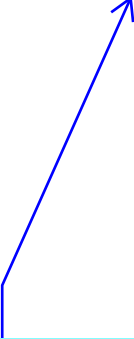


PRELIMINARY DRAINAGE REPORT
for
THE COMMONS AT FALCON FIELD

El Paso County, Colorado

March 2024

PCD FILE NO. SP-232



Please relabel to Preliminary drainage report for the commons at falcon field and final drainage report for Pre-development (Early) grading for The Commons at Falcon Field.

Prepared for:

Falcon Field, LLC

30 N. Tejon Street, #516
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(719) 475-7474

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PRELIMINARY DRAINAGE REPORT
for
THE COMMONS AT FALCON FIELD
Falcon, Colorado

1.0 CERTIFICATION STATEMENTS

ENGINEER'S STATEMENT

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports, and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omission on my part in preparing this report.

Tim D. McConnell, P.E. Date
Colorado P.E. License No. 33797
For and on Behalf of Drexel, Barrell & Co.

DEVELOPER'S STATEMENT

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

Business Name: Falcon Field, LLC.

By: Date
PJ Anderson
Title: Owner
Address: 30 N. Tejon St., #516
Colorado Springs, CO 80903

EL PASO COUNTY

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Criteria Manual, as amended.

Joshua Palmer, P.E. Date
County Engineer/ECM Administrator
CONDITIONS:

PRELIMINARY DRAINAGE REPORT
for
THE COMMONS AT FALCON FIELD
Falcon, Colorado

2.0 PURPOSE

This report is prepared by Drexel, Barrel & Co in support of The Commons at Falcon Field project. The purpose of this report is to identify onsite and offsite drainage patterns, storm sewer, inlet locations, and areas tributary to the site, and to safely route developed storm water runoff to adequate outfall facilities.

3.0 GENERAL SITE DESCRIPTION

Location

The Commons at Falcon Field site is approximately 57.7 acres and is bounded by U.S. Highway 24 along the northwest, a school to the south, and a large-lot residential development to the east and northeast. The site is in the east half of Section 7, Township 13 South, Range 64 West of the 6th PM.

Drainage Fees will be assessed and paid according to the current rates at the time of platting. All easements for utilities and drainage features will be provided with the final plat process.

Existing Site Conditions

The site is currently open grass land with one single-family residence and barn. The residence is supported by a well and individual septic system. There are no known utilities on site. Offsite runoff enters the site through a box culvert under Highway 24, along the northern boundary of the property. The box culvert discharges through the site in an open drainage to the south. Smaller offsite basins, including Highway 24 along the northern boundary currently discharge onto the property, these basins are further described below.

Proposed Site Conditions

The Commons at Falcon Field is a proposed mixed-use commercial and residential development and is proposed to consist of 169 single-family lots and 8 commercial pads, along with associated roadways and open space. This development is anticipated to be phased into three separate areas – commercial, south residential and east residential. A pre-subdivision early grading request has been submitted to the County to allow for the overlot grading, box culvert installation and subsequent floodplain modification to occur. Overlot grading is proposed to cover the entirety of the site to allow for a balanced cut-fill scenario.

Soils

According to the Soil Survey of El Paso County Area, Colorado, prepared by the U.S. Department of Agriculture Soil Conservation Service, the site is partially underlain by Blakeland Loamy Sand (Soil No. 8), and predominantly by Columbine gravelly sandy loam (Soil No. 19). Both soils are type 'A' hydrological soil group. See appendix for map.

Climate

This area of El Paso County can be described as the foothills, with total precipitation amounts typical of a semi-arid region, roughly 15 inches annually. The climate of the site is typical of a sub-humid to semi-arid climate with mild summers and winters. The average temperature is 31 degrees F in the winter and 68.4 degrees F in the summer.

Floodplain Statement

The Flood Insurance Rate Maps (FIRM No. 08041C0553-G & 08041C0561-G both dated 12/7/18) indicate that there is a Zone A floodplain area that covers the "Falcon Creek East Tributary" that bisects the site, but this area is not a designated regulatory floodway. This reach of the channel is the subject of a FEMA floodplain study currently being completed by separate report and analysis.

Previous Drainage Studies

The site is located within the East Tributary Basin of the Falcon Basin Watershed, as studied in the Falcon Drainage Basin Planning Study, prepared by Matrix Design Group, September, 2015. DBPS recommendations are presented later in this report.

4.0 DRAINAGE CRITERIA

The drainage analysis has been prepared in accordance with the current El Paso County Drainage Criteria Manual. Calculations were performed to determine runoff quantities during the 5-year and 100-year frequency storms for historic and developed conditions using the Rational Method as required for basins containing less than 100 acres.

In addition, the following Mile High Flood District (MHFD) provided spreadsheet MHFD-Detention v4.04 was used for preliminary design of the detention facilities.

5.0 DBPS ANALYSIS

Existing Conditions

The Falcon DBPS watershed establishes three major basins, including the "East Tributary" which covers this property. The DBPS completed hydrologic analysis for the Falcon Basin Watershed, using HEC-HMS v.3.5 software, for historical, existing and future land use conditions by applying a 24-hour storm event with 2-, 5-, 10-, 25-, 50-, and 100-year recurrence intervals and current drainage conveyance infrastructure.

As mentioned earlier, offsite flows from the Woodmen Hills Detention Pond #4 enter the

Commons at Falcon Field site via two 4.83'x12' box culverts underneath U.S. Highway 24 (DBPS identifier ET14), these combine with onsite flows and follow the historic reach RET100 of the Falcon Creek East Tributary to the south.

The following table details the HEC-HMS analysis of existing conditions across the Commons at Falcon Field development.

**Peak Discharges for the Existing Condition at Points of Interest in vicinity of
The Commons at Falcon Field Development (DBPS)**

| Location | Existing Conditions (source: Falcon Basin, Drainage Planning Study, HEC-HMS model) | | | | | | | |
|---|---|--------------|-----------------|------|-------|-------|-------|--------|
| | HEC-HMS Element | Area (sq mi) | Peak Flow (cfs) | | | | | |
| | | | 2-yr | 5-yr | 10-yr | 25-yr | 50-yr | 100-yr |
| East tributary at North Property Line of Commons at Falcon Field | RET090 | 1.66 | 14 | 36 | 55 | 170 | 230 | 320 |
| East tributary through Commons at Falcon Field | RET100 | 1.78 | 15 | 39 | 64 | 170 | 270 | 370 |
| Local Basin | ET100 | 0.05 | 1 | 6 | 10 | 21 | 27 | 34 |
| East tributary South of Commons at Falcon Field Property Line | RET110 | 1.83 | 15 | 40 | 65 | 170 | 270 | 380 |

The DBPS flow rates shown in the table above were used as the basis of the existing condition analysis of the Commons at Falcon Field development. Site specific basins have been allocated and referenced on the existing conditions map in the appendix.

The Falcon DBPS recommends junction and reach improvements for RET100 (reach through project site) and subsequent reach RET110 downstream junction ET13 at Pinto Pony to Falcon Highway. The existing culvert at Highway 24 was identified as undersized, but not included in the DBPS recommendations as it is a CDOT owned structure. The improvements include the following recommendations:

- Reach RET100 – Installation of small drop structures with toe protection
- Reach RET110 – Installation of small drop structures with toe protection
- Junction ET13 (Pinto Pony) – replace existing crossing with (2) 6'x8' culverts
- Junction ET11 (Falcon Highway) – replace existing crossing with (2) 6'x8' culverts

The Commons at Falcon Field development includes regrading and rerouting a portion of the East Tributary. The improvements intercept the Highway 24 (CDOT owned) culvert immediately south of the Highway 24 and convey via 8'x4' concrete box culvert 750-lf to the south through the project site towards an open channel. The proposed box culvert discharges to the proposed open channel via a headwall. The proposed open channel conveys the flow 275-lf downstream to tie into the existing creek and will be vegetated with mowable short grasses. The open channel is proposed with a 20-foot bottom width in a v-shape with two 10-foot sections set as a 2% slope to the invert.

as this section of the channel will be piped, the proposed box culvert will be privately owned and maintained. Please indicate that in the report.

As early grading is proposed with this application that includes the install of proposed box culvert, an approved CLOMR will be required prior to any grading. Final hydraulic design and construction drawings of the proposed box culvert, channel, and downstream facilities will be required within at this stage. Please provide the hydraulic designs within this report and submit CD's for review.

marketplace project will be the

In addition to the DBPS, a site specific analysis of the existing conditions was completed. The flows determined by the DBPS for the tributary entering the site from the north (RET090), were used in combination with rational method analysis for the surrounding onsite/offsite flows. For this preliminary analysis, the DBPS flows were directly added to the rational method flows. Further analysis of the flows will be completed with the CLOMR study.

Basin OS1 represents a portion of the southern half of U.S. Highway 24 along the northwest boundary for the Commons at Falcon Field site. Due to no curb and gutter along this stretch of U.S. Highway 24, flows from this basin discharge directly into basin E1 as overland flow and are represented by **Design Point DPA**. Runoff rates at existing DPA are $Q_5=3.4$ cfs and $Q_{100}=7.6$ cfs.

Basin E1 covers 13.85 acres of open space in the northwestern portion of the site. Flows from this basin combine with those from DPA and travel to the southwest towards the East Tributary of Falcon Creek. The east tributary bisects basin E1 running from northern most portion of the site south towards the bottom of Basin E1 where flows are discharged at rates of $Q_5=3.2$ cfs and $Q_{100}=22.4$ cfs. These flows are consistent with those established by the DBPS for the local basin, see table above. These flows combine with those from the DBPS RET090 at **Design Point DPB** with rates of $Q_5=41.0$ cfs and $Q_{100}=346.4$ cfs, and discharge to the south as defined channel flow. As previously mentioned for the purposes of this preliminary analysis, the DBPS flows were directly added to the Rational Method flows. Further detailed flow analysis will be completed as part of the CLOMR study.

Basin OS2 represents the remainder of the southern half of U.S. Highway 24 along the northwest boundary for the Commons at Falcon Field site. Due to no curb and gutter along this stretch of U.S. Highway 24, flows from this basin discharge as overland flow directly into basin E2 and are represented by **Design Point DPC**. Runoff rates at existing DPC are $Q_5=1.4$ cfs and $Q_{100}=3.2$ cfs.

Basin OS3 is located along the southeastern edge of Basin E2. This basin consists native grasses and vegetation, and a small outbuilding. The flows within this basin travel to the southwest where they will discharge as overland flow into basin E2 at **Design Point DPD**. The runoff rates entering basin E2 at DPD are $Q_5=0.8$ cfs and $Q_{100}=5.6$ cfs.

Basin E2 is 12.88 acres of open space located to the west of Basin E1. The basin is sloped to the southeast at roughly 3% before turning directly south upon reaching the eastern border of the basin. Flows combine with those from Basin OS2 then travel south until roughly halfway down the basin where Basin OS3 adds to the flows. From there the combined flows from Basins OS2, OS3, and E2 continue to the south as overland flow where they are released from the site at **Design Point DPE** at rates of $Q_5=3.8$ cfs and $Q_{100}=23.9$ cfs.

RATIONAL METHOD RUNOFF SUMMARY

| BASIN & DESIGN POINT SUMMARY | | | | |
|---|-----------|------------------|-----------|-------------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| EXISTING BASINS | | | | |
| OS1 | A | 1.34 | 3.4 | 7.6 |
| E1 | | 13.85 | 3.2 | 22.4 |
| DPA+E1+RET090 | B | 15.19 | 41.0 | 346.4 |
| OS2 | C | 0.60 | 1.4 | 3.2 |
| OS3 | D | 2.56 | 0.8 | 5.6 |
| E2 | | 12.88 | 2.5 | 18.6 |
| DPC+DPD+E2 | E | 16.04 | 3.8 | 23.9 |
| E3 | F | 13.11 | 2.7 | 19.6 |
| OS4 | G | 3.29 | 2.4 | 9.7 |
| E4 | | 1.57 | 0.3 | 2.6 |
| DPG+E4 | H | 4.86 | 1.9 | 9.0 |
| E5 | I | 5.91 | 2.2 | 11.7 |
| OS5 | J | 5.50 | 6.6 | 6.6 |
| OS6 | K | 0.91 | 2.6 | 5.5 |
| E6 | | 10.37 | 1.7 | 12.5 |
| DPJ+DPL+E6 | M | 16.78 | 4.1 | 21.9 |

Basin E3 represents 13.11 acres in the southwest corner of the site. Basin E3 directs flows from the north and south to the southern border, exiting the site as overland flow with runoff rates of $Q_5=2.7$ cfs and $Q_{100}=19.6$ cfs at **Design Point DPF**.

Basin OS4 runs down the western side of the site and includes a portion of the southern edge of U.S. Highway 24. The 3.29-acre, offsite basin, generates flows that will travel to the south until they reach **Design Point DPG** at the bottom of the basin. Where they will enter Basin E4 as overland flow at rates of $Q_5=2.4$ cfs and $Q_{100}=9.7$ cfs.

Basin E4 is a small, 1.57-acre basin on the far western side of the site. The basin will receive offsite flows from Basin OS4 before directing all of flow south where it exits the western side of the site as overland flow at **Design Point DPH**, with runoff rates of $Q_5=1.9$ cfs and $Q_{100}=9.0$ cfs.

Basin E5 is 5.91 acres located on the eastern side of the site, sandwiched between the southern portions of Basin E1 and E6. The basin directs all of its flows south as overland flow towards the existing **Design Point DPG** which sits on the southern border of the site, directly in the middle of Basin E5. This basin generates runoff rates of $Q_5=2.4$ cfs and $Q_{100}=9.7$ cfs.

Basin OS5 is an offsite basin located to the north of Rio Lane and includes the northern half of Rio Lane. Runoff from this basin is captured by a roadside ditch and travels towards an existing 18" CMP culvert underneath Rio Lane, located approximately two-thirds of the way along the project boundary. The full-flow capacity of this existing 18" CMP culvert at 1.0% (field-surveyed grade) has been conservatively used to determine

the flows entering from the project site from this basin, rather than using the Rational Method calculation. This approach considers the existing roadside ditch along the north side of Rio Lane as emergency overflow for flows not captured by the existing culvert. Field observations indicate no evidence of roadway overtopping in this area. As a result, flows of $Q_{100}=6.6$ cfs exit the 18" CMP culvert and enter the project site at DPJ as concentrated flow.

Basin OS6 covers the southern half of Rio Lane along the northern boundary of the Commons at Falcon Field site. Due to no curb and gutter along Rio Lane, flows from this basin discharge as overland flow directly into Basin E6 and are represented by **Design Point DPL** with runoff rates of $Q_5=2.6$ cfs and $Q_{100}=5.5$ cfs.

Basin E6 represents the eastern most basin of the site. At 10.37 acres, the basin directs flows from its northwestern corner, and from Basins OS5, and OS6, southeast until they reach the existing **Design Point M** where they exit the site as overland flow. Runoff rates at DPM will be $Q_5=4.1$ cfs and $Q_{100}=21.9$ cfs.

7.0 PRESUBDIVISION OVERLOT GRADING

Presubdivision grading has been applied for at the Preliminary Plan stage to allow for overlot grading to occur. The site has been designed to balance, but all three phases need to be overlot graded simultaneously to allow this to happen.

For the purpose of site specific analysis, the project has been divided into several grouped drainage basins as shown on the proposed overlot drainage plan. These basins follow the same naming conventions as the proposed drainage basins described later in this report.

A-Group Basins are 2 basins that will represent the entire A-basin area, they will represent flows generated within the eastern residential portion of the site that will primarily be intercepted by Pond A, ultimately discharging out to the redefined tributary open channel. **Pond A will act as a temporary sediment basin until full development occurs.**

Basin A1 is 17.73 acres, representing the large majority of the eastern residential portion of the site. All of the runoff generated within this basin will be directed via temporary diversion swales towards Pond A. The runoff reaching this pond during this preliminary overlot phase will be $Q_5=4.1$ cfs and $Q_{100}=30.1$ cfs.

Basin A2 covers a portion of Tract A along the east and southern boundary. Flows generated by this 2.45-acre basin are directed offsite as overland sheet flow. The majority of this basin will be regraded but will remain undeveloped as an open space tract. A 0.25-acre portion of this basin covers the rear of the lots at the southeast corner of the property. It is anticipated that this area will fall under ECM 1.7.1.C.1. as the ability to capture and treat flows generated by Basin A2 is restricted due to grading constraints. Any area outside of the criteria exclusion limit will be considered for runoff reduction. This will be further analyzed at the final drainage report stage.

RATIONAL METHOD RUNOFF SUMMARY

| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|----|-----------|-----|------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| PRELIMINARY-BASINS | | | | |
| A1 | 1 | 17.73 | 4.1 | 30.1 |
| A2 | | 2.45 | 0.6 | 4.6 |
| B1 | | 12.47 | 3.0 | 22.4 |
| OB1 | O1 | 1.34 | 3.4 | 7.6 |
| OB2 | O2 | 0.60 | 1.4 | 3.2 |
| B1+O1+O2 | 2 | 14.41 | 5.9 | 28.9 |
| C1 | | 6.29 | 1.5 | 11.1 |
| OC | | 2.55 | 0.6 | 4.5 |
| C1+OC | 3 | 8.85 | 2.1 | 15.6 |
| D1 | 4 | 15.99 | 3.7 | 27.4 |
| D2 | | 2.81 | 0.7 | 5.2 |
| OD | | 2.70 | 0.7 | 4.8 |
| D2+OD | 5 | 5.51 | 1.4 | 10.2 |

B-group basins represent the central commercial portion of the site that will be intercepted by Pond B, ultimately discharging out to the redefined tributary open channel. Pond B will act as a temporary sediment basin until full development occurs.

Basin OB1 represents 1.34 acres of US-HWY 24, which acts as the northwestern boundary for the site. The runoff generated by this basin, $Q_5=3.4$ cfs and $Q_{100}=7.6$ cfs, will discharge directly onto the project site at **Design Point O1**, where flows will continue to the southeast. Flows will be directed through the site towards the temporary sediment basin (Pond B) via temporary diversion swales.

Basin OSB2 represents 0.60-acres of US-HWY 24, which acts as the northwestern boundary for the site. The runoff generated by this basin, $Q_5=1.4$ cfs and $Q_{100}=3.2$ cfs, is directed northeast towards the proposed Woodmen Road extension and **Design Point O2**.

Basin B covers the entirety of the central commercial portion of the site. Similar to basin A1, all of the runoff generated within this 12.47 acre basin will be directed southeast via temporary diversion swale towards the temporary sediment basin at Pond B. The runoff reaching Pond B during this preliminary overlot phase, which includes basins OSB1 and OSB2, will be $Q_5=5.9$ cfs and $Q_{100}=28.9$ cfs.

C-group basins represent the western commercial portion of the site that will be intercepted by a temporary sediment basin along the western project boundary, ultimately discharging to the south towards the temporary sediment basin located at Design Point 4.

Basin OC covers an offsite area along the western boundary of the project site, and includes a portion of U.S. Highway 24. The 2.55-acre offsite basin, will follow current conditions along the western property boundary of the site until they reach the southeast

corner where they will enter Basin C1 at rates of $Q_5=0.6$ cfs and $Q_{100}=11.1$ cfs and be captured by a proposed temporary sediment basin at Design Point 3.

identify that the outfall of this temp. sediment basin will be directed to the temp. sediment pond at DP4 as shown on the drainage map.

Basin C1 is 6.29 acres, representing the entire western commercial portion of the site. The runoff generated within this basin will amount to $Q_5=1.5$ cfs and $Q_{100}=11.1$ cfs, this runoff will be directed via temporary diversion swales towards the southwest corner of the basin and the temporary sediment basin at Design Point 3.

D-group basins represent the southern residential portion of the site that will be intercepted by a temporary sediment basin located at Pond C, ultimately discharging out to follow historic conditions to the southeast.

Basin OD is located along the eastern side of the upper half of Basin D2. The flows within this basin will flow to the southwest where they will naturally gather and channel along the westerly edge of Basin D2. The runoff rates entering Basin D2 are $Q_5=0.7$ cfs and $Q_{100}=4.8$ cfs.

Basin D1 covers the majority of the southern residential portion of the site. All of the runoff generated within this 15.99 acre basin captured by Pond C at the southern side of the basin. Runoff, at rates of $Q_5=3.7$ cfs and $Q_{100}=27.4$ cfs, will be channeled south within temporary diversion swales into the temporary sediment basin at Design Point 4.

Basin D2 represents the southern and eastern boundaries for the southern residential portion of the site. Within this 2.81 acre basin, runoff will be generated at rates of $Q_5=0.7$ cfs and $Q_{100}=5.2$ cfs. This runoff, as well as that from basin OD, will be directed via temporary diversion swale towards the temporary sediment basin at Design Point 4. The temporary sediment basin outfalls have been design to release at historic rates. Installation of a level spreader berm at the pond outfall will allow flows to dissipate into a more natural overland condition rather than discharging as concentrated flow.

Please clarify how the flows from this basin will enter the temp. sediment basin. There is a swale that is created along the east property line that directs concentrated flow to the south onto the adjacent property. This basin should be broken up into two basins, one for the east side and another for the south west side, as the outfall is two different locations

8.0 PROPOSED CONDITION

For the purposes of site specific analysis, the project is divided into several grouped drainage basins as shown on the proposed drainage map.

The Rational Method was used to determine runoff coefficients for various storm recurrence intervals. Mile High Flood District Model was used for preliminary pond sizing, see appendix for calculations and a summary table.

A site investigation is currently underway to evaluate existing groundwater conditions. In order to mitigate potential issues, the site grading in several areas of the site will be raised from the existing condition and as such, will increase the separation above shallow water areas. The results of the groundwater monitoring will indicate whether further mitigation measures will be required on the site, particularly at the detention basins. This analysis will be completed at the final plat stage with the construction documents.

The existing channel through the site is proposed to be piped via 8'x4' box culvert from the existing outfall south of U.S. Highway 24, through the site before discharging into a

please label this on the overlot grading drainage plan.

redefined open channel to the south of the proposed Retail Row St. A CLOMR study is currently underway to determine the feasibility of this approach.

Any underdrain system to be installed will be the responsibility of the Falcon Field District. State and Groundwater District permitting for discharges will be the responsibility of the of the District.

A-group basins represent flows at the eastern residential portion of the site that will be intercepted by Pond A, ultimately discharging out to the redefined tributary open channel.

Rational Method Runoff Summary (A-group)

| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|-----|-----------|------|------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| A-BASINS | | | | |
| A1 | 1 | 0.34 | 1.3 | 2.5 |
| A2 | | 1.51 | 2.5 | 6.8 |
| DP1+A2 | 2 | 1.85 | 3.8 | 9.3 |
| A3 | 3 | 1.49 | 3.2 | 6.9 |
| A4 | 4 | 0.60 | 2.2 | 4.2 |
| A5 | 5 | 2.47 | 4.0 | 9.0 |
| A6 | 6 | 1.74 | 2.9 | 6.4 |
| A7 | | 1.47 | 2.4 | 5.3 |
| DP5+DP6+A7 | 7 | 5.67 | 8.4 | 18.7 |
| A8 | 8 | 0.65 | 1.4 | 3.0 |
| A9 | | 2.55 | 4.9 | 10.8 |
| DP8+A9 | 9 | 3.21 | 6.4 | 14.1 |
| A10 | 10 | 3.25 | 5.6 | 12.2 |
| DP7+DP10 | 10A | 8.92 | 17.8 | 39.4 |
| A11 | | 0.99 | 2.0 | 4.3 |
| DP3+A11 | 11 | 2.48 | 5.3 | 11.5 |
| DP10A+DP9+DP11 | 11A | 14.61 | 28.8 | 63.4 |
| A12 | | 0.97 | 0.2 | 1.8 |
| DP2+DP11A+A12 | 12 | 17.42 | 31.8 | 70.1 |
| A13 | 13 | 2.39 | 1.4 | 7.7 |
| A14 | 14 | 0.53 | 0.2 | 1.4 |

Basin A1 is located on the western side of Rio Lane. Runoff will flow south via curb and gutter at rates of $Q_5=1.3$ cfs and $Q_{100}=2.5$ cfs towards **Design Point DP1**. At DP1, flows will be captured by a proposed 5' Type R storm inlet located at the southwestern most point of Rio Lane.

Basin A2 covers the eastern side of Rio Lane and a portion of the rear of the lots along Jacamar Place. Flows of $Q_5=2.5$ cfs and $Q_{100}=6.8$ cfs will travel south overland and via curb and gutter towards the knuckle with Retail Row St. and **Design Point DP2**.

Review C2: Flow from the culvert is being diverted from going south within the proposed site to the east within the roadside ditch and ultimately into adjacent property owners. Please address downstream easements needed for the diverted flows and any necessary improvements beyond the roadside ditch.

Review C3: Unresolved. Please address comment above.

DP2 is a proposed 5' Type R storm inlet that is located at the very southeastern point of Rio Lane. The inlet will capture all of the runoff generated within basin A2, before combining that with the runoff captured by DP1. Together this runoff, at rates of $Q_5=3.8$ cfs and $Q_{100}=9.3$ cfs, will be expelled into the proposed full-spectrum detention facility **Pond A**.

Basin A3 is located between Rio Lane to the west and Jacamar Place to the east. Runoff flows overland and via curb and gutter from the northwest corner of the basin, down Jacamar Place, to the southeast corner at **DP3** with runoff rates of $Q_5=3.2$ cfs and $Q_{100}=6.9$ cfs. From this point flows will continue to the east via curb and gutter flow to **DP11**.

As described in the existing conditions section of this report, an existing 18" CMP culvert currently discharges onto the project site. In the developed condition, it is proposed that this culvert flow be directed to the east via 18" RCP storm sewer extension, under the proposed Tody Way intersection with Rio Lane. The roadside ditch east of the proposed Tody Way and Rio Lane intersection is to be redefined with outfall protection to protect from downstream erosion and scour.

Basin A4 covers the south side of Rio Lane, which is to be upgraded to a local roadway section with curb and gutter and sidewalk on the south side. Runoff generated by this basin ($Q_5=2.2$ cfs and $Q_{100}=4.2$ cfs) is directed to the east via curb and gutter towards the intersection with Tody Way and **Design Point DP4**. From this point flows will continue to the south via curb and gutter flow.

Basin A5 is 2.47 acres made up of 10 residential lots on the north side of Sapoya Place. Runoff ($Q_5=4.0$ cfs and $Q_{100}=9.0$ cfs) flows from northwest to southeast as side lot flow and curb and gutter flow towards the intersection with Tody Way and **Design Point DP5**.

DP5 covers flows generated by Basin A4 (DP4) and Basin A5 and directs them to the south via cross-pan and curb and gutter flow.

Basin A6 covers 1.74-acres of lots along the east side of Motmot Way. Flows generated by this basin ($Q_5=2.9$ cfs and $Q_{100}=6.4$ cfs) are directed towards the south of the basin via side-lot swale and curb and gutter towards **Design Point DP6** at the intersection with Buteos Lane. From DP6 these flows will continue to the south via cross-pan and curb and gutter flow.

Basin A7 covers an area of residential lots along Tody Way and generates flows of $Q_5=2.4$ cfs and $Q_{100}=5.3$ cfs. Flows will generally travel as curb and gutter flows towards **Design Point DP7** at the southwest end of the basin.

DP7 consists of flows generated by Basins A5 (DP5), A6 (DP6), and A7. Flows at this design point are captured by a proposed public at-grade inlet. Flows captured by this inlet will discharge to the south via proposed public storm sewer. Bypass flows will continue on to the west towards a low point at DP9.

Basin A8 covers the front of a portion of residential lots along the east side of Jacamar

Place. Flows from this basin ($Q_5=1.4$ cfs and $Q_{100}=3.0$ cfs) are directed south via curb and gutter towards **Design Point DP8**. From this point flows will continue to the east via curb and gutter flow.

Basin A9 covers the central portion of residential lots along Motmot Way and generates flows of $Q_5=4.9$ cfs and $Q_{100}=10.8$ cfs. Flows will generally travel as side-lot swale and curb and gutter flow to the east and south towards **Design Point DP9** at the southwest corner of the basin.

DP9 covers flow from Basin A8 (DP8), Basin A9 and bypass flows from DP7. Flows at this design point are captured by a proposed public sump curb inlet and will be discharged to the south via public storm system.

Basin A10 covers 3.25 acres of residential lots along Tody Way. Flows from this basin will be directed via side lot swales and curb and gutter at rates of $Q_5=5.6$ cfs and $Q_{100}=12.2$ cfs, towards a proposed public at-grade curb inlet at **Design Point DP10**. Flows captured by this inlet will discharge to the north via proposed public storm sewer. Bypass flows will continue on to the west towards a low point at DP11A.

Design Point 10A represents the combining of flows from DP7 and DP10 at a proposed storm sewer manhole. Piped flows reaching this point will continue to the west via proposed storm sewer.

Basin A11 consists of residential lots along the south side of Buteos Lane. Flows from this basin travel via side-lot swale to the north and then as curb and gutter flow to the west at rates of $Q_5=2.0$ cfs and $Q_{100}=4.3$ cfs towards a proposed low point and public sump curb inlet at **Design Point DP11**. Flows captured by this inlet will discharge to the north via proposed public storm sewer.

Design Point 11A represents the combining of flows from DP9, DP10A, and DP11 at a proposed storm sewer manhole. Piped flows reaching this point will continue to the west via proposed storm sewer.

Basin A12 covers the area of the proposed full-spectrum detention facility **Pond A**. Flows generated by this basin ($Q_5=0.2$ cfs and $Q_{100}=1.8$ cfs) will be captured by the pond in their entirety.

Design Point DP12 represents all flows reaching the full-spectrum detention facility (Basins A1-A14). Pond A will discharge at historic rates into the redefined open channel. See further detention facility discussion below.

Basin A13 covers a portion of Tract A along the east and southern boundary. Flows generated by this 2.39-acre basin are directed offsite as overland sheet flow. The majority of this basin will be regraded but will remain undeveloped as an open space tract. A 0.25-acre portion of this basin covers the rear of the lots at the southeast corner of the property. It is anticipated that this area will fall under ECM 1.7.1.C.1, as the ability to capture and treat flows generated by Basin A13 is restricted due to grading constraints. Any area outside of the criteria exclusion limit will be considered for runoff reduction. This will be further analyzed at the final drainage report stage.

Please discuss emergency path of runoff in case the sump inlet gets clogged and or whether the spread of flow will meet criteria.

Basin A14 is 0.53-acres located between the 2 northern proposed full-spectrum detention facilities, **Pond A and B**. This basin will generate runoff at rates of $Q_5=0.2$ cfs and $Q_{100}=1.4$ cfs. All of the runoff generated and passed through this basin will be directed south where it will follow historical drainage patterns, eventually joining the East Tributary of Falcon Creek.

B-group basins represent the central commercial portion of the site that will be intercepted by Pond B, ultimately discharging out to the redefined tributary open channel.

Basin OSB1 represents 2.09 acres of US-HWY 24, which acts as the northwestern boundary for the site. The runoff generated by this basin, $Q_5=9.7$ cfs and $Q_{100}=17.4$ cfs, is directed northeast via proposed curb and gutter towards a proposed sump curb inlet at **Design Point DP1**, where captured flows will continue to the southeast via proposed private storm sewer. The ultimate cross section of Highway 24 is unknown at the time of this Preliminary Drainage Report. Conversations are ongoing with CDOT regarding the timeline of improvements. The intention of this storm extension is to provide for an outlet at the associated low point.

Basin B1 is 2.15 acres at the northeast corner of the commercial area. Flows from this basin are intended to be captured and routed to a proposed storm sewer stub at the southwest corner. Final design of this onsite private storm system will be by the future lot developer. The runoff flows generated by this basin are $Q_5=9.0$ cfs and $Q_{100}=16.4$ cfs. Flows from this stub will travel to the southwest via proposed storm sewer towards **Design Point DP1A** where they combine with piped flows from DP1.

Basin B2 is located along the northern boundary of the commercial area. Flows of $Q_5=4.6$ cfs and $Q_{100}=8.5$ cfs are generated by this basin and are intended be captured and routed to a proposed storm sewer stub at the southwest corner at **Design Point 2** and routed to the southeast via private storm sewer. Final design of the connection to the stub and onsite private storm system will be by the future lot developer.

Rational Method Runoff Summary (B-group)

| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|----|-----------|------|------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| B-BASINS | | | | |
| OSB1 | 1 | 2.09 | 9.7 | 17.4 |
| B1 | | 2.15 | 9.0 | 16.4 |
| | 1A | 4.25 | 18.2 | 32.8 |
| B2 | 2 | 1.11 | 4.6 | 8.5 |
| B3 | 3 | 0.39 | 1.8 | 3.3 |
| B4 | 4 | 1.54 | 6.5 | 11.8 |
| | 4A | 7.29 | 30.0 | 54.3 |
| B5 | 5 | 2.13 | 8.9 | 16.3 |
| | 5A | 9.42 | 37.3 | 67.8 |

in general identify how the flow will be conveyed prior to the development of the individual lot as the contours do not direct the flow to the proposed stubs in all the commercial lots



Please be clear on this statement. Does it mean the private storm pipes at the southeast corner will be proposed by others? If that is the case, how will the runoff from this development be handled? Please provide a safe route for the runoff from this development.

| | | | | |
|----|----|-------|------|------|
| B6 | 6 | 1.75 | 7.3 | 13.4 |
| B7 | | 0.91 | 4.2 | 7.6 |
| | 7 | 2.67 | 11.6 | 20.9 |
| B8 | | 0.75 | 3.5 | 6.2 |
| | 8 | 3.41 | 14.9 | 27.0 |
| | 8A | 12.84 | 50.6 | 91.6 |
| B9 | | 1.42 | 0.6 | 4.3 |
| | 9 | 14.26 | 50.8 | 95.1 |

Basin B3 covers a portion of the proposed Woodmen road right-of-way at the center of the commercial area. Flows of $Q_5=1.8$ cfs and $Q_{100}=3.3$ cfs are generated by this basin, and travel via curb and gutter to the south towards a proposed at-grade curb inlet at **Design Point 3**. Captured flows continue to the east via proposed storm sewer. Bypass flows will continue to the south as curb and gutter flow towards DP7.

Basin B4 is located along the northern boundary of the commercial area, to the southwest of Basin B2. Flows of $Q_5=6.5$ cfs and $Q_{100}=11.8$ cfs are generated by this basin, and travel overland to the southeast towards **Design Point 4** and a proposed storm sewer stub. As with the previous commercial lot basins, the intention is to provide a storm sewer stub for the future lot developer to connect to.

Design Point 4A represents the combining of flows from DP1A, DP2, DP3 and DP4 at a proposed storm sewer manhole. Flows reaching this Design Point will continue to the south via proposed storm sewer.

Basin B5 is located in the central portion of the commercial area. Flows of $Q_5=8.9$ cfs and $Q_{100}=16.3$ cfs are generated by this basin. Roadway flow is proposed to be captured by sump curb inlets located close to the intersection with Retail Row Street, **Design Point 5**. As with the previous commercial lot basins, the intention is to provide a storm sewer stub, located at the southeast corner for the future lot developer to connect to.

Design Point 5A represents the combining of flows from DP4A and Basin 5 at a proposed storm sewer manhole. Flows reaching this Design Point will continue to the south via proposed storm sewer.

Basin B6 covers 1.75-acres in the central portion of the commercial area. Flows of $Q_5=7.3$ cfs and $Q_{100}=13.4$ cfs are generated by this basin. As with the previous commercial lot basins, the intention is to provide a storm sewer stub, located at the southeast corner at **Design Point 6** for the future lot developer to connect to. Piped flows will discharge to the south via proposed storm sewer.

Basin B7 covers a portion of Woodmen Road and Retail Row St. right-of-way at the center of the commercial area. Flows of $Q_5=4.2$ cfs and $Q_{100}=7.6$ cfs are generated by this basin, and travel via curb and gutter to the south and northeast towards a proposed low point and public sump curb inlet at **Design Point DP7**. Captured flows at this inlet combine with those from DP6 and continue to the south via proposed storm sewer.

Basin B8 covers the southern portion of Retail Row St. right-of-way at the center of the

commercial area, to the south of Basin B7. Flows of $Q_5=3.5$ cfs and $Q_{100}=6.2$ cfs are generated by this basin, and travel via curb and gutter to the northeast towards a proposed low point and public sump curb inlet at **Design Point DP8**. Flows captured by this inlet combine with the piped flows from DP7 and continue to the south and west via proposed storm sewer.

Design Point 8A represents the combining of piped flows from DP5A and DP8 at a proposed storm sewer manhole. Flows reaching this Design Point will continue to the south via proposed storm sewer towards the full-spectrum detention facility **Pond B**.

Basin B9 covers the area of the proposed full-spectrum detention facility Pond B. Flows generated by this basin ($Q_5=0.6$ cfs and $Q_{100}=4.3$ cfs) will be captured by the pond in their entirety.

Design Point DP9 represents all flows reaching the full-spectrum detention facility (Basins OSB1 & B1-B9). Pond B will discharge at historic rates into the redefined open channel. See further detention facility discussion below.

C-group basins represent the western commercial portion of the site that will be intercepted by Pond C, ultimately discharging out to follow historic conditions to the southeast.

Basin OSC1 represents 0.56-acres of US-HWY 24, which acts as the northwestern boundary for the site. The runoff generated by this basin, $Q_5=2.6$ cfs and $Q_{100}=4.6$ cfs, is directed northeast via proposed curb and gutter towards the proposed Woodmen Road extension and **Design Point DPC1**.

Basin C1 covers a portion of Woodmen Road right-of-way at the center of the commercial area, adjacent to Basin B3. Flows of $Q_5=1.3$ cfs and $Q_{100}=2.3$ cfs are generated by this basin, and travel via curb and gutter to the south towards a proposed at-grade public curb inlet at **Design Point DP1**.

Rational Method Runoff Summary (C-group)

| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|----|-----------|------|------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| C-BASINS | | | | |
| OSC1 | C1 | 0.56 | 2.6 | 4.6 |
| C1 | | 0.27 | 1.3 | 2.3 |
| | 1 | 0.83 | 3.8 | 6.7 |
| C2 | | 2.24 | 9.4 | 17.1 |
| | 2 | 2.52 | 13.7 | 24.8 |
| C3 | 3 | 1.32 | 5.5 | 10.1 |
| C4 | | 1.51 | 6.3 | 11.5 |
| | 4 | 5.34 | 24.0 | 43.7 |
| OSC2 | | 2.98 | 2.6 | 10.1 |
| C5 | | 0.88 | 4.1 | 7.3 |

| | | | | |
|----|----|------|------|------|
| | 5 | 3.86 | 6.3 | 16.5 |
| C6 | | 0.66 | 3.1 | 5.5 |
| | 6 | 4.52 | 10.0 | 23.8 |
| | 6A | 9.86 | 31.8 | 62.7 |

DP1 combines flows from DPC1 and Basin C1 at a proposed at-grade curb inlet. Captured flows will continue to the west via proposed storm sewer. Bypass flows will continue to the south via curb and gutter towards DP5.

Basin C2 covers 2.24-acres in the west-central portion of the commercial area. Flows of $Q_5=9.4$ cfs and $Q_{100}=17.1$ cfs are generated by this basin are intended be captured and routed to a proposed storm sewer stub at the southwest corner at **Design Point DP2**.

DP2 combines flows from DP1 and Basin C2 and continues on to the south via proposed storm sewer.

Basin C3 covers 1.32-acres in the commercial area. Runoff rates of $Q_5=5.5$ cfs and $Q_{100}=10.1$ cfs are generated by this basin and as with other commercial basins, are intended be captured and routed to a proposed storm sewer stub at the southwest corner at **Design Point DP3**.

Basin C4 is located along the western boundary of the commercial area. Runoff rates of $Q_5=6.3$ cfs and $Q_{100}=11.5$ cfs are generated by this basin as with other commercial basins, are intended be captured and routed to a proposed storm sewer stub at the southwest corner at **Design Point DP4**.

DP4 represents the combining of flows from DP2, DP3 and Basin C4 at a proposed storm sewer manhole. Flows reaching this Design Point will continue to the south via proposed storm sewer.

Basin OSC2 covers an offsite area along the western boundary of the project site, and includes a portion of U.S. Highway 24. The 2.98-acre offsite basin, will follow current conditions along the western property boundary of the site until they reach the southeast corner where they will enter Basin C5 at rates of $Q_5=2.6$ cfs and $Q_{100}=10.1$ cfs and be captured by the proposed public sump curb inlet at Design Point DP5.

Basin C5 covers a portion of Woodmen Road and Retail Row St. right-of-way to the west and south of the commercial area. Flows of $Q_5=4.1$ cfs and $Q_{100}=7.3$ cfs are generated by this basin, and travel via curb and gutter to the southwest towards a proposed lot point and public sump inlet at **Design Point DP5**. From this point flows will continue to the south via proposed storm sewer.

Basin C6 covers a portion of Retail Row St. right-of-way to the south of the commercial area. Flows of $Q_5=3.1$ cfs and $Q_{100}=5.5$ cfs are generated by this basin, and travel via curb and gutter to the southwest towards a proposed low point and public sump curb inlet at **Design Point DP6**. Flows captured by this inlet combine with the piped flows from DP5 and continue to the south via proposed storm sewer.

Design Point 6A represents the combining of flows from DP4 and DP6 at a proposed storm

sewer manhole. Piped flows reaching this Design Point will continue to the south via proposed storm sewer into the D-group basins.

D-group basins represent the southern residential portion of the site that will be intercepted by Pond C, ultimately discharging out to follow historic conditions to the southeast.

Basin D1 is located to the north of this residential portion of the development. Flows generated by this basin ($Q_5=2.0$ cfs and $Q_{100}=5.0$ cfs) travel to the southeast via side lot swale and curb and gutter towards **Design Point DP1**.

Basin D2 is located at the northwest corner of this residential portion of the development along Dovekie Drive. Flows generated by this basin ($Q_5=3.1$ cfs and $Q_{100}=7.6$ cfs) travel to the east and south via curb and gutter eventually reaching **Design Point DP2**.

DP2 combines flows from Basins D1 and D2, to be captured by a proposed public sump curb inlet at the intersection of Dovekie Drive and Becard Road. Captured flows will continue to the east via public storm sewer.

Basin D3 is located centrally within this residential portion of the development and covers the front of lots along the south side of Dovekie Drive. Flows generated by this basin ($Q_5=1.4$ cfs and $Q_{100}=3.3$ cfs) travel to the northwest via side lot swale and curb and gutter towards a low point and proposed inlet structure at **Design Point DP3**. Flows from this point continue to the west via proposed storm sewer.

Design Point 3A represents the combining of flows from DP6A(C-Basins), DP2 and DP3 at a proposed storm sewer manhole. Flows reaching this Design Point will continue to the south via proposed storm sewer.

Rational Method Runoff Summary (D-group)

| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|----|-----------|------|------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| D-BASINS | | | | |
| D1 | 1 | 1.35 | 2.0 | 5.0 |
| D2 | | 1.93 | 3.1 | 7.6 |
| | 2 | 3.28 | 4.8 | 11.9 |
| D3 | 3 | 0.91 | 1.4 | 3.3 |
| | 3A | 14.05 | 33.1 | 68.0 |
| D4 | 4 | 2.75 | 4.2 | 10.3 |
| D5 | | 0.65 | 1.3 | 2.8 |
| | 5 | 3.40 | 5.4 | 13.0 |
| D6 | 6 | 2.87 | 5.5 | 12.2 |
| D7 | | 0.70 | 2.5 | 4.7 |
| | 7 | 6.98 | 12.7 | 28.4 |
| D8 | 8 | 0.42 | 0.8 | 1.7 |
| D9 | 9 | 0.29 | 0.6 | 1.3 |

| | | | | |
|------|----|-------|------|-------|
| D10 | 10 | 1.31 | 2.7 | 6.0 |
| D11 | | 0.62 | 2.9 | 5.1 |
| | 11 | 1.93 | 5.3 | 10.6 |
| D12 | | 1.52 | 0.6 | 4.3 |
| | 12 | 25.18 | 53.8 | 115.5 |
| OSD1 | D1 | 2.94 | 1.0 | 7.3 |
| D13 | | 1.45 | 0.5 | 3.3 |
| | 13 | 4.39 | 1.4 | 9.6 |
| D14 | 14 | 0.86 | 0.3 | 2.3 |

Basin D4 is located centrally within this residential portion of the development. Flows generated by this basin ($Q_5=4.2$ cfs and $Q_{100}=10.3$ cfs) travel to the south and east via side lot swale and curb and gutter towards **Design Point 4** at the intersection of Becard Road and Hoopoe Lane. Flows from this point continue to the south via cross-pan and curb and gutter flow.

Basin D5 is located centrally within this residential portion of the development, to the south of Basin D4 along Becard Road. Flows generated by this basin ($Q_5=1.3$ cfs and $Q_{100}=2.8$ cfs) travel to the north and east via side lot swale and curb and gutter towards **Design Point DP5**, where flows combine with those from DP4 and continue to the south.

Basin D6 is 2.87 acres along Hoopoe Lane at the eastern boundary. Flows of $Q_5=5.5$ cfs and $Q_{100}=12.2$ cfs travel to the west and south as curb and gutter flow towards a low point and proposed sump curb inlet at **Design Point DP6**.

Basin D7 covers the front of a portion of residential lots at the south of this residential area along Hoopoe Lane. Flows of $Q_5=2.5$ cfs and $Q_{100}=4.7$ cfs travel to the east and south as curb and gutter flow, combining with street flows from DP5 towards a proposed sump curb inlet at **Design Point DP7**. Captured flows combine with those from DP6 and continue via proposed storm sewer to the west.

Basin D8 covers the rear of a portion of residential lots directly south of Basin D5. Flows of $Q_5=0.8$ cfs and $Q_{100}=1.7$ cfs travel to the east and south towards **Design Point 8** and the adjacent detention facility. Final grading within this basin will be established at the final drainage report stage to ensure all flows generated by this basin are directed to the detention facility.

Basin D9 covers the rear of a portion of residential lots directly west of Basin D7. Flows of $Q_5=0.6$ cfs and $Q_{100}=1.3$ cfs travel to the west and south towards **Design Point DP9** and the adjacent detention facility. Final grading within this basin will be established at the final drainage report stage to ensure all flows generated by this basin are directed to the detention facility.

Basin D10 covers the southern portion along Dovekie Drive at the western boundary. Flows of $Q_5=2.7$ cfs and $Q_{100}=6.0$ cfs generated by this basin will be directed east via side lot swale before traveling south via curb and gutter towards a low point and proposed inlet structure at **Design Point DP10**.

Basin D11 covers the southern portion of Dovekie Drive adjacent to Basin D10. Flows of $Q_5=2.9$ cfs and $Q_{100}=5.1$ cfs generated by this basin will travel south via curb and gutter towards a low point and proposed inlet structure at **Design Point DP11**. Captured flows will combine with those from DP10 and discharge directly into the proposed detention facility.

Basin D12 covers the area of the proposed full-spectrum detention facility **Pond C**. Flows generated by this basin ($Q_5=0.6$ cfs and $Q_{100}=4.3$ cfs) will be captured by the pond in their entirety.

Design Point D12 represents all flows reaching the full-spectrum detention facility (Basins OSC1-OSC2, C1-C6 & D1-D12). **Pond C** will discharge at historic rates to the south to follow historic drainage patterns. See further detention facility discussion below.

Basin OSD1 is located along the eastern side of the upper half of Basin D13. This basin consists almost entirely of native grasses and vegetation, aside from a single small shed. The flows within this basin will flow to the southwest where they will naturally gather and channel along the westerly edge of Basin D13 towards **Design Point DPD1**. The runoff rates entering Basin D13 at DPD2 are $Q_5=1.0$ cfs and $Q_{100}=7.3$ cfs.

Basin D13 covers Tract G along the eastern boundary of this residential area. Flows generated by this 1.45-acre basin are directed offsite as overland sheet flow. The majority of this basin will be regraded but will remain undeveloped as an open space tract. A 0.04-acre portion of this basin covers the rear of the lots at the northern corner of the basin. It is anticipated that this area will fall under ECM 1.7.1.C.1. as the ability to capture and treat flows generated by Basin A15 is restricted due to grading constraints. Any area outside of the criteria exclusion limit will be considered for runoff reduction. This will be further analyzed at the final drainage report stage.

Basin D14 covers a section of open space area along the southern boundary of this residential area. While this area will be regraded, the area will remain undeveloped. Flows generated by this 0.86-acre basin are directed offsite. It is anticipated that this area will fall under ECM 1.7.1.C.1. as the ability to capture and treat flows generated by Basin A15 is restricted due to grading constraints. Any area outside of the criteria exclusion limit will be considered for runoff reduction. This will be further analyzed at the final drainage report stage.

Please compare the runoff at all design points (DPs) between the existing and proposed conditions as they leave the site.

9.0 PROPOSED FULL-SPECTRUM DETENTION FACILITIES

As previously mentioned, three separate full-spectrum Extended Detention Basin facilities are proposed with this development. Further detailed design of these detention facilities will be coordinated with the CLOMR study and addressed at the Final Drainage Report stage.

Pond A, a private 1.91 ac-ft full-spectrum Extended Detention Basin is proposed in the southwestern corner of the A-basin neighborhood, to intercept and treat flows from the neighborhood area and discharge at historic rates into the adjacent redefined East Tributary. In accordance with El Paso County criteria, an outlet structure with a

please discuss for each of the ponds the emergency spillways and their overflow path

permanent micropool will release flows at or slightly below historic rates. Pond design will be finalized with the final drainage report.

Pond B, is a proposed private 2.25 ac-ft full-spectrum Extended Detention Basin, designed to intercept the flows generated by the B-basin commercial region of the site, treat and discharge at historic rates into the adjacent redefined East Tributary. As with Pond A, in accordance with El Paso County criteria, an outlet structure with permanent micropool will release flows at or slightly below historic rates. Pond design will be finalized with the final drainage report.

Pond C, is a proposed private 2.81 ac-ft full-spectrum Extended Detention Basin intended to intercept the flows generated by both the C and D-basin areas of the site. As with both other ponds, in accordance with El Paso County criteria, an outlet structure with permanent micropool will release flows at or slightly below historic rates. Pond design will be finalized with the final drainage report. The concentrated piped outflow from the detention facility will discharge onto a proposed grassed berm level spreader prior to discharge on to the adjacent properties to the south. The intention of this level spreader is to provide for stabilized conveyance at the historic level.

Maintenance access will be provided to each of the ponds. Private maintenance agreements and O&M manuals will be established for all 3 ponds prior to Final Plat.

10.0 FOUR-STEP PROCESS

In conformance with the Four-Step Process, outlined in the DCM, Volume 2, the site development design is focused on reducing runoff volumes, treating the water quality capture volume, and creating stabilized drainage ways. Methods will be discussed further in the Final Drainage Report.

Unresolved from Submittal 2: 4-step process must be included in PDR as well

11.0 CONDITIONAL LETTER OF MAP REVISION (CLOMR)

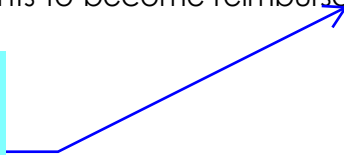
A Conditional Letter of Map Revision (CLOMR) is currently in the design stage for the Falcon Creek East Tributary reach that bisects the site. Excerpts from the CLOMR report are included in the appendix, and a copy of the full report has been submitted as part of this Preliminary Plan application.

12.0 DRAINAGE/BRIDGE FEES

Design, phasing, responsibility for and maintenance of any proposed improvements will be discussed in the final drainage report(s) as development of the site proceeds. Fees will be assessed and paid according to the current rates at the time of platting.

The Falcon DBPS – Fee Development categorizes improvements into Developer Costs, County Costs and Metro District Costs. Items identified as Developer Costs (those incurred by the Developer) are currently eligible for reimbursement. Reach RET100 is identified as a County cost, and as such the developer intends to amend the Falcon DBPS to allow for the costs of the 1,000-lf of RET100 improvements to become reimbursable by the process outlined in County criteria.

Please be aware that that staff may not be in support of the box culvert being fully reimbursable. It may be limited to the current estimated cost of the improvements identified in the DBPS.



13.0 CONCLUSIONS

The Commons at Falcon Field project has been designed in accordance with El Paso County criteria. The full-spectrum detention facilities have been designed to limit the release of storm runoff to historic flows. This development will not negatively impact the downstream facilities.

A small portion of the site will remain in the 100-year floodplain after grading is complete, and will be addressed further at the Final Drainage Report stage and by the CLOMR study currently underway. Upon completion, a LOMR will be submitted to FEMA after construction to revise the FIRM map. Future buildings will not be constructed in the floodplain.

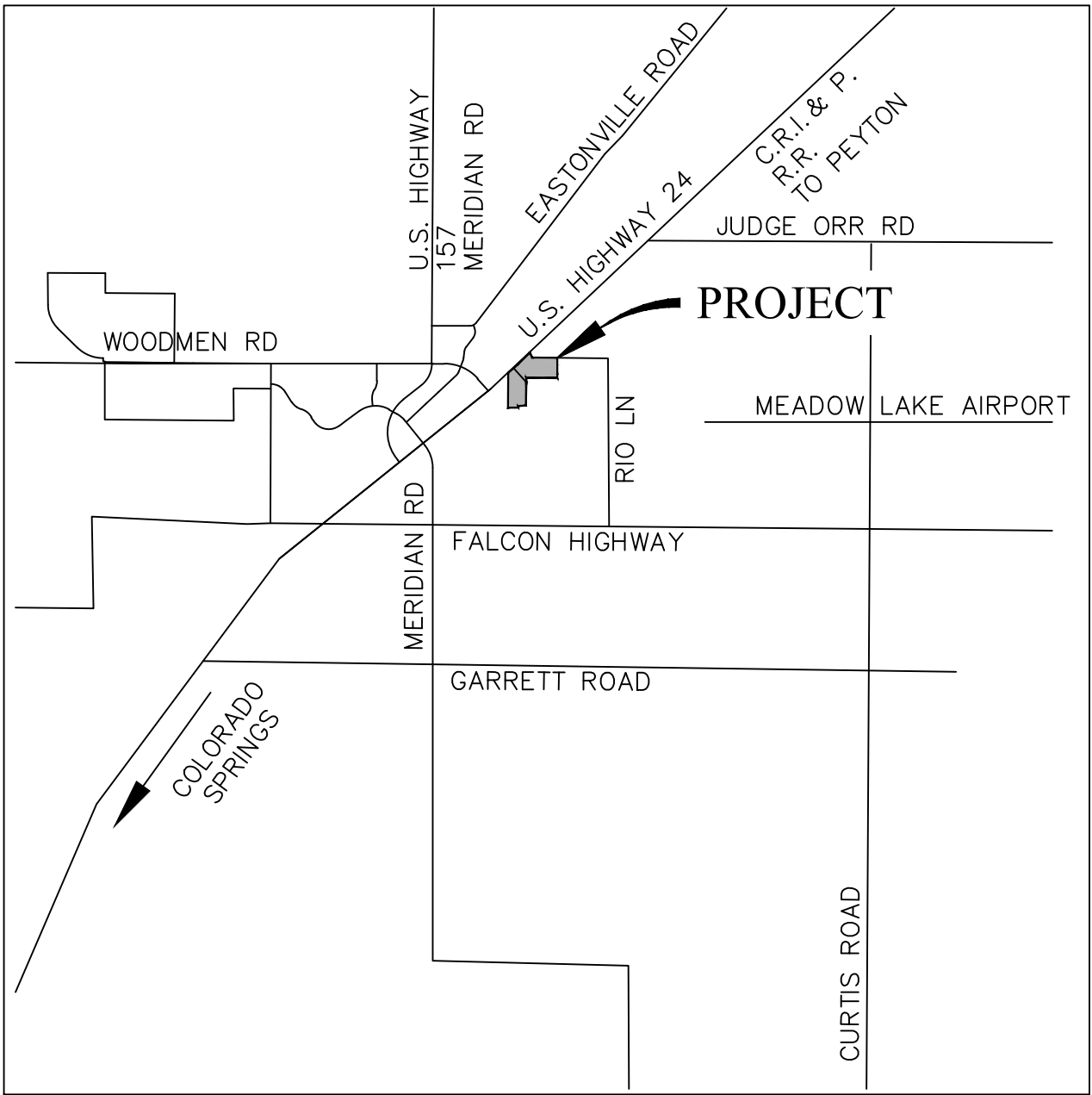
14.0 REFERENCES

The sources of information used in the development of this study are listed below:

1. City of Colorado Springs/El Paso County Drainage Criteria Manual, May 2014.
2. Urban Storm Drainage Criteria Manuals, Urban Drainage and Flood Control District. June 2001, Revised April 2008.
3. Natural Resources Conservation Service (NRCS) Web Soil Survey
4. Federal Emergency Management Agency, Flood Insurance Rate Map, El Paso County, Colorado and Unincorporated Areas, Map Numbers 8041C0553G & 8041C0561G, Effective Date December 7, 2018.
6. EL Paso County Board Resolution No 15-042: El Paso County adoption of Chapter 6 and Section 3.2.1, Chapter 13 of the City of Colorado Springs Drainage Criteria Manual, May 2014.
7. Falcon Drainage Basin Planning Study. Prepared by Matrix Design Group, September 2015.

Appendix

Vicinity Map



Vicinity Map
Not to scale



THE COMMONS AT FALCON FIELD
EL PASO COUNTY, CO
VICINITY MAP

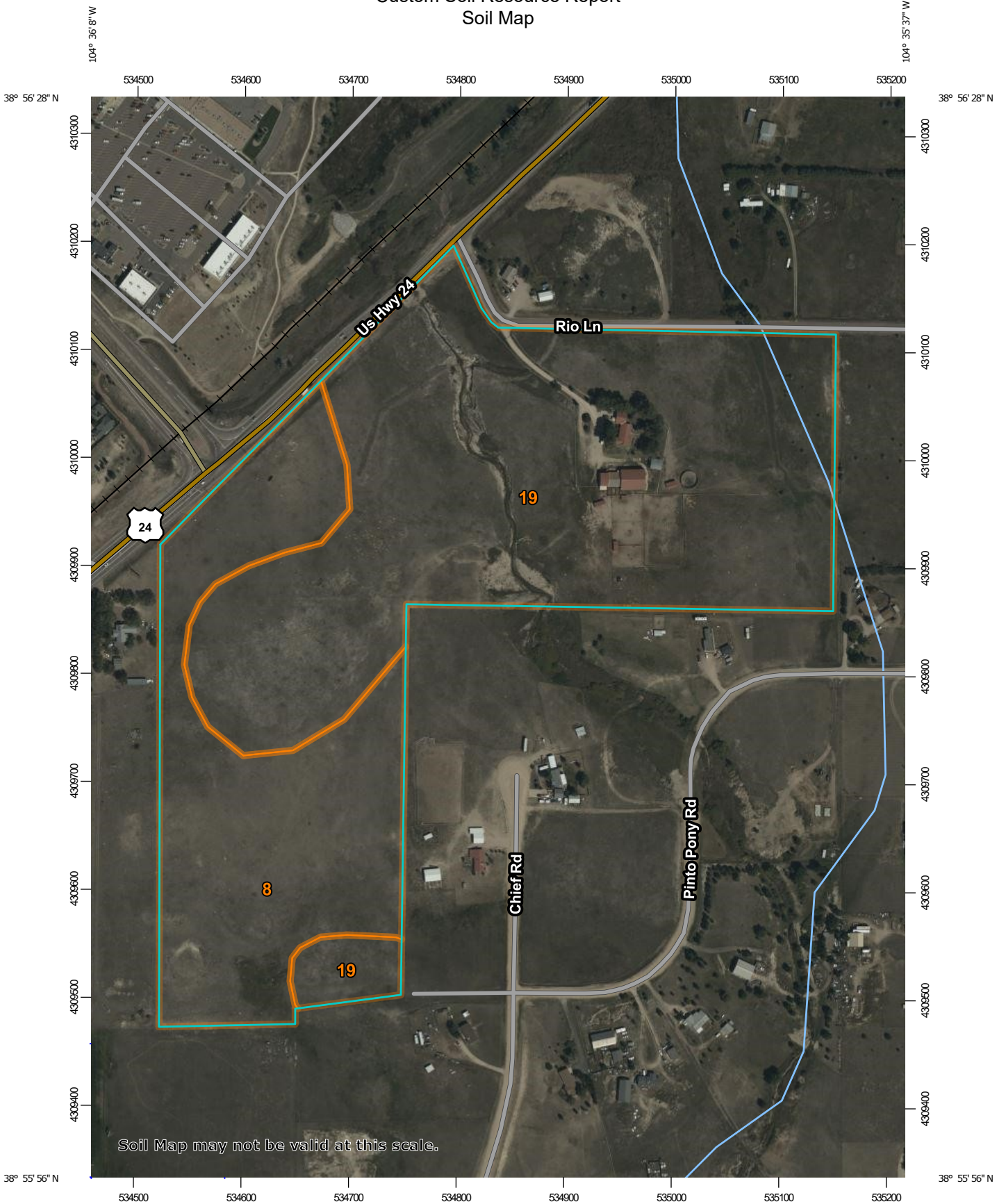
Drexel, Barrell & Co.
Engineers • Surveyors

DATE:
JOB NO:
21604-00CSCV

DWG. NO.
VMAP
SHEET 1 OF 1

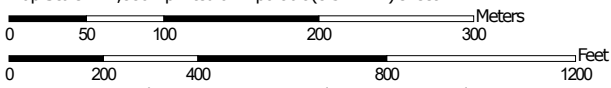
Soils Map

Custom Soil Resource Report Soil Map



Soil Map may not be valid at this scale.

Map Scale: 1:4,880 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)


Soils


 Soil Map Unit Polygons


 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit


 Clay Spot


 Closed Depression

 Gravel Pit

 Gravelly Spot


 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water


 Perennial Water

 Rock Outcrop


 Saline Spot

 Sandy Spot

 Severely Eroded Spot


 Sinkhole

 Slide or Slip


 Sodic Spot


 Spoil Area

 Stony Spot


 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals


Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 19, Aug 31, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 11, 2018—Oct 20, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
|------------------------------------|--|--------------|----------------|
| 8 | Blakeland loamy sand, 1 to 9 percent slopes | 18.8 | 32.8% |
| 19 | Columbine gravelly sandy loam, 0 to 3 percent slopes | 38.6 | 67.2% |
| Totals for Area of Interest | | 57.4 | 100.0% |

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

Custom Soil Resource Report

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

8—Blakeland loamy sand, 1 to 9 percent slopes

Map Unit Setting

National map unit symbol: 369v
Elevation: 4,600 to 5,800 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 46 to 48 degrees F
Frost-free period: 125 to 145 days
Farmland classification: Not prime farmland

Map Unit Composition

Blakeland and similar soils: 98 percent
Minor components: 2 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Blakeland

Setting

Landform: Hills, flats
Landform position (three-dimensional): Side slope, talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium derived from sedimentary rock and/or eolian deposits derived from sedimentary rock

Typical profile

A - 0 to 11 inches: loamy sand
AC - 11 to 27 inches: loamy sand
C - 27 to 60 inches: sand

Properties and qualities

Slope: 1 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 5 percent
Available water supply, 0 to 60 inches: Low (about 4.5 inches)

Interpretive groups

Land capability classification (irrigated): 3e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: A
Ecological site: R049XB210CO - Sandy Foothill
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit: 1 percent

Custom Soil Resource Report

Hydric soil rating: No

Pleasant

Percent of map unit: 1 percent

Landform: Depressions

Hydric soil rating: Yes

19—Columbine gravelly sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 367p

Elevation: 6,500 to 7,300 feet

Mean annual precipitation: 14 to 16 inches

Mean annual air temperature: 46 to 50 degrees F

Frost-free period: 125 to 145 days

Farmland classification: Not prime farmland

Map Unit Composition

Columbine and similar soils: 97 percent

Minor components: 3 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Columbine

Setting

Landform: Flood plains, fan terraces, fans

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Alluvium

Typical profile

A - 0 to 14 inches: gravelly sandy loam

C - 14 to 60 inches: very gravelly loamy sand

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Very low (about 2.5 inches)

Interpretive groups

Land capability classification (irrigated): 4e

Land capability classification (nonirrigated): 6e

Hydrologic Soil Group: A

Ecological site: R049XY214CO - Gravelly Foothill

Custom Soil Resource Report

Hydric soil rating: No

Minor Components

Fluvaquentic haplaquolls

Percent of map unit: 1 percent

Landform: Swales

Hydric soil rating: Yes

Other soils

Percent of map unit: 1 percent

Hydric soil rating: No

Pleasant

Percent of map unit: 1 percent

Landform: Depressions

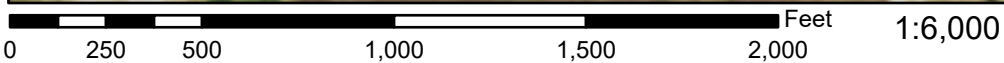
Hydric soil rating: Yes

Floodplain Map

National Flood Hazard Layer FIRMMette



104°36'16"W 38°56'26"N



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

| | | |
|-----------------------------|--|--|
| SPECIAL FLOOD HAZARD AREAS | | Without Base Flood Elevation (BFE) <i>Zone A, V, A99</i> |
| | | With BFE or Depth <i>Zone AE, AO, AH, VE, AR</i> |
| | | Regulatory Floodway |
| OTHER AREAS OF FLOOD HAZARD | | 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile <i>Zone X</i> |
| | | Future Conditions 1% Annual Chance Flood Hazard <i>Zone X</i> |
| | | Area with Reduced Flood Risk due to Levee. See Notes. <i>Zone X</i> |
| | | Area with Flood Risk due to Levee <i>Zone D</i> |
| OTHER AREAS | | NO SCREEN Area of Minimal Flood Hazard <i>Zone X</i> |
| | | Effective LOMRs |
| GENERAL STRUCTURES | | Area of Undetermined Flood Hazard <i>Zone D</i> |
| | | Channel, Culvert, or Storm Sewer |
| | | Levee, Dike, or Floodwall |
| OTHER FEATURES | | 20.2 Cross Sections with 1% Annual Chance Water Surface Elevation 17.5 |
| | | Coastal Transect |
| | | Base Flood Elevation Line (BFE) |
| | | Limit of Study |
| | | Jurisdiction Boundary |
| MAP PANELS | | Coastal Transect Baseline |
| | | Profile Baseline |
| | | Hydrographic Feature |
| | | Digital Data Available |
| | | No Digital Data Available |
| | | Unmapped |



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 4/12/2022 at 2:02 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Hydrology Calculations

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
PROJECT NO: 21604-00
DESIGN BY: KGV
REV. BY: TDM
AGENCY: El Paso County
REPORT TYPE: Preliminary
DATE: 3/17/2024



| | C2* | C5* | C10* | C100* | % IMPERV |
|-----------------|-----|------|------|-------|----------|
| Pasture/Meadow | | 0.08 | | 0.35 | 0 |
| Roofs | | 0.73 | | 0.81 | 90 |
| Lawns | | 0.08 | | 0.35 | 0 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.59 | | 0.70 | 80 |

EXISTING CONIDTION

| SUB-BASIN | SURFACE DESIGNATION | AREA ACRE | COMPOSITE RUNOFF COEFFICIENTS | | | | % IMPERV |
|-----------|---------------------|--------------|-------------------------------|------|-----|------|----------|
| | | | C2 | C5 | C10 | C100 | |
| OS1 | Pasture/Meadow | 0.67 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.67 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | | 0.49 | | 0.66 | 50% |
| | TOTAL OS1 | | 1.34 | | | | |
| OS2 | Pasture/Meadow | 0.30 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.30 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | | 0.49 | | 0.66 | 50% |
| | TOTAL OS2 | | 0.60 | | | | |
| OS3 | Pasture/Meadow | 2.56 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.04 | | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | | 0.09 | | 0.36 | 1% |
| | TOTAL OS3 | | 2.56 | | | | |
| OS4 | Pasture/Meadow | 2.90 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.10 | | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.29 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | | 0.17 | | 0.42 | 12% |
| | TOTAL OS4 | | 3.29 | | | | |
| OS5 | Pasture/Meadow | 5.22 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.05 | | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.23 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.59 | | 0.70 | 80 |
| | | | | | | | |

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| | C2* | C5* | C10* | C100* | % IMPERV |
|-----------------|-----|------|------|-------|----------|
| Pasture/Meadow | | 0.08 | | 0.35 | 0 |
| Roofs | | 0.73 | | 0.81 | 90 |
| Lawns | | 0.08 | | 0.35 | 0 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.59 | | 0.70 | 80 |

| | WEIGHTED AVERAGE | | | | | |
|------------------|------------------|-------|------|--|------|-----|
| TOTAL OS5 | 5.50 | | 0.12 | | 0.38 | 5% |
| OS6 | | | | | | |
| | Pasture/Meadow | 0.39 | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.52 | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | 0.55 | | 0.70 | 57% |
| TOTAL OS6 | 0.91 | | | | | |
| E1 | | | | | | |
| | Pasture/Meadow | 13.74 | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.00 | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.11 | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | 0.08 | | 0.35 | 1% |
| TOTAL E1 | 13.85 | | | | | |
| E2 | | | | | | |
| | Pasture/Meadow | 12.88 | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.00 | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | 0.08 | | 0.35 | 0% |
| TOTAL E2 | 12.88 | | | | | |
| E3 | | | | | | |
| | Pasture/Meadow | 13.11 | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.00 | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | 0.08 | | 0.35 | 0% |
| TOTAL E3 | 13.11 | | | | | |
| E4 | | | | | | |
| | Pasture/Meadow | 1.57 | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | 0.08 | | 0.35 | 0 |

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| | C2* | C5* | C10* | C100* | % IMPERV |
|-----------------|-----|------|------|-------|----------|
| Pasture/Meadow | | 0.08 | | 0.35 | 0 |
| Roofs | | 0.73 | | 0.81 | 90 |
| Lawns | | 0.08 | | 0.35 | 0 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.59 | | 0.70 | 80 |

| | | | | | | | |
|-----------------|------------------|-------|--|------|--|------|-----|
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL E4 | | 1.57 | | | | | |
| E5 | Pasture/Meadow | 5.49 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.25 | | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.17 | | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | | 0.12 | | 0.38 | 6% |
| TOTAL E5 | | 5.91 | | | | | |
| E6 | Pasture/Meadow | 10.37 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | | 0.73 | | 0.81 | 90 |
| | Lawns | 0.00 | | 0.08 | | 0.35 | 0 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.59 | | 0.70 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL E6 | | 10.37 | | | | | |

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**RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF
 EXISTING TIME OF CONCENTRATION**

| SUB-BASIN DATA | | | | | | | INITIAL/OVERLAND TIME (t _i) | | | TRAVEL TIME (t _t) | | | | TIME OF CONCENTRATION | | FINAL t _c |
|----------------|------------|----------------|------------------|-------|------|------|---|-------|----------------|-------------------------------|-------|------|----------------|-----------------------|----------------|----------------------|
| BASIN | DESIGN PT: | C ₅ | C ₁₀₀ | AREA | COMP | | LENGTH | SLOPE | t _i | LENGTH | SLOPE | VEL. | t _t | COMP. | MINIMUM | t _c |
| | | | | Ac | | | Ft | % | Min | Ft | % | FPS | Min | t _c | t _c | Min |
| OS1 | A | 0.49 | 0.66 | 1.34 | 0.66 | 0.88 | 30 | 2.0 | 5.0 | | | | | 5.0 | 5.0 | 5.0 |
| E1 | | 0.08 | 0.35 | 13.85 | 1.16 | 4.88 | 300 | 3.0 | 22.8 | 530 | 3.0 | 3.5 | 2.5 | 25.3 | 5.0 | 25.3 |
| DPA+E1 | B | 0.12 | 0.38 | 15.19 | | | From E1 | | | | | | | 25.3 | 5.0 | 25.3 |
| OS2 | C | 0.49 | 0.66 | 0.60 | 0.29 | 0.39 | 30 | 1.0 | 6.2 | | | | 0.0 | 6.2 | 5.0 | 6.2 |
| OS3 | D | 0.09 | 0.36 | 2.56 | 0.23 | 0.91 | 75 | 2.1 | 12.8 | | | | 1.0 | 13.8 | 5.0 | 13.8 |
| E2 | | 0.08 | 0.35 | 12.88 | 1.03 | 4.51 | 300 | 2.3 | 24.9 | 1360 | 3.5 | 4.0 | 5.7 | 30.6 | 5.0 | 30.6 |
| DPC+DPD+E2 | E | 0.10 | 0.36 | 16.04 | | | From E2 | | | | | | | 30.6 | 5.0 | 30.6 |
| E3 | F | 0.08 | 0.35 | 13.11 | 1.05 | 4.59 | 300 | 2.3 | 24.9 | 1120 | 4.2 | 5.0 | 3.7 | 28.6 | 5.0 | 28.6 |
| OS4 | G | 0.17 | 0.42 | 3.29 | 0.57 | 1.38 | 75 | 3.9 | 9.5 | | | | | 9.5 | 5.0 | 9.5 |
| E4 | | 0.08 | 0.35 | 1.57 | 0.13 | 0.55 | 300 | 2.7 | 23.8 | 250 | 4.0 | 5.0 | 0.8 | 24.7 | 5.0 | 24.7 |
| DPG+E4 | H | 0.14 | 0.40 | 4.86 | | | From E4 | | | | | | | 24.7 | 5.0 | 24.7 |
| E5 | I | 0.12 | 0.38 | 5.91 | 0.72 | 2.24 | 100 | 2.0 | 14.5 | 550 | 2.2 | 1.8 | 5.1 | 19.6 | 5.0 | 19.6 |
| OS5 | J | 0.12 | 0.38 | 5.50 | 0.66 | 2.09 | 75 | 2.0 | 12.6 | | | | | 12.6 | 5.0 | 12.6 |
| OS6 | L | 0.55 | 0.70 | 0.91 | 0.50 | 0.63 | 30 | 2.0 | 4.5 | | | | | 4.5 | 5.0 | 5.0 |
| E6 | | 0.08 | 0.35 | 10.37 | 0.83 | 3.63 | 300 | 2.3 | 24.9 | 1080 | 1.5 | 1.2 | 15.0 | 39.9 | 5.0 | 39.9 |
| DPJ+DPL+E6 | M | 0.12 | 0.38 | 16.78 | | | From E6 | | | | | | | 39.9 | 5.0 | 39.9 |

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RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

EXISTING **RUNOFF** **5 YR** **STORM** **P1=** **1.50**

| BASIN (S) | DESIGN POINT | AREA (AC) | DIRECT RUNOFF | | C * A | I (IN/HR) | Q (CFS) |
|---------------|--------------|-----------|---------------|----------------------|-------|-----------|---------|
| | | | RUNOFF COEFF | t _c (MIN) | | | |
| OS1 | A | 1.34 | 0.49 | 5.0 | 0.66 | 5.17 | 3.4 |
| E1 | | 13.85 | 0.08 | 25.3 | 1.16 | 2.73 | 3.2 |
| RET090 (DBPS) | | | | | | | 36.0 |
| DPA+E1+RET090 | B | 15.19 | 0.12 | 25.3 | 1.82 | 2.73 | 41.0 |
| OS2 | C | 0.60 | 0.49 | 6.2 | 0.29 | 4.83 | 1.4 |
| OS3 | D | 2.56 | 0.09 | 13.8 | 0.23 | 3.65 | 0.8 |
| E2 | | 12.88 | 0.08 | 30.6 | 1.03 | 2.45 | 2.5 |
| DPC+DPD+E2 | E | 16.04 | 0.10 | 30.6 | 1.55 | 2.45 | 3.8 |
| E3 | F | 13.11 | 0.08 | 28.6 | 1.05 | 2.55 | 2.7 |
| OS4 | G | 3.29 | 0.17 | 9.5 | 0.57 | 4.20 | 2.4 |
| E4 | | 1.57 | 0.08 | 24.7 | 0.13 | 2.78 | 0.3 |
| DPG+E4 | H | 4.86 | 0.14 | 24.7 | 0.69 | 2.78 | 1.9 |
| E5 | I | 5.91 | 0.12 | 19.6 | 0.72 | 3.12 | 2.2 |
| OS5 | J | 5.50 | 0.12 | 12.6 | 0.66 | 3.78 | 6.6 |
| OS6 | L | 0.91 | 0.55 | 5.0 | 0.50 | 5.17 | 2.6 |
| E6 | | 10.37 | 0.08 | 39.9 | 0.83 | 2.05 | 1.7 |
| DPJ+DPL+E6 | M | 16.78 | 0.12 | 39.9 | 1.99 | 2.05 | 4.1 |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
 PROJECT NO: 21604-00
 DESIGN BY: KGV
 REV. BY: TDM
 AGENCY: El Paso County
 REPORT TYPE: Preliminary
 DATE: 3/17/2024



Drexel, Barrell & Co.

RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

EXISTING RUNOFF 100 YR STORM P1= **2.52**

| BASIN (S) | DESIGN POINT | AREA (AC) | DIRECT RUNOFF | | C * A | I (IN/HR) | Q (CFS) |
|---------------|--------------|-----------|---------------|----------------------|-------|-----------|---------|
| | | | RUNOFF COEFF | t _c (MIN) | | | |
| OS1 | A | 1.34 | 0.66 | 5.0 | 0.88 | 8.68 | 7.6 |
| E1 | | 13.85 | 0.35 | 25.3 | 4.88 | 4.59 | 22.4 |
| RET090 (DBPS) | | | | | | | 320.0 |
| DPA+E1+RET090 | B | 15.19 | 0.38 | 25.3 | 5.76 | 4.59 | 346.4 |
| OS2 | C | 0.60 | 0.66 | 6.2 | 0.39 | 8.12 | 3.2 |
| OS3 | D | 2.56 | 0.36 | 13.8 | 0.91 | 6.13 | 5.6 |
| E2 | | 12.88 | 0.35 | 30.6 | 4.51 | 4.12 | 18.6 |
| DPC+DPD+E2 | E | 16.04 | 0.36 | 30.6 | 5.81 | 4.12 | 23.9 |
| E3 | F | 13.11 | 0.35 | 28.6 | 4.59 | 4.28 | 19.6 |
| OS4 | G | 3.29 | 0.42 | 9.5 | 1.38 | 7.05 | 9.7 |
| E4 | | 1.57 | 0.35 | 24.7 | 0.55 | 4.66 | 2.6 |
| DPG+E4 | H | 4.86 | 0.40 | 24.7 | 1.93 | 4.66 | 9.0 |
| E5 | I | 5.91 | 0.38 | 19.6 | 2.24 | 5.23 | 11.7 |
| OS5 | J | 5.50 | 0.38 | 12.6 | 2.09 | 6.35 | 6.6 |
| OS6 | L | 0.91 | 0.70 | 5.0 | 0.63 | 8.68 | 5.5 |
| E6 | | 10.37 | 0.35 | 39.9 | 3.63 | 3.44 | 12.5 |
| DPJ+DPL+E6 | M | 16.78 | 0.38 | 39.9 | 6.35 | 3.44 | 21.9 |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field - Overlot
PROJECT NO: 21604-00
DESIGN BY: KGV
REV. BY: TDM
AGENCY: El Paso County
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Drexel, Barrell & Co.

| | C2* | C5* | C10* | C100* | % IMPERV |
|-----------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Roofs | | 0.73 | | 0.81 | 90 |
| Drive and Walks | | 0.90 | | 0.96 | 100 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

DEVELOPED CONIDTION

| SUB-BASIN | SURFACE DESIGNATION | AREA ACRE | COMPOSITE RUNOFF COEFFICIENTS | | | | % IMPERV |
|------------------|---------------------|--------------|-------------------------------|------|-----|------|----------|
| | | | C2 | C5 | C10 | C100 | |
| A-BASINS | | | | | | | |
| A1 | Open Space | 17.73 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | | 0.73 | | 0.81 | 90 |
| | Drive and Walks | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL A1 | | 17.73 | | | | | |
| A2 | Open Space | 2.45 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | | 0.73 | | 0.81 | 90 |
| | Drive and Walks | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL A2 | | 2.45 | | | | | |
| B1 | Open Space | 12.47 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | | 0.73 | | 0.81 | 90 |
| | Drive and Walks | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL B1 | | 12.47 | | | | | |
| OB1 | Open Space | 0.67 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | | 0.73 | | 0.81 | 90 |
| | Drive and Walks | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Paved | 0.67 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.49 | | 0.66 | 50% |
| TOTAL OB1 | | 1.34 | | | | | |
| OB2 | Open Space | 0.30 | | 0.08 | | 0.35 | 0 |
| | Roofs | 0.00 | | 0.73 | | 0.81 | 90 |
| | Drive and Walks | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Paved | 0.30 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.49 | | 0.66 | 50% |
| TOTAL OB2 | | 0.60 | | | | | |

PROJECT INFORMATION

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Drexel, Barrell & Co.

| | C2* | C5* | C10* | C100* | % IMPERV |
|-----------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Roofs | | 0.73 | | 0.81 | 90 |
| Drive and Walks | | 0.90 | | 0.96 | 100 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | |
|-----------------|------------------|-------|------|------|-----|
| C1 | Open Space | 6.29 | 0.08 | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | 0.81 | 90 |
| | Drive and Walks | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 |
| | WEIGHTED AVERAGE | | 0.08 | 0.35 | 0% |
| TOTAL C1 | 6.29 | | | | |
| OC | Open Space | 2.55 | 0.08 | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | 0.81 | 90 |
| | Drive and Walks | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 |
| | WEIGHTED AVERAGE | | 0.08 | 0.35 | 0% |
| TOTAL OC | 2.55 | | | | |
| D1 | Open Space | 15.99 | 0.08 | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | 0.81 | 90 |
| | Drive and Walks | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 |
| | WEIGHTED AVERAGE | | 0.08 | 0.35 | 0% |
| TOTAL D1 | 15.99 | | | | |
| D2 | Open Space | 2.81 | 0.08 | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | 0.81 | 90 |
| | Drive and Walks | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 |
| | WEIGHTED AVERAGE | | 0.08 | 0.35 | 0% |
| TOTAL D2 | 2.81 | | | | |
| OD | Open Space | 2.70 | 0.08 | 0.35 | 0 |
| | Roofs | 0.00 | 0.73 | 0.81 | 90 |
| | Drive and Walks | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 |
| | WEIGHTED AVERAGE | | 0.08 | 0.35 | 0% |
| TOTAL OD | 2.70 | | | | |

PROJECT INFORMATION

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 PROJECT NO: 21604-00
 DESIGN BY: KGV
 REV. BY: TDM
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RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF DEVELOPED TIME OF CONCENTRATION

| SUB-BASIN DATA | | | | | INITIAL/OVERLAND TIME (t _i) | | | TRAVEL TIME (t _t) | | | | TIME OF CONCENTRATION | | FINAL t _c |
|----------------|------------|----------------|------------------|-------|---|-------|----------------|-------------------------------|-------|------|----------------|-----------------------|----------------|----------------------|
| BASIN | DESIGN PT. | C _s | C ₁₀₀ | AREA | LENGTH | SLOPE | t _i | LENGTH | SLOPE | VEL. | t _t | COMP. | MINIMUM | |
| | | | | Ac | Ft | % | Min | Ft | % | FPS | Min | t _c | t _c | Min |
| A1 | 1 | 0.08 | 0.35 | 17.73 | 100 | 1.0 | 18.7 | 1250 | 1.0 | 5.0 | 4.2 | 22.9 | 5.0 | 22.9 |
| A2 | | 0.08 | 0.35 | 2.45 | 100 | 1.0 | 18.7 | 150 | 25.0 | 10.0 | 0.3 | 19.0 | 5.0 | 19.0 |
| B1 | | 0.08 | 0.35 | 12.47 | 100 | 1.0 | 18.7 | 750 | 2.0 | 7.0 | 1.8 | 20.5 | 5.0 | 20.5 |
| OB1 | O1 | 0.49 | 0.66 | 1.34 | 30 | 2.0 | 4.9 | | | | | 4.9 | 5.0 | 5.0 |
| OB2 | O2 | 0.49 | 0.66 | 0.60 | 30 | 1.0 | 6.1 | | | | | 6.1 | 5.0 | 6.1 |
| B1+O1+O2 | 2 | 0.14 | 0.39 | 14.41 | From Basin B1 | | | 20.5 | | | | 20.5 | 5.0 | 20.5 |
| C1 | | 0.08 | 0.35 | 6.29 | 100 | 1.0 | 18.7 | 860 | 1.5 | 6.0 | 2.4 | 21.1 | 5.0 | 21.1 |
| OC | | 0.08 | 0.35 | 2.55 | 100 | 1.0 | 18.7 | 756 | 1.5 | 6.0 | 2.1 | 20.8 | 5.0 | 20.8 |
| C1+OC | 3 | 0.08 | 0.35 | 8.85 | From Basin C1 | | | 21.1 | | | | 21.1 | 5.0 | 21.1 |
| D1 | 4 | 0.08 | 0.35 | 15.99 | 100 | 1.0 | 18.7 | 1350 | 1.5 | 6.0 | 3.8 | 22.5 | 5.0 | 22.5 |
| D2 | | 0.08 | 0.35 | 2.81 | 100 | 1.0 | 18.7 | 150 | 0.5 | 3.8 | 0.7 | 19.4 | 5.0 | 19.4 |
| OD | | 0.08 | 0.35 | 2.70 | 100 | 1.0 | 18.7 | 660 | 1.5 | 6.0 | 1.8 | 20.5 | 5.0 | 20.5 |
| D2+OD | 5 | 0.08 | 0.35 | 5.51 | From Basin D2 | | | 19.4 | | | | 19.4 | 5.0 | 19.4 |

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 REV. BY: TDM
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RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

| DEVELOPED | | RUNOFF | | 5 YR | | STORM | | P1= | |
|-----------|--------------|-----------|--------------|----------------------|-------|-----------|---------|------|--|
| | | | | DIRECT RUNOFF | | | | 1.50 | |
| BASIN (S) | DESIGN POINT | AREA (AC) | RUNOFF COEFF | t _c (MIN) | C * A | I (IN/HR) | Q (CFS) | | |
| A1 | 1 | 17.73 | 0.08 | 22.9 | 1.42 | 2.89 | 4.1 | | |
| A2 | | 2.45 | 0.08 | 19.0 | 0.20 | 3.17 | 0.6 | | |
| B1 | | 12.47 | 0.08 | 20.5 | 1.00 | 3.05 | 3.0 | | |
| OB1 | O1 | 1.34 | 0.49 | 5.0 | 0.66 | 5.17 | 3.4 | | |
| OB2 | O2 | 0.60 | 0.49 | 6.1 | 0.29 | 4.86 | 1.4 | | |
| B1+O1+O2 | 2 | 14.41 | 0.14 | 20.5 | 1.95 | 3.05 | 5.9 | | |
| C1 | | 6.29 | 0.08 | 21.1 | 0.50 | 3.01 | 1.5 | | |
| OC | | 2.55 | 0.08 | 20.8 | 0.20 | 3.03 | 0.6 | | |
| C1+OC | 3 | 8.85 | 0.08 | 21.1 | 0.71 | 3.01 | 2.1 | | |
| D1 | 4 | 15.99 | 0.08 | 22.5 | 1.28 | 2.92 | 3.7 | | |
| D2 | | 2.81 | 0.08 | 19.4 | 0.23 | 3.14 | 0.7 | | |
| OD | | 2.70 | 0.08 | 20.5 | 0.22 | 3.05 | 0.7 | | |
| D2+OD | 5 | 5.51 | 0.08 | 19.4 | 0.44 | 3.14 | 1.4 | | |

PROJECT INFORMATION

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REV. BY: TDM
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REPORT TYPE: Preliminary
DATE: 3/17/2024



Drexel, Barrell & Co.

RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

| DEVELOPED | | RUNOFF | | 100 YR | STORM | P1= | 2.52 |
|-----------|--------------|-----------|---------------|----------------------|-------|-----------|---------|
| BASIN (S) | DESIGN POINT | AREA (AC) | DIRECT RUNOFF | | C * A | I (IN/HR) | Q (CFS) |
| | | | RUNOFF COEFF | t _c (MIN) | | | |
| A1 | 1 | 17.73 | 0.35 | 22.9 | 6.21 | 4.85 | 30.1 |
| A2 | | 2.45 | 0.35 | 19.0 | 0.86 | 5.32 | 4.6 |
| B1 | | 12.47 | 0.35 | 20.5 | 4.37 | 5.13 | 22.4 |
| OB1 | O1 | 1.34 | 0.66 | 5.0 | 0.88 | 8.68 | 7.6 |
| OB2 | O2 | 0.60 | 0.66 | 6.1 | 0.39 | 8.17 | 3.2 |
| B1+O1+O2 | 2 | 14.41 | 0.39 | 20.5 | 5.63 | 5.13 | 28.9 |
| C1 | | 6.29 | 0.35 | 21.1 | 2.20 | 5.05 | 11.1 |
| OC | | 2.55 | 0.35 | 20.8 | 0.89 | 5.09 | 4.5 |
| C1+OC | 3 | 8.85 | 0.35 | 21.1 | 3.10 | 5.05 | 15.6 |
| D1 | 4 | 15.99 | 0.35 | 22.5 | 5.60 | 4.89 | 27.4 |
| D2 | | 2.81 | 0.35 | 19.4 | 0.99 | 5.27 | 5.2 |
| OD | | 2.70 | 0.35 | 20.5 | 0.94 | 5.12 | 4.8 |
| D2+OD | 5 | 5.51 | 0.35 | 19.4 | 1.93 | 5.27 | 10.2 |

PROJECT INFORMATION

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Drexel, Barrell & Co.

| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

DEVELOPED CONDITON

| SUB-BASIN | SURFACE DESIGNATION | AREA ACRE | COMPOSITE RUNOFF COEFFICIENTS | | | | % IMPERV |
|-----------------|--------------------------|--------------|-------------------------------|------|-----|------|----------|
| | | | C2 | C5 | C10 | C100 | |
| A-BASINS | | | | | | | |
| A1 | Open Space | 0.07 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.27 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.74 | | 0.84 | 80% |
| TOTAL A1 | | 0.34 | | | | | |
| A2 | Open Space | 0.86 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.35 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.30 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.33 | | 0.53 | 35% |
| TOTAL A2 | | 1.51 | | | | | |
| A3 | Open Space | 0.36 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.70 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.42 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.49 | | 0.64 | 59% |
| TOTAL A3 | | 1.49 | | | | | |
| A4 | Open Space | 0.12 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.48 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.74 | | 0.84 | 80% |
| TOTAL A4 | | 0.60 | | | | | |
| A5 | Open Space | 0.61 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 1.31 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.54 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.46 | | 0.61 | 57% |

PROJECT INFORMATION

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| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | | |
|------------------|--------------------------|------|------|------|-----|--|
| TOTAL A5 | | 2.47 | | | | |
| A6 | Open Space | 0.00 | 0.08 | 0.35 | 0 | |
| | Commercial Development | 0.00 | 0.81 | 0.88 | 95 | |
| | Residential (< 1/8 Acre) | 1.74 | 0.45 | 0.59 | 65 | |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 | |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 | |
| | WEIGHTED AVERAGE | | 0.45 | 0.59 | 65% | |
| TOTAL A6 | | 1.74 | | | | |
| A7 | Open Space | 0.00 | 0.08 | 0.35 | 0 | |
| | Commercial Development | 0.00 | 0.81 | 0.88 | 95 | |
| | Residential (< 1/8 Acre) | 1.47 | 0.45 | 0.59 | 65 | |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 | |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 | |
| | WEIGHTED AVERAGE | | 0.45 | 0.59 | 65% | |
| TOTAL A7 | | 1.47 | | | | |
| A8 | Open Space | 0.00 | 0.08 | 0.35 | 0 | |
| | Commercial Development | 0.00 | 0.81 | 0.88 | 95 | |
| | Residential (< 1/8 Acre) | 0.65 | 0.45 | 0.59 | 65 | |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 | |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 | |
| | WEIGHTED AVERAGE | | 0.45 | 0.59 | 65% | |
| TOTAL A8 | | 0.65 | | | | |
| A9 | Open Space | 0.00 | 0.08 | 0.35 | 0 | |
| | Commercial Development | 0.00 | 0.81 | 0.88 | 95 | |
| | Residential (< 1/8 Acre) | 2.55 | 0.45 | 0.59 | 65 | |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 | |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 | |
| | WEIGHTED AVERAGE | | 0.45 | 0.59 | 65% | |
| TOTAL A9 | | 2.55 | | | | |
| A10 | Open Space | 0.00 | 0.08 | 0.35 | 0 | |
| | Commercial Development | 0.00 | 0.81 | 0.88 | 95 | |
| | Residential (< 1/8 Acre) | 3.25 | 0.45 | 0.59 | 65 | |
| | Streets: Paved | 0.00 | 0.90 | 0.96 | 100 | |
| | Streets: Gravel | 0.00 | 0.80 | 0.85 | 80 | |
| | WEIGHTED AVERAGE | | 0.45 | 0.59 | 65% | |
| TOTAL A10 | | 3.25 | | | | |

PROJECT INFORMATION

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PROJECT NO: 21604-00
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| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | | | |
|------------------|--------------------------|------|--|------|--|------|-----|
| A11 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.90 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.09 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.49 | | 0.62 | 68% |
| TOTAL A11 | | 0.99 | | | | | |
| A12 | Open Space | 0.97 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL A12 | | 0.97 | | | | | |
| A13 | Open Space | 2.16 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.23 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.12 | | 0.37 | 6% |
| TOTAL A13 | | 2.39 | | | | | |
| A14 | Open Space | 0.53 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL A14 | | 0.53 | | | | | |

Area tributary to Pond A **18.02** 0.46 0.64 0.59

| B-BASINS | | | | | | | |
|-------------|--------------------------|------|--|------|--|------|------|
| OSB1 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 2.09 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 | 100% |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
PROJECT NO: 21604-00
DESIGN BY: KGV
REV. BY: TDM
AGENCY: El Paso County
REPORT TYPE: Preliminary
DATE: 4/1/2024



| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| TOTAL OSB1 | | 2.09 | | | | |
|-----------------|--------------------------|------|--|------|--|------|
| B1 | Open Space | 0.00 | | 0.08 | | 0.35 |
| | Commercial Development | 2.15 | | 0.81 | | 0.88 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 |
| | WEIGHTED AVERAGE | | | 0.81 | | 0.88 |
| TOTAL B1 | | 2.15 | | | | |
| B2 | Open Space | 0.00 | | 0.08 | | 0.35 |
| | Commercial Development | 1.11 | | 0.81 | | 0.88 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 |
| | WEIGHTED AVERAGE | | | 0.81 | | 0.88 |
| TOTAL B2 | | 1.11 | | | | |
| B3 | Open Space | 0.00 | | 0.08 | | 0.35 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 |
| | Streets: Paved | 0.39 | | 0.90 | | 0.96 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 |
| TOTAL B3 | | 0.39 | | | | |
| B4 | Open Space | 0.00 | | 0.08 | | 0.35 |
| | Commercial Development | 1.54 | | 0.81 | | 0.88 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 |
| | WEIGHTED AVERAGE | | | 0.81 | | 0.88 |
| TOTAL B4 | | 1.54 | | | | |
| B5 | Open Space | 0.00 | | 0.08 | | 0.35 |
| | Commercial Development | 2.13 | | 0.81 | | 0.88 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 |
| | WEIGHTED AVERAGE | | | 0.81 | | 0.88 |
| TOTAL B5 | | 2.13 | | | | |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
PROJECT NO: 21604-00
DESIGN BY: KGV
REV. BY: TDM
AGENCY: El Paso County
REPORT TYPE: Preliminary
DATE: 4/1/2024



| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | | | |
|-----------------|--------------------------|------|--|------|--|------|------|
| B6 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 1.75 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.81 | | 0.88 | 95% |
| TOTAL B6 | | 1.75 | | | | | |
| B7 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.91 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 | 100% |
| TOTAL B7 | | 0.91 | | | | | |
| B8 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Streets: Paved | 0.75 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 | 100% |
| TOTAL B8 | | 0.75 | | | | | |
| B9 | Open Space | 1.42 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL B9 | | 1.42 | | | | | |

Area tributary to Pond B 14.26 0.76 0.85 0.87

| C-BASINS | | | | | | | |
|-------------------|--------------------------|------|--|------|--|------|------|
| OSC1 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.56 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 | 100% |
| TOTAL OSC1 | | 0.56 | | | | | |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
PROJECT NO: 21604-00
DESIGN BY: KGV
REV. BY: TDM
AGENCY: El Paso County
REPORT TYPE: Preliminary
DATE: 4/1/2024



Drexel, Barrell & Co.

| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | | | |
|-------------------|--------------------------|------|--|------|--|------|------|
| OSC2 | Open Space | 2.59 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.39 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.19 | | 0.43 | 13% |
| TOTAL OSC2 | | 2.98 | | | | | |
| C1 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.27 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 | 100% |
| TOTAL C1 | | 0.27 | | | | | |
| C2 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 2.24 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.81 | | 0.88 | 95% |
| TOTAL C2 | | 2.24 | | | | | |
| C3 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 1.32 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.81 | | 0.88 | 95% |
| TOTAL C3 | | 1.32 | | | | | |
| C4 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 1.51 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.81 | | 0.88 | 95% |
| TOTAL C4 | | 1.51 | | | | | |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
PROJECT NO: 21604-00
DESIGN BY: KGV
REV. BY: TDM
AGENCY: El Paso County
REPORT TYPE: Preliminary
DATE: 4/1/2024



Drexel, Barrell & Co.

| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | | | |
|-----------------|--------------------------|------|--|------|--|------|------|
| C5 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Streets: Paved | 0.88 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 | 100% |
| TOTAL C5 | | 0.88 | | | | | |
| C6 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.66 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 | 100% |
| TOTAL C6 | | 0.66 | | | | | |

D-BASINS

| | | | | | | | |
|-------------------|--------------------------|------|--|------|--|------|-----|
| OSD1 | Open Space | 2.94 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL OSD1 | | 2.94 | | | | | |
| D1 | Open Space | 0.38 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.96 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.35 | | 0.52 | 47% |
| TOTAL D1 | | 1.35 | | | | | |
| D2 | Open Space | 0.38 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 1.55 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.38 | | 0.54 | 52% |
| TOTAL D2 | | 1.93 | | | | | |
| D3 | Open Space | 0.13 | | 0.08 | | 0.35 | 0 |

PROJECT INFORMATION

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PROJECT NO: 21604-00
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AGENCY: El Paso County
REPORT TYPE: Preliminary
DATE: 4/1/2024



| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | | | |
|-----------------|--------------------------|------|--|------|--|------|-----|
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.78 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.40 | | 0.56 | 56% |
| TOTAL D3 | | 0.91 | | | | | |
| D4 | Open Space | 0.63 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 2.11 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.36 | | 0.53 | 50% |
| TOTAL D4 | | 2.75 | | | | | |
| D5 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.65 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.45 | | 0.59 | 65% |
| TOTAL D5 | | 0.65 | | | | | |
| D6 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 2.87 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.45 | | 0.59 | 65% |
| TOTAL D6 | | 2.87 | | | | | |
| D7 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.25 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.45 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.74 | | 0.83 | 87% |
| TOTAL D7 | | 0.70 | | | | | |
| D8 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
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DESIGN BY: KGV
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Drexel, Barrell & Co.

| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | | | |
|------------------|--------------------------|------|--|------|--|------|------|
| | Residential (< 1/8 Acre) | 0.42 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.45 | | 0.59 | 65% |
| TOTAL D8 | | 0.42 | | | | | |
| D9 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.29 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.45 | | 0.59 | 65% |
| TOTAL D9 | | 0.29 | | | | | |
| D10 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 1.31 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.45 | | 0.59 | 65% |
| TOTAL D10 | | 1.31 | | | | | |
| D11 | Open Space | 0.00 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.62 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.90 | | 0.96 | 100% |
| TOTAL D11 | | 0.62 | | | | | |
| D12 | Open Space | 1.52 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL D12 | | 1.52 | | | | | |
| D13 | Open Space | 1.38 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.07 | | 0.45 | | 0.59 | 65 |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
 PROJECT NO: 21604-00
 DESIGN BY: KGV
 REV. BY: TDM
 AGENCY: El Paso County
 REPORT TYPE: Preliminary
 DATE: 4/1/2024



| | C2* | C5* | C10* | C100* | % IMPERV |
|--------------------------|-----|------|------|-------|----------|
| Open Space | | 0.08 | | 0.35 | 0 |
| Commercial Development | | 0.81 | | 0.88 | 95 |
| Residential (< 1/8 Acre) | | 0.45 | | 0.59 | 65 |
| Streets: Paved | | 0.90 | | 0.96 | 100 |
| Streets: Gravel | | 0.80 | | 0.85 | 80 |

| | | | | | | | |
|------------------|--------------------------|------|--|------|--|------|-----|
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.10 | | 0.36 | 3% |
| TOTAL D13 | | 1.45 | | | | | |
| D14 | Open Space | 0.86 | | 0.08 | | 0.35 | 0 |
| | Commercial Development | 0.00 | | 0.81 | | 0.88 | 95 |
| | Residential (< 1/8 Acre) | 0.00 | | 0.45 | | 0.59 | 65 |
| | Streets: Paved | 0.00 | | 0.90 | | 0.96 | 100 |
| | Streets: Gravel | 0.00 | | 0.80 | | 0.85 | 80 |
| | WEIGHTED AVERAGE | | | 0.08 | | 0.35 | 0% |
| TOTAL D14 | | 0.86 | | | | | |

Area tributary to Pond C **35.56** **0.37** **0.50** **0.45**

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
 PROJECT NO: 21604-00
 DESIGN BY: KGV
 REV. BY: TDM
 AGENCY: El Paso County
 REPORT TYPE: Preliminary
 DATE: 4/1/2024



**RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF
 DEVELOPED TIME OF CONCENTRATION**

| SUB-BASIN DATA | | | | | | INITIAL/OVERLAND TIME (t) | | | TRAVEL TIME (t) | | | | PIPE TRAVEL TIME (t) | | | | TIME OF CONCENTRATION | | FINAL t _c | |
|------------------|------------|----------------|------------------|-------|-------|---------------------------|------------|------|-----------------|-------|------|------|----------------------|-------|------|-----|-----------------------|----------------|----------------------|------|
| BASIN | DESIGN PT: | C _s | C ₁₀₀ | AREA | COMP | LENGTH | SLOPE | t | LENGTH | SLOPE | VEL. | t | LENGTH | SLOPE | VEL. | t | COMP. | MINIMUM | t _c | |
| | | | | Ac | | Ft | % | Min | Ft | % | FPS | Min | Ft | % | FPS | Min | t _c | t _c | Min | |
| A-BASINS | | | | | | | | | | | | | | | | | | | | |
| A1 | 1 | 0.74 | 0.84 | 0.34 | 0.25 | 0.29 | 20 | 3.3 | 2.0 | 518 | 3.3 | 7.7 | 1.1 | | | | | 3.1 | 5.0 | 5.0 |
| A2 | | 0.33 | 0.53 | 1.51 | 0.50 | 0.79 | 20 | 3.5 | 4.2 | 534 | 3.4 | 7.8 | 1.1 | | | | | 5.3 | 5.0 | 5.3 |
| DP1+A2 | 2 | 0.40 | 0.59 | 1.85 | 0.75 | 1.08 | From DP1 | | | | | | | 70 | 0.7 | 5.8 | 0.2 | 5.2 | 5.0 | 5.2 |
| A3 | 3 | 0.49 | 0.64 | 1.49 | 0.73 | 0.95 | 100 | 6.3 | 6.1 | 937 | 2.4 | 6.2 | 2.5 | | | | | 8.6 | 5.0 | 8.6 |
| A4 | 4 | 0.74 | 0.84 | 0.60 | 0.44 | 0.50 | 40 | 1.8 | 3.5 | 820 | 1.8 | 6.3 | 2.2 | | | | | 5.7 | 5.0 | 5.7 |
| A5 | 5 | 0.46 | 0.61 | 2.47 | 1.13 | 1.51 | 100 | 1.0 | 11.8 | 664 | 0.5 | 3.8 | 2.9 | | | | | 14.7 | 5.0 | 14.7 |
| A6 | 6 | 0.45 | 0.59 | 1.74 | 0.78 | 1.03 | 100 | 1.0 | 11.9 | 435 | 1.8 | 5.7 | 1.3 | | | | | 13.2 | 5.0 | 13.2 |
| A7 | | 0.45 | 0.59 | 1.47 | 0.66 | 0.86 | 100 | 1.5 | 10.4 | 700 | 0.5 | 3.8 | 3.1 | | | | | 13.5 | 5.0 | 13.5 |
| DP5+DP6+A7 | 7 | 0.45 | 0.60 | 5.67 | 2.57 | 3.40 | From DP5 | | | 700 | 0.5 | 3.8 | 3.1 | | | | | 17.8 | 5.0 | 17.8 |
| A8 | 8 | 0.45 | 0.59 | 0.65 | 0.29 | 0.39 | 40 | 2.0 | 6.0 | 390 | 1.8 | 5.7 | 1.1 | | | | | 7.1 | 5.0 | 7.1 |
| A9 | | 0.45 | 0.59 | 2.55 | 1.15 | 1.51 | 100 | 4.1 | 7.4 | 540 | 1.8 | 5.7 | 1.6 | | | | | 9.0 | 5.0 | 9.0 |
| DP8+A9 | 9 | 0.45 | 0.59 | 3.21 | 1.44 | 1.89 | From DP8 | | | 250 | 0.5 | 3.8 | 1.1 | | | | | 8.2 | 5.0 | 8.2 |
| A10 | 10 | 0.45 | 0.59 | 3.25 | 1.46 | 1.92 | 100 | 2.0 | 9.5 | 880 | 0.5 | 4.9 | 3.0 | | | | | 12.5 | 5.0 | 12.5 |
| DP7+DP10 | 10A | 0.45 | 0.60 | 8.92 | 4.03 | 5.32 | From DP9 | | | | | | | 30 | 0.5 | 5.8 | 0.1 | 8.3 | 5.0 | 8.3 |
| A11 | | 0.49 | 0.62 | 0.99 | 0.49 | 0.62 | 100 | 2.1 | 8.7 | 316 | 0.9 | 4.9 | 1.1 | | | | | 9.8 | 5.0 | 9.8 |
| DP3+A11 | 11 | 0.49 | 0.63 | 2.48 | 1.22 | 1.57 | From DP3 | | | | | | | 5.5 | 1.0 | 5.8 | 0.02 | 8.6 | 5.0 | 8.6 |
| DP10A+DP9+DP11 | 11A | 0.46 | 0.60 | 14.61 | 6.69 | 8.78 | From DP10A | | | | | | | 206 | 0.5 | 5.8 | 0.6 | 8.9 | 5.0 | 8.9 |
| A12 | | 0.08 | 0.35 | 0.97 | 0.08 | 0.34 | 100 | 1.0 | 18.7 | 280 | 2.8 | 5.8 | 0.8 | | | | | 19.5 | 5.0 | 19.5 |
| DP2+DP11A+A12 | 12 | 0.43 | 0.57 | 17.42 | 7.52 | 9.86 | From DP11A | | | | | | | 150 | 0.5 | 5.8 | 0.4 | 9.3 | 5.0 | 9.3 |
| A13 | 13 | 0.12 | 0.37 | 2.39 | 0.28 | 0.89 | 25 | 18.0 | 3.4 | 72 | 10.0 | 14.0 | 0.1 | | | | | 3.5 | 5.0 | 5.0 |
| A14 | 14 | 0.08 | 0.35 | 0.53 | 0.04 | 0.19 | 25 | 2.1 | 7.3 | 311 | 3.0 | 5.8 | 0.9 | | | | | 8.2 | 5.0 | 8.2 |
| B-BASINS | | | | | | | | | | | | | | | | | | | | |
| OSB1 | 1 | 0.90 | 0.96 | 2.09 | 1.88 | 2.01 | 40 | 2.0 | 1.9 | 362 | 1.0 | 6.1 | 1.0 | | | | | 2.9 | 5.0 | 5.0 |
| B1 | | 0.81 | 0.88 | 2.15 | 1.74 | 1.90 | 60 | 2.3 | 3.2 | 511 | 3.3 | 6.6 | 1.3 | | | | | 4.5 | 5.0 | 5.0 |
| DP1+B1 | 1A | 0.85 | 0.92 | 4.25 | 3.63 | 3.90 | From DP1 | | | | | | | 250 | 1.0 | 7.2 | 0.6 | 5.6 | 5.0 | 5.6 |
| B2 | 2 | 0.81 | 0.88 | 1.11 | 0.90 | 0.98 | 40 | 4.0 | 2.2 | 308 | 4.0 | 7.0 | 0.7 | | | | | 2.9 | 5.0 | 5.0 |
| B3 | 3 | 0.90 | 0.96 | 0.39 | 0.35 | 0.38 | 20 | 2.0 | 1.3 | 199 | 2.0 | 7.0 | 0.5 | | | | | 1.8 | 5.0 | 5.0 |
| B4 | 4 | 0.81 | 0.88 | 1.54 | 1.25 | 1.36 | 50 | 3.5 | 2.5 | 326 | 3.3 | 6.3 | 0.9 | | | | | 3.4 | 5.0 | 5.0 |
| DP1A+DP2+DP3+DP4 | 4A | 0.84 | 0.91 | 7.29 | 6.13 | 6.61 | From DP1A | | | | | | | 195 | 1.0 | 7.2 | 0.5 | 6.0 | 5.0 | 6.0 |
| B5 | 5 | 0.81 | 0.88 | 2.13 | 1.73 | 1.88 | 60 | 3.5 | 2.8 | 286 | 2.6 | 5.5 | 0.9 | | | | | 3.6 | 5.0 | 5.0 |
| DP4A+DP5 | 5A | 0.83 | 0.90 | 9.42 | 7.86 | 8.49 | From DP4A | | | | | | | 245 | 1.0 | 7.2 | 0.6 | 6.6 | 5.0 | 6.6 |
| B6 | 6 | 0.81 | 0.88 | 1.75 | 1.42 | 1.54 | 50 | 3.9 | 2.4 | 388 | 3.6 | 6.9 | 0.9 | | | | | 3.4 | 5.0 | 5.0 |
| B7 | | 0.90 | 0.96 | 0.91 | 0.82 | 0.88 | 40 | 2.0 | 1.9 | 863 | 2.3 | 7.0 | 2.1 | | | | | 3.9 | 5.0 | 5.0 |
| DP6+B7 | 7 | 0.84 | 0.91 | 2.67 | 2.24 | 2.42 | From DP6 | | | | | | | 20 | 1.0 | 7.2 | 0.0 | 5.0 | 5.0 | 5.0 |
| B8 | | 0.90 | 0.96 | 0.75 | 0.67 | 0.72 | 40 | 1.0 | 2.4 | 618 | 2.8 | 7.0 | 1.5 | | | | | 3.8 | 5.0 | 5.0 |
| DP7+DP8 | 8 | 0.85 | 0.92 | 3.41 | 2.92 | 3.14 | From DP7 | | | | | | | 50 | 1.0 | 7.2 | 0.1 | 5.2 | 5.0 | 5.2 |
| DP8+DP5A | 8A | 0.84 | 0.91 | 12.84 | 10.77 | 11.63 | From DP5A | | | | | | | 115 | 1.0 | 7.2 | 0.3 | 6.9 | 5.0 | 6.9 |
| B9 | | 0.08 | 0.35 | 1.42 | 0.11 | 0.50 | 30 | 13.0 | 4.4 | 259 | 20.0 | 14.0 | 0.3 | | | | | 4.8 | 5.0 | 5.0 |
| DP8A+B9 | 9 | 0.76 | 0.85 | 14.26 | 10.89 | 12.13 | From DP8A | | | | | | | 46 | 1.0 | 7.2 | 0.1 | 7.0 | 5.0 | 7.0 |
| C-BASINS | | | | | | | | | | | | | | | | | | | | |
| OSC1 | C1 | 0.90 | 0.96 | 0.56 | 0.50 | 0.53 | 40 | 2.5 | 1.7 | 165 | 2.0 | 5.2 | 0.5 | | | | | 2.3 | 5.0 | 5.0 |
| C1 | | 0.90 | 0.96 | 0.27 | 0.25 | 0.26 | 40 | 2.5 | 1.7 | 193 | 2.0 | 5.2 | 0.6 | | | | | 2.4 | 5.0 | 5.0 |
| DP1+C1 | 1 | 0.90 | 0.96 | 0.83 | 0.75 | 0.80 | From OSC1 | | | 185 | 2.0 | 5.2 | 0.5 | | | | | 5.5 | 5.0 | 5.5 |
| C2 | | 0.81 | 0.88 | 2.24 | 1.82 | 1.97 | 60 | 3.2 | 2.9 | 412 | 3.0 | 6.5 | 1.1 | | | | | 3.9 | 5.0 | 5.0 |
| DP1+C2 | 2 | 1.12 | 1.21 | 2.52 | 2.81 | 3.03 | From DP1 | | | | | | | 260 | 1.0 | 7.2 | 0.6 | 6.1 | 5.0 | 6.1 |
| C3 | 3 | 0.81 | 0.88 | 1.32 | 1.07 | 1.16 | 70 | 2.6 | 3.3 | 496 | 2.7 | 5.8 | 1.4 | | | | | 4.7 | 5.0 | 5.0 |
| C4 | | 0.81 | 0.88 | 1.51 | 1.22 | 1.33 | 60 | 4.8 | 2.5 | 371 | 4.7 | 7.5 | 0.8 | | | | | 3.3 | 5.0 | 5.0 |
| DP2+DP3+C4 | 4 | 0.95 | 1.03 | 5.34 | 5.10 | 5.52 | From DP2 | | | | | | | 286 | 1.0 | 7.2 | 0.7 | 6.8 | 5.0 | 6.8 |
| OSC2 | | 0.19 | 0.43 | 2.98 | 0.56 | 1.28 | 50 | 2.5 | 8.9 | 575 | 2.0 | 5.2 | 1.8 | | | | | 6.8 | 5.0 | 6.8 |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
 PROJECT NO: 21604-00
 DESIGN BY: KGV
 REV. BY: TDM
 AGENCY: El Paso County
 REPORT TYPE: Preliminary
 DATE: 4/1/2024



**RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF
 DEVELOPED TIME OF CONCENTRATION**

| SUB-BASIN DATA | | | | | | | INITIAL/OVERLAND TIME (t) | | | TRAVEL TIME (t) | | | | PIPE TRAVEL TIME (t) | | | | TIME OF CONCENTRATION | | FINAL |
|----------------|------------|----------------|------------------|------|------|------|---------------------------|-------|-----|-----------------|-------|------|-----|----------------------|-------|------|-----|-----------------------|----------------|----------------|
| BASIN | DESIGN PT: | C _s | C ₁₀₀ | AREA | COMP | | LENGTH | SLOPE | t | LENGTH | SLOPE | VEL. | t | LENGTH | SLOPE | VEL. | t | COMP. | MINIMUM | t _c |
| | | | | Ac | | | Ft | % | Min | Ft | % | FPS | Min | Ft | % | FPS | Min | t _c | t _c | Min |
| C5 | | 0.90 | 0.96 | 0.88 | 0.79 | 0.84 | 40 | 2.0 | 1.9 | 938 | 2.0 | 5.2 | 3.0 | | | | | 4.9 | 5.0 | 5.0 |
| OSC2+C5 | 5 | 0.35 | 0.55 | 3.86 | 1.35 | 2.13 | From OSC2 | | | 100 | 2.0 | 5.2 | 0.3 | | | | | 7.2 | 5.0 | 7.2 |
| C6 | | 0.90 | 0.96 | 0.66 | 0.59 | 0.63 | 40 | 2.0 | 1.9 | 703 | 2.0 | 5.2 | 2.3 | | | | | 4.1 | 5.0 | 5.0 |
| DP5+C6 | 6 | 0.43 | 0.61 | 4.52 | 1.94 | 2.76 | From DP5 | | | | | | | 58 | 1.0 | 7.2 | 0.1 | 5.1 | 5.0 | 5.1 |
| DP4+DP6 | 6A | 0.71 | 0.84 | 9.86 | 7.04 | 8.28 | From DP4 | | | | | | | 430 | 1.0 | 7.2 | 1.0 | 7.8 | 5.0 | 7.8 |

| D-BASINS | | | | | | | | | | | | | | | | | | | | |
|---------------------------|----|------|------|-------|-------|-------|-----------|------|------|------|------|------|-----|-----|-----|-----|-----|------|-----|------|
| D1 | 1 | 0.35 | 0.52 | 1.35 | 0.46 | 0.70 | 70 | 2.7 | 8.5 | 594 | 2.6 | 9.6 | 1.0 | | | | | 9.5 | 5.0 | 9.5 |
| D2 | | 0.38 | 0.54 | 1.93 | 0.73 | 1.05 | 60 | 2.7 | 7.5 | 559 | 1.2 | 7.2 | 1.3 | | | | | 8.8 | 5.0 | 8.8 |
| DP1+D2 | 2 | 0.36 | 0.53 | 3.28 | 1.19 | 1.75 | From DP1 | | | 430 | 1.2 | 7.2 | 1.0 | | | | | 10.5 | 5.0 | 10.5 |
| D3 | 3 | 0.40 | 0.56 | 0.91 | 0.36 | 0.51 | 70 | 1.2 | 10.3 | 592 | 1.4 | 6.4 | 1.5 | | | | | 11.9 | 5.0 | 11.9 |
| DP6A(C)+DP2+DP3 | 3A | 0.61 | 0.75 | 14.05 | 8.60 | 10.53 | From DP3 | | | | | | | 83 | 1.0 | 7.2 | 0.2 | 12.1 | 5.0 | 12.1 |
| D4 | 4 | 0.36 | 0.53 | 2.75 | 1.00 | 1.47 | 70 | 2.3 | 8.8 | 475 | 3.3 | 8.8 | 0.9 | | | | | 9.7 | 5.0 | 9.7 |
| D5 | | 0.45 | 0.59 | 0.65 | 0.29 | 0.39 | 50 | 1.5 | 7.5 | 386 | 1.9 | 7.2 | 0.9 | | | | | 8.4 | 5.0 | 8.4 |
| DP4+D5 | 5 | 0.38 | 0.55 | 3.40 | 1.30 | 1.86 | From DP4 | | | 30 | 1.9 | 7.2 | 0.1 | | | | | 9.7 | 5.0 | 9.7 |
| D6 | 6 | 0.45 | 0.59 | 2.87 | 1.29 | 1.69 | 60 | 3.0 | 6.5 | 1520 | 3.2 | 10.1 | 2.5 | | | | | 9.0 | 5.0 | 9.0 |
| D7 | | 0.74 | 0.83 | 0.70 | 0.52 | 0.58 | 100 | 2.0 | 5.4 | 587 | 3.8 | 11.6 | 0.8 | | | | | 6.2 | 5.0 | 6.2 |
| DP5+DP6+D7 | 7 | 0.45 | 0.59 | 6.98 | 3.11 | 4.13 | From DP5 | | | 307 | 3.8 | 11.6 | 0.4 | | | | | 10.2 | 5.0 | 10.2 |
| D8 | 8 | 0.45 | 0.59 | 0.42 | 0.19 | 0.25 | 80 | 1.5 | 9.5 | 362 | 1.5 | 6.4 | 0.9 | | | | | 10.4 | 5.0 | 10.4 |
| D9 | 9 | 0.45 | 0.59 | 0.29 | 0.13 | 0.17 | 80 | 4.0 | 6.8 | 229 | 5.5 | 12.2 | 0.3 | | | | | 7.2 | 5.0 | 7.2 |
| D10 | 10 | 0.45 | 0.59 | 1.31 | 0.59 | 0.78 | 70 | 3.9 | 6.5 | 479 | 4.5 | 11.6 | 0.7 | | | | | 7.2 | 5.0 | 7.2 |
| D11 | | 0.90 | 0.96 | 0.62 | 0.56 | 0.59 | 40 | 3.9 | 1.5 | 429 | 4.1 | 11.6 | 0.6 | | | | | 2.1 | 5.0 | 5.0 |
| DP10+D11 | 11 | 0.59 | 0.71 | 1.93 | 1.15 | 1.37 | From DP10 | | | | | | | 50 | 1.0 | 7.2 | 0.1 | 7.3 | 5.0 | 7.3 |
| D12 | | 0.08 | 0.35 | 1.52 | 0.12 | 0.53 | 80 | 25.0 | 5.8 | 166 | 25.0 | 4.0 | 0.7 | | | | | 6.5 | 5.0 | 6.5 |
| DP3A+DP7+DP8+DP9+DP11+D12 | 12 | 0.53 | 0.67 | 25.18 | 13.29 | 16.98 | From DP7 | | | | | | | 150 | 1.0 | 7.2 | 0.3 | 10.5 | 5.0 | 10.5 |
| OSD1 | D1 | 0.08 | 0.35 | 2.94 | 0.24 | 1.03 | 40 | 2.5 | 8.9 | 165 | 2.0 | 7.0 | 0.4 | | | | | 9.3 | 5.0 | 9.3 |
| D13 | | 0.10 | 0.36 | 1.45 | 0.14 | 0.52 | 80 | 5.9 | 9.3 | 1093 | 2.7 | 5.4 | 3.4 | | | | | 12.6 | 5.0 | 12.6 |
| DPD2+D13 | 13 | 0.09 | 0.35 | 4.39 | 0.38 | 1.56 | From D13 | | | 430 | 1.2 | 7.2 | 1.0 | | | | | 13.6 | 5.0 | 13.6 |
| D14 | 14 | 0.08 | 0.35 | 0.86 | 0.07 | 0.30 | 80 | 15.0 | 6.9 | 183 | 6.5 | 6.8 | 0.4 | | | | | 7.4 | 5.0 | 7.4 |

PROJECT INFORMATION

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REV. BY: TDM
AGENCY: El Paso County
REPORT TYPE: Preliminary
DATE: 4/1/2024



RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

| DEVELOPED | RUNOFF | | 5 YR | STORM | P1= | 1.50 | |
|---------------------------|--------------|-----------|---------------|----------------------|-------|-----------|---------|
| BASIN (S) | DESIGN POINT | AREA (AC) | DIRECT RUNOFF | | C * A | I (IN/HR) | Q (CFS) |
| | | | RUNOFF COEFF | t _c (MIN) | | | |
| DP5+DP6+D7 | 7 | 6.98 | 0.45 | 10.2 | 3.11 | 4.10 | 12.7 |
| D8 | 8 | 0.42 | 0.45 | 10.4 | 0.19 | 4.06 | 0.8 |
| D9 | 9 | 0.29 | 0.45 | 7.2 | 0.13 | 4.63 | 0.6 |
| D10 | 10 | 1.31 | 0.45 | 7.2 | 0.59 | 4.63 | 2.7 |
| D11 | | 0.62 | 0.90 | 5.0 | 0.56 | 5.17 | 2.9 |
| DP10+D11 | 11 | 1.93 | 0.59 | 7.3 | 1.15 | 4.61 | 5.3 |
| D12 | | 1.52 | 0.08 | 6.5 | 0.12 | 4.77 | 0.6 |
| DP3A+DP7+DP8+DP9+DP11+D12 | 12 | 25.18 | 0.53 | 10.5 | 13.29 | 4.05 | 53.8 |
| OSD1 | D1 | 2.94 | 0.08 | 9.3 | 0.24 | 4.24 | 1.0 |
| D13 | | 1.45 | 0.10 | 12.6 | 0.14 | 3.78 | 0.5 |
| DPD2+D13 | 13 | 4.39 | 0.09 | 13.6 | 0.38 | 3.66 | 1.4 |
| D14 | 14 | 0.86 | 0.08 | 7.4 | 0.07 | 4.59 | 0.3 |

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RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

DEVELOPED RUNOFF 100 YR STORM P1= **2.52**

| BASIN (S) | DESIGN POINT | AREA (AC) | DIRECT RUNOFF | | C * A | I (IN/HR) | Q (CFS) |
|------------------|--------------|-----------|---------------|----------------------|-------|-----------|---------|
| | | | RUNOFF COEFF | t _c (MIN) | | | |
| A-BASINS | | | | | | | |
| A1 | 1 | 0.34 | 0.84 | 5.0 | 0.29 | 8.68 | 2.5 |
| A2 | | 1.51 | 0.53 | 5.3 | 0.79 | 8.53 | 6.8 |
| DP1+A2 | 2 | 1.85 | 0.59 | 5.2 | 1.08 | 8.58 | 9.3 |
| A3 | 3 | 1.49 | 0.64 | 8.6 | 0.95 | 7.32 | 6.9 |
| A4 | 4 | 0.60 | 0.84 | 5.7 | 0.50 | 8.36 | 4.2 |
| A5 | 5 | 2.47 | 0.61 | 14.7 | 1.51 | 5.96 | 9.0 |
| A6 | 6 | 1.74 | 0.59 | 13.2 | 1.03 | 6.23 | 6.4 |
| A7 | | 1.47 | 0.59 | 13.5 | 0.86 | 6.18 | 5.3 |
| DP5+DP6+A7 | 7 | 5.67 | 0.60 | 17.8 | 3.40 | 5.48 | 18.7 |
| A8 | 8 | 0.65 | 0.59 | 7.1 | 0.39 | 7.79 | 3.0 |
| A9 | | 2.55 | 0.59 | 9.0 | 1.51 | 7.19 | 10.8 |
| DP8+A9 | 9 | 3.21 | 0.59 | 8.2 | 1.89 | 7.43 | 14.1 |
| A10 | 10 | 3.25 | 0.59 | 12.5 | 1.92 | 6.38 | 12.2 |
| DP7+DP10 | 10A | 8.92 | 0.60 | 8.3 | 5.32 | 7.40 | 39.4 |
| A11 | | 0.99 | 0.62 | 9.8 | 0.62 | 6.99 | 4.3 |
| DP3+A11 | 11 | 2.48 | 0.63 | 8.6 | 1.57 | 7.31 | 11.5 |
| DP10A+DP9+DP11 | 11A | 14.61 | 0.60 | 8.9 | 8.78 | 7.23 | 63.4 |
| A12 | | 0.97 | 0.35 | 19.5 | 0.34 | 5.25 | 1.8 |
| DP2+DP11A+A12 | 12 | 17.42 | 0.57 | 9.3 | 9.86 | 7.11 | 70.1 |
| A13 | 13 | 2.39 | 0.37 | 5.0 | 0.89 | 8.68 | 7.7 |
| A14 | 14 | 0.53 | 0.35 | 8.2 | 0.19 | 7.43 | 1.4 |
| B-BASINS | | | | | | | |
| OSB1 | 1 | 2.09 | 0.96 | 5.0 | 2.01 | 8.68 | 17.4 |
| B1 | | 2.15 | 0.88 | 5.0 | 1.90 | 8.68 | 16.4 |
| DP1+B1 | 1A | 4.25 | 0.92 | 5.6 | 3.90 | 8.40 | 32.8 |
| B2 | 2 | 1.11 | 0.88 | 5.0 | 0.98 | 8.68 | 8.5 |
| B3 | 3 | 0.39 | 0.96 | 5.0 | 0.38 | 8.68 | 3.3 |
| B4 | 4 | 1.54 | 0.88 | 5.0 | 1.36 | 8.68 | 11.8 |
| DP1A+DP2+DP3+DP4 | 4A | 7.29 | 0.91 | 6.0 | 6.61 | 8.21 | 54.3 |
| B5 | 5 | 2.13 | 0.88 | 5.0 | 1.88 | 8.68 | 16.3 |
| DP4A+DP5 | 5A | 9.42 | 0.90 | 6.6 | 8.49 | 7.98 | 67.8 |
| B6 | 6 | 1.75 | 0.88 | 5.0 | 1.54 | 8.68 | 13.4 |

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DESIGN BY: KGV
REV. BY: TDM
AGENCY: El Paso County
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RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

DEVELOPED **RUNOFF** **100 YR** **STORM** **P1=** **2.52**

| BASIN (S) | DESIGN POINT | AREA (AC) | DIRECT RUNOFF | | C * A | I (IN/HR) | Q (CFS) |
|-----------------|--------------|-----------|---------------|----------------------|-------|-----------|---------|
| | | | RUNOFF COEFF | t _c (MIN) | | | |
| B7 | | 0.91 | 0.96 | 5.0 | 0.88 | 8.68 | 7.6 |
| DP6+B7 | 7 | 2.67 | 0.91 | 5.0 | 2.42 | 8.66 | 20.9 |
| B8 | | 0.75 | 0.96 | 5.0 | 0.72 | 8.68 | 6.2 |
| DP7+DP8 | 8 | 3.41 | 0.92 | 5.2 | 3.14 | 8.60 | 27.0 |
| DP8+DP5A | 8A | 12.84 | 0.91 | 6.9 | 11.63 | 7.88 | 91.6 |
| B9 | | 1.42 | 0.35 | 5.0 | 0.50 | 8.68 | 4.3 |
| DP8A+B9 | 9 | 14.26 | 0.85 | 7.0 | 12.13 | 7.84 | 95.1 |
| C-BASINS | | | | | | | |
| OSC1 | C1 | 0.56 | 0.96 | 5.0 | 0.53 | 8.68 | 4.6 |
| C1 | | 0.27 | 0.96 | 5.0 | 0.26 | 8.68 | 2.3 |
| DPC1+C1 | 1 | 0.83 | 0.96 | 5.5 | 0.80 | 8.44 | 6.7 |
| C2 | | 2.24 | 0.88 | 5.0 | 1.97 | 8.68 | 17.1 |
| DP1+C2 | 2 | 2.52 | 1.21 | 6.1 | 3.03 | 8.18 | 24.8 |
| C3 | 3 | 1.32 | 0.88 | 5.0 | 1.16 | 8.68 | 10.1 |
| C4 | | 1.51 | 0.88 | 5.0 | 1.33 | 8.68 | 11.5 |
| DP2+DP3+C4 | 4 | 5.34 | 1.03 | 6.8 | 5.52 | 7.92 | 43.7 |
| OSC2 | | 2.98 | 0.43 | 6.8 | 1.28 | 7.89 | 10.1 |
| C5 | | 0.88 | 0.96 | 5.0 | 0.84 | 8.68 | 7.3 |
| OSC2+C5 | 5 | 3.86 | 0.55 | 7.2 | 2.13 | 7.77 | 16.5 |
| C6 | | 0.66 | 0.96 | 5.0 | 0.63 | 8.68 | 5.5 |
| DP5+C6 | 6 | 4.52 | 0.61 | 5.1 | 2.76 | 8.61 | 23.8 |
| DP4+DP6 | 6A | 9.86 | 0.84 | 7.8 | 8.28 | 7.57 | 62.7 |
| D-BASINS | | | | | | | |
| D1 | 1 | 1.35 | 0.52 | 9.5 | 0.70 | 7.06 | 5.0 |
| D2 | | 1.93 | 0.54 | 8.8 | 1.05 | 7.25 | 7.6 |
| DP1+D2 | 2 | 3.28 | 0.53 | 10.5 | 1.75 | 6.81 | 11.9 |
| D3 | 3 | 0.91 | 0.56 | 11.9 | 0.51 | 6.50 | 3.3 |
| DP6A(C)+DP2+DP3 | 3A | 14.05 | 0.75 | 12.1 | 10.53 | 6.46 | 68.0 |
| D4 | 4 | 2.75 | 0.53 | 9.7 | 1.47 | 7.01 | 10.3 |
| D5 | | 0.65 | 0.59 | 8.4 | 0.39 | 7.37 | 2.8 |
| DP4+D5 | 5 | 3.40 | 0.55 | 9.7 | 1.86 | 7.00 | 13.0 |
| D6 | 6 | 2.87 | 0.59 | 9.0 | 1.69 | 7.19 | 12.2 |

PROJECT INFORMATION

PROJECT: Commons at Falcon Field
PROJECT NO: 21604-00
DESIGN BY: KGV
REV. BY: TDM
AGENCY: El Paso County
REPORT TYPE: Preliminary
DATE: 4/1/2024



RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

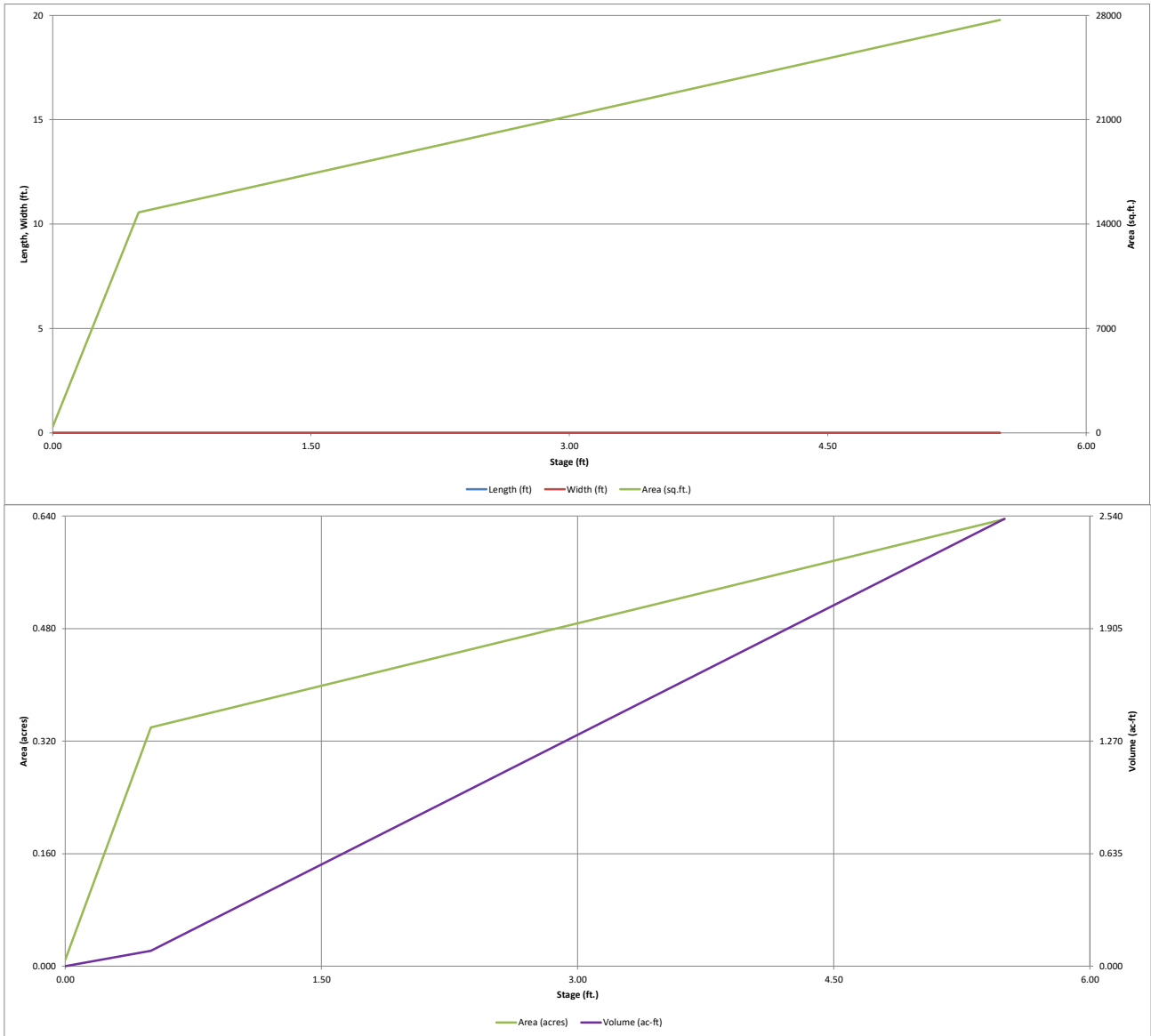
DEVELOPED **RUNOFF** **100 YR** **STORM** **P1=** **2.52**

| BASIN (S) | DESIGN POINT | AREA (AC) | DIRECT RUNOFF | | C * A | I (IN/HR) | Q (CFS) |
|---------------------------|--------------|-----------|---------------|----------------------|-------|-----------|---------|
| | | | RUNOFF COEFF | t _c (MIN) | | | |
| D7 | | 0.70 | 0.83 | 6.2 | 0.58 | 8.12 | 4.7 |
| DP5+DP6+D7 | 7 | 6.98 | 0.59 | 10.2 | 4.13 | 6.88 | 28.4 |
| D8 | 8 | 0.42 | 0.59 | 10.4 | 0.25 | 6.82 | 1.7 |
| D9 | 9 | 0.29 | 0.59 | 7.2 | 0.17 | 7.77 | 1.3 |
| D10 | 10 | 1.31 | 0.59 | 7.2 | 0.78 | 7.78 | 6.0 |
| D11 | | 0.62 | 0.96 | 5.0 | 0.59 | 8.68 | 5.1 |
| DP10+D11 | 11 | 1.93 | 0.71 | 7.3 | 1.37 | 7.74 | 10.6 |
| D12 | | 1.52 | 0.35 | 6.5 | 0.53 | 8.01 | 4.3 |
| DP3A+DP7+DP8+DP9+DP11+D12 | 12 | 25.18 | 0.67 | 10.5 | 16.98 | 6.80 | 115.5 |
| OSD1 | D1 | 2.94 | 0.35 | 9.3 | 1.03 | 7.12 | 7.3 |
| D13 | | 1.45 | 0.36 | 12.6 | 0.52 | 6.34 | 3.3 |
| DPD2+D13 | 13 | 4.39 | 0.35 | 13.6 | 1.56 | 6.15 | 9.6 |
| D14 | 14 | 0.86 | 0.35 | 7.4 | 0.30 | 7.70 | 2.3 |

Hydraulic Calculations

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

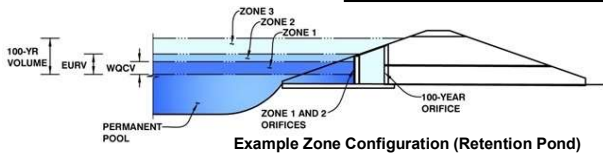


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: The Commons at Falcon Field

Basin ID: Pond A



| | Estimated Stage (ft) | Estimated Volume (ac-ft) | Outlet Type |
|--------------------------|----------------------|--------------------------|----------------------|
| Zone 1 (WQCV) | 1.24 | 0.351 | Orifice Plate |
| Zone 2 (EURV) | 3.34 | 0.938 | Orifice Plate |
| Zone 3 (100-year) | 4.58 | 0.676 | Weir&Pipe (Restrict) |
| Total (all zones) | | 1.965 | |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
 Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain
 Underdrain Orifice Area = ft²
 Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
 Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
 Orifice Plate: Orifice Vertical Spacing = inches
 Orifice Plate: Orifice Area per Row = inches

Calculated Parameters for Plate
 WQ Orifice Area per Row = ft²
 Elliptical Half-Width = feet
 Elliptical Slot Centroid = feet
 Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (required) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|--------------------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|
| Stage of Orifice Centroid (ft) | 0.00 | 1.11 | 2.23 | | | | | |
| Orifice Area (sq. inches) | 3.92 | 3.75 | 1.00 | | | | | |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|--------------------------------|------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Stage of Orifice Centroid (ft) | | | | | | | | |
| Orifice Area (sq. inches) | | | | | | | | |

User Input: Vertical Orifice (Circular or Rectangular)

| | Not Selected | Not Selected |
|---|--------------|--------------|
| Invert of Vertical Orifice = | N/A | N/A |
| Depth at top of Zone using Vertical Orifice = | N/A | N/A |
| Vertical Orifice Diameter = | N/A | N/A |

ft (relative to basin bottom at Stage = 0 ft)
 ft (relative to basin bottom at Stage = 0 ft)
 inches

Calculated Parameters for Vertical Orifice

| | Not Selected | Not Selected |
|-----------------------------|--------------|--------------|
| Vertical Orifice Area = | N/A | N/A |
| Vertical Orifice Centroid = | N/A | N/A |

ft²
 feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe)

| | Zone 3 Weir | Not Selected |
|---------------------------------------|-------------|--------------|
| Overflow Weir Front Edge Height, Ho = | 3.50 | N/A |
| Overflow Weir Front Edge Length = | 2.92 | N/A |
| Overflow Weir Gate Slope = | 0.00 | N/A |
| Horiz. Length of Weir Sides = | 2.92 | N/A |
| Overflow Gate Type = | Type C Gate | N/A |
| Debris Clogging % = | 50% | N/A |

ft (relative to basin bottom at Stage = 0 ft)
 feet
 H:V
 feet
 %

Calculated Parameters for Overflow Weir

| | Zone 3 Weir | Not Selected |
|---|-------------|--------------|
| Height of Gate Upper Edge, H _u = | 3.50 | N/A |
| Overflow Weir Slope Length = | 2.92 | N/A |
| Gate Open Area / 100-yr Orifice Area = | 4.53 | N/A |
| Overflow Gate Open Area w/o Debris = | 5.93 | N/A |
| Overflow Gate Open Area w/ Debris = | 2.97 | N/A |

feet
 feet
 ft²
 ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

| | Zone 3 Restrictor | Not Selected |
|---|-------------------|--------------|
| Depth to Invert of Outlet Pipe = | 0.00 | N/A |
| Outlet Pipe Diameter = | 18.00 | N/A |
| Restrictor Plate Height Above Pipe Invert = | 12.50 | |

ft (distance below basin bottom at Stage = 0 ft)
 inches
 inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

| | Zone 3 Restrictor | Not Selected |
|--|-------------------|--------------|
| Outlet Orifice Area = | 1.31 | N/A |
| Outlet Orifice Centroid = | 0.58 | N/A |
| Half-Central Angle of Restrictor Plate on Pipe = | 1.97 | N/A |

ft²
 feet
 radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

| | | |
|-------------------------------------|-------|---|
| Spillway Invert Stage = | 4.35 | ft (relative to basin bottom at Stage = 0 ft) |
| Spillway Crest Length = | 75.00 | feet |
| Spillway End Slopes = | 4.00 | H:V |
| Freeboard above Max Water Surface = | 1.00 | feet |

Calculated Parameters for Spillway

| | | |
|------------------------------------|------|---------|
| Spillway Design Flow Depth = | 0.35 | feet |
| Stage at Top of Freeboard = | 5.70 | feet |
| Basin Area at Top of Freeboard = | 0.64 | acres |
| Basin Volume at Top of Freeboard = | 2.53 | acre-ft |

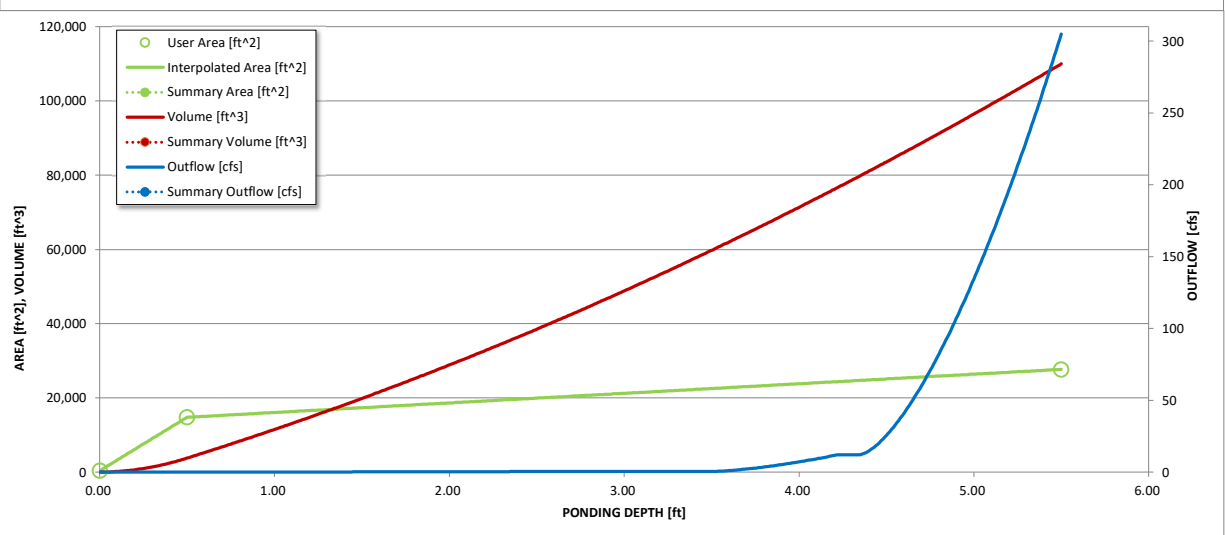
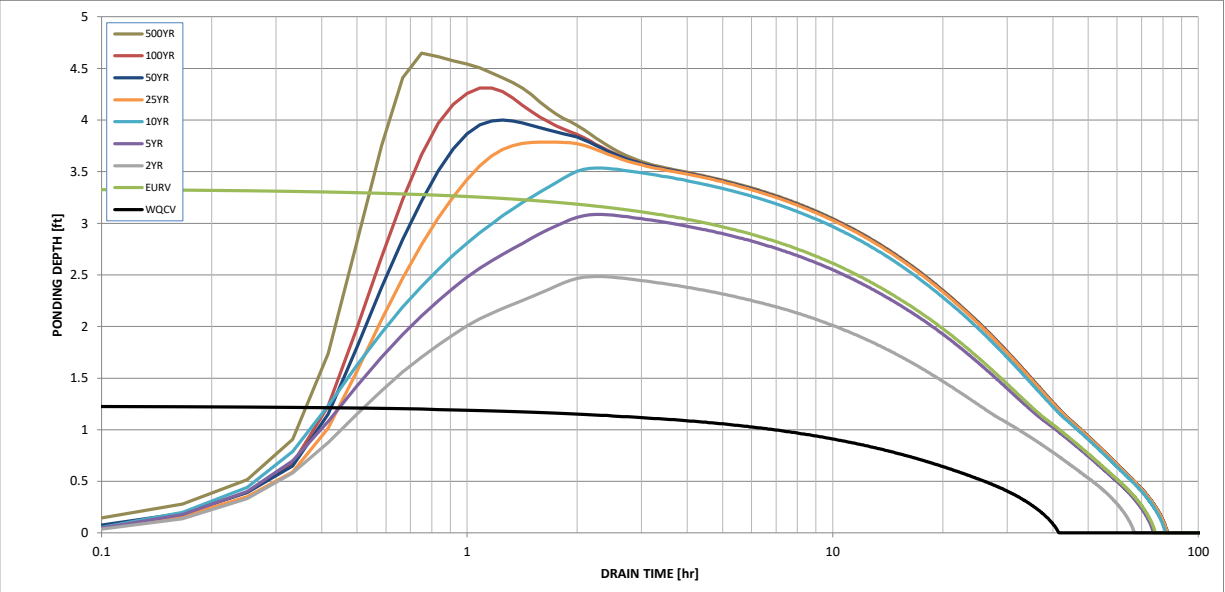
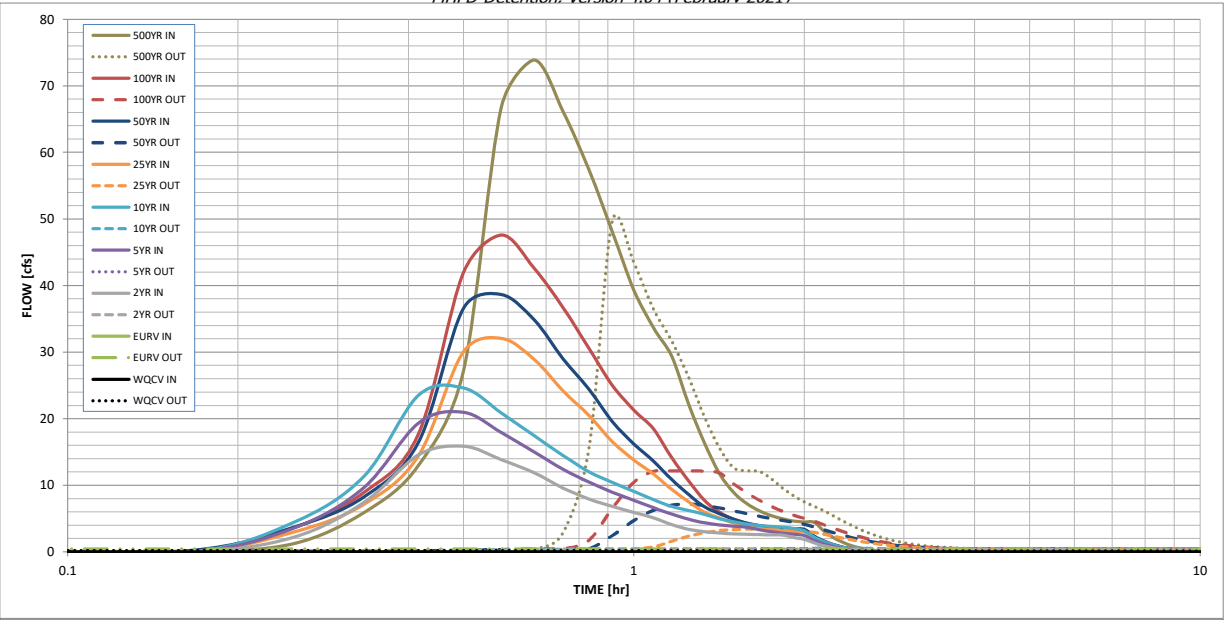
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

| | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year | 500 Year |
|---|-------|-------|--------|--------|-----------------|-----------------|-----------------|----------------|----------|
| Design Storm Return Period = | N/A | N/A | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 | 3.49 |
| One-Hour Rainfall Depth (in) = | 0.351 | 1.289 | 0.932 | 1.229 | 1.466 | 1.795 | 2.119 | 2.519 | 3.908 |
| CUHP Runoff Volume (acre-ft) = | N/A | N/A | 0.932 | 1.229 | 1.466 | 1.795 | 2.119 | 2.519 | 3.908 |
| Inflow Hydrograph Volume (acre-ft) = | N/A | N/A | 0.2 | 0.3 | 0.4 | 3.8 | 7.5 | 12.3 | 28.0 |
| CUHP Predevelopment Peak Q (cfs) = | N/A | N/A | | | | | | | |
| OPTIONAL Override Predevelopment Peak Q (cfs) = | N/A | N/A | | | | | | | |
| Predevelopment Unit Peak Flow, q (cfs/acre) = | N/A | N/A | 0.01 | 0.02 | 0.02 | 0.21 | 0.42 | 0.68 | 1.55 |
| Peak Inflow Q (cfs) = | N/A | N/A | 15.9 | 20.9 | 24.6 | 32.1 | 38.7 | 47.6 | 73.9 |
| Peak Outflow Q (cfs) = | 0.2 | 0.5 | 0.4 | 0.4 | 0.6 | 3.4 | 7.2 | 12.2 | 49.7 |
| Ratio Peak Outflow to Predevelopment Q = | N/A | N/A | N/A | 1.5 | 1.5 | 0.9 | 1.0 | 1.0 | 1.8 |
| Structure Controlling Flow = | Plate | Plate | Plate | Plate | Overflow Weir 1 | Overflow Weir 1 | Overflow Weir 1 | Outlet Plate 1 | Spillway |
| Max Velocity through Gate 1 (fps) = | N/A | N/A | N/A | N/A | 0.0 | 0.5 | 1.1 | 2.0 | 2.0 |
| Max Velocity through Gate 2 (fps) = | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Time to Drain 97% of Inflow Volume (hours) = | 38 | 68 | 60 | 67 | 72 | 71 | 70 | 68 | 63 |
| Time to Drain 99% of Inflow Volume (hours) = | 40 | 73 | 64 | 72 | 77 | 77 | 77 | 76 | 74 |
| Maximum Ponding Depth (ft) = | 1.24 | 3.34 | 2.48 | 3.08 | 3.53 | 3.79 | 4.00 | 4.31 | 4.65 |
| Area at Maximum Ponding Depth (acres) = | 0.38 | 0.51 | 0.46 | 0.49 | 0.52 | 0.53 | 0.55 | 0.57 | 0.58 |
| Maximum Volume Stored (acre-ft) = | 0.355 | 1.290 | 0.875 | 1.160 | 1.388 | 1.519 | 1.633 | 1.811 | 2.000 |

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)



| S-A-V-D Chart Axis Override | X-axis | Left Y-Axis | Right Y-Axis |
|-----------------------------|--------|-------------|--------------|
| minimum bound | | | |
| maximum bound | | | |

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

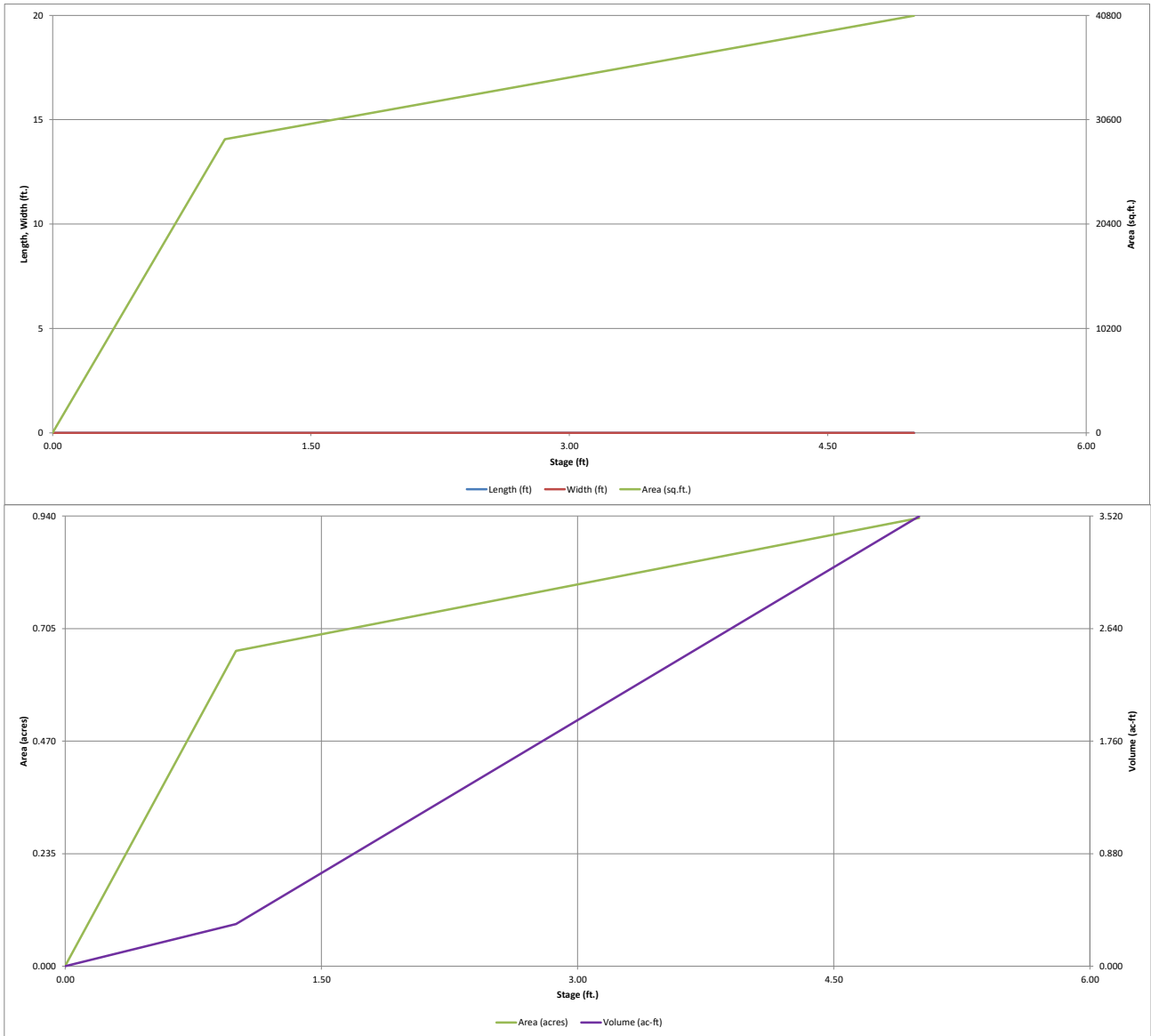
Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

| Time Interval | SOURCE | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | |
|---------------|---------|------------|------------|--------------|--------------|---------------|---------------|---------------|----------------|----------------|-------|
| | TIME | WQCV [cfs] | EURV [cfs] | 2 Year [cfs] | 5 Year [cfs] | 10 Year [cfs] | 25 Year [cfs] | 50 Year [cfs] | 100 Year [cfs] | 500 Year [cfs] | |
| 5.00 min | 0:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| | 0:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| | 0:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.25 | 0.02 | 1.23 |
| | 0:15:00 | 0.00 | 0.00 | 2.17 | 3.53 | 4.38 | 2.95 | 3.65 | 3.60 | 5.89 | 5.89 |
| | 0:20:00 | 0.00 | 0.00 | 7.42 | 9.63 | 11.30 | 7.11 | 8.25 | 8.89 | 12.99 | 12.99 |
| | 0:25:00 | 0.00 | 0.00 | 14.55 | 19.34 | 23.49 | 14.43 | 16.44 | 17.71 | 27.18 | 27.18 |
| | 0:30:00 | 0.00 | 0.00 | 15.87 | 20.94 | 24.62 | 30.06 | 36.58 | 41.95 | 66.74 | 66.74 |
| | 0:35:00 | 0.00 | 0.00 | 13.86 | 17.91 | 20.88 | 32.08 | 38.67 | 47.57 | 73.87 | 73.87 |
| | 0:40:00 | 0.00 | 0.00 | 11.87 | 15.00 | 17.43 | 28.90 | 34.83 | 42.58 | 66.15 | 66.15 |
| | 0:45:00 | 0.00 | 0.00 | 9.59 | 12.40 | 14.49 | 24.16 | 28.99 | 36.70 | 57.37 | 57.37 |
| | 0:50:00 | 0.00 | 0.00 | 7.94 | 10.48 | 12.03 | 20.50 | 24.42 | 30.55 | 48.06 | 48.06 |
| | 0:55:00 | 0.00 | 0.00 | 6.83 | 8.96 | 10.41 | 16.54 | 19.55 | 24.99 | 39.32 | 39.32 |
| | 1:00:00 | 0.00 | 0.00 | 5.94 | 7.73 | 9.08 | 13.81 | 16.21 | 21.29 | 33.65 | 33.65 |
| | 1:05:00 | 0.00 | 0.00 | 5.12 | 6.62 | 7.84 | 11.68 | 13.63 | 18.45 | 29.35 | 29.35 |
| | 1:10:00 | 0.00 | 0.00 | 4.08 | 5.70 | 6.82 | 9.38 | 10.85 | 14.12 | 22.10 | 22.10 |
| | 1:15:00 | 0.00 | 0.00 | 3.39 | 4.89 | 6.23 | 7.51 | 8.59 | 10.64 | 16.35 | 16.35 |
| | 1:20:00 | 0.00 | 0.00 | 3.02 | 4.36 | 5.64 | 5.96 | 6.76 | 7.72 | 11.74 | 11.74 |
| | 1:25:00 | 0.00 | 0.00 | 2.82 | 4.06 | 4.98 | 5.09 | 5.75 | 5.97 | 8.94 | 8.94 |
| | 1:30:00 | 0.00 | 0.00 | 2.71 | 3.86 | 4.52 | 4.34 | 4.89 | 4.93 | 7.24 | 7.24 |
| | 1:35:00 | 0.00 | 0.00 | 2.64 | 3.72 | 4.19 | 3.85 | 4.33 | 4.27 | 6.17 | 6.17 |
| | 1:40:00 | 0.00 | 0.00 | 2.59 | 3.32 | 3.96 | 3.52 | 3.96 | 3.83 | 5.44 | 5.44 |
| | 1:45:00 | 0.00 | 0.00 | 2.55 | 3.02 | 3.80 | 3.31 | 3.72 | 3.53 | 4.95 | 4.95 |
| | 1:50:00 | 0.00 | 0.00 | 2.53 | 2.81 | 3.69 | 3.16 | 3.55 | 3.34 | 4.63 | 4.63 |
| | 1:55:00 | 0.00 | 0.00 | 2.16 | 2.65 | 3.50 | 3.07 | 3.45 | 3.24 | 4.49 | 4.49 |
| | 2:00:00 | 0.00 | 0.00 | 1.88 | 2.46 | 3.15 | 3.01 | 3.39 | 3.21 | 4.44 | 4.44 |
| | 2:05:00 | 0.00 | 0.00 | 1.33 | 1.73 | 2.21 | 2.12 | 2.38 | 2.26 | 3.13 | 3.13 |
| | 2:10:00 | 0.00 | 0.00 | 0.91 | 1.20 | 1.53 | 1.47 | 1.65 | 1.57 | 2.16 | 2.16 |
| | 2:15:00 | 0.00 | 0.00 | 0.62 | 0.81 | 1.05 | 1.01 | 1.13 | 1.08 | 1.49 | 1.49 |
| | 2:20:00 | 0.00 | 0.00 | 0.41 | 0.53 | 0.69 | 0.67 | 0.75 | 0.71 | 0.98 | 0.98 |
| | 2:25:00 | 0.00 | 0.00 | 0.26 | 0.34 | 0.45 | 0.44 | 0.49 | 0.46 | 0.64 | 0.64 |
| | 2:30:00 | 0.00 | 0.00 | 0.15 | 0.22 | 0.28 | 0.28 | 0.31 | 0.29 | 0.40 | 0.40 |
| | 2:35:00 | 0.00 | 0.00 | 0.08 | 0.12 | 0.15 | 0.16 | 0.17 | 0.16 | 0.22 | 0.22 |
| | 2:40:00 | 0.00 | 0.00 | 0.03 | 0.05 | 0.06 | 0.07 | 0.08 | 0.07 | 0.09 | 0.09 |
| | 2:45:00 | 0.00 | 0.00 | 0.01 | 0.01 | 0.01 | 0.02 | 0.02 | 0.02 | 0.02 | 0.02 |
| | 2:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 2:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 5:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 6:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

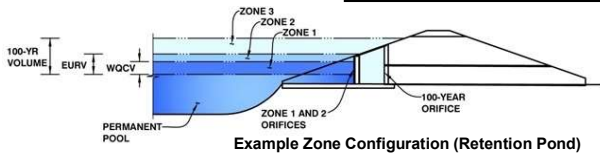


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: The Commons at Falcon Field

Basin ID: Pond B



| | Estimated Stage (ft) | Estimated Volume (ac-ft) | Outlet Type |
|--------------------------|----------------------|--------------------------|----------------------|
| Zone 1 (WQCV) | 1.15 | 0.423 | Orifice Plate |
| Zone 2 (EURV) | 2.75 | 1.163 | Orifice Plate |
| Zone 3 (100-year) | 3.55 | 0.644 | Weir&Pipe (Circular) |
| Total (all zones) | | 2.230 | |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

| | | |
|-----------------------------------|-----|--|
| Underdrain Orifice Invert Depth = | N/A | ft (distance below the filtration media surface) |
| Underdrain Orifice Diameter = | N/A | inches |

Calculated Parameters for Underdrain

| | | |
|-------------------------------|-----|-----------------|
| Underdrain Orifice Area = | N/A | ft ² |
| Underdrain Orifice Centroid = | N/A | feet |

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

| | | |
|--|-------|---|
| Invert of Lowest Orifice = | 0.00 | ft (relative to basin bottom at Stage = 0 ft) |
| Depth at top of Zone using Orifice Plate = | 2.75 | ft (relative to basin bottom at Stage = 0 ft) |
| Orifice Plate: Orifice Vertical Spacing = | 11.10 | inches |
| Orifice Plate: Orifice Area per Row = | 4.35 | sq. inches (use rectangular openings) |

Calculated Parameters for Plate

| | | |
|----------------------------|-----------|-----------------|
| WQ Orifice Area per Row = | 3.021E-02 | ft ² |
| Elliptical Half-Width = | N/A | feet |
| Elliptical Slot Centroid = | N/A | feet |
| Elliptical Slot Area = | N/A | ft ² |

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (required) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|--------------------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|
| Stage of Orifice Centroid (ft) | 0.00 | 0.92 | 1.83 | | | | | |
| Orifice Area (sq. inches) | 4.35 | 4.35 | 4.35 | | | | | |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|--------------------------------|------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Stage of Orifice Centroid (ft) | | | | | | | | |
| Orifice Area (sq. inches) | | | | | | | | |

User Input: Vertical Orifice (Circular or Rectangular)

| | Not Selected | Not Selected | |
|---|--------------|--------------|---|
| Invert of Vertical Orifice = | N/A | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Depth at top of Zone using Vertical Orifice = | N/A | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Vertical Orifice Diameter = | N/A | N/A | inches |

Calculated Parameters for Vertical Orifice

| | Not Selected | Not Selected | |
|-----------------------------|--------------|--------------|-----------------|
| Vertical Orifice Area = | N/A | N/A | ft ² |
| Vertical Orifice Centroid = | N/A | N/A | feet |

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

| | Zone 3 Weir | Not Selected | |
|---------------------------------------|--------------|--------------|---|
| Overflow Weir Front Edge Height, Ho = | 2.80 | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Overflow Weir Front Edge Length = | 6.00 | N/A | feet |
| Overflow Weir Grate Slope = | 0.00 | N/A | H:V |
| Horiz. Length of Weir Sides = | 6.00 | N/A | feet |
| Overflow Grate Type = | Type C Grate | N/A | |
| Debris Clogging % = | 50% | N/A | % |

Calculated Parameters for Overflow Weir

| | Zone 3 Weir | Not Selected | |
|--|-------------|--------------|-----------------|
| Height of Grate Upper Edge, H _u = | 2.80 | N/A | feet |
| Overflow Weir Slope Length = | 6.00 | N/A | feet |
| Grate Open Area / 100-yr Orifice Area = | 14.18 | N/A | |
| Overflow Grate Open Area w/o Debris = | 25.06 | N/A | ft ² |
| Overflow Grate Open Area w/ Debris = | 12.53 | N/A | ft ² |

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

| | Zone 3 Circular | Not Selected | |
|----------------------------------|-----------------|--------------|--|
| Depth to Invert of Outlet Pipe = | 0.00 | N/A | ft (distance below basin bottom at Stage = 0 ft) |
| Circular Orifice Diameter = | 18.00 | N/A | inches |

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

| | Zone 3 Circular | Not Selected | |
|--|-----------------|--------------|-----------------|
| Outlet Orifice Area = | 1.77 | N/A | ft ² |
| Outlet Orifice Centroid = | 0.75 | N/A | feet |
| Half-Central Angle of Restrictor Plate on Pipe = | N/A | N/A | radians |

User Input: Emergency Spillway (Rectangular or Trapezoidal)

| | | |
|-------------------------------------|-------|---|
| Spillway Invert Stage = | 3.25 | ft (relative to basin bottom at Stage = 0 ft) |
| Spillway Crest Length = | 25.00 | feet |
| Spillway End Slopes = | 4.00 | H:V |
| Freeboard above Max Water Surface = | 1.00 | feet |

Calculated Parameters for Spillway

| | | |
|------------------------------------|------|---------|
| Spillway Design Flow Depth = | 0.74 | feet |
| Stage at Top of Freeboard = | 4.99 | feet |
| Basin Area at Top of Freeboard = | 0.94 | acres |
| Basin Volume at Top of Freeboard = | 3.51 | acre-ft |

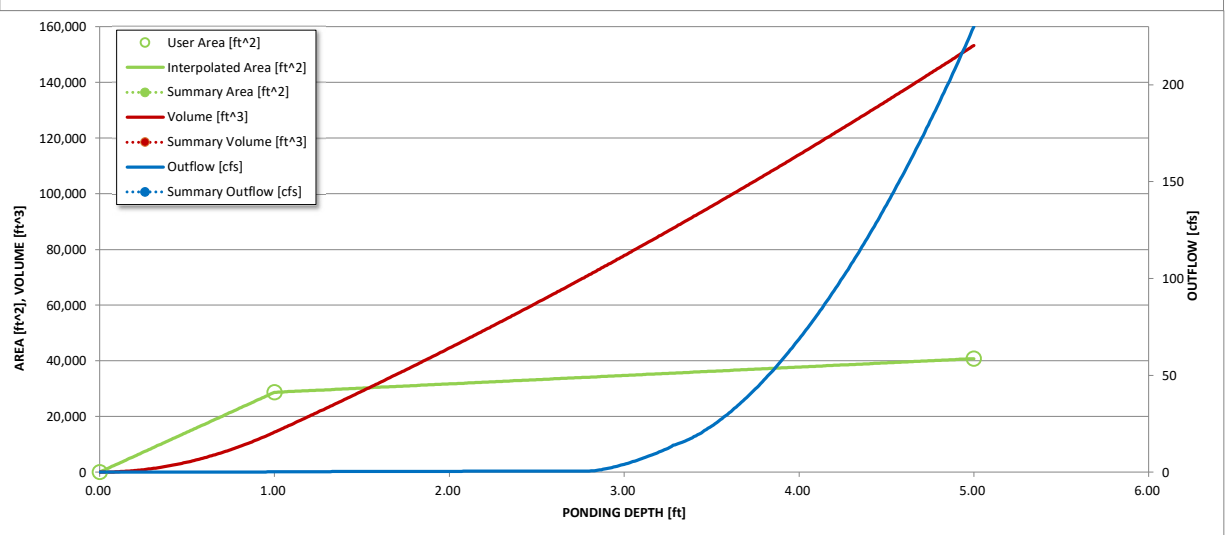
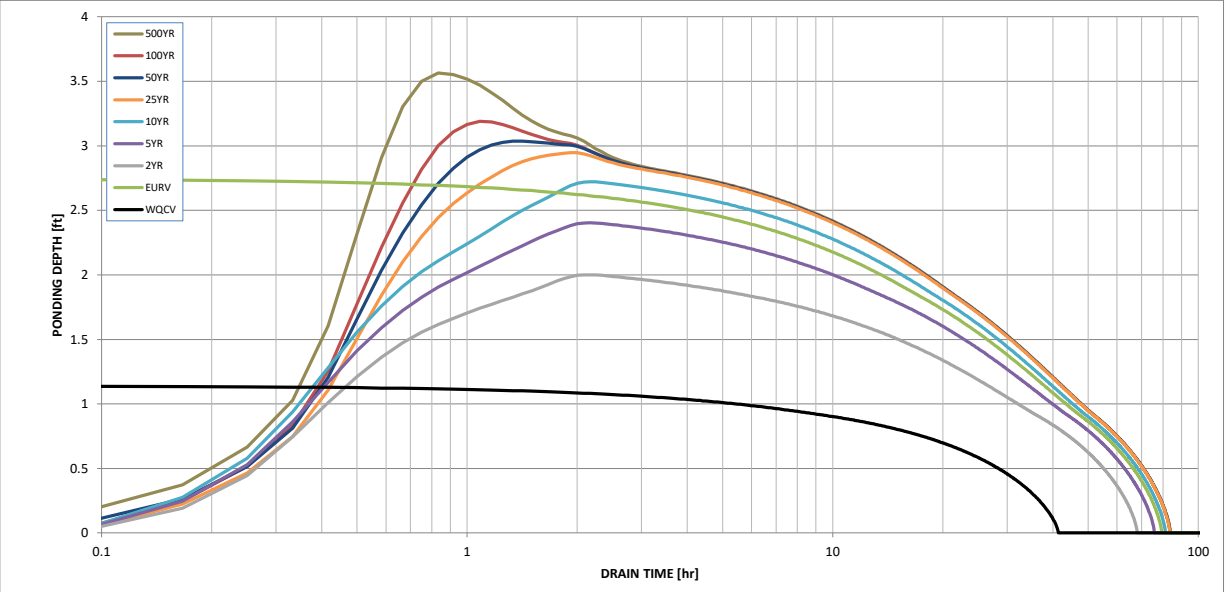
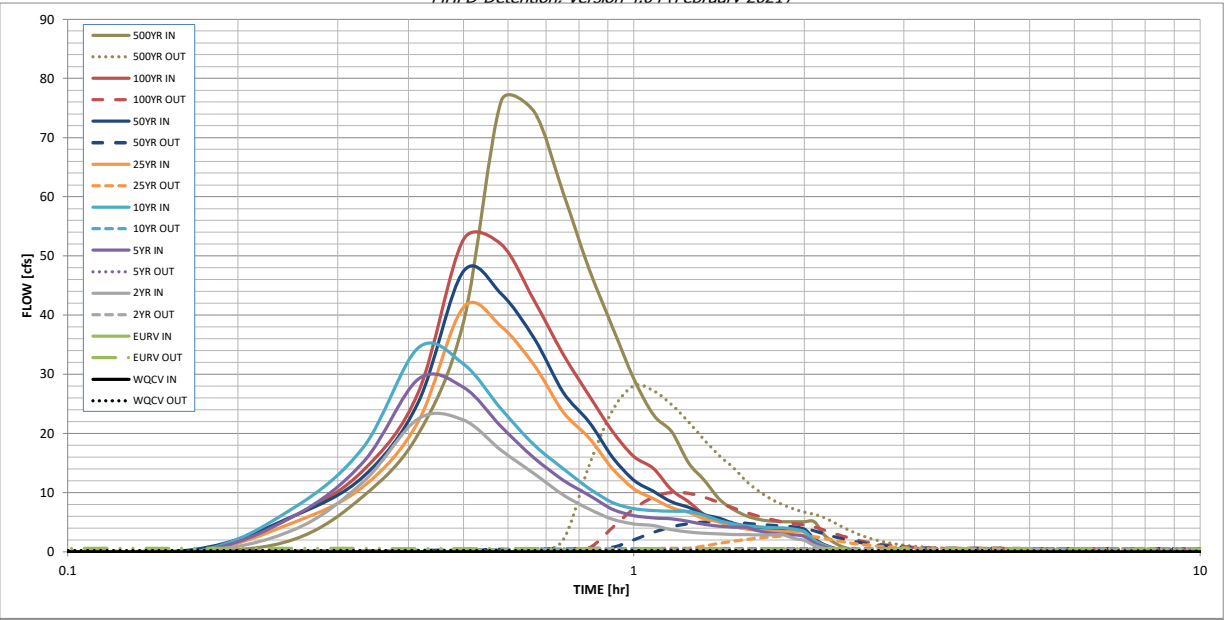
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

| | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year | 500 Year |
|---|-------|-------|--------|--------|---------|-----------------|-----------------|-----------------|----------|
| Design Storm Return Period = | N/A | N/A | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 | 3.49 |
| One-Hour Rainfall Depth (in) = | 0.423 | 1.586 | 1.078 | 1.392 | 1.645 | 1.928 | 2.205 | 2.520 | 3.625 |
| CUHP Runoff Volume (acre-ft) = | N/A | N/A | 1.078 | 1.392 | 1.645 | 1.928 | 2.205 | 2.520 | 3.625 |
| Inflow Hydrograph Volume (acre-ft) = | N/A | N/A | 0.1 | 0.3 | 0.4 | 3.2 | 6.3 | 10.4 | 23.5 |
| CUHP Predevelopment Peak Q (cfs) = | N/A | N/A | | | | | | | |
| OPTIONAL Override Predevelopment Peak Q (cfs) = | N/A | N/A | | | | | | | |
| Predevelopment Unit Peak Flow, q (cfs/acre) = | N/A | N/A | 0.01 | 0.02 | 0.03 | 0.23 | 0.46 | 0.76 | 1.71 |
| Peak Inflow Q (cfs) = | N/A | N/A | 22.5 | 29.2 | 34.5 | 41.3 | 47.4 | 52.8 | 76.2 |
| Peak Outflow Q (cfs) = | 0.2 | 0.6 | 0.4 | 0.5 | 0.6 | 2.8 | 5.1 | 10.0 | 28.1 |
| Ratio Peak Outflow to Predevelopment Q = | N/A | N/A | N/A | 2.0 | 1.6 | 0.9 | 0.8 | 1.0 | 1.2 |
| Structure Controlling Flow = | Plate | Plate | Plate | Plate | Plate | Overflow Weir 1 | Overflow Weir 1 | Overflow Weir 1 | Spillway |
| Max Velocity through Grate 1 (fps) = | N/A | N/A | N/A | N/A | N/A | 0.1 | 0.2 | 0.4 | 0.5 |
| Max Velocity through Grate 2 (fps) = | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Time to Drain 97% of Inflow Volume (hours) = | 38 | 71 | 62 | 68 | 73 | 74 | 73 | 72 | 68 |
| Time to Drain 99% of Inflow Volume (hours) = | 40 | 76 | 65 | 73 | 77 | 79 | 79 | 78 | 77 |
| Maximum Ponding Depth (ft) = | 1.15 | 2.75 | 2.00 | 2.40 | 2.72 | 2.95 | 3.04 | 3.19 | 3.56 |
| Area at Maximum Ponding Depth (acres) = | 0.67 | 0.78 | 0.73 | 0.76 | 0.78 | 0.79 | 0.80 | 0.81 | 0.84 |
| Maximum Volume Stored (acre-ft) = | 0.429 | 1.589 | 1.023 | 1.320 | 1.565 | 1.738 | 1.810 | 1.931 | 2.243 |

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)



| S-A-V-D Chart Axis Override | X-axis | Left Y-Axis | Right Y-Axis |
|-----------------------------|--------|-------------|--------------|
| minimum bound | | | |
| maximum bound | | | |

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

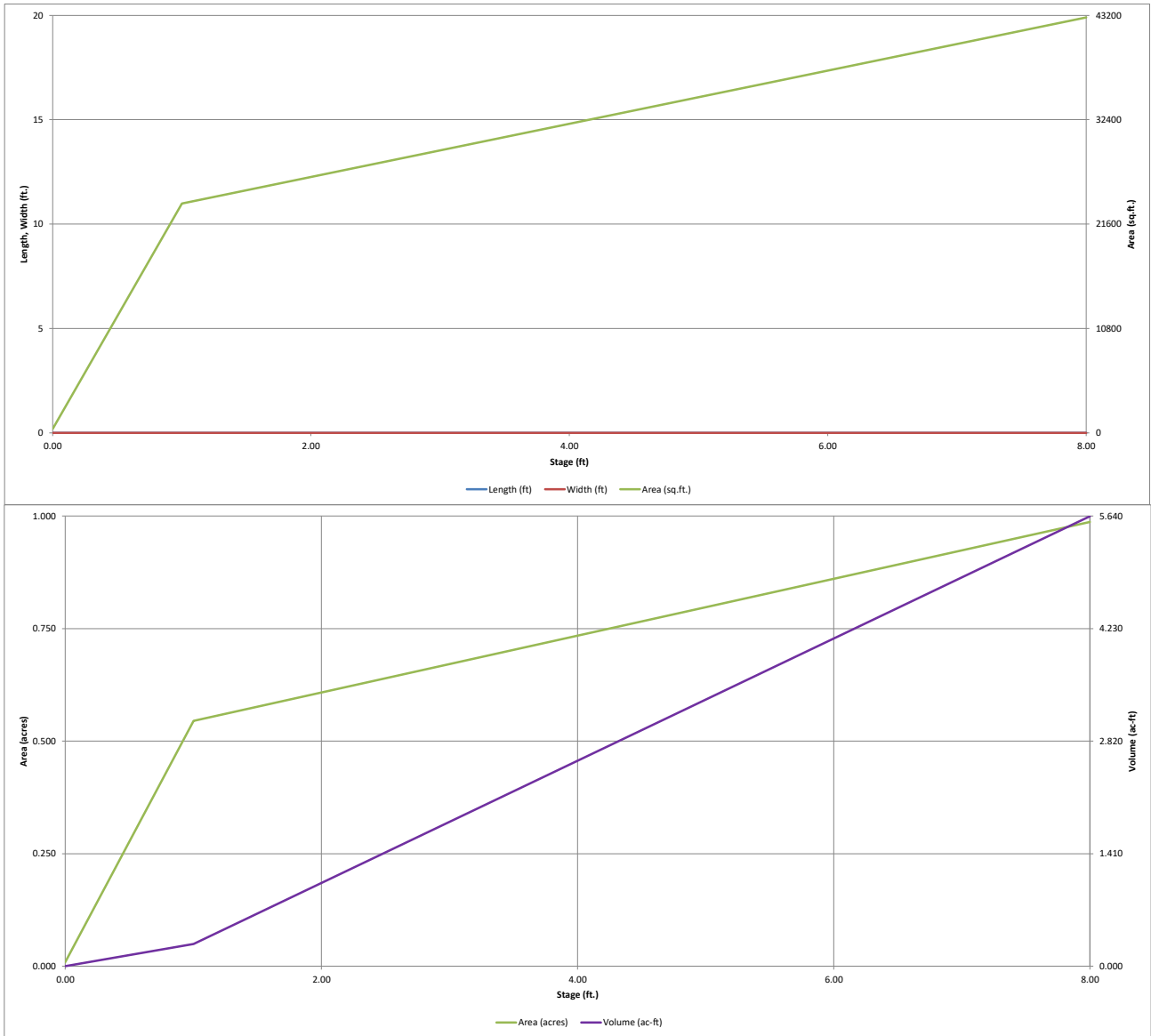
Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

| Time Interval | SOURCE | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | |
|---------------|---------|------------|------------|--------------|--------------|---------------|---------------|---------------|----------------|----------------|-------|
| | TIME | WQCV [cfs] | EURV [cfs] | 2 Year [cfs] | 5 Year [cfs] | 10 Year [cfs] | 25 Year [cfs] | 50 Year [cfs] | 100 Year [cfs] | 500 Year [cfs] | |
| 5.00 min | 0:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| | 0:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| | 0:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.41 | 0.04 | 2.04 |
| | 0:15:00 | 0.00 | 0.00 | 3.66 | 5.95 | 7.35 | 4.93 | 6.02 | 5.99 | 9.45 | 9.45 |
| | 0:20:00 | 0.00 | 0.00 | 11.90 | 15.24 | 17.77 | 11.10 | 12.78 | 13.86 | 19.93 | 19.93 |
| | 0:25:00 | 0.00 | 0.00 | 22.52 | 29.19 | 34.48 | 22.07 | 25.33 | 26.98 | 38.83 | 38.83 |
| | 0:30:00 | 0.00 | 0.00 | 22.25 | 27.79 | 31.75 | 41.29 | 47.40 | 52.76 | 76.18 | 76.18 |
| | 0:35:00 | 0.00 | 0.00 | 17.16 | 21.14 | 24.13 | 38.06 | 43.58 | 51.95 | 74.37 | 74.37 |
| | 0:40:00 | 0.00 | 0.00 | 13.22 | 15.87 | 18.07 | 31.53 | 36.08 | 42.45 | 60.71 | 60.71 |
| | 0:45:00 | 0.00 | 0.00 | 9.64 | 12.13 | 14.07 | 23.65 | 27.00 | 33.39 | 47.84 | 47.84 |
| | 0:50:00 | 0.00 | 0.00 | 7.24 | 9.56 | 10.72 | 19.28 | 21.98 | 26.47 | 38.04 | 38.04 |
| | 0:55:00 | 0.00 | 0.00 | 5.53 | 7.22 | 8.35 | 14.09 | 16.03 | 20.37 | 29.23 | 29.23 |
| | 1:00:00 | 0.00 | 0.00 | 4.73 | 6.13 | 7.30 | 10.66 | 12.11 | 16.12 | 23.13 | 23.13 |
| | 1:05:00 | 0.00 | 0.00 | 4.45 | 5.73 | 6.97 | 8.97 | 10.20 | 14.07 | 20.26 | 20.26 |
| | 1:10:00 | 0.00 | 0.00 | 3.74 | 5.58 | 6.86 | 7.44 | 8.43 | 10.44 | 14.97 | 14.97 |
| | 1:15:00 | 0.00 | 0.00 | 3.37 | 5.12 | 6.82 | 6.65 | 7.52 | 8.45 | 12.07 | 12.07 |
| | 1:20:00 | 0.00 | 0.00 | 3.15 | 4.63 | 6.18 | 5.58 | 6.30 | 6.27 | 8.87 | 8.87 |
| | 1:25:00 | 0.00 | 0.00 | 3.03 | 4.35 | 5.27 | 5.04 | 5.68 | 5.10 | 7.16 | 7.16 |
| | 1:30:00 | 0.00 | 0.00 | 2.94 | 4.19 | 4.73 | 4.29 | 4.83 | 4.34 | 6.05 | 6.05 |
| | 1:35:00 | 0.00 | 0.00 | 2.90 | 4.09 | 4.40 | 3.87 | 4.35 | 3.93 | 5.45 | 5.45 |
| | 1:40:00 | 0.00 | 0.00 | 2.89 | 3.50 | 4.21 | 3.62 | 4.08 | 3.76 | 5.21 | 5.21 |
| | 1:45:00 | 0.00 | 0.00 | 2.89 | 3.16 | 4.09 | 3.50 | 3.94 | 3.68 | 5.10 | 5.10 |
| | 1:50:00 | 0.00 | 0.00 | 2.89 | 2.96 | 4.05 | 3.44 | 3.87 | 3.67 | 5.09 | 5.09 |
| | 1:55:00 | 0.00 | 0.00 | 2.28 | 2.85 | 3.86 | 3.41 | 3.83 | 3.67 | 5.09 | 5.09 |
| | 2:00:00 | 0.00 | 0.00 | 1.93 | 2.63 | 3.40 | 3.40 | 3.83 | 3.67 | 5.09 | 5.09 |
| | 2:05:00 | 0.00 | 0.00 | 1.09 | 1.50 | 1.95 | 1.96 | 2.21 | 2.12 | 2.93 | 2.93 |
| | 2:10:00 | 0.00 | 0.00 | 0.61 | 0.85 | 1.10 | 1.13 | 1.26 | 1.21 | 1.68 | 1.68 |
| | 2:15:00 | 0.00 | 0.00 | 0.31 | 0.45 | 0.58 | 0.60 | 0.67 | 0.64 | 0.89 | 0.89 |
| | 2:20:00 | 0.00 | 0.00 | 0.14 | 0.23 | 0.29 | 0.31 | 0.35 | 0.34 | 0.46 | 0.46 |
| | 2:25:00 | 0.00 | 0.00 | 0.05 | 0.09 | 0.10 | 0.12 | 0.13 | 0.13 | 0.18 | 0.18 |
| | 2:30:00 | 0.00 | 0.00 | 0.01 | 0.01 | 0.01 | 0.02 | 0.02 | 0.02 | 0.02 | 0.02 |
| | 2:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 2:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 2:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 2:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 2:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 5:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 6:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

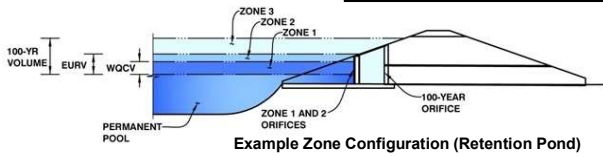


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-*Detention, Version 4.04 (February 2021)*

Project: The Commons at Falcon Field

Basin ID: Pond C



| | Estimated Stage (ft) | Estimated Volume (ac-ft) | Outlet Type |
|--------------------------|----------------------|--------------------------|----------------------|
| Zone 1 (WQCV) | 1.57 | 0.594 | Orifice Plate |
| Zone 2 (EURV) | 3.54 | 1.268 | Orifice Plate |
| Zone 3 (100-year) | 5.11 | 1.185 | Weir&Pipe (Circular) |
| Total (all zones) | | 3.048 | |

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

| | | |
|-----------------------------------|-----|--|
| Underdrain Orifice Invert Depth = | N/A | ft (distance below the filtration media surface) |
| Underdrain Orifice Diameter = | N/A | inches |

Calculated Parameters for Underdrain

| | | |
|-------------------------------|-----|-----------------|
| Underdrain Orifice Area = | N/A | ft ² |
| Underdrain Orifice Centroid = | N/A | feet |

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

| | | |
|--|-------|---|
| Invert of Lowest Orifice = | 0.00 | ft (relative to basin bottom at Stage = 0 ft) |
| Depth at top of Zone using Orifice Plate = | 3.59 | ft (relative to basin bottom at Stage = 0 ft) |
| Orifice Plate: Orifice Vertical Spacing = | 14.40 | inches |
| Orifice Plate: Orifice Area per Row = | 5.34 | sq. inches (use rectangular openings) |

Calculated Parameters for Plate

| | | |
|----------------------------|-----------|-----------------|
| WQ Orifice Area per Row = | 3.708E-02 | ft ² |
| Elliptical Half-Width = | N/A | feet |
| Elliptical Slot Centroid = | N/A | feet |
| Elliptical Slot Area = | N/A | ft ² |

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

| | Row 1 (required) | Row 2 (optional) | Row 3 (optional) | Row 4 (optional) | Row 5 (optional) | Row 6 (optional) | Row 7 (optional) | Row 8 (optional) |
|--------------------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|------------------|
| Stage of Orifice Centroid (ft) | 0.00 | 1.20 | 2.39 | | | | | |
| Orifice Area (sq. inches) | 5.34 | 5.34 | 5.34 | | | | | |

| | Row 9 (optional) | Row 10 (optional) | Row 11 (optional) | Row 12 (optional) | Row 13 (optional) | Row 14 (optional) | Row 15 (optional) | Row 16 (optional) |
|--------------------------------|------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Stage of Orifice Centroid (ft) | | | | | | | | |
| Orifice Area (sq. inches) | | | | | | | | |

User Input: Vertical Orifice (Circular or Rectangular)

| | Not Selected | Not Selected | |
|---|--------------|--------------|---|
| Invert of Vertical Orifice = | N/A | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Depth at top of Zone using Vertical Orifice = | N/A | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Vertical Orifice Diameter = | N/A | N/A | inches |

Calculated Parameters for Vertical Orifice

| | Not Selected | Not Selected | |
|-----------------------------|--------------|--------------|-----------------|
| Vertical Orifice Area = | N/A | N/A | ft ² |
| Vertical Orifice Centroid = | N/A | N/A | feet |

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

| | Zone 3 Weir | Not Selected | |
|---------------------------------------|--------------|--------------|---|
| Overflow Weir Front Edge Height, Ho = | 3.90 | N/A | ft (relative to basin bottom at Stage = 0 ft) |
| Overflow Weir Front Edge Length = | 4.92 | N/A | feet |
| Overflow Weir Grate Slope = | 0.00 | N/A | H:V |
| Horiz. Length of Weir Sides = | 4.92 | N/A | feet |
| Overflow Grate Type = | Type C Grate | N/A | |
| Debris Clogging % = | 50% | N/A | % |

Calculated Parameters for Overflow Weir

| | Zone 3 Weir | Not Selected | |
|--|-------------|--------------|-----------------|
| Height of Grate Upper Edge, H _u = | 3.90 | N/A | feet |
| Overflow Weir Slope Length = | 4.92 | N/A | feet |
| Grate Open Area / 100-yr Orifice Area = | 9.53 | N/A | |
| Overflow Grate Open Area w/o Debris = | 16.85 | N/A | ft ² |
| Overflow Grate Open Area w/ Debris = | 8.42 | N/A | ft ² |

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

| | Zone 3 Circular | Not Selected | |
|----------------------------------|-----------------|--------------|--|
| Depth to Invert of Outlet Pipe = | 2.83 | N/A | ft (distance below basin bottom at Stage = 0 ft) |
| Circular Orifice Diameter = | 18.00 | N/A | inches |

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

| | Zone 3 Circular | Not Selected | |
|--|-----------------|--------------|-----------------|
| Outlet Orifice Area = | 1.77 | N/A | ft ² |
| Outlet Orifice Centroid = | 0.75 | N/A | feet |
| Half-Central Angle of Restrictor Plate on Pipe = | N/A | N/A | radians |

User Input: Emergency Spillway (Rectangular or Trapezoidal)

| | | |
|-------------------------------------|-------|---|
| Spillway Invert Stage = | 5.00 | ft (relative to basin bottom at Stage = 0 ft) |
| Spillway Crest Length = | 20.00 | feet |
| Spillway End Slopes = | 4.00 | H:V |
| Freeboard above Max Water Surface = | 1.00 | feet |

Calculated Parameters for Spillway

| | | |
|------------------------------------|------|---------|
| Spillway Design Flow Depth = | 1.00 | feet |
| Stage at Top of Freeboard = | 7.00 | feet |
| Basin Area at Top of Freeboard = | 0.92 | acres |
| Basin Volume at Top of Freeboard = | 4.68 | acre-ft |

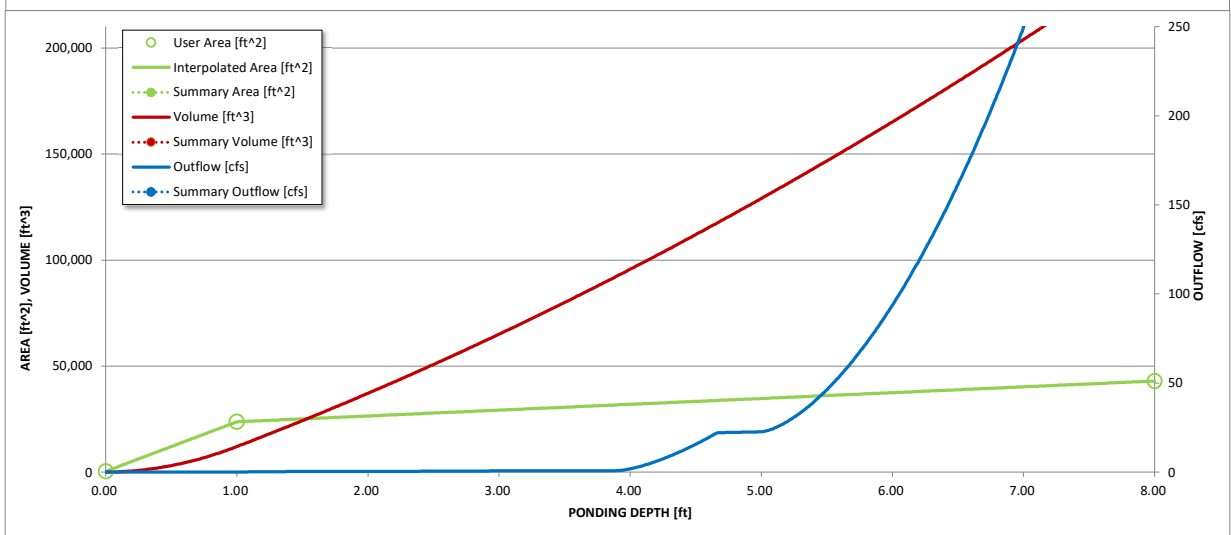
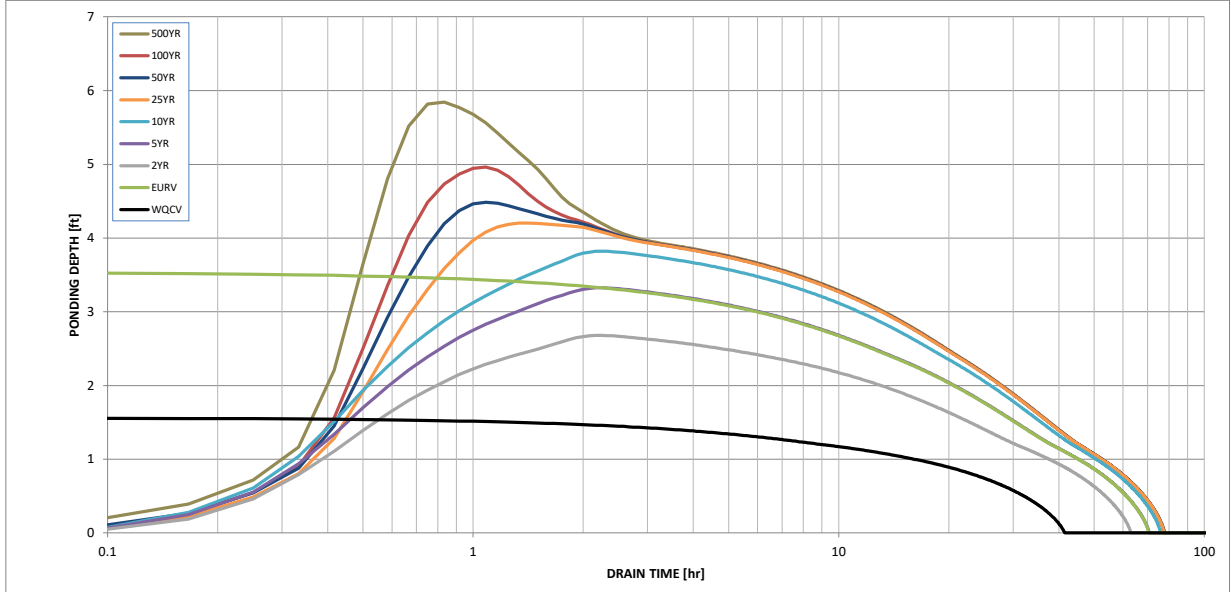
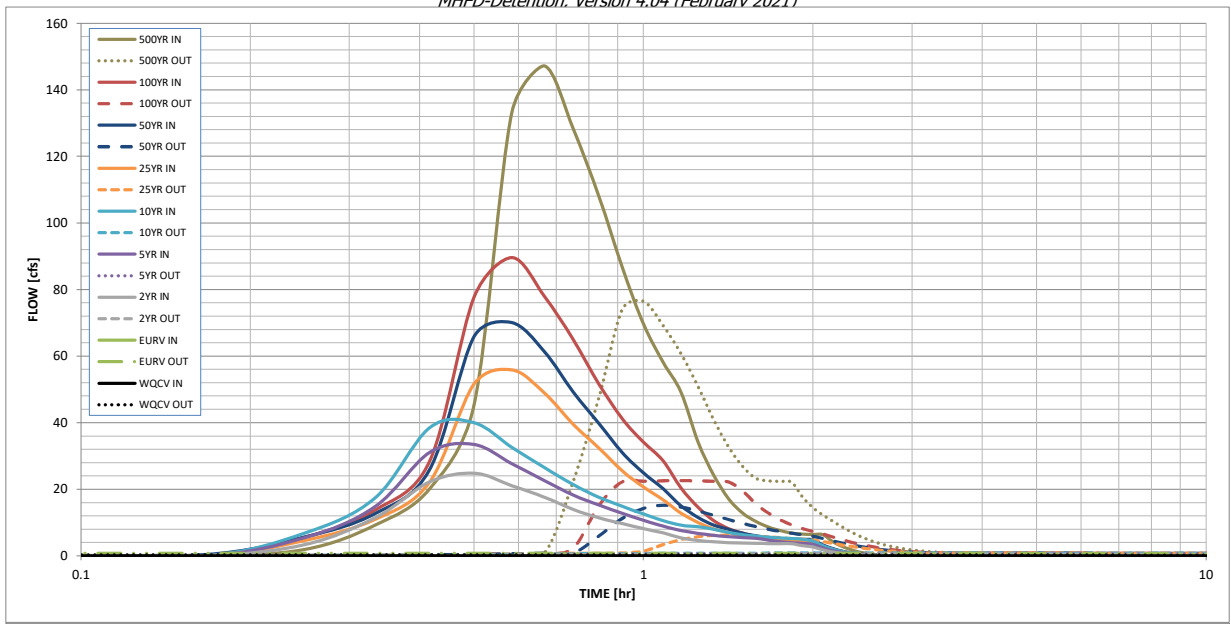
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

| | WQCV | EURV | 2 Year | 5 Year | 10 Year | 25 Year | 50 Year | 100 Year | 500 Year |
|---|-----------|-------|--------|--------|---------|-----------------|-----------------|----------------|----------|
| Design Storm Return Period = | N/A | N/A | 1.19 | 1.50 | 1.75 | 2.00 | 2.25 | 2.52 | 3.49 |
| One-Hour Rainfall Depth (in) = | 0.594 | 1.862 | 1.363 | 1.823 | 2.189 | 2.818 | 3.430 | 4.220 | 6.954 |
| CUHP Runoff Volume (acre-ft) = | N/A | N/A | 1.363 | 1.823 | 2.189 | 2.818 | 3.430 | 4.220 | 6.954 |
| Inflow Hydrograph Volume (acre-ft) = | N/A | N/A | 0.4 | 0.8 | 1.1 | 9.7 | 19.2 | 31.5 | 71.0 |
| CUHP Predevelopment Peak Q (cfs) = | N/A | N/A | | | | | | | |
| OPTIONAL Override Predevelopment Peak Q (cfs) = | N/A | N/A | | | | | | | |
| Predevelopment Unit Peak Flow, q (cfs/acre) = | N/A | N/A | 0.01 | 0.02 | 0.03 | 0.26 | 0.52 | 0.85 | 1.92 |
| Peak Inflow Q (cfs) = | N/A | N/A | 24.8 | 33.5 | 39.9 | 55.9 | 70.0 | 89.6 | 147.2 |
| Peak Outflow Q (cfs) = | 0.3 | 0.8 | 0.6 | 0.8 | 0.9 | 6.2 | 15.2 | 22.6 | 76.4 |
| Ratio Peak Outflow to Predevelopment Q = | N/A | N/A | N/A | 0.9 | 0.8 | 0.6 | 0.8 | 0.7 | 1.1 |
| Structure Controlling Flow = | Plate | Plate | Plate | Plate | Plate | Overflow Weir 1 | Overflow Weir 1 | Outlet Plate 1 | Spillway |
| Max Velocity through Grate 1 (fps) = | N/A | N/A | N/A | N/A | N/A | 0.3 | 0.9 | 1.3 | 1.4 |
| Max Velocity through Grate 2 (fps) = | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A |
| Time to Drain 97% of Inflow Volume (hours) = | 38 | 63 | 57 | 63 | 67 | 67 | 66 | 64 | 57 |
| Time to Drain 99% of Inflow Volume (hours) = | 40 | 67 | 60 | 67 | 72 | 73 | 72 | 72 | 69 |
| Maximum Ponding Depth (ft) = | 1.57 | 3.54 | 2.68 | 3.32 | 3.82 | 4.20 | 4.49 | 4.96 | 5.84 |
| Area at Maximum Ponding Depth (acres) = | 0.58 | 0.71 | 0.65 | 0.69 | 0.72 | 0.75 | 0.76 | 0.80 | 0.85 |
| Maximum Volume Stored (acre-ft) = | 0.598 | 1.865 | 1.275 | 1.712 | 2.065 | 2.345 | 2.556 | 2.931 | 3.655 |

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)



| S-A-V-D Chart Axis Override | X-axis | Left Y-Axis | Right Y-Axis |
|-----------------------------|--------|-------------|--------------|
| minimum bound | | | |
| maximum bound | | | |

DETENTION BASIN OUTLET STRUCTURE DESIGN

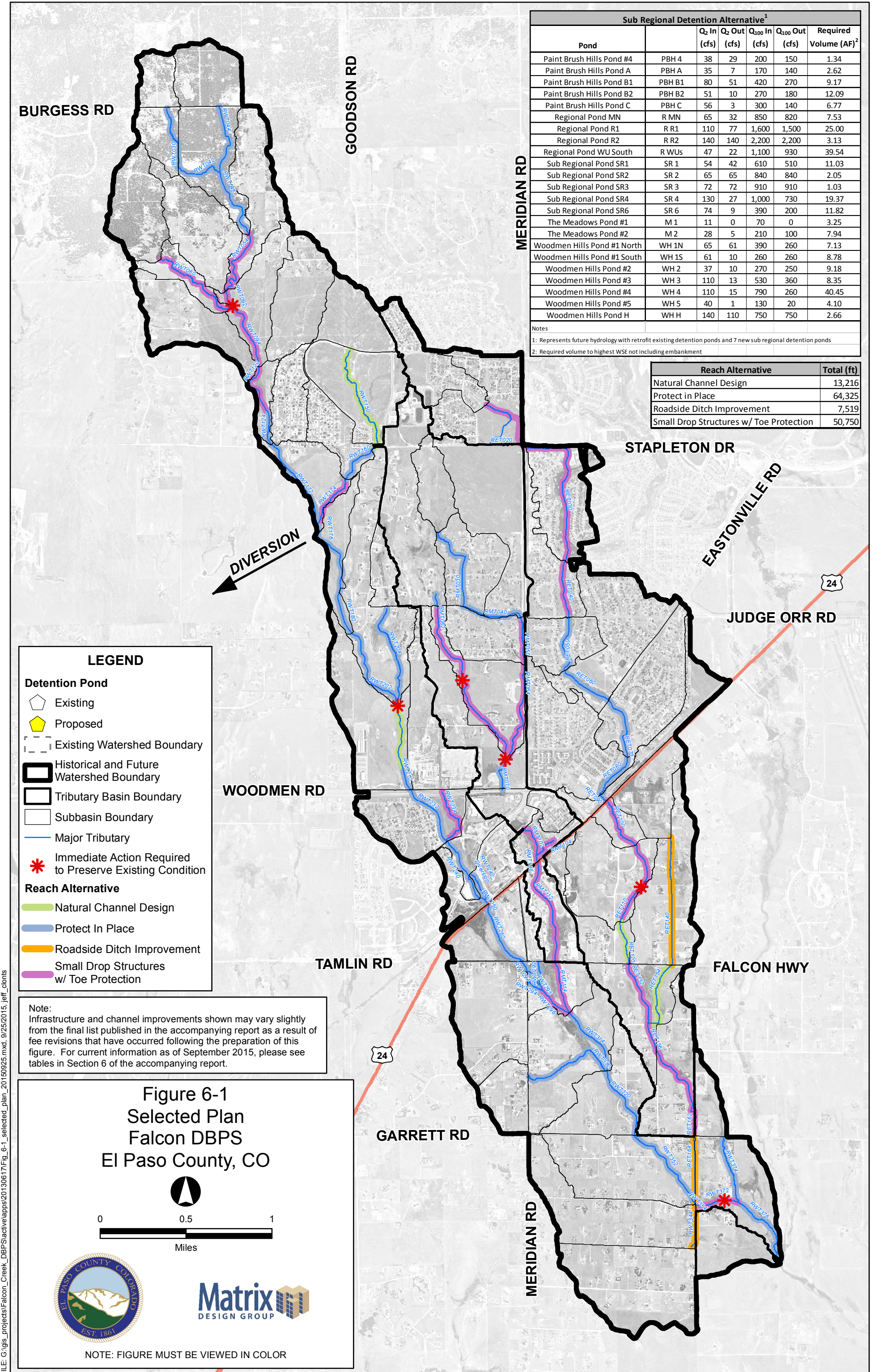
Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

| Time Interval | SOURCE | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | CUHP | |
|---------------|---------|------------|------------|--------------|--------------|---------------|---------------|---------------|----------------|----------------|--------|
| | TIME | WQCV [cfs] | EURV [cfs] | 2 Year [cfs] | 5 Year [cfs] | 10 Year [cfs] | 25 Year [cfs] | 50 Year [cfs] | 100 Year [cfs] | 500 Year [cfs] | |
| 5.00 min | 0:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| | 0:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| | 0:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.38 | 0.04 | 1.93 |
| | 0:15:00 | 0.00 | 0.00 | 0.00 | 3.36 | 5.46 | 6.80 | 4.59 | 5.68 | 5.62 | 9.20 |
| | 0:20:00 | 0.00 | 0.00 | 0.00 | 11.41 | 14.77 | 17.39 | 10.96 | 12.73 | 13.76 | 20.13 |
| | 0:25:00 | 0.00 | 0.00 | 0.00 | 22.19 | 31.05 | 38.49 | 22.12 | 25.83 | 28.33 | 45.28 |
| | 0:30:00 | 0.00 | 0.00 | 0.00 | 24.85 | 33.49 | 39.94 | 51.59 | 65.81 | 77.69 | 133.04 |
| | 0:35:00 | 0.00 | 0.00 | 0.00 | 21.09 | 27.67 | 32.62 | 55.89 | 70.03 | 89.59 | 147.17 |
| | 0:40:00 | 0.00 | 0.00 | 0.00 | 17.62 | 22.61 | 26.50 | 48.90 | 61.37 | 77.97 | 128.29 |
| | 0:45:00 | 0.00 | 0.00 | 0.00 | 13.96 | 18.23 | 21.40 | 39.54 | 49.22 | 64.99 | 108.06 |
| | 0:50:00 | 0.00 | 0.00 | 0.00 | 11.42 | 15.28 | 17.60 | 32.41 | 39.77 | 51.65 | 86.76 |
| | 0:55:00 | 0.00 | 0.00 | 0.00 | 9.67 | 12.81 | 14.91 | 25.56 | 31.06 | 41.21 | 69.62 |
| | 1:00:00 | 0.00 | 0.00 | 0.00 | 8.16 | 10.69 | 12.56 | 20.66 | 24.88 | 34.17 | 58.19 |
| | 1:05:00 | 0.00 | 0.00 | 0.00 | 6.88 | 8.91 | 10.56 | 16.84 | 20.14 | 28.56 | 49.16 |
| | 1:10:00 | 0.00 | 0.00 | 0.00 | 5.42 | 7.63 | 9.19 | 12.83 | 15.04 | 20.39 | 34.25 |
| | 1:15:00 | 0.00 | 0.00 | 0.00 | 4.62 | 6.77 | 8.72 | 10.04 | 11.56 | 14.64 | 24.26 |
| | 1:20:00 | 0.00 | 0.00 | 0.00 | 4.23 | 6.15 | 8.01 | 8.11 | 9.25 | 10.58 | 17.22 |
| | 1:25:00 | 0.00 | 0.00 | 0.00 | 3.98 | 5.75 | 7.04 | 7.02 | 7.95 | 8.18 | 12.85 |
| | 1:30:00 | 0.00 | 0.00 | 0.00 | 3.85 | 5.48 | 6.37 | 6.02 | 6.80 | 6.80 | 10.34 |
| | 1:35:00 | 0.00 | 0.00 | 0.00 | 3.76 | 5.30 | 5.92 | 5.34 | 6.02 | 5.89 | 8.66 |
| | 1:40:00 | 0.00 | 0.00 | 0.00 | 3.69 | 4.70 | 5.60 | 4.94 | 5.56 | 5.29 | 7.57 |
| | 1:45:00 | 0.00 | 0.00 | 0.00 | 3.64 | 4.26 | 5.39 | 4.65 | 5.23 | 4.90 | 6.85 |
| | 1:50:00 | 0.00 | 0.00 | 0.00 | 3.62 | 3.96 | 5.24 | 4.47 | 5.02 | 4.70 | 6.51 |
| | 1:55:00 | 0.00 | 0.00 | 0.00 | 3.06 | 3.75 | 4.97 | 4.36 | 4.91 | 4.63 | 6.41 |
| | 2:00:00 | 0.00 | 0.00 | 0.00 | 2.65 | 3.48 | 4.46 | 4.30 | 4.84 | 4.60 | 6.37 |
| | 2:05:00 | 0.00 | 0.00 | 0.00 | 1.82 | 2.38 | 3.04 | 2.93 | 3.29 | 3.14 | 4.33 |
| | 2:10:00 | 0.00 | 0.00 | 0.00 | 1.20 | 1.58 | 2.03 | 1.96 | 2.19 | 2.09 | 2.87 |
| | 2:15:00 | 0.00 | 0.00 | 0.00 | 0.79 | 1.03 | 1.34 | 1.30 | 1.45 | 1.38 | 1.88 |
| | 2:20:00 | 0.00 | 0.00 | 0.00 | 0.49 | 0.65 | 0.85 | 0.82 | 0.91 | 0.87 | 1.17 |
| | 2:25:00 | 0.00 | 0.00 | 0.00 | 0.29 | 0.41 | 0.52 | 0.52 | 0.58 | 0.55 | 0.74 |
| | 2:30:00 | 0.00 | 0.00 | 0.00 | 0.15 | 0.24 | 0.29 | 0.30 | 0.33 | 0.31 | 0.41 |
| | 2:35:00 | 0.00 | 0.00 | 0.00 | 0.07 | 0.11 | 0.13 | 0.14 | 0.15 | 0.14 | 0.18 |
| | 2:40:00 | 0.00 | 0.00 | 0.00 | 0.02 | 0.03 | 0.04 | 0.04 | 0.04 | 0.04 | 0.05 |
| | 2:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 2:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 2:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 3:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| | 4:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 5:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:05:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:10:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:15:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:20:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:25:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:30:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:35:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:40:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:45:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:50:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 5:55:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 6:00:00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | |

DBPS Excerpts



| Sub Regional Detention Alternative ¹ | | | | | | |
|---|--------|-------------------------|--------------------------|---------------------------|----------------------------|-----------------------------------|
| Pond | | Q ₂ In (cfs) | Q ₂ Out (cfs) | Q ₁₀₀ In (cfs) | Q ₁₀₀ Out (cfs) | Required Volume (AF) ² |
| Paint Brush Hills Pond #4 | PBH 4 | 38 | 29 | 200 | 150 | 1.34 |
| Paint Brush Hills Pond A | PBH A | 35 | 7 | 170 | 140 | 2.62 |
| Paint Brush Hills Pond B1 | PBH B1 | 80 | 51 | 420 | 270 | 9.17 |
| Paint Brush Hills Pond B2 | PBH B2 | 51 | 10 | 270 | 180 | 12.09 |
| Paint Brush Hills Pond C | PBH C | 56 | 3 | 300 | 140 | 6.77 |
| Regional Pond MN | R MN | 65 | 32 | 850 | 820 | 7.53 |
| Regional Pond R1 | R R1 | 110 | 77 | 1,600 | 1,500 | 25.00 |
| Regional Pond R2 | R R2 | 140 | 140 | 2,200 | 2,200 | 3.13 |
| Regional Pond WU South | R WUs | 47 | 22 | 1,100 | 930 | 39.54 |
| Sub Regional Pond SR1 | SR 1 | 54 | 42 | 610 | 510 | 11.03 |
| Sub Regional Pond SR2 | SR 2 | 65 | 65 | 840 | 840 | 2.05 |
| Sub Regional Pond SR3 | SR 3 | 72 | 72 | 910 | 910 | 1.03 |
| Sub Regional Pond SR4 | SR 4 | 130 | 27 | 1,000 | 730 | 19.37 |
| Sub Regional Pond SR6 | SR 6 | 74 | 9 | 390 | 200 | 11.82 |
| The Meadows Pond #1 | M 1 | 11 | 0 | 70 | 0 | 3.25 |
| The Meadows Pond #2 | M 2 | 28 | 5 | 210 | 100 | 7.94 |
| Woodmen Hills Pond #1 North | WH 1N | 65 | 61 | 390 | 260 | 7.13 |
| Woodmen Hills Pond #1 South | WH 1S | 61 | 10 | 260 | 260 | 8.78 |
| Woodmen Hills Pond #2 | WH 2 | 37 | 10 | 270 | 250 | 9.18 |
| Woodmen Hills Pond #3 | WH 3 | 110 | 13 | 530 | 360 | 8.35 |
| Woodmen Hills Pond #4 | WH 4 | 110 | 15 | 790 | 260 | 40.45 |
| Woodmen Hills Pond #5 | WH 5 | 40 | 1 | 130 | 20 | 4.10 |
| Woodmen Hills Pond H | WH H | 140 | 110 | 750 | 750 | 2.66 |

| Reach Alternative | Total (ft) |
|---|------------|
| Natural Channel Design | 13,216 |
| Protect in Place | 64,325 |
| Roadside Ditch Improvement | 7,519 |
| Small Drop Structures w/ Toe Protection | 50,750 |

Notes
 1: Represents future hydrology with retrofit existing detention ponds and 7 new sub regional detention ponds
 2: Required volume to highest WSE not including embankment

LEGEND

Detention Pond
 Existing (pentagon symbol)
 Proposed (yellow pentagon symbol)

Existing Watershed Boundary (dashed line)
 Historical and Future Watershed Boundary (thick black line)
 Tributary Basin Boundary (thin black line)
 Subbasin Boundary (thin grey line)

Major Tributary (blue line)

Reach Alternative
 Natural Channel Design (green line)
 Protect In Place (blue line)
 Roadside Ditch Improvement (orange line)
 Small Drop Structures w/ Toe Protection (purple line)

Immediate Action Required to Preserve Existing Condition (red asterisk symbol)

Note:
 Infrastructure and channel improvements shown may vary slightly from the final list published in the accompanying report as a result of fee revisions that have occurred following the preparation of this figure. For current information as of September 2015, please see tables in Section 6 of the accompanying report.

**Figure 6-1
 Selected Plan
 Falcon DBPS
 El Paso County, CO**










0 0.5 1
 Miles

NOTE: FIGURE MUST BE VIEWED IN COLOR

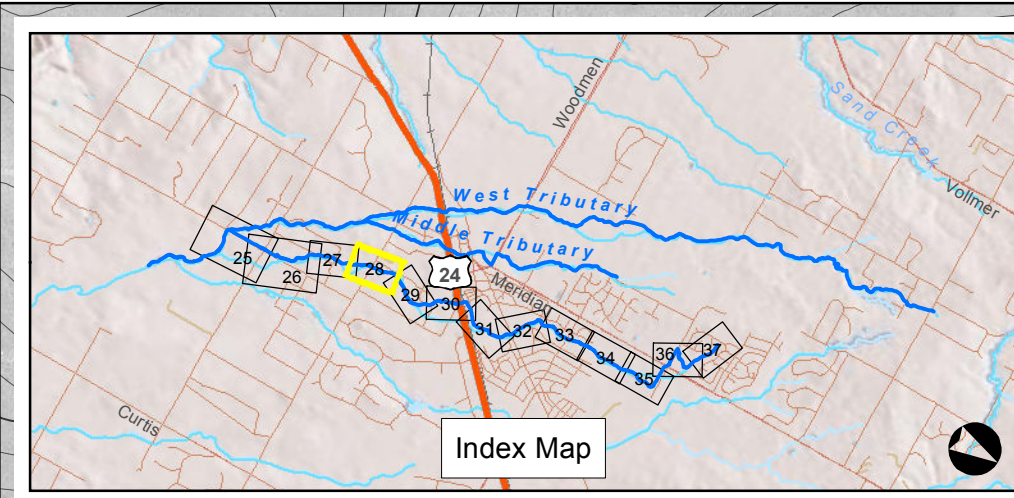
Sheet 4-28

East Tributary Floodplain Falcon DBPS El Paso County, CO

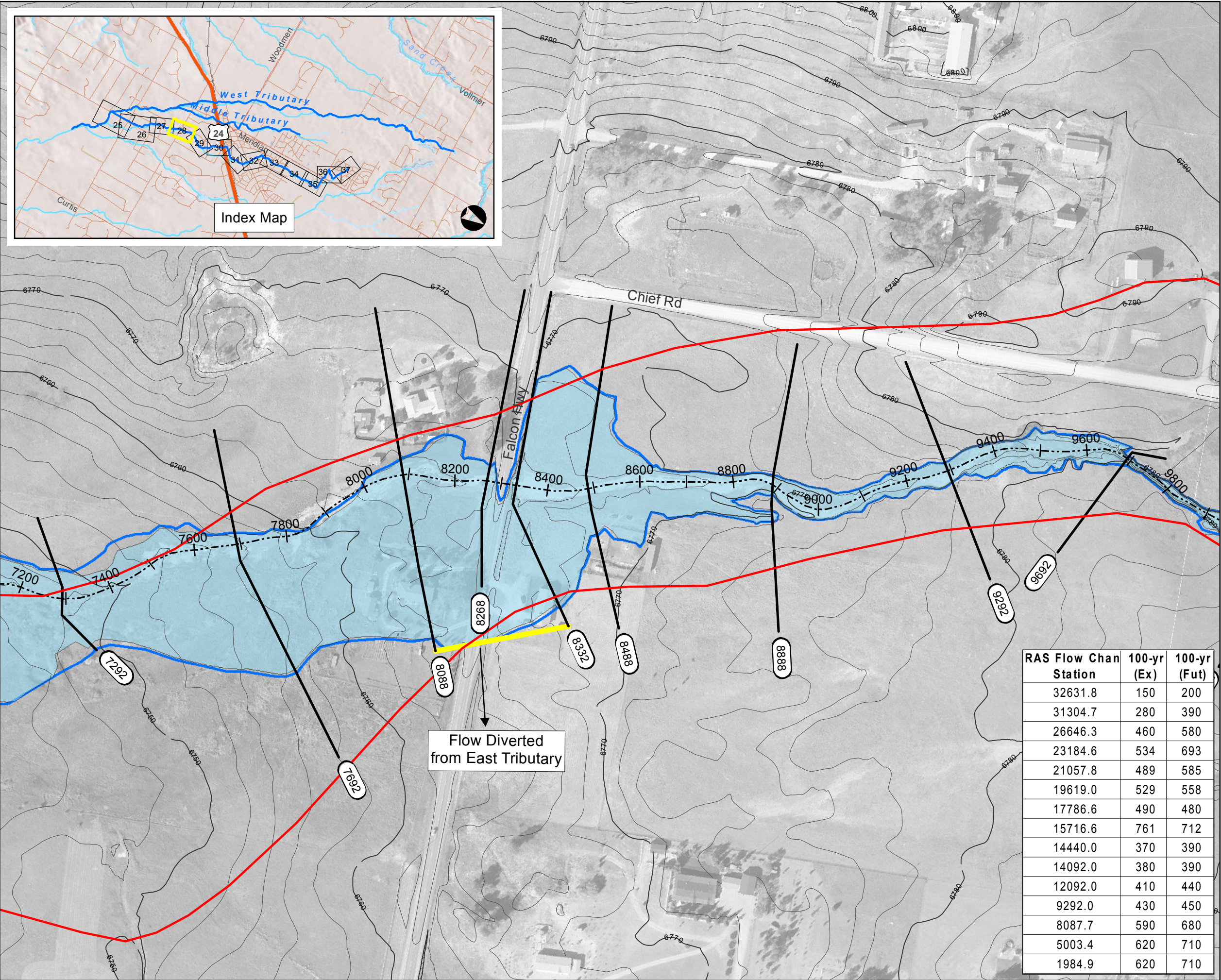
Legend

-  Approximate 100-yr Floodplain Existing
-  Approximate 100-yr Floodplain Existing (Based on Assumed Split Flow Condition)
-  Approximate 100-yr Floodplain Future
-  Approximate 100-yr Floodplain Future (Based on Assumed Split Flow Condition)
-  Shallow Flooding
-  HEC-RAS Centerline
-  XSCutLines (East Trib)
-  FEMA Regulatory Floodplain (Effective as of 1999)*
-  Study Limit

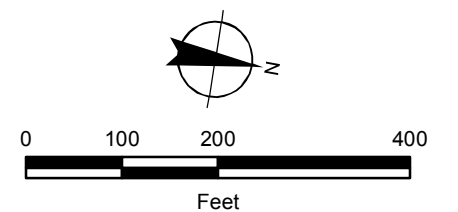
*Letters of Map Change completed after 1999 are not shown



FILE: G:\gis_projects\Falcon_Creek_DBPS\active\apps\20111222\MapBook_FP_East_Trib.mxd, 12/22/2011, wison_wheeler

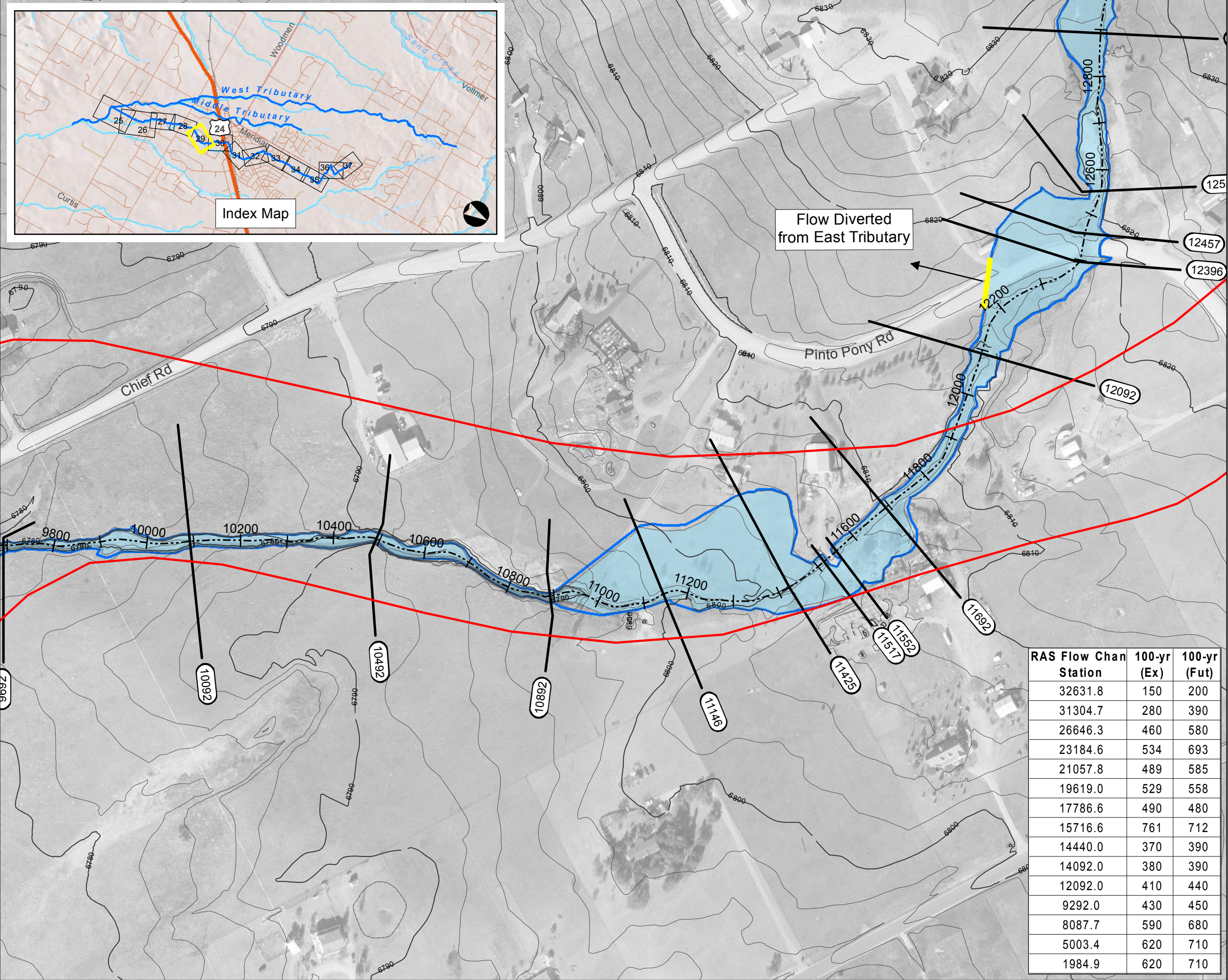
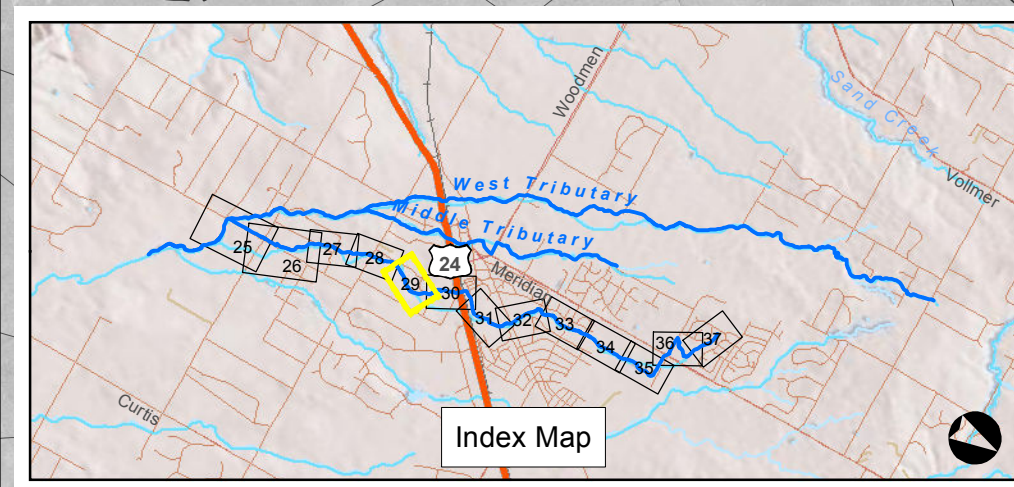


| RAS Flow Chan Station | 100-yr (Ex) | 100-yr (Fut) |
|-----------------------|-------------|--------------|
| 32631.8 | 150 | 200 |
| 31304.7 | 280 | 390 |
| 26646.3 | 460 | 580 |
| 23184.6 | 534 | 693 |
| 21057.8 | 489 | 585 |
| 19619.0 | 529 | 558 |
| 17786.6 | 490 | 480 |
| 15716.6 | 761 | 712 |
| 14440.0 | 370 | 390 |
| 14092.0 | 380 | 390 |
| 12092.0 | 410 | 440 |
| 9292.0 | 430 | 450 |
| 8087.7 | 590 | 680 |
| 5003.4 | 620 | 710 |
| 1984.9 | 620 | 710 |



Sheet 4-29

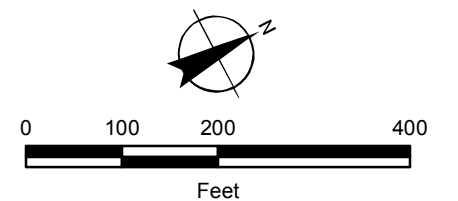
East Tributary Floodplain Falcon DBPS El Paso County, CO



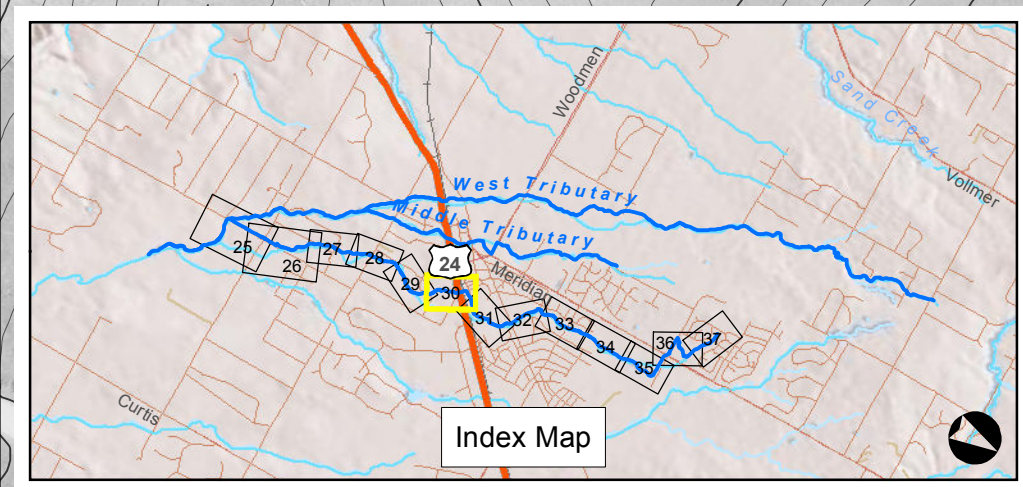
Legend

- Approximate 100-yr Floodplain Existing
- Approximate 100-yr Floodplain Existing (Based on Assumed Split Flow Condition)
- Approximate 100-yr Floodplain Future
- Approximate 100-yr Floodplain Future (Based on Assumed Split Flow Condition)
- Shallow Flooding
- HEC-RAS Centerline
- XSCutLines (East Trib)
- FEMA Regulatory Floodplain (Effective as of 1999)*
- Study Limit

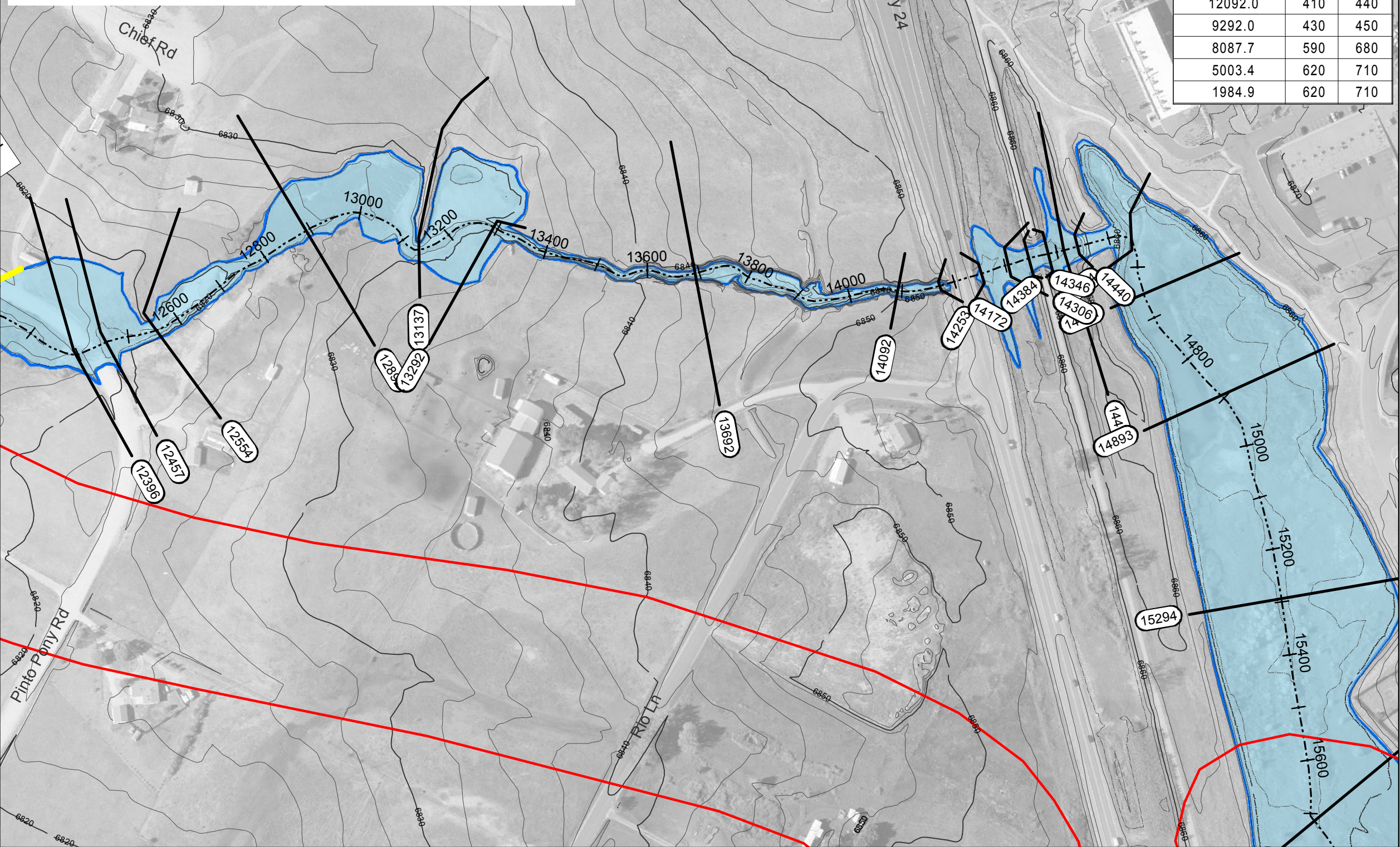
*Letters of Map Change completed after 1999 are not shown



FILE: G:\gis_projects\Falcon_Creek_DBPS\active\apps\20111222\MapBook_FP_East_Trib.mxd, 12/22/2011, wilson_wheeler



| RAS Flow Chan Station | 100-yr (Ex) | 100-yr (Fut) |
|-----------------------|-------------|--------------|
| 32631.8 | 150 | 200 |
| 31304.7 | 280 | 390 |
| 26646.3 | 460 | 580 |
| 23184.6 | 534 | 693 |
| 21057.8 | 489 | 585 |
| 19619.0 | 529 | 558 |
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| 14092.0 | 380 | 390 |
| 12092.0 | 410 | 440 |
| 9292.0 | 430 | 450 |
| 8087.7 | 590 | 680 |
| 5003.4 | 620 | 710 |
| 1984.9 | 620 | 710 |

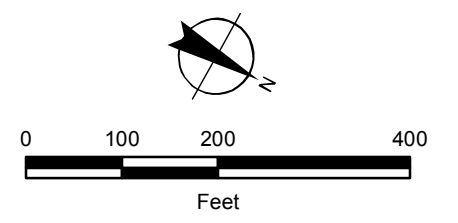


Sheet 4-30

East Tributary Floodplain Falcon DBPS El Paso County, CO

- Legend**
- Approximate 100-yr Floodplain Existing
 - Approximate 100-yr Floodplain Existing (Based on Assumed Split Flow Condition)
 - Approximate 100-yr Floodplain Future
 - Approximate 100-yr Floodplain Future (Based on Assumed Split Flow Condition)
 - Shallow Flooding
 - HEC-RAS Centerline
 - XSCutLines (East Trib)
 - FEMA Regulatory Floodplain (Effective as of 1999)*
 - Study Limit

*Letters of Map Change completed after 1999 are not shown



FILE: G:\gis_projects\Falcon_Creek_DBPS\active\apps\20111222\MapBook_FP_East_Trib.mxd, 12/22/2011, wilson_wheeler

CLOMR Excerpts

**REQUEST FOR CONDITIONAL LETTER OF MAP REVISION
UNNAMED TRIBUTARY TO BLACK SQUIRREL CREEK
FALCON FIELD**

1.0 INTRODUCTION

1.1 Background

The following report and supporting documentation are being submitted to FEMA for the purpose of requesting a Conditional Letter of Map Revision (CLOMR) for a portion of the Unnamed Tributary to Black Squirrel Creek (UTBSC) in El Paso County, Colorado.

Falcon Field consists of approximately 57 acres adjacent to and southeast of Highway 24 near Rio Lane as shown in **Figure 1**. The UTBSC flows southeast across the property and is proposed to be contained within an 8'x4' box culvert and open channel that will discharge into the existing tributary. A general site layout of the Falcon Field development is shown in the construction drawings included in **Appendix 1**.

The improvements associated with Falcon Field are in general conformance with the Falcon Basin, Drainage Basin Planning Study (Falcon DBPS), prepared by El Paso County in 2015. The hydrologic analysis completed for the Falcon DBPS was used as the basis for the current CLOMR.

The Effective FEMA Flood Insurance Rate Map (FIRM) Numbers 08041C0553G and 08041C0561G in **Appendix 5** show the UTBSC 100-year Zone A floodplain across the center of the Falcon Field. This report includes detailed hydraulic models showing that the proposed 100-year floodplain will be contained within a proposed box culvert and open channel.

It is the Owner/Developer's intent to comply with all floodplain regulations.

1.2 General Location and Project Description

This CLOMR is limited to the 57-acre parcel located at the southwest corner of Highway 24 and Rio Lane, in the east half of Section 7, Township 13 South, Range 64 West of the 6th P.M. in El Paso County, Colorado. The subject property will be developed with a mixed-use commercial and residential development (Falcon Field).

The Falcon Field development includes regrading the site and containing the UTBSC across the site. Approximately 1024 feet of the tributary will be impacted by the development, which intercepts the existing creek south of Highway 24 and conveys it via an 8'x4' box culvert and open channel to the existing creek downstream. The box culvert and open channel are designed to convey the full 100-year discharge.

REQUEST FOR CONDITIONAL LETTER OF MAP REVISION
UNNAMED TRIBUTARY TO BLACK SQUIRREL CREEK,
FALCON FIELD



Figure 1 – Vicinity Map

1.3 Regulatory Floodplain

The Effective Zone A limits for the UTBSC on the Falcon Field site are defined on Map Numbers 8041C0553G and 08041C0561G dated December 7, 2018. No flow rates, floodway data or flood profiles were defined for this section of UTBSC in the effective FIS for El Paso County, Colorado, Revised December 7, 2018.

2.0 PREVIOUS STUDIES

El Paso County completed hydrologic and hydraulic analyses summarized in a report titled Falcon Basin, Drainage Basin Planning Study, Selected Plan Report, Final, September 2015 (Falcon DBPS). The Falcon DBPS encompasses three unnamed tributaries to Black Squirrel Creek, including the “East Tributary” which flows across the subject property. Select output from the Falcon DBPS is included in **Appendix 2**.

3.0 HYDROLOGIC ANALYSIS

The Falcon DBPS completed hydrologic analysis for the Falcon Basin Watershed, using HEC-HMS v3.5 software, for historical, existing, and future land use conditions by applying a 24-hour storm event with 2-, 5-, 10-, 25-, 50-, and 100-year recurrence intervals and current drainage infrastructure. Chapter 3 and Appendix A of the Falcon DBPS include a detailed discussion of the hydrologic analysis. An electronic copy of the HEC-HMS model (File: Aug15_Working_Falcon_DBPS_S.hms) is also provided.

El Paso County requires regional drainage infrastructure to be sized for future land use conditions. Therefore, peak discharges with existing drainage infrastructure and future land use conditions near Falcon Field are summarized in Table 3-1.

Table 3-1. Future Land Use Conditions Peak Discharges near Falcon Field on the East Tributary, Falcon DBPS

| Model Location | Physical Location | Proximity to Falcon Field | Q100 (cfs) |
|----------------|-------------------|---------------------------|------------|
| JET090 | Highway 24 | Upstream of Site | 390 |
| JET100 | Pinto Pony Road | Downstream of Site | 390 |

4.0 HYDRAULIC ANALYSIS

4.1 General

The effective FIRM identifies an approximate Zone A floodplain across the Falcon Field property with no flood profiles, discharges, or BFE's defined. The Falcon Field development includes filling and regrading the site and rerouting the UTBSC through a box culvert and open channel across the site.

4.2 Vertical Datum

The effective FIRM is on the North American Vertical Datum of 1988 (NAVD88). The survey completed for the site, the design and construction drawings, and the hydraulic analysis completed for this CLOMR are all on the NAVD88. The Falcon DBPS was completed on the NGVD29.

4.3 Horizontal Datum

The field survey, design, construction drawings and hydraulic modeling for the Falcon Field project were completed on the North American Datum of 1983 (NAD83), Colorado State Plane coordinate system, Central Zone.

4.4 Box Culvert Hydraulic Analysis

Under existing conditions, the UTBSC discharges to an open channel through the site from 2-12'H x 4.83'W box culverts under Highway 24. The Falcon Field property limits are approximately 46 feet downstream of the Highway 24 box culvert exit. There is an 8-foot concrete vertical wall/drop immediately downstream of the culvert, then a short riprap channel section (shown in the photo below), before the open channel returns to a vegetated

REQUEST FOR CONDITIONAL LETTER OF MAP REVISION
UNNAMED TRIBUTARY TO BLACK SQUIRREL CREEK,
FALCON FIELD

section through the site. This section of the tributary was realigned with the construction of the upstream railroad and highway and does not follow the historic flow path.



Existing 2-12'H x 4.83' W box culverts under Highway 24

The proposed 8' x 4' box culvert will begin at the upstream property boundary (approximately the fence line shown in the photo above) at a headwall and convey the tributary flows 750 feet downstream to a proposed open channel. StormCAD was used to evaluate the hydraulic performance of the box culvert. The profile and output for the 100-year storm event is included in **Appendix 3**, and the model files are provided.

4.5 Open Channel Hydraulic Analysis

The proposed box culvert discharges to a proposed open channel via a headwall. The proposed open channel conveys the UTBSC 275 feet downstream to the existing creek, and will be vegetated with mowable short grasses. The open channel has a 20-foot bottom width in a v-shape with two 10-foot sections set at a 2% slope to the invert. The side slopes above the v-shape bottom are set at a 3H:1V slope. HEC-RAS version 6.2 was used to model the proposed open channel and existing creek downstream. The profile and output for the 100-year storm event is included in **Appendix 3**, and the model files are provided.

The proposed geometry includes six cross sections over a modeled reach of 400 feet. Roughness coefficients (n-values) of 0.04 and 0.08 were used for the proposed and existing channel, respectively. The model was computed in a subcritical flow regime for the design flow of 390 cfs, with a normal depth starting water surface elevation.

5.0 NFIP REGULATION COMPLIANCE

5.1 Floodplain Work Map and Annotated FIRM

The effective Zone A 100-year floodplain delineation for the UTBSC begins downstream of Highway 24. The 100-year flood discharge will be contained in the proposed box culvert. The proposed floodplain for the on-site open channel is delineated on the Floodplain Work Map and Annotated FIRM in **Appendix 5**. The proposed Zone AE floodplain ties into the effective Zone A floodplain approximately 225 feet downstream of the Falcon Field downstream property limits.

5.2 Forms and Notifications

The appropriate FEMA forms are located in **Appendix 4**. Modifications to 100-year floodplain elevations and delineations are limited to the Falcon Field development. Furthermore, there are no proposed increases to the BFE's or floodplain extents. Therefore, individual legal notices are not required for this CLOMR submittal.

5.3 Compliance with Section 65.12

Although there are no increases to BFE's due to the proposed project, an alternatives evaluation was performed to evaluate options for closed conduit and open channel conveyance of the UTBSC. The alternatives evaluation can be provided upon request.

Furthermore, no structures are located in areas that would be impacted by the floodplain modifications proposed by this CLOMR.

5.4 Endangered Species Act (ESA)

ESA Compliance information is provided in **Appendix 6**.

REQUEST FOR CONDITIONAL LETTER OF MAP REVISION
UNNAMED TRIBUTARY TO BLACK SQUIRREL CREEK,
FALCON FIELD

6.0 CONCLUSIONS

The Falcon Field development will relocate a portion of an Unnamed Tributary to Black Squirrel Creek (East Tributary). This report and supporting documentation are being submitted to FEMA for the purpose of requesting a CLOMR to conditionally change the floodplain in accordance with NFIP regulations.

7.0 REFERENCES

Bentley (formerly Haestad Methods, Inc.), StormCAD v4.1.1.

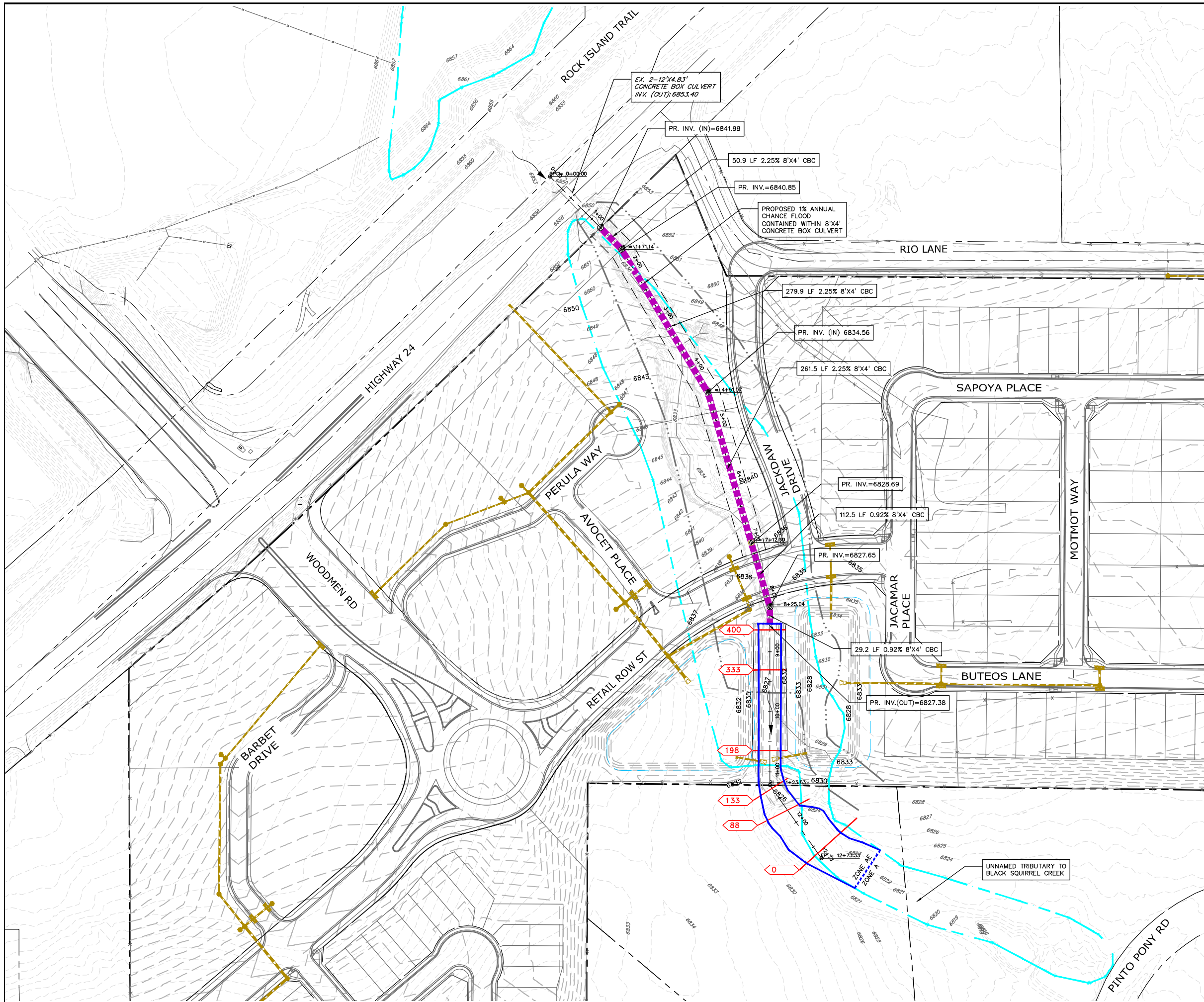
El Paso County, *Drainage Criteria Manual*, October 2018.

FEMA, *FIRM Numbers 08041C0553G and 08041C0561G, El Paso County, Colorado and Incorporated Areas*, Revised December 7, 2018.

FEMA, *FIS Number 08041CV001A, El Paso County, Colorado and Incorporated Areas*, Revised December 7, 2018.

Matrix Design Group, *Falcon Drainage Basin Planning Study, Selected Plan Report, Final*, September 2015.

USACE, *Hydrologic Engineering Center River Analysis System (HEC-RAS), Version 6.2*, March 2022.



LEGEND

- EX. CONTOUR: Dashed line with elevation (e.g., 6805)
- PR. CONTOUR: Dashed line with elevation (e.g., 6905)
- EX. STORM SEWER OR: Dashed line with 'S'
- PR. CBC STORM SEWER: Purple dashed line
- PR. ON-SITE STORM DRAINAGE: Yellow dashed line
- EFFECTIVE FEMA FLOODPLAIN (ZONE A): Red dashed line
- PROPOSED FLOODPLAIN (ZONE AE): Blue solid line
- CROSS SECTION: Red line with '400'
- CROSS SECTION LABEL: Red box with '400'
- FLOW DIRECTION: Arrow
- PR. ON-SITE DETENTION: Blue dashed line
- PR. EASEMENT: Dashed line
- PR. LOT: Dashed line
- EX. PROPERTY LINE: Dashed line
- PR. SITE BOUNDARY: Thick black line
- PROPOSED CURB LINE & SIDEWALK: Thick black line

NOTES

- SPATIAL PROJECTION IS NAD83 COLORADO STATE PLANE, CENTRAL ZONE (FEET).
- VERTICAL DATUM IS NAVD88.

PREPARED BY:

DBC
Drexel, Barrell & Co.
Engineers - Surveyors
1376 MINERS DRIVE, STE 107
LAFAYETTE, COLORADO 800026
CONTACT: MICHELLE IBLINGS, P.E.
(303) 442-4338
LAFAYETTE
COLORADO SPRINGS

OWNER/CLIENT:

PJ ANDERSON
FALCON FIELD, LLC
3230 ELECTRA DR. N
COLORADO SPRINGS, CO 80906

EXHIBIT FOR:

FALCON FIELD

FALCON, COLORADO

| ISSUE | DATE |
|---------|--------|
| EXHIBIT | 8/3/23 |
| | |
| | |
| | |
| | |

| | |
|--------------|---------------|
| DESIGNED BY: | MLI |
| DRAWN BY: | CAF |
| CHECKED BY: | MLI |
| FILE NAME: | 21705-00 FPWM |



DRAWING SCALE:
HORIZONTAL: SEE PLAN
VERTICAL: N/A

CLOMR FLOODPLAIN WORK MAP

PROJECT: 21705-00BLWR
DRAWING NO.



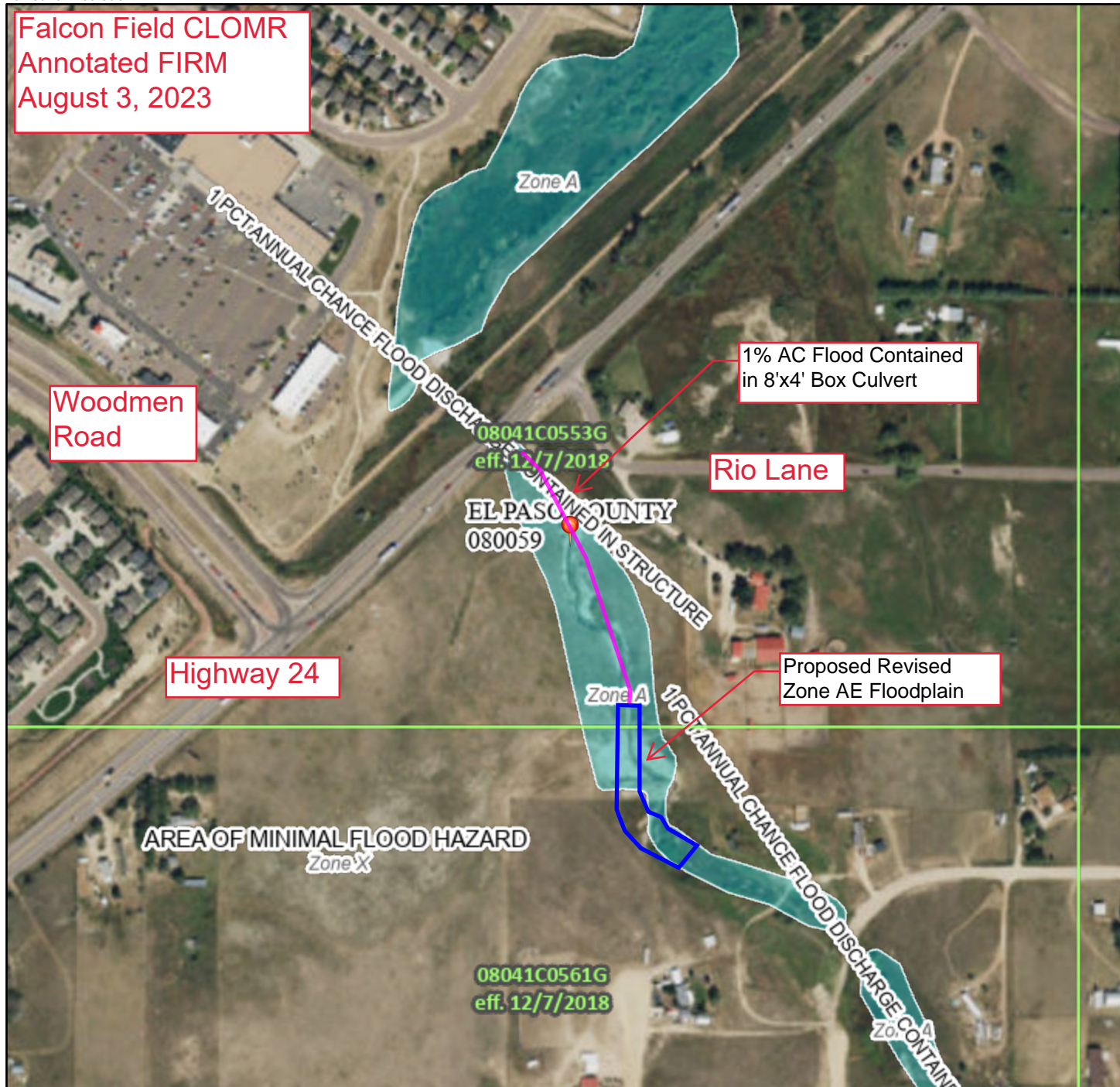
H:\21705-00BLWR\Plans\Sheets\21705-00 FPWM.dwg, 8/3/2023 2:02:51 PM

National Flood Hazard Layer FIRMMette



104°36'13"W 38°56'34"N

Falcon Field CLOMR
Annotated FIRM
August 3, 2023



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

| | | |
|----------------------------|--|---|
| SPECIAL FLOOD HAZARD AREAS | | Without Base Flood Elevation (BFE) <i>Zone A, V, A99</i> |
| | | With BFE or Depth <i>Zone AE, AO, AH, VE, AR</i> |
| | | Regulatory Floodway |

| | | |
|-----------------------------|--|--|
| OTHER AREAS OF FLOOD HAZARD | | 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile <i>Zone X</i> |
| | | Future Conditions 1% Annual Chance Flood Hazard <i>Zone X</i> |
| | | Area with Reduced Flood Risk due to Levee. See Notes. <i>Zone X</i> |
| | | Area with Flood Risk due to Levee <i>Zone D</i> |

| | | |
|--------------------|--|--|
| OTHER AREAS | | NO SCREEN Area of Minimal Flood Hazard <i>Zone X</i> |
| | | Effective LOMRs |
| GENERAL STRUCTURES | | Area of Undetermined Flood Hazard <i>Zone D</i> |
| | | Channel, Culvert, or Storm Sewer |
| | | Levee, Dike, or Floodwall |

| | | |
|----------------|--|---|
| OTHER FEATURES | | 20.2 Cross Sections with 1% Annual Chance Water Surface Elevation |
| | | 17.5 Cross Sections with 1% Annual Chance Water Surface Elevation |
| | | Coastal Transect |
| | | Base Flood Elevation Line (BFE) |
| | | Limit of Study |
| | | Jurisdiction Boundary |
| OTHER FEATURES | | Coastal Transect Baseline |
| | | Profile Baseline |
| | | Hydrographic Feature |

| | | |
|------------|--|---------------------------|
| MAP PANELS | | Digital Data Available |
| | | No Digital Data Available |
| | | Unmapped |

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **10/11/2021 at 10:04 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



DEPARTMENT OF THE ARMY
CORPS OF ENGINEERS, ALBUQUERQUE DISTRICT
SOUTHERN COLORADO REGULATORY BRANCH
201 WEST 8TH STREET, SUITE 350
PUEBLO, COLORADO 81003

August 26, 2022

Regulatory Division

SUBJECT: Jurisdictional Determination – Action No. SPA-2021-00180, Falcon Field

P. J. Anderson
Falcon Field, LLC
31 North Tejon Street, Suite 516
Colorado Springs, CO 80903
pja5713@gmail.com

Dear P.J. Anderson:

This letter responds to your request for a jurisdictional determination (JD) for property located at latitude 38.936555635255, longitude -104.600429740897, in El Paso County, Colorado. We have assigned Action No. SPA-2021-00180 to your request. Please reference this number in all future correspondence concerning the site.

Based on the information provided, we have determined that the site does not contain waters of the United States that are subject to regulation under Section 404 of the Clean Water Act. The attached JD form describes the area that was evaluated and determined to contain no waters of the United States. If you intend to conduct work that could result in a discharge of dredged or fill material into waters of the United States, please contact this office for a determination of Department of the Army permit requirements and refer to Action No. SPA-2021-00180.

The basis for this approved JD (attached) is that the project site contains isolated wetlands and/or other waters. Wetland 1 through 5 are intrastate, isolated waters that do not flow into a traditional navigable waterway (attached). A copy of this JD is also available at <http://www.spa.usace.army.mil/reg/JD>. This approved JD is valid for 5 years unless new information warrants revision of the determination before the expiration date.

You may accept or appeal this approved JD or provide new information in accordance with the attached Notification of Administration Appeal Options and Process and Request for Appeal. If you elect to appeal this approved JD, you must complete Section II of the form and return it to the Army Engineer Division, South Pacific, CESP-D-PDS-O, Attn: Travis Morse, Administrative Appeal Review Officer, P.O. Box 36023, 450 Golden Gate Avenue, San Francisco, CA 94102 within 60 days of the date of this notice. Failure to notify the Corps within 60 days of the date of this notice means that you accept the approved JD in its entirety and waive all rights to appeal the approved JD.

If you have any questions, please contact Kraig Jashinsky at (719) 439-7281 or by email at Kraig.A.Jashinsky@usace.army.mil. At your convenience, please complete a Customer Service Survey online at <https://regulatory.ops.usace.army.mil/customer-service-survey/>.

Sincerely,

Kara A. Hellige
Chief, Southern Colorado Regulatory Branch

cc:
Daniel Maynard, Bristlecone Ecology, LLC, dmaynard@bristleconeecology.com

APPROVED JURISDICTIONAL DETERMINATION FORM
U.S. Army Corps of Engineers

This form should be completed by following the instructions provided in Section IV of the JD Form Instructional Guidebook.

SECTION I: BACKGROUND INFORMATION

A. REPORT COMPLETION DATE FOR APPROVED JURISDICTIONAL DETERMINATION (JD): August 26, 2022

B. DISTRICT OFFICE, FILE NAME, AND NUMBER: Albuquerque District, Falcon Field AJD Request, SPA-2021-00180

C. PROJECT LOCATION AND BACKGROUND INFORMATION:

State: **Colorado** County/parish/borough: **El Paso County** City:
Center coordinates of site (lat/long in degree decimal format): Lat. **38.936555635255°**, Long. **-104.600429740897°**
Universal Transverse Mercator: **13 534630.43 4309812.02**

Name of nearest waterbody: **Jimmy Camp Creek**

Name of nearest Traditional Navigable Water (TNW) into which the aquatic resource flows:

Name of watershed or Hydrologic Unit Code (HUC): **Chico, 11020004**

Check if map/diagram of review area and/or potential jurisdictional areas is/are available upon request.

Check if other sites (e.g., offsite mitigation sites, disposal sites, etc...) are associated with this action and are recorded on a different JD form:

D. REVIEW PERFORMED FOR SITE EVALUATION (CHECK ALL THAT APPLY):

Office (Desk) Determination. Date: **July 14, 2022**

Field Determination. Date(s): **June 28, 2022**

SECTION II: SUMMARY OF FINDINGS

A. RHA SECTION 10 DETERMINATION OF JURISDICTION.

There **Are no** "navigable waters of the U.S." within Rivers and Harbors Act (RHA) jurisdiction (as defined by 33 CFR part 329) in the review area. [Required]

Waters subject to the ebb and flow of the tide.

Waters are presently used, or have been used in the past, or may be susceptible for use to transport interstate or foreign commerce.

Explain:

B. CWA SECTION 404 DETERMINATION OF JURISDICTION.

There **Are no** "waters of the U.S." within Clean Water Act (CWA) jurisdiction (as defined by 33 CFR part 328) in the review area. [Required]

1. Waters of the U.S.

a. Indicate presence of waters of U.S. in review area (check all that apply):¹

TNWs, including territorial seas

Wetlands adjacent to TNWs

Relatively permanent waters² (RPWs) that flow directly or indirectly into TNWs

Non-RPWs that flow directly or indirectly into TNWs

Wetlands directly abutting RPWs that flow directly or indirectly into TNWs

Wetlands adjacent to but not directly abutting RPWs that flow directly or indirectly into TNWs

Wetlands adjacent to non-RPWs that flow directly or indirectly into TNWs

Impoundments of jurisdictional waters

Isolated (interstate or intrastate) waters, including isolated wetlands

b. Identify (estimate) size of waters of the U.S. in the review area:

Non-wetland waters: linear feet, wide, and/or acres.

Wetlands: acres.

c. Limits (boundaries) of jurisdiction based on: Pick List

Elevation of established OHWM (if known):

2. Non-regulated waters/wetlands (check if applicable):³

Potentially jurisdictional waters and/or wetlands were assessed within the review area and determined to be not jurisdictional.

Explain: **The review area contains five wetlands totalling 7.15 acres. A review of the downstream connectivity of the associated wetlands found there to be a lack of a connection to downstream waters. The drainage features and associated wetlands presented a southward flow path until reaching E. Blaney Road. The flow path consisting of a varying degree of broken stream channel and connected wetlands terminated across a portion of flat terrain with not apparent wetland vegetation. Flow does not appear to reach any downstream waters via the nearby roadside ditch. Flow also does not cross E. Blaney Road due to the lack of culverts and a slight elevation rise. Due to a lack of downstream connectivity, the drainage features and associated wetlands under review are found to be isolated.**

¹ Boxes checked below shall be supported by completing the appropriate sections in Section III below.

² For purposes of this form, an RPW is defined as a tributary that is not a TNW and that typically flows year-round or has continuous flow at least "seasonally" (e.g., typically 3 months).

³ Supporting documentation is presented in Section III.F.

SECTION III: CWA ANALYSIS

A. TNWs AND WETLANDS ADJACENT TO TNWs

The agencies will assert jurisdiction over TNWs and wetlands adjacent to TNWs. If the aquatic resource is a TNW, complete Section III.A.1 and Section III.D.1. only; if the aquatic resource is a wetland adjacent to a TNW, complete Sections III.A.1 and 2 and Section III.D.1.; otherwise, see Section III.B below.

1. TNW

Identify TNW:

Summarize rationale supporting determination:

2. Wetland adjacent to TNW

Summarize rationale supporting conclusion that wetland is “adjacent”:

B. CHARACTERISTICS OF TRIBUTARY (THAT IS NOT A TNW) AND ITS ADJACENT WETLANDS (IF ANY):

This section summarizes information regarding characteristics of the tributary and its adjacent wetlands, if any, and it helps determine whether or not the standards for jurisdiction established under *Rapanos* have been met.

The agencies will assert jurisdiction over non-navigable tributaries of TNWs where the tributaries are “relatively permanent waters” (RPWs), i.e. tributaries that typically flow year-round or have continuous flow at least seasonally (e.g., typically 3 months). A wetland that directly abuts an RPW is also jurisdictional. If the aquatic resource is not a TNW, but has year-round (perennial) flow, skip to Section III.D.2. If the aquatic resource is a wetland directly abutting a tributary with perennial flow, skip to Section III.D.4.

A wetland that is adjacent to but that does not directly abut an RPW requires a significant nexus evaluation. Corps districts and EPA regions will include in the record any available information that documents the existence of a significant nexus between a relatively permanent tributary that is not perennial (and its adjacent wetlands if any) and a traditional navigable water, even though a significant nexus finding is not required as a matter of law.

If the waterbody⁴ is not an RPW, or a wetland directly abutting an RPW, a JD will require additional data to determine if the waterbody has a significant nexus with a TNW. If the tributary has adjacent wetlands, the significant nexus evaluation must consider the tributary in combination with all of its adjacent wetlands. This significant nexus evaluation that combines, for analytical purposes, the tributary and all of its adjacent wetlands is used whether the review area identified in the JD request is the tributary, or its adjacent wetlands, or both. If the JD covers a tributary with adjacent wetlands, complete Section III.B.1 for the tributary, Section III.B.2 for any onsite wetlands, and Section III.B.3 for all wetlands adjacent to that tributary, both onsite and offsite. The determination whether a significant nexus exists is determined in Section III.C below.

1. Characteristics of non-TNWs that flow directly or indirectly into TNW

(i) General Area Conditions:

Watershed size: **Pick List**
Drainage area: **Pick List**
Average annual rainfall: inches
Average annual snowfall: inches

(ii) Physical Characteristics:

(a) Relationship with TNW:

- Tributary flows directly into TNW.
- Tributary flows through **Pick List** tributaries before entering TNW.

Project waters are **Pick List** river miles from TNW.
Project waters are **Pick List** river miles from RPW.
Project waters are **Pick List** aerial (straight) miles from TNW.
Project waters are **Pick List** aerial (straight) miles from RPW.
Project waters cross or serve as state boundaries. Explain:

Identify flow route to TNW⁵:
Tributary stream order, if known:

⁴ Note that the Instructional Guidebook contains additional information regarding swales, ditches, washes, and erosional features generally and in the arid West.

⁵ Flow route can be described by identifying, e.g., tributary a, which flows through the review area, to flow into tributary b, which then flows into TNW.

(b) General Tributary Characteristics (check all that apply):

- Tributary is:** Natural
 Artificial (man-made). Explain:
 Manipulated (man-altered). Explain:

Tributary properties with respect to top of bank (estimate):

Average width: feet
Average depth: feet
Average side slopes: **Pick List.**

Primary tributary substrate composition (check all that apply):

- | | | |
|--|--|-----------------------------------|
| <input type="checkbox"/> Silts | <input type="checkbox"/> Sands | <input type="checkbox"/> Concrete |
| <input type="checkbox"/> Cobbles | <input type="checkbox"/> Gravel | <input type="checkbox"/> Muck |
| <input type="checkbox"/> Bedrock | <input type="checkbox"/> Vegetation. Type/% cover: | |
| <input type="checkbox"/> Other. Explain: | | |

Tributary condition/stability [e.g., highly eroding, sloughing banks]. Explain:

Presence of run/riffle/pool complexes. Explain:

Tributary geometry: **Pick List**

Tributary gradient (approximate average slope): %

(c) Flow:

Tributary provides for: **Pick List**

Estimate average number of flow events in review area/year: **Pick List**

Describe flow regime:

Other information on duration and volume:

Surface flow is: **Pick List.** Characteristics:

Subsurface flow: **Pick List.** Explain findings:

- Dye (or other) test performed:

Tributary has (check all that apply):

- | | |
|---|---|
| <input type="checkbox"/> Bed and banks | |
| <input type="checkbox"/> OHWM ⁶ (check all indicators that apply): | |
| <input type="checkbox"/> clear, natural line impressed on the bank | <input type="checkbox"/> the presence of litter and debris |
| <input type="checkbox"/> changes in the character of soil | <input type="checkbox"/> destruction of terrestrial vegetation |
| <input type="checkbox"/> shelving | <input type="checkbox"/> the presence of wrack line |
| <input type="checkbox"/> vegetation matted down, bent, or absent | <input type="checkbox"/> sediment sorting |
| <input type="checkbox"/> leaf litter disturbed or washed away | <input type="checkbox"/> scour |
| <input type="checkbox"/> sediment deposition | <input type="checkbox"/> multiple observed or predicted flow events |
| <input type="checkbox"/> water staining | <input type="checkbox"/> abrupt change in plant community |
| <input type="checkbox"/> other (list): | |
| <input type="checkbox"/> Discontinuous OHWM. ⁷ Explain: | |

If factors other than the OHWM were used to determine lateral extent of CWA jurisdiction (check all that apply):

- | | |
|--|--|
| <input type="checkbox"/> High Tide Line indicated by: | <input type="checkbox"/> Mean High Water Mark indicated by: |
| <input type="checkbox"/> oil or scum line along shore objects | <input type="checkbox"/> survey to available datum; |
| <input type="checkbox"/> fine shell or debris deposits (foreshore) | <input type="checkbox"/> physical markings; |
| <input type="checkbox"/> physical markings/characteristics | <input type="checkbox"/> vegetation lines/changes in vegetation types. |
| <input type="checkbox"/> tidal gauges | |
| <input type="checkbox"/> other (list): | |

(iii) **Chemical Characteristics:**

Characterize tributary (e.g., water color is clear, discolored, oily film; water quality; general watershed characteristics, etc.).

Explain:

Identify specific pollutants, if known:

(iv) **Biological Characteristics. Channel supports (check all that apply):**

- Riparian corridor. Characteristics (type, average width):
 Wetland fringe. Characteristics:
 Habitat for:

⁶A natural or man-made discontinuity in the OHWM does not necessarily sever jurisdiction (e.g., where the stream temporarily flows underground, or where the OHWM has been removed by development or agricultural practices). Where there is a break in the OHWM that is unrelated to the waterbody's flow regime (e.g., flow over a rock outcrop or through a culvert), the agencies will look for indicators of flow above and below the break.

⁷Ibid.

- Federally Listed species. Explain findings:
- Fish/spawn areas. Explain findings:
- Other environmentally-sensitive species. Explain findings:
- Aquatic/wildlife diversity. Explain findings:

2. Characteristics of wetlands adjacent to non-TNW that flow directly or indirectly into TNW

(i) Physical Characteristics:

(a) General Wetland Characteristics:

Properties:

Wetland size: _____ acres

Wetland type. Explain:

Wetland quality. Explain:

Project wetlands cross or serve as state boundaries. Explain:

(b) General Flow Relationship with Non-TNW:

Flow is: **Pick List**. Explain:

Surface flow is: **Pick List**

Characteristics:

Subsurface flow: **Pick List**. Explain findings:

Dye (or other) test performed:

(c) Wetland Adjacency Determination with Non-TNW:

Directly abutting

Not directly abutting

Discrete wetland hydrologic connection. Explain:

Ecological connection. Explain:

Separated by berm/barrier. Explain:

(d) Proximity (Relationship) to TNW

Project wetlands are **Pick List** river miles from TNW.

Project waters are **Pick List** aerial (straight) miles from TNW.

Flow is from: **Pick List**.

Estimate approximate location of wetland as within the **Pick List** floodplain.

(ii) Chemical Characteristics:

Characterize wetland system (e.g., water color is clear, brown, oil film on surface; water quality; general watershed characteristics; etc.). Explain:

Identify specific pollutants, if known:

(iii) Biological Characteristics. Wetland supports (check all that apply):

Riparian buffer. Characteristics (type, average width):

Vegetation type/percent cover. Explain:

Habitat for:

Federally Listed species. Explain findings:

Fish/spawn areas. Explain findings:

Other environmentally-sensitive species. Explain findings:

Aquatic/wildlife diversity. Explain findings:

3. Characteristics of all wetlands adjacent to the tributary (if any)

All wetland(s) being considered in the cumulative analysis: **Pick List**

Approximately _____ acres in total are being considered in the cumulative analysis.

For each wetland, specify the following:

Directly abuts? (Y/N)

Size (in acres)

Directly abuts? (Y/N)

Size (in acres)

Summarize overall biological, chemical and physical functions being performed:

C. SIGNIFICANT NEXUS DETERMINATION

A significant nexus analysis will assess the flow characteristics and functions of the tributary itself and the functions performed by any wetlands adjacent to the tributary to determine if they significantly affect the chemical, physical, and biological integrity of a TNW. For each of the following situations, a significant nexus exists if the tributary, in combination with all of its adjacent wetlands, has more than a speculative or insubstantial effect on the chemical, physical and/or biological integrity of a TNW. Considerations when evaluating significant nexus include, but are not limited to the volume, duration, and frequency of the flow of water in the tributary and its proximity to a TNW, and the functions performed by the tributary and all its adjacent wetlands. It is not appropriate to determine significant nexus based solely on any specific threshold of distance (e.g. between a tributary and its adjacent wetland or between a tributary and the TNW). Similarly, the fact an adjacent wetland lies within or outside of a floodplain is not solely determinative of significant nexus.

Draw connections between the features documented and the effects on the TNW, as identified in the *Rapanos* Guidance and discussed in the Instructional Guidebook. Factors to consider include, for example:

- Does the tributary, in combination with its adjacent wetlands (if any), have the capacity to carry pollutants or flood waters to TNWs, or to reduce the amount of pollutants or flood waters reaching a TNW?
- Does the tributary, in combination with its adjacent wetlands (if any), provide habitat and lifecycle support functions for fish and other species, such as feeding, nesting, spawning, or rearing young for species that are present in the TNW?
- Does the tributary, in combination with its adjacent wetlands (if any), have the capacity to transfer nutrients and organic carbon that support downstream foodwebs?
- Does the tributary, in combination with its adjacent wetlands (if any), have other relationships to the physical, chemical, or biological integrity of the TNW?

Note: the above list of considerations is not inclusive and other functions observed or known to occur should be documented below:

1. **Significant nexus findings for non-RPW that has no adjacent wetlands and flows directly or indirectly into TNWs.** Explain findings of presence or absence of significant nexus below, based on the tributary itself, then go to Section III.D:
2. **Significant nexus findings for non-RPW and its adjacent wetlands, where the non-RPW flows directly or indirectly into TNWs.** Explain findings of presence or absence of significant nexus below, based on the tributary in combination with all of its adjacent wetlands, then go to Section III.D:
3. **Significant nexus findings for wetlands adjacent to an RPW but that do not directly abut the RPW.** Explain findings of presence or absence of significant nexus below, based on the tributary in combination with all of its adjacent wetlands, then go to Section III.D:

D. DETERMINATIONS OF JURISDICTIONAL FINDINGS. THE SUBJECT WATERS/WETLANDS ARE (CHECK ALL THAT APPLY):

1. **TNWs and Adjacent Wetlands.** Check all that apply and provide size estimates in review area:

- TNWs: linear feet, wide, Or acres.
- Wetlands adjacent to TNWs: acres.

2. **RPWs that flow directly or indirectly into TNWs.**

- Tributaries of TNWs where tributaries typically flow year-round are jurisdictional. Provide data and rationale indicating that tributary is perennial:
- Tributaries of TNW where tributaries have continuous flow “seasonally” (e.g., typically three months each year) are jurisdictional. Data supporting this conclusion is provided at Section III.B. Provide rationale indicating that tributary flows seasonally:

Provide estimates for jurisdictional waters in the review area (check all that apply):

- Tributary waters: linear feet wide.
 - Other non-wetland waters: acres.
- Identify type(s) of waters:

3. **Non-RPWs⁸ that flow directly or indirectly into TNWs.**

- Waterbody that is not a TNW or an RPW, but flows directly or indirectly into a TNW, and it has a significant nexus with a TNW is jurisdictional. Data supporting this conclusion is provided at Section III.C.

Provide estimates for jurisdictional waters within the review area (check all that apply):

- Tributary waters: linear feet, wide.
 - Other non-wetland waters: acres.
- Identify type(s) of waters:

⁸See Footnote # 3.

4. Wetlands directly abutting an RPW that flow directly or indirectly into TNWs.

- Wetlands directly abut RPW and thus are jurisdictional as adjacent wetlands.
- Wetlands directly abutting an RPW where tributaries typically flow year-round. Provide data and rationale indicating that tributary is perennial in Section III.D.2, above. Provide rationale indicating that wetland is directly abutting an RPW:

- Wetlands directly abutting an RPW where tributaries typically flow "seasonally." Provide data indicating that tributary is seasonal in Section III.B and rationale in Section III.D.2, above. Provide rationale indicating that wetland is directly abutting an RPW:

Provide acreage estimates for jurisdictional wetlands in the review area: acres.

5. Wetlands adjacent to but not directly abutting an RPW that flow directly or indirectly into TNWs.

- Wetlands that do not directly abut an RPW, but when considered in combination with the tributary to which they are adjacent and with similarly situated adjacent wetlands, have a significant nexus with a TNW are jurisdictional. Data supporting this conclusion is provided at Section III.C.

Provide acreage estimates for jurisdictional wetlands in the review area: acres.

6. Wetlands adjacent to non-RPWs that flow directly or indirectly into TNWs.

- Wetlands adjacent to such waters, and have when considered in combination with the tributary to which they are adjacent and with similarly situated adjacent wetlands, have a significant nexus with a TNW are jurisdictional. Data supporting this conclusion is provided at Section III.C.

Provide estimates for jurisdictional wetlands in the review area: acres.

7. Impoundments of jurisdictional waters.⁹

As a general rule, the impoundment of a jurisdictional tributary remains jurisdictional.

- Demonstrate that impoundment was created from "waters of the U.S.," or
- Demonstrate that water meets the criteria for one of the categories presented above (1-6), or
- Demonstrate that water is isolated with a nexus to commerce (see E below).

E. ISOLATED [INTERSTATE OR INTRA-STATE] WATERS, INCLUDING ISOLATED WETLANDS, THE USE, DEGRADATION OR DESTRUCTION OF WHICH COULD AFFECT INTERSTATE COMMERCE, INCLUDING ANY SUCH WATERS (CHECK ALL THAT APPLY):¹⁰

- which are or could be used by interstate or foreign travelers for recreational or other purposes.
- from which fish or shellfish are or could be taken and sold in interstate or foreign commerce.
- which are or could be used for industrial purposes by industries in interstate commerce.
- Interstate isolated waters. Explain:
- Other factors. Explain:

Identify water body and summarize rationale supporting determination:

Provide estimates for jurisdictional waters in the review area (check all that apply):

- Tributary waters: linear feet, wide.
- Other non-wetland waters: acres.
Identify type(s) of waters:
- Wetlands: acres.

F. NON-JURISDICTIONAL WATERS, INCLUDING WETLANDS (CHECK ALL THAT APPLY):

- If potential wetlands were assessed within the review area, these areas did not meet the criteria in the 1987 Corps of Engineers Wetland Delineation Manual and/or appropriate Regional Supplements.
- Review area included isolated waters with no substantial nexus to interstate (or foreign) commerce.
 - Prior to the Jan 2001 Supreme Court decision in "*SWANCC*," the review area would have been regulated based solely on the "Migratory Bird Rule" (MBR).
- Waters do not meet the "Significant Nexus" standard, where such a finding is required for jurisdiction. Explain:
- Other: (explain, if not covered above):

⁹ To complete the analysis refer to the key in Section III.D.6 of the Instructional Guidebook.

¹⁰ Prior to asserting or declining CWA jurisdiction based solely on this category, Corps Districts will elevate the action to Corps and EPA HQ for review consistent with the process described in the Corps/EPA Memorandum Regarding CWA Act Jurisdiction Following Rapanos.

Provide acreage estimates for non-jurisdictional waters in the review area, where the sole potential basis of jurisdiction is the MBR factors (i.e., presence of migratory birds, presence of endangered species, use of water for irrigated agriculture), using best professional judgment (check all that apply):

- Non-wetland waters (i.e., rivers, streams): linear feet, wide.
- Lakes/ponds: acres.
- Other non-wetland waters: acres. List type of aquatic resource:
- Wetlands: **7.15** acres.

Provide acreage estimates for non-jurisdictional waters in the review area that do not meet the “Significant Nexus” standard, where such a finding is required for jurisdiction (check all that apply):

- Non-wetland waters (i.e., rivers, streams): linear feet, wide.
- Lakes/ponds: acres.
- Other non-wetland waters: acres. List type of aquatic resource:
- Wetlands: acres.

SECTION IV: DATA SOURCES.

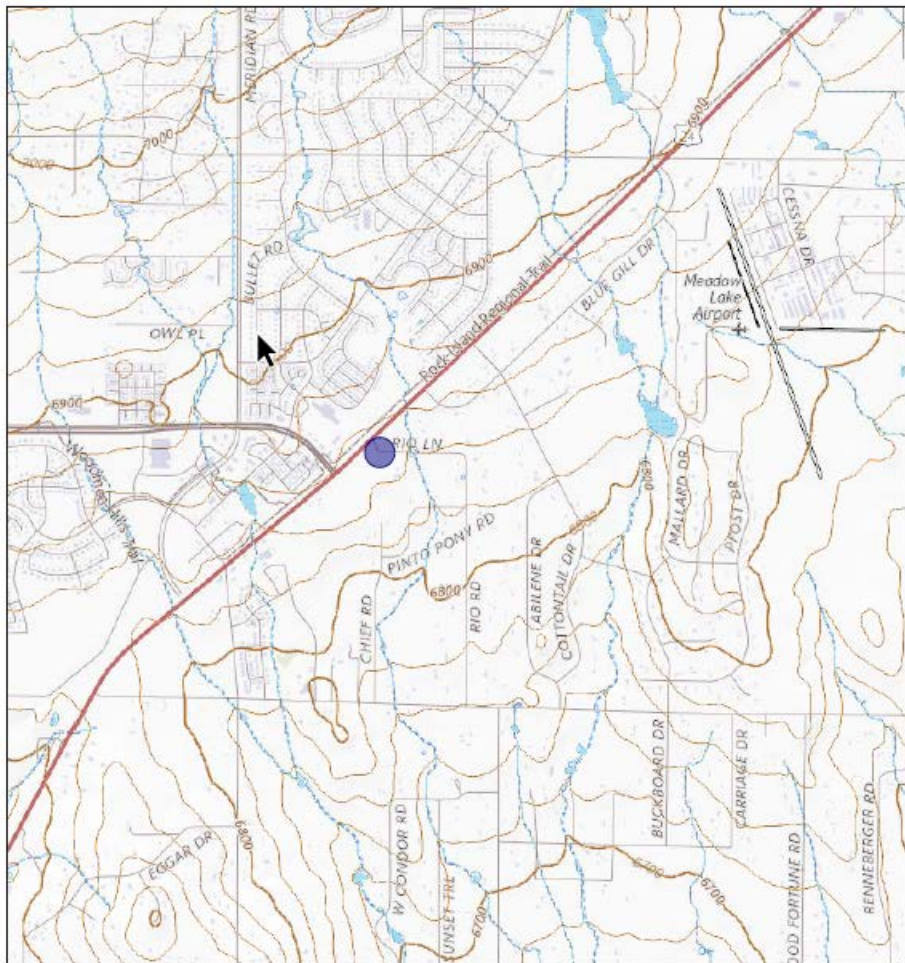
A. SUPPORTING DATA. Data reviewed for JD (check all that apply - checked items shall be included in case file and, where checked and requested, appropriately reference sources below):

- Maps, plans, plots or plat submitted by or on behalf of the applicant/consultant: **2021-180 Falcon Field AJD Request 12-15-2020_29-Nov-21.pdf**
- Data sheets prepared/submitted by or on behalf of the applicant/consultant.
 - Office concurs with data sheets/delineation report.
 - Office does not concur with data sheets/delineation report.
- Data sheets prepared by the Corps:
- Corps navigable waters’ study:
- U.S. Geological Survey Hydrologic Atlas:
 - USGS NHD data.
 - USGS 8 and 12 digit HUC maps.
- U.S. Geological Survey map(s). Cite scale & quad name: **1:24K; Falcon**
- USDA Natural Resources Conservation Service Soil Survey. Citation: **2021-180 Soil Map**
- National wetlands inventory map(s). Cite name: **2021-180 NWI Map**
- State/Local wetland inventory map(s):
- FEMA/FIRM maps:
- 100-year Floodplain Elevation is: (National Geodectic Vertical Datum of 1929)
- Photographs: Aerial (Name & Date): **2021-180 Aerial - May 2020**
or Other (Name & Date):
- Previous determination(s). File no. and date of response letter:
- Applicable/supporting case law:
- Applicable/supporting scientific literature:
- Other information (please specify): **2021-180 EPA Watershed Report, 2021-180 Flow Path and Pictures, 2021-180 Inspection Report - June 2022, 2021-180 NE Stream StreamStats, 2021-180 SW Stream StreamStats, 2021-180 USGS Topo**


B. ADDITIONAL COMMENTS TO SUPPORT JD:

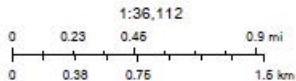
A review of the downstream connectivity of the associated wetlands found there to be a lack of a connection to downstream waters. The drainage features and associated wetlands presented a southward flow path until reaching E. Blaney Road. The flow path consisting of a varying degree of broken stream channel and connected wetlands terminated across a portion of flat terrain with not apparent wetland vegetation. Flow does not appear to reach any downstream waters via the nearby roadside ditch. Flow also does not cross E. Blaney Road due to the lack of culverts and a slight elevation rise. Due to a lack of downstream connectivity, the drainage features and associated wetlands under review are found to be isolated.

2021-180 USGS Topo

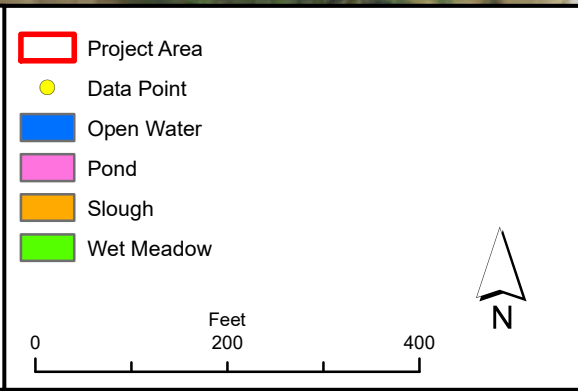
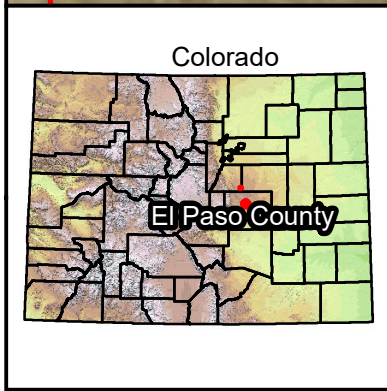
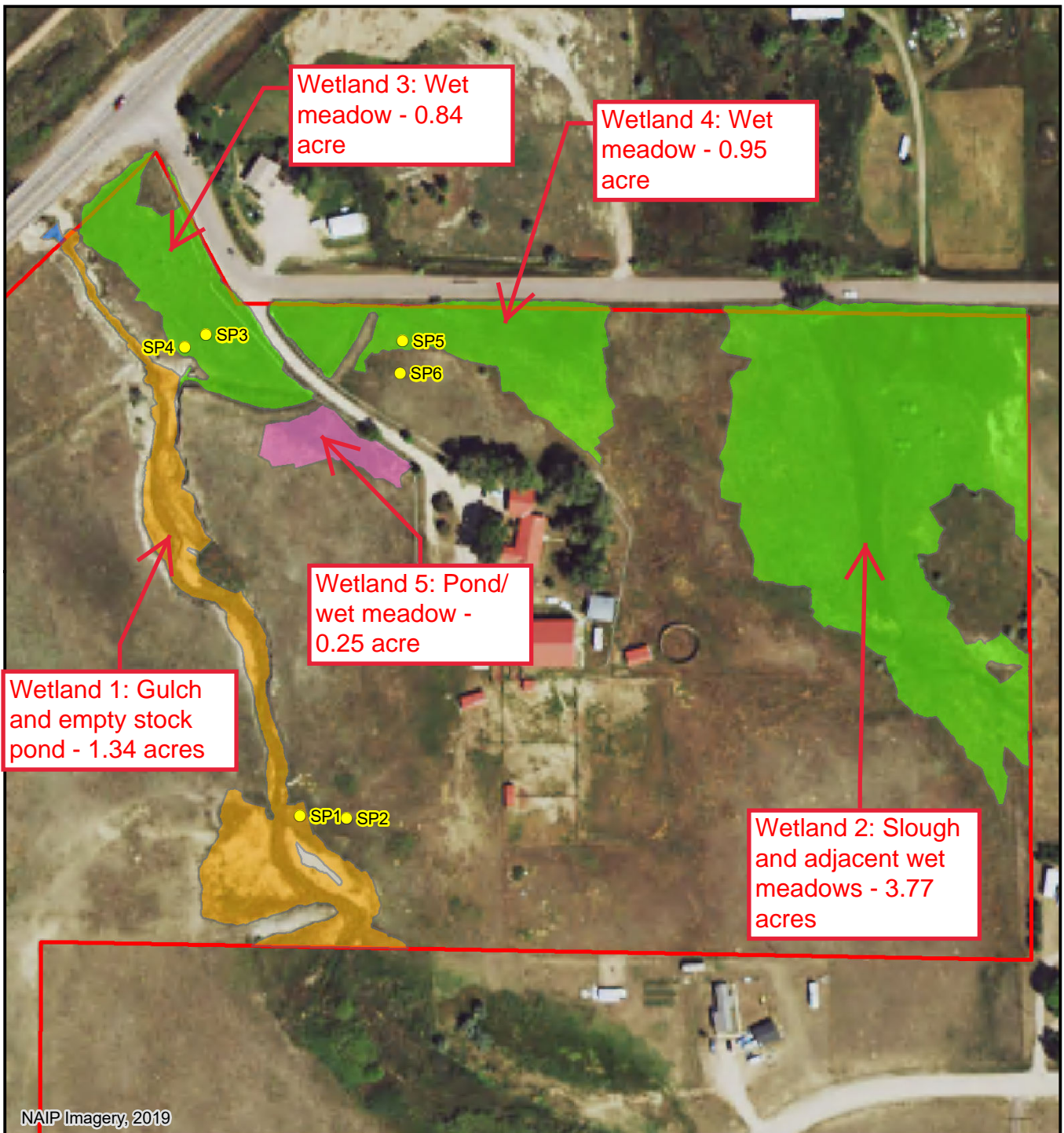


7/25/2022, 4:06:58 PM

-  Override 1
-  Inland Waters
-  Forest/shrub Wetlands
-  Normal Intermediate Contours
-  Emergent Wetlands
-  Normal Index Contours



USGS The National Map: National Boundaries Dataset, SDEP Elevation Program, Geographic Names Information System, National Hydrography Dataset, National Land Cover Database, National Structures Dataset, and National Transportation Dataset, USGS Global Ecosystems; U.S. Census



Falcon Field

Wetlands

bristlecone ecology

NAIP Imagery, 2019

Drainage Maps

Review C2: identify the flow entering the site from the upstream through the existing culvert.
 Review C3: Unresolved. Please discuss off site flows that affect the subject project for both drainage conditions.

Please add off site flow directions, and elevation of contours.

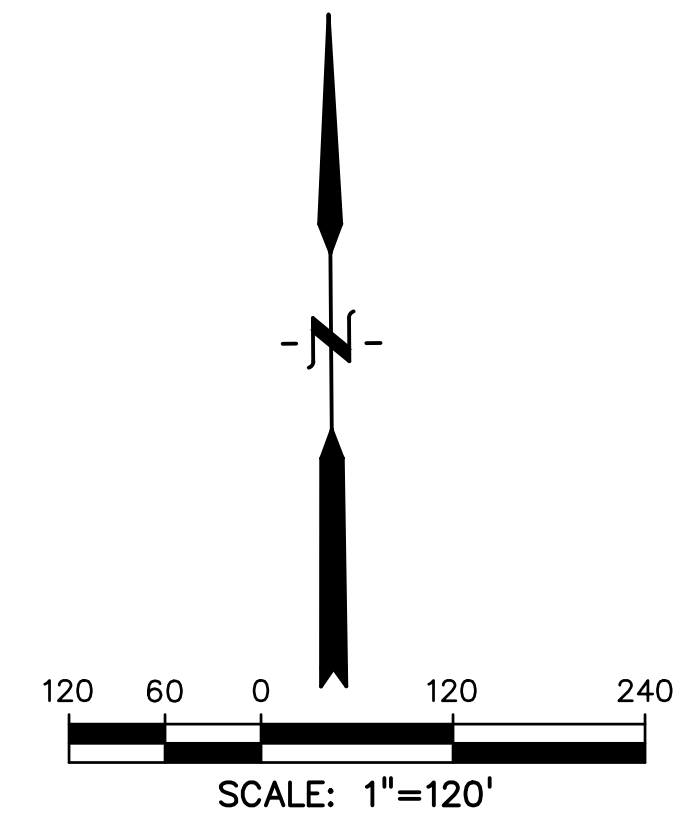
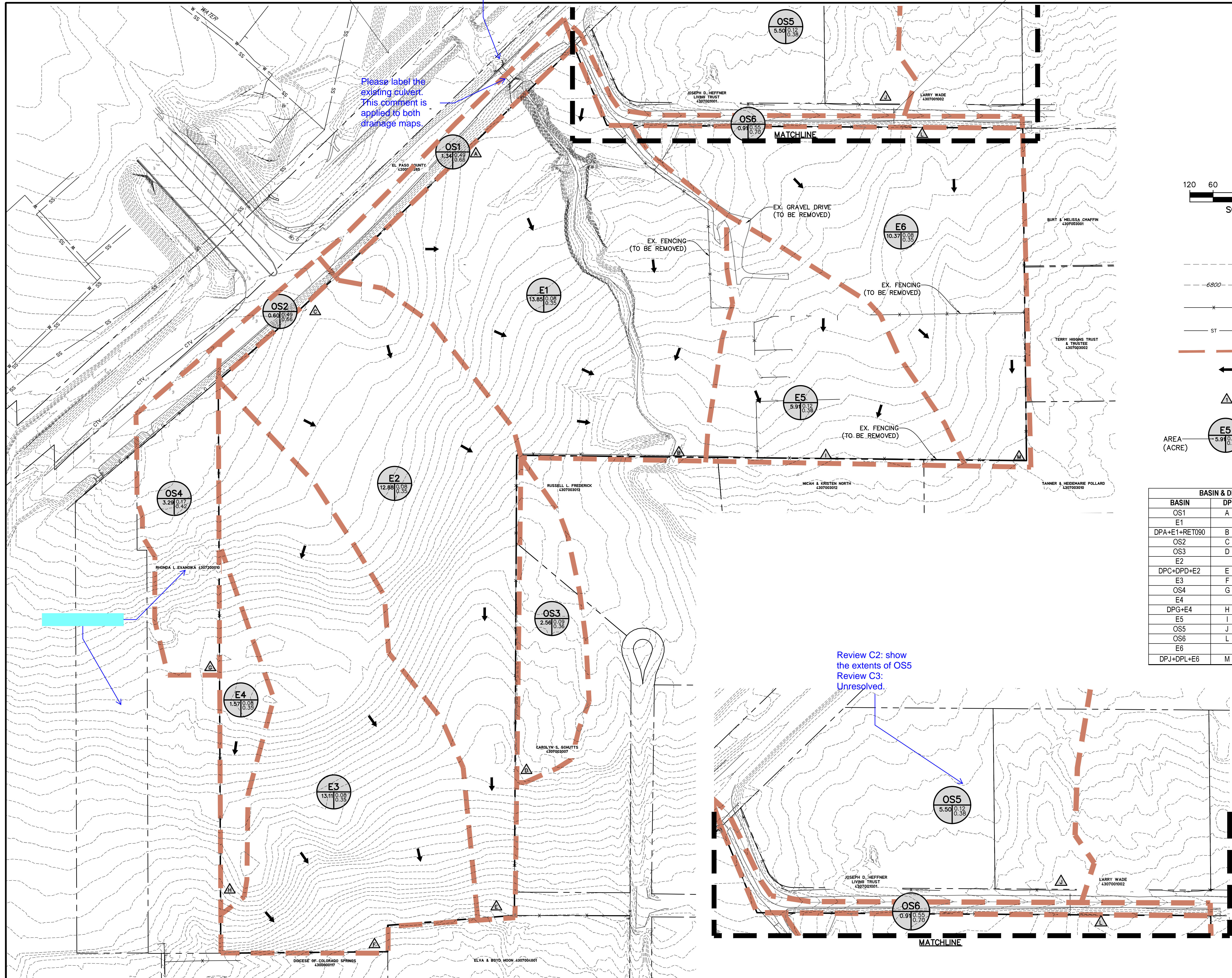
Please show and label the existing roadside ditch and 18 inches CMP culvert discussed in basin OS 5, in the text.

Unresolved from Submittal 2: this should be included as an offsite basin

Unresolved from Submittal 2: call out culvert

Please label the existing culvert. This comment is applied to both drainage maps

Review C2: show the extents of OS5
 Review C3: Unresolved.



- LEGEND**
- - - - - EX. MINOR CONTOUR
 - - - - - EX. MAJOR CONTOUR
 - - - - - EX. FENCE LINE
 - - - - - EX. STORM DRAIN
 - - - - - BASIN BOUNDARY
 - ← FLOW DIRECTION
 - ▲ DESIGN POINT
 - BASIN
 - AREA (ACRE)
 - C5
 - C100

| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|----|-----------|----------|------------|
| BASIN | DP | AREA (AC) | Q5 (cfs) | Q100 (cfs) |
| OS1 | A | 1.34 | 3.4 | 7.6 |
| E1 | | 13.85 | 3.2 | 22.4 |
| DP+A+E1+RET090 | B | 15.19 | 41.0 | 346.4 |
| OS2 | C | 0.60 | 1.4 | 3.2 |
| OS3 | D | 2.56 | 0.8 | 5.6 |
| E2 | | 12.88 | 2.5 | 18.6 |
| DP+C+DPD+E2 | E | 16.04 | 3.6 | 23.9 |
| E3 | F | 13.11 | 2.7 | 19.6 |
| OS4 | G | 3.29 | 2.4 | 9.7 |
| E4 | | 1.57 | 0.3 | 2.6 |
| DPG+E4 | H | 4.86 | 1.9 | 9.0 |
| E5 | I | 5.91 | 2.2 | 11.7 |
| OS5 | J | 5.50 | 6.6 | 6.6 |
| OS6 | L | 0.91 | 2.6 | 5.5 |
| E6 | | 10.37 | 1.7 | 12.5 |
| DPJ+DPL+E6 | M | 16.78 | 4.1 | 21.9 |

PREPARED BY:

DREXEL, BARRELL & CO.
 Engineers & Surveyors
 101 SAWATCH STREET, STE 4100
 COLORADO SPGS, COLORADO 80903
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CLIENT:
FALCON FIELD, LLC.
 3230 ELECTRA DR. N.
 COLORADO SPRINGS, CO 80906
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DRAINAGE PLANS FOR
THE COMMONS AT FALCON FIELD
 12445 RIO LANE, AND VACANT LAND
 PEYTON, EL PASO COUNTY, COLORADO

| ISSUE | DATE |
|---------------|---------|
| INITIAL ISSUE | 1/31/24 |
| RESUBMITTAL | 3/17/24 |

DESIGNED BY: TDM
 DRAWN BY: CGH
 CHECKED BY: KGV
 FILE NAME: 21604-00EDR

PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF DREXEL, BARRELL & CO.

DRAWING SCALE:
 HORIZONTAL: 1" = 120"
 VERTICAL: N/A

OVERALL EXISTING DRAINAGE MAP
 PROJECT NO. 21604-00CSCV
 DRAWING NO.

EDR1
 SHEET: 1 OF 7

All disturbance areas must be included within the drainage basin delineation. This comment is applied to all applicable drainage maps.

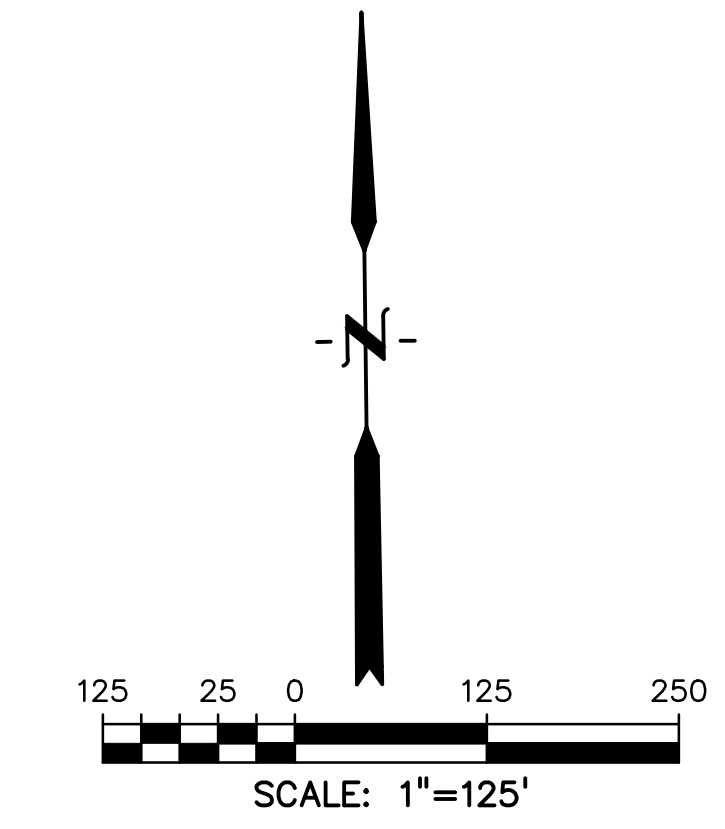
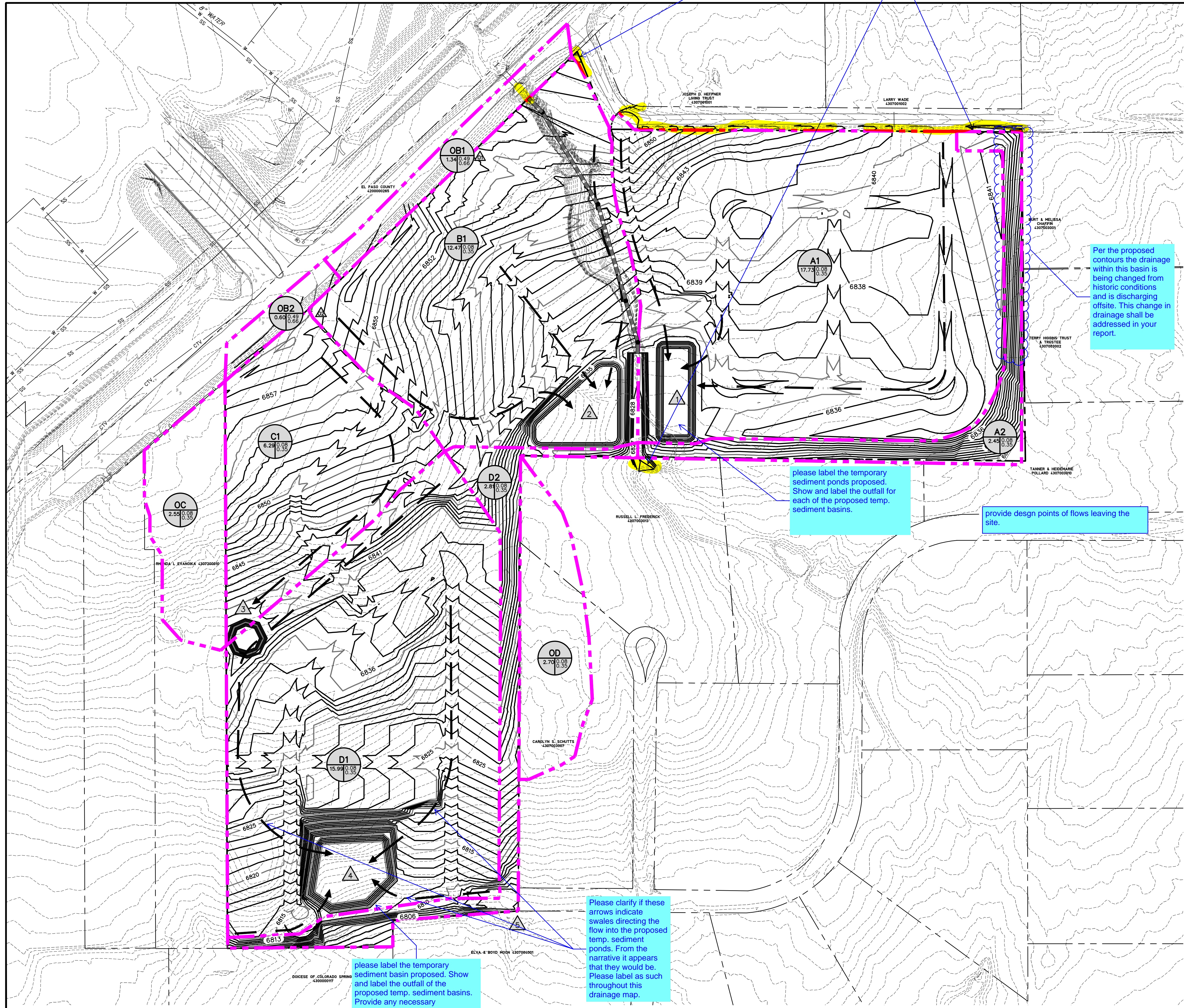
Per the proposed contours the drainage within this basin is being changed from historic conditions and is discharging offsite. This change in drainage shall be addressed in your report.

please label the temporary sediment ponds proposed. Show and label the outfall for each of the proposed temp. sediment basins.

provide design points of flows leaving the site.

please label the temporary sediment basin proposed. Show and label the outfall of the proposed temp. sediment basins. Provide any necessary downstream mitigation due to the concentrated flow at the outfall

Please clarify if these arrows indicate swales directing the flow into the proposed temp. sediment ponds. From the narrative it appears that they would be. Please label as such throughout this drainage map.



LEGEND

- EX. MINOR CONTOUR
- - - - - EX. MAJOR CONTOUR
- PR. MINOR CONTOUR
- PR. MAJOR CONTOUR
- ST — STORM DRAIN
- EX. STORM DRAIN
- - - - - BASIN BOUNDARY
- ← FLOW DIRECTION
- ▲ DESIGN POINT
- BASIN
- C5
- C100

AREA (ACRE)

E5: 5.91 (0.12, 0.36)

| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|----|-----------|-----|------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| PRELIMINARY-BASINS | | | | |
| A1 | 1 | 17.73 | 4.1 | 30.1 |
| A2 | | 2.45 | 0.6 | 4.6 |
| B1 | | 12.47 | 3.0 | 22.4 |
| OB | O1 | 1.34 | 3.4 | 7.6 |
| OB2 | O2 | 0.60 | 1.4 | 3.2 |
| B1+O1+O2 | 2 | 14.41 | 5.9 | 28.9 |
| C1 | | 6.29 | 1.5 | 11.1 |
| OC | | 2.55 | 0.5 | 4.4 |
| C1+OC | 3 | 8.85 | 2.0 | 15.5 |
| D1 | 4 | 15.99 | 3.7 | 27.4 |
| D2 | | 2.81 | 0.7 | 5.2 |
| OD | | 2.70 | 0.7 | 4.8 |
| D2+OD | 5 | 5.51 | 1.4 | 10.2 |

PREPARED BY:

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DRAINAGE PLANS FOR
THE COMMONS AT FALCON FIELD
 12445 RIO LANE, AND VACANT LAND
 PEYTON, EL PASO COUNTY, COLORADO

| ISSUE | DATE |
|---------------|---------|
| INITIAL ISSUE | 1/31/24 |
| RESUBMITTAL | 3/17/24 |

DESIGNED BY: TDM
 DRAWN BY: CGH
 CHECKED BY: KGV
 FILE NAME: 21604-00DR-0G
 PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF DREXEL, BARRELL & CO.

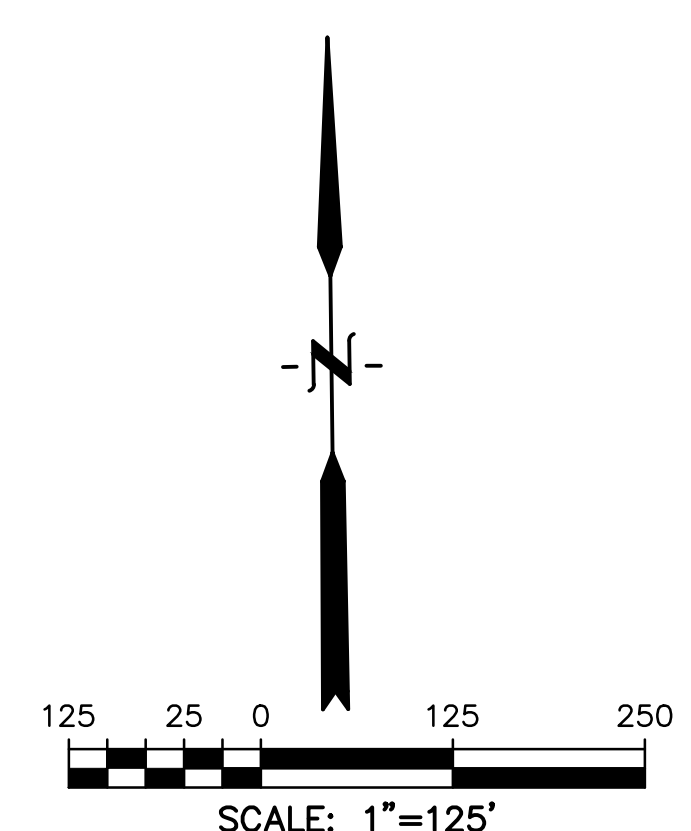
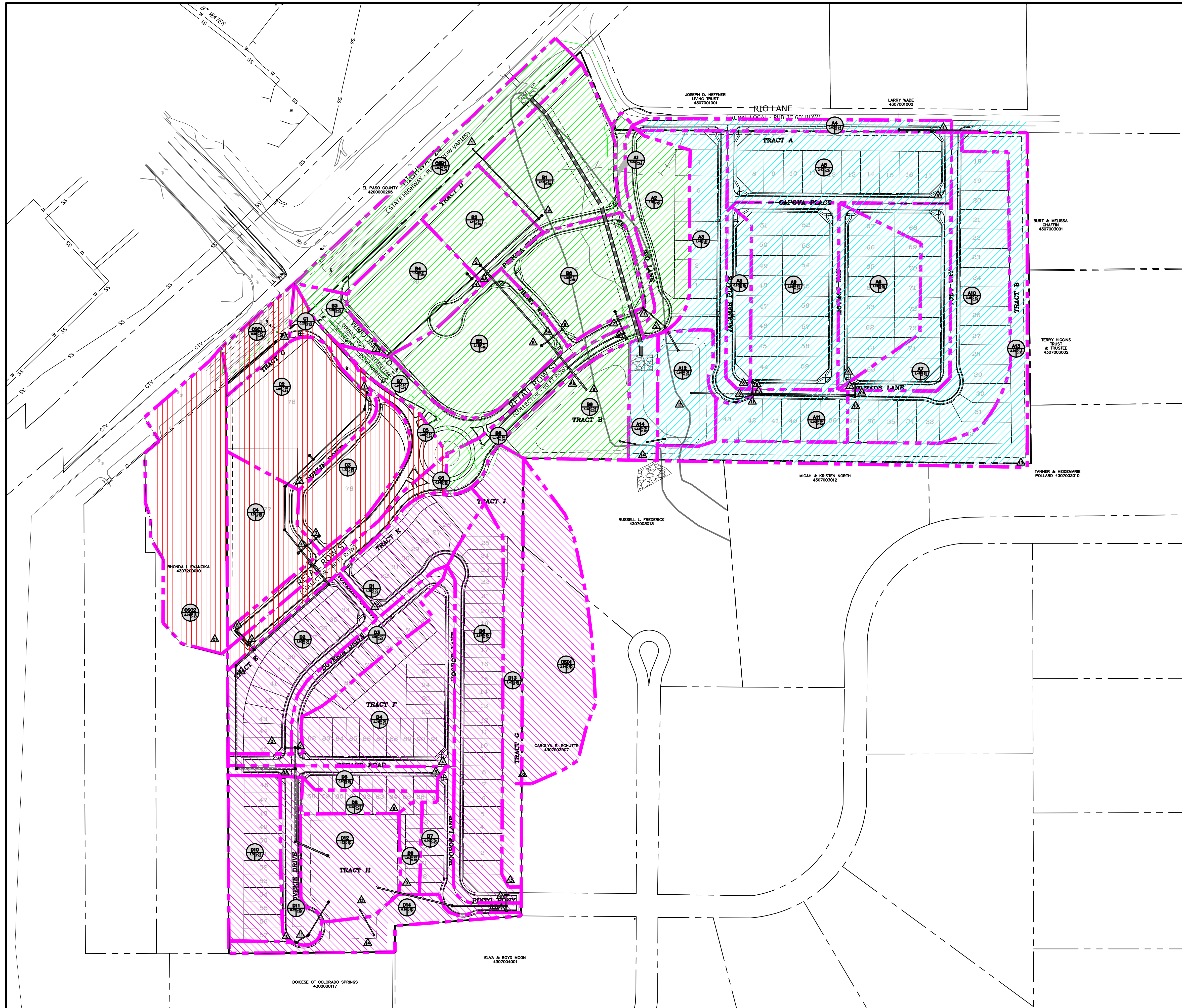
DRAWING SCALE:
 HORIZONTAL: 1" = 125'
 VERTICAL: N/A

OVERLOT PROPOSED DRAINAGE MAP

PROJECT NO. 21604-00CSCV
 DRAWING NO.

OVDR

SHEET: 2 OF 7



- LEGEND**
- EX. MINOR CONTOUR
 - - - - 6800 - - - - EX. MAJOR CONTOUR
 - PR. MINOR CONTOUR
 - - - - 6800 - - - - PR. MAJOR CONTOUR
 - STORM DRAIN
 - ST --- EX. STORM DRAIN
 - BASIN BOUNDARY
 - ← FLOW DIRECTION
 - ▲ DESIGN POINT
 - BASIN
 - E5
 - C5
 - C100
 - AREA (ACRE)
 - 5.91
 - 0.12
 - 0.36
 - A-BASINS
 - B-BASINS
 - C-BASINS
 - D-BASINS

Unresolved from Submittal 2:
shade SPA areas separately
We need to know what areas are
planned for each EDB and what
areas are planned for
exclusions/RR.

PREPARED BY:



CLIENT:

FALCON FIELD, LLC.
3230 ELECTRA DR. N.
COLORADO SPRINGS, CO 80906
(719) 475-7474
CONTACT: PJ ANDERSON

DRAINAGE PLANS FOR
THE COMMONS AT FALCON FIELD
12445 RIO LANE, AND VACANT LAND
PEYTON, EL PASO COUNTY, COLORADO

| ISSUE | DATE |
|---------------|---------|
| INITIAL ISSUE | 1/31/24 |
| RESUBMITTAL | 3/17/24 |

| | |
|--------------|------------|
| DESIGNED BY: | TDM |
| DRAWN BY: | CGH |
| CHECKED BY: | KGV |
| FILE NAME: | 21604-00DR |

PREPARED UNDER MY DIRECT
SUPERVISION FOR AND ON
BEHALF OF
DREXEL, BARRELL & CO.

DRAWING SCALE:
HORIZONTAL: 1" = 125"
VERTICAL: N/A

**OVERALL
PROPOSED
DRAINAGE MAP**

PROJECT NO. 21604-00CSCV
DRAWING NO.

DR1

SHEET: 3 OF 7

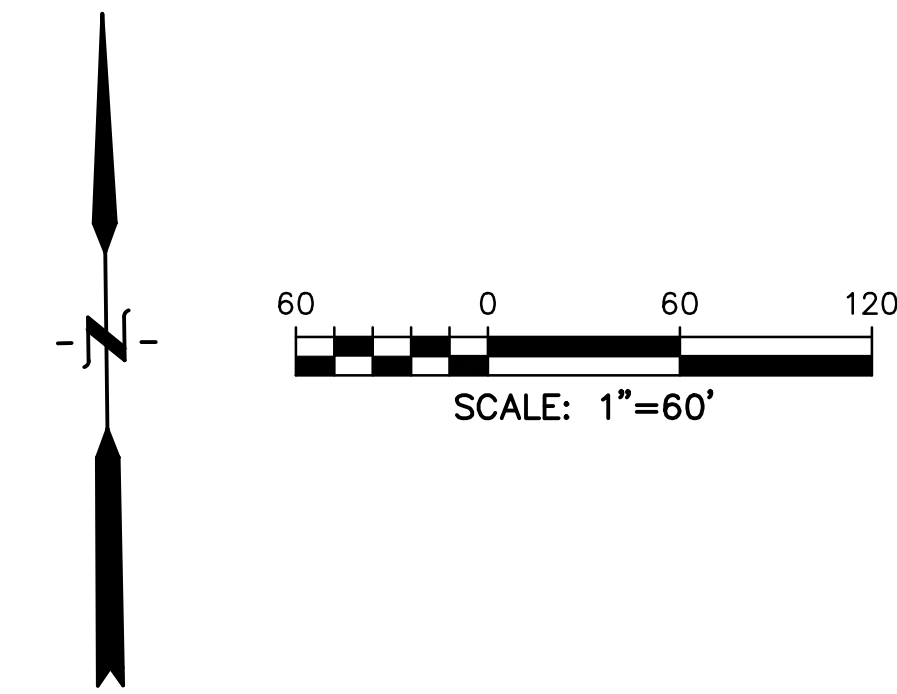
Review C2: Show and address downstream easements needed for the diverted flows (not to be conveyed in roadside ditches or public storm drain).

Review C3: Unresolved. Please address the comment above.

As indicated in comment provided in the narrative, Final hydraulic design of the Box culvert and channel required at this stage as the early grading would install these facilities.

Analysis to a suitable outfall is required. Downstream analysis of the existing channel to Falcon Hwy is needed.

Staff recommends setting up a meeting with staff to further discuss options.



PREPARED BY:

DREXEL, BARRELL & CO.
 Engineers-Surveyors
 101 SAWATCH STREET, STE #100
 COLORADO SPGS, COLORADO 80903
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DRAINAGE PLANS FOR
THE COMMONS AT FALCON FIELD
 12445 RIO LANE, AND VACANT LAND
 PEYTON, EL PASO COUNTY, COLORADO

| ISSUE | DATE |
|---------------|---------|
| INITIAL ISSUE | 1/31/24 |
| RESUBMITTAL | 3/17/24 |

DESIGNED BY: TDM
 DRAWN BY: CGH
 CHECKED BY: KGV
 FILE NAME: 21604-00DR

PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF DREXEL, BARRELL & CO.

DRAWING SCALE:
 HORIZONTAL: 1" = 60"
 VERTICAL: N/A

PROPOSED DRAINAGE MAP

PROJECT NO. 21604-00CSCV
 DRAWING NO.

DR2

SHEET: 4 OF 7

- LEGEND**
- EX. MINOR CONTOUR
 - - - - - EX. MAJOR CONTOUR
 - PR. MINOR CONTOUR
 - - - - - PR. MAJOR CONTOUR
 - STORM DRAIN
 - EX. STORM DRAIN
 - BASIN BOUNDARY
 - ← FLOW DIRECTION
 - ▲ DESIGN POINT
 - BASIN
 - C5
 - C100

AREA (ACRE)

| Basin | Area (Ac) |
|-------|-----------|
| E5 | 5.91 |

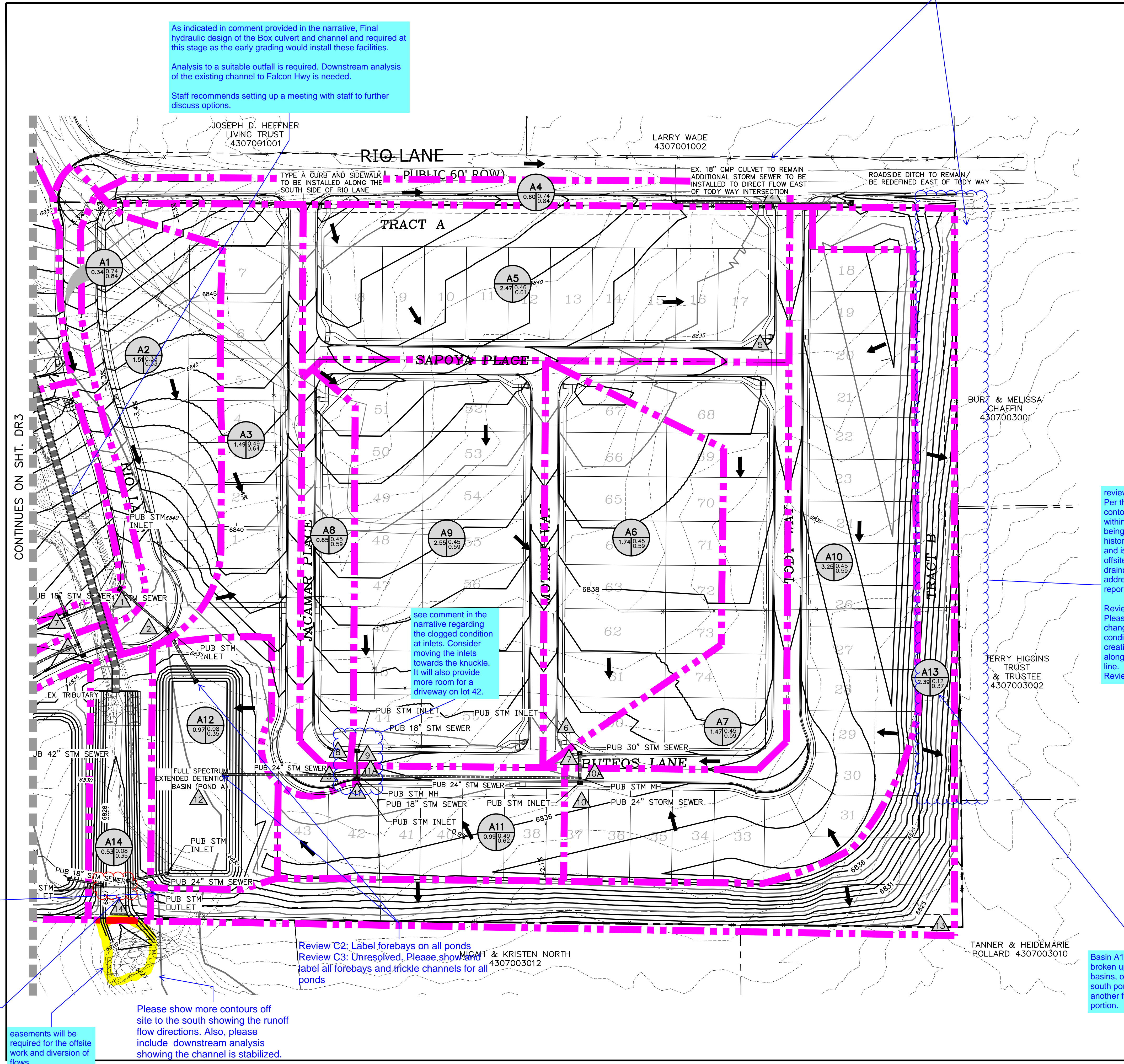
Review 1 comment: Per the proposed contours the drainage within this basin is being changed from historic conditions and is discharging offsite. This change in drainage shall be addressed in your report.

Review 2 comment: Please address the change from historic conditions and the creation of a swale along the property line.

Review 3: unresolved

BASIN & DESIGN POINT SUMMARY

| BASIN | DP | AREA (AC) | Q5 | Q100 |
|-----------------|-----|-----------|------|------|
| A-BASINS | | | | |
| A1 | 1 | 0.34 | 1.3 | 2.5 |
| A2 | 2 | 1.51 | 2.5 | 6.8 |
| DP1+A2 | 2 | 1.85 | 3.8 | 9.3 |
| A3 | 3 | 1.49 | 3.2 | 6.9 |
| A4 | 4 | 0.60 | 2.2 | 4.2 |
| A5 | 5 | 2.47 | 4.0 | 9.0 |
| A6 | 6 | 1.74 | 2.9 | 6.4 |
| A7 | 7 | 1.47 | 2.4 | 5.3 |
| DP5+DP6+ | 7 | 5.67 | 8.4 | 18.7 |
| A8 | 8 | 0.65 | 1.4 | 3.0 |
| A9 | 9 | 2.55 | 4.9 | 10.8 |
| DP8+A9 | 9 | 3.21 | 6.4 | 14.1 |
| A10 | 10 | 3.25 | 5.6 | 12.2 |
| DP9+DP12 | 10A | 8.92 | 17.8 | 39.4 |
| A11 | 11 | 0.99 | 2.0 | 4.3 |
| DP3+A11 | 11 | 2.48 | 5.3 | 11.5 |
| DP10A+DP | 11A | 14.61 | 28.8 | 63.4 |
| A12 | 12 | 0.97 | 0.2 | 1.8 |
| DP2+DP11 | 12 | 17.42 | 31.8 | 70.1 |
| A13 | 13 | 2.39 | 1.4 | 7.7 |
| A14 | 14 | 0.53 | 0.2 | 1.4 |



Please show and label emergency spillway. This comment is applied to all pond A, B, C.

Please show and label erosion protection at the outfall of two outlet pipes from Pond A, and B. Erosion protection must be installed to the toe of slope of the channel.

easements will be required for the offsite work and diversion of flows.

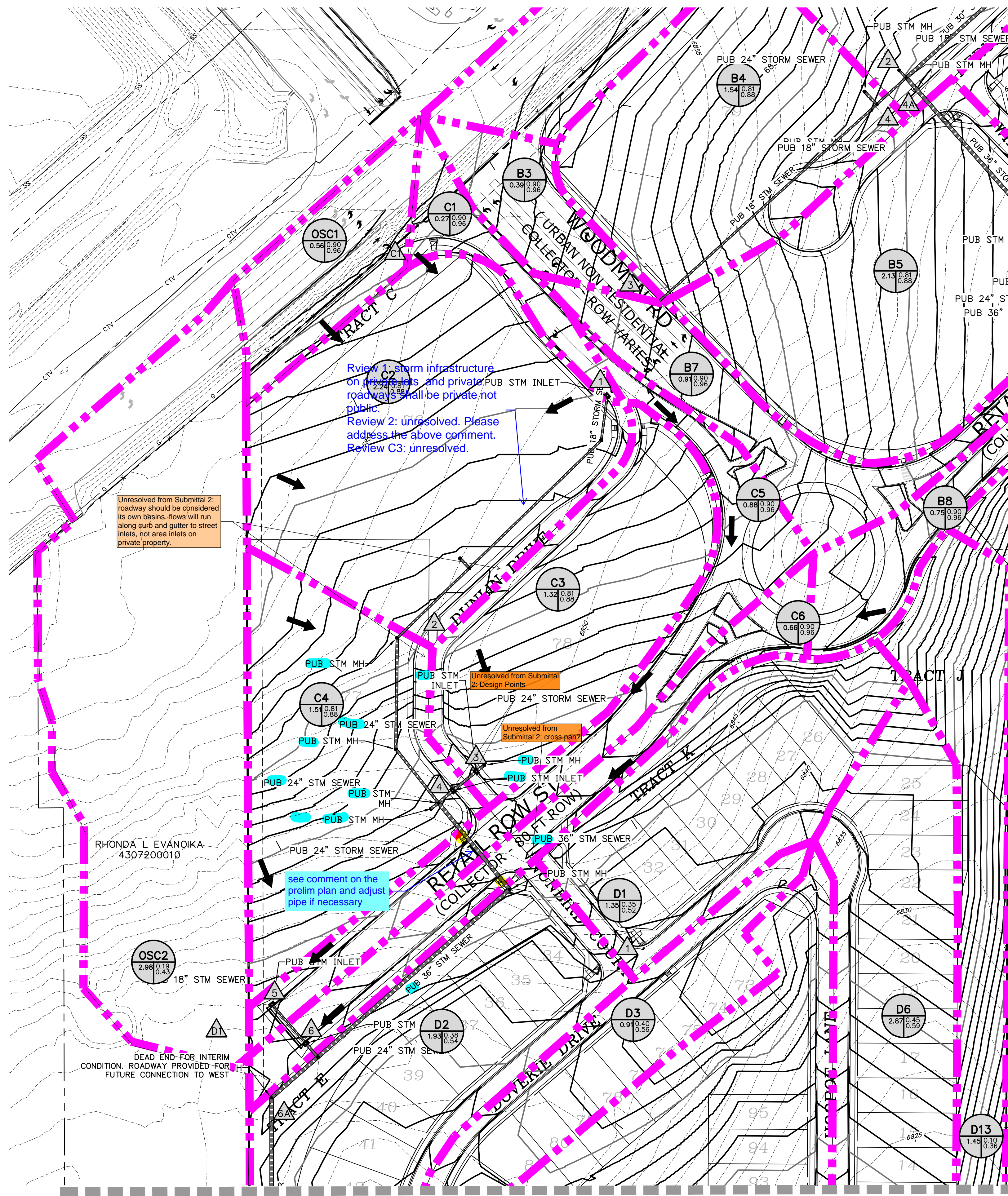
Please show more contours off site to the south showing the runoff flow directions. Also, please include downstream analysis showing the channel is stabilized.

Review C2: Label forebays on all ponds
 Review C3: Unresolved. Please show and label all forebays and trickle channels for all ponds

see comment in the narrative regarding the clogged condition at inlets. Consider moving the inlets towards the knuckle. It will also provide more room for a driveway on lot 42.

Basin A13 should be broken up into two basins, one for the south portion and another for the east portion.

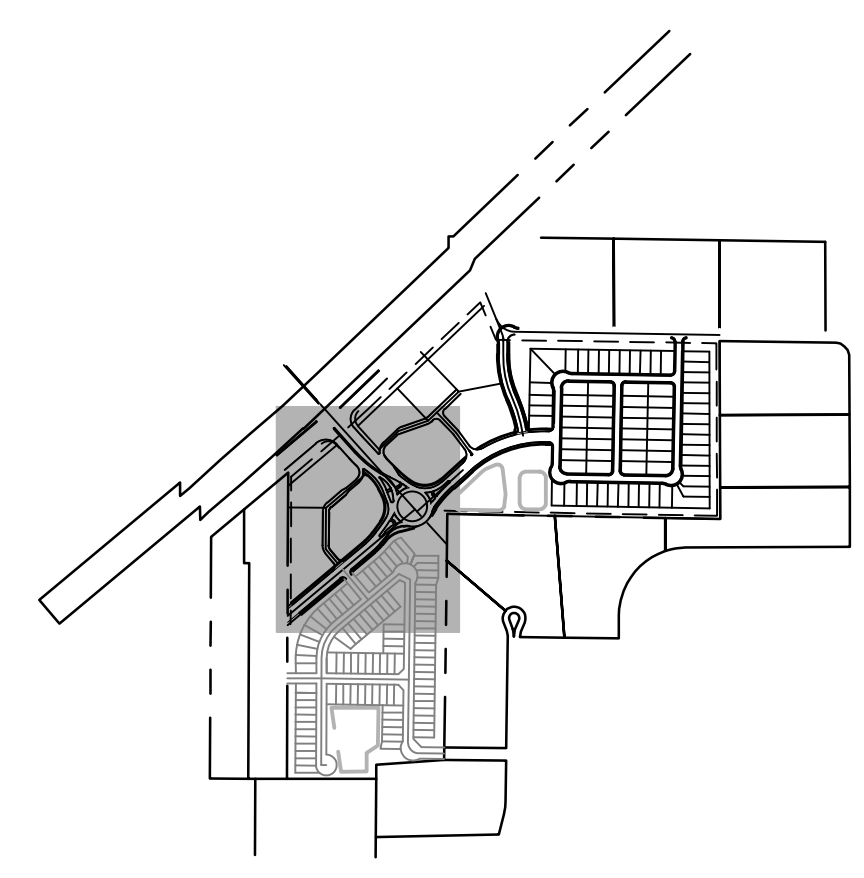
CONTINUES ON SHT. DR3



60 0 60 120
SCALE: 1"=60'

LEGEND

- EX. MINOR CONTOUR
- EX. MAJOR CONTOUR
- PR. MINOR CONTOUR
- PR. MAJOR CONTOUR
- STORM DRAIN
- EX. STORM DRAIN
- BASIN BOUNDARY
- FLOW DIRECTION
- DESIGN POINT
- BASIN
- AREA (ACRE)



| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|----|-----------|------|------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| C-BASINS | | | | |
| OSC1 | C1 | 0.56 | 2.6 | 4.6 |
| C1 | | 0.27 | 1.3 | 2.3 |
| | 1 | 0.83 | 3.8 | 6.7 |
| C2 | | 2.24 | 9.4 | 17.1 |
| | 2 | 2.52 | 13.7 | 24.8 |
| C3 | | 1.32 | 5.5 | 10.1 |
| C4 | | 1.51 | 6.3 | 11.5 |
| | 4 | 5.34 | 24.0 | 43.7 |
| OSC2 | | 2.98 | 2.6 | 10.1 |
| C5 | | 0.88 | 4.1 | 7.3 |
| | 5 | 3.86 | 6.3 | 16.5 |
| C6 | | 0.66 | 3.1 | 5.5 |
| | 6 | 4.52 | 10.0 | 23.8 |
| 6A | | 9.86 | 31.8 | 62.7 |

Review 1: storm infrastructure on private lots and private roadways shall be private not public.
Review 2: unresolved. Please address the above comment.
Review 3: unresolved.

Unresolved from Submittal 2: roadway should be considered its own basins. flows will run along curb and gutter to street inlets, not area inlets on private property.

Unresolved from Submittal 2: Design Points

Unresolved from Submittal 2: cross ramp

see comment on the prelim plan and adjust pipe if necessary

RHONDA L EVANOIKA
4307200010

DEAD END FOR INTERIM CONDITION. ROADWAY PROVIDED FOR FUTURE CONNECTION TO WEST

CONTINUES ON SHT. DR5

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DRAINAGE PLANS FOR
THE COMMONS AT FALCON FIELD
12445 RIO LANE, AND VACANT LAND
PEYTON, EL PASO COUNTY, COLORADO

| ISSUE | DATE |
|---------------|---------|
| INITIAL ISSUE | 1/31/24 |
| RESUBMITTAL | 3/17/24 |

DESIGNED BY: TDM
DRAWN BY: CGH
CHECKED BY: KGV
FILE NAME: 21604-00DR

PREPARED UNDER MY DIRECT SUPERVISION FOR AND ON BEHALF OF DREXEL, BARRELL & CO.

DRAWING SCALE:
HORIZONTAL: 1" = 60"
VERTICAL: N/A

PROPOSED DRAINAGE MAP

PROJECT NO. 21604-00SCV
DRAWING NO.

DR4

SHEET: 6 OF 7

PREPARED BY:

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DRAINAGE PLANS FOR
THE COMMONS AT FALCON FIELD
12445 RIO LANE, AND VACANT LAND
PEYTON, EL PASO COUNTY, COLORADO

| ISSUE | DATE |
|---------------|---------|
| INITIAL ISSUE | 1/31/24 |
| RESUBMITTAL | 3/17/24 |

DESIGNED BY: TDM
 DRAWN BY: CGH
 CHECKED BY: KGV
 FILE NAME: 21604-00DR

PREPARED UNDER MY DIRECT
 SUPERVISION FOR AND ON
 BEHALF OF
 DREXEL, BARRELL & CO.

DRAWING SCALE:
 HORIZONTAL: 1" = 60"
 VERTICAL: N/A

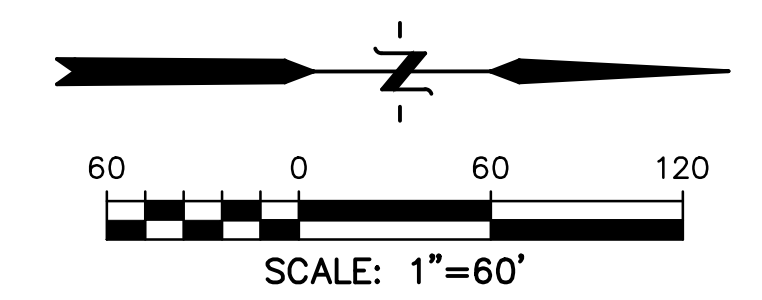
PROPOSED
 DRAINAGE MAP

PROJECT NO. 21604-00CSV
 DRAWING NO.

DR5

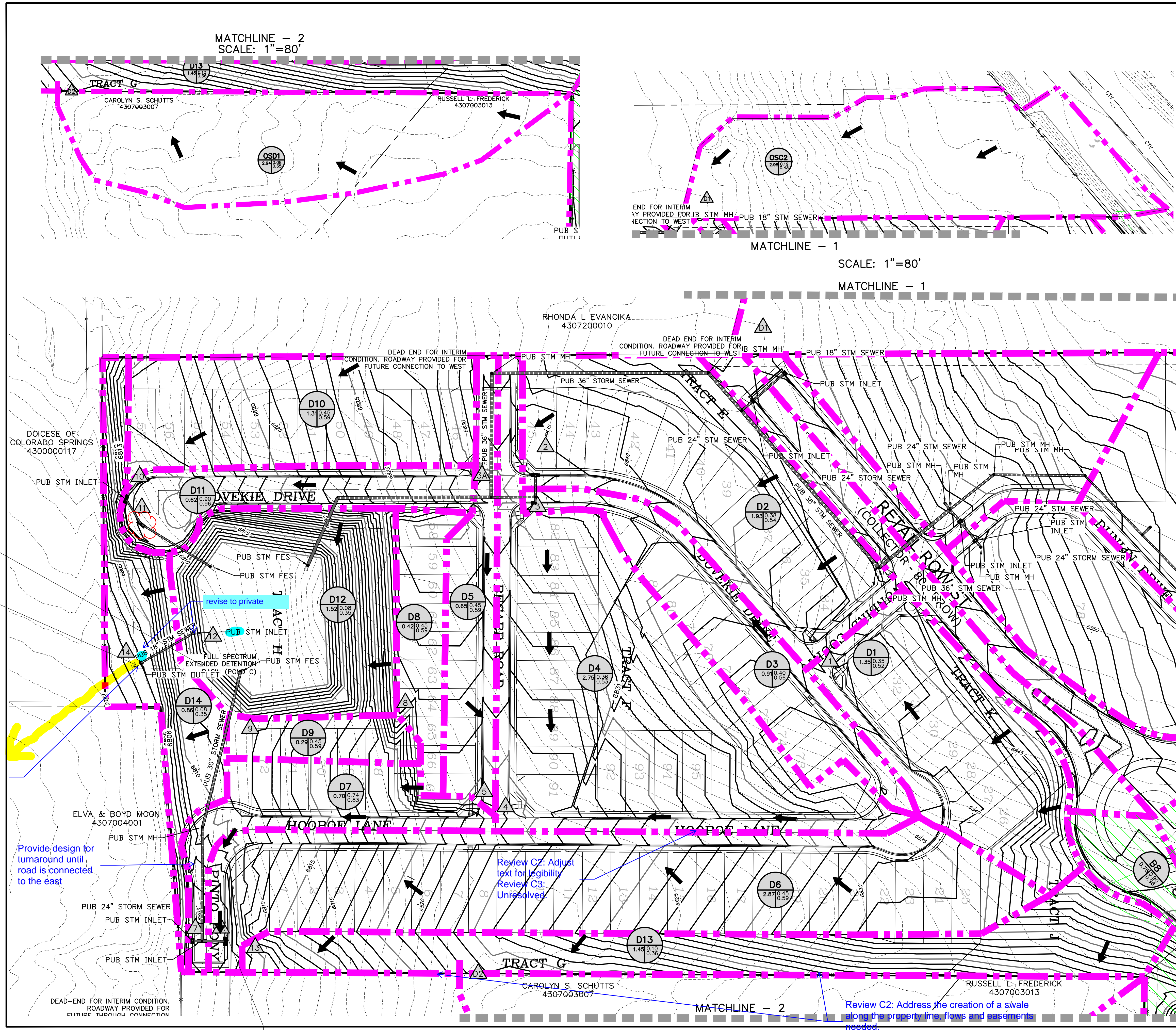
SHEET: 7 OF 7

| BASIN & DESIGN POINT SUMMARY | | | | |
|------------------------------|----|-----------|------|------|
| BASIN | DP | AREA (AC) | Q5 | Q100 |
| D-BASINS | | | | |
| D1 | 1 | 1.35 | 2.0 | 5.0 |
| D2 | 2 | 1.93 | 3.1 | 7.6 |
| | 2 | 3.28 | 4.8 | 11.9 |
| D3 | 3 | 0.91 | 1.4 | 3.3 |
| | 3A | 14.05 | 33.1 | 68.0 |
| D4 | 4 | 2.75 | 4.2 | 10.3 |
| D5 | 5 | 0.65 | 1.3 | 2.8 |
| | 5 | 3.40 | 5.4 | 13.0 |
| D6 | 6 | 2.87 | 5.5 | 12.2 |
| D7 | 7 | 0.70 | 2.5 | 4.7 |
| | 7 | 6.98 | 12.7 | 28.4 |
| D8 | 8 | 0.42 | 0.8 | 1.7 |
| D9 | 9 | 0.29 | 0.6 | 1.3 |
| D10 | 10 | 1.31 | 2.7 | 6.0 |
| D11 | 11 | 0.62 | 2.9 | 5.1 |
| | 11 | 1.93 | 5.3 | 10.6 |
| D12 | 12 | 1.52 | 0.6 | 4.3 |
| D13 | 13 | 2.94 | 1.0 | 7.3 |
| OSD1 | D1 | 1.45 | 0.5 | 3.3 |
| | 13 | 4.39 | 1.4 | 9.6 |
| D14 | 14 | 0.86 | 0.3 | 2.3 |



- LEGEND
- EX. MINOR CONTOUR
 - 6800--- EX. MAJOR CONTOUR
 - PR. MINOR CONTOUR
 - 6800--- PR. MAJOR CONTOUR
 - ST --- EX. STORM DRAIN
 - BASIN BOUNDARY
 - ← FLOW DIRECTION
 - ▲ DESIGN POINT
 - BASIN
 - AREA (ACRE)
 - C5
 - C100

CONTINUED FROM SHT. DR4



Unresolved from Submittal 2:
 adjust overlapping text

Unresolved from Submittal 2:
 this does not look to be a
 suitable outfall, please confirm

Review C2: show all
 necessary downstream
 conveyances and
 easements to an adequate
 suitable outfall.

Review C3: Unresolved.

Provide design for
 turnaround until
 road is connected
 to the east

Review C2: Adjust
 text for legibility
 Review C3:
 Unresolved.

Review C2: Address the creation of a swale
 along the property line, flows and easements
 needed.
 Review C3: Unresolved.

Unresolved from Submittal 2:
 adjust overlapping text/arrow