EPC STORMWATER REVIEW COMMENTS ARE SHOWN IN ORANGE BOXES WITH BLACK TEXT

Item Numbers refer to SWMP Checklist

El Paso County
STORMWATER MANAGEMENT PLAN

Lot 7, Block 1 Meadow Lake Airport Filing No. 2

Part of Sec. 4, T. 13 S., R. 64 W., 6th P.M.

8140 and 8150 Cessna Drive, Peyton

PCD Project Number: PPR-21-038

July 8, 2021

prepared for
Ryan Schneider

Qualified Stormwater Manager
Name: Erik S. Watts
Company: Oliver E. Watts Consulting Engineer Inc
Address: 614 Elkton Drive Colorado Springs, CO 80907

Contractor
Name: _____
Company: _____
Address: _____

Oliver E. Watts, Consulting Engineer, Inc.
Colorado Springs, Colorado

OLIVER E. WATTS, PE-LS
OLIVER E. WATTS, CONSULTING ENGINEER, INC.
CIVIL ENGINEERING AND SURVEYING
614 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907
(719) 593-0173
fax (719) 265-9660
olliewatts@aol.com
Celebrating over 42 years in business

July 8, 2021

El Paso County D.O.T.
3275 Akers Drive
Colorado Springs, CO 80922

ATTN: Permits Unit

SUBJECT: Stormwater Management Plan
Lot 7, Block 1, Meadow Lake Airport Filing No 2

Transmitted herewith for your review and approval is the SWMP for Lot 7, Block 1, Meadow Lake Airport Filing No 2.

Please contact our office if we may provide any further information.

Oliver E. Watts, Consulting Engineer, Inc.

BY: _____
Erik S. Watts, Authorized Representative
Erosion Control Supervisor

The developer / owner has read and will comply with all of the requirements specified in this stormwater management report.

By: _____
Ryan Schneider
2610 Fairmount Street
Colorado Springs, CO 80909

Table of Contents

1. SITE DESCRIPTION:

Lot 7, Block 1 Meadow Lake Airport Filing No. 2 is located at 8140 and 8150 Cessna Drive, , just east of the runway in Meadow Lake Airport in Section 4, Township 13 South, Range 64 West of the 6th P.M., in El Paso County. The overall Site totals 3.262 acres. Grading is to occur on 2.6 acres of the lot.

Lat: 38° 56' 57" N **Long:** 104° 34' 12" W

a) Construction activity description: Construction activity for the site will include; overlot grading, and construction of hanger and taxi way / driveway and parking lot. The site will be reseeded once all construction has been completed.

b) Sequence / time line of activities: The site will be overlot graded per the enclosed grading plan. All site grading is to be in compliance with El Paso County Code. Grading for the site is scheduled to be completed by spring 2022. Total site landscaping / reseeding should be completed and acceptable ground cover / vegetation established by late September 2022.

c) Site area: The site is 3.262 acres total. It is part of the larger, 210 acre Meadow Lake Airport. The portion of the site that is to experience grading is approximately 2.6 acres. The Site is vegetated with grasses, and some scrub brush, where not covered by taxi ways / drive isles: Approximately 85% of the site has some form of vegetation on it. Said vegetation, is outside of taxi ways / drive isles. The site is to be graded so as to comply with the Grading and Erosion Control Plans, which accompany the submittal.

d) Runoff: Overall runoff from the Site will remain at historic levels. Attached is the "Description of Runoff" section from the lots drainage letter:

EXISTING DRAINAGE CONDITIONS

The site is located as shown on the enclosed drainage plans in the Meadow Lake Airport of El Paso County. The natural basin consists of basins A through C on the enclosed existing conditions map that discharges 0.57 cfs (5-year runoff) / 4.12 cfs (100-year runoff) historically, as shown on the existing conditions drainage plan.

PROPOSED DRAINAGE CONDITIONS

The area will be graded to conform to the existing topography shown on the drainage plan, routing all runoff into a lot area at the southeast portion of the construction site. No clearing will be necessary within the construction site.

Runoff will be routed southeasterly along airport taxiways to a northerly taxiway where storm sewers will pick up the runoff from respective basins and direct it to the proposed full spectrum detention pond located in the southeast corner of the lot. The total runoff into the pond will be 2.9 cfs / 7.7 cfs. As shown on the enclosed computation sheet the 100 year storage required is 0.248 acre feet, which occurs at a depth of 4.13'.

The outfall storm sewer will be routed to the existing drainage channel along side of the main entry taxiway as shown on the enclosed drainage plan. 196.74 LF of 12" PVC is proposed for an outfall storm sewer.

FOUR STEP PROCESS
The following process has been followed to minimize adverse impacts of urbanization

Step1 Employ Runoff Reduction Practices *– The extent of impervious materials is minimized consistent with the objectives of the facility. No curb and gutter or other items that might concentrate runoff are proposed.*

Step 2 Stabilize Drainageways *–The development of this project does not create Drainageways and is not anticipated to have any negative effects on downstream Drainageways. Grass swales along the north side of the building are minimized and slopes are minimized, and they will outfall onto the proposed parking lot. Runoff across the asphalt pavement will not be concentrated along the south limit.*

Step 3 Provide Water Quality Capture Volume *– The limit of disturbance for the proposed construction is less than one acre, and no water quality provisions are required or necessary.*

Step 4 Consider Need for Industrial and Commercial BMP's *– This submittal provides a final grading and erosion control plans with BMP's in place. The proposed project will use silt fence, a vehicle tracking control pad, and concrete washout area, reseeding and landscaping to mitigate the potential for erosion across the site. The proposed BMP's are considered fully adequate.*

This parcel is not within the limits of a designated flood plain or flood hazard area, as identified on FEMA Panel No. 08041C0554 G, dated December 7, 2018, a copy of which is enclosed for reference.

The method used for all computations is that specified in the City-County Drainage Criteria Manual, using the rational method for areas of the size of the site and the SCS method for the review of the major basin involved. All computations are enclosed for reference and review.

The local USDA/SCS office has mapped the soils in the area. A soils map interpretation sheet is enclosed for reference. All soils in this area are of hydrologic group "A", Blakeland Series. It has slow surface water runoff, but high eroadability and moderate blowing (wind) hazard, and is listed as having high potential for successful reseeding, especially with 'native' grasses. Potential erosion impacts would affect taxi ways / drive isles. Runoff would be carried down the slopes, southward and into the taxi ways / drive isles. Erosion control measures; silt fencing, and reseeding will serve to mitigate this hazard. See page 2, Erosion Control Plan for details.

e) Existing vegetation: As stated previously; Item 1, C "Site Area," vegetation consists of grasses, and some scrub brush. Approximately 85% of the site has some form of vegetation on it. This was determined, per visual inspection at the time of the site topographic survey dated 8-14-20. Per the enclosed Grading and Erosion Control Plans: The area is to be graded as shown and erosion control measures, as shown, and listed in said Plans implemented.

f) Potential pollution sources:
Potential pollution sources which shall be evaluated for potential to contribute to

stormwater discharge for the subject site may include the following: disturbed and stored soils, vehicle tracking of sediments, management of contaminated soils, loading and unloading operations, outdoor storage of materials (building material, chemicals, etc.), vehicle and equipment maintenance and fueling, significant dust or particulate generating processes, routine maintenance activities involving fertilizers, pesticides, detergents, fuels, solvents, oils. etc., on-site waste management practices (waste piles, liquid wastes, dumpsters), concrete truck / equipment washing, including the truck chute and associated fixtures, non-industrial waste sources (worker trash and portable toilet) and other areas or procedures where potential spills can occur. The locations of these areas that affect the site are shown on the enclosed plans.

TABLE 1: POTENTIAL POLLUTION SOURCES

Potential Pollution Sources	**Possible Site Contributions of Pollutants to Stormwater Discharges**
All disturbed and stored soils	Stockpiles of fill from the excavations, topsoil stockpiles. Imported borrow stockpile.
Vehicle tracking of sediments	See the enclosed drawings for vehicle entrance and exit.
Management of contaminated soils	No contaminated soils are expected to be encountered.
Loading and unloading operations	Loading and unloading of building materials, etc.
Outdoor storage activities (building material, fertilizers, chemicals, etc.)	Building materials and equipment storage areas (no fertilizers, petroleum or chemical products will be stored on-site).
Vehicle and equipment maintenance and fueling	Fueling will occur on-site using mobile equipment (will not be stored on-site). Equipment maintenance will occur off-site.
Significant dust or particulate-generating processes	Vehicle tracking, soil removed from excavation, stockpiles.
Routine maintenance activities involving fertilizers, pesticides, detergents, fuels, solvents, oils, etc.	All equipment maintenance will occur off-site. No fertilizers, pesticides, detergents, and/or solvents will be used or stored on-site.
On-site waste management practices (waste piles, liquid wastes, dumpsters, etc.)	All waste will be removed from site as soon as possible.
Concrete truck/equipment washing, including the concrete truck chute and associated fixtures and equipment	Washout as shown or relocated by contractor.
Dedicated asphalt and concrete batch plants	No dedicated asphalt and concrete batch plants are on-site.
Non-industrial waste sources such as worker trash and portable toilets	Worker trash will be removed from the site as soon as possible. Portable toilets will be provided.
Other areas or procedures where potential spills can occur	Petroleum releases from equipment are possible.

BMP's for Pollutant Prevention:
The following are common practices to mitigate potential pollutants:

- Wind erosion shall be controlled by sprinkling site roadways and/or temporary stabilizing stockpiles. Each dump truck hauling materials to or from the site shall be required to cover its bed with a tarpaulin.

Add: Portable toilets will be secured at all four corners to prevent overturning and cleaned on a weekly basis. They will be inspected daily for spills.

Discuss inspection procedure for checking waste disposal bins for leaks and overflowing capacity. And discuss frequency that they will be emptied (or at what level of capacity would trigger the need to be emptied)

- Sanitary facilities shall be placed a minimum of 10' from any curb and 50' from any inlet. If not feasible for the site, a secondary containment shall be implemented.
- Equipment fueling and maintenances services – a designated fueling area will be established to contain any spill resulting from fueling, maintenance or repair of equipment. Contractors shall be responsible for containment, cleanup and disposal of any leak or spill and any associated costs of said cleanup / disposal.
- Chemical products shall be protected from precipitation, free from ground contact, and stored properly to prevent damage from equipment, vehicles or workers.
- Material stockpiles (soils, soil amendments, debris/trash piles) – All construction trash and debris will be deposited in the site dumpster(s).
- Sediment and mitigation of sediment – Sweeping operations will take place as necessary to maintain roadways / parking areas. The perimeter of the site will be evaluated for any potential impact resulting from trucking operations or sediment mitigation from the site. BMP devices will be placed to protect storm system inlets should any roadway / parking area tracking or sediment mitigation occur.
- Snow removal and/or stockpiling will be considered prior to placement at the site. Snow stockpiles should be kept away from any stormwater conveyance system(s) to include; inlets, ponds, outfall locations, roadway surfaces, etc.

g) Non stormwater discharge: No springs are known to exist. Any additional discharge is confined to the surface and runoff routed to the subdivision detention pond.

h) Receiving water(s), size, type and description of outfall(s): Sand Creek and ultimately Fountain Creek is the receiving water for stormwater discharge from this Site. Outfalls are shown on the enclosed grading plan. NOTE: There are no streams cross this project.

2. SITE MAP:
Enclosed are a vicinity map and grading and erosion control plans for review. Details for the BMP's are shown of the plans.

3. BMPs FOR STORMWATER POLLUTION PREVENTION:

a) Erosion and sediment controls:

1) Structural practices: As indicated on the enclosed Grading and Erosion Control Plans, erosion will be contained through the use of said silt fencing. See Plans for locations and details on silt fencing. The portion of the lot that has experienced grading will be landscaped or reseeded per County Code (see DCM Volume II for details).

2) Non-Structural practices: Permanent stabilization practices will be implemented on this Site through reseeding. Said seeding activities will occur when all grading / construction for the site is finished. See the enclosed Grading and Erosion Control Plans for details.

b) Materials handling and Spills Prevention: There are no plans to have any On-Site batch plant(s). Equipment fueling and maintenances services – a designated fueling area will be established to contain any spill resulting from fueling, maintenance or repair of equipment. Contractors shall be responsible for containment, cleanup and disposal of any leak or spill and any associated costs of said cleanup / disposal. Vehicle refueling will take place away from

areas containing or conveying water, or near the existing road, in accordance with State approved practices. Should a fuel or fluid spill occur, the contractor will County and State guidelines concerning spills such as; berming the area around the spill and remove all contaminated soil in an approved container and disposing of said containing at a County / State approved facility / Site.

Item 22. Discuss long term stormwater quality and that water will be conveyed to the existing EDB.

4. FINAL STABILIZATION AND LONG TERM STORMWATER MANAGEMENT:
As stated earlier, copies of the Grading and Erosion Control Plans are submitted for your review. These Plans should adequately address this section. Our office will have inspectors monitoring the Site during construction to insure compliance with applicable State and El Paso County Code(s). The Permittee will contact your office upon final stabilization, once the vegetation / ground cover reaches 70% of pre-disturbance levels. See re-seed section, page 7, for suggested final stabilization seed mix, for areas outside the improvements. The temporary BMP's will be removed upon receiving permission from El Paso County.

5. OTHER CONTROLS:
Please review the enclosed Grading and Erosion Control Plan. It details said controls. Waste disposal will be in accordance with El Paso County standards. A rock mat (VTC) will be installed where shown on the grading plan to remove any soil from vehicles before entering The taxi way / drive isle.

add "or snow melt event that causes surface erosion"

6. INSPECTION AND MAINTENANCE:
The Qualified Stormwater Manager will monitor the day to day Site activities during construction. A copy of this report will be kept in the vehicle of said inspector.
Inspections will occur and reports will be filled out and signed by the Qualified Stormwater Manager every 14 days, and/or after a precipitation event as required, to ensure adequate operation and design of selected BMP's. Signed copies of said inspection reports will be kept by the permit holder and at this office. Silt fencing will need to be replaced and/or repaired as need be. All litter and debris should be removed from the lot and disposed off of the site (i.e. in a trash bag, trash can, dumpster).

7. SWMP REVISION PROCEDURES:
This SWMP should be revised as necessary to address the various phases of grading, construction, and changing site conditions and BMP needs.
The need for revision could include the following: Continued overlot grading, removal of one of more BMP as items are completed, the weather and precipitation could affect and cause a needed revision in the SWMP. The Qualified Stormwater Manager will revise accordingly.

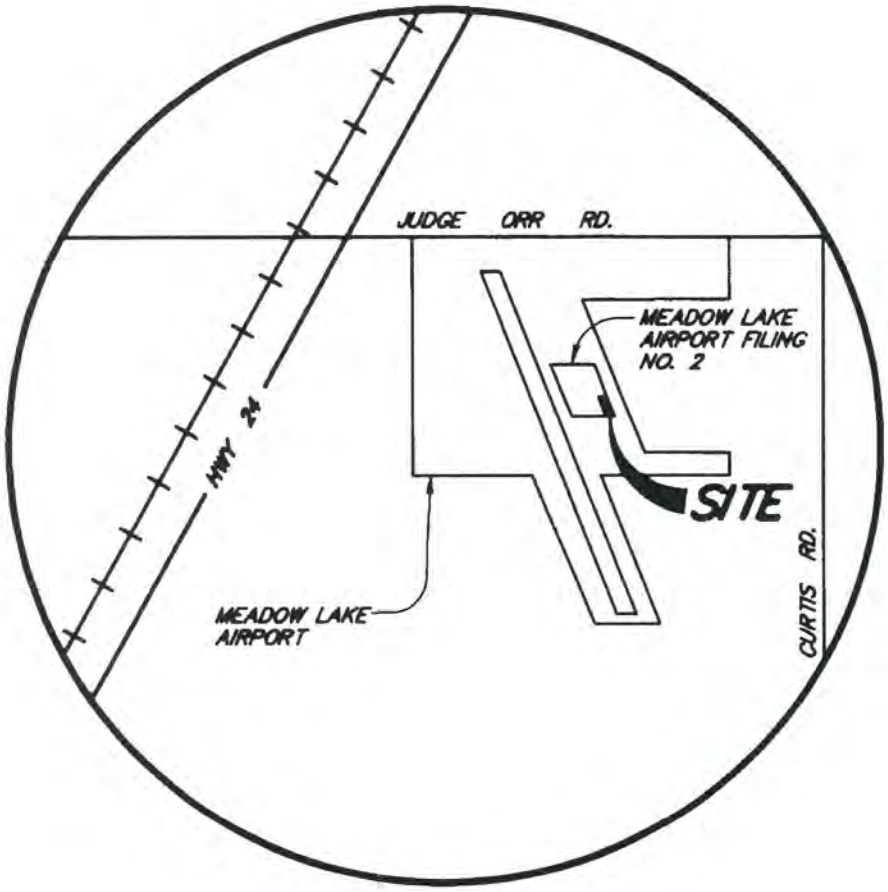
8. FINAL STABILIZATION:
Re-seed mixture
All disturbed areas shall be re-graded and, those areas not covered by improvements could be reseeded with the following native grass mixture for sandy soils:

GRASS	VARIETY	AMOUNT IN PLS LBS PER ACRE
Sideoats Grama	El Reno	3.0 Lbs
Western Wheatgrass	Barton	2.5 Lbs
Slender Wheat Grass	Native	2.0 Lbs
Little Bluestem	Pastura	2.0 Lbs

Sand Dropseed	Native	0.5 Lbs
Switch Grass	Nebraska 28	3.0 Lbs
Weeping Love Grass	Morpha	1.0 Lbs

9. EROSION CONTROL MEASURES OWNER / OPERATED BY ANOTHER ENTITY:

This project does NOT rely on control measures owned or operated by another entity.



NORTH

VICINITY MAP

N.T.S.

SCALE

MAJOR BASIN	SUB BASIN	AREA		BASIN		T_c MIN	I in./hr.		SOIL GRP	DEV. TYPE	C		FLOW 5-ry qp -CFS-	FLOW 100-yr qp -CFS-	RETURN PERIOD -years-	
		PLANIM READ	ACRES	LENGTH -FT.-	HEIGHT -FT.-											
HISTORIC	A-C	COGO	2.943	300	6	25			A	R/L	0.08	0.35			5	100
			V=0.7	+220	4	+5										
						30	2.4	4.0					0.57	4.12		

HYDROLOGICAL COMPUTATION – BASIC DATA
PROJ: 8140 CESSNA BY: O.E. WATTS
RATIONAL METHOD DATE: 7/9/21

OLIVER E. WATTS, CONSULTING ENGINEER, INC.
614 ELKTON DRIVE COLORADO SPRINGS, CO 80907

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Y

STREET AND STORM SEWER CALCULATIONS

STREET	LOCATION	DISTANCE -ft.-	ELEVATION & SLOPE	TOTAL RUNOFF -cfs- 5-yr./100-yr	STREET FLOW / CAPACITY -cfs- 5-yr./100-yr	PIPE FLOW -cfs-	TYPE PIPE, CATCH BASIN & SLOPE %
PRIVATE	C		31.61/30.11	1.3/3.5		3.5	FIREBAUGH GRATE
		130.37	0.67%			3.5	12” PVC S=0.67% V=4.45
	B		30.70/29.20				FIREBAUGH GRATE
		31.24		2.2/5.9		5.9	12” PVC V=7.5
	C		24				OUTLET STRUCTURE
		228.16	0.87%				12” PVC
	DITCH		22				

STREET AND STORM SEWER CALCULATIONS
PROJECT: 8140 CESSNA BY: O.E. WATTS
DATE: 7/9/21

OLIVER E. WATTS, CONSULTING ENGINEER, INC.
614 ELKTON DRIVE COLORADO SPRINGS, CO 80907

MAJOR BASIN	SUB BASIN	AREA		BASIN		T_c MIN	I in./hr.		SOIL GRP	DEV. TYPE	C		FLOW 5-ry qp -CFS-	FLOW 100-yr qp -CFS-	RETURN PERIOD -years-	
		PLANIM READ	ACRES	LENGTH -FT.-	HEIGHT -FT.-											
DEVELOPED	A	COGO	0.196	300	5	12			A	ROOF	0.73	0.81			5	100
		V=1.1	0.062	+150	2.5	+2				A.C.	0.90	0.96				
			0.434							R/L	0.08	0.36				
						14	3.4	2.8		MIX	0.338	0.535	0.8	1.8		
	B	COGO	0.124	300	6	6.0			A	FOOF	0.73	0.81				
		V=2.0	0.198	+1580	1.8	+1.5				A.C.	0.90	0.96				
			0.568							R/L	0.08	0.35				
			0.890			7.5	4.6	7.8		MI	0.352	0.550	1.4	3.8		
	C	COGO	0.150	300	5	20.7			A	ROOF	0.73	0.81				
		V=2.0	0.279	+165	+1.65	+0.1				A.C.	0.90	0.96				
			0.932			20.8				R/L	0.08	0.365				
			1.361				3.0	4.9		MI	0.320	0.523	1.3	3.4		
	+B		0.890	+136.37	V=4.05	+0.5										
	TOTAL		2.251			21.3	2.9	4.9		MI	0.333	0.536	2.2	5.9		
	+A		0.692	+31.24	V=7.64	+0.1					0.338	0.535				
	TOTAL		2.943			24.4	2.9	4.9		MI	0.334	0.536	2.9	7.7		

HYDROLOGICAL COMPUTATION – BASIC DATA
PROJ: 8140 CESSNA BY: O.E. WATTS
RATIONAL METHOD DATE: 7/9/21

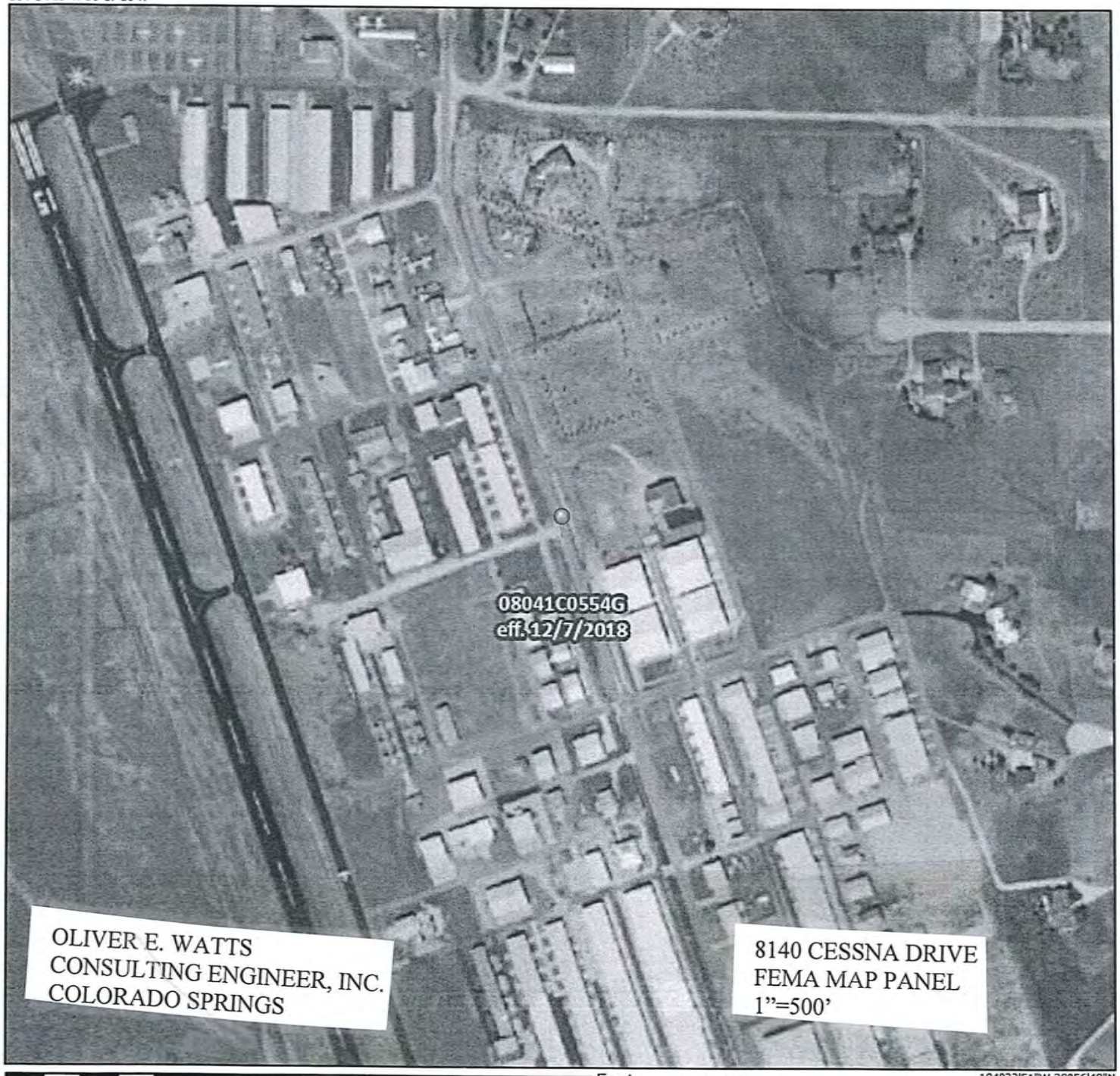
OLIVER E. WATTS, CONSULTING ENGINEER, INC.
614 ELKTON DRIVE COLORADO SPRINGS, CO 80907

PAGE **2**
OF
Y

National Flood Hazard Layer FIRMette

FEMA

104°34'29"W 38°57'16"N



0 250 500 1,000 1,500 2,000 Feet

1:6,000

104°33'51"W 38°56'48"N

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) Zone A, V, A99
- With BFE or Depth Zone AE, AO, AH, VE, AR
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Area of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
- Future Conditions 1% Annual Chance Flood Hazard Zone X
- Area with Reduced Flood Risk due to Levee. See Notes. Zone X
- Area with Flood Risk due to Levee Zone D

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard Zone X
- Effective LOMRs
- Area of Undetermined Flood Hazard Zone

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 Cross Sections with 1% Annual Chance
- 17.5 Water Surface Elevation
- Coastal Transect
- Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

The pin displayed on the map is an approximate point selected by the user and does not represe an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 7/9/2021 at 12:31 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

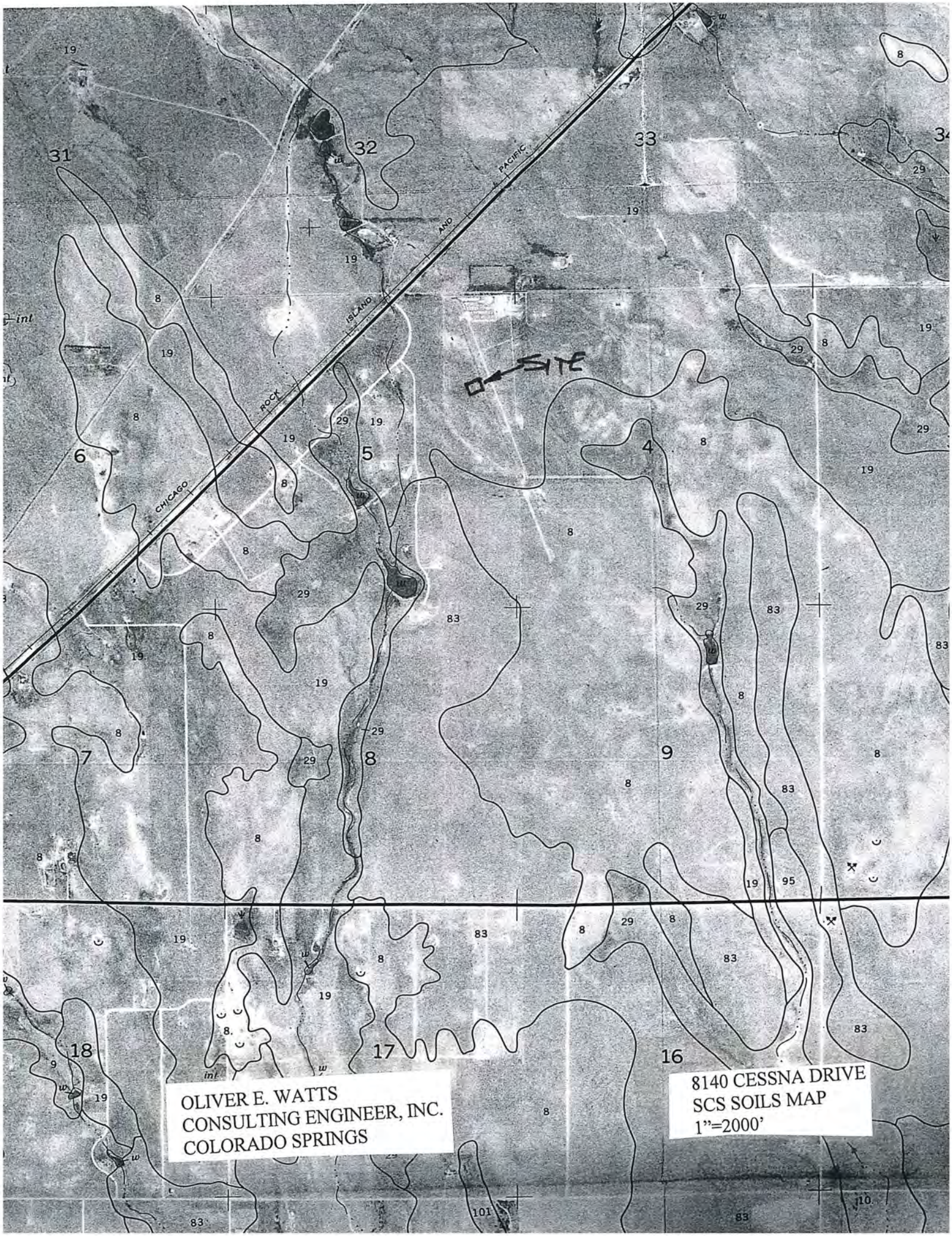

SITE
CHICAGO ROCK ISLAND AND PACIFIC
OLIVER E. WATTS
CONSULTING ENGINEER, INC.
COLORADO SPRINGS
8140 CESSNA DRIVE
SCS SOILS MAP
1"=2000'

TABLE 16.--SOIL AND WATER FEATURES

[Absence of an entry indicates the feature is not a concern. See "flooding" in Glossary for definition of terms as "rare," "brief," and "very brief." The symbol > means greater than]

Soil name and map symbol	Hydrologic group	Flooding			Bedrock		Potential frost action
		Frequency	Duration	Months	Depth	Hardness	
					In		
Alamosa: 1	C	Frequent	Brief	May-Jun	>60	---	High.
Ascalon: 2, 3	B	None	---	---	>60	---	Moderate.
Badland: 4	D	---	---	---	---	---	---
Bijou: 5, 6, 7	B	None	---	---	>60	---	Low.
Blakeland: 8	A	None	---	---	>60	---	Low.
[1]9: Blakeland part	A	None	---	---	>60	---	Low.
Fluvaquentic Haplaquolls part	D	Common	Very brief	Mar-Aug	>60	---	High.
Blendon: 10	B	None	---	---	>60	---	Moderate.
Bresser: 11, 12, 13	B	None	---	---	>60	---	Low.
Brussett: 14, 15	B	None	---	---	>60	---	Moderate.
Chaseville: 16, 17	A	None	---	---	>60	---	Low.
[1]18: Chaseville part	A	None	---	---	>60	---	Low.
Midway part	D	None	---	---	10-20	Rippable	Moderate.
Columbine: 19	A	None to rare	---	---	>60	---	Low.
Connerton: [1]20: Connerton part	B	None	---	---	>60	---	High.
Rock outcrop part	D	---	---	---	---	---	---
Cruckton: 21	B	None	---	---	>60	---	Moderate.
Cushman: 22, 23	C	None	---	---	20-40	Rippable	Moderate.
[1]24: Cushman part	C	None	---	---	20-40	Rippable	Moderate.
Kutch part	C	None	---	---	20-40	Rippable	Moderate.
Elbeth: 25, 26	B	None	---	---	>60	---	Moderate.
[1]27: Elbeth part	B	None	---	---	>60	---	Moderate.

See footnote at end of table.

Table 6-6. Runoff Coefficients for Rational Method
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis--Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

3.2 Time of Concentration

One of the basic assumptions underlying the Rational Method is that runoff is a function of the average rainfall rate during the time required for water to flow from the hydraulically most remote part of the drainage area under consideration to the design point. However, in practice, the time of concentration can be an empirical value that results in reasonable and acceptable peak flow calculations.

For urban areas, the time of concentration (t_c) consists of an initial time or overland flow time (t_i) plus the travel time (t_t) in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For non-urban areas, the time of concentration consists of an overland flow time (t_i) plus the time of travel in a concentrated form, such as a swale or drainageway. The travel portion (t_t) of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or drainageway. Initial time, on the other hand, will vary with surface slope, depression storage, surface cover, antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. The time of concentration is represented by Equation 6-7 for both urban and non-urban areas.

$$t_c = t_i + t_t \quad \text{(Eq. 6-7)}$$

Where:

t_c = time of concentration (min)

t_i = overland (initial) flow time (min)

t_t = travel time in the ditch, channel, gutter, storm sewer, etc. (min)

3.2.1 Overland (Initial) Flow Time

The overland flow time, t_i, may be calculated using Equation 6-8.

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S^{0.33}} \quad \text{(Eq. 6-8)}$$

Where:

t_i = overland (initial) flow time (min)
C_5 = runoff coefficient for 5-year frequency (see Table 6-6)
L = length of overland flow (300 ft maximum for non-urban land uses, 100 ft maximum for urban land uses)
S = average basin slope (ft/ft)

Note that in some urban watersheds, the overland flow time may be very small because flows quickly concentrate and channelize.

3.2.2 Travel Time

For catchments with overland and channelized flow, the time of concentration needs to be considered in combination with the travel time, t_t, which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, the overland travel time, t_t, can be estimated with the help of Figure 6-25 or Equation 6-9 (Guo 1999).

$$V = C_v S_w^{0.5} \quad \text{(Eq. 6-9)}$$

Where:

V = velocity (ft/s)

C_v = conveyance coefficient (from Table 6-7)

S_w = watercourse slope (ft/ft)

Table 6-7. Conveyance Coefficient, C_v

Type of Land Surface	C_v
Heavy meadow	2.5
Tillage/field	5
Riprap (not buried)*	6.5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

* For buried riprap, select C_v value based on type of vegetative cover.

The travel time is calculated by dividing the flow distance (in feet) by the velocity calculated using Equation 6-9 and converting units to minutes.

The time of concentration (t_c) is then the sum of the overland flow time (t_i) and the travel time (t_t) per Equation 6-7.

3.2.3 First Design Point Time of Concentration in Urban Catchments

Using this procedure, the time of concentration at the first design point (typically the first inlet in the system) in an urbanized catchment should not exceed the time of concentration calculated using Equation 6-10. The first design point is defined as the point where runoff first enters the storm sewer system.

$$t_c = \frac{L}{180} + 10 \qquad \text{(Eq. 6-10)}$$

Where:

t_c = maximum time of concentration at the first design point in an urban watershed (min)

L = waterway length (ft)

Equation 6-10 was developed using the rainfall-runoff data collected in the Denver region and, in essence, represents regional "calibration" of the Rational Method. Normally, Equation 6-10 will result in a lesser time of concentration at the first design point and will govern in an urbanized watershed. For subsequent design points, the time of concentration is calculated by accumulating the travel times in downstream drainageway reaches.

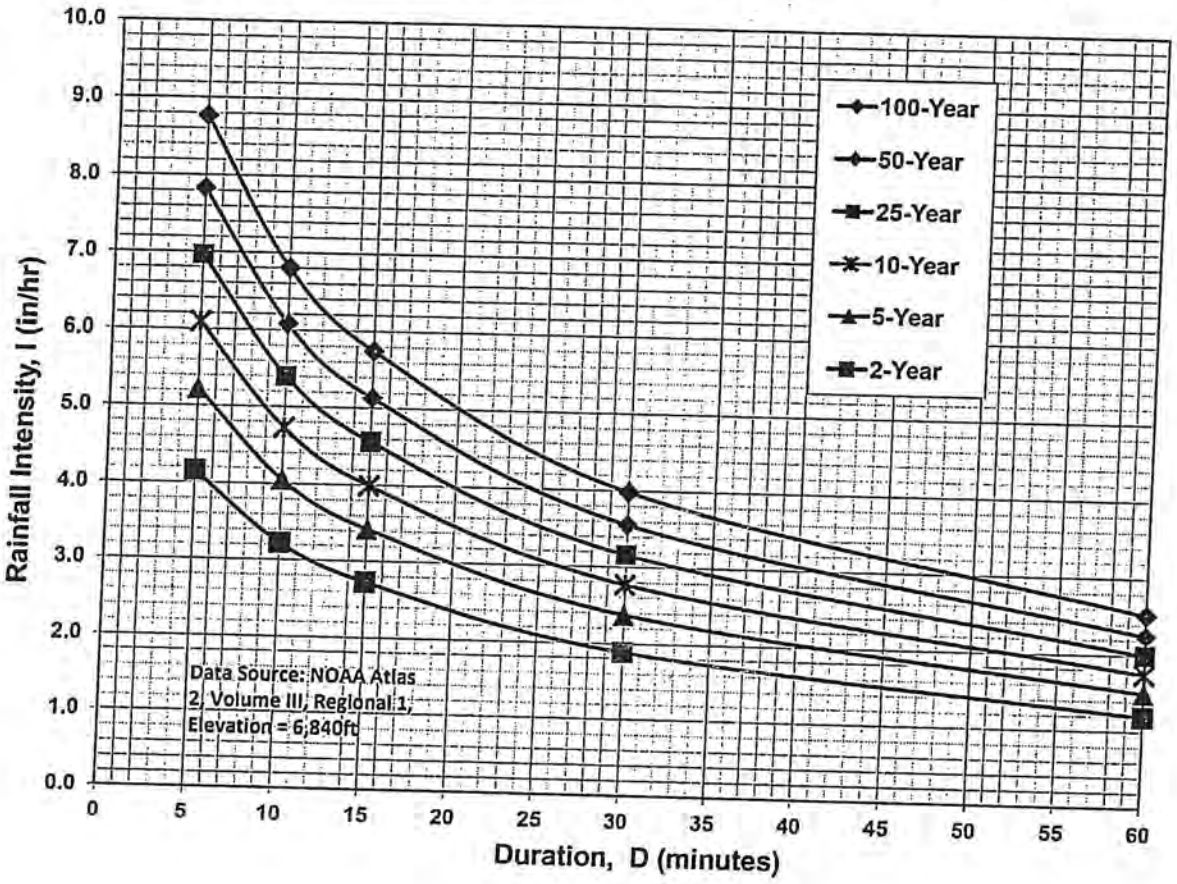
3.2.4 Minimum Time of Concentration

If the calculations result in a t_c of less than 10 minutes for undeveloped conditions, it is recommended that a minimum value of 10 minutes be used. The minimum t_c for urbanized areas is 5 minutes.

3.2.5 Post-Development Time of Concentration

As Equation 6-8 indicates, the time of concentration is a function of the 5-year runoff coefficient for a drainage basin. Typically, higher levels of imperviousness (higher 5-year runoff coefficients) correspond to shorter times of concentration, and lower levels of imperviousness correspond to longer times of

Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



IDF Equations

$I_{100} = -2.52 \ln(D) + 12.735$

$I_{50} = -2.25 \ln(D) + 11.375$

$I_{25} = -2.00 \ln(D) + 10.111$

$I_{10} = -1.75 \ln(D) + 8.847$

$I_{5} = -1.50 \ln(D) + 7.583$

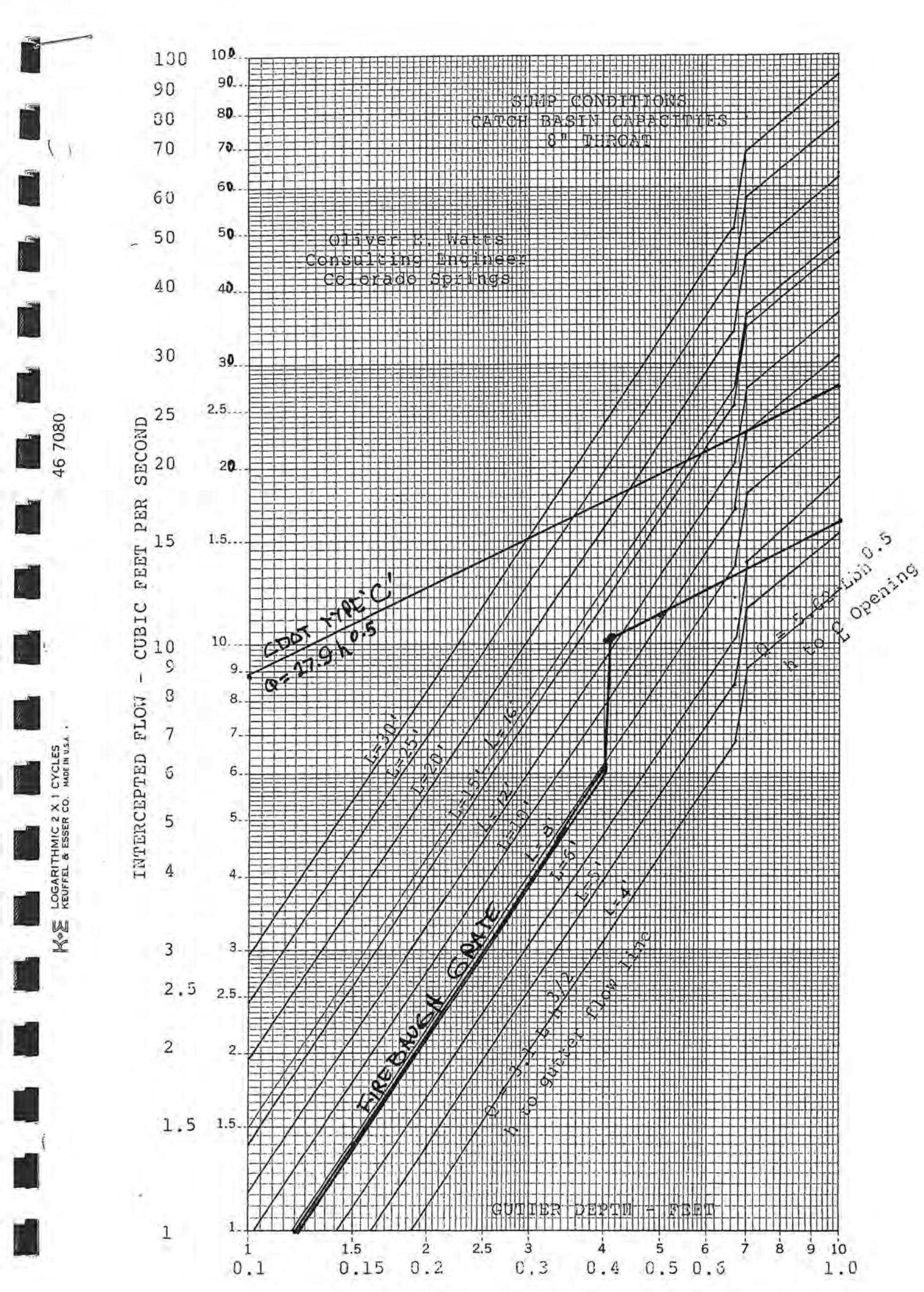
$I_{2} = -1.19 \ln(D) + 6.035$

Note: Values calculated by equations may not precisely duplicate values read from figure.

$$Q=\frac{0.463}{n} D^{8/3} S^{\frac{1}{2}} \qquad Q=KS^{\frac{1}{2}}$$

DIAMETER -IN.-	AREA -FT2-	D 8/3 -FT-	K			
			N=0.010	N=0.013	N=0.024	N=0.0
2	0.02182	0.008413	0.3895	---	---	---
4	0.08727	0.053420	2.4733	---	---	---
6	0.19630	0.157500	7.2922	5.609	---	---
8	0.34910	0.339200	15.7050	12.081	---	---
10	0.54540	0.615000	28.4745	21.903	---	---
12	0.78540	1.000000	46.3000	35.615	---	---
15	1.22720	1.813100	83.9465	64.574	---	---
18	1.76710	2.948300	136.5100	105.000	56.88	52.
21	2.40530	4.447400	205.9100	158.400	85.80	79.
24	3.14160	6.349600	293.9900	226.140	122.49	113.
27	3.97610	8.692700	402.4700	309.590	167.70	154.
30	4.90870	11.512600	533.0300	410.030	222.10	205.
33	5.93960	14.844100	---	528.680	---	---
36	7.06860	18.720800	866.7700	666.700	361.20	333.
39	8.29580	23.175100	---	825.400	---	---
42	9.62110	28.238900	---	1005.000	544.80	502.
48	12.56640	40.317500	---	1436.000	777.80	718.
54	15.90430	55.195000	---	1966.000	1065.00	983.
60	19.63500	73.100400	---	2604.000	1410.00	1302.
66	23.75830	94.254200	---	3357.000	1818.00	1678.
72	28.27430	118.869400	---	4234.000	2293.00	2117.
78	33.18310	147.152900	---	5241.000	2839.00	2620.
84	38.48450	179.306000	---	6386.000	3459.00	3193.
90	44.17860	215.524500	---	7676.000	4158.00	3838.
96	50.26550	256.000000	---	9118.000	4939.00	4559.
108	63.61730	350.466600	---	12480.000	6761.00	6140.
120	78.53980	464.158900	---	16530.000	8954.00	8265.

Oliver E. Watts
Consulting Engi
Colorado Sprin



46 7080

LOGARITHMIC 2 X 1 CYCLES
KEUFFEL & ESSER CO. MADE IN U.S.A.

CESSNA DRIVE
60' R.O.W.
(PUBLIC)

CESSNA DRIVE

LOT 4
FILING NO. 3
ZONING: R-4 / GA-O

20' ROAD EASEMENT

LOT 1
FILING NO. 10
0.21 ACRES
9,060 S.F.
ZONING: R-4 / GA-O

LOT 2
FILING NO. 10
0.19 ACRES
8,146 S.F.
ZONING: R-4 / GA-O

LOT 3
FILING NO. 10
0.19 ACRES
8,146 S.F.
ZONING: R-4 / GA-O

LOT 4
FILING NO. 10
EXIST. BUILDING
0.19 ACRES
8,146 S.F.
ZONING: R-4 / GA-O

LOT 5
FILING NO. 10
EXIST. BUILDING
0.27 ACRES
11,848 S.F.
ZONING: R-4 / GA-O

FILING NO. 10

LOT 5
FILING NO. 4
4.82 ACRES
210,000 S.F.
ZONING: R-4 / GA-O

LOT 4
FILING NO. 4
0.39 ACRES
16,880 S.F.
ZONING: R-4 / GA-O

LOT 3
FILING NO. 4
0.39 ACRES
16,988 S.F.
ZONING: R-4 / GA-O

LOT 2
FILING NO. 4
0.39 ACRES
16,880 S.F.
ZONING: R-4 / GA-O

LOT 1
FILING NO. 4
0.39 ACRES
16,880 S.F.
ZONING: R-4 / GA-O

FILING NO. 4

LOT 1
FILING NO. 13
0.85 ACRES
37,026 S.F.
ZONING: R-4 / GA-O

LOT 2
FILING NO. 13
0.78 ACRES
33,976 S.F.
ZONING: R-4 / GA-O

FILING NO. 13

LOT 2
FILING NO. 2
1.80 ACRES
78,408 S.F.
ZONING: R-4 / GA-O

A.C. PAV'T.
EXIST. DRAINAGEWAY
CONC. PAV'T.
INV OUT 22.00
S 19°35'42" E 128.00'
4" PVC
40' TAXI EASEMENT
40' TAXIWAY INGRESS/EGRESS EASEMENT
SAND FILTER BASIN
10
11,640 SF
0.27 AC.
ROOF DRAINS
DETENTION POND EASEMENT
N 70°24'18" E 92.50'
S 70°24'18" W 92.50'
153.66LF
EXIST. BUILDING
10' EASEMENT
N 19°35'42" W 128.00'
12" PVC OUTLET
229.16 LF
S=0.87%
50' TAXI EASEMENT
64.28LF
40.00LF
ASPHALT PAVEMENT
S 19°35'42" E 490.00'
DUTLET STRUCTURE
INV. OUT = 24.00
N 19°35'42" W 122.04'
POND EASEMENT
100-YEAR WATER LEVEL
28.35'
CONCRETE TRICKLE CHANNEL
10' DIKE
S 19°35'42" E 122.04'
31.24LF
12" PVC
CATCH BASIN
TOP 30.76
INV. 29.25
TAXIWAY EASEMENT
75.00'
136.37LF
12" PVC
S=0.67%
CATCH BASIN
TOP 31.61
INV. 30.11
LIMIT OF DISTURBANCE
DEMO EXISTING STORAGE BUILDING
1 STORY METAL COMMERCIAL BUILDING
TAXI EASEMENT
142,100 SF
3.262 AC.
6-3-16
16-4872-05
GRAVEL SURFACE
ROAD EASEMENT
TBM
EL. 6840.00
N 19°35'42" W 490.00'
N 70°24'18" E 290.00'
S 70°24'18" W 290.00'
6825
6826
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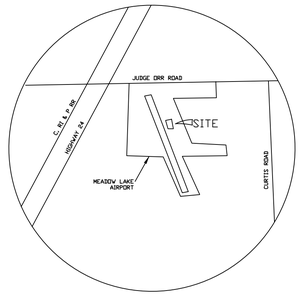
SF
SP
CWD
VTC
SSA

VICINITY MAP

N.T.S.



30 0 30 60 90

Scale 1" = 30'

Contour Interval: 1

ASSUMED DATUM

LEGEND:

- FOUND YELLOW #17655 CAP ON #4 REBAR
- FOUND #4 REBAR
- FOUND TELEPHONE PEDESTAL
- FOUND WELL

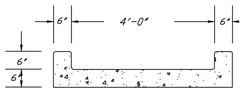
CONTOUR LEGEND:

ORIGINAL CONTOURS:

FINISH CONTOURS:

TRICKLE CHANNEL

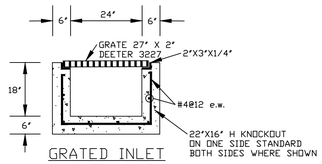
1/2" = 1'-0"



GRATED INLET

1/2"=1'-0"

INLETS SHALL CONFORM TO FIREBAUGH PRECAST CO. STD. INLET OR EQUAL



GRADING AND EROSION CONTROL PLAN

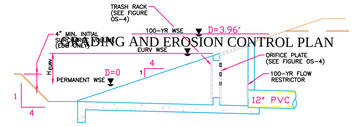
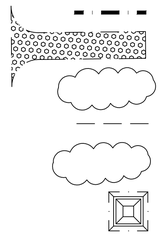


FIGURE OS-2 TYPICAL OUTLET STRUCTURE FOR FULL SPECTRUM DETENTION

EROSION CONTROL LEGEND:



- SILT FENCE (SF)
- VEHICLE TRACKING CONTROL (VTC)
- STOCK PILE PROTECTION (SP)
- LIMIT OF DISTURBANCE 2.292 AC.
- STABILIZED STAGING AREA (SSA)
- CONCRETE WASHOUT (CWD)

DRAWN BY: D.E. WATTS	APPROVED BY:	REVISIONS	OLIVER E. WATTS CONSULTING ENGINEER COLORADO SPRINGS
DATE: 7-13-21	PROJ. NO.		
DWG. NO.: 20-5498-06	DWG.		
SURVEYED BY: DEW, ESW, 8-14-20			

PROJECT: 8140 CESSNA DRIVE
LOT 7, BLOCK 1, MEADOW LAKE AIRPORT FIL. NO. 2
EL PASO COUNTY

SHT. NAME: GRADING AND EROSION CONTROL PLAN

SHT. NO.: 1 OF 1

the design, dimensions, and/ or elevations which shall be confirmed at the job site. The County through the approval of this document assumes no responsibility for completeness and/ or accuracy of this document.
Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and Engineering Criteria Manual, as amended.

In accordance with ECM Section 1.12, these construction documents will be valid for construction for a period of 2 years from the date signed by the El Paso County Engineer. If construction has not started within those 2 years, the plans will need to be resubmitted for approval, including payment of review fees at the Planning and Community Development Director's discretion.

_____
County Engineer/ECM Administrator Date

Review Engineer:
The Grading and Erosion Control Plan was reviewed and found to meet the checklist requirements except where otherwise noted or allowed by an approved deviation request.

_____
Review Engineer Date

Engineer's Statement (for standalone GEC Plan):
This Grading and Erosion Control Plan was prepared under my direction and supervision and is correct to the best of my knowledge and belief. Said Plan has been prepared according to the criteria established by the County for Grading and Erosion Control Plans. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this plan.

_____
Engineer of Record Signature Date
□liver E. Watts, CO□ PELS#9853
□liver E Watts Consulting Engineer, inc.
614 Elkton Drive Colorado Springs, CO 80907
719-593-0173
olliewatts@aol.com

□wner's Statement (for standalone GEC Plan):
I, the owner/developer have read and will comply with the requirements of the Grading and Erosion Control Plan.

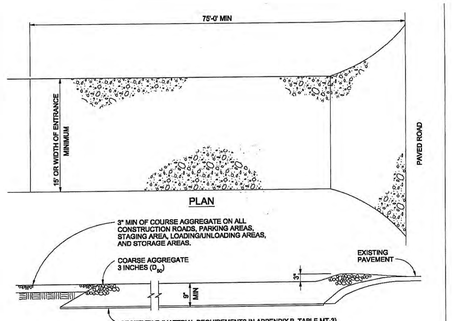
_____
□wner Signature Date

El Paso County (standalone GEC Plan):
County plan review is provided only for general conformance with County Design Criteria. The County is not responsible for the accuracy and adequacy of

STANDARD NOTES FOR EL PASO COUNTY GRADING AND EROSION CONTROL PLANS

1. Stormwater discharges from construction sites shall not cause or threaten to cause pollution, contamination, or degradation of State Waters. All work and earth disturbance shall be done in a manner that minimizes pollution of any on-site or off-site waters, including wetlands.
2. Notwithstanding anything depicted in these plans in words or graphic representation, all design and construction related to roads, storm drainage and erosion control shall conform to the standards and requirements of the most recent version of the relevant adopted El Paso County standards, including the Land Development Code, the Engineering Criteria Manual, the Drainage Criteria Manual, and the Drainage Criteria Manual Volume 2. Any deviations from regulations and standards must be requested, and approved, in writing.
3. A separate Stormwater Management Plan (SMWP) for this project shall be completed and an Erosion and Stormwater Quality Control Permit (ESQCP) issued prior to commencing construction. Management of the SWMP during construction is the responsibility of the designated Qualified Stormwater Manager or Certified Erosion Control Inspector. The SWMP shall be located on site at all times during construction and shall be kept up to date with work progress and changes in the field.
4. □nce the ESQCP is approved and a "Notice to Proceed" has been issued, the contractor may install the initial stage erosion and sediment control measures as indicated on the approved GEC. A Preconstruction Meeting between the contractor, engineer, and El Paso County will be held prior to any construction. It is the responsibility of the applicant to coordinate the meeting time and place with County staff.
5. Control measures must be installed prior to commencement of activities that could contribute pollutants to stormwater. control measures for all slopes, channels, ditches, and disturbed land areas shall be installed immediately upon completion of the disturbance.
6. All temporary sediment and erosion control measures shall be maintained and remain in effective operating condition until permanent soil erosion control measures are implemented and final stabilization is established. All persons engaged in land disturbance activities shall assess the adequacy of control measures at the site and identify if changes to those control measures are needed to ensure the continued effective performance of the control measures. All changes to temporary sediment and erosion control measures must be incorporated into the Stormwater Management Plan.
7. Temporary stabilization shall be implemented on disturbed areas and stockpiles where ground disturbing construction activity has permanently ceased or temporarily ceased for longer than 14 days.
8. Final stabilization must be implemented at all applicable construction sites. Final stabilization is achieved when all ground disturbing activities are complete and all disturbed areas either have a uniform vegetative cover with individual plant density of 70 percent of pre-disturbance levels established or equivalent permanent alternative stabilization method is implemented. All temporary sediment and erosion control measures shall be removed upon final stabilization and before permit closure.
9. All permanent stormwater management facilities shall be installed as designed in the approved plans. Any proposed changes that effect the design or function of permanent stormwater management structures must be approved by the ECM Administrator prior to implementation.
10. Earth disturbances shall be conducted in such a manner so as to effectively minimize accelerated soil erosion and resulting sedimentation. All disturbances shall be designed, constructed, and completed so that the exposed area of any disturbed land shall be limited to the shortest practical period of time. Pre-existing vegetation shall be protected and maintained within 50 horizontal feet of waters of the state unless shown to be infeasible and specifically requested and approved.
11. Compaction of soil must be prevented in areas designated for infiltration control measures or where final stabilization will be achieved by vegetative cover. Areas designated for infiltration control measures shall also be protected from sedimentation during construction until final stabilization is achieved. If compaction prevention is not feasible due to site constraints, all areas designated for infiltration and vegetation control measures must be loosened prior to installation of the control measure(s).
12. Any temporary or permanent facility designed and constructed for the conveyance of stormwater around, through, or from the earth disturbance area shall be a stabilized conveyance designed to minimize erosion and the discharge of sediment off site.
13. Concrete wash water shall be contained and disposed of in accordance with the SWMP. No wash water shall be discharged to or allowed to enter State Waters, including any surface or subsurface storm drainage system or facilities. Concrete washouts shall not be located in an area where shallow groundwater may be present, or within 50 feet of a surface water body, creek or stream.
14. During dewatering operations of uncontaminated ground water may be discharged on site, but shall not leave the site in the form of surface runoff unless an approved State dewatering permit is in place.
15. Erosion control blanketing or other protective covering shall be used on slopes steeper than 3:1.
16. Contractor shall be responsible for the removal of all wastes from the construction site for disposal in accordance with local and State regulatory requirements. No construction debris, tree slash, building material wastes or unused building materials shall be buried, dumped, or discharged at the site.
17. Waste materials shall not be temporarily placed or stored in the street, alley, or other public way, unless in accordance with an approved Traffic Control Plan. Control measures may be required by El Paso County Engineering if deemed necessary, based on specific conditions and circumstances.
18. Tracking of soils and construction debris off-site shall be minimized. Materials tracked off-site shall be cleaned up and properly disposed of immediately.
19. The owner/developer shall be responsible for the removal of all construction debris, dirt, trash, rock, sediment, soil, and sand that may accumulate in roads, storm drains and other drainage conveyance systems and stormwater appurtenances as a result of site development.
20. The quantity of materials stored on the project site shall be limited, as much as practical, to that quantity required to perform the work in an orderly sequence. All materials stored on-site shall be stored in a neat, orderly manner, in their original containers, with original manufacturer's labels.
21. No chemical(s) having the potential to be released in stormwater are to be stored or used onsite unless permission for the use of such chemical(s) is granted in writing by the ECM Administrator. In granting approval for the use of such chemical(s), special conditions and monitoring may be required.
22. Bulk storage of allowed petroleum products or other allowed liquid chemicals in excess of 55 gallons shall require adequate secondary containment protection to contain all spills onsite and to prevent any spilled materials from entering State Waters, any surface or subsurface storm drainage system or other facilities.
23. No person shall cause the impediment of stormwater flow in the curb and gutter or ditch except with approved sediment control measures.
24. □wner/developer and their agents shall comply with the "Colorado Water Quality Control Act" (Title 25, Article 8, CRS), and the "Clean Water Actnd s' (33 USC 1344), in addition to the requirements of the Land Development Code, DCM Volume II and the ECM Appendix I. All appropriate permits must be obtained by the contractor prior to construction (1041, NPDES, Floodplain, 404, fugitive dust, etc.). In the event of conflicts between these requirements and other laws, rules, or regulations of other Federal, State, local, or County agencies, the most restrictive laws, rules, or regulations shall apply.
25. All construction traffic must enter/exit the site only at approved construction access points.
26. Prior to construction the Permittee shall verify the location of existing utilities.
27. A water source shall be available on site during earthwork operations and shall be utilized as required to minimize dust from earthwork equipment and wind.
28. The soils report for this site has been prepared by _____ and shall be considered a part of these plans.
29. At least ten (10) days prior to the anticipated start of construction, for projects that will disturb one (1) acre or more, the owner or operator of construction activity shall submit a permit application for stormwater discharge to the Colorado Department of Public Health and Environment, Water Quality Division. The application contains certification of completion of a stormwater management plan (SWMP), of which this Grading and Erosion Control Plan may be a part. For information or application materials contact:
Colorado Department of Public Health and Environment
Water Quality Control Division
WQCD - Permits
4300 Cherry Creek Drive South
Denver, CO 80246-1530
Attn: Permits Unit

DRAWN BY: D.E. WATTS DATE: 7-13-21 DWG. NO.: 20-5498-07 SURVEYED BY: DEW, ESW, 8-14-20	APPROVED BY: PROJ. NO. DWG.	REVISIONS	OLIVER E. WATTS CONSULTING ENGINEER COLORADO SPRINGS	PROJECT 8140 CESSNA DRIVE LOT 7, BLOCK 1, MEADOW LAKE AIRPORT FIL. NO. 2 EL PASO COUNTY	SHT. NAME EROSION CONTROL DETAILS	SHT. NO. OF



SECTION

VEHICLE TRACKING

NTS

VEHICLE TRACKING NOTES

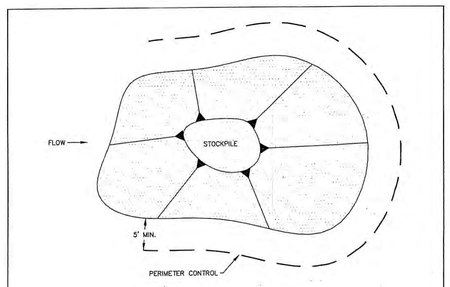
INSTALLATION REQUIREMENTS

1. ALL ENTRANCES TO THE CONSTRUCTION SITE ARE TO BE STABILIZED PRIOR TO CONSTRUCTION BEGINNING.
2. CONSTRUCTION ENTRANCES ARE TO BE BUILT WITH AN APRON TO ALLOW FOR TURNING TRAFFIC, BUT SHOULD NOT BE BUILT OVER EXISTING PAVEMENT EXCEPT FOR A SLIGHT OVERLAP.
3. AREAS TO BE STABILIZED ARE TO BE PROPERLY GRADED AND COMPACTED PRIOR TO LAYING DOWN GEOTEXTILE AND STONE.
4. CONSTRUCTION ROADS, PARKING AREAS, LOADING/UNLOADING ZONES, STORAGE AREAS, AND STAGING AREAS ARE TO BE STABILIZED.
5. CONSTRUCTION ROADS ARE TO BE BUILT TO CONFORM TO SITE GRADES, BUT SHOULD NOT HAVE SIDE SLOPES OR ROAD GRADES THAT ARE EXCESSIVELY STEEP.

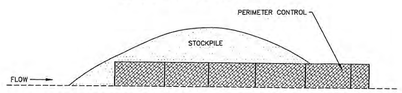
MAINTENANCE REQUIREMENTS

1. REGULAR INSPECTIONS ARE TO BE MADE OF ALL STABILIZED AREAS, ESPECIALLY AFTER STORM EVENTS.
2. STONES ARE TO BE REAPPLIED PERIODICALLY AND WHEN REPAIR IS NECESSARY.
3. SEDIMENT TRACKED ONTO PAVED ROADS IS TO BE REMOVED DAILY BY SHOVELING OR SWEEPING. SEDIMENT IS NOT TO BE WASHED DOWN STORM SEWER DRAINS.
4. STORM SEWER INLET PROTECTION IS TO BE IN PLACE, INSPECTED, AND CLEANED IF NECESSARY.
5. OTHER ASSOCIATED SEDIMENT CONTROL MEASURES ARE TO BE INSPECTED TO ENSURE GOOD WORKING CONDITION.

City of Colorado Springs Stormwater Quality	Figure VT-2 Vehicle Tracking Application Examples



STOCKPILE PROTECTION PLAN



STOCKPILE PROTECTION ELEVATION

INSTALLATION NOTES

1. INSTALL PERIMETER CONTROL AROUND STOCKPILE ON DOWNGRADIENT SIDE. PERIMETER CONTROL MUST BE SUITABLE TO SITE CONDITIONS AND INSTALLED ACCORDING TO THE RELEVANT DETAIL.
2. FOR STOCKPILES ON THE INTERIOR PORTION OF A CONSTRUCTION SITE, WHERE OTHER DOWNGRADIENT CONTROLS INCLUDING PERIMETER CONTROL ARE IN PLACE, STOCKPILE PERIMETER CONTROLS MAY NOT BE REQUIRED.

MAINTENANCE NOTES

1. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN CONTROL MEASURES IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
2. IF PERIMETER CONTROLS MUST BE MOVED TO ACCESS STOCKPILE, REPLACE PERIMETER CONTROLS BY THE END OF THE WORK DAY.
3. ACCUMULATED SEDIMENT MUST BE REMOVED ACCORDING TO PERIMETER CONTROL DETAIL.

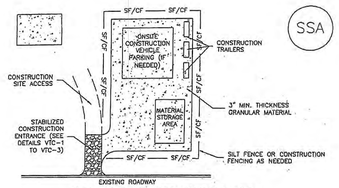
SP

STORMWATER ENTERPRISE — STOCKPILE PROTECTION

APPROVED: DIRECT SUPERVISOR

ISSUED: 10/7/19 | REVISED: 8/19/2020 | DRAWING NO. 900-SP

Stabilized Staging Area (SSA) SM-6



SSA-1. STABILIZED STAGING AREA

STABILIZED STAGING AREA INSTALLATION NOTES

1. SEE PLAN VIEW FOR
   -LOCATION OF STAGING AREA(S).
   -CONTRACTOR MAY ADJUST LOCATION AND SIZE OF STAGING AREA WITH APPROVAL FROM THE LOCAL JURISDICTION.
2. STABILIZED STAGING AREA SHOULD BE APPROPRIATE FOR THE NEEDS OF THE SITE. OVERSIZING RESULTS IN A LARGER AREA TO STABILIZE FOLLOWING CONSTRUCTION.
3. STAGING AREA SHALL BE STABILIZED PRIOR TO OTHER OPERATIONS ON THE SITE.
4. THE STABILIZED STAGING AREA SHALL CONSIST OF A MINIMUM 3" THICK GRANULAR MATERIAL.
5. UNLESS OTHERWISE SPECIFIED BY LOCAL JURISDICTION, ROCK SHALL CONSIST OF DOT SECT. §703, AASHTO #3 COARSE AGGREGATE OR 6" (MINUS) ROCK.
6. ADDITIONAL PERIMETER BMPs MAY BE REQUIRED INCLUDING BUT NOT LIMITED TO SILT FENCE AND CONSTRUCTION FENCING.

STABILIZED STAGING AREA MAINTENANCE NOTES

1. INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
4. ROCK SHALL BE REAPPLIED OR REGRADED AS NECESSARY IF RUTTING OCCURS OR UNDERLYING SUBGRADE BECOMES EXPOSED.

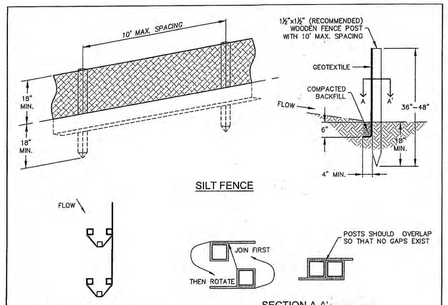
STABILIZED STAGING AREA MAINTENANCE NOTES

5. STABILIZED STAGING AREA SHALL BE ENLARGED IF NECESSARY TO CONTAIN PARKING, STORAGE, AND UNLOADING/LOADING OPERATIONS.
6. THE STABILIZED STAGING AREA SHALL BE REMOVED AT THE END OF CONSTRUCTION. THE GRANULAR MATERIAL SHALL BE REMOVED OR, IF APPROVED BY THE LOCAL JURISDICTION, USED ON SITE, AND THE AREA COVERED WITH TOPSOIL, SEEDED AND MULCHED OR OTHERWISE STABILIZED IN A MANNER APPROVED BY LOCAL JURISDICTION.

NOTE: MANY MUNICIPALITIES PROHIBIT THE USE OF RECYCLED CONCRETE AS GRANULAR MATERIAL FOR STABILIZED STAGING AREAS DUE TO DIFFICULTIES WITH RE-ESTABLISHMENT OF VEGETATION IN AREAS WHERE RECYCLED CONCRETE WAS PLACED.

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

(DETAILS ADAPTED FROM DOUGLAS COUNTY, COLORADO, NOT AVAILABLE IN AUTOCAD)



SILT FENCE

J-HOOK INSTALLATION

SECTION A-A'

INSTALLATION NOTES

1. SILT FENCE MUST BE PLACED ON A FLAT SURFACE 2'-5' AWAY FROM TOE OF THE SLOPE TO ALLOW FOR PONDING AND DEPOSITION.
2. COMPACT THE TRENCH USING A JUMPING JACK OR WHEEL ROLLING TO THE POINT THAT THE FENCE RESISTS BEING PULLED OUT OF THE GROUND BY HAND.
3. SILT FENCE SHALL BE TAUT WITH NO SAGS AFTER IT HAS BEEN ANCHORED.
4. FABRIC SHALL BE ATTACHED TO POSTS WITH 1" HEAVY DUTY STAPLES OR 1" NAILS. THESE SHOULD BE PLACED VERTICALLY DOWN THE POST, 3" APART.
5. THE PREFERRED INSTALLATION METHOD USES A TRENCHER OR SILT FENCE INSTALLATION DEVICE.
6. INSTALL SILT FENCE ALONG THE CONTOUR OF THE SLOPES OR IN A MANNER TO AVOID CREATING CONCENTRATED FLOW (SUCH AS A "J-HOOK" INSTALLATION).

MAINTENANCE NOTES

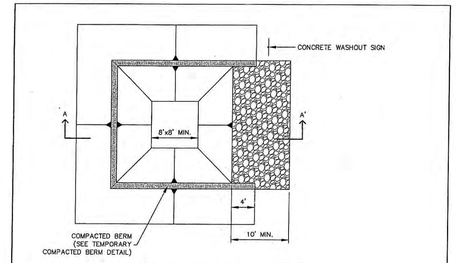
1. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN CONTROL MEASURES IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
2. ACCUMULATED SEDIMENT MUST BE REMOVED WHEN THE HEIGHT REACHES ½ OF THE DESIGN HEIGHT OF THE SILT FENCE.
3. SILT FENCE MUST REMAIN UNTIL THE UPSTREAM DISTURBANCE AREA IS STABILIZED.
4. PERMANENTLY STABILIZE AREA AFTER SILT FENCE IS REMOVED.

SF

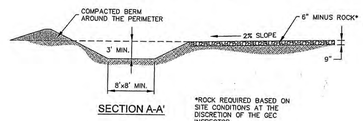
STORMWATER ENTERPRISE — SILT FENCE

APPROVED: CITY ENGINEER

ISSUED: 10/7/19 | REVISED: 8/19/2020 | DRAWING NO. 900-SF



CONCRETE WASHOUT AREA PLAN



SECTION A-A'

*ROCK REQUIRED BASED ON SITE CONDITIONS AT THE DISCRETION OF THE GEC INSPECTOR

CWA

STORMWATER ENTERPRISE — CONCRETE WASHOUT AREA

APPROVED: CITY ENGINEER

ISSUED: 10/7/19 | REVISED: 8/19/2020 | DRAWING NO. 900-CWA-1

INSTALLATION NOTES

1. SEE PLAN VIEW FOR:
   -LOCATION OF CONCRETE WASHOUT AREA
2. LOCATE AT LEAST 50' AWAY FROM STATE WATERS MEASURED HORIZONTALLY.
3. AN IMPERMEABLE LINER (16 MIL. MINIMUM THICKNESS) IS REQUIRED IF CONCRETE WASH AREA IS LOCATED WITHIN 400' OF STATE WATERS OR 1000' OF WELLS OR DRINKING WATER SOURCES.
4. DO NOT LOCATE IN AREAS WHERE SHALLOW GROUNDWATER MAY BE PRESENT.
5. THE CONCRETE WASH AREA SHALL BE INSTALLED PRIOR TO CONCRETE PLACEMENT ON SITE.
6. CONCRETE WASH AREA SHALL INCLUDE A FLAT SUBSURFACE PIT THAT IS AT LEAST 8' BY 8'.
7. BERM SURROUNDING SIDES AND BACK OF CONCRETE WASH AREA SHALL HAVE A MINIMUM HEIGHT OF 2 FEET.
8. CONCRETE WASH AREA ENTRANCE SHALL BE SLOPED 2% TOWARDS THE CONCRETE WASH AREA.
9. SIGNS SHALL BE PLACED AT THE CONCRETE WASH AREA.
10. USE EXCAVATED MATERIAL FOR PERIMETER BERM CONSTRUCTION.

MAINTENANCE NOTES

1. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN CONTROL MEASURES IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
2. THE CONCRETE WASH AREA SHALL BE REPAIRED, CLEANED, OR ENLARGED AS NECESSARY TO MAINTAIN CAPACITY FOR CONCRETE WASTE. CONCRETE MATERIALS ACCUMULATED IN THE PIT SHALL BE REMOVED ONCE THE MATERIALS HAVE REACHED A DEPTH OF ⅓ THE HEIGHT OF THE CONCRETE WASH AREA.
3. CONCRETE WASHOUT WATER, WASTED PIECES OF CONCRETE, AND ALL OTHER DEBRIS IN THE SUBSURFACE PIT SHALL BE TRANSPORTED FROM THE JOB SITE IN A WATER-TIGHT CONTAINER AND DISPOSED OF PROPERLY.
4. THE CONCRETE WASH AREA SHALL REMAIN IN PLACE UNTIL ALL CONCRETE FOR THE PROJECT IS PLACED.
5. PERMANENTLY STABILIZE AREA AFTER CONCRETE WASH AREA IS REMOVED.

CWA

STORMWATER ENTERPRISE — CONCRETE WASHOUT AREA

APPROVED: CITY ENGINEER

ISSUED: 10/7/19 | REVISED: 8/19/2020 | DRAWING NO. 900-CWA-2

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DRAWN BY: O.E. WATTS	APPROVED BY:	REVISIONS	OLIVER E. WATTS CONSULTING ENGINEER COLORADO SPRINGS	PROJECT: COLORADO SPRINGS	SHT. NAME: EROSION CONTROL DETAILS	SHT. NO. OF
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