050.68	7,046.73	125.9	0.031	5.60	9.22	18.62	7,051.60	7,047.29
Pipe - (23)	18.0	0.013	7,070.99	7,070.68	62.3	0.005	5.10	4.53	7.43	7,071.91	7,071.74
Pipe - (24)	18.0	0.013	7,070.48	7,065.06	215.2	0.025	9.50	9.74	16.67	7,071.67	7,065.87
Pipe - (26)	30.0	0.013	7,100.17	7,099.04	226.9	0.005	20.20	6.38	29.00	7,101.71	7,100.57
Pipe - (27)	36.0	0.013	7,098.39	7,097.91	97.4	0.005	18.60	6.28	47.16	7,099.77	7,099.41
Pipe - (28)	30.0	0.013	7,097.69	7,097.27	83.9	0.005	21.20	6.45	29.01	7,099.39	7,099.25
Pipe - (28) (1)	36.0	0.013	7,097.08	7,095.68	258.5	0.005	21.20	6.68	49.03	7,098.56	7,097.06
Pipe - (29)	30.0	0.013	7,103.64	7,102.11	306.2	0.005	20.20	6.38	29.00	7,105.17	7,103.63
Pipe - (29) (1)	30.0	0.013	7,101.90	7,100.37	306.2	0.005	20.20	6.38	29.00	7,103.44	7,101.90
Pipe - (31)	30.0	0.013	7,104.14	7,104.27	27.3	-0.005	20.20	6.38	29.00	7,106.04	7,106.01
Pipe - (31) (1)	30.0	0.013	7,104.47	7,104.81	68.0	-0.005	10.30	5.41	29.00	7,106.06	7,106.07
Pipe - (32)	18.0	0.013	7,099.07	7,098.99	26.1	0.003	2.60	3.17	5.75	7,100.49	7,100.48
Pipe - (35)	48.0	0.013	7,086.94	7,095.25	193.2	-0.043	16.40	12.71	297.90	7,096.44	7,088.50
Pipe - (36)	18.0	0.013	7,050.75	7,054.25	84.3	-0.041	1.60	7.10	21.37	7,054.72	7,051.03

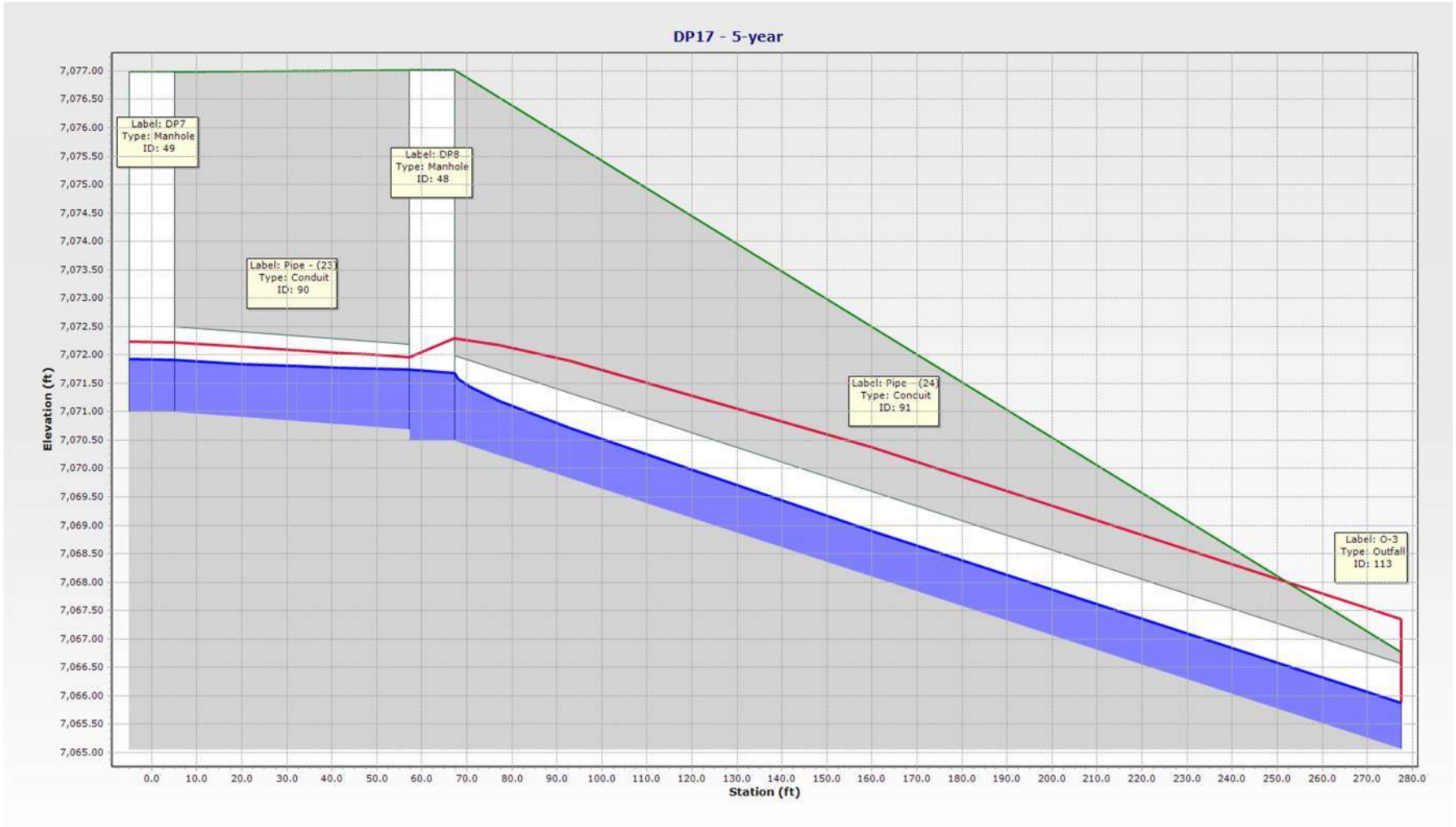
Scenario: 5-year
Current Time Step: 0.000 h
FlexTable: Manhole Table

Label	Flow (Total Out) (cfs)	Energy Grade Line (In) (ft)	Energy Grade Line (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Elevation (Ground) (ft)	Elevation (Invert) (ft)
Structure - (30)	20.20	7,106.42	7,105.81	7,106.01	7,105.17	7,112.47	7,103.64
DP1	20.20	7,106.22	7,106.50	7,106.07	7,106.04	7,112.36	7,104.27
Structure - (35)	20.20	7,104.12	7,104.07	7,103.47	7,103.44	7,109.81	7,101.90
DP2	10.30	7,106.35	7,106.34	7,106.07	7,106.06	7,109.81	7,104.81
Structure - (42)	16.40	7,088.70	7,088.37	7,088.50	7,087.99	7,108.01	7,086.75
Structure - (31)	20.20	7,102.39	7,102.34	7,101.74	7,101.71	7,106.31	7,100.17
DP5	18.60	7,100.51	7,100.31	7,100.48	7,099.77	7,103.58	7,098.39
DP6	21.20	7,099.84	7,099.94	7,099.41	7,099.39	7,103.12	7,097.69
DP4	2.60	7,100.53	7,100.52	7,100.49	7,100.49	7,103.06	7,099.07
Structure - (48)	21.20	7,099.65	7,099.14	7,099.25	7,098.56	7,102.82	7,097.08
Structure - (2)	16.40	7,087.64	7,087.30	7,087.44	7,086.87	7,102.69	7,085.68
OS2.1	16.40	7,093.57	7,093.55	7,093.14	7,093.12	7,093.12	7,086.94
Structure - (3)	16.40	7,077.85	7,076.96	7,076.55	7,076.53	7,089.15	7,075.34
Structure - (40)	16.40	7,071.48	7,070.43	7,070.02	7,070.00	7,082.43	7,068.81
DP8	9.50	7,071.97	7,072.29	7,071.74	7,071.67	7,077.01	7,070.48
DP7	5.10	7,072.24	7,072.23	7,071.93	7,071.91	7,076.99	7,070.99
Structure - (39)	16.40	7,066.26	7,065.47	7,065.06	7,065.04	7,076.23	7,063.85
Structure - (5)	16.40	7,057.78	7,057.00	7,056.59	7,056.57	7,067.85	7,055.38
DP10	3.30	7,056.29	7,056.28	7,056.13	7,056.12	7,061.26	7,055.26
DP11	5.60	7,056.17	7,056.05	7,056.09	7,055.67	7,061.25	7,054.75
DP26A	11.10	7,049.92	7,049.47	7,049.65	7,049.13	7,060.56	7,048.16
Structure - (25)	5.60	7,052.26	7,051.98	7,052.02	7,051.60	7,060.21	7,050.68
DP26.1	1.60	7,054.90	7,054.89	7,054.73	7,054.72	7,055.95	7,054.25
Structure - (7)	11.10	7,040.66	7,040.03	7,039.72	7,039.70	7,051.41	7,038.76
DP14	1.20	7,045.40	7,045.39	7,045.25	7,045.24	7,050.83	7,044.83
DP15	1.90	7,045.12	7,045.03	7,045.05	7,044.84	7,050.83	7,044.32
Structure - (21)	1.90	7,040.19	7,039.98	7,040.00	7,039.79	7,050.22	7,039.27
Structure - (8)	11.10	7,029.19	7,028.43	7,028.10	7,028.10	7,042.20	7,027.16
DP18	2.50	7,030.98	7,030.97	7,030.75	7,030.74	7,038.47	7,030.14
DP19	5.00	7,028.23	7,027.40	7,027.07	7,027.05	7,038.47	7,026.19
Structure - (9)	11.10	7,015.24	7,014.42	7,014.11	7,014.09	7,032.45	7,014.97
Structure - (10)	11.10	7,008.18	7,008.00	7,007.88	7,007.67	7,022.16	7,006.73
DP22	5.50	7,013.66	7,013.64	7,013.37	7,013.35	7,018.05	7,012.44
DP23	9.10	7,013.48	7,013.27	7,013.32	7,012.86	7,018.05	7,011.66
DP27	9.60	7,004.63	7,004.30	7,004.40	7,004.00	7,016.70	7,003.13
DP25.1	6.50	7,007.65	7,007.65	7,007.39	7,007.39	7,010.45	7,006.67


DP18 - 5-year
Elevation (ft)
Station (ft)
Label: Pipe - (31)
Type: Conduit
ID: 77
Label: Pipe - (31) (1)
Type: Conduit
ID: 78
Label: Structure - (35)
Type: Manhole
ID: 35
Label: Structure - (31)
Type: Manhole
ID: 38
Label: Pipe - (29)
Type: Conduit
ID: 76
Label: Pipe - (29) (1)
Type: Conduit
ID: 79
Label: Structure - (48)
Type: Manhole
ID: 42
Label: DP5
Type: Manhole
ID: 39
Label: DP6
Type: Manhole
ID: 40
Label: DP2
Type: Manhole
ID: 36
Label: DP1
Type: Manhole
ID: 34
Label: Structure - (30)
Type: Manhole
ID: 33
Label: Pipe - (28) (1)
Type: Conduit
ID: 86
Label: O-2
Type: Outfall
ID: 112
Label: Pipe - (26)
Type: Conduit
ID: 82
Label: Pipe - (27)
Type: Conduit
ID: 83
Label: Pipe - (28)
Type: Conduit
ID: 85


DP20 - 5-year
Elevation (ft)
Station (ft)
Label: DP4
Type: Manhole
ID: 41
Label: Pipe - (32)
Type: Conduit
ID: 84
Label: DP5
Type: Manhole
ID: 39


DP12 - 5-year
Label: Pipe - (35)
Type: Conduit
ID: 80
Label: Structure - (2)
Type: Manhole
ID: 43
Label: Pipe - (2)
Type: Conduit
ID: 87
Label: Structure - (40)
Type: Manhole
ID: 47
Label: Pipe - (3) (1)
Type: Conduit
ID: 89
Label: Structure - (5)
Type: Manhole
ID: 51
Label: Pipe - (4)
Type: Conduit
ID: 92
Label: Pipe - (5)
Type: Conduit
ID: 93
Label: Structure - (42)
Type: Manhole
ID: 37
Label: OSZ.1
Type: Manhole
ID: 45
Label: Structure - (3)
Type: Manhole
ID: 46
Label: DP26A
Type: Manhole
ID: 55
Label: Pipe - (6)
Type: Conduit
ID: 96
Label: Pipe - (7)
Type: Conduit
ID: 99
Label: Pipe - (1)
Type: Conduit
ID: 81
Label: Pipe - (3)
Type: Conduit
ID: 88
Label: Structure - (39)
Type: Manhole
ID: 50
Label: Structure - (7)
Type: Manhole
ID: 58
Label: Structure - (9)
Type: Manhole
ID: 67
Label: O-8
Type: Outfall
ID: 118
Label: Structure - (8)
Type: Manhole
ID: 63
Label: Structure - (10)
Type: Manhole
ID: 69
Label: Pipe - (8)
Type: Conduit
ID: 103
Label: Pipe - (9)
Type: Conduit
ID: 106
Label: DP27
Type: Manhole
ID: 72
Label: Pipe - (10)
Type: Conduit
ID: 107
Label: Pipe - (11)
Type: Conduit
ID: 74
Elevation (ft)
7,110.00
7,108.00
7,106.00
7,104.00
7,102.00
7,100.00
7,098.00
7,096.00
7,094.00
7,092.00
7,090.00
7,088.00
7,086.00
7,084.00
7,082.00
7,080.00
7,078.00
7,076.00
7,074.00
7,072.00
7,070.00
7,068.00
7,066.00
7,064.00
7,062.00
7,060.00
7,058.00
7,056.00
7,054.00
7,052.00
7,050.00
7,048.00
7,046.00
7,044.00
7,042.00
7,040.00
7,038.00
7,036.00
7,034.00
7,032.00
7,030.00
7,028.00
7,026.00
7,024.00
7,022.00
7,020.00
7,018.00
7,016.00
7,014.00
7,012.00
7,010.00
7,008.00
7,006.00
7,004.00
7,002.00
7,000.00
0.0
200.0
400.0
600.0
800.0
1,000.0
1,200.0
1,400.0
1,600.0
1,800.0
2,000.0
2,200.0
2,400.0
2,600.0
2,800.0
3,000.0
3,200.0
3,400.0
3,600.0
3,800.0
4,000.0
4,200.0
4,400.0
4,600.0
4,800.0
5,000.0
Station (ft)


DP17 - 5-year
Label: DP7
Type: Manhole
ID: 49
Label: DP8
Type: Manhole
ID: 48
Label: Pipe - (23)
Type: Conduit
ID: 90
Label: Pipe - (24)
Type: Conduit
ID: 91
Label: O-3
Type: Outfall
ID: 113
Elevation (ft)
Station (ft)
7,077.00
7,076.50
7,076.00
7,075.50
7,075.00
7,074.50
7,074.00
7,073.50
7,073.00
7,072.50
7,072.00
7,071.50
7,071.00
7,070.50
7,070.00
7,069.50
7,069.00
7,068.50
7,068.00
7,067.50
7,067.00
7,066.50
7,066.00
7,065.50
7,065.00
0.0
10.0
20.0
30.0
40.0
50.0
60.0
70.0
80.0
90.0
100.0
110.0
120.0
130.0
140.0
150.0
160.0
170.0
180.0
190.0
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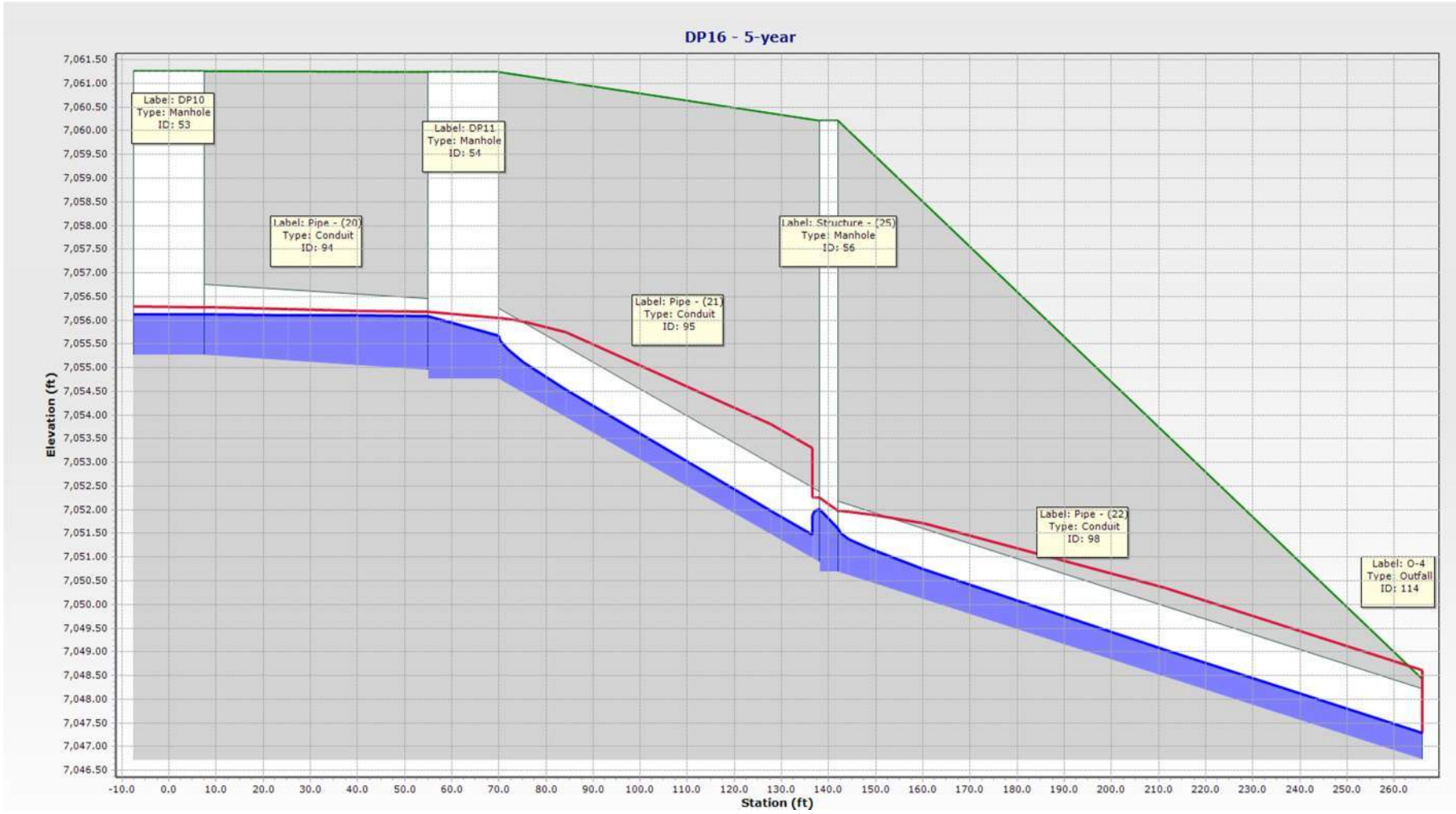
260.0
270.0
280.0


DP16 - 5-year
Label: DP10
Type: Manhole
ID: 53
Label: DP11
Type: Manhole
ID: 54
Label: Pipe - (20)
Type: Conduit
ID: 94
Label: Structure - (25)
Type: Manhole
ID: 56
Label: Pipe - (21)
Type: Conduit
ID: 95
Label: Pipe - (22)
Type: Conduit
ID: 98
Label: O-4
Type: Outfall
ID: 114
Elevation (ft)
Station (ft)


DP19 - 5-year
Label: DP26A
Type: Manhole
ID: 55
Label: DP26.1
Type: Manhole
ID: 57
Label: Pipe - (36)
Type: Conduit
ID: 97
Elevation (ft)
7,060.50
7,060.00
7,059.50
7,059.00
7,058.50
7,058.00
7,057.50
7,057.00
7,056.50
7,056.00
7,055.50
7,055.00
7,054.50
7,054.00
7,053.50
7,053.00
7,052.50
7,052.00
7,051.50
7,051.00
7,050.50
7,050.00
7,049.50
7,049.00
7,048.50
7,048.00
-2.0 0.0 2.0 4.0 6.0 8.0 10.0 12.0 14.0 16.0 18.0 20.0 22.0 24.0 26.0 28.0 30.0 32.0 34.0 36.0 38.0 40.0 42.0 44.0 46.0 48.0 50.0 52.0 54.0 56.0 58.0 60.0 62.0 64.0 66.0 68.0 70.0 72.0 74.0 76.0 78.0 80.0 82.0 84.0
Station (ft)


DP15 - 5-year
Label: DP14
Type: Manhole
ID: 59
Label: DP15
Type: Manhole
ID: 60
Label: Pipe - (17)
Type: Conduit
ID: 100
Label: Structure - (21)
Type: Manhole
ID: 51
Label: Pipe - (18)
Type: Conduit
ID: 101
Label: Pipe - (19)
Type: Conduit
ID: 102
Label: O-5
Type: Outfall
ID: 115
Elevation (ft)
Station (ft)
7,051.00
7,050.50
7,050.00
7,049.50
7,049.00
7,048.50
7,048.00
7,047.50
7,047.00
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7,046.00
7,045.50
7,045.00
7,044.50
7,044.00
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7,043.00
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7,042.00
7,041.50
7,041.00
7,040.50
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7,038.50
7,038.00
7,037.50
7,037.00
7,036.50
7,036.00
7,035.50
7,035.00
-10.0
0.0
10.0
20.0
30.0
40.0
50.0
60.0
70.0
80.0
90.0
100.0
110.0
120.0
130.0
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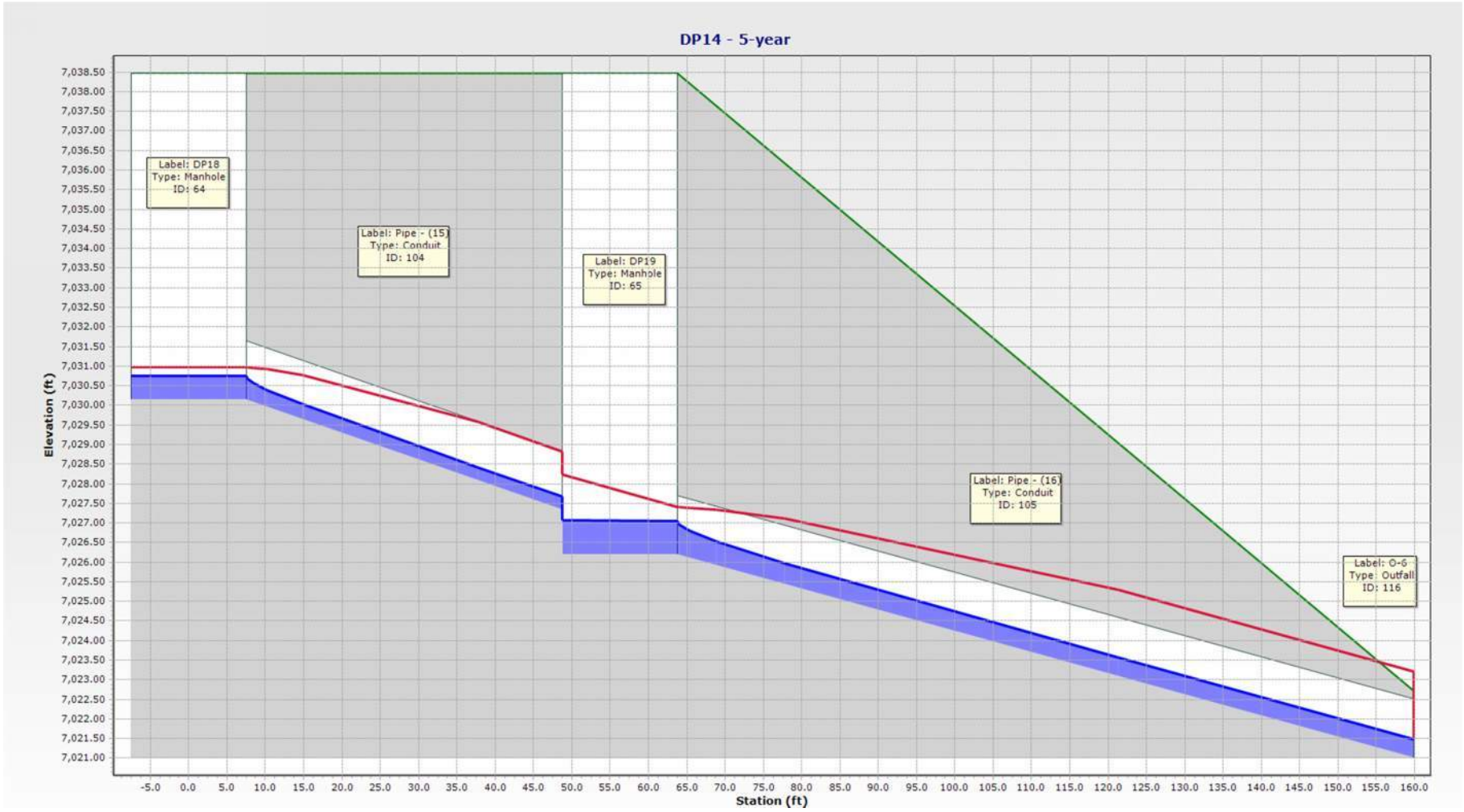
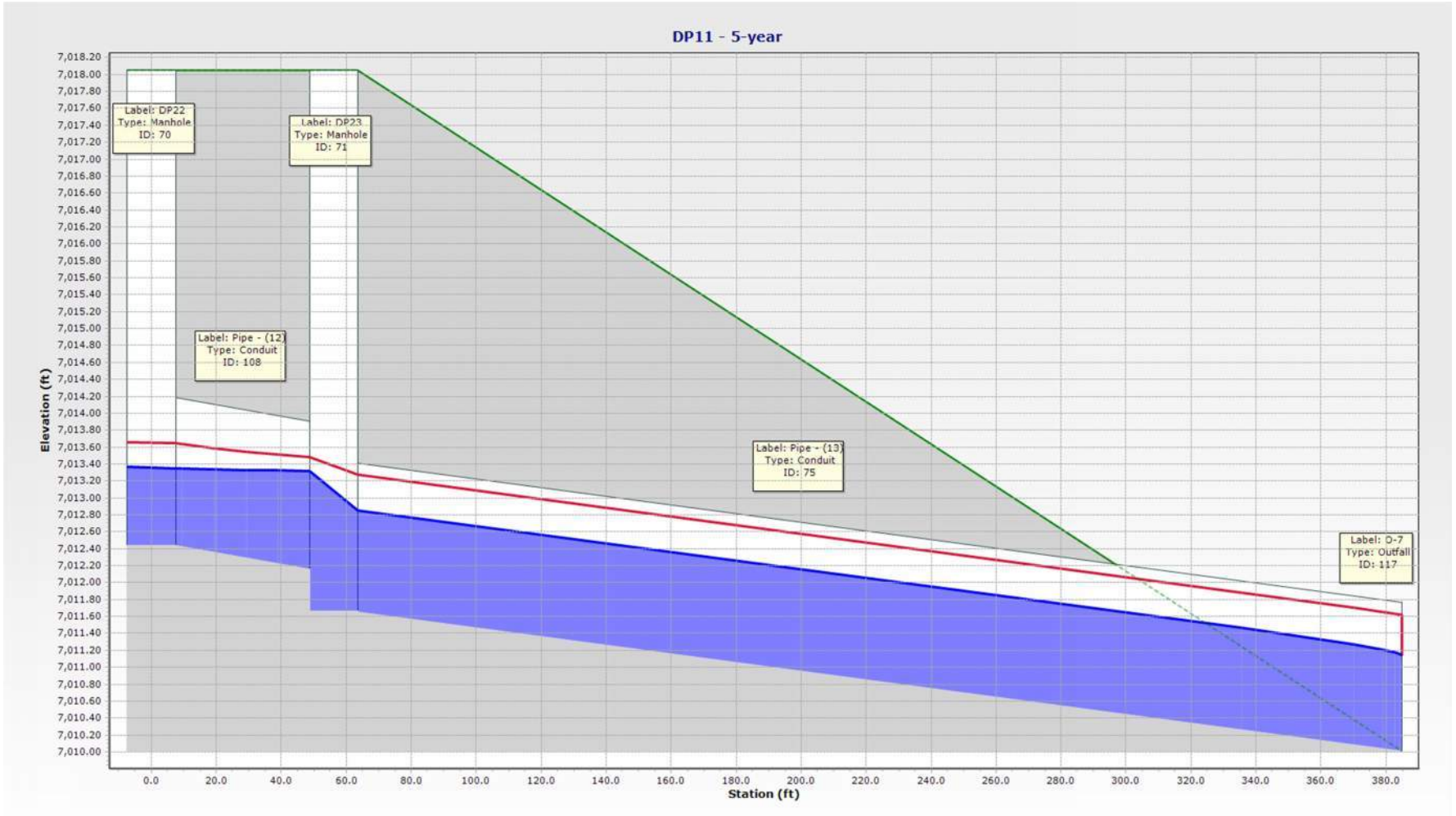
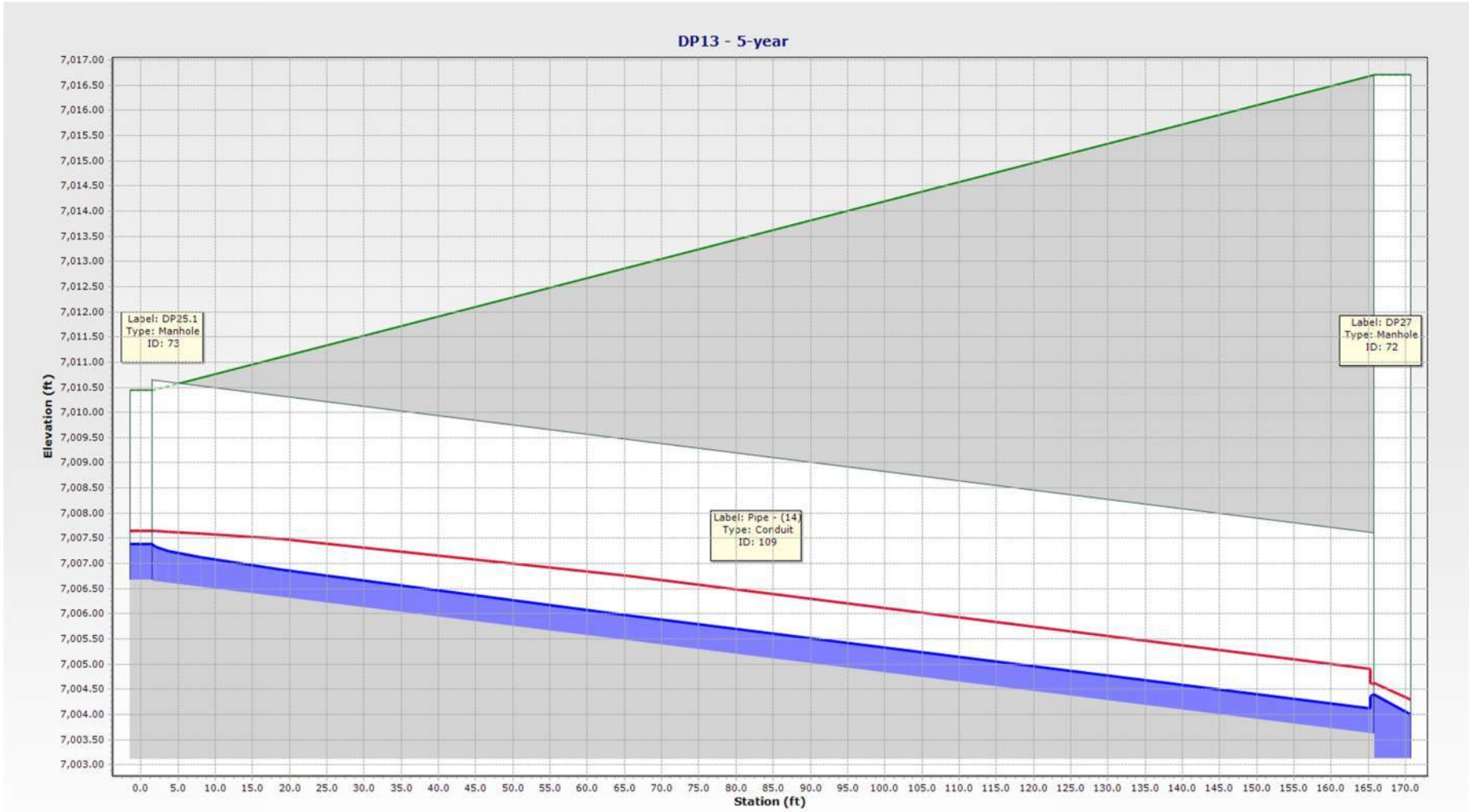
150.0
160.0
170.0
180.0
190.0
200.0
210.0
220.0
230.0
240.0


DP14 - 5-year
Elevation (ft)
Station (ft)
Label: DP18
Type: Manhole
ID: 64
Label: Pipe - (15)
Type: Conduit
ID: 104
Label: DP19
Type: Manhole
ID: 65
Label: Pipe - (16)
Type: Conduit
ID: 105
Label: O-6
Type: Outfall
ID: 116


DP11 - 5-year
Elevation (ft)
Station (ft)
Label: DP22
Type: Manhole
ID: 70
Label: DP23
Type: Manhole
ID: 71
Label: Pipe - (12)
Type: Conduit
ID: 108
Label: Pipe - (13)
Type: Conduit
ID: 75
Label: O-7
Type: Outfall
ID: 117

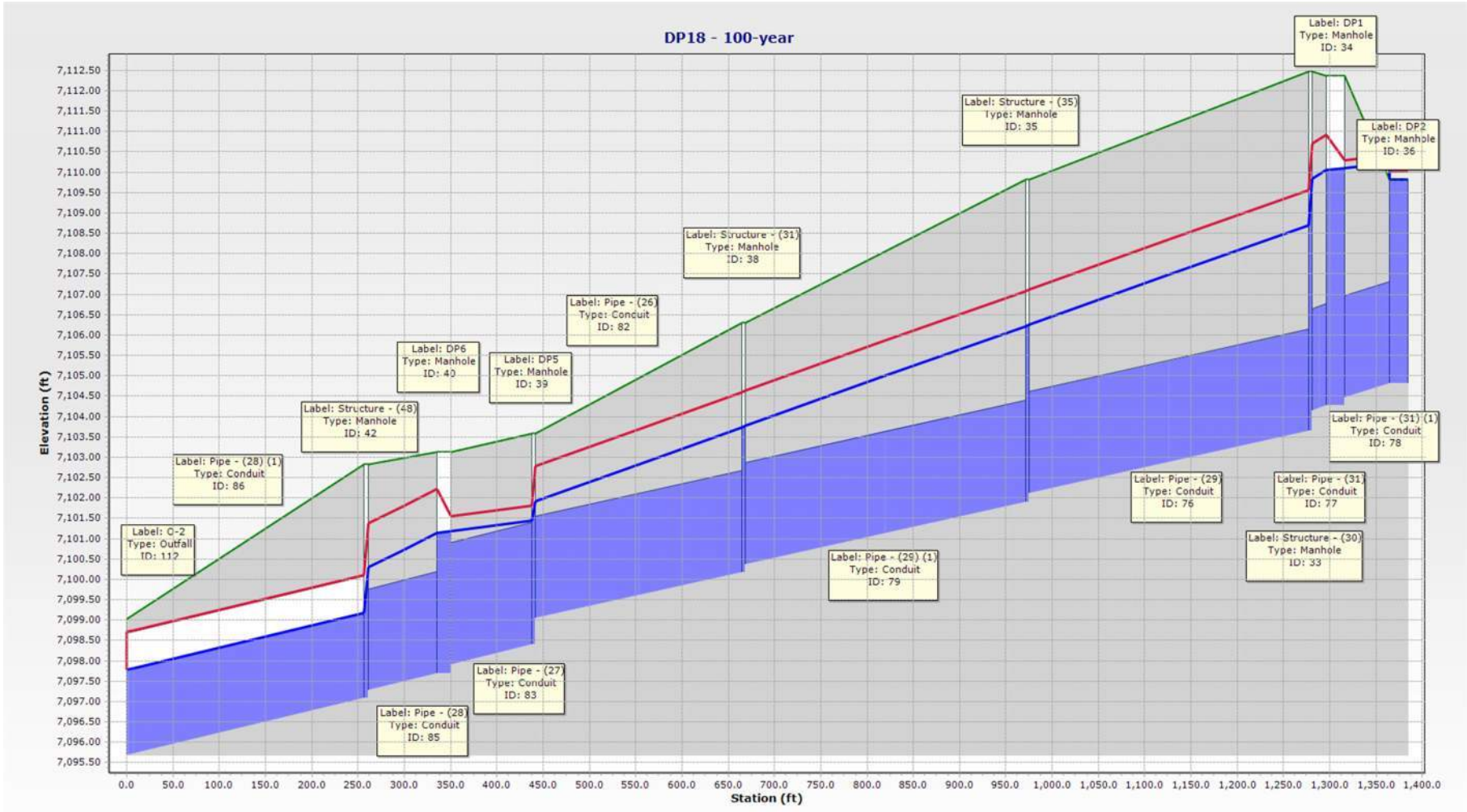

DP13 - 5-year
Elevation (ft)
Station (ft)
Label: DP25.1
Type: Manhole
ID: 73
Label: DP27
Type: Manhole
ID: 72
Label: Pipe - (14)
Type: Conduit
ID: 109

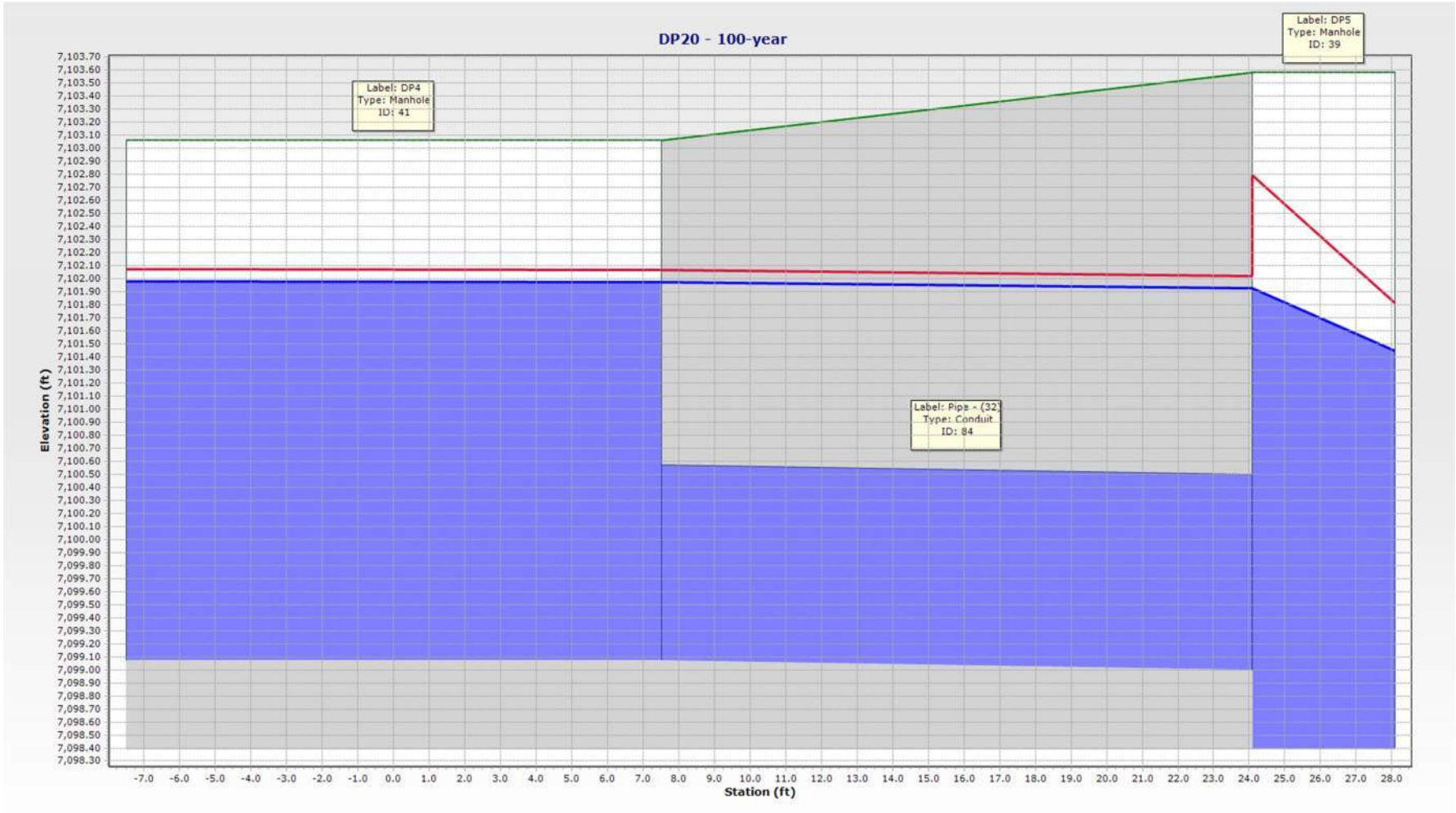
Scenario: 100-year
Current Time Step: 0.000 h
FlexTable: Conduit Table

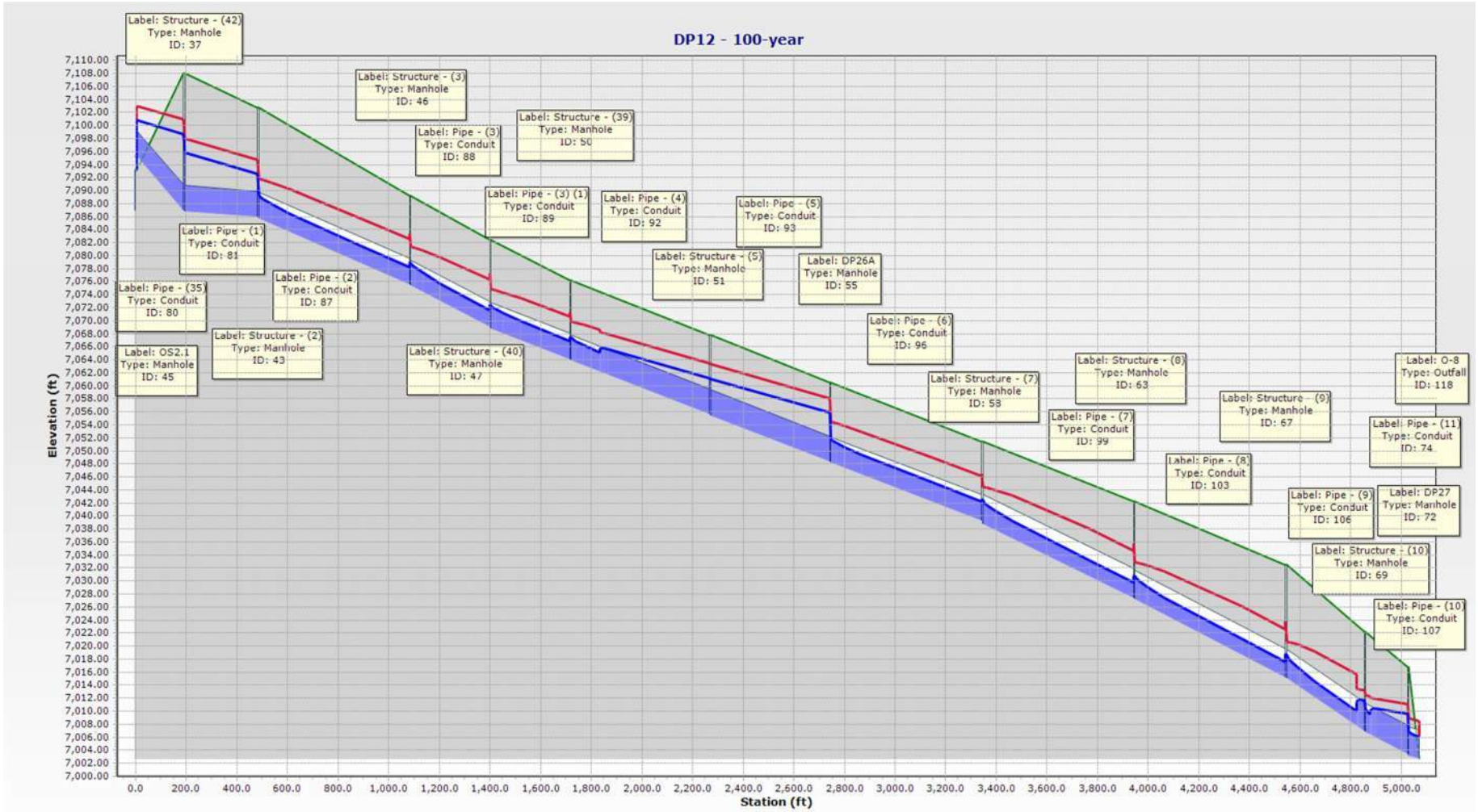
Label	Diameter (in)	Manning's n	Invert (Start) (ft)	Invert (Stop) (ft)	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Flow (cfs)	Velocity (ft/s)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
Pipe - (1)	48.0	0.013	7,086.75	7,085.88	291.3	0.003	150.60	11.98	78.67	7,095.76	7,092.56
Pipe - (2)	48.0	0.013	7,085.68	7,075.54	599.8	0.017	150.60	16.53	186.75	7,089.28	7,078.27
Pipe - (3)	48.0	0.013	7,075.34	7,069.01	316.6	0.020	150.60	17.70	203.11	7,078.95	7,071.61
Pipe - (3) (1)	48.0	0.013	7,068.81	7,064.05	317.1	0.015	150.60	15.73	175.91	7,072.41	7,066.92
Pipe - (4)	48.0	0.013	7,063.85	7,055.58	551.6	0.015	150.60	15.73	175.87	7,067.45	7,061.21
Pipe - (5)	48.0	0.013	7,055.38	7,048.25	475.2	0.015	150.60	11.98	175.91	7,061.10	7,055.87
Pipe - (6)	48.0	0.013	7,048.16	7,039.26	599.8	0.015	157.90	15.76	174.89	7,051.81	7,042.24
Pipe - (7)	54.0	0.013	7,038.76	7,027.36	600.1	0.019	157.90	17.69	271.02	7,042.44	7,029.83
Pipe - (8)	54.0	0.013	7,027.16	7,015.16	600.1	0.020	157.90	18.04	278.06	7,030.84	7,017.59
Pipe - (9)	54.0	0.013	7,013.15	7,006.91	312.2	0.020	157.90	18.04	277.99	7,016.83	7,011.69
Pipe - (10)	54.0	0.013	7,006.73	7,003.32	170.2	0.020	157.90	18.04	278.06	7,010.41	7,009.54
Pipe - (11)	54.0	0.013	7,003.13	7,002.69	43.2	0.010	161.20	13.80	196.63	7,006.84	7,006.03
Pipe - (12)	21.0	0.013	7,012.44	7,012.16	56.2	0.005	10.90	4.53	11.20	7,017.14	7,016.87
Pipe - (13)	21.0	0.013	7,011.66	7,010.01	328.8	0.005	18.00	7.48	11.20	7,015.91	7,011.56
Pipe - (14)	48.0	0.013	7,006.65	7,003.61	168.2	0.018	126.30	10.05	193.10	7,010.85	7,009.54
Pipe - (15)	18.0	0.013	7,030.14	7,027.33	56.2	0.050	5.50	10.85	23.47	7,031.05	7,027.83
Pipe - (16)	18.0	0.013	7,026.19	7,021.01	103.6	0.050	9.00	12.41	23.47	7,027.35	7,021.66
Pipe - (17)	18.0	0.013	7,044.83	7,044.52	62.3	0.005	6.30	3.57	7.43	7,046.42	7,046.20
Pipe - (18)	18.0	0.013	7,044.32	7,039.48	69.4	0.070	9.50	14.23	27.76	7,045.51	7,041.15
Pipe - (19)	18.0	0.013	7,039.27	7,034.92	108.6	0.040	9.50	11.60	21.02	7,040.46	7,035.63
Pipe - (20)	18.0	0.013	7,055.26	7,054.95	62.5	0.005	5.40	3.06	7.43	7,057.04	7,056.88
Pipe - (21)	18.0	0.013	7,054.75	7,050.89	77.5	0.050	11.30	13.16	23.47	7,056.04	7,052.81
Pipe - (22)	18.0	0.013	7,050.68	7,046.73	125.9	0.031	11.30	11.04	18.62	7,051.97	7,047.57
Pipe - (23)	18.0	0.013	7,070.99	7,070.68	62.3	0.005	9.60	5.43	7.43	7,075.56	7,075.04
Pipe - (24)	18.0	0.013	7,070.48	7,065.06	215.2	0.025	20.60	11.66	16.67	7,074.83	7,066.53
Pipe - (26)	30.0	0.013	7,100.17	7,099.04	226.9	0.005	36.60	7.46	29.00	7,103.73	7,101.93
Pipe - (27)	36.0	0.013	7,098.39	7,097.91	97.4	0.005	34.30	4.85	47.16	7,101.44	7,101.19
Pipe - (28)	30.0	0.013	7,097.69	7,097.27	83.9	0.005	40.90	8.33	29.01	7,101.13	7,100.30
Pipe - (28) (1)	36.0	0.013	7,097.08	7,095.68	258.5	0.005	40.90	7.76	49.03	7,099.17	7,097.77
Pipe - (29)	30.0	0.013	7,103.64	7,102.11	306.2	0.005	36.60	7.46	29.00	7,108.70	7,106.26
Pipe - (29) (1)	30.0	0.013	7,101.90	7,100.37	306.2	0.005	36.60	7.46	29.00	7,106.22	7,103.78
Pipe - (31)	30.0	0.013	7,104.14	7,104.27	27.3	-0.005	36.60	7.46	29.00	7,110.05	7,109.84
Pipe - (31) (1)	30.0	0.013	7,104.47	7,104.81	68.0	-0.005	17.80	3.63	29.00	7,110.23	7,110.10
Pipe - (32)	18.0	0.013	7,099.07	7,098.99	26.1	0.003	4.30	2.43	5.75	7,101.97	7,101.93
Pipe - (35)	48.0	0.013	7,086.94	7,095.25	193.2	-0.043	150.60	11.98	297.90	7,100.83	7,098.71
Pipe - (36)	18.0	0.013	7,050.75	7,054.25	84.3	-0.041	11.80	6.68	21.37	7,056.94	7,055.87

Scenario: 100-year
Current Time Step: 0.000 h
FlexTable: Manhole Table

Label	Flow (Total Out) (cfs)	Energy Grade Line (In) (ft)	Energy Grade Line (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Elevation (Ground) (ft)	Elevation (Invert) (ft)
Structure - (30)	36.60	7,110.70	7,109.56	7,109.84	7,108.70	7,112.47	7,103.64
DP1	36.60	7,110.30	7,110.92	7,110.10	7,110.05	7,112.36	7,104.27
Structure - (35)	36.60	7,107.12	7,107.08	7,106.26	7,106.22	7,109.81	7,101.90
DP2	17.80	7,110.03	7,110.02	7,109.82	7,109.81	7,109.81	7,104.81
Structure - (42)	150.60	7,100.94	7,098.00	7,098.71	7,095.76	7,108.01	7,086.75
Structure - (31)	36.60	7,104.64	7,104.60	7,103.78	7,103.73	7,106.31	7,100.17
DP5	34.30	7,102.79	7,101.81	7,101.93	7,101.44	7,103.58	7,098.39
DP6	40.90	7,101.55	7,102.21	7,101.19	7,101.13	7,103.12	7,097.69
DP4	4.30	7,102.07	7,102.06	7,101.98	7,101.97	7,103.06	7,099.07
Structure - (48)	40.90	7,101.38	7,100.11	7,100.30	7,099.17	7,102.82	7,097.08
Structure - (2)	150.60	7,094.79	7,091.77	7,092.56	7,089.28	7,102.69	7,085.68
OS2.1	150.60	7,095.47	7,095.35	7,093.23	7,093.12	7,093.12	7,086.94
Structure - (3)	150.60	7,083.32	7,081.43	7,079.07	7,078.95	7,089.15	7,075.34
Structure - (40)	150.60	7,077.25	7,074.89	7,072.53	7,072.41	7,082.43	7,068.81
DP8	20.60	7,075.50	7,076.94	7,075.04	7,074.83	7,077.01	7,070.48
DP7	9.60	7,076.04	7,076.02	7,075.58	7,075.56	7,076.99	7,070.99
Structure - (39)	150.60	7,071.36	7,069.94	7,067.58	7,067.45	7,076.23	7,063.85
Structure - (5)	150.60	7,063.44	7,063.33	7,061.21	7,061.10	7,067.85	7,055.38
DP10	5.40	7,057.20	7,057.19	7,057.05	7,057.04	7,061.26	7,055.26
DP11	11.30	7,057.02	7,056.80	7,056.88	7,056.04	7,061.25	7,054.75
DP26A	157.90	7,058.11	7,054.48	7,055.87	7,051.81	7,060.56	7,048.16
Structure - (25)	11.30	7,053.45	7,052.73	7,052.81	7,051.97	7,060.21	7,050.68
DP26.1	11.80	7,056.68	7,056.65	7,055.99	7,055.96	7,055.95	7,054.25
Structure - (7)	157.90	7,046.40	7,044.44	7,042.54	7,042.44	7,051.41	7,038.76
DP14	6.30	7,046.63	7,046.62	7,046.43	7,046.42	7,050.83	7,044.83
DP15	9.50	7,046.39	7,046.13	7,046.20	7,045.51	7,050.83	7,044.32
Structure - (21)	9.50	7,041.60	7,041.08	7,041.15	7,040.46	7,050.22	7,039.27
Structure - (8)	157.90	7,035.71	7,032.84	7,030.84	7,030.84	7,042.20	7,027.16
DP18	5.50	7,031.44	7,031.43	7,031.07	7,031.05	7,038.47	7,030.14
DP19	9.00	7,029.14	7,027.93	7,027.38	7,027.35	7,038.47	7,026.19
Structure - (9)	157.90	7,021.99	7,018.83	7,016.93	7,016.83	7,032.45	7,014.97
Structure - (10)	157.90	7,013.22	7,012.41	7,011.69	7,010.41	7,022.16	7,006.73
DP22	10.90	7,017.47	7,017.46	7,017.15	7,017.14	7,018.05	7,012.44
DP23	18.00	7,017.19	7,016.78	7,016.87	7,015.91	7,018.05	7,011.66
DP27	161.20	7,011.08	7,008.89	7,009.54	7,006.84	7,016.70	7,003.13
DP25.1	126.30	7,012.02	7,012.02	7,010.45	7,010.45	7,010.45	7,006.67


DP18 - 100-year
Elevation (ft)
Station (ft)
Label: DP1
Type: Manhole
ID: 34
Label: DP2
Type: Manhole
ID: 36
Label: Structure - (35)
Type: Manhole
ID: 35
Label: Structure - (31)
Type: Manhole
ID: 38
Label: Pipe - (26)
Type: Conduit
ID: 82
Label: DP6
Type: Manhole
ID: 40
Label: DP5
Type: Manhole
ID: 39
Label: Structure - (48)
Type: Manhole
ID: 42
Label: Pipe - (28) (1)
Type: Conduit
ID: 86
Label: O-2
Type: Outfall
ID: 112
Label: Pipe - (31) (1)
Type: Conduit
ID: 78
Label: Pipe - (29)
Type: Conduit
ID: 76
Label: Pipe - (31)
Type: Conduit
ID: 77
Label: Structure - (30)
Type: Manhole
ID: 33
Label: Pipe - (29) (1)
Type: Conduit
ID: 79
Label: Pipe - (27)
Type: Conduit
ID: 83
Label: Pipe - (28)
Type: Conduit
ID: 85


DP20 - 100-year
Label: DP4
Type: Manhole
ID: 41
Label: DP5
Type: Manhole
ID: 39
Label: Pipe - (32)
Type: Conduit
ID: 84
Elevation (ft)
Station (ft)


DP12 - 100-year
Label: Structure - (42)
Type: Manhole
ID: 37
Label: Structure - (3)
Type: Manhole
ID: 46
Label: Pipe - (3)
Type: Conduit
ID: 88
Label: Structure - (39)
Type: Manhole
ID: 50
Label: Pipe - (3) (1)
Type: Conduit
ID: 89
Label: Pipe - (4)
Type: Conduit
ID: 92
Label: Pipe - (5)
Type: Conduit
ID: 93
Label: Pipe - (1)
Type: Conduit
ID: 81
Label: Structure - (5)
Type: Manhole
ID: 51
Label: DP26A
Type: Manhole
ID: 55
Label: Pipe - (35)
Type: Conduit
ID: 80
Label: Pipe - (2)
Type: Conduit
ID: 87
Label: Pipe - (6)
Type: Conduit
ID: 96
Label: Structure - (2)
Type: Manhole
ID: 43
Label: OS2.1
Type: Manhole
ID: 45
Label: Structure - (40)
Type: Manhole
ID: 47
Label: Structure - (7)
Type: Manhole
ID: 58
Label: Structure - (8)
Type: Manhole
ID: 63
Label: O-8
Type: Outfall
ID: 118
Label: Pipe - (7)
Type: Conduit
ID: 99
Label: Structure - (9)
Type: Manhole
ID: 67
Label: Pipe - (11)
Type: Conduit
ID: 74
Label: Pipe - (8)
Type: Conduit
ID: 103
Label: Pipe - (9)
Type: Conduit
ID: 106
Label: DP27
Type: Manhole
ID: 72
Label: Structure - (10)
Type: Manhole
ID: 69
Label: Pipe - (10)
Type: Conduit
ID: 107
Elevation (ft)
7,110.00
7,108.00
7,106.00
7,104.00
7,102.00
7,100.00
7,098.00
7,096.00
7,094.00
7,092.00
7,090.00
7,088.00
7,086.00
7,084.00
7,082.00
7,080.00
7,078.00
7,076.00
7,074.00
7,072.00
7,070.00
7,068.00
7,066.00
7,064.00
7,062.00
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7,056.00
7,054.00
7,052.00
7,050.00
7,048.00
7,046.00
7,044.00
7,042.00
7,040.00
7,038.00
7,036.00
7,034.00
7,032.00
7,030.00
7,028.00
7,026.00
7,024.00
7,022.00
7,020.00
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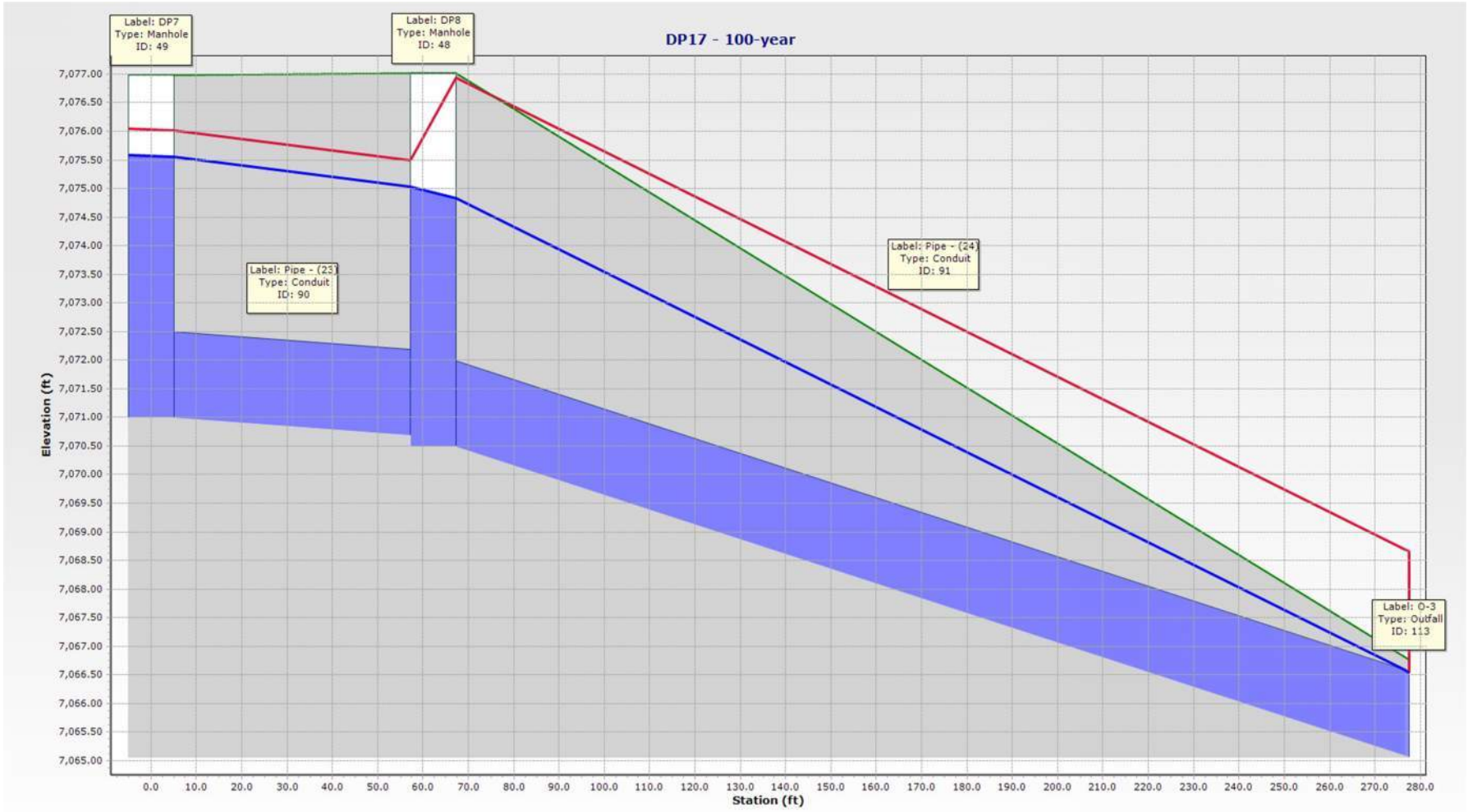
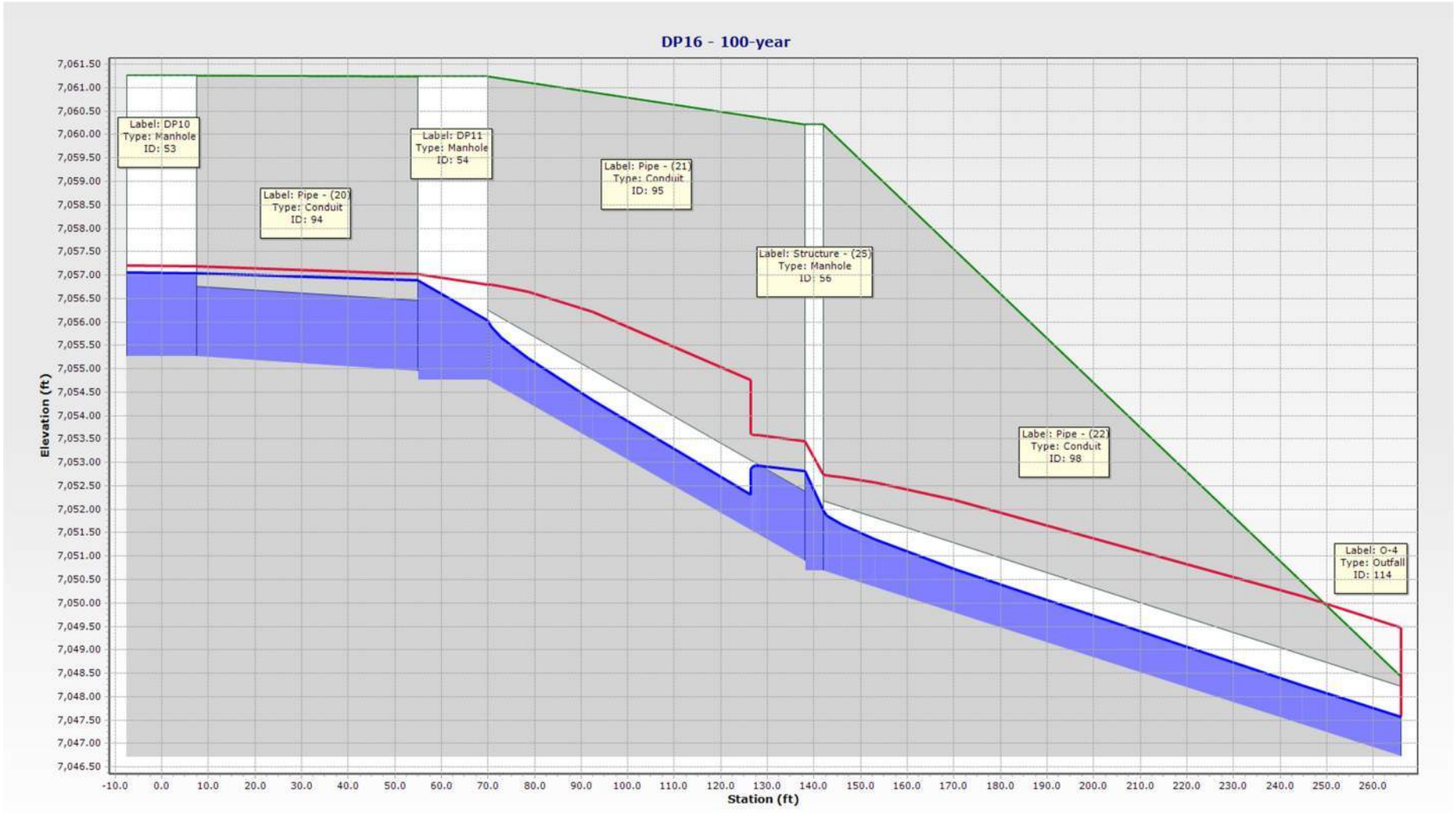
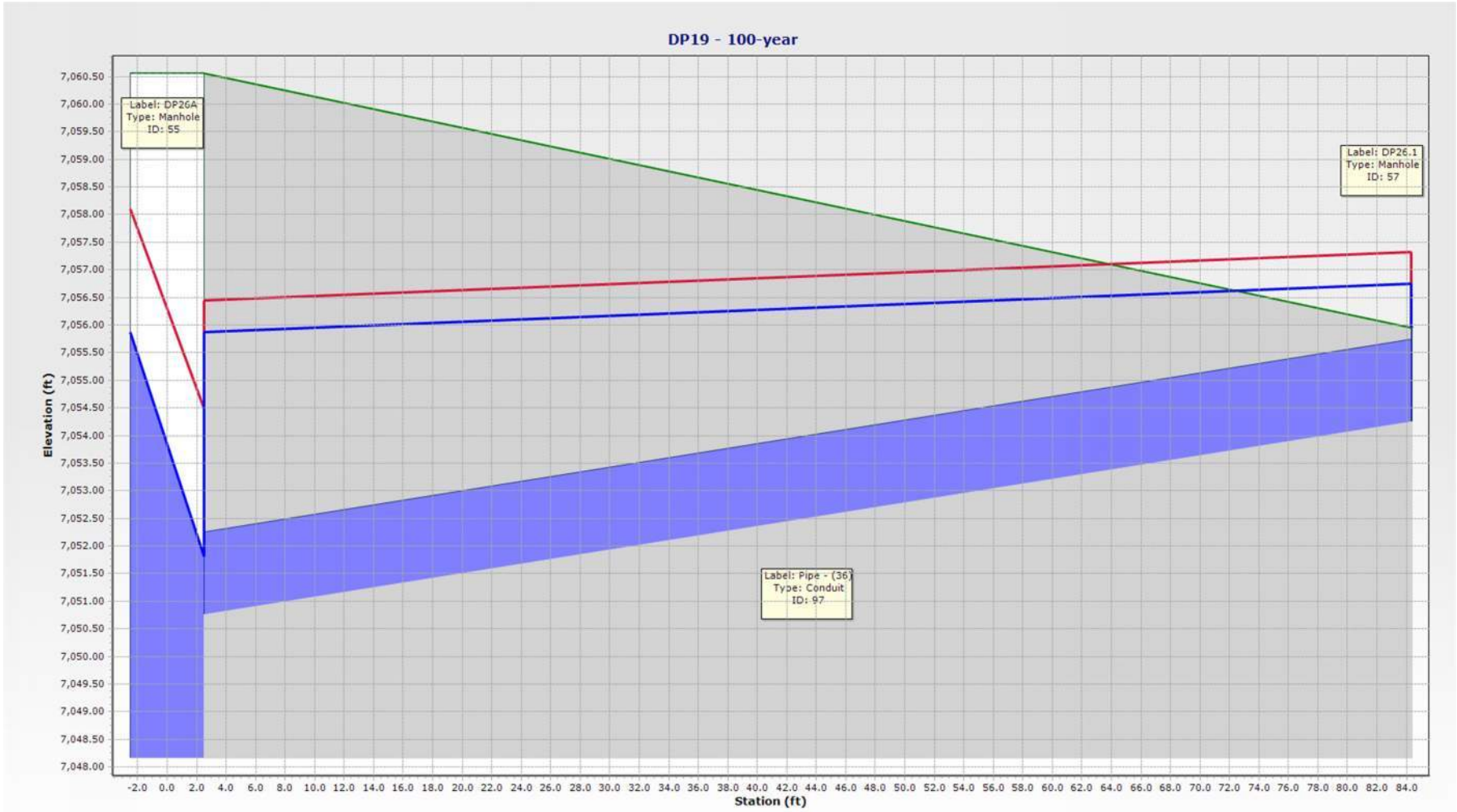
7,016.00
7,014.00
7,012.00
7,010.00
7,008.00
7,006.00
7,004.00
7,002.00
7,000.00
0.0
200.0
400.0
600.0
800.0
1,000.0
1,200.0
1,400.0
1,600.0
1,800.0
2,000.0
2,200.0
2,400.0
2,600.0
2,800.0
3,000.0
3,200.0
3,400.0
3,600.0
3,800.0
4,000.0
4,200.0
4,400.0
4,600.0
4,800.0
5,000.0
Station (ft)


DP17 - 100-year
Label: DP7
Type: Manhole
ID: 49
Label: DP8
Type: Manhole
ID: 48
Label: Pipe - (23)
Type: Conduit
ID: 90
Label: Pipe - (24)
Type: Conduit
ID: 91
Label: O-3
Type: Outfall
ID: 113
Elevation (ft)
7,077.00
7,076.50
7,076.00
7,075.50
7,075.00
7,074.50
7,074.00
7,073.50
7,073.00
7,072.50
7,072.00
7,071.50
7,071.00
7,070.50
7,070.00
7,069.50
7,069.00
7,068.50
7,068.00
7,067.50
7,067.00
7,066.50
7,066.00
7,065.50
7,065.00
0.0
10.0
20.0
30.0
40.0
50.0
60.0
70.0
80.0
90.0
100.0
110.0
120.0
130.0
140.0
150.0
160.0
170.0
180.0
190.0
200.0
210.0
220.0
230.0
240.0
250.0
260.0
270.0
280.0
Station (ft)


DP16 - 100-year
Elevation (ft)
Station (ft)
Label: DP10
Type: Manhole
ID: 53
Label: DP11
Type: Manhole
ID: 54
Label: Pipe - (20)
Type: Conduit
ID: 94
Label: Pipe - (21)
Type: Conduit
ID: 95
Label: Structure - (25)
Type: Manhole
ID: 56
Label: Pipe - (22)
Type: Conduit
ID: 98
Label: O-4
Type: Outfall
ID: 114


DP19 - 100-year
Label: DP26A
Type: Manhole
ID: 55
Label: DP26.1
Type: Manhole
ID: 57
Label: Pipe - (36)
Type: Conduit
ID: 97
Elevation (ft)
Station (ft)
7,060.50
7,060.00
7,059.50
7,059.00
7,058.50
7,058.00
7,057.50
7,057.00
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7,056.00
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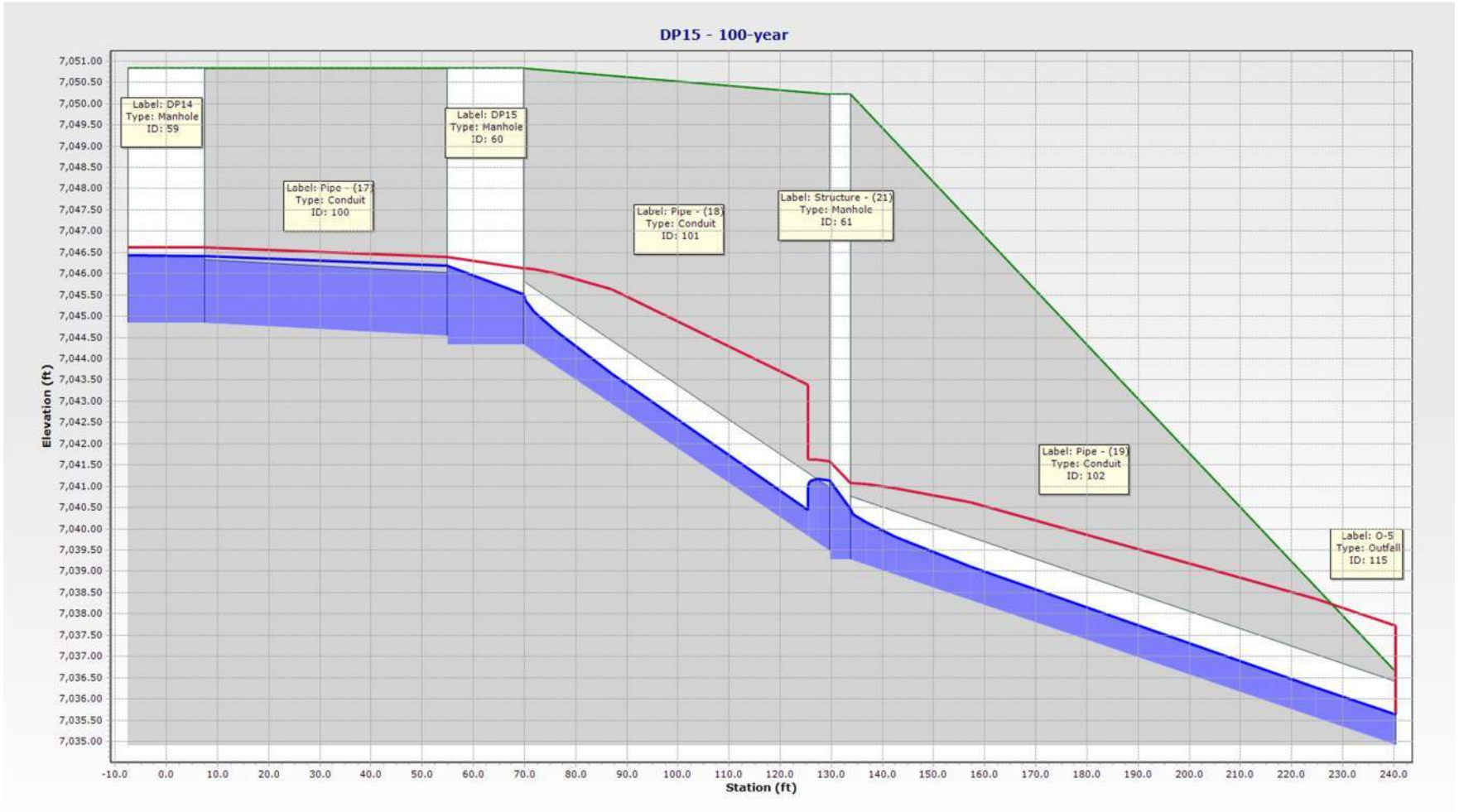
7,051.50
7,051.00
7,050.50
7,050.00
7,049.50
7,049.00
7,048.50
7,048.00
-2.0 0.0 2.0 4.0 6.0 8.0 10.0 12.0 14.0 16.0 18.0 20.0 22.0 24.0 26.0 28.0 30.0 32.0 34.0 36.0 38.0 40.0 42.0 44.0 46.0 48.0 50.0 52.0 54.0 56.0 58.0 60.0 62.0 64.0 66.0 68.0 70.0 72.0 74.0 76.0 78.0 80.0 82.0 84.0


DP15 - 100-year
Label: DP14
Type: Manhole
ID: 59
Label: DP15
Type: Manhole
ID: 60
Label: Pipe - (17
Type: Conduit
ID: 100
Label: Pipe - (18
Type: Conduit
ID: 101
Label: Structure - (21)
Type: Manhole
ID: 61
Label: Pipe - (19)
Type: Conduit
ID: 102
Label: O-5
Type: Outfall
ID: 115
Elevation (ft)
Station (ft)
7,051.00
7,050.50
7,050.00
7,049.50
7,049.00
7,048.50
7,048.00
7,047.50
7,047.00
7,046.50
7,046.00
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7,039.00
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7,038.00
7,037.50
7,037.00
7,036.50
7,036.00
7,035.50
7,035.00
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0.0
10.0
20.0
30.0
40.0
50.0
60.0
70.0
80.0
90.0
100.0
110.0
120.0
130.0
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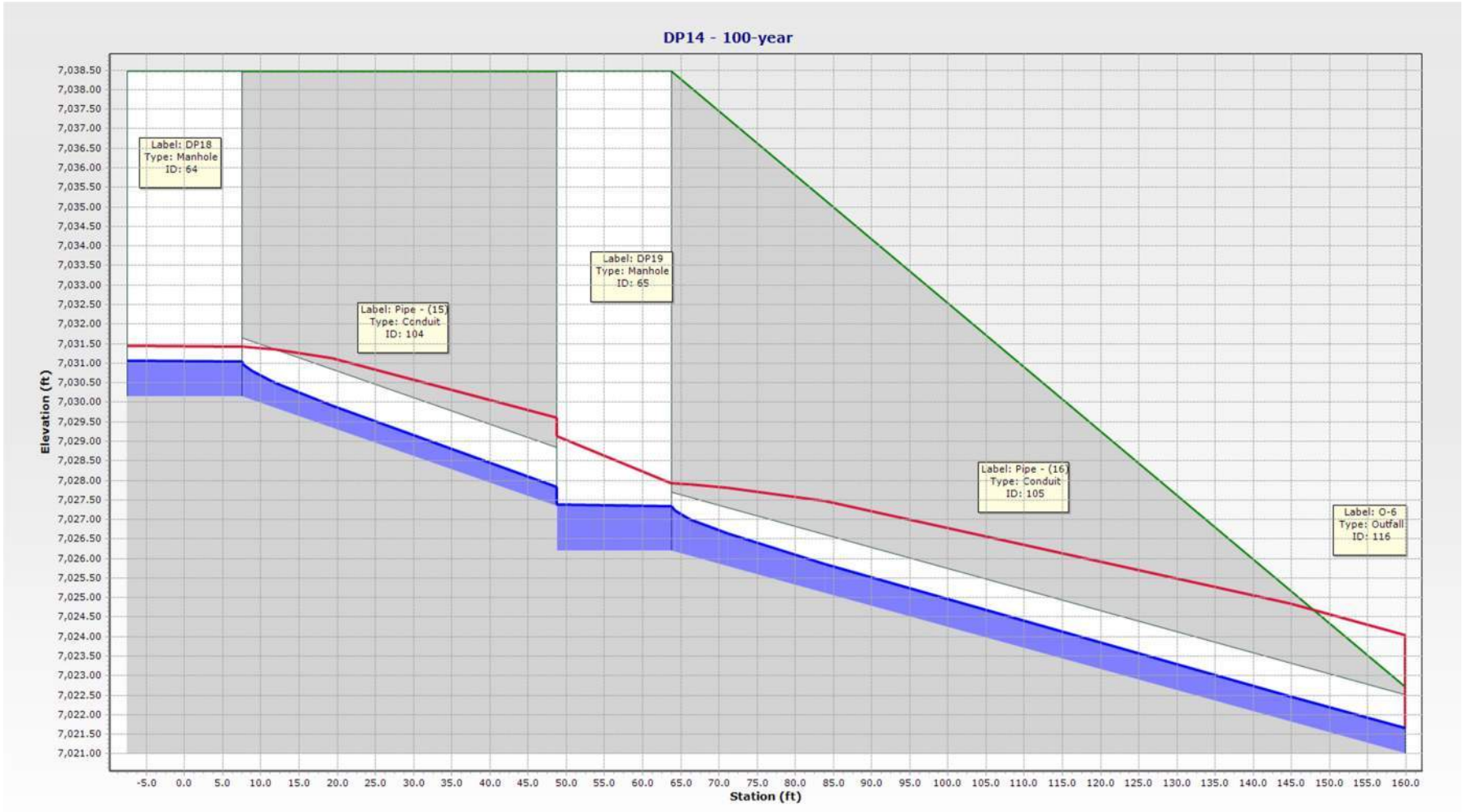
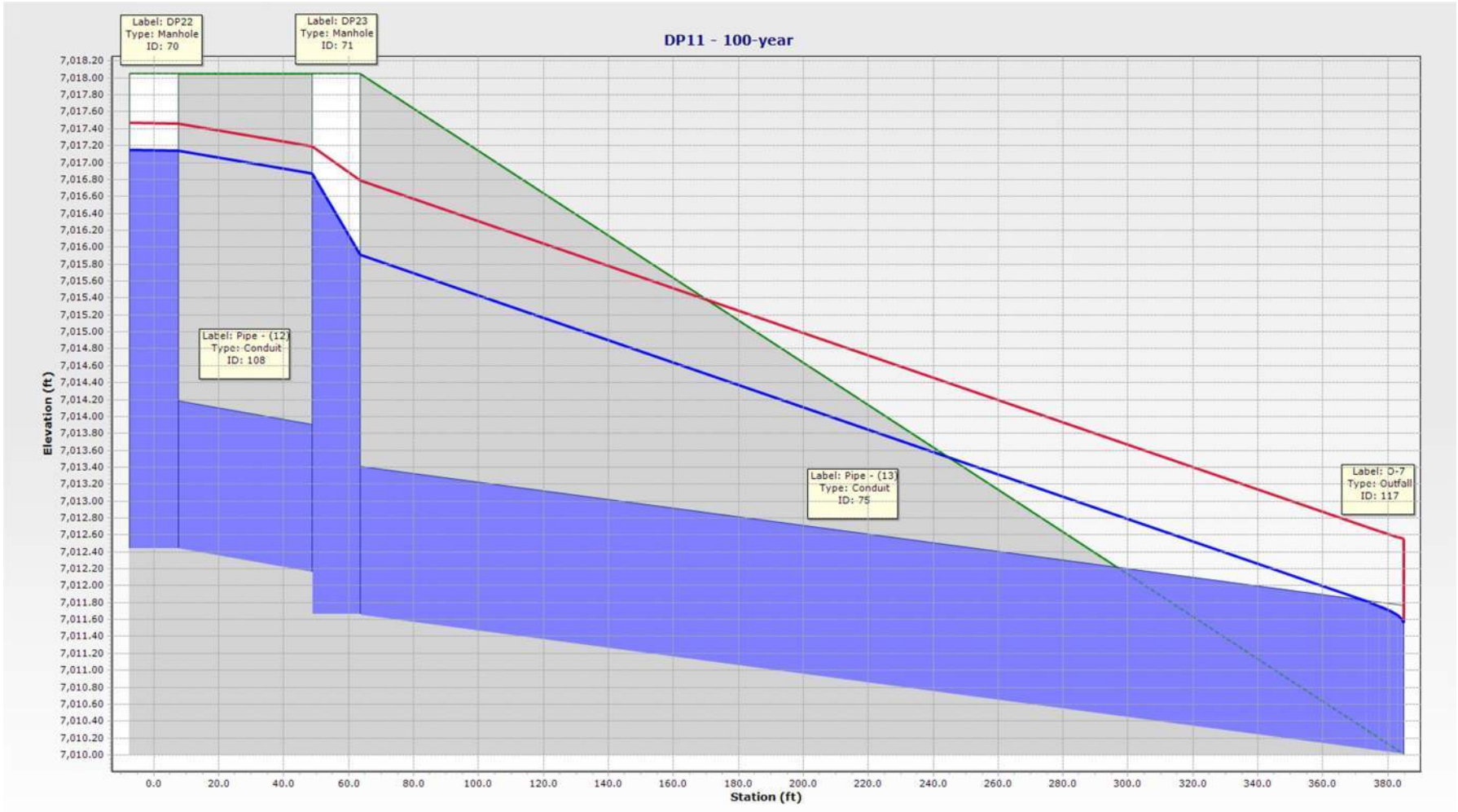
190.0
200.0
210.0
220.0
230.0
240.0


DP14 - 100-year
Label: DP18
Type: Manhole
ID: 64
Label: Pipe - (15)
Type: Conduit
ID: 104
Label: DP19
Type: Manhole
ID: 65
Label: Pipe - (16)
Type: Conduit
ID: 105
Label: O-6
Type: Outfall
ID: 116
Elevation (ft)
Station (ft)


DP11 - 100-year
Label: DP22
Type: Manhole
ID: 70
Label: DP23
Type: Manhole
ID: 71
Label: Pipe - (12)
Type: Conduit
ID: 108
Label: Pipe - (13)
Type: Conduit
ID: 75
Label: O-7
Type: Outfall
ID: 117
Elevation (ft)
Station (ft)
7,018.20
7,018.00
7,017.80
7,017.60
7,017.40
7,017.20
7,017.00
7,016.80
7,016.60
7,016.40
7,016.20
7,016.00
7,015.80
7,015.60
7,015.40
7,015.20
7,015.00
7,014.80
7,014.60
7,014.40
7,014.20
7,014.00
7,013.80
7,013.60
7,013.40
7,013.20
7,013.00
7,012.80
7,012.60
7,012.40
7,012.20
7,012.00
7,011.80
7,011.60
7,011.40
7,011.20
7,011.00
7,010.80
7,010.60
7,010.40
7,010.20
7,010.00
0.0
20.0
40.0
60.0
80.0
100.0
120.0
140.0
160.0
180.0
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220.0
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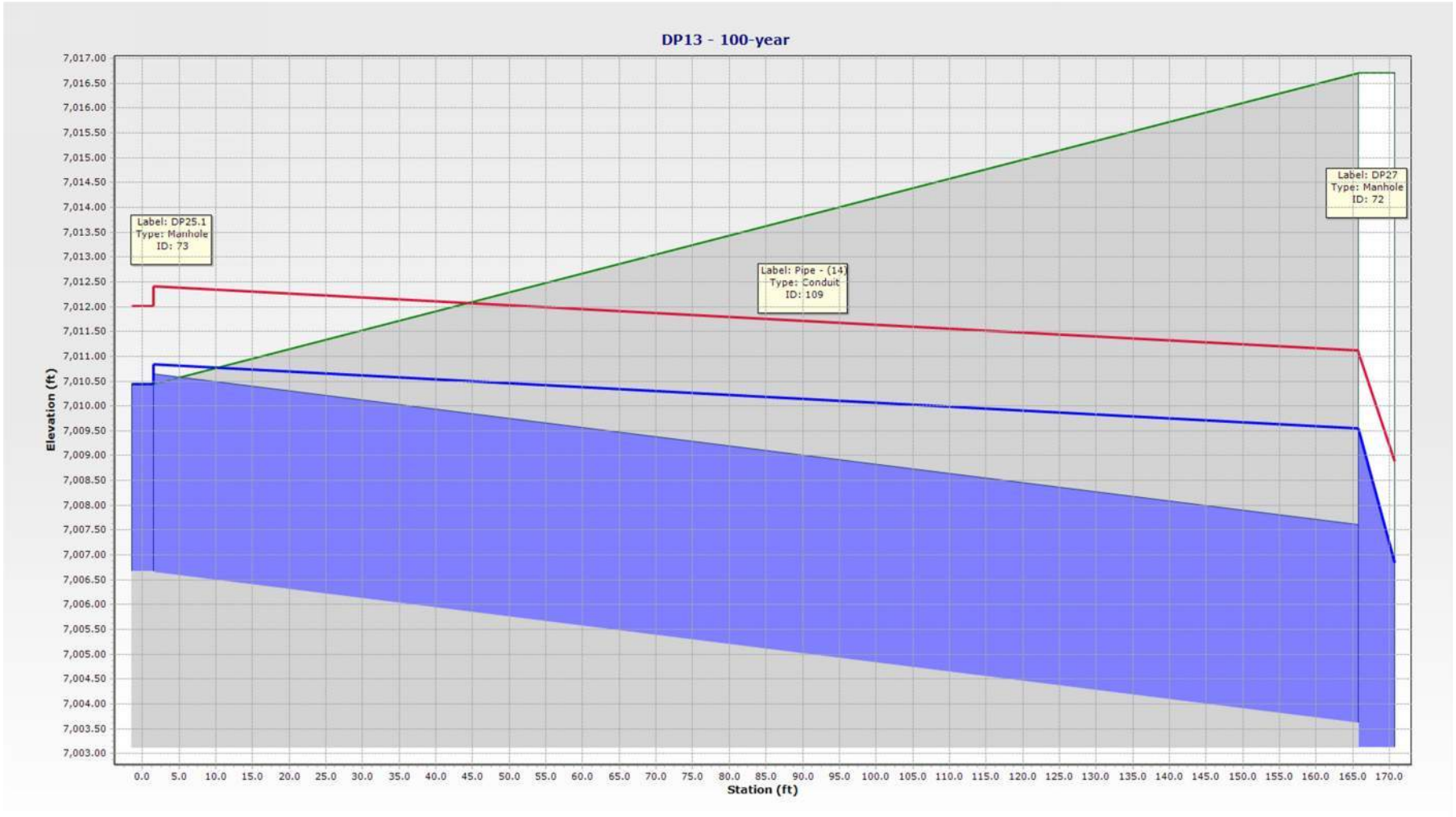
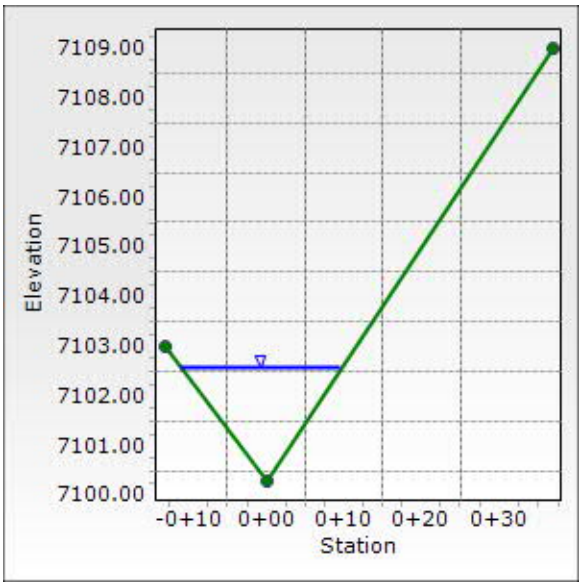
280.0
300.0
320.0
340.0
360.0
380.0


DP13 - 100-year
Label: DP25.1
Type: Manhole
ID: 73
Label: Pipe - (14)
Type: Conduit
ID: 109
Label: DP27
Type: Manhole
ID: 72
Elevation (ft)
Station (ft)
7,017.00
7,016.50
7,016.00
7,015.50
7,015.00
7,014.50
7,014.00
7,013.50
7,013.00
7,012.50
7,012.00
7,011.50
7,011.00
7,010.50
7,010.00
7,009.50
7,009.00
7,008.50
7,008.00
7,007.50
7,007.00
7,006.50
7,006.00
7,005.50
7,005.00
7,004.50
7,004.00
7,003.50
7,003.00
0.0 5.0 10.0 15.0 20.0 25.0 30.0 35.0 40.0 45.0 50.0 55.0 60.0 65.0 70.0 75.0 80.0 85.0 90.0 95.0 100.0 105.0 110.0 115.0 120.0 125.0 130.0 135.0 140.0 145.0 150.0 155.0 160.0 165.0 170.0

Cross Section for Proposed Swale - Cross Section AA

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	0.009 ft/ft
Normal Depth	27.0 in
Discharge	174.50 cfs



Worksheet for Proposed Swale - Cross Section AA

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.009 ft/ft
Discharge	174.50 cfs

Section Definitions

Station (ft)	Elevation (ft)
-0+13	7,103.00
0+00	7,100.34
0+37	7,109.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(-0+13, 7,103.00)	(0+00, 7,100.34)	0.020
(0+00, 7,100.34)	(0+37, 7,109.00)	0.020

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	27.0 in
Roughness Coefficient	0.020
Elevation	7,102.59 ft
Elevation Range	7,100.3 to 7,109.0 ft
Flow Area	23.2 ft²
Wetted Perimeter	21.1 ft
Hydraulic Radius	13.2 in
Top Width	20.63 ft
Normal Depth	27.0 in
Critical Depth	29.5 in
Critical Slope	0.006 ft/ft
Velocity	7.51 ft/s
Velocity Head	0.88 ft
Specific Energy	3.13 ft
Froude Number	1.248
Flow Type	Supercritical

Worksheet for Proposed Swale - Cross Section AA

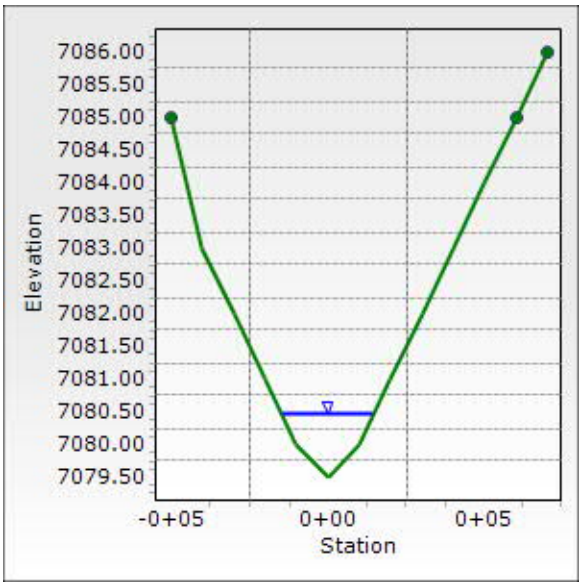
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	27.0 in
Critical Depth	29.5 in
Channel Slope	0.009 ft/ft
Critical Slope	0.006 ft/ft

Cross Section for Proposed Swale - Cross Section BB

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	0.021 ft/ft
Normal Depth	11.9 in
Discharge	10.70 cfs



Worksheet for Proposed Swale - Cross Section BB

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.021 ft/ft
Discharge	10.70 cfs

Section Definitions

Station (ft)	Elevation (ft)
-0+05	7,085.00
-0+04	7,083.00
-0+03	7,082.00
-0+02	7,081.00
-0+01	7,080.00
0+00	7,079.46
0+01	7,080.00
0+02	7,081.00
0+03	7,082.00
0+04	7,083.00
0+05	7,084.00
0+06	7,085.00
0+07	7,086.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(-0+05, 7,085.00)	(0+06, 7,085.00)	0.020
(0+06, 7,085.00)	(0+07, 7,086.00)	0.020

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	11.9 in
Roughness Coefficient	0.020
Elevation	7,080.45 ft
Elevation Range	7,079.5 to 7,086.0 ft
Flow Area	1.7 ft²
Wetted Perimeter	3.6 ft
Hydraulic Radius	5.6 in

Worksheet for Proposed Swale - Cross Section BB

Results		
Top Width	2.91	ft
Normal Depth	11.9	in
Critical Depth	14.3	in
Critical Slope	0.009	ft/ft
Velocity	6.46	ft/s
Velocity Head	0.65	ft
Specific Energy	1.64	ft
Froude Number	1.511	
Flow Type	Supercritical	

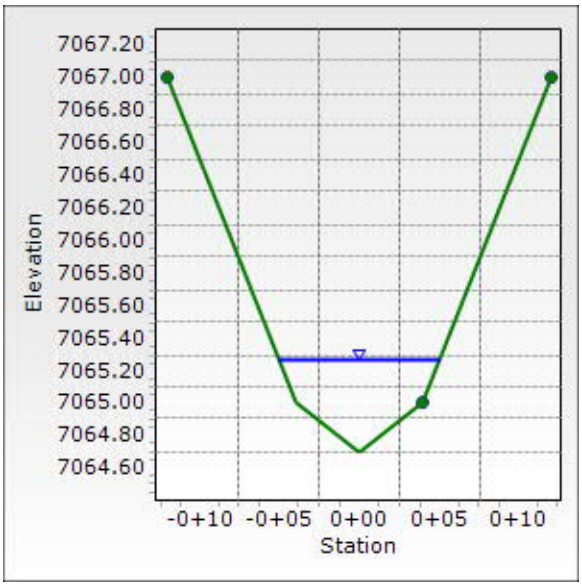
GVF Input Data		
Downstream Depth	0.0	in
Length	0.0	ft
Number Of Steps	0	

GVF Output Data		
Upstream Depth	0.0	in
Profile Description	N/A	
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	11.9	in
Critical Depth	14.3	in
Channel Slope	0.021	ft/ft
Critical Slope	0.009	ft/ft

Cross Section for Proposed Swale - Cross Section CC

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	0.021 ft/ft
Normal Depth	7.2 in
Discharge	20.60 cfs



Worksheet for Proposed Swale - Cross Section CC

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.021 ft/ft
Discharge	20.60 cfs

Section Definitions

Station (ft)	Elevation (ft)
-0+12	7,067.00
-0+04	7,065.00
0+00	7,064.66
0+04	7,065.00
0+12	7,067.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(-0+12, 7,067.00)	(0+04, 7,065.00)	0.020
(0+04, 7,065.00)	(0+12, 7,067.00)	0.020

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	7.2 in
Roughness Coefficient	0.020
Elevation	7,065.26 ft
Elevation Range	7,064.7 to 7,067.0 ft
Flow Area	3.7 ft²
Wetted Perimeter	10.2 ft
Hydraulic Radius	4.4 in
Top Width	10.10 ft
Normal Depth	7.2 in
Critical Depth	9.0 in
Critical Slope	0.008 ft/ft
Velocity	5.52 ft/s
Velocity Head	0.47 ft
Specific Energy	1.08 ft
Froude Number	1.599

Worksheet for Proposed Swale - Cross Section CC

Results	
Flow Type	Supercritical

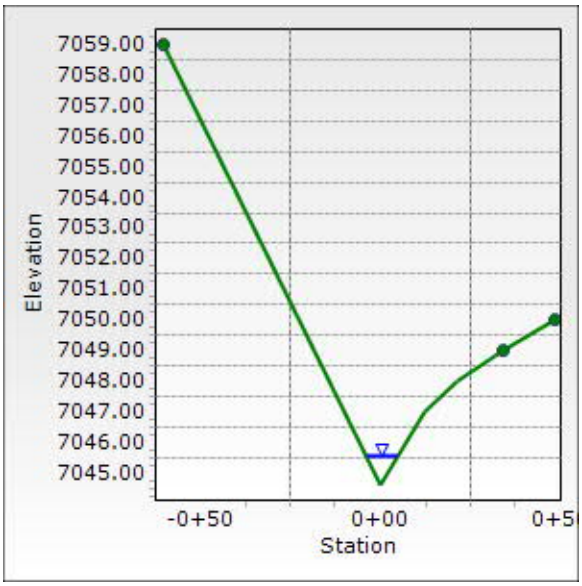
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	7.2 in
Critical Depth	9.0 in
Channel Slope	0.021 ft/ft
Critical Slope	0.008 ft/ft

Cross Section for Proposed Swale - Cross Section DD

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	0.020 ft/ft
Normal Depth	11.7 in
Discharge	27.80 cfs



Worksheet for Proposed Swale - Cross Section DD

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.020 ft/ft
Discharge	27.80 cfs

Section Definitions

Station (ft)	Elevation (ft)
-0+60	7,059.00
0+00	7,044.56
0+12	7,047.00
0+21	7,048.00
0+34	7,049.00
0+48	7,050.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(-0+60, 7,059.00)	(0+34, 7,049.00)	0.020
(0+34, 7,049.00)	(0+48, 7,050.00)	0.020

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	11.7 in
Roughness Coefficient	0.020
Elevation	7,045.54 ft
Elevation Range	7,044.6 to 7,059.0 ft
Flow Area	4.3 ft²
Wetted Perimeter	9.1 ft
Hydraulic Radius	5.7 in
Top Width	8.87 ft
Normal Depth	11.7 in
Critical Depth	14.2 in
Critical Slope	0.007 ft/ft
Velocity	6.42 ft/s
Velocity Head	0.64 ft
Specific Energy	1.62 ft

Worksheet for Proposed Swale - Cross Section DD

Results	
Froude Number	1.618
Flow Type	Supercritical

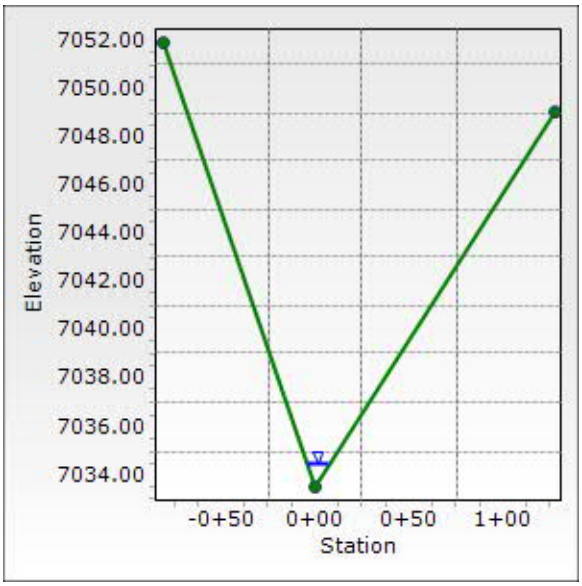
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	11.7 in
Critical Depth	14.2 in
Channel Slope	0.020 ft/ft
Critical Slope	0.007 ft/ft

Cross Section for Proposed Swale - Cross Section EE

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	0.020 ft/ft
Normal Depth	11.1 in
Discharge	33.80 cfs



Worksheet for Proposed Swale - Cross Section EE

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.020 ft/ft
Discharge	33.80 cfs

Section Definitions

Station (ft)	Elevation (ft)
-0+80	7,051.93
0+00	7,033.54
1+27	7,049.03

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(-0+80, 7,051.93)	(0+00, 7,033.54)	0.020
(0+00, 7,033.54)	(1+27, 7,049.03)	0.020

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	11.1 in
Roughness Coefficient	0.020
Elevation	7,034.47 ft
Elevation Range	7,033.5 to 7,051.9 ft
Flow Area	5.4 ft²
Wetted Perimeter	11.8 ft
Hydraulic Radius	5.5 in
Top Width	11.65 ft
Normal Depth	11.1 in
Critical Depth	13.5 in
Critical Slope	0.007 ft/ft
Velocity	6.25 ft/s
Velocity Head	0.61 ft
Specific Energy	1.53 ft
Froude Number	1.616
Flow Type	Supercritical

Worksheet for Proposed Swale - Cross Section EE

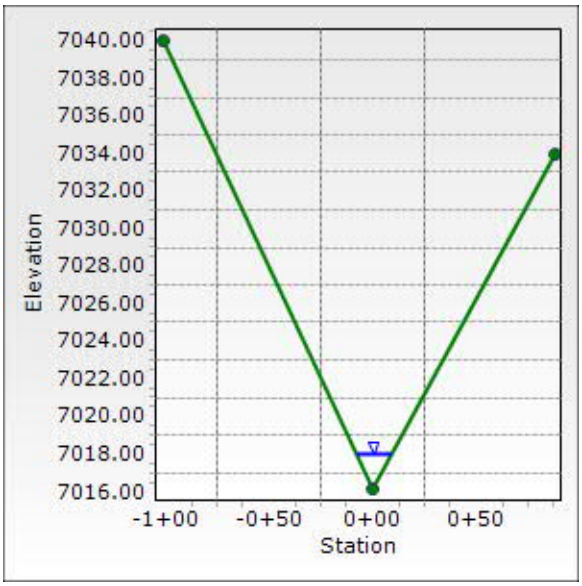
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	11.1 in
Critical Depth	13.5 in
Channel Slope	0.020 ft/ft
Critical Slope	0.007 ft/ft

Cross Section for Proposed Swale - Cross Section FF

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	0.023 ft/ft
Normal Depth	21.5 in
Discharge	151.70 cfs



Worksheet for Proposed Swale - Cross Section FF

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.023 ft/ft
Discharge	151.70 cfs

Section Definitions

Station (ft)	Elevation (ft)
-1+01	7,040.00
0+00	7,016.22
0+88	7,034.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(-1+01, 7,040.00)	(0+00, 7,016.22)	0.020
(0+00, 7,016.22)	(0+88, 7,034.00)	0.020

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	21.5 in
Roughness Coefficient	0.020
Elevation	7,018.01 ft
Elevation Range	7,016.2 to 7,040.0 ft
Flow Area	14.7 ft²
Wetted Perimeter	16.8 ft
Hydraulic Radius	10.5 in
Top Width	16.46 ft
Normal Depth	21.5 in
Critical Depth	27.9 in
Critical Slope	0.006 ft/ft
Velocity	10.30 ft/s
Velocity Head	1.65 ft
Specific Energy	3.44 ft
Froude Number	1.920
Flow Type	Supercritical

Worksheet for Proposed Swale - Cross Section FF

GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	21.5 in
Critical Depth	27.9 in
Channel Slope	0.023 ft/ft
Critical Slope	0.006 ft/ft

Appendix D
Reference Materials

MASTER DEVELOPMENT DRAINAGE PLAN FOR STERLING RANCH

OCTOBER 2018

Prepared for:

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Project #09-002
SKP-18-003
SF-17-024

at DP87 culminating in peak runoff rates within Sand Creek of Q5 = 374.6 cfs, Q100 = 1905.9 cfs.

Basin SC3-16A (Q5 = 120.4 cfs, Q100 = 351.8 cfs) consists of a 168.1 acre area located within Sterling Ranch, that is located north of Briargate Parkway and east of Sand Creek Channel. This portion of Sterling Ranch is planned to house residential development that ranges from low density rural lots 1 acres in size to medium density urban residential with lots ranging in size from 0.1 to 0.2 acres. Runoff from the basin shall be collected and conveyed within street and storm sewer systems to a full spectrum detention pond (FSD16A), at the northwest corner of Briargate Parkway and Sterling Ranch Road. The treated detained flows from the pond will discharge to DP22 at peak flow rates of 8.8 cfs and 128.3 cfs in the 5 and 100 year events respectively.

Basin SC3-16B (Q5 = 53.7 cfs, Q100 = 143.8 cfs) consists of a 50.7 acre area located within Sterling Ranch, that is located north of Briargate Parkway and east of Sand Creek Channel. This portion of Sterling Ranch is planned for a low to medium density residential lots ranging in size from 0.1 to 0.2 acres lots and portions of roadways. Runoff from the basin shall be collected and conveyed within street and storm sewer systems to a full spectrum detention pond (FSD16B), at the northeast corner of Briargate Parkway and Sterling Ranch Road. The treated detained flows from the pond will discharge to DP22 at peak flow rates of 0.4 cfs and 28.1 cfs in the 5 and 100 year events respectively. The combined peak flow rates from SC3-16B and FSD14A (DP22, Q5=8.8 cfs and Q100=174.9 cfs) will be conveyed south via storm sewer system to DP21.

Basin SC3-14B (Q5 = 34.3 cfs, Q100 = 94.1 cfs) consists of a 34.7 acre area located within of Sterling. Ranch, that is located between south of Briargate Parkway and east of Sterling Ranch Road, east of Sand Creek. This portion of Sterling Ranch is planned for a low to medium density residential lots ranging in size from 0.1 to 0.33 acres lots and portions of roadways. Runoff from the basin shall be collected and conveyed within street and storm sewer systems to a full spectrum detention pond (FSD14B), at the south end of the basin. The treated detained flows from the pond will discharge to DP21 at peak flow rates of 0.3 cfs and 19.3 cfs in the 5 and 100 year events respectively. The combined peak flow rates from DP22 and FSD14B (DP21, Q5=8.8 cfs and Q100=174.9 cfs) will be conveyed to Pond W3 above the intersection of Sand Creek channel and Sterling Ranch Road.

Basin SC3-14A (Q5 = 175.4 cfs, Q100 = 466.3 cfs) consists of a 164.9 acre area located within of Sterling. Ranch, that is located between south of Briargate Parkway and east of Sterling Ranch Road, east of Sand Creek. This portion of Sterling Ranch is planned for a k-8 school site, several single family residential lots ranging in size from 0.2 to 0.33 acres lots as well as portions of park and open space. Runoff from the basin shall be collected and conveyed within street and storm sewer systems and directed to a full spectrum detention pond (FSD14A), at the southwest corner of the basin. The treated detained flows from the pond will discharge to Pond W3 at peak flow rates of 7.5 cfs and 142.2 cfs in the 5 and 100 year events respectively.

Basin SC3-13 (Q5 = 57.8 cfs, Q100 = 136.9 cfs) consists of a 41.0 acre area located within of Sterling. Ranch, that is located just the east of the Barbarick Subdivision and north of Sterling Ranch Road. This portion of Sterling Ranch is planned for residential lots ranging in size from 0.1 to 0.2 acres in size. Runoff from the basin shall be collected by storm sewer systems and conveyed to a full spectrum detention pond (FSD13) located in the south end of the basin, adjacent to sand creek. The treated detained flows from the pond will discharge into Sand Creek at peak flow rates of 4.2 cfs and 47.2 cfs in the 5 and 100 year events respectively.

Runoff from DP87, DP21 and from FSD Ponds 13, and 14A will combine within the Sand Creek Channel at proposed Regional Pond Detention Facility W3. The purpose of the regional pond is to reduce the post development flow rates within the Sand Creek Channel at the Southern Sterling Ranch boundary to at or below the existing flow rates calculated by this report. The pond is also necessary due to the drainage basin diversion, as discussed in other parts of this report. The total combined discharge reaching the regional facility (Pond W-3) has been calculated at 374.5 cfs and 2204.1 cfs in the 5 and 100 year events respectively.

As conceptually designed the proposed facility will utilize a check/diversion wall located upstream of the existing stock pond and proposed detention facility that will function to divert base flows within the channel to aid in retaining a fixed water surface within the existing stock pond and in larger storm events diverted flows safely around the amenity to the west side to detention Pond W3. A small controlled outlet structure along with an improved downstream embankment will be added to the existing stock pond to stabilize it and retain a fixed maximum water surface elevation. In the larger detention pond eight (8) small 24" storm sewer pipe located within a separate embankment will allow for free flow discharge of 2 year runoff and begin to detain flows of 5 years and larger events. Flows exiting the small storm pipes or overtopping the separated embankment will enter a concrete forebay that conveys drainage to two (2) cell 8'h x10'w concrete box culvert (CBC) under Proposed Sterling Ranch Road to DP68. As the anticipated flow rate leaving the pond is planned to be less than 1,500 cfs, and the proposed culvert crossing is conceptually planned to have an open area of less than 200 ft sq of open area and thus will need to meet the headwater requirements of Table 6-5 of the DCM, which in this concept design is a ratio of about ~1.3. The total combined discharge calculated to leave the regional facility (Pond W-3) has been calculated at 200.3 cfs and 1,350.6 cfs in the 5 and 100 year events respectively, with a maximum 100 year water surface of 7017.3, a

HW/D ratio of ~1.3. The peak detained volume has been estimated at 78.2 ac-ft. A low point in Sterling Ranch Road will be designed adjacent to the facility to provide a safe overflow route. An exhibit showing the concept design and its various elements is included in the appendix of this report.

As previously discussed a Condition Letter of Map Revision and Letter of Map Revision (CLOMR/LOMR) will need to be processed through the Federal Emergency Management Agency (FEMA) to revise the hydrology to the Sand Creek Channel and allow for the remapping of the revised floodplains. It should be noted that the DBPS flow rates for Reach SC-8 (Reach 163) adjacent to this location were estimate to be 2,630 cfs and that the effective FEMA 100 year flow rate is 2,600cfs. A comparison table of the various flow rates is provided later in this text and on the accompanying drainage maps.

The final design of the culvert crossing and final determination of approved rates as well as the final pond design will be discussed within the future Sterling Ranch Channel Design Report and Sand Creek CLOMR/LOMR documents. No deviations for this pond and accompanying outlet structure are anticipated at this time.

It is important to note that the planned discharge outlet pipe for the FSD pond located to the west of the pond W3 will need to be extended to the downstream outlet side of the culvert to ensure that the 100 year water surface elevation with W3 does not affect the functionality of the adjacent FSD and its storm sewer systems.

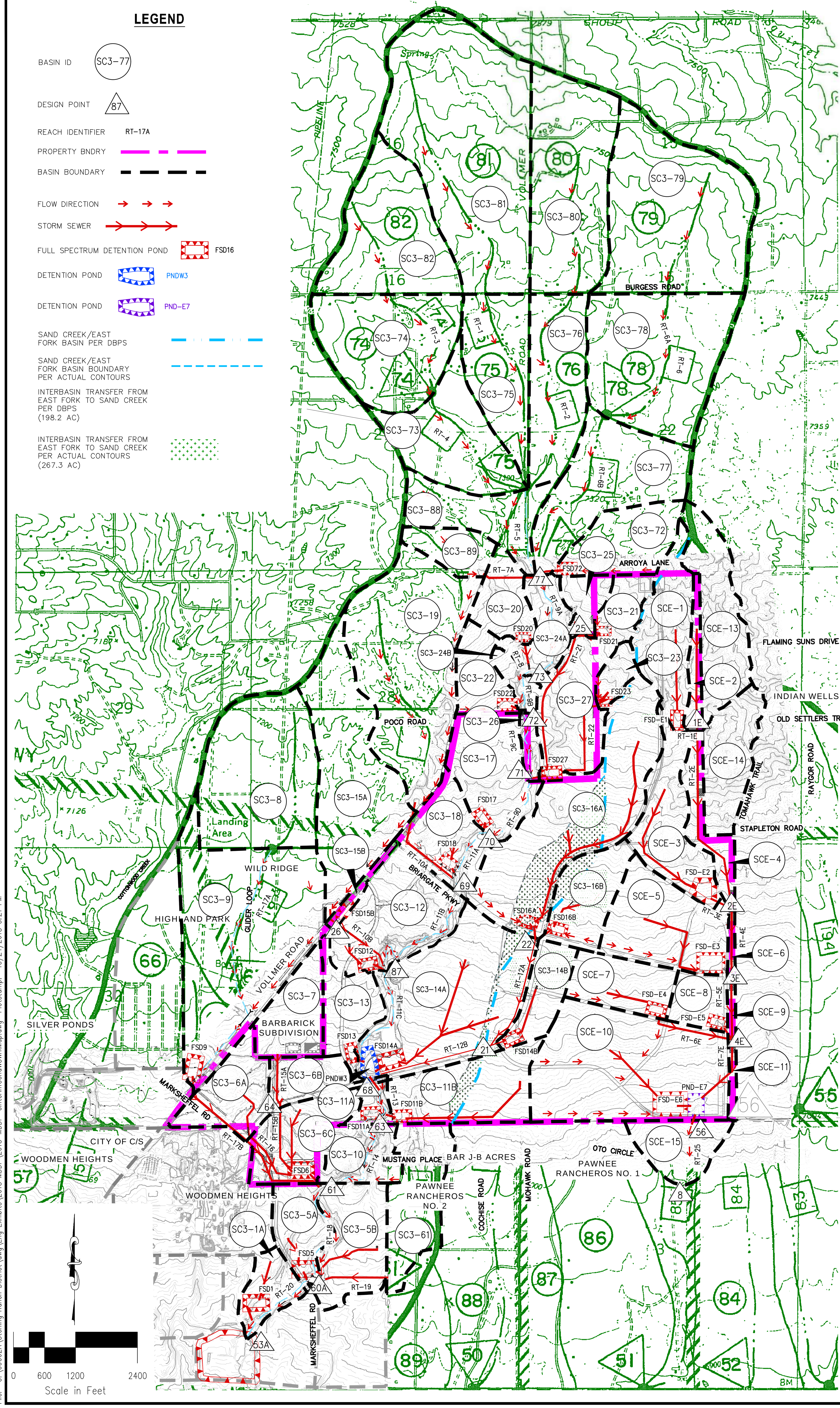
In regards to timing, the need to construction this facility can be tied to the Sand Creek Channel improvements which is discussed within this report and also within the Subdivision Improvements Agreement. In no case should runoff from the East Fork of Sand Creek be diverted to the Main Branch of the Sand Creek Channel prior to the construction and of this facility.

Basin SC3-11A (Q5 = 7.8 cfs, Q100 = 24.3 cfs) consists of a 10.7 acre area located within of Sterling. Ranch, that is south of Sterling Ranch Road, west of Sand Creek. This portion of Sterling Ranch consists of single family residential for lots ranging in size from 0.2 to 0.3 acres in size and open space associated with the Sand Creek Channel. Runoff from the developed portion of the basin shall be collected and conveyed within street and storm sewer systems to a full spectrum detention pond FSD11A. The treated detained flows from the pond will discharge into Sand Creek at peak flow rates of 0.9 cfs and 12.3 cfs in the 5 and 100 year events respectively just upstream of DP-63. It should be noted that this detention facility may not be necessary if grading can be oriented to force surface runoff to the west.

Basin SC3-11B (Q5 = 81.3 cfs, Q100 = 213.7 cfs) consists of a 76.6 acre area located within of Sterling. Ranch, that is south of Sterling Ranch Road, east of Sand Creek. This portion of Sterling Ranch consists of single family residential planned for lots ranging in size from 0.2 to 0.3 acres in size and a portion of a park site and collector roadways. Runoff from the developed portion of the basin shall be collected and conveyed within street and storm sewer systems westward to a full spectrum detention pond FSD11B. The treated detained flows from the pond will discharge into Sand Creek at peak flow rates of 4.5 cfs and 69.5 cfs in the 5 and 100 year events respectively. The runoff from DP68 and from FSD ponds 11A and 11B combine at DP63 at peak flow rates of Q5 = 201.0 cfs, Q100 = 1385.1, which is less than the anticipated existing modeled flow rates of Q5 = 430.7 cfs, Q100 = 1911.5 at DP63. Runoff from DP63 continues south within the Sand Creek Channel toward DP61.

Basin SC3-7 (Q5 = 69.9 cfs, Q100 = 157.2 cfs) consists of a 45.7 acre industrial zoned area, referred to as the Barbarick Subdivision, located outside of Sterling Ranch. Per the Final Drainage Report for Barbarick Subdivision, Portions of Lots 1, 2 and Lots 3and 4 the filing consists of four lots which upon which development will be constructed which will include adding a proposed Extended Detention Basin within Lot 4. This detention basin will provide water quality treatment for portions of Lots 1 & 2, and Lots 3 & 4. The EBD will structure will outfall at the south end of Lot 4 at the Barbarick Subdivision/Sterling Ranch property line. Per the report the proposed total outflow from the EDB pond will be Q5 = 0.3 cfs, Q100 = 45.9** cfs(**which includes pass through flows of 29.4 cfs). A second Sand Filter Basin water quality detention catchment will be provided at the southeast/downstream end of Lot 2. The SFB will outfall at the southeast corner of the Lot 2 at the Barbarick Subdivision/Sterling Ranch property line. Per the report the proposed total outflow the SFB pond will be Q5 = 0.1 cfs, Q100 = 3.6 cfs. At the initial writing of this report, neither EDB nor SFB structure has been fully constructed, and thus the assumption was made to utilize the full un-detained untreated runoff from the offsite development for onsite drainage planning purposes. Thus the downstream facilities planned within Sterling Ranch will account for the total un-detained runoff from the parcel of Q5 = 69.9 cfs, Q100 = 157.2 cfs and will plan to treat the total runoff onsite facilities. This provides a conservative approach for master planning. Runoff discharged from the property will be collected by proposed storm sewer within Sterling Ranch and routed to DP64. These facilities and their effects on drainage will be re-reviewed with subsequent drainage report and shall be implemented into final design and construction.

Basin SC3-6B (Q5=43.4 cfs, Q100=102.7 cfs) consists of a 30.9 acre area located within of Sterling Ranch, that is north of Sterling



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BASIN SUMMARY

BASIN	CN	AREA (ACRES)	AREA (SQ MI)	Q_2 (CFS)	Q_5 (CFS)	Q_{10} (CFS)	Q_{25} (CFS)	Q_{50} (CFS)	Q_{100} (CFS)
SC3–1A	73	27.8	0.044	16.3	23.3	33.0	45.8	57.1	68.9
SC3–5A	84	39.1	0.061	40.6	53.7	71.0	92.4	110.6	129.1
SC3–5B	81	63.0	0.098	53.8	73.0	98.5	130.8	158.6	187.0
SC3–6A	88	49.3	0.077	61.4	79.3	102.2	130.1	153.6	177.1
SC3–6B	85	30.9	0.048	32.9	43.4	57.0	73.9	88.2	102.7
SC3–6C	82	58.0	0.091	53.9	72.5	97.1	128.0	154.5	181.5
SC3–7	88	45.7	0.071	54.0	69.9	90.3	115.2	136.2	157.2
SC3–8	62	143.4	0.224	25.4	42.1	66.7	100.7	132.3	166.2
SC3–9	66	217.4	0.340	45.8	71.5	108.6	158.9	204.9	254.0
SC3–10	63	36.0	0.056	7.6	12.3	19.4	29.1	38.0	47.7
SC3–11A	70	10.7	0.017	5.3	7.8	11.3	15.9	20.0	24.3
SC3–11B	80	76.6	0.120	59.4	81.3	110.8	148.1	180.5	213.7
SC3–12	81	88.2	0.138	77.8	105.6	142.5	189.1	229.1	270.0
SC3–13	85	41.0	0.064	43.9	57.8	76.0	98.5	117.6	136.9
SC3–14A	79	164.9	0.258	127.6	175.4	239.8	321.9	393.2	466.3
SC3–14B	77	34.7	0.054	24.6	34.3	47.4	64.2	79.0	94.1
SC3–15A	62	139.7	0.218	21.3	35.5	56.3	85.3	112.1	141.0
SC3–15B	87	7.9	0.012	10.8	14.0	18.2	23.3	27.6	31.9
SC3–16A	74	168.1	0.263	84.4	120.4	170.0	234.8	292.2	351.8
SC3–16B	78	50.7	0.079	39.0	53.7	73.6	99.0	121.1	143.8
SC3–17	73	70.6	0.110	41.8	59.6	85.2	119.0	149.1	180.6
SC3–18	81	53.8	0.084	49.3	67.1	91.0	121.2	147.3	174.0
SC3–19	62	184.0	0.287	28.8	47.7	75.7	114.4	150.2	188.8
SC3–20	65	34.2	0.053	9.9	15.5	23.8	35.1	45.5	56.6
SC3–21	66	23.3	0.036	7.0	10.8	16.3	23.7	30.4	37.5
SC3–22	65	33.9	0.053	9.4	14.8	22.5	32.9	42.5	52.6
SC3–23	67	14.5	0.023	5.5	8.3	12.4	18.0	23.0	28.4
SC3–24A	65	35.7	0.056	13.0	20.4	31.1	45.7	59.0	73.2
SC3–24B	65	12.2	0.019	3.4	5.3	8.1	11.8	15.2	18.9
SC3–25	66	19.0	0.030	5.8	8.9	13.4	19.5	25.1	31.0
SC3–26	63	10.0	0.016	2.5	4.0	6.2	9.2	12.1	15.1
SC3–27	71	70.0	0.109	35.1	51.2	73.8	103.7	130.3	158.3
SC3–61	63	65.5	0.102	13.7	22.0	34.4	51.6	67.6	84.8
SC3–72	64	56.2	0.088	12.8	20.2	31.4	46.7	60.9	76.0
SC3–73	63	90.0	0.141	16.4	26.4	41.3	62.1	81.3	102.0
SC3–74	63	119.7	0.187	22.3	36.5	57.3	85.9	112.3	140.7
SC3–75	63	79.3	0.124	13.1	21.5	33.7	50.5	66.1	82.8
SC3–76	63	86.4	0.135	14.2	23.1	36.4	54.6	71.4	89.6
SC3–77	62	106.9	0.167	16.6	27.6	43.8	66.2	87.0	109.4
SC3–78	63	155.6	0.243	28.1	45.3	70.6	106.2	139.1	174.5
SC3–79	63	189.0	0.295	34.9	57.0	89.5	134.3	175.6	220.1
SC3–80	63	147.7	0.231	27.3	44.3	69.6	104.5	136.8	171.4
SC3–81	62	262.9	0.411	42.6	70.2	111.0	167.4	219.6	275.7
SC3–82	62	117.8	0.184	20.0	33.2	52.8	80.0	105.1	132.3
SC3–88	62	60.2	0.094	10.5	17.4	27.6	41.8	54.9	69.0
SC3–89	62	27.5	0.043	6.1	10	15.7	23.6	30.8	38.6
SCE–1	65	64.4	0.101	23.3	35.9	53.8	79.1	102.4	127.4
SCE–2	64	15.0	0.023	4.4	7.0	10.8	15.9	20.7	25.7
SCE–3	70	67.5	0.105	30.6	45.2	65.9	93.3	118.0	143.9
SCE–4	70	29.5	0.046	13.3	19.6	28.6	40.6	257.8	62.6
SCE–5	87	85.5	0.134	100.4	130.6	169.6	217.4	257.8	298.4
SCE–6	64	3.8	0.006	1.6	2.5	3.7	5.4	7.0	8.6
SCE–7	89	44.9	0.070	58.9	75.5	96.6	122.2	143.7	165.2
SCE–8	92	25.5	0.040	38.6	48.4	60.7	75.4	87.7	99.9
SCE–9	64	4.0	0.006	1.5	2.4	3.6	5.3	6.8	8.5
SCE–10	83	174.3	0.272	7.6	189.4	19.4	29.1	398.9	467.5
SCE–11	64	5.8	0.009	2.3	3.6	5.5	8.0	10.3	12.8
SCE–13	63	78.6	0.123	19.6	31.3	48.7	73.1	95.7	120.0
SCE–14	63	52.5	0.082	13.2	21.2	33.3	49.9	65.2	81.7
SCE–15	51	39.7	0.062	2.2	5.1	10.1	17.7	25.1	33.4

DESIGN POINT SUMMARY

DESIGN POINT	AREA (SQ MI)	Q_2 (CFS)	Q_5 (CFS)	Q_{10} (CFS)	Q_{25} (CFS)	Q_{50} (CFS)	Q_{100} (CFS)	LOCATION
DP–74	0.371	39.3	65.3	104.8	158.9	209.1	262.8	
DP–75	1.413	141.2	235.1	376.6	566.6	750.9	950.5	
DP–77	2.343	209.9	351.9	580.6	886.6	1168.4	1467.7	ARROYA LANE X–ING
DP–78	0.538	59.7	98.4	154.0	232.6	306.2	385.3	
DP–73	2.471	207.5	354.3	588.5	897.1	1187.2	1506.7	
DP–72	2.543	206.2	352.5	586.7	897.2	1195.3	1518.6	POCO ROAD X–ING
DP–71	2.757	205.9	349.3	610.5	932.4	1226.9	1612.2	STERLING RANCH NORTHERN BNDRY
DP–70	2.867	205.3	349.8	614.0	940.1	1260.6	1636.7	
DP–69	3.238	212.7	366.6	653.7	1010.6	1364.1	1775.7	BRIARGATE PARKWAY X–ING
DP–87	3.594	216.9	374.6	681.9	1072.1	1471.5	1905.9	
DP–68	4.312	214.6	374.5	714.9	1187.6	1674.9	2204.1	UPSTREAM OF POND W3
DP–64	0.119	85.9	112.1	145.9	187.5	222.6	258.0	
DP–63	4.449	154.4	201.0	375.7	815.9	1112.1	1385.1	STERLING RANCH SOUTHERN BNDRY
DP–61	5.356	156.6	223.9	428.0	928.2	1287.3	1620.1	COLORADO SPRINGS/EL PASO BNDRY
DP–60A	5.617	161.6	224.8	439.1	950.4	1320.5	1661.8	MARKSHEFFEL X–ING
DP–53A	5.661	161.6	225.7	441.8	951.1	1326.0	1668.9	SAND CREEK AND POND 3
DP–1E	0.247	23.9	38.3	70.1	132.8	173.0	220.9	
DP–2E	0.486	48.9	76.8	123.0	228.7	319.7	419.4	
DP–3E	0.626	48.5	75.7	122.2	271.1	387.1	500.1	
DP–4E	0.745	48.1	76.2	122.4	286.9	407.3	534.8	
DP–56	1.017	23.1	35.3	71.5	108.3	152.1	196.4	NEAR SE PROP CORNER
DP–8	1.079	24.1	37.2	73.5	111.3	155.4	200.7	BELOW SE PROP CORNER
DP–21	0.396	0.6	8.8	17.8	57.1	116.8	174.9	
DP–22	0.342	0.6	8.8	17.6	56.8	105.1	156.4	
DP–25	0.066	5.9	9.1	16.3	35.1	46.4	58.2	
DP–26	0.012	0.1	1.1	3.2	7.3	9.5	12.0	

DESIGN POINT SUMMARY (VOLUME)

DESIGN POINT	AREA (SQ MI)	V_2 (AC–FT)	V_5 (AC–FT)	V_{10} (AC–FT)	V_{25} (AC–FT)	V_{50} (AC–FT)	V_{100} (AC–FT)	LOCATION
DP–74	0.371	5.9	9.0	13.6	19.8	25.5	31.6	
DP–75	1.413	22.7	34.5	51.7	75.4	97.1	120.5	
DP–77	2.343	37.7	57.4	85.9	125.1	161.1	199.9	ARROYA LANE X–ING
DP–78	0.538	8.9	13.5	20.1	29.3	37.7	46.7	
DP–73	2.471	40.0	60.8	91.0	132.5	170.7	211.7	
DP–72	2.543	41.3	62.9	94.0	136.8	176.2	218.5	POCO ROAD X–ING
DP–71	2.757	46.3	70.0	104.3	151.3	194.5	240.8	STERLING RANCH NORTHERN BNDRY
DP–70	2.867	49.5	74.5	110.6	160.1	205.4	254.0	
DP–69	3.238	57.5	86.1	127.4	183.8	235.3	290.6	BRIARGATE PARKWAY X–ING
DP–87	3.594	66.5	98.9	145.6	209.1	267.1	329.1	
DP–68	4.312	81.8	123.7	183.9	264.9	338.0	415.8	UPSTREAM OF POND W3
DP–64	0.119	7.0	9.1	11.8	15.2	18.1	21.1	
DP–63	4.449	85.6	129.5	192.3	276.7	352.8	433.5	STERLING RANCH SOUTHERN BNDRY
DP–61	5.356	103.7	157.8	235.1	338.4	431.3	529.8	COLORADO SPRINGS/EL PASO BNDRY
DP–60A	5.617	111.0	168.6	250.4	359.5	457.7	561.5	MARKSHEFFEL X–ING
DP–53A	5.661	112.0	170.0	252.6	362.6	461.7	566.5	SAND CREEK AND POND 3
DP–1E	0.247	3.1	5.2	8.4	12.7	16.6	20.9	
DP–2E	0.480	6.1	10.4	16.9	25.7	33.7	42.2	
DP–3E	0.620	7.0	13.7	23.4	36.1	47.4	59.3	
DP–4E	0.736	7.6	15.6	27.2	43.0	57.2	72.0	
DP–56	1.017	7.7	16.1	28.6	51.3	71.7	92.9	NEAR SE PROP CORNER
DP–8	1.079	8.0	16.7	26.6	53.0	74.0	95.9	BELOW SE PROP CORNER
DP–21	0.396	6.3	11.3	18.3	27.5	35.6	44.0	
DP–22	0.736	6.3	10.7	16.7	24.6	31.5	38.7	
DP–25	1.017	1.3	1.9	2.8	4.1	5.2	6.4	
DP–26	1.079	0.7	0.9	1.2	1.5	1.8	2.1	

WATER QUALITY & DETENTION POND SUMMARY

FSD1						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	16.3	23.3	33.0	45.8	57.1	68.9
ALLOWABLE RELEASE (CFS)	0.1	1.7	3.3	10.9	17.5	25.5
MODELED RELEASE (CFS)	0.1	1.6	3.2	10.9	17.4	25.4
STORED VOLUME (AC–FT)	2.4	2.6	3.0	3.6	1.9	2.2

FSD5						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	40.6	53.7	71.0	92.4	110.6	129.1
ALLOWABLE RELEASE (CFS)	0.1	1.4	2.6	11.3	19.8	30.2
MODELED RELEASE (CFS)	0.1	1.4	2.6	11.2	19.7	30.1
STORED VOLUME (AC–FT)	3.0	3.2	3.8	4.1	4.7	5.2

FSD6						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	196.5	258.5	339.1	438.7	523.3	608.6
ALLOWABLE RELEASE (CFS)	0.5	7.6	14.6	58.4	99.6	149.7
MODELED RELEASE (CFS)	0.5	7.5	14.5	58.2	99.6	149.6
STORED VOLUME (AC–FT)	15.5	16.4	18.7	20.8	23.3	26.0

FSD9						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	64.6	105.6	169.5	252.3	327.1	410.1
ALLOWABLE RELEASE (CFS)	1.7	24.9	49.8	141.1	207.2	290.0
MODELED RELEASE (CFS)	1.7	24.9	49.8	141.1	207.0	289.9
STORED VOLUME (AC–FT)	8.7	8.7	9.6	10.8	12.3	13.8

FSD11A						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	5.3	7.8	11.3	15.9	20.0	24.3
ALLOWABLE RELEASE (CFS)	0.1	1.6	3.2	7.5	9.7	12.4
MODELED RELEASE (CFS)	0.2	0.9	3.0	7.5	9.7	12.3
STORED VOLUME (AC–FT)	0.3	0.3	0.4	0.4	0.5	0.6

FSD11B						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	59.4	81.3	110.8	148.1	180.5	213.7
ALLOWABLE RELEASE (CFS)	0.3	4.5	8.7	29.6	47.7	69.6
MODELED RELEASE (CFS)	0.3	4.5	8.6	29.5	47.7	69.5
STORED VOLUME (AC–FT)	4.8	4.9	5.5	6.4	7.3	8.2

FSD12						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	77.8	105.6	142.5	189.1	229.1	270.0
ALLOWABLE RELEASE (CFS)	0.9	13.2	26.7	62.0	80.2	103.2
MODELED RELEASE (CFS)	0.9	9.0	26.7	61.9	80.1	103.1
STORED VOLUME (AC–FT)	5.2	5.5	5.8	6.7	7.8	8.9

FSD13						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	43.9	57.8	76.0	98.5	117.6	136.9
ALLOWABLE RELEASE (CFS)	0.4	6.1	12.3	28.6	37.0	47.6
MODELED RELEASE (CFS)	0.4	4.2	12.3	28.6	36.9	47.2
STORED VOLUME (AC–FT)	3.1	3.1	3.3	3.8	4.4	5.0

FSD14A						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	127.6	175.4	239.8	321.9	393.2	466.3
ALLOWABLE RELEASE (CFS)	0.5	7.5	14.4	56.2	95.2	142.4
MODELED RELEASE (CFS)	0.5	7.5	14.4	56.2	95.1	142.2
STORED VOLUME (AC–FT)	9.9	10.6	11.9	13.5	15.3	17.3

FSD14B						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	24.6	34.3	47.4	64.2	79.0	94.1
ALLOWABLE RELEASE (CFS)	0.0	0.3	0.5	5.7	11.8	19.3
MODELED RELEASE (CFS)	0.0	0.3	0.5	4.5	11.8	19.3
STORED VOLUME (AC–FT)	1.9	2.5	3.3	3.5	3.5	3.8

FSD15B						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	10.8	14.0	18.2	23.3	27.6	31.9
ALLOWABLE RELEASE (CFS)	0.1	1.6	3.2	7.3	9.5	12.0
MODELED RELEASE (CFS)	0.1	1.1	3.2	7.3	9.5	12.0
STORED VOLUME (AC–FT)	0.6	0.6	0.7	0.8	0.9	1.0

FSD16A						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	84.4	120.4	170.0	234.8	292.2	351.8
ALLOWABLE RELEASE (CFS)	0.6	8.8	17.3	56.2	88.4	128.3
MODELED RELEASE (CFS)	0.6	8.8	17.3	56.2	88.3	128.3
STORED VOLUME (AC–FT)	7.6	7.7	8.9	10.4	12.1	13.8

SAND CREEK FLOW COMPARISON CHART

DESIGN POINT	AREA (SQ MI)	Q_{100} (CFS)	DESCRIPTION
DP–77	2.343	1468	PROPOSED CONDITION
	2.91	2262	SAND CREEK DBPS
		2600	FEMA
DP–71	2.757	1612	PROPOSED CONDITION
		2260	SAND CREEK DBPS
DP–63	4.449	1385	PROPOSED CONDITION
	4.33	2630	SAND CREEK DBPS
		2600	FEMA
DP–60A	5.661	1662	PROPOSED CONDITION
	5.38	3295	SAND CREEK DBPS

EFSC DBPS DESIGN POINT SUMMARY (PEAK FLOW)

DBPS DESIGN POINT	AREA (SQ MI)	Q_{10} (CFS) (EXIST)	Q_{100} (CFS) (EXIST)	AREA (SQ MI)	Q_{10} (CFS) (PROP)	Q_{100} (CFS) (PROP)
DP–50	0.32	47.0	195.7	0.32	146.7	370.3
DP–51 (BASIN 86)	0.33	17.7	74.1	0.33	110.0	233.5
DP–52	1.67	80.5	456.5	1.67	1207.9	2123.0
DP–56	0.79	63.6	265.0	0.79	513.0	908.2

Values reported from SCDBPS, (DP 50, 51, 52 Not analyzed as a part of this study)
DBPS Reach 85(Basin91)=Q10=28.8cfs Q100=115.2cfs (EXISTING) / Q10=345.7cfs Q100=588.9cfs (PROPOSED)

WATER QUALITY & DETENTION POND SUMMARY

FSD16B						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	39.0	53.7	73.6	99.0	121.1	143.8
ALLOWABLE RELEASE (CFS)	0.0	0.4	0.7	8.3	17.2	28.2
MODELED RELEASE (CFS)	0.0	0.4	0.7	7.9	17.2	28.1
STORED VOLUME (AC–FT)	3.0	3.9	5.1	5.1	5.3	5.8

FSD17						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	41.8	59.6	85.2	119.0	149.1	180.6
ALLOWABLE RELEASE (CFS)	0.7	11.1	22.5	52.0	67.2	86.3
MODELED RELEASE (CFS)	0.7	8.4	22.4	52.0	67.2	86.1
STORED VOLUME (AC–FT)	2.6	2.6	2.8	3.4	4.0	4.7

FSD18						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	49.3	67.1	91.0	121.2	147.3	174.0
ALLOWABLE RELEASE (CFS)	0.6	9.2	18.4	42.2	54.6	69.9
MODELED RELEASE (CFS)	0.6	6.3	18.4	42.2	54.6	69.6
STORED VOLUME (AC–FT)	3.2	3.2	3.4	4.0	4.7	5.3

FSD20						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	9.9	15.5	23.8	35.1	45.5	56.6
ALLOWABLE RELEASE (CFS)	0.4	5.5	11.1	25.7	33.2	42.5
MODELED RELEASE (CFS)	0.4	2.8	10.9	25.7	33.0	42.4
STORED VOLUME (AC–FT)	0.7	0.8	0.8	0.9	1.0	1.2

FSD21						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	7.0	10.8	16.3	23.7	30.4	37.5
ALLOWABLE RELEASE (CFS)	0.3	4.0	8.0	18.3	23.7	30.3
MODELED RELEASE (CFS)	0.3	3.3	8.0	18.3	23.7	30.1
STORED VOLUME (AC–FT)	0.5	0.5	0.5	0.6	0.7	0.8

FSD22						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	9.4	14.8	22.5	32.9	42.5	52.6
ALLOWABLE RELEASE (CFS)	0.4	5.8	11.5	26.5	34.3	43.9
MODELED RELEASE (CFS)	0.4	5.8	11.4	26.5	34.3	43.8
STORED VOLUME (AC–FT)	0.6	0.6	0.7	0.8	0.9	1.0

FSD23						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	5.5	8.3	12.4	18.0	23.0	28.4
ALLOWABLE RELEASE (CFS)	0.2	2.4	4.9	11.2	14.5	18.6
MODELED RELEASE (CFS)	0.2	2.0	4.9	11.2	14.5	18.6
STORED VOLUME (AC–FT)	0.3	0.3	0.4	0.4	0.5	0.6

FSD27						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	38.8	57.6	84.1	119.7	159.2	206.3
ALLOWABLE RELEASE (CFS)	1.4	21.1	42.4	97.8	126.4	161.9
MODELED RELEASE (CFS)	1.4	18.4	42.3	97.7	126.2	161.9
STORED VOLUME (AC–FT)	2.7	2.8	2.9	3.2	3.7	4.2

FSD72						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	12.8	20.2	31.4	46.7	60.9	76.0
ALLOWABLE RELEASE (CFS)	0.6	9.6	19.3	44.4	57.4	73.4
MODELED RELEASE (CFS)	0.6	9.3	19.2	44.4	57.4	73.4
STORED VOLUME (AC–FT)	1.0	1.0	1.1	1.1	1.2	1.3

PNDW3						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	214.6	374.5	714.9	1187.6	1674.9	2204.1
MODELED RELEASE (CFS)	154.3	200.3	366.8	799.9	1085.6	1350.6
STORED VOLUME (AC–FT)	2.8	9.5	26.3	41.2	57.2	78.2

FSD–E1						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	23.3	35.9	53.8	79.1	102.4	127.4
ALLOWABLE RELEASE (CFS)	0.7	11.0	22.1	50.9	65.7	84.1
MODELED RELEASE (CFS)	0.7	5.4	19.9	48.9	62.8	84.0
STORED VOLUME (AC–FT)	1.3	1.3	1.5	1.8	2.1	2.5

FSD–E2						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	30.6	45.2	65.9	93.3	118.0	143.9
ALLOWABLE RELEASE (CFS)	0.6	9.5	19.2	45.5	59.8	77.6
MODELED RELEASE (CFS)	0.6	3.2	18.5	41.3	58.5	74.7
STORED VOLUME (AC–FT)	2.1	2.3	2.4	2.8	3.3	3.8

FSD–E3						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	100.4	130.6	169.6	217.4	257.8	298.4
ALLOWABLE RELEASE (CFS)	0.9	13.2	26.5	61.6	79.8	102.6
MODELED RELEASE (CFS)	1.0	6.8	25.7	56.0	79.8	101.3
STORED VOLUME (AC–FT)	7.0	7.2	7.7	8.9	10.1	11.4

FSD–E4						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	58.9	75.5	96.6	122.2	143.7	165.2
ALLOWABLE RELEASE (CFS)	0.3	4.4	8.8	23.0	32.2	43.7
MODELED RELEASE (CFS)	0.9	2.8	8.7	21.9	32.2	43.6
STORED VOLUME (AC–FT)	4.2	4.3	4.7	5.4	6.2	6.9

FSD–E5						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	38.6	48.4	60.7	75.4	87.7	99.9
ALLOWABLE RELEASE (CFS)	0.0	0.2	0.4	4.2	8.7	14.3
MODELED RELEASE (CFS)	0.0	0.2	0.5	2.2	5.1	10.0
STORED VOLUME (AC–FT)	3.0	3.7	4.4	4.8	5.0	5.3

FSD–E6						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	141.6	189.4	252.5	331.4	398.9	467.5
ALLOWABLE RELEASE (CFS)	0.2	1.9	3.2	37.4	77.3	125.6
MODELED RELEASE (CFS)	0.2	0.9	3.2	18.3	64.1	123.3
STORED VOLUME (AC–FT)	13.0	17.0	21.9	22.2	22.6	23.7

PND–E7						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	46.5	75.4	121.2	285.2	402.4	548.0
MODELED RELEASE (CFS)	23.1	35.3	71.5	108.3	152.1	196.4
STORED VOLUME (AC–FT)	1.0	1.8	4.6	10.5	17.9	28.0

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2018 STERLING RANCH MDDP

DEVELOPED HYDROLOGIC CONDITIONS MAP

PROJECT NO. 09–002 | FILE: \dwg\Eng Exhibits\2018–MDDP–PROPCOND.dwg

DESIGNED BY: JD
DRAWN BY: JD
CHECKED BY: VAS

SCALE
HORIZ: 1"=2400'
VERT: 1"=2400'

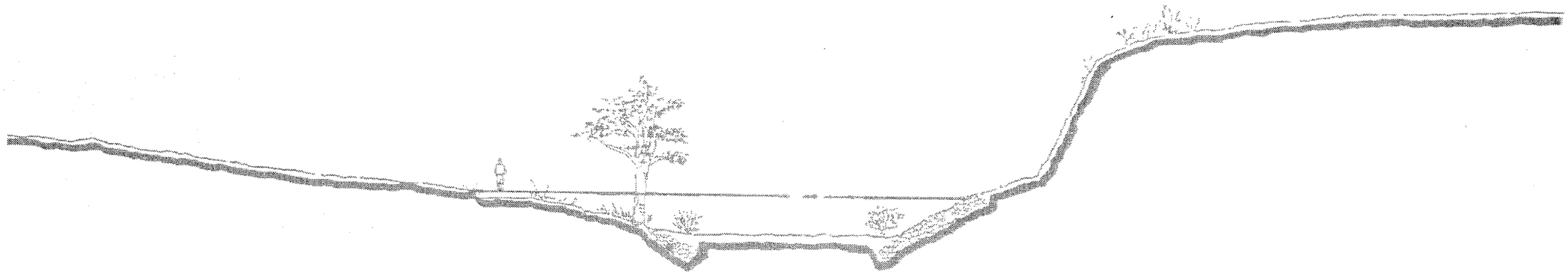
DATE: 10–21–2018

DM2

SAND CREEK DRAINAGE BASIN PLANNING STUDY

PRELIMINARY DESIGN REPORT

CITY OF COLORADO SPRINGS, EL PASO COUNTY, COLORADO



PREPARED FOR:

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PREPARED BY:

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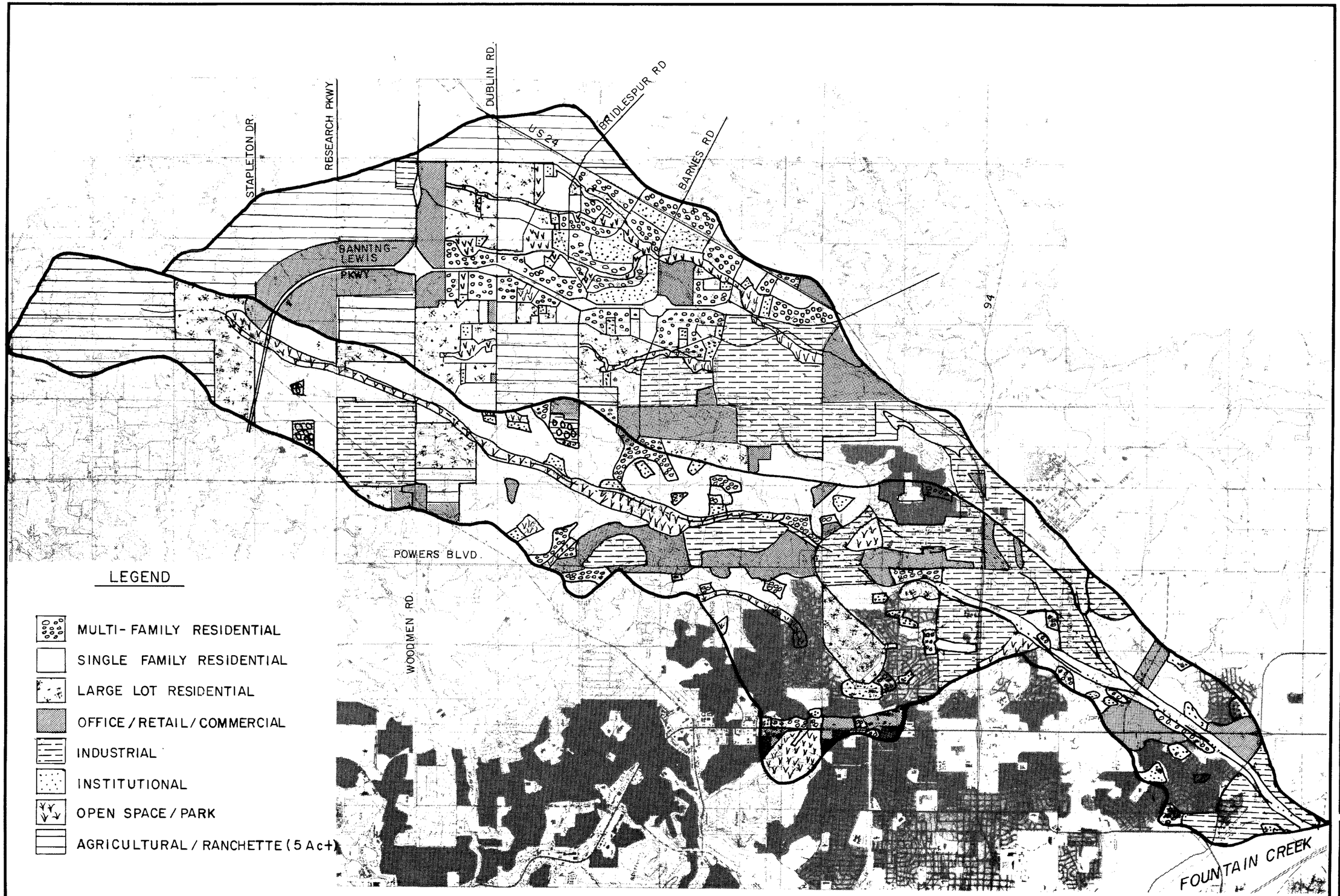

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80905-1308

SAND CREEK DRAINAGE BASIN PLANNING STUDY

PROPOSED LAND USE

Project No.	90-04-09
Date:	9/90
Design:	
Drawn:	EAK
Check:	
Revisions:	

7

FIG. II-3

Table III-1. Percent Impervious Values.

Land Use Classification	Percent Impervious	Land Use Density
Multi-Family Residential	65-80	10-24 DU/AC
Single-Family Residential	45-65	6-10 DU/AC
Low Density Residential	30-45	1-6 DU/AC
Large Lot Residential/ Agricultural	5-20	1 DU/AC
Office/Commercial	80-90	
Industrial	85-95	
Institutional	50-75	
Dedicated Open Space/Park	5-10	
Rangeland - Poor to Good Condition	5- 20	

NOTE: The above data was used in the preparation of the hydrologic analysis for the Sand Creek Drainage Basin Planning Study. These data are not intended to reflect future land use planning within the City or the County.

Table III-2: Summary of Peak Discharges
24-hour Duration Storm, AMC-II
Baseline Hydrologic Conditions

Design Point	Location	Area s.m.	100-year (cfs) Existing	100-year (cfs) Future	10-year (cfs) Existing	10-year (cfs) Future
	SAND CREEK (1)					
1	@ Fountain Creek	54.1	16900	25800	7470	11800
12	Hancock Blvd.	53.1	16100	25000	7250	11600
19	Fountain Blvd.	50.7	13600	22100	6230	10800
27	West Fork Sand Creek	23.0	11300	18900	5920	8790
99	C.R.I. & P. RR	16.0	5820	14530	2360	7400
20	North Carefree	13.5	4030	10260	1520	4810
37	Stetson Hills Blvd.	10.0	3230	6690	840	3060
60	Woodmen Road	5.4	2630	3300	760	950
75	Black Forest Road	1.4	1000	1030	320	350
	WEST FORK SAND CREEK					
27	@ Sand Creek	5.0	6840	6840	3200	3200
52	U. S. 24	4.8	6860	6860	3230	3230
59	Constitution Ave.	2.1	3450	3450	1680	1680
69	South Carefree	1.0	1630	1630	810	810
	CENTER TRIBUTARY SAND CREEK					
42	Airport Road	1.6	1530	2010	650	1200
43	Powers Blvd.	1.3	1300	1710	590	980
44	U. S. 24	1.1	1200	1680	580	960
45	Galley Road	0.8	1180	1340	530	650
	EAST FORK SAND CREEK					
1	@ Center Tributary	24.3	3970	15600	700	6530
9	@ East Fork Sub. Tributary	19.8	3730	13990	650	6050
29	@ W. Bierstadt Creek	10.6	2080	7460	400	3330
40	@ Tamlin Road	4.6	950	3570	210	1820
52	@ Woodmen Road	1.7	460	2120	80	1210
	EAST FORK SUB-TRIBUTARY SAND CREEK					
11	@ Constitution Avenue	5.9	1330	4100	240	1630
15	@ Chicago & Rock Island RR	5.2	1250	3540	230	1370
26	@ Confluence w/Toy Ranch	1.0	220	820	50	370
47	@ Proposed Dublin Blvd.	0.4	100	300	20	140
	WEST BIERSTADT CREEK					
31	@ Confluence w/ East Fork	1.8	480	1590	80	600
39	@ Tamlin Road	0.8	270	680	50	290
54	@ Woodmen Road	0.5	230	420	55	150
	EAST BIERSTADT CREEK					
32	@ Conf. w/W Bierstadt	2.4	520	1520	90	580
38	@ Chicago & Rock Island RR	0.4	120	350	15	130

(1) Future baseline condition discharges for Sand Creek compiled with the assumption that the discharges from the East Fork Sand Creek basin are maintained at existing rates as shown on this Table.

EAST BIERSTADT CREEK

WEST BIERSTADT CREEK

EAST FORK SAND CREEK

WEST TRIB TO RANCHES BASIN SAND CREEK

CENTER TRIBUTARY SAND CREEK

LOWER BASIN SAND CREEK

UPPER BASIN SAND CREEK

WEST FORK SAND CREEK

FOUNTAIN CREEK

WOODMEN

HILLTOP BLVD

STETSON

CONSTITUTION

VOLLMER RD

MARKSHEFFEL RD

POWERS BLVD

FOUNTAIN

GALLEY

PLATTE

HANCOCK

Kiowa Engineering Corporation
419 W. Bijou Street
Colorado Springs, Colorado
80905-1308

SAND CREEK DRAINAGE BASIN PLANNING STUDY

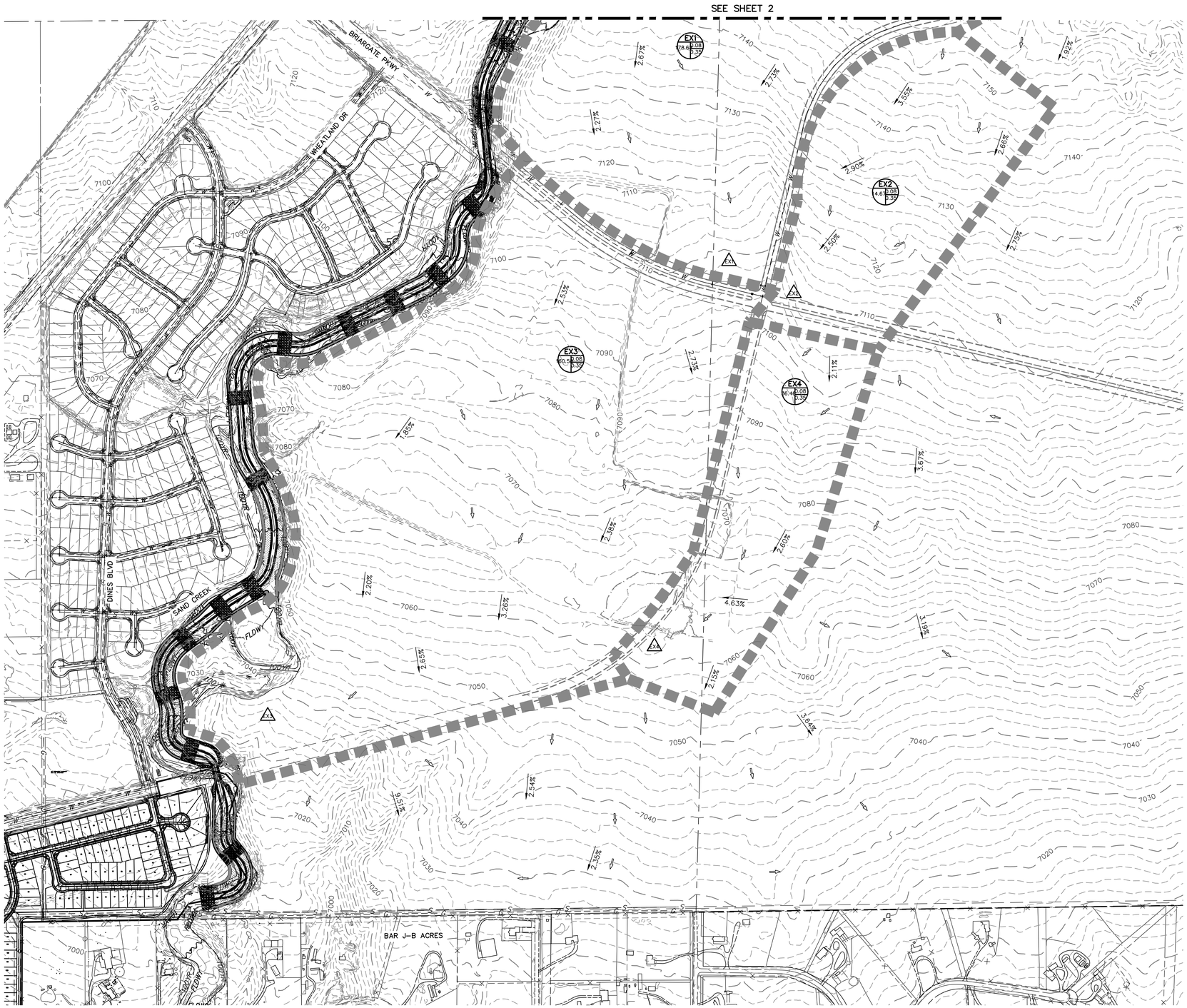
REGIONAL SUB-BASINS

Project No	90-04-09
Date:	11/90
Design:	
Drawn:	EAK
Check:	
Revisions:	

11

FIG. III-1

Appendix E
Drainage Maps



LEGEND:

- PROPOSED STORM SEWER
- 5000 FUTURE RD MAJOR CONTOUR
- FUTURE RD MINOR CONTOUR
- 5000 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- 5000 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- DRAINAGE BASIN
- A = BASIN DESIGNATION
  B = AREA IN ACRES
  C = 5-YR RUNOFF COEFFICIENT
  D = 100-YR RUNOFF COEFFICIENT
- DESIGN POINT
- HP HIGH POINT
- LP LOW POINT
- DRAINAGE ARROW
- EXISTING DRAINAGE ARROW
- PROPOSED DRAINAGE SWALE

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
EX1	178.68	2%	0.08	0.35	51.2	24.0	176.3
EX2	14.67	2%	0.08	0.35	31.9	2.8	20.6
EX3	160.58	2%	0.08	0.35	50.9	21.7	159.2
EX4	36.46	2%	0.08	0.35	39.5	6.0	44.3

N

300 150 0 300 600

ORIGINAL SCALE: 1" = 300'

Know what's **below.** **Call** before you dig.

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SUITE 201
COLORADO SPRINGS, CO 80903
JAMES F. MORLEY
(719) 471-1742

J·R ENGINEERING
A Westrian Company
Centennial 303-740-9393 • Colorado Springs 719-593-2593
Fort Collins 970-491-9888 • www.jrengineering.com

No.	REVISION	BY	DATE

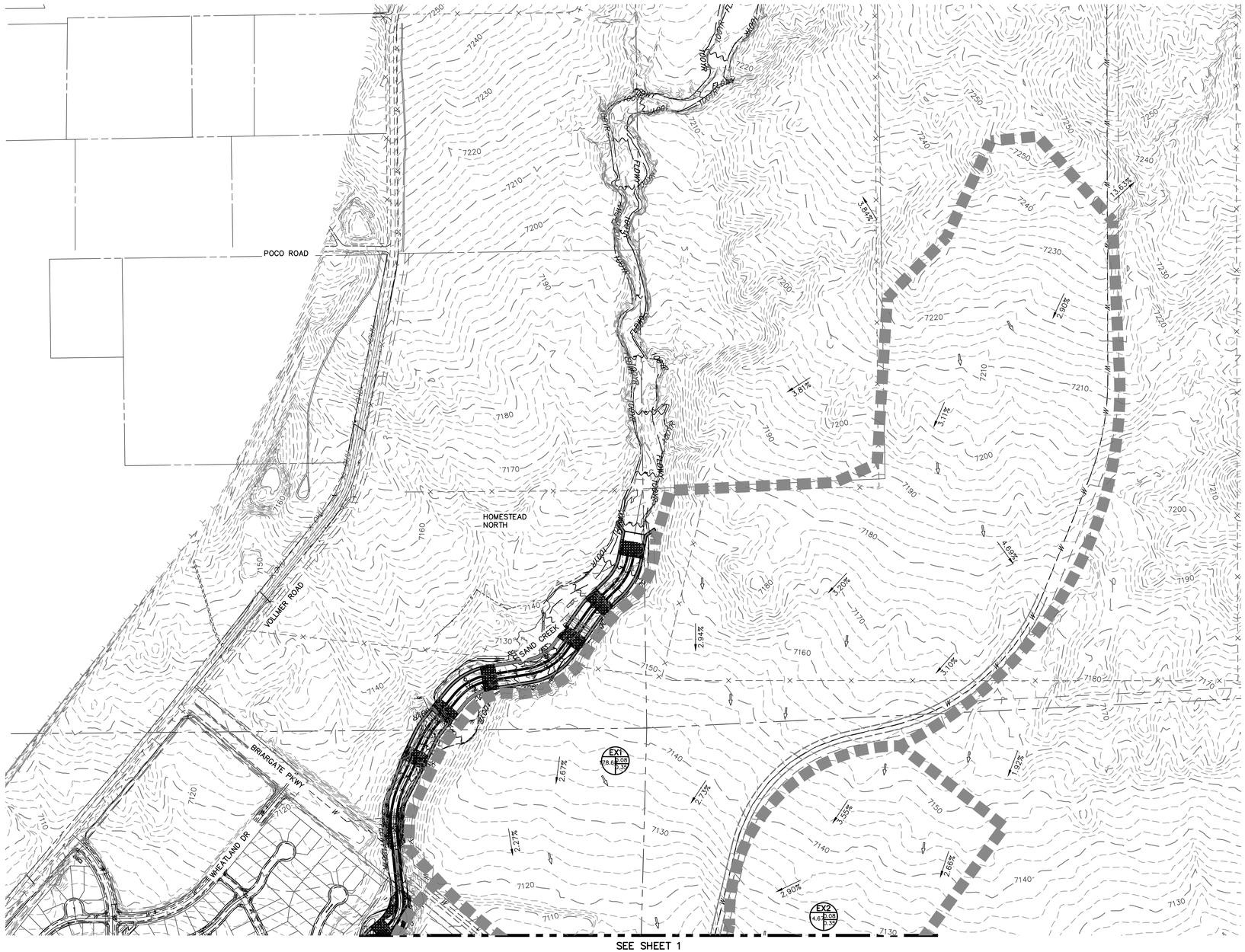
H-SCALE	1" = 300'
V-SCALE	N/A
DATE	12/31/21
DESIGNED BY	RAB
DRAWN BY	CGV
CHECKED BY	

STERLING RANCH ROAD & BRIARGATE DRAINAGE MAPS

EXISTING DRAINAGE MAP

SHEET 1 OF 6

JOB NO. 25188.03



LEGEND:

- PROPOSED STORM SEWER
- 5000 FUTURE RD MAJOR CONTOUR
- FUTURE RD MINOR CONTOUR
- 5000 PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- 5000 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- DRAINAGE BASIN
- A = BASIN DESIGNATION
  B = AREA IN ACRES
  C = 5-YR RUNOFF COEFFICIENT
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- DESIGN POINT
- HP HIGH POINT
- LP LOW POINT
- DRAINAGE ARROW
- EXISTING DRAINAGE ARROW
- PROPOSED DRAINAGE SWALE

BASIN SUMMARY TABLE

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V-SCALE	N/A
DATE	12/31/21
DESIGNED BY	RAB
DRAWN BY	CGV
CHECKED BY	

STERLING RANCH ROAD & BRIARGATE DRAINAGE MAPS

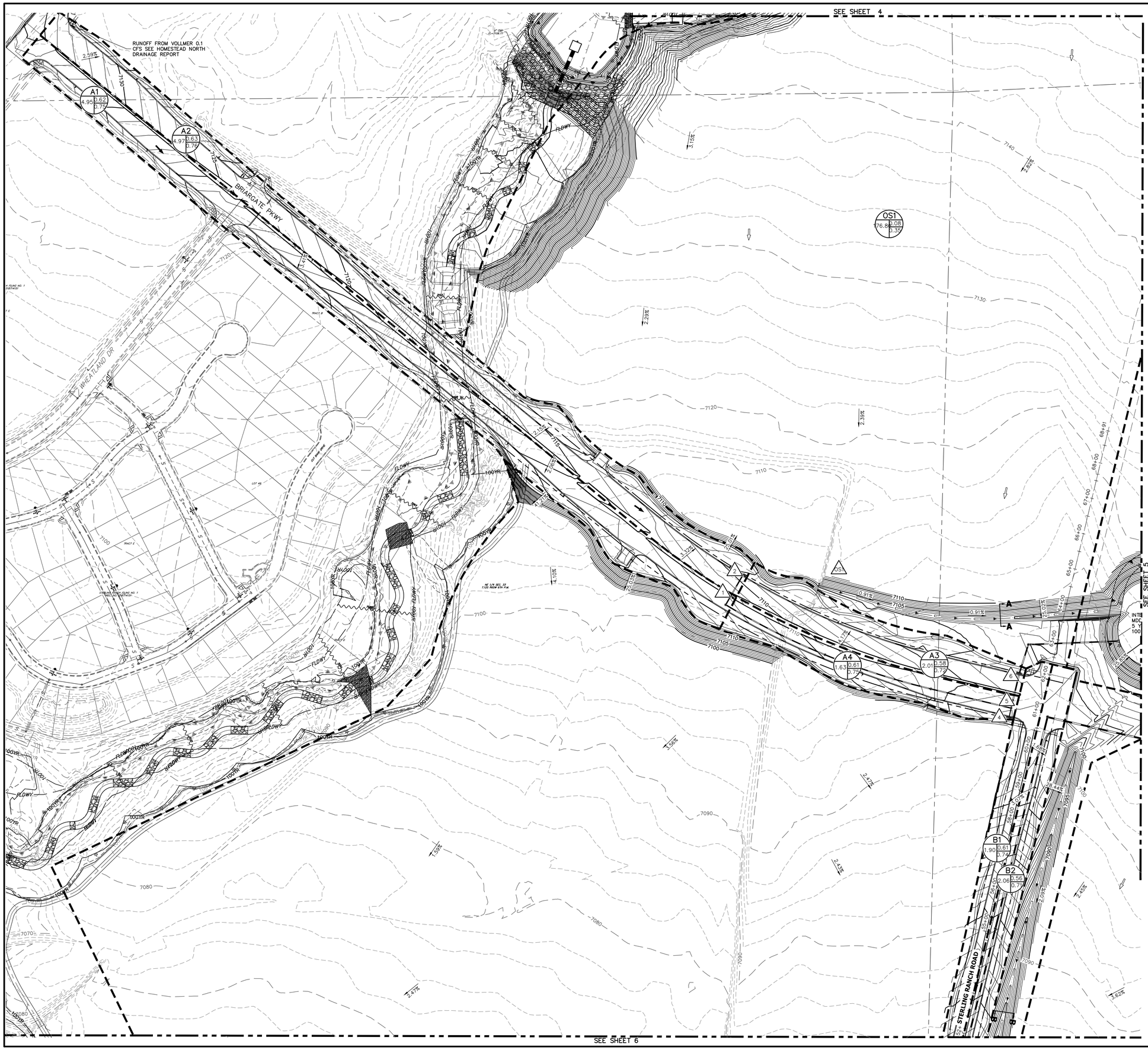
EXISTING DRAINAGE MAP

SHEET 2 OF 6

JOB NO. 25188.03

SEE SHEET 4

RUNOFF FROM VOLLMER 0.1 CFS SEE HOMESTEAD NORTH DRAINAGE REPORT



KEY MAP
SCALE: NTS

LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
C: C –100 YR
D: C –5 YR

DESIGN POINT

PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA

EXISTING STORM SEWER

STORM SEWER PROPOSED

PROPOSED R.O.W

PROPOSED PROPERTY LINES

PROPOSED SIDEWALK

EXISTING PROPERTY LINE

ROW EXISTING

FL EXISTING

SIDEWALK EXISTING

DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING 6100 PROPOSED 6100

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A1	4.95	67%	0.62	0.75	16.4	10.5	21.2
A2	4.97	68%	0.63	0.76	17.0	10.4	21.1
A3	2.01	62%	0.58	0.72	8.8	5.1	10.5
A4	1.63	66%	0.61	0.75	10.0	4.1	8.5
B1	1.90	65%	0.61	0.74	7.8	5.2	10.7
B2	2.06	60%	0.56	0.71	8.2	5.1	10.9
B3	1.27	64%	0.60	0.73	8.6	3.3	6.8
B4	1.33	61%	0.57	0.72	9.1	3.3	6.8
B5	0.89	61%	0.58	0.72	7.6	2.3	4.9
B6	0.91	63%	0.59	0.73	7.1	2.5	5.2
B7	1.08	52%	0.50	0.66	8.0	2.4	5.3
B8	1.16	58%	0.55	0.70	7.6	2.9	6.2
B9	1.98	51%	0.49	0.65	7.9	4.3	9.7
B10	2.19	53%	0.51	0.67	7.9	5.0	11.0
B11	126.23	2%	0.08	0.35	52.6	16.6	121.6
C1	5.87	2%	0.08	0.35	16.1	1.6	11.8
OS1	176.86	2%	0.08	0.35	51.2	23.8	174.5
OS2	39.27	2%	0.08	0.35	30.4	7.7	56.7

100 50 0 100 200
ORIGINAL SCALE: 1" = 100'

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No.	REVISION	BY	DATE

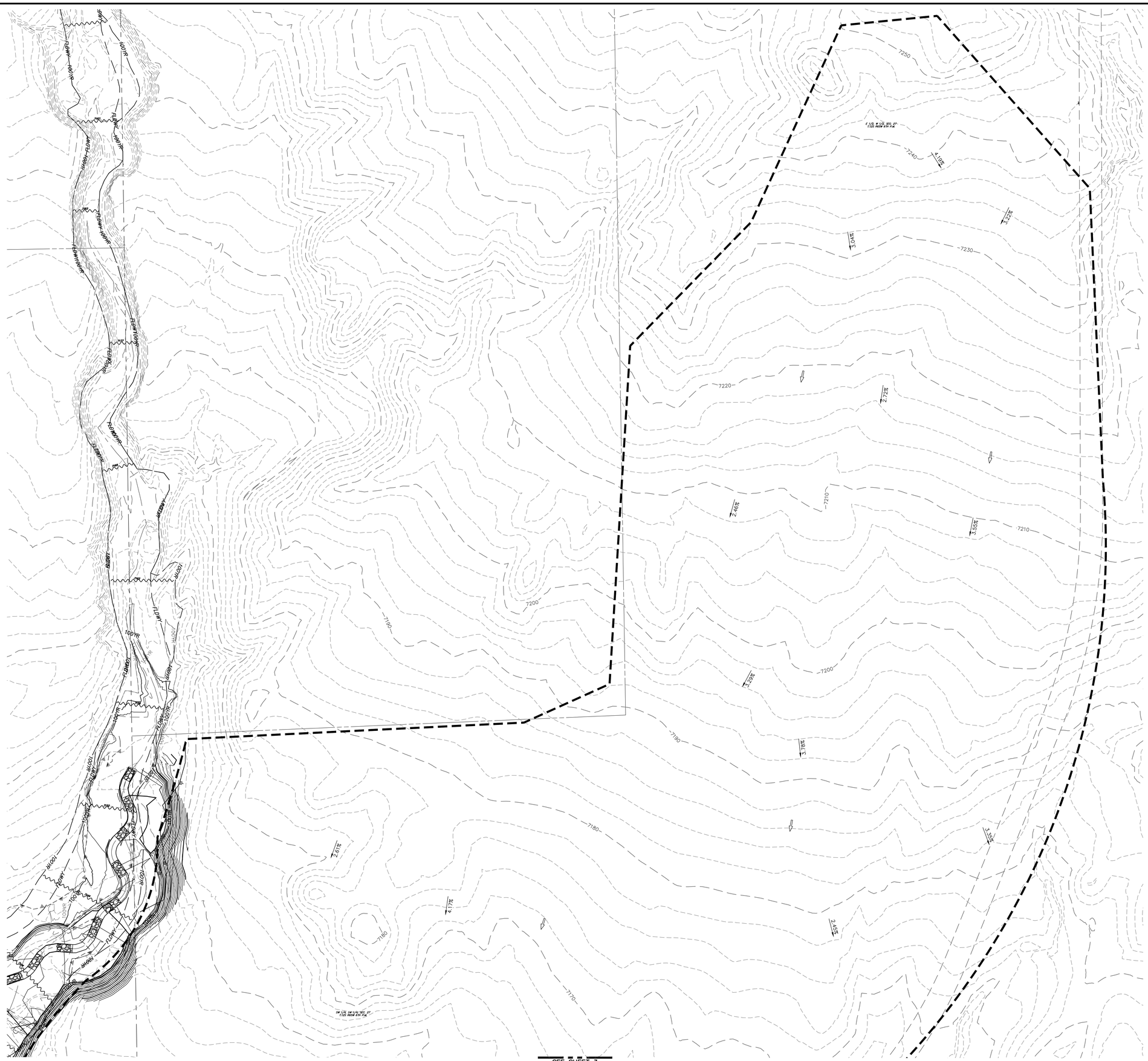
H–SCALE	1"=100'	DESIGNED BY	RAB
V–SCALE	N/A	DRAWN BY	CGV
DATE	12/31/21	CHECKED BY	

STERLING RANCH ROAD & BRIARGATE DRAINAGE MAPS

PROPOSED DRAINAGE MAP

SHEET 3 OF 6

JOB NO. 25188.03



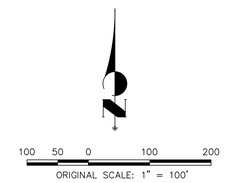
KEY MAP
SCALE: NTS

LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
C: C-100 YR
D: C-5 YR

DESIGN POINT
PROPOSED FLOW DIRECTION
BASIN DRAINAGE AREA
EXISTING STORM SEWER
STORM SEWER PROPOSED
PROPOSED R.O.W
PROPOSED PROPERTY LINES
PROPOSED SIDEWALK
EXISTING PROPERTY LINE
ROW EXISTING
FL EXISTING
SIDEWALK EXISTING
DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING 6100 PROPOSED 6100



ORIGINAL SCALE: 1" = 100'

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C_5	C_{100}	t_c (min)	Q_5 (cfs)	Q_{100} (cfs)
A1	4.95	67%	0.62	0.75	16.4	10.5	21.2
A2	4.97	68%	0.63	0.76	17.0	10.4	21.1
A3	2.01	62%	0.58	0.72	8.8	5.1	10.5
A4	1.63	66%	0.61	0.75	10.0	4.1	8.5
B1	1.90	65%	0.61	0.74	7.8	5.2	10.7
B2	2.06	60%	0.56	0.71	8.2	5.1	10.9
B3	1.27	64%	0.60	0.73	8.6	3.3	6.8
B4	1.33	61%	0.57	0.72	9.1	3.3	6.8
B5	0.89	61%	0.58	0.72	7.6	2.3	4.9
B6	0.91	63%	0.59	0.73	7.1	2.5	5.2
B7	1.08	52%	0.50	0.66	8.0	2.4	5.3
B8	1.16	58%	0.55	0.70	7.6	2.9	6.2
B9	1.98	51%	0.49	0.65	7.9	4.3	9.7
B10	2.19	53%	0.51	0.67	7.9	5.0	11.0
B11	126.23	2%	0.08	0.35	52.6	16.6	121.6
C1	5.87	2%	0.08	0.35	16.1	1.6	11.8
OS1	176.86	2%	0.08	0.35	51.2	23.8	174.5
OS2	39.27	2%	0.08	0.35	30.4	7.7	56.7

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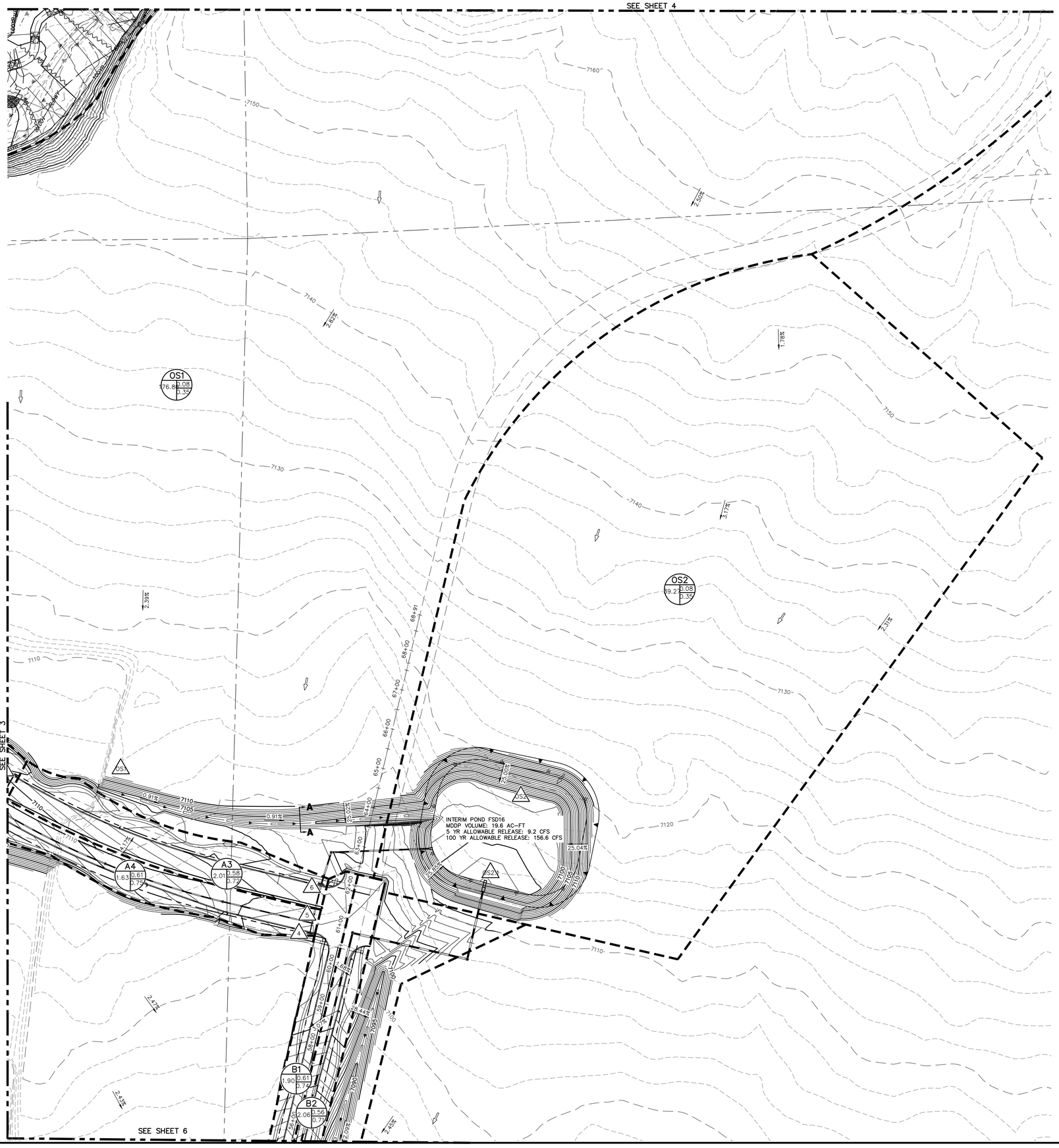
No.	REVISION	BY	DATE

H-SCALE	1"=100'
V-SCALE	N/A
DATE	12/31/21
DESIGNED BY	RAB
DRAWN BY	CGV
CHECKED BY	

STERLING RANCH ROAD & BRIARGATE DRAINAGE MAPS

PROPOSED DRAINAGE MAP

SHEET 4 OF 6
JOB NO. 25188.03

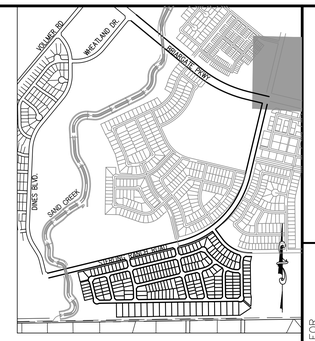


LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
C: C -100 YR
D: C-5 YR

DESIGN POINT
PROPOSED FLOW DIRECTION
BASIN DRAINAGE AREA
EXISTING STORM SEWER
STORM SEWER PROPOSED
PROPOSED R.O.W
PROPOSED PROPERTY LINES
PROPOSED SIDEWALK
EXISTING PROPERTY LINE
ROW EXISTING
FL EXISTING
SIDEWALK EXISTING
DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING 6100 *PROPOSED* 6100



KEY MAP
SCALE: NTS

BASIN SUMMARY TABLE							
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B10	2.19	53%	0.51	0.67	7.9	5.0	11.0
B11	126.23	2%	0.08	0.35	52.6	16.6	121.6
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OS1	176.86	2%	0.08	0.35	51.2	23.8	174.5
OS2	39.27	2%	0.08	0.35	30.4	7.7	56.7

100 50 0 100 200
ORIGINAL SCALE: 1" = 100'

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H-SCALE	1"=100'	DATE	12/31/21
V-SCALE	N/A	DESIGNED BY	RAB
		DRAWN BY	CGV
		CHECKED BY	

STERLING RANCH ROAD & BRIARGATE
DRAINAGE MAPS

PROPOSED DRAINAGE MAP

SHEET 5 OF 6

JOB NO. 25188.03

BASIN SUMMARY TABLE

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LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
D: C –100 YR
D: C –5 YR

DESIGN POINT

PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA

EXISTING STORM SEWER

STORM SEWER PROPOSED

PROPOSED R.O.W

PROPOSED PROPERTY LINES

PROPOSED SIDEWALK

EXISTING PROPERTY LINE

ROW EXISTING

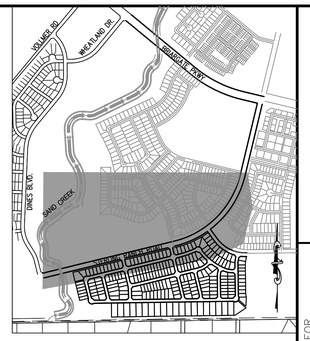
FL EXISTING

SIDEWALK EXISTING

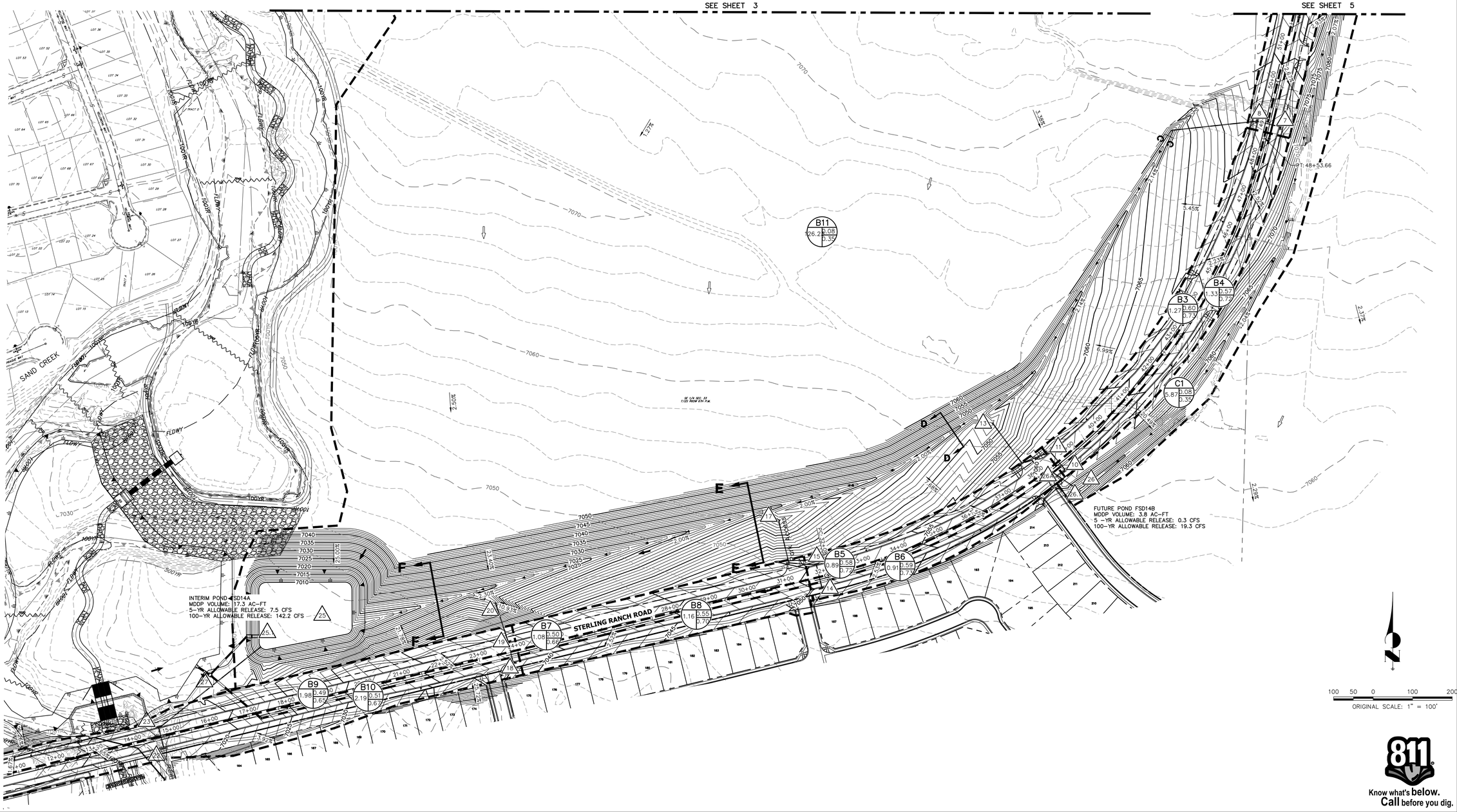
DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING 6100

PROPOSED 6100



KEY MAP
SCALE: NTS



100 50 0 100 200
ORIGINAL SCALE: 1" = 100'

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DRAWN BY	CGV
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STERLING RANCH ROAD & BRIARGATE DRAINAGE MAPS

PROPOSED DRAINAGE MAP

SHEET 6 OF 6

JOB NO. 25188.03