## TRAFFIC ANALYSIS REPORT

## Lewis Palmer Middle School Monument, CO

Prepared for:
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FHU Reference No. I22227-0।
May 2023
PCD File No. CDR-23-005

## Traffic Engineer's Statement

The attached traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.

Lyle E. DeVries PE \#: 37203 Date

## Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.
$\frac{\text { Chris CiclQun }}{\text { Chris Coulter }}$
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## I. INTRODUCTION

The Lewis-Palmer School District \#38 is proposing to reconfigure an existing parking lot at the Lewis Palmer Middle School (LPMS) in unincorporated El Paso County, Colorado to only serve buses during school peak hours and implement a one-way inbound access to this lot for buses. The new access would be located north of the existing parking lot access on Woodmoor Drive. Woodmoor Drive is an important transportation connector located in Monument, Colorado. The collector roadway serves multiple residential developments and provides access to highway (HWY) 105 and Interstate (I-) 25. The roadway network adjacent to the site can be seen on Figure I.

The proposed parking lot/access reconfiguration is intended to improve school traffic circulation and provide exclusive bus access and parking. Figure 2 shows the layout of the proposed bus access. This access will serve as an inbound only bus access during school peak hours and provide parking for buses. During off peak hours, parents and visitors will be allowed to use this lot for parking purposes and may use the inbound access outside of normal school hours.

FHU has completed an assessment of current and future (with reconfiguration) transportation conditions along Woodmoor Drive and surrounding the Lewis Palmer Middle School. This assessment provides considerations for future improvements and determines safety and efficiency needs for the proposed new access while serving the needs of multiple user types. The parameters of this analysis have been coordinated with El Paso County Staff. Based on staff input, this report includes information on existing traffic conditions, redistributed traffic with implementation of the bus only parking lot, total traffic volume projections, sight distance needs, multimodal circulation needs, and any recommended roadway improvements.

The following two future scenarios have been analyzed for this report:

- Short-term Future - Time period for the completion of the bus only access, currently anticipated as the Year 2023.
- Long-term Future - The Long-term Future scenario reflects projected Year 2045 traffic conditions.


NORTH
FIGURE I


## II. EXISTING CONDITIONS

## II.A. Surrounding Land Use

Much of the area adjacent to the project site has been developed. The land uses surrounding the site are primarily residential with some commercial development north of the Middle School.

## II.B. School Traffic Circulation

Currently, there are four parking lots which serve the school, one parking lot north of the school near the track field, two rows of parking along the one-way drive adjacent to the school, and one parking lot south of the school. The school traffic plan is provided to parents to help govern circulation. Each of these lots and their accesses serve multiple users and vehicle types. Figure 3 shows the current and proposed lots and access locations. Current usage/guidance is described as follows:

- Lot A: provides 39 spaces, including accessible parking. Provides student drop-off and pickup from the north along Woodmoor Drive. It is accessed via full movement Access 3.
- Lot B: provides 15 spaces. Accessed via full movement Access I to Woodmoor Drive.
- Lot C: provides 38 parking spaces, including accessible parking. Parking aisle is one-way southbound during peak periods. During AM school peak, aisle serves bus only traffic. During PM school peak, aisle sequentially serves bus traffic then student pickup. Lot provides general parking during off peak hours.
- Lot D: provides approximately 35 unmarked spaces for faculty and staff. Student drop-off is provided from the south via Woodmoor Drive adjacent to the Willow Park Way north curb line.

A field visit was conducted to observe school traffic operations during the 2022/2023 school year. These observations confirmed that the above described usage is typical with the below exceptions:

- Lot A is accommodating drop-off and pick-up activity from both directions along Woodmoor Drive.
- Some student drop-off activity from the south is occurring on both the north and south sides of Willow Park Way, contradicting instructions (shown right with blue lines).



## II.C. Roadway Network

The existing roadway network adjacent to the vicinity of the site includes Woodmoor Drive and Willow Park Way. The roadway network is as follows:

Woodmoor Drive: Woodmoor Drive is a three-lane collector in the project area serving mainly residential developments and provides connectivity to HWY 105 and I-25. The posted speed limit is 30 miles per hour (mph); however, during school peaks the speed limit is reduced to 20 mph 620 feet

north of the parking lot access to 250 feet south of Willow Park Way. Given this study is primarily focused on school hours, this roadway was analyzed with a posted speed of 20 mph .

Willow Park Way: Willow Park Way is a two-lane minor local street which provides access to the Lewis Palmer Middle School as well as a few other commercial developments. There is no posted speed limit, but, for the purpose of this study, the speed limit was assumed to be 20 mph .

## II.D. Traffic Volumes

Weekday AM and PM school peak hour turning movement counts (TMCs) were collected on Wednesday, December 14, 2022, at the following intersections:

- Woodmoor Drive \& Existing Parking Lot Access
- Woodmoor Drive \& Main School Access
- Woodmoor Drive \& Willow Park Way
- Willow Park Way \& Secondary School Access

The peak hour traffic counts were collected in 15-minute intervals between 6:30 and 8:30 AM and 2:00 to 4:00 PM. The AM peak hour was found to be 7:00 to 8:00 AM, and the PM peak was 2:15 to 3:15 PM. Appendix A contains the TMCs. Peak hour traffic volumes are shown on Figure 4. Daily traffic volumes are also depicted on Figure 4 as multiples of the counted peak hour traffic levels.

Based on the counts collected, Peak Hour Factors (PHFs) were found to range from 0.33 to 0.92 . The majority of heavy vehicle percentages were found to be under 8 percent; however, the southbound volume at the secondary school access as well as the eastbound movements at the intersection of Woodmoor Drive with Willow Park Way were found to be a bit higher, likely reflecting buses leaving the school. In order to reflect school peaking conditions, existing PHFs and heavy vehicle percentages were applied for both existing and future conditions by approach at each study intersection.

## II.E. Traffic Operations

Existing operational conditions were analyzed at each study intersection. The analysis is based on procedures documented in the Highway Capacity Manual (6th Edition). This analysis procedure provides a Level of Service (LOS), a qualitative measure of traffic operational conditions, based on intersection capacity and vehicle delay. LOS is described by a letter designation ranging from A to F, LOS A represents almost free-flow conditions, while LOS F represents congested conditions. LOS is calculated for movements which must yield right-of-way for unsignalized intersections.

Figure 4 shows the results of the existing conditions analysis. Appendix B contains LOS worksheets. As shown, all stop-controlled movements operate at LOS D or better with the exception of the following:

## Willow Park Way/Woodmoor Drive

- Eastbound movement operates at LOS F during the AM peak hour
- Westbound movement operates at LOS F during the AM peak hour


## Woodmoor Drive/Parking Lot A Access

- Westbound left/through movement operates at LOS E during the AM peak hour


NORTH
FIGURE 4

## III. FUTURE CONDITIONS

## III.A. Redistributed Traffic

The new bus access would restrict the north parking lot to bus only during school peak hours, and all buses are anticipated to use the reconfigured lot to the north, which would have one inbound only access (proposed) and the second access would be outbound only for buses. Further, new circulation patterns for buses will cause all buses to arrive and depart from the school traveling southbound, meaning no school buses are anticipated to make a northbound left turn into this lot. Table I outlines the current and proposed users and accesses for each lot.

Table I. Proposed Reconfigurations to Parking and Access

| Parking Lot | Current |  |  | Future w/ Reconfiguration |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Uses | Entrance Access | Exit Access | Use | Entrance Access | Exit Access |
| A | Student pickup/drop-off | 3 | 3 | Bus only during school peaks | 3 a | 3 |
|  | Visitors | 3 | 3 | Visitors at other times | 3 | 3 |
| B | Faculty and Staff | 1 | 1 | Faculty and Staff | 1 | 1 |
| C | Bus Only During School Peaks | I | 2 | Student pickup/drop-off | 1 | 2 |
|  | Student Pickup/Drop-off | 1 | 2 | Visitors | 1 | 2 |
|  | Visitors | 1 | 2 |  |  |  |
| D | Faculty and Staff | 4, 2 | 4, 2 | Faculty and Staff | 4, 2 | 4, 2 |

## III.B. Future Traffic Conditions

Because the implementation of the new parking lot access will restrict parking lot A to allow only buses during the peak hours, peak hour volumes will be redistributed so all buses use the parking lot and all passenger cars only use the main and secondary school accesses. Figure 5 shows the redistributed existing volumes and subsequent traffic operations. Of note, it is recommended and assumed that all passenger vehicle pick up and drop off traffic will utilize intersection I as inbound access to the aisle through parking lot C and access 2 as outbound only, creating a drop off/pick up loop. As shown, congested (LOS F) conditions are expected to occur at Willow Park Way intersections for passenger vehicle movements exiting the site during school pick up and drop off activities. Recommended mitigation measures are highlighted in Section III.C.

Future traffic was estimated for the short-term (2023) and long-term (2045) timeframes and accounts for existing traffic already using the transportation system, plus a general upward factoring of current traffic levels to capture the effects of anticipated future growth in the area. Because the Middle School is not anticipated to increase attendance in the future and the surrounding area is mostly developed, only the northbound and southbound through traffic along Woodmoor Drive was increased to account for growth. The Pikes Peak Area Council of Governments (PPACG) regional model was used to determine an annual growth rate of $0.6 \%$ per year along Woodmoor Drive.

Year 2023 traffic projections were developed assuming 0.6 percent growth per year for one year, this results in minimal growth along Woodmoor Drive. It is important to note that the growth factor was applied to only the northbound and southbound through movements on Woodmoor Drive.
LEGEND

$$
\begin{aligned}
\text { XXX(XXX) } & =\text { AM(PM) Peak Hour Traffic Volumes } \\
\text { XXXX } & =\text { Daily Traffic Volumes } \\
\text { x/x } & =\text { AM/PM Peak Hour Unsignalized } \\
& \text { Intersection Level of Service } \\
& =\text { Stop Sign }
\end{aligned}
$$

## Short-term Future Traffic Operations

Figure 6 depicts short-term future AM and PM peak hour turning movement projections for the study area intersections and Appendix C contains the LOS worksheets. Using the existing PHFs and heavy vehicle percentages outlined in Section 0, all unsignalized movements are anticipated to remain at acceptable operations with the exception of the following:

## Woodmoor Drive/Willow Park Way

- Eastbound movement anticipated to operate at LOS F in the AM peak hour
- Westbound movement anticipated to operate at LOS F in the AM peak hour


## Willow Park Way/Secondary School Access

- Southbound movement anticipated to operate at LOS F during the AM peak hour


## Long-term Future Traffic Operations

Figure $\mathbf{7}$ shows the long-term peak hour turning movement projections for the study area intersections and Appendix D contains the LOS worksheets. It is important to note that the growth factor was only applied to northbound and southbound through movements on Woodmoor Drive.

Using the existing PHFs and heavy vehicle percentages outlined in Section 0, all unsignalized movements are projected to remain acceptable with the exception of the following:

## Woodmoor Drive/Willow Park Way

- Eastbound movements are anticipated to operate at LOS F during the AM peak hour
- Westbound movements are anticipated to operate at LOS F during the AM peak hour


## Willow Park Way/Secondary School Access

- Southbound movement anticipated to operate at LOS F during the AM peak hour

As noted in the short-term conditions above, the intersection of Willow Park Way with Secondary School Access will not meet signal warrant thresholds and does not warrant any improvements. The eastbound movement at Woodmoor Drive with Willow Park way is anticipated to experience very highs delays and queueing. Recommended mitigation measures are described in Section III.C.



LEGEND

$$
\begin{aligned}
\text { XXX(XXX) } & =\text { AM(PM) Peak Hour Traffic Volumes } \\
\text { XXXX } & =\text { Daily Traffic Volumes } \\
\text { x/x } & =\text { AM/PM Peak Hour Unsignalized } \\
& \text { Intersection Level of Service } \\
& =\text { Stop Sign }
\end{aligned}
$$

NORTH

## III.C. Recommended New Circulation Pattern

Upon exiting the pick up and drop off aisle in parking lot C , operational analyses of future conditions indicate that vehicles will encounter congested conditions while attempting to exit the school approaching Willow Park Way and Woodmoor Drive. The southbound movement at intersection 2 may reach queue lengths up to of 100 feet in length, and eastbound movements at the Woodmoor Drive/Willow Park Way would operate at poor LOS with lengthy queues blocking intersection 2. This condition would result in congestion along the parking lot C aisle and impede efficient student pick up and drop off.

In view of the anticipated congestion, it is recommended that adjustments be made in passenger vehicle patterns with the proposed change to parking lot usage, access and configuration. Figure 8 depicts a recommended option for acceptably accommodating outbound traffic to maintain efficiency.

Figure 8. Recommended Passenger Vehicle Circulation


As shown all southbound traffic would turn right on Willow Park Way to make use of additional curb space, travel west to the end of the cul-de-sac, U-turn, then approach Woodmoor Drive. At Woodmoor Drive, all traffic would be required to make a right turn to efficiently exit the site. The objective of this pattern is to minimize left turn movements, which experience greater delay and congestion than right turns, and prevent crossing vehicle paths. At the intersection of Woodmoor Drive with Willow Park Way, it is recommended that a channelizing island be painted to facilitate free flow eastbound to southbound right turns into the existing southbound auxiliary lane.

It is recommended that this configuration should be monitored upon implementation with respect to the following items:

- U-turn activity: The limitation to outbound right turns only onto Woodmoor Drive will likely result in some traffic seeking to make a U-turn from southbound Woodmoor Drive to northbound Woodmoor Drive. It is likely that this pattern will cause some school traffic to utilize neighborhood streets east of the school to complete this maneuver. It is recommended that guidance be provided to school traffic to ensure U-turns are completed in an appropriate manner, and monitoring occur to limit neighborhood impact.
- Ease of entry/exit: The ease of entry into and exit from Lot $C$ to pick up or drop off students should be monitored to ensure that traffic backups onto Woodmoor Drive are kept to a minimum.

The recommended traffic pattern may require alterations in the field upon implementation. Such alterations may include need for uniformed traffic control or adjustments to routing.

## III.D. Traffic Control Needs

Current traffic control at the study intersections is shown in Table 2.
Table 2. Current Traffic Control

| Intersection | Traffic Control Type |
| :--- | :--- |
| Woodmoor Drive \& Parking Lot Access | Two-Way Stop Control (TWSC) (EB \& WB) |
| Woodmoor Drive \& Main School Access | TWSC (EB) |
| Woodmoor Drive \& Willow Park Way | TWSC (EB \& WB) |
| Willow Park Way \& Secondary School Access | TWSC (NB \& SB) |

As shown, all of the study intersections are currently unsignalized. The Manual on Uniform Traffic Control Devices (MUTCD, 2009 Edition) outlines 9 warrants that may be used to justify installing a traffic signal at an intersection. The warrants are listed as follows:
I. Eight-Hour Vehicular Volume
2. Four-Hour Vehicular Volume
3. Peak Hour
4. Pedestrian Volume
5. School Crossing
6. Coordinated Signal System
7. Crash Experience
8. Roadway Network
9. Intersection Near a Grade Crossing

Of these nine, warrants I, 2, and 5 are applicable to conditions at the study intersections. Given the failing LOS on the eastbound and westbound approaches, the intersection of Woodmoor Drive with Willow Park Way is the only intersection that may need a signal in order to facilitate acceptable operations. Projected vehicular traffic volumes and pedestrian volumes were compared with warrant criteria to assess this potential. Because the intersection traffic counts only covered peak periods, a scaling factor of 0.0571 was used to estimate the fourth and eighth highest hour volumes using information from the Missouri Department of Transportation (MoDOT). Based on this information, it is estimated that the eighth highest hour comprises approximately 75 percent of the peak recorded hour. Each of the eight highest hours are estimated by scaling in linear fashion.

Utilizing the scaling assumptions, the evaluation of traffic-volume based Warrant I (eight-hour volume) and 2 (Four-Hour Volume) indicates that traffic volumes do not meet any of the specified conditions. A review of pedestrian volumes at the study intersections revealed a maximum of only 4 pedestrians crossing the intersection of Woodmoor Drive with Willow Park Way during the peak hour. Warrant 5 of the MUTCD specifies a need for at least 20 pedestrians during the peak hour to satisfy this warrant. Therefore, none of the study intersections are anticipated to meet signal warrant criteria.

## III.E. Access Design Considerations

The proposed bus access would be inbound only, meaning sight distance for outbound movements will not be of concern. The proposed access location will be approximately 200 feet north of the current Parking Lot A access. This spacing does not meet El Paso County Engineering Criteria Manual, (ECM) standards, which identify a minimum access spacing of 260 feet along roadways posted at 20 mph for single unit trucks (it is assumed that buses operate at single unit trucks).

Although traffic volumes for entering turning movements are not anticipated to meet auxiliary lane thresholds, it is recommended that a right turn deceleration lane be provided to provide refuge for slower moving buses. The EI Paso County Engineering Criteria Manual, (ECM) requires a right turn deceleration right turn lane width of 12 feet and a total length of 235 feet ( 25 mph design speed). Proximity of the existing handicap spaces along Woodmoor Drive would limit the recommended right turn deceleration lane length to 75 feet including taper, and existing topography along the west side of Woodmoor Drive would limit the available turn lane width to approximately 9 feet at its most narrow location. The turn lane dimensions have been maximized given these constraints, as shown on Figure 9.

Upon El Paso County review and approval of this traffic study, necessary deviation requests will be submitted to support access spacing and design deviations from the ECM.

The minimal storage length introduces a potential concern for vehicles coming around the Woodmoor Drive horizontal curve lacking adequate sight distance to see buses slowing at the proposed access. Calculations were completed to determine the distance needed for buses to slow to a reasonable turning speed in order to enter the parking lot. Table $\mathbf{3}$ outlines the calculation parameters and results.

## Table 3. Slowing Distance - Woodmoor Drive/New Bus Access

| Movement | Planned | Slowing Distance for School Buses |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Lane <br> Length | Posted <br> Speed | Turning <br> Speed | Assumed <br> Deceleration | Slowing <br> Time | Slowing <br> Distance |
| Southbound Right | 75 ft | 20 mph | 9 mph | $-4.49 \mathrm{ft} / \mathrm{sec}^{2}$ | 3.59 sec | 52.5 ft |

As shown above, the planned 75 feet of lane length (including taper) is anticipated to be sufficient room for buses to slow from the posted speed of 20 mph during school hours to a safe right turning speed of 9 mph . Buses will need just over 50 feet of space to make this adjustment. Therefore, buses will be able to decelerate within the lane and should not create sight distance safety issues. Also, in order to minimize outbound movements at this access, "DO NOT ENTER" signage should be considered within the parking lot.

Figure 9 shows the turning template for a typical bus. The new bus access configuration is anticipated to acceptably accommodate the turning movements of a typical bus. It should be noted that due to size constraints, bus drivers will likely need to coordinate to ensure all parking lots are filled and emptied during arrival and departure. Buses must leave the parking lot in a first in/first out sequence. If not used properly, buses will block entrances and exits which could lead to blockages and queueing onto Woodmoor Drive.

The intersection of Woodmoor Drive with Access I currently includes a southbound right turn deceleration lane and northbound center two way left turn lane. The southbound right turn lane is approximately 240 feet long and the northbound two way left turn lane provides approximately 250 feet of storage length to the next adjacent intersection. Operational analyses indicate these lengths will be adequate to accommodate anticipated queue lengths, and these lengths also satisfy ECM criteria for a 25 mph design speed. Of note, these lanes are likely to experience some queueing during peak school times, particularly during the PM peak hour as waiting drivers stage in the center northbound left turn lane. This condition should be monitored upon implementation.

## III.F. Parking Lot C Queueing

As previously discussed, all private vehicle pick up and drop off activity will occur in Lot $C$ upon completion of the proposed reconfiguration. This activity will place demand on the sidewalk space along the west edge of Parking Lot C. This sidewalk length currently totals approximately 560 feet, plus the additional 140 feet of curb length provided along Willow Park Way west of Access 2 for a total of 700 feet of curb space. It is anticipated that the recommended circulation pattern shown in Figure $\mathbf{8}$ would make use of this full 700 feet of curb space for pick up and drop off activity.

Based on calculations from school calculator worksheets provided by the North Carolina Department of Transportation (NDOT), the LPMS school size can translate to a need for roughly I500 feet of queueing space needed for pick up and drop off. It is likely that this length is conservative, given that some similar schools to LPMS reviewed provide roughly 500 feet of curbside space for pick up and drop off. In light of the range of potential curb space needs, it is possible that the existing curb space available in Parking Lot C will not be sufficient to accommodate all demand. In the event that offsite queueing space is needed along Woodmoor Drive, such space would be provided within the existing right and left turn auxiliary lanes approaching Access I. This condition should be monitored upon implementation.

## III.G. Pedestrian and Bicyclist Safety

Pedestrian and bicyclist counts were taken at each intersection within the study area. Pedestrian and bicyclist volumes were generally low with no more than 4 crossings at any location during the peak hours. Currently, there are sidewalks within the school property; however, there are no sidewalks along Woodmoor Drive or controlled crossings. Due to the low volume of pedestrians and cyclists in this area, the lack of sidewalks, crosswalks, and bicycle lanes may not be an issue. However, adequate sidewalks are desirable to maximize student safety walking to and from the school. Provision of a sidewalk along the west side of Woodmoor Drive from Willow Park Way would be appropriate for future consideration; with or without the proposed reconfiguration of school traffic and parking currently under consideration.

## Lewis Palmer Middle School

PARKING LOT A - AUTOTURN


## IV. SUMMARY AND RECOMMENDATIONS

A new inbound only bus access is proposed for the Lewis Palmer Middle School parking lot to convert to bus only during school peaks. The proposed bus access would be constructed north of the existing parking lot access along Woodmoor Drive in Monument, Colorado. Surrounding areas are primarily residential with some commercial space north of the school.

Two future scenarios were analyzed for this report:

- Short-term Future - Time period for the completion of the new access, estimated as the Year 2023.
- Long-term Future - The year 2045 was used to assess traffic impacts of the development in the long-term future.

The following is a summary of the findings and recommendations related to the analysis for the development:

- The planned 75 feet of turn lane length (including taper) for the southbound right turn deceleration lane at the new bus access is anticipated to provide enough space for buses to slow from the posted speed of 20 mph to a safe turning speed of 9 mph without causing sight distance issues for vehicles traveling around the Woodmoor Drive horizontal curve.
- The eastbound approach to the intersection of Woodmoor Drive with Willow Park Way is anticipated to experience delays and queueing during school pick up and drop off activities. In view of this, it is recommended that the configuration shown in Figure 8 be implemented to provide enhanced exit efficiency and safety. This configuration would restrict left turns and route exiting traffic west along Willow Park Way in order to reach Woodmoor Drive.
- Monitoring of both bus and parent traffic within the study area should occur upon implementation of these changes to determine if adjustments should be made if problems emerge.
- Access 3 should be monitored to ensure parents and visitors are aware of the new access restriction and do not use this lot during peak hours.
- Access 3A should be monitored to ensure efficiency entering the parking lot. Woodmoor Drive should not experience blockages due to the new access.
- Access 2 should be monitored to ensure drivers unfamiliar with this access treat it as outbound only.
- Access I and parking area C should be monitored to ensure that additional student drop-off and pickup activities do not cause concern or backups onto Woodmoor Drive.
- U-turn activity along Woodmoor Drive should be monitored.
- Provision of a sidewalk along the west side of Woodmoor Drive from Willow Park Way would be appropriate for future consideration; with or without the proposed reconfiguration of school traffic and parking currently under consideration.
- Upon El Paso County review and approval of this traffic study, necessary deviation requests will be submitted to support access spacing and design deviations from the ECM.


## APPENDIX A. EXISTING TRAFFIC COUNTS

(303) 216-2439 www.alltrafficdata.net

Location: 1 WOODMOOR DR \& PARKING LOT ACCESS AM
Date: Wednesday, December 14, 2022
Peak Hour: 07:00 AM - 08:00 AM
Peak 15-Minutes: 07:15 AM - 07:30 AM


Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

| Interval <br> Start Time | PARKING LOT ACCESS Eastbound |  |  |  | PARKING LOT ACCESS Westbound |  |  |  | WOODMOOR DR Northbound |  |  |  |  | WOODMOOR DR <br> Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru | Right |  |  | West | East | South |  |
| 6:30 AM | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 |  | 0 | 0 | 4 | 1 | 0 | 0 |  | 21 | 0 | 28 | 559 | 0 | 0 | 0 | 0 |
| 6:45 AM | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |  | 0 | 7 | 14 | 0 | 0 | 0 |  | 34 | 3 | 61 | 612 | 0 | 0 | 0 | 0 |
| 7:00 AM | 0 | 9 | 2 | 63 | 0 | 2 | 0 | 0 |  | 0 | 45 | 16 | 0 | 0 | 0 |  | 34 | 41 | 212 | 652 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 6 | 3 | 93 | 0 | 0 | 1 | 2 |  | 0 | 36 | 30 | 1 | 0 | 0 |  | 46 | 40 | 258 | 540 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 |  | 0 | 0 | 24 | 0 | 0 | 0 |  | 56 | 0 | 81 | 363 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |  | 0 | 0 | 36 | 4 | 0 | 0 |  | 60 | 0 | 101 |  | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 58 | 0 | 0 | 0 |  | 40 | 0 | 100 |  | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 |  | 0 | 0 | 34 | 0 | 0 | 0 |  | 46 | 0 | 81 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 16 | 7 | 157 | 0 | 7 | 1 | 3 |  | 0 | 88 | 216 | 6 | 0 | 0 |  | 337 | 84 | 922 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 15 | 5 | 156 | 0 | 3 | 1 | 3 |  | 0 | 81 | 106 | 5 | 0 |  | 0 | 196 | 81 | 65 | 52 | 0 | 0 | 0 | 0 |



Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

| Interval | MAIN | Eastb | L ACC | CESS | West | und | WOODMOOR DR Northbound |  |  |  |  | WOODMOOR DR Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn Left | Thru Right | U-Turn | Left | Thru R | Right |  | J-Turn | Left |  | Thru | Right |  |  | West | East | South |  |
| 6:30 AM | 0 | 1 | 0 | 0 |  |  | 0 | 5 | 5 | 0 |  | 0 | 0 |  | 22 | 1 | 34 | 525 | 0 |  | 0 | 0 |
| 6:45 AM | 0 | 0 | 0 | 0 |  |  | 0 | 17 | 21 | 0 |  | 0 | 0 |  | 28 | 8 | 74 | 579 | 0 |  | 0 | 0 |
| 7:00 AM | 0 | 0 | 0 | 0 |  |  | 0 | 38 | 67 | 0 |  | 1 | 0 |  | 88 | 8 | 202 | 613 | 0 |  | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 |  |  | 0 | 11 | 60 | 0 |  | 0 | 0 |  | 141 | 3 | 215 | 515 | 0 |  | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 |  |  | 0 | 8 | 25 | 0 |  | 0 | 0 |  | 52 | 3 | 88 | 388 | 0 |  | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 |  |  | 0 | 5 | 40 | 0 |  | 0 | 0 |  | 60 | 3 | 108 |  | 0 |  | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 |  |  | 0 | 4 | 59 | 0 |  | 0 | 0 |  | 41 | 0 | 104 |  | 0 |  | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 |  |  | 0 | 6 | 33 | 0 |  | 0 | 0 |  | 49 | 0 | 88 |  | 0 |  | 0 | 0 |
| Count Total | 0 | 1 | 0 | 0 |  |  | 0 | 94 | 310 | 0 | 0 | 1 | 0 | 0 | 481 | 26 | 913 |  | 0 |  | 0 | 0 |
| Peak Hour | 0 | 0 | 0 | 0 |  |  | 0 | 62 | 192 | 0 | 0 | 1 |  | 0 | 341 | 17 | 613 | 3 | 0 |  | 0 | 0 |

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Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.

## Traffic Counts - Motorized Vehicles

| Interval | WILLOW PARK WAY <br> Eastbound |  |  |  | WILLOW PARK WAY <br> Westbound |  |  |  |  | WOODMOOR DR Northbound |  |  |  | WOODMOOR DR Southbound |  |  |  |  | Total |  | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru R | Right |  | U-Turn | Left | Thru | Right |  | J-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 6:30 AM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 1 | 0 | 10 | 9 | 0 |  | 0 | 0 | 23 | 0 |  | 45 |  | 794 | 0 | 0 | 0 | 0 |
| 6:45 AM | 0 | 2 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 26 | 37 | 1 |  | 0 | 0 | 27 | 0 |  | 103 | 853 | 0 | 0 | 0 | 0 |
| 7:00 AM | 0 | 0 | 0 | 90 | 0 | 1 | 0 | 1 | 1 | 0 | 62 | 108 | 0 |  | 0 | 0 | 75 | 1 |  | 338 | 865 | 0 | 0 | 0 | 4 |
| 7:15 AM | 0 | 2 | 0 | 60 | 0 | 1 | 0 | 1 | 1 | 0 | 36 | 61 | 0 |  | 0 | 1 | 145 | 1 |  | 308 | 642 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 5 | 0 | 15 | 0 | 1 | 0 | 2 | 2 | 0 | 3 | 26 | 0 |  | 0 | 1 | 50 | 1 |  | 104 | 434 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 2 | 2 | 0 | 6 | 38 | 1 |  | 0 | 1 | 60 | 0 |  | 115 |  | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 7 | 0 | 0 | 0 |  | 1 | 0 | 3 | 64 | 0 |  | 0 | 0 | 39 | 1 |  | 115 |  | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 4 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 1 | 4 | 36 | 0 |  | 0 | 0 | 47 | 0 |  | 100 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 15 | 0 | 197 | 0 | 3 | 0 | 8 | 8 | 1 | 150 | 379 | 2 | 2 | 0 | 3 | 466 |  | 4 | 1,228 |  | 0 | 0 | 0 | 4 |
| Peak Hour | 0 | 9 | 0 | 170 | 0 | 3 | 0 | 6 | 6 | 0 | 107 | 233 | 1 | 1 | 0 | 3 | 330 |  | 3 | 86 | 5 | 0 | 0 | 0 | 4 |

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Note: Total study counts contained in parentheses.

## Traffic Counts - Motorized Vehicles

| Interval Start Time | WILLOW PARK WAY <br> Eastbound |  |  |  | WILLOW PARK WAY <br> Westbound |  |  |  | SECONDARY SCHOOL ACCESS Northbound |  |  |  | SECONDARY SCHOOL ACCESS Southbound |  |  |  |  |  | Rolling | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | eft | Thru R |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  | Total | Hour | West | East |  |  |
| 6:30 AM | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 300 | 0 | 0 | 0 | 0 |
| 6:45 AM | 0 | 0 | 5 | 0 | 0 | 1 | 12 | 13 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 38 | 310 | 0 | 0 | 0 | 0 |
| 7:00 AM | 0 | 0 | 52 | 0 | 0 | 0 | 62 | 1 | 0 | 0 | 0 | 0 | 0 | 36 | 0 |  | 0 | 151 | 285 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 42 | 0 | 0 | 0 | 38 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 0 | 0 | 0 | 99 | 145 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 0 | 0 | 0 | 22 | 63 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 2 | 0 | 0 | 1 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 13 |  | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 1 | 0 | 0 | 1 | 3 | 0 | 0 | 0 | 0 | 1 | 0 | 5 | 0 | 0 | 0 | 11 |  | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 4 | 0 | 1 | 2 | 2 | 0 | 0 | 0 | 0 | 3 | 0 | 5 | 0 | 0 | 0 | 17 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 0 | 112 | 0 | 1 | 5 | 129 | 21 | 0 | 0 | 0 | 4 | 0 | 91 | 0 |  | 0 | 363 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 0 | 103 | 0 | 0 | 1 | 116 | 14 | 0 | 0 | 0 | 0 | 0 | 76 | 0 | 0 | 0 | 31 | 10 | 0 | 0 | 0 | 0 |

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Location: 1 WOODMOOR DR \& PARKING LOT ACCESS PM
Date: Wednesday, December 14, 2022
Peak Hour: 02:30 PM - 03:30 PM
Peak 15-Minutes: 02:30 PM - 02:45 PM

Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

| Interval Start Time | PARKING LOT ACCESS Eastbound |  |  |  | PARKING LOT ACCESS Westbound |  |  |  |  | WOODMOOR DR Northbound |  |  |  |  | WOODMOOR DR Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru R | Right |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 2:00 PM | 0 | 0 | 1 | 0 | 0 | 1 |  | 0 | 1 |  | 0 | 14 | 29 | 2 | 0 | 0 | 29 | 3 | 80 | 485 | 0 | 0 | 0 | 0 |
| 2:15 PM | 0 | 0 | 0 | 4 | 0 | 1 |  | 0 | 0 |  | 2 | 3 | 39 | 3 | 0 | 1 | 28 | 0 | 81 | 491 | 0 | 0 | 0 | 0 |
| 2:30 PM | 0 | 2 | 0 | 45 | 0 | 2 |  | 0 | 4 |  | 2 | 19 | 42 | 3 | 4 | 1 | 32 | 23 | 179 | 499 | 4 | 0 | 4 | 0 |
| 2:45 PM | 0 | 9 | 0 | 25 | 0 | 1 |  | 0 | 0 |  | 1 | 7 | 48 | 1 | 0 | 0 | 49 | 4 | 145 | 400 | 2 | 0 | 2 | 0 |
| 3:00 PM | 0 | 3 | 0 | 5 | 0 | 1 |  | 0 | 0 |  | 0 | 2 | 41 | 1 | 0 | 0 | 32 | 1 | 86 | 340 | 0 | 0 | 0 | 0 |
| 3:15 PM | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 51 | 1 | 0 | 0 | 37 | 0 | 89 |  | 0 | 0 | 0 | 0 |
| 3:30 PM | 0 | 1 | 1 | 0 | 0 | 2 |  | 0 | 0 |  | 0 | 1 | 37 | 2 | 0 | 0 | 36 | 0 | 80 |  | 0 | 0 | 0 | 0 |
| 3:45 PM | 0 | 0 | 2 | 1 | 0 | 1 |  | 0 | 0 |  | 0 | 3 | 41 | 0 | 0 | 0 | 37 | 0 | 85 |  | 0 | 0 | 1 | 0 |
| Count Total | 0 | 15 | 4 | 80 | 0 | 9 |  | 0 | 5 |  | 5 | 49 | 328 | 13 | 4 | 2 | 280 | 31 | 825 |  | 6 | 0 | 7 | 0 |
| Peak Hour | 0 | 14 | 0 | 75 | 0 | 4 |  | 0 | 4 |  | 3 | 28 | 182 | 6 | 4 | 1 | 150 | 28 | 499 | 9 | 6 | 0 | 6 | 0 |

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Location: 2 WOODMOOR DR \& MAIN SCHOOL ACCESS PM
Date: Wednesday, December 14, 2022
Peak Hour: 02:00 PM - 03:00 PM
Peak 15-Minutes: 02:45 PM - 03:00 PM


Note: Total study counts contained in parentheses.

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Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.

## Traffic Counts - Motorized Vehicles

| Interval <br> Start Time | WILLOW PARK WAY <br> Eastbound |  |  |  | WILLOW PARK WAY <br> Westbound |  |  |  |  |  | WOODMOOR DR <br> Northbound |  |  |  | WOODMOOR DR Southbound |  |  |  |  | Total |  | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru |  |  | U-Turn | Left | Thru | Right |  |  | Left | Thru | Right |  |  | West | East | South | orth |
| 2:00 PM | 0 | 1 | 0 | 11 | 0 | 1 |  | 0 |  | 0 | 0 | 10 | 53 | 0 | 0 | 0 | 0 | 30 | 1 |  | 107 |  | 716 | 0 | 0 | 0 | 0 |
| 2:15 PM | 0 | 2 | 0 | 6 | 0 | 0 |  | 0 |  | 0 | 0 | 19 | 76 | 0 | 0 | 1 | 0 | 30 | 0 |  | 134 | 723 | 0 | 0 | 0 | 0 |
| 2:30 PM | 0 | 0 | 0 | 64 | 0 | 2 |  | 0 |  | 1 | 0 | 23 | 64 | 5 | 5 | 0 | 1 | 70 | 1 |  | 231 | 701 | 0 | 0 | 0 | 12 |
| 2:45 PM | 0 | 5 | 0 | 70 | 0 | 1 |  | 0 |  | 3 | 0 | 7 | 72 | 1 |  | 0 | 1 | 80 | 4 |  | 244 | 591 | 0 | 0 | 0 | 0 |
| 3:00 PM | 0 | 1 | 0 | 17 | 0 | 0 |  | 0 |  | 0 | 0 | 9 | 41 | 1 |  | 0 | 1 | 43 | 1 |  | 114 | 449 | 0 | 0 | 0 | 0 |
| 3:15 PM | 0 | 5 | 0 | 14 | 0 | 0 |  | 0 |  | 1 | 0 | 4 | 52 | 0 | 0 | 0 | 0 | 36 | 0 |  | 112 |  | 0 | 0 | 0 | 0 |
| 3:30 PM | 0 | 1 | 0 | 23 | 0 | 1 |  | 0 |  | 2 | 0 | 11 | 41 | 0 | 0 | 0 | 1 | 39 | 2 |  | 121 |  | 0 | 0 | 0 | 0 |
| 3:45 PM | 0 | 3 | 0 | 4 | 0 | 1 |  | 0 |  | 0 | 0 | 3 | 49 | 0 | 0 | 0 | 1 | 41 | 0 |  | 102 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 18 | 0 | 209 | 0 | 6 |  | 0 |  | 7 | 0 | 86 | 448 | 7 | 7 | 1 | 5 | 369 | 9 | 9 | 1,165 |  | 0 | 0 | 0 | 12 |
| Peak Hour | 0 | 8 | 0 | 157 | 0 | 3 |  | 0 |  | 4 | 0 | 58 | 253 | 7 | 7 | 1 | 3 | 223 |  | 6 | 72 | 23 | 0 | 0 | 0 | 12 |

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Location: 4 SECONDARY SCHOOL ACCESS \& WILLOW PARK WAY PM
Date: Wednesday, December 14, 2022
Peak Hour: 02:15 PM - 03:15 PM
Peak 15-Minutes: 02:30 PM - 02:45 PM


Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.

## Traffic Counts - Motorized Vehicles

| Interval <br> Start Time | WILLOW PARK WAY <br> Eastbound |  |  |  | WILLOW PARK WAY <br> Westbound |  |  |  | SECONDARY SCHOOL ACCESS$\qquad$ Northbound |  |  |  | SECONDARY SCHOOL ACCESS Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru R | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  |  | West | East | South |  |
| 2:00 PM | 0 | 0 | 6 | 0 | 0 | 0 | 10 | 1 | 0 | 0 | 0 | 0 | 0 | 6 | 0 |  | 0 | 23 | 228 | 0 | 0 | 0 | 0 |
| 2:15 PM | 0 | 0 | 0 | 0 | 0 | 1 | 20 | 0 | 0 | 0 | 0 | 3 | 0 | 4 | 0 |  | 0 | 28 | 233 | 1 | 0 | 0 | 0 |
| 2:30 PM | 0 | 0 | 44 | 1 | 0 | 2 | 21 | 0 | 0 | 0 | 0 | 3 | 0 | 21 | 0 |  | 0 | 92 | 228 | 38 | 0 | 0 | 1 |
| 2:45 PM | 0 | 0 | 14 | 0 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 11 | 0 | 47 | 0 |  | 2 | 85 | 173 | 1 | 0 | 0 | 0 |
| 3:00 PM | 0 | 0 | 1 | 0 | 0 | 3 | 5 | 2 | 20 | 0 | 0 | 4 | 0 | 13 | 0 |  | 0 | 28 | 97 | 2 | 0 | 0 | 0 |
| 3:15 PM | 0 | 0 | 6 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 4 | 0 | 9 | 0 |  | 0 | 23 |  | 0 | 0 | 0 | 0 |
| 3:30 PM | 0 | 0 | 7 | 0 | 0 | 5 | 6 | 2 | 0 | 0 | 0 | 2 | 0 | 15 | 0 |  | 0 | 37 |  | 0 | 0 | 0 | 0 |
| 3:45 PM | 0 | 0 | 4 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 |  | 0 | 9 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 0 | 82 | 1 | 0 | 11 | 80 | 5 | 50 | 0 | 0 | 27 | 0 | 117 | 0 |  | 2 | 325 |  | 42 | 0 | 0 | 1 |
| Peak Hour | 0 | 0 | 59 | 1 | 0 | 6 | 57 | 2 | 20 | 0 | 0 | 21 | 0 | 85 | 0 | 0 | 2 | 233 |  | 42 | 0 | 0 | 1 |

## APPENDIX B. EXISTING LEVEL OF SERVICE WORKSHEETS




| Intersection |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.9 |  |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBU | SBT | SBR |
| Lane Configurations | $\mathbf{r}$ |  | 1 | 4 |  | A | 「 |
| Traffic Vol, veh/h | 0 | 0 | 62 | 192 | 1 | 341 | 17 |
| Future Vol, veh/h | 0 | 0 | 62 | 192 | 1 | 341 | 17 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | - | None |
| Storage Length | 0 | - | 100 | - | - | - | 200 |
| Veh in Median Storage, \# | 0 | - | - | 0 | - | 0 | - |
| Grade, \% | 0 | - | - | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 64 | 64 | 60 | 60 | 60 |
| Heavy Vehicles, \% | 2 | 2 | 8 | 8 | 1 | 1 | 1 |
| Mvmt Flow | 0 | 0 | 97 | 300 | 2 | 568 | 28 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 18.9 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | $\dagger$ |  |  | $\leqslant$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  |  |
| Traffic Vol, veh/h | 9 | 0 | 170 | 3 | 0 | 6 | 107 | 233 | 1 | 3 | 330 | 3 |  |
| Future Vol, veh/h | 9 | 0 | 170 | 3 | 0 | 6 | 107 | 233 | 1 | 3 | 330 | 3 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control Star | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | 100 | - | - | 100 | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 47 | 47 | 47 | 75 | 75 | 75 | 50 | 50 | 50 | 57 | 57 | 57 |  |
| Heavy Vehicles, \% | 13 | 13 | 13 | 0 | 0 | 0 | 6 | 6 | 6 | 0 | 0 | 0 |  |
| Mvmt Flow | 19 | 0 | 362 | 4 | 0 | 8 | 214 | 466 | 2 | 5 | 579 | 5 |  |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 3.9 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\stackrel{+}{*}$ |  |  | \& |  |  | * |  |  | 4 |  |
| Traffic Vol, veh/h | 0 | 100 | 0 | 1 | 109 | 1 | 0 | 0 | 0 | 74 | 0 | 0 |
| Future Vol, veh/h | 0 | 100 | 0 | 1 | 109 | 1 | 0 | 0 | 0 | 74 | 0 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 48 | 48 | 48 | 44 | 44 | 44 | 92 | 92 | 92 | 51 | 51 | 51 |
| Heavy Vehicles, \% | 1 | 1 | 1 | 1 | 1 | 1 | 2 | 2 | 2 | 28 | 28 | 28 |
| Mvmt Flow | 0 | 208 | 0 | 2 | 248 | 2 | 0 | 0 | 0 | 145 | 0 | 0 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.7 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBU | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | $\uparrow$ | F |  | ${ }^{7}$ | $\uparrow$ | F |  | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Vol, veh/h | 14 | 0 | 79 | 5 | 0 | 4 | 5 | 31 | 170 | 8 | 4 | 2 | 141 | 28 |
| Future Vol, veh/h | 14 | 0 | 79 | 5 | 0 | 4 | 5 | 31 | 170 | 8 | 4 | 2 | 141 | 28 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control S | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | - | None | - | - | - | None |
| Storage Length | - | - | - | - | - | 0 | - | 100 | - | 100 | - | 100 | - | 125 |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | - | 0 | - | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | - | 0 | - | - | - | 0 | - |
| Peak Hour Factor | 43 | 43 | 43 | 46 | 46 | 46 | 92 | 81 | 81 | 81 | 92 | 65 | 65 | 65 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 3 | 3 | 3 | 2 | 0 | 0 | 0 |
| Mumt Flow | 33 | 0 | 184 | 11 | 0 | 9 | 5 | 38 | 210 | 10 | 4 | 3 | 217 | 43 |



| Approach | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| HCM Control Delay, s | 11.8 | 13.6 |  |  |
| HCM LOS | B | B |  |  |


| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1WBLn1WBLn2 | SBL | SBT | SBR |  |  |
| :--- | :---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- |
| Capacity (veh/h) | + | - | - | 741 | 312 | 835 | $\sim$ | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |




| Major/Minor | Minor2 | Minor1 |  |  |  | Major1 |  |  | Major2 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 818 | 819 | 328 | 955 | 819 | 329 | 332 | 0 | 0 | - | 333 | 0 | 0 |
| Stage 1 | 336 | 338 | - | 477 | 477 | - | - | - | - | - | - | - | - |
| Stage 2 | 482 | 481 |  | 478 | 342 | - |  | - | - | - | - | - | - |
| Critical Hdwy | 7.23 | 6.63 | 6.33 | 7.1 | 6.5 | 6.2 | 4.19 | - | - | - | 4.1 | - | - |
| Critical Hdwy Stg 1 | 6.23 | 5.63 | - | 6.1 | 5.5 | - | - | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | 6.23 | 5.63 | - | 6.1 | 5.5 | - | - | - | - | - | - | - | - |
| Follow-up Hdwy | 3.617 | 4.117 | 3.417 | 3.5 | 4 | 3.3 | 2.281 | - | - | - | 2.2 | - | - |
| Pot Cap-1 Maneuver | 282 | 298 | 689 | 240 | 312 | 717 | 1189 | - | - | - | 1238 | - | - |
| Stage 1 | 656 | 621 | - | 573 | 559 | - | - | - | - | - - | - | - | - |
| Stage 2 | 545 | 536 | - | 572 | 642 | - | - | - | - | - - | - | - | - |
| Platoon blocked, \% |  |  |  |  |  |  |  | - | - |  |  | - | - |
| Mov Cap-1 Maneuver | 264 | 280 | 689 | 134 | 293 | 717 | 1189 | - |  | $\sim-5$ | $\sim-5$ | - | - |
| Mov Cap-2 Maneuver | 264 | 280 | - | 134 | 293 | - | - | - | - | - - | - | - | - |
| Stage 1 | 615 | 621 | - | 537 | 524 | - | - | - | - | - | - | - | - |
| Stage 2 | 503 | 503 | - | 335 | 642 | - | - | - | - | - - | - | - | - |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 15.5 |  |  | 20.6 |  |  | 1.5 |  |  |  |  |  |  |
| HCM LOS | C |  |  | C |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvm |  | NBL | NBT | NBR | EBLn1 | BLn1 | SBL | SBT | SBR |  |  |  |  |
| Capacity (veh/h) |  | 1189 | - | - | 639 | 250 | + | - | - |  |  |  |  |
| HCM Lane V/C Ratio |  | 0.063 | - | - | 0.469 | 0.08 | - | - | - |  |  |  |  |
| HCM Control Delay (s) |  | 8.2 | - | - | 15.5 | 20.6 | - | - | - |  |  |  |  |
| HCM Lane LOS |  | A | - | - | C | C | - | - | - |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | 0.2 | - |  | 2.5 | 0.3 | - | - | - |  |  |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ : Volume exceeds cap | apacity | \$: Dela | lay exc | eeds 3 | 00s | : Com | putatio | Not D | fined | *: All | major volur | e | atoon |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 6.3 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \& |  |  | * |  |  | 4 |  |
| Traffic Vol, veh/h | 0 | 59 | 1 | 6 | 57 | 2 | 0 | 0 | 21 | 85 | 0 | 2 |
| Future Vol, veh/h | 0 | 59 | 1 | 6 | 57 | 2 | 0 | 0 | 21 | 85 | 0 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 33 | 33 | 33 | 65 | 65 | 65 | 48 | 48 | 48 | 45 | 45 | 45 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 3 | 3 | 3 | 0 | 0 | 0 | 25 | 25 | 25 |
| Mvmt Flow | 0 | 179 | 3 | 9 | 88 | 3 | 0 | 0 | 44 | 189 | 0 | 4 |



## APPENDIX C. SHORT-TERM FUTURE LEVEL OF SERVICE WORKSHEETS

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ | 「 | ${ }^{*}$ | $\uparrow$ | 「 | ${ }^{7}$ | $\uparrow$ | 7 |  |
| Traffic Vol, veh/h | 3 | O | 21 | 5 | 0 | 3 | 0 | 117 | 10 | 0 | 275 | 0 |  |
| Future Vol, veh/h | 3 | 0 | 21 | 5 | 0 | 3 | 0 | 117 | 10 | 0 | 275 | 0 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - |  | None |  |
| Storage Length | - | - | - | - | - | 0 | 100 | - | - | 100 | - | 125 |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 42 | 42 | 42 | 36 | 36 | 36 | 60 | 60 | 60 | 72 | 72 | 72 |  |
| Heavy Vehicles, \% | 100 | 100 | 100 | 0 | 100 | 0 | 100 | 0 | 0 | 0 | 0 | 100 |  |
| Mvmt Flow | 7 | 0 | 50 | 14 | 0 | 8 | 0 | 195 | 17 | 0 | 382 | 0 |  |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | 4 | 个 | $\mathbf{T}$ |
| Traffic Vol, veh/h | 0 | 0 | 234 | 128 | 207 | 96 |
| Future Vol, veh/h | 0 | 0 | 234 | 128 | 207 | 96 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | 100 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 64 | 64 | 60 | 60 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 8 | 1 | 0 |
| Mvmt Flow | 0 | 0 | 366 | 200 | 345 | 160 |






| Major/Minor N | Major1 |  | Major2 |  |  | Minor1 |  | Minor2 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 2 | 0 | 0 | 2 | 0 | 0 | 8 | 8 | 2 | 8 | 8 | 2 |  |
| Stage 1 | - | - | - | - | - | - | 2 | 2 | - | 6 | 6 | - |  |
| Stage 2 | - | - | - | - | - | - | 6 | 6 | - | 2 | 2 | - |  |
| Critical Hdwy | 4.11 | - | - | 4.11 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |  |
| Follow-up Hdwy | 2.209 | - | - | 2.209 | - | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |  |
| Pot Cap-1 Maneuver | 1627 | - | - | 1627 | - | - | 1011 | 887 | 1082 | 1011 | 887 | 1082 |  |
| Stage 1 | - | - | - | - | - | - | 1021 | 894 |  | 1016 | 891 | - |  |
| Stage 2 | - | - | - | - | - | - | 1016 | 891 | - | 1021 | 894 | - |  |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 1627 | - | - | 1627 | - | - | 1010 | 886 | 1082 | 1010 | 886 | 1082 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 1010 | 886 | - | 1010 | 886 | - |  |
| Stage 1 | - | - | - | - | - | - | 1021 | 894 | - | 1016 | 890 | - |  |
| Stage 2 | - | - | - | - | - | - | 1015 | 890 | - | 1021 | 894 | - |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 0 |  |  | 3.6 |  |  | 0 |  |  | 14.5 |  |  |  |
| HCM LOS |  |  |  |  |  |  | A |  |  | B |  |  |  |
| Minor Lane/Major Mvm |  |  | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 |  |  |  |  |
| Capacity (veh/h) |  | - | 1627 | - | - | 1627 | - | - | 1010 |  |  |  |  |
| HCM Lane V/C Ratio |  | - | - | - | - | 0.001 | - | - | 0.633 |  |  |  |  |
| HCM Control Delay (s) |  | 0 | 0 | - | - | 7.2 | 0 | - | 14.5 |  |  |  |  |
| HCM Lane LOS |  | A | A | - | - | A | A | - | B |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | - | 0 | - | - | 0 | - | - | 4.7 |  |  |  |  |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | 4 | 4 | $\mathbf{T}$ |
| Traffic Vol, veh/h | 4 | 11 | 136 | 186 | 173 | 33 |
| Future Vol, veh/h | 4 | 11 | 136 | 186 | 173 | 33 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | 100 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 47 | 47 | 83 | 83 | 69 | 69 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 2 | 0 | 0 |
| Mvmt Flow | 9 | 23 | 164 | 224 | 251 | 48 |





| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 12.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | \& |  |  | $\uparrow$ |  |  | \& |  |
| Traffic Vol, veh/h | 0 | 0 | 1 | 6 | 1 | 0 | 0 | 0 | 21 | 213 | 0 | 2 |
| Future Vol, veh/h | 0 | 0 | 1 | 6 | 1 | 0 | 0 | 0 | 21 | 213 | 0 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 33 | 33 | 33 | 65 | 65 | 65 | 48 | 48 | 48 | 45 | 45 | 45 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 3 | 3 | 3 | 0 | 0 | 0 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 3 | 9 | 2 | 0 | 0 | 0 | 44 | 473 | 0 | 4 |



## APPENDIX D. LONG-TERM FUTURE LEVEL OF SERVICE WORKSHEETS




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.3 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | 4 | 个 | $\mathbf{T}$ |
| Traffic Vol, veh/h | 0 | 0 | 234 | 146 | 236 | 96 |
| Future Vol, veh/h | 0 | 0 | 234 | 146 | 236 | 96 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | 100 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 64 | 64 | 60 | 60 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 8 | 1 | 0 |
| Mvmt Flow | 0 | 0 | 366 | 228 | 393 | 160 |




| Major/Minor $\quad$ N | Minor2 |  |  | Minor1 |  |  | Major1 |  |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1157 | 1154 | 396 | 1473 | 1155 | 747 | 398 | 0 | 0 | 748 | 0 | 0 |
| Stage 1 | 406 | 406 | - | 747 | 747 | - | - | - | - | - | - | - |
| Stage 2 | 751 | 748 | - | 726 | 408 | - | - | - | - | - | - | - |
| Critical Hdwy | 7.12 | 6.52 | 6.22 | 7.1 | 6.5 | 6.2 | 4.16 | - | - | 4.1 | - | - |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - | 6.1 | 5.5 | - | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - | 6.1 | 5.5 | - | - | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 | 3.5 | 4 | 3.3 | 2.254 | - | - | 2.2 | - | - |
| Pot Cap-1 Maneuver | 173 | 197 | 653 | 106 | 199 | 416 | 1139 | - | - | 870 | - | - |
| Stage 1 | 622 | 598 | - | 408 | 423 | - | - | - | - | - | - | - |
| Stage 2 | 403 | 420 | - | 419 | 600 | - | - | - | - | - | - | - |
| Platoon blocked, \% |  |  |  |  |  |  |  | - | - |  | - | - |
| Mov Cap-1 Maneuver | 169 | 196 | 653 | $\sim 2$ | 198 | 416 | 1139 | - | - | 870 | - | - |
| Mov Cap-2 Maneuver | 169 | 196 | - | $\sim 2$ | 198 | - | - | - | - | - | - | - |
| Stage 1 | 622 | 594 | - |  | 423 | - | - | - | - | - | - | - |
| Stage 2 | 395 | 420 | - | 8 | 596 | - | - | - | - | - | - | - |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| HCM Control Delay, s | 174.7 |  |  | 1598.5 |  |  | 0 |  |  | 0.1 |  |  |
| HCM LOS | F |  |  | F |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT | NBR | EBLn1W | BLn1 | SBL | SBT | SBR |  |  |  |
| Capacity (veh/h) |  | 1139 | - | - | 532 | 6 | 870 | - | - |  |  |  |
| HCM Lane V/C Ratio |  | - | - | - | 1.308 | 2 | 0.006 | - | - |  |  |  |
| HCM Control Delay (s) |  | 0 | - | - | $174 . \$ 1$ | 598.5 | 9.2 | - | - |  |  |  |
| HCM Lane LOS |  | A | - | - | F | F | A | - | - |  |  |  |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | 29.4 | 2.5 | 0 | - | - |  |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |
| $\sim:$ Volume exceeds capacity |  | \$: Delay exceeds 300s + |  |  |  | +: Computation Not Defined |  |  |  | *: All major volume in platoon |  |  |






| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.2 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | $\mathbf{Y}$ |  |  | 4 | 个 | $\mathbf{T}$ |
| Traffic Vol, veh/h | 4 | 11 | 136 | 212 | 197 | 33 |
| Future Vol, veh/h | 4 | 11 | 136 | 212 | 197 | 33 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | 100 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 47 | 47 | 83 | 83 | 69 | 69 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 2 | 0 | 0 |
| Mvmt Flow | 9 | 23 | 164 | 255 | 286 | 48 |





| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 12.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | \& |  |  | $\uparrow$ |  |  | \& |  |
| Traffic Vol, veh/h | 0 | 0 | 1 | 6 | 1 | 0 | 0 | 0 | 21 | 213 | 0 | 2 |
| Future Vol, veh/h | 0 | 0 | 1 | 6 | 1 | 0 | 0 | 0 | 21 | 213 | 0 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 33 | 33 | 33 | 65 | 65 | 65 | 48 | 48 | 48 | 45 | 45 | 45 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 3 | 3 | 3 | 0 | 0 | 0 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 3 | 9 | 2 | 0 | 0 | 0 | 44 | 473 | 0 | 4 |



