Please add a signatures page after the cover page. Visit https://planningdevelopment.elpasoco.com/planning-deve lopment-forms/\#1584029763943-19bc4c03-3586 for El Paso County standard signature blocks.

## TRAFFIC ANALYSIS REPORT

Lewis Palmer Middle School
Monument, CO

Prepared for:
Lewis Palmer District Transportation
99 Santa Fe Ave
Monument, CO 80132

Prepared by:
Felsburg Holt \& Ullevig
6400 South Fiddlers Green Circle, Suite 1500
Greenwood Village, CO 801 I I
303.72I.I440

Project Manager: Lyle DeVries, PE, PTOE
Project Engineer: Faith Kelley, El

FHU Reference No. I22227-OI
January 2023
Please add "PCD File No. CDR-23-005"

## TABLE OF CONTENTS

Page
I. INTRODUCTION ..... I
II. EXISTING CONDITIONS ..... 4
II.A. Surrounding Land Use ..... 4
II.B. School Traffic Circulation ..... 4
II.C. Roadway Network ..... 4
II.D. Traffic Volumes ..... 6
II.E. Traffic Operations ..... 6
III. FUTURE CONDITIONS ..... 8
III.A. Redistributed Traffic ..... 8
III.B. Future Traffic Conditions ..... 8
III.C. Traffic Control Needs ..... 9
III.D. Access Design Considerations ..... 14
III.E. Pedestrian and Bicyclist Safety ..... 14
IV. SUMMARY AND RECOMMENDATIONS ..... 16

## Appendices

Appendix A. Existing Traffic Counts
Appendix B. Existing Level of Service Worksheets
Appendix C. Short-term Future Level of Service Worksheets
Appendix D. Long-term Future Level of Service Worksheets

## List of Figures

Page
Figure I. Vicinity Map ..... 2
Figure 2. Site Plan ..... 3
Figure 3. LPMS Parking Lots and Lot Access Locations ..... 5
Figure 4. Existing (2022) Traffic Conditions ..... 7
Figure 5. Existing (2022) Redistributed Traffic Conditions ..... II
Figure 6. Short-term Future Traffic Conditions ..... 12
Figure 7. Long-term Future Traffic Conditions ..... 13
Figure 8. AutoTurn Bus Results ..... 15
List of Tables
Page
Table I. Proposed Reconfigurations to Parking and Access ..... 8
Table 2. Current Traffic Control ..... 9
Table 3. Slowing Distance - Woodmoor Drive/New Bus Access ..... 14

## I. INTRODUCTION

The Lewis-Palmer School District \#38 is proposing to reconfigure an existing parking lot at the Lewis Palmer Middle School (LPMS) in unincorporated El Paso County, Colorado to only serve buses during school peak hours and implement a one-way inbound access to this lot for buses. The new access would be located north of the existing parking lot access on Woodmoor Drive. Woodmoor Drive is an important transportation connector located in Monument, Colorado. The collector roadway serves multiple residential developments and provides access to highway (HWY) 105 and Interstate (I-) 25. The roadway network adjacent to the site can be seen on Figure I.

The proposed parking lot/access reconfiguration is intended to improve school traffic circulation and provide exclusive bus access and parking. Figure 2 shows the layout of the proposed bus access. This access will serve as an inbound only bus access during school peak hours and provide parking for buses. During off peak hours, parents and visitors will be allowed to use this lot for parking purposes outside of normal school houff

FHU has completed an assessment of current and future (with reconfiguration) transportation conditions along Woodmoor Drive and surrounding the Lewis Palmer Middle School. This assessment provides considerations for future improvements and determines safety and efficiency needs for the proposed new access while serving the needs of multiple user types. The parameters of this analysis have been coordinated with El Paso County Staff. Based on staff input, this report includes information on existing traffic conditions, redistributed traffic with implementation of the bus only parking lot, total traffic volume projections, sight distance needs, multimodal circulation needs, and any recommended roadway improvements.

The following two future scenarios have been anklyzed for this report:

- Short-term Future - Time period for the completion of the bus only access, currently anticipated as the Year 2023.
- Long-term Future - The Long-term Future scenario reflects projected Year 2045 traffic conditions.

Please indicate whether parents/visitors/students etc. will be allowed to use the inbound only entrance during non peak hour


NORTH
FIGURE I


## II. EXISTING CONDITIONS

## II.A. Surrounding Land Use

Much of the area adjacent to the project site has been developed. The land uses surrounding the site are primarily residential with some commercial development north of the Middle School.

## II.B. School Traffic Circulation

Currently, there are four parking lots which serve the school, one parking lot north of the school near the track field, two rows of parking along the one-way drive adjacent to the school, and one parking lot south of the school. Each of these lots and their accesses serve multiple users and vehicle types. Figure 3 shows the current and proposed lots and access locations and current usage is described as follows:

- Lot A: provides 39 spaces, including accessible parking. Provides student drop-off and pickup from the north along Woodmoor Drive. It is accessed via full movement Access 3.
- Lot B: provides I5 spaces. Accessed via full movement Access I to Woodmoor Drive.
- Lot C: provides 38 parking spaces, including accessible parking. Parking aisle is one-way southbound during peak periods. During AM school peak, aisle serves bus only traffic. During PM school peak, aisle sequentially serves bus traffic then student pickup. Lot provides general parking during off peak hours.
- Lot D: provides approximately 35 unmarked spaces for faculty and staff. Student drop-off provided from the south via Woodmoor Drive adjacent the Willow Park Way curb line.


## II.C. Roadway Network

The existing roadway network adjacent to the vicinity of the site includes Woodmoor Drive and Willow Park Way. The roadway network is as follows:

Woodmoor Drive: Woodmoor Drive is a three-lane collector in the project area serving mainly residential developments and provides connectivity to HWY 105 and I-25. The posted speed limit is 30 miles per hour ( mph ); however, during school peaks the speed limit is reduced to 20 mph 620 feet north of the parking lot access to 250 feet south of Willow Park Way. Given this study is primarily focused on school hours, this roadway was analyzed with a posted speed of 20 mph .

Willow Park Way: Willow Park Way is a two-lane minor local street which provides access to the Lewis Palmer Middle School as well as a few other commercial developments. There is no posted speed limit, but, for the purpose of this study, the speed limit was assumed to be 20 mph .


## II.D. Traffic Volumes

Weekday AM and PM school peak hour turning movement counts (TMCs) were collected on
Wednesday, December 14, 2022, at the following intersections:

- Woodmoor Drive \& Existing Parking Lot Access
- Woodmoor Drive \& Main School Access
- Woodmoor Drive \& Willow Park Way
- Willow Park Way \& Secondary School Access

The peak hour traffic counts were collected in 15-minute intervals between 6:30 and 8:30 AM and 2:00 to 4:00 PM. The AM peak hour was found to be 7:00 to 8:00 AM, and the PM peak was 2:15 to 3:15 PM. Appendix A contains the TMCs. Peak hour traffic volumes are shown on Figure 4.

Based on the counts collected, Peak Hour Factors (PHFs) were found to range from 0.33 to 0.92 . The majority of heavy vehicle percentages were found to be under 8 percent; however, the southbound volume at the secondary school access as well as the eastbound movements at the intersection of Woodmoor Drive with Willow Park Way were found to be a bit higher, likely reflecting buses leaving the school. In order to reflect school peaking conditions, existing PHFs and heavy vehicle percentages were applied for both existing and future conditions by approach at each study intersection.

## II.E. Traffic Operations

Existing operational conditions were analyzed at each study intersection. The analysis is based on procedures documented in the Highway Capacity Manual (6th Edition). This analysis procedure provides a Level of Service (LOS), a qualitative measure of traffic operational conditions, based on intersection capacity and vehicle delay. LOS is described by a letter designation ranging from A to F, LOS A represents almost free-flow conditions, while LOS F represents congested conditions. LOS is calculated for movements which must yield right-of-way for unsignalized intersections.

Figure 4 shows the results of the existing conditions analysis. Appendix B contains LOS worksheets. As shown, all stop-controlled movements operate at LOS D or better with the exception of the eastbound and westbound movements at Willow Park Way with oodmoor Drive during the AM peak hour which currently operate at LOS F.

The parking lot access intersection indicates a LOS of E on the westbound thru/left turn. Revise accordingly.


FIGURE 4

## III. FUTURE CONDITIONS

## III.A. Redistributed Traffic

The new bus access would restrict the north parking lot to bus only during school peak hours, and all buses are anticipated to use the reconfigured lot to the north, which would have one inbound only access (proposed) and the second access would be outbound only for buses. Further, new circulation patterns for buses will cause all buses to arrive and depart from the school traveling southbound, meaning no school buses are anticipated to make a northbound left turn into this lot. Table I outlines the current and proposed users and accesses for each lot.

Table I. Proposed Reconfigurations to Parking and Access

| Parking Lot | Current |  |  | Future w/ Reconfiguration |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Uses | Entrance Access | Exit Access | Use | Entrance Access | Exit <br> Access |
| A | Student pickup/drop-off | 3 | 3 | Bus only during school peaks | 3 a | 3 |
|  | Visitors | 3 | 3 | Visitors at other times | 3 | 3 |
| B | Faculty and Staff | I | 1 | Faculty and Staff | 1 | 1 |
| C | Bus Only During School Peaks | 1 | 2 | Student pickup/drop-off | 1 | 2 |
|  | Student Pickup/Drop-off | 1 | 2 | Visitors | I | 2 |
|  | Visitors | I | 2 |  |  |  |
| D | Faculty and Staff | 4, 2 | 4, 2 | Faculty and Staff | 4, 2 | 4, 2 |

## III.B. Future Traffic Conditions

Because the implementation of the new parking lot access will restrict the parking lot to allow only buses during the peak hours, peak hour volumes had to be redistributed so all buses use the parking lot and all passenger cars and heavy vehicles only use the main and secondary school accesses. Figure 5 shows the redistributed existing volumes and subsequent traffic operations.

Future traffic was estimated for the short-term (2023) and long-term (2045) timeframes and accounts for existing traffic already using the transportation system, plus a general upward factoring of current traffic levels to capture the effects of anticipated future growth in the area. Because the Middle School is not anticipated to increase attendance in the future and the surrounding area is mostly developed, only the northbound and southbound through traffic along Woodmoor Drive was increased to account for growth. The Pikes Peak Area Council of Governments (PPACG) regional model was used to determine an annual growth rate of $0.6 \%$ per year along Woodmoor Drive.

## Short-term Future Traffic Operations

Year 2023 traffic projections were developed assuming 0.6 percent growth per year for one year, this results in very minimal growth along Woodmoor Drive. It is important to note that the growth factor was applied to only the northbound and southbound through movements on Woodmoor Drive.

Figure 6 depicts short-term future AM and PM peak hour turning movement projections for the study area intersections and Appendix C contains the LOS worksheets. Using the existing PHFs and heavy vehicle percentages outlined in Section II.D, all unsignalized movements are anticipated to remain at acceptable operations with the exception of the eastbound and westbound movements at Woodmoor

Drive with Willow Park Way in the AM peak hour. The westbound movement is not anticipated to queue longer than 50 feet and the volume to capacity ratio (v/c) is well below I. However, the eastbound movement is expected to experience a queue length of 375 feet, and the $v / \mathrm{c}$ is 1.05 . This movement experience a $\mathrm{v} / \mathrm{c}$ of just under I in existing conditions and the current queue length is 300 feet; therefore, the redistribution of traffic is not anticipated to drastically reduce these operations.

## Long-term Future Traffic Operations

Figure 7 shows the long-term peak hour turning movement projections for the study area intersections and Appendix D contains the LOS worksheets. It is important to note that the growth factor was only applied to northbound and southbound through movements on Woodmoor Drive.

Using the existing PHFs and heavy vehicle percentages outlined in Section II.D, all unsignalized movements are projected to remain acceptable with the exception of the eastbound and westbound through movements at the intersection of Woodmoor Drive with Willow Park Way in the AM peak hour. The westbound movement is not anticipated to queue longer than 50 feet and the volume to capacity ratio (v/c) is well below I . However, the eastbound movement is expected to experience a queue length of 475 feet, and the $\mathrm{v} / \mathrm{c}$ is I.2. This movement experiences a $\mathrm{v} / \mathrm{c}$ of just under I in existing conditions and the current queue length is 300 feet; therefore, the redistribution of traffic is not anticipated to drastically worsen these operations.

## III.C. Traffic Control Needs

Current traffic control at the study intersections is shown in Table 2.

## Table 2. Current Traffic Control

| Intersection | Traffic Control Type |
| :--- | :--- |
| Woodmoor Drive \& Parking Lot Access | Two-Way Stop Control (TWSC) (EB \& WB) |
| Woodmoor Drive \& Main School Access | TWSC (EB) |
| Woodmoor Drive \& Willow Park Way | TWSC (EB \& WB) |
| Willow Park Way \& Secondary School Access | TWSC (NB \& SB) |

As shown, all of the study intersections are currently unsignalized. The Manual on Uniform Traffic Control Devices (MUTCD, 2009 Edition) outlines 9 warrants that may be used to justify installing a traffic signal at an intersection. The warrants are listed as follows:
I. Eight-Hour Vehicular Volume
2. Four-Hour Vehicular Volume
3. Peak Hour
4. Pedestrian Volume
5. School Crossing
6. Coordinated Signal System
7. Crash Experience
8. Roadway Network
9. Intersection Near a Grade Crossing

Of these nine, warrants I, 2 , and 5 are applicable to conditions at the study intersections. Given the failing LOS on the eastbound and westbound approaches, the intersection of Woodmoor Drive with Willow Park Way is the only intersection that may need a signal in order to facilitate acceptable operations. Projected vehicular traffic volumes and pedestrian volumes were compared with warrant criteria to assess this potential. Because the intersection traffic counts only covered peak periods, a scaling factor was used to estimate the fourth and eighth highest hour volumes using information from
the Missouri Department of Transportation (MoDOT). Based on this information, it is estimated that the eighth highest hour comprises approximately 75 percent of the peak recorded hour. Each of the eight highest hours are estimated by scaling in linear fashion.

Utilizing the scaling assumptions, the evaluation of traffic-volume based Warrant I (eight-hour volume) and 2 (Four-Hour Volume) indicates that traffic volumes do not meet any of the specified conditions. A review of pedestrian volumes at the study intersections revealed a maximum of only 4 pedestrians crossing the intersection of Woodmoor Drive with Willow Park Way during the peak hour. Warrant 5 of the MUTCD specifies a need for at least 20 pedestrians during the peak hour to satisfy this warrant. Therefore, none of the study intersections are anticipated to meet signal warrant criteria.



NORTH
FIGURE 6


NORTH
FIGURE 7

Please address the ECM 2.4 access criteria for the proposed access. If criteria such as access spacing is not met then please submit a deviation request for consideration by the ECM administrator.
staff recommends stating that volume thresholds for a right turn aux. lane are not met at this proposed access

## Considerations

be inbound only meaning sight distance for outbound movements will ited spacing between the proposed access and the existing handicap spaces along Woodmoor Drive result is a small auxiliary right turn lane of only 60 feet. The minimal storage space introduces a concern for vehicles coming around the Woodmoor Drife horizontal curve lacking adequate sight distance to see buses slowing at the proposed access. Calculations were completed to determine the distance needed for buses to slow to a reasonable turning speed in order to enter the parking lot. Table $\mathbf{3}$ outlines the calculation parameters and results. provide specs of turn lane to include

## Table 3. Slowing Distance - Woodmoor Drive/New Bus Acrecommended

| Movement | Planned <br> Storage <br> Length | Sosted <br> Speed |  |  |  |  | Turning Distance for School Buses <br> Speed |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Assumed <br> Deceleration | Slowing <br> Time | Slowing <br> Distance |  |  |  |  |
| Southbound Right | 60 ft | 20 mph | 9 mph | $-4.49 \mathrm{ft}^{2} / \mathrm{sec}^{2}$ | 3.59 sec | 52.5 ft |  |

As shown above, the planned 60 feet of storage is anticipated to be enough room for buses to slow from the posted speed of 20 mph during school hours to a safe right turning speed of 9 mph . Buses will need just over 50 feet of space to make this adjustment. Therefore, buses will not need to slow down until fully in the storage lane and should not create sight distance safety issues.

Figure 8 shows the turning template for a typical bus. The new bus access configuration is anticipated to acceptably accommodate the turning movements of a typical bus. It should be noted that due to size constraints, bus drivers will likely need to coordinate to ensure all parking lots are filled and emptied during arrival and departure. If not used properly, buses will block entrances and exits which could lead to blockages and queueing onto Woodmoor Drive.

## III.E. Pedestrian and Bicyclist Safety

Pedestrian and bicyclist counts were taken at each intersection within the study area. Pedestrian and bicyclist volumes were generally low with no more than 4 crossings at any location during the peak hours. Currently, there are sidewalks along the school property; however, there are no sidewalks along Woodmoor Drive or controlled crossings. Due to the low volume of pedestrians and cyclists in this area, the lack of sidewalks, crosswalks, and bicycle lanes may not be an issue. However, to accommodate transportation safety for students, it is essential to have adequate sidewalks to the school access. It is recommended that a six-foot sidewalk be constructed on the west side of Woodmoor Drive from Willow Park Way to the new bus access.

Please discuss the turn lanes at the main entrance. Are the existing right turn lane and two-way left turn lane sufficient for the additional traffic being added at this access? are any modifications/improvements needed? do they currently meet ECM criteria? please address.

Also discuss/analyze queuing length for school drop off and loading zones. Refer to MSTA guidelines. see link below:https://connect.ncdot.gov/municipalities/Sch ool/pages/default.aspx

## Lewis Palmer Middle School

PARKING LOT A - AUTOTURN


## IV. SUMMARY AND RECOMMENDATIONS

A new inbound only bus access is proposed for the Lewis Palmer Middle School parking lot to convert to bus only during school peaks. The proposed bus access would be constructed north of the existing parking lot access along Woodmoor Drive in Monument, Colorado. Surrounding areas are primarily residential with some commercial space north of the school.

Two future scenarios were analyzed for this report:

- Short-term Future - Time period for the completion of the new access, estimated as the Year 2023.
- Long-term Future - The year 2045 was used to assess traffic impacts of the development in the long-term future.

The following is a summary of the findings and recommendations related to the analysis for the development:

- No geometric or traffic control improvements are anticipated for the existing intersections in the study area based on the redevelopment.
- The planned 60 feet of storage for the southbound right turn lane at the new bus access is anticipated to provide enough space for buses to slow from the posted speed of 20 mph to a safe turning speed of 9 mph without causing sight distance issues for vehicles traveling around the Woodmoor Drive horizontal curve.
- Monitoring of both bus and parent traffic within the study area should occur upon implementation of these changes to determine if adjustments should be made if problems emerge.
- Access 3 should be monitored to ensure parents and visitors are aware of the new access restriction and do not use this lot during peak hours.
- Access 3A should be monitored to ensure efficiency entering the parking lot. Woodmoor Drive should not experience blockages due to the new access.
- Access 2 should be monitored to ensure drivers unfamiliar with this access treat it as outbound only.
- Access I and parking area C should be monitored to ensure that additional student drop-off and pickup activities do not cause concern.
- Lewis Palmer Middle School facilitates the need for pedestrian and bicyclist improvements in the area. A six-foot sidewalk will need to be constructed on the west side of Woodmoor Drive between the new bus access and Willow Park Way.


## APPENDIX A. EXISTING TRAFFIC COUNTS

(303) 216-2439 www.alltrafficdata.net

Location: 1 WOODMOOR DR \& PARKING LOT ACCESS AM
Date: Wednesday, December 14, 2022
Peak Hour: 07:00 AM - 08:00 AM
Peak 15-Minutes: 07:15 AM - 07:30 AM


Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

| Interval <br> Start Time | PARKING LOT ACCESS Eastbound |  |  |  | PARKING LOT ACCESS Westbound |  |  |  | WOODMOOR DR Northbound |  |  |  |  | WOODMOOR DR <br> Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru | Right |  |  | West | East | South |  |
| 6:30 AM | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 |  | 0 | 0 | 4 | 1 | 0 | 0 |  | 21 | 0 | 28 | 559 | 0 | 0 | 0 | 0 |
| 6:45 AM | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |  | 0 | 7 | 14 | 0 | 0 | 0 |  | 34 | 3 | 61 | 612 | 0 | 0 | 0 | 0 |
| 7:00 AM | 0 | 9 | 2 | 63 | 0 | 2 | 0 | 0 |  | 0 | 45 | 16 | 0 | 0 | 0 |  | 34 | 41 | 212 | 652 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 6 | 3 | 93 | 0 | 0 | 1 | 2 |  | 0 | 36 | 30 | 1 | 0 | 0 |  | 46 | 40 | 258 | 540 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 |  | 0 | 0 | 24 | 0 | 0 | 0 |  | 56 | 0 | 81 | 363 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |  | 0 | 0 | 36 | 4 | 0 | 0 |  | 60 | 0 | 101 |  | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 58 | 0 | 0 | 0 |  | 40 | 0 | 100 |  | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 |  | 0 | 0 | 34 | 0 | 0 | 0 |  | 46 | 0 | 81 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 16 | 7 | 157 | 0 | 7 | 1 | 3 |  | 0 | 88 | 216 | 6 | 0 | 0 |  | 337 | 84 | 922 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 15 | 5 | 156 | 0 | 3 | 1 | 3 |  | 0 | 81 | 106 | 5 | 0 |  | 0 | 196 | 81 | 65 | 52 | 0 | 0 | 0 | 0 |



Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

| Interval | MAIN | Eastb | L ACC | CESS | West | und | WOODMOOR DR Northbound |  |  |  |  | WOODMOOR DR Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn Left | Thru Right | U-Turn | Left | Thru R | Right |  | J-Turn | Left |  | Thru | Right |  |  | West | East | South |  |
| 6:30 AM | 0 | 1 | 0 | 0 |  |  | 0 | 5 | 5 | 0 |  | 0 | 0 |  | 22 | 1 | 34 | 525 | 0 |  | 0 | 0 |
| 6:45 AM | 0 | 0 | 0 | 0 |  |  | 0 | 17 | 21 | 0 |  | 0 | 0 |  | 28 | 8 | 74 | 579 | 0 |  | 0 | 0 |
| 7:00 AM | 0 | 0 | 0 | 0 |  |  | 0 | 38 | 67 | 0 |  | 1 | 0 |  | 88 | 8 | 202 | 613 | 0 |  | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 |  |  | 0 | 11 | 60 | 0 |  | 0 | 0 |  | 141 | 3 | 215 | 515 | 0 |  | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 |  |  | 0 | 8 | 25 | 0 |  | 0 | 0 |  | 52 | 3 | 88 | 388 | 0 |  | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 |  |  | 0 | 5 | 40 | 0 |  | 0 | 0 |  | 60 | 3 | 108 |  | 0 |  | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 |  |  | 0 | 4 | 59 | 0 |  | 0 | 0 |  | 41 | 0 | 104 |  | 0 |  | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 |  |  | 0 | 6 | 33 | 0 |  | 0 | 0 |  | 49 | 0 | 88 |  | 0 |  | 0 | 0 |
| Count Total | 0 | 1 | 0 | 0 |  |  | 0 | 94 | 310 | 0 | 0 | 1 | 0 | 0 | 481 | 26 | 913 |  | 0 |  | 0 | 0 |
| Peak Hour | 0 | 0 | 0 | 0 |  |  | 0 | 62 | 192 | 0 | 0 | 1 |  | 0 | 341 | 17 | 613 | 3 | 0 |  | 0 | 0 |

aLL TRAFFIC DATA SERVICES
(303) 216-2439 www.alltrafficdata.net


Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.

## Traffic Counts - Motorized Vehicles

| Interval | WILLOW PARK WAY <br> Eastbound |  |  |  | WILLOW PARK WAY <br> Westbound |  |  |  |  | WOODMOOR DR Northbound |  |  |  | WOODMOOR DR Southbound |  |  |  |  | Total |  | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru R | Right |  | U-Turn | Left | Thru | Right |  | J-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 6:30 AM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 1 | 0 | 10 | 9 | 0 |  | 0 | 0 | 23 | 0 |  | 45 |  | 794 | 0 | 0 | 0 | 0 |
| 6:45 AM | 0 | 2 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 26 | 37 | 1 |  | 0 | 0 | 27 | 0 |  | 103 | 853 | 0 | 0 | 0 | 0 |
| 7:00 AM | 0 | 0 | 0 | 90 | 0 | 1 | 0 | 1 | 1 | 0 | 62 | 108 | 0 |  | 0 | 0 | 75 | 1 |  | 338 | 865 | 0 | 0 | 0 | 4 |
| 7:15 AM | 0 | 2 | 0 | 60 | 0 | 1 | 0 | 1 | 1 | 0 | 36 | 61 | 0 |  | 0 | 1 | 145 | 1 |  | 308 | 642 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 5 | 0 | 15 | 0 | 1 | 0 | 2 | 2 | 0 | 3 | 26 | 0 |  | 0 | 1 | 50 | 1 |  | 104 | 434 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 2 | 2 | 0 | 6 | 38 | 1 |  | 0 | 1 | 60 | 0 |  | 115 |  | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 7 | 0 | 0 | 0 |  | 1 | 0 | 3 | 64 | 0 |  | 0 | 0 | 39 | 1 |  | 115 |  | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 4 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 1 | 4 | 36 | 0 |  | 0 | 0 | 47 | 0 |  | 100 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 15 | 0 | 197 | 0 | 3 | 0 | 8 | 8 | 1 | 150 | 379 | 2 | 2 | 0 | 3 | 466 |  | 4 | 1,228 |  | 0 | 0 | 0 | 4 |
| Peak Hour | 0 | 9 | 0 | 170 | 0 | 3 | 0 | 6 | 6 | 0 | 107 | 233 | 1 | 1 | 0 | 3 | 330 |  | 3 | 86 | 5 | 0 | 0 | 0 | 4 |

aLL TRAFFIC DATA SERVICES
(303) 216-2439 www.alltrafficdata.net


Note: Total study counts contained in parentheses.

## Traffic Counts - Motorized Vehicles

| Interval Start Time | WILLOW PARK WAY Eastbound |  |  |  | WILLOW PARK WAY <br> Westbound |  |  |  | SECONDARY SCHOOL ACCESS Northbound |  |  |  | SECONDARY SCHOOL ACCESS Southbound |  |  |  |  |  | Rolling | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | eft | Thru R |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  | Total | Hour | West | East | uth |  |
| 6:30 AM | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 300 | 0 | 0 | 0 | 0 |
| 6:45 AM | 0 | 0 | 5 | 0 | 0 | 1 | 12 | 13 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 38 | 310 | 0 | 0 | 0 | 0 |
| 7:00 AM | 0 | 0 | 52 | 0 | 0 | 0 | 62 | 1 | 0 | 0 | 0 | 0 | 0 | 36 | 0 |  | 0 | 151 | 285 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 42 | 0 | 0 | 0 | 38 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 0 | 0 | 0 | 99 | 145 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 0 | 0 | 0 | 22 | 63 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 2 | 0 | 0 | 1 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 13 |  | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 1 | 0 | 0 | 1 | 3 | 0 | 0 | 0 | 0 | 1 | 0 | 5 | 0 | 0 | 0 | 11 |  | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 4 | 0 | 1 | 2 | 2 | 0 | 0 | 0 | 0 | 3 | 0 | 5 | 0 | 0 | 0 | 17 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 0 | 112 | 0 | 1 | 5 | 129 | 21 | 0 | 0 | 0 | 4 | 0 | 91 | 0 |  | 0 | 363 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 0 | 103 | 0 | 0 | 1 | 116 | 14 | 0 | 0 | 0 | 0 | 0 | 76 | 0 | 0 | 0 | 31 | 10 | 0 | 0 | 0 | 0 |

(303) 216-2439 www.alltrafficdata.net

Location: 1 WOODMOOR DR \& PARKING LOT ACCESS PM
Date: Wednesday, December 14, 2022
Peak Hour: 02:30 PM - 03:30 PM
Peak 15-Minutes: 02:30 PM - 02:45 PM

Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

| Interval Start Time | PARKING LOT ACCESS Eastbound |  |  |  | PARKING LOT ACCESS Westbound |  |  |  |  | WOODMOOR DR Northbound |  |  |  |  | WOODMOOR DR Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru R | Right |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 2:00 PM | 0 | 0 | 1 | 0 | 0 | 1 |  | 0 | 1 |  | 0 | 14 | 29 | 2 | 0 | 0 | 29 | 3 | 80 | 485 | 0 | 0 | 0 | 0 |
| 2:15 PM | 0 | 0 | 0 | 4 | 0 | 1 |  | 0 | 0 |  | 2 | 3 | 39 | 3 | 0 | 1 | 28 | 0 | 81 | 491 | 0 | 0 | 0 | 0 |
| 2:30 PM | 0 | 2 | 0 | 45 | 0 | 2 |  | 0 | 4 |  | 2 | 19 | 42 | 3 | 4 | 1 | 32 | 23 | 179 | 499 | 4 | 0 | 4 | 0 |
| 2:45 PM | 0 | 9 | 0 | 25 | 0 | 1 |  | 0 | 0 |  | 1 | 7 | 48 | 1 | 0 | 0 | 49 | 4 | 145 | 400 | 2 | 0 | 2 | 0 |
| 3:00 PM | 0 | 3 | 0 | 5 | 0 | 1 |  | 0 | 0 |  | 0 | 2 | 41 | 1 | 0 | 0 | 32 | 1 | 86 | 340 | 0 | 0 | 0 | 0 |
| 3:15 PM | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 51 | 1 | 0 | 0 | 37 | 0 | 89 |  | 0 | 0 | 0 | 0 |
| 3:30 PM | 0 | 1 | 1 | 0 | 0 | 2 |  | 0 | 0 |  | 0 | 1 | 37 | 2 | 0 | 0 | 36 | 0 | 80 |  | 0 | 0 | 0 | 0 |
| 3:45 PM | 0 | 0 | 2 | 1 | 0 | 1 |  | 0 | 0 |  | 0 | 3 | 41 | 0 | 0 | 0 | 37 | 0 | 85 |  | 0 | 0 | 1 | 0 |
| Count Total | 0 | 15 | 4 | 80 | 0 | 9 |  | 0 | 5 |  | 5 | 49 | 328 | 13 | 4 | 2 | 280 | 31 | 825 |  | 6 | 0 | 7 | 0 |
| Peak Hour | 0 | 14 | 0 | 75 | 0 | 4 |  | 0 | 4 |  | 3 | 28 | 182 | 6 | 4 | 1 | 150 | 28 | 499 | 9 | 6 | 0 | 6 | 0 |

(303) 216-2439
www.alltrafficdata.net
Location: 2 WOODMOOR DR \& MAIN SCHOOL ACCESS PM
Date: Wednesday, December 14, 2022
Peak Hour: 02:00 PM - 03:00 PM
Peak 15-Minutes: 02:45 PM - 03:00 PM


Note: Total study counts contained in parentheses.

| Interval | MAIN SCHOOL ACCESS Eastbound |  |  |  | Westbound |  | WOODMOOR DR Northbound |  |  |  | WOODMOOR DR Southbound |  |  |  |  | Total | Rolling Hour |  | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn Left | Thru Right | U-Turn | Left | Thru R | Right | U-Turn | Left |  | Thru | Right |  |  |  | West | East | South |  |
| 2:00 PM | 0 | 0 | 0 | 0 |  |  | 0 | 25 | 45 | 0 | 0 | 0 |  | 30 | 2 | 102 |  | 508 | 0 |  | 0 | 0 |
| 2:15 PM | 0 | 1 | 0 | 2 |  |  | 0 | 5 | 50 | 0 | 0 | 0 |  | 31 | 0 | 89 |  | 498 | 0 |  | 0 | 0 |
| 2:30 PM | 0 | 1 | 0 | 1 |  |  | 0 | 14 | 56 | 0 | 0 | 0 |  | 82 | 0 | 15 |  | 502 | 0 |  | 0 | 0 |
| 2:45 PM | 0 | 0 | 0 | 2 |  |  | 0 | 24 | 56 | 0 | 0 | 0 |  | 76 | 5 | 163 |  | 431 | 0 |  | 0 | 0 |
| 3:00 PM | 0 | 2 | 0 | 6 |  |  | 0 | 4 | 41 | 0 | 0 | 0 |  | 39 | 0 | 92 |  | 360 | 0 |  | 0 | 0 |
| 3:15 PM | 0 | 0 | 0 | 0 |  |  | 0 | 5 | 51 | 0 | 0 | 0 |  | 37 | 0 | 93 |  |  | 0 |  | 0 | 0 |
| 3:30 PM | 0 | 0 | 0 | 0 |  |  | 0 | 1 | 42 | 0 | 0 | 0 |  | 40 | 0 | 83 |  |  | 0 |  | 0 | 0 |
| 3:45 PM | 0 | 0 | 0 | 0 |  |  | 0 | 4 | 47 | 0 | 0 | 0 |  | 41 | 0 | 92 |  |  | 0 |  | 0 | 0 |
| Count Total | 0 | 4 | 0 | 11 |  |  | 0 | 82 | 388 | 0 | 0 | 0 |  | 376 | 7 | 86 |  |  | 0 |  | 0 | 0 |
| Peak Hour | 0 | 2 | 0 | 5 |  |  | 0 | 68 | 207 | 0 | 0 | 0 | 0 | 219 |  | 7 | 508 |  | 0 |  | 0 | 0 |

aLL TRAFFIC DATA SERVICES
(303) 216-2439 www.alltrafficdata.net

Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.

## Traffic Counts - Motorized Vehicles

| Interval | WILLOW PARK WAY <br> Eastbound |  |  |  | WILLOW PARK WAY <br> Westbound |  |  |  |  | WOODMOOR DR Northbound |  |  |  | WOODMOOR DR Southbound |  |  |  |  | Total |  | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  | U-Turn | Left | Thru | Right |  |  | Left | Thru | Right |  |  | West | East | South | orth |
| 2:00 PM | 0 | 1 | 0 | 11 | 0 | 1 | 0 |  | 0 | 0 | 10 | 53 | 0 |  | 0 | 0 | 30 | 1 |  | 107 |  | 716 | 0 | 0 | 0 | 0 |
| 2:15 PM | 0 | 2 | 0 | 6 | 0 | 0 | 0 |  | 0 | 0 | 19 | 76 | 0 |  | 1 | 0 | 30 | 0 |  | 134 | 723 | 0 | 0 | 0 | 0 |
| 2:30 PM | 0 | 0 | 0 | 64 | 0 | 2 | 0 |  | 1 | 0 | 23 | 64 | 5 |  | 0 | 1 | 70 | 1 |  | 231 | 701 | 0 | 0 | 0 | 12 |
| 2:45 PM | 0 | 5 | 0 | 70 | 0 | 1 | 0 |  | 3 | 0 | 7 | 72 | 1 |  | 0 | 1 | 80 | 4 |  | 244 | 591 | 0 | 0 | 0 | 0 |
| 3:00 PM | 0 | 1 | 0 | 17 | 0 | 0 | 0 |  | 0 | 0 | 9 | 41 | 1 |  | 0 | 1 | 43 | 1 |  | 114 | 449 | 0 | 0 | 0 | 0 |
| 3:15 PM | 0 | 5 | 0 | 14 | 0 | 0 | 0 |  | 1 | 0 | 4 | 52 | 0 |  | 0 | 0 | 36 | 0 |  | 112 |  | 0 | 0 | 0 | 0 |
| 3:30 PM | 0 | 1 | 0 | 23 | 0 | 1 | 0 |  | 2 | 0 | 11 | 41 | 0 |  | 0 | 1 | 39 | 2 |  | 121 |  | 0 | 0 | 0 | 0 |
| 3:45 PM | 0 | 3 | 0 | 4 | 0 | 1 | 0 |  | 0 | 0 | 3 | 49 | 0 |  | 0 | 1 | 41 | 0 |  | 102 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 18 | 0 | 209 | 0 | 6 | 0 | 0 | 7 | 0 | 86 | 448 | 7 | 7 | 1 | 5 | 369 | 9 | 9 | 1,165 |  | 0 | 0 | 0 | 12 |
| Peak Hour | 0 | 8 | 0 | 157 | 0 | 3 | 0 | - | 4 | 0 | 58 | 253 | 7 | 7 | 1 | 3 | 223 |  | 6 | 72 | 23 | 0 | 0 | 0 | 12 |

(303) 216-2439 www.alltrafficdata.net

Location: 4 SECONDARY SCHOOL ACCESS \& WILLOW PARK WAY PM
Date: Wednesday, December 14, 2022
Peak Hour: 02:15 PM - 03:15 PM
Peak 15-Minutes: 02:30 PM - 02:45 PM


Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.

## Traffic Counts - Motorized Vehicles

| Interval <br> Start Time | WILLOW PARK WAY <br> Eastbound |  |  |  | WILLOW PARK WAY <br> Westbound |  |  |  | SECONDARY SCHOOL ACCESS$\qquad$ Northbound |  |  |  | SECONDARY SCHOOL ACCESS Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru R | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  |  | West | East | South |  |
| 2:00 PM | 0 | 0 | 6 | 0 | 0 | 0 | 10 | 1 | 0 | 0 | 0 | 0 | 0 | 6 | 0 |  | 0 | 23 | 228 | 0 | 0 | 0 | 0 |
| 2:15 PM | 0 | 0 | 0 | 0 | 0 | 1 | 20 | 0 | 0 | 0 | 0 | 3 | 0 | 4 | 0 |  | 0 | 28 | 233 | 1 | 0 | 0 | 0 |
| 2:30 PM | 0 | 0 | 44 | 1 | 0 | 2 | 21 | 0 | 0 | 0 | 0 | 3 | 0 | 21 | 0 |  | 0 | 92 | 228 | 38 | 0 | 0 | 1 |
| 2:45 PM | 0 | 0 | 14 | 0 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 11 | 0 | 47 | 0 |  | 2 | 85 | 173 | 1 | 0 | 0 | 0 |
| 3:00 PM | 0 | 0 | 1 | 0 | 0 | 3 | 5 | 2 | 20 | 0 | 0 | 4 | 0 | 13 | 0 |  | 0 | 28 | 97 | 2 | 0 | 0 | 0 |
| 3:15 PM | 0 | 0 | 6 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 4 | 0 | 9 | 0 |  | 0 | 23 |  | 0 | 0 | 0 | 0 |
| 3:30 PM | 0 | 0 | 7 | 0 | 0 | 5 | 6 | 2 | 0 | 0 | 0 | 2 | 0 | 15 | 0 |  | 0 | 37 |  | 0 | 0 | 0 | 0 |
| 3:45 PM | 0 | 0 | 4 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 |  | 0 | 9 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 0 | 82 | 1 | 0 | 11 | 80 | 5 | 50 | 0 | 0 | 27 | 0 | 117 | 0 |  | 2 | 325 |  | 42 | 0 | 0 | 1 |
| Peak Hour | 0 | 0 | 59 | 1 | 0 | 6 | 57 | 2 | 20 | 0 | 0 | 21 | 0 | 85 | 0 | 0 | 2 | 233 |  | 42 | 0 | 0 | 1 |

## APPENDIX B. EXISTING LEVEL OF SERVICE WORKSHEETS






| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 18.9 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | $\dagger$ |  |  | $\leqslant$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  |  |
| Traffic Vol, veh/h | 9 | 0 | 170 | 3 | 0 | 6 | 107 | 233 | 1 | 3 | 330 | 3 |  |
| Future Vol, veh/h | 9 | 0 | 170 | 3 | 0 | 6 | 107 | 233 | 1 | 3 | 330 | 3 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control Star | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | 100 | - | - | 100 | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 47 | 47 | 47 | 75 | 75 | 75 | 50 | 50 | 50 | 57 | 57 | 57 |  |
| Heavy Vehicles, \% | 13 | 13 | 13 | 0 | 0 | 0 | 6 | 6 | 6 | 0 | 0 | 0 |  |
| Mvmt Flow | 19 | 0 | 362 | 4 | 0 | 8 | 214 | 466 | 2 | 5 | 579 | 5 |  |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 3.9 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\stackrel{+}{*}$ |  |  | \& |  |  | * |  |  | 4 |  |
| Traffic Vol, veh/h | 0 | 100 | 0 | 1 | 109 | 1 | 0 | 0 | 0 | 74 | 0 | 0 |
| Future Vol, veh/h | 0 | 100 | 0 | 1 | 109 | 1 | 0 | 0 | 0 | 74 | 0 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 48 | 48 | 48 | 44 | 44 | 44 | 92 | 92 | 92 | 51 | 51 | 51 |
| Heavy Vehicles, \% | 1 | 1 | 1 | 1 | 1 | 1 | 2 | 2 | 2 | 28 | 28 | 28 |
| Mvmt Flow | 0 | 208 | 0 | 2 | 248 | 2 | 0 | 0 | 0 | 145 | 0 | 0 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.7 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBU | NBL | NBT | NBR | SBU | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | $\uparrow$ | F |  | ${ }^{7}$ | $\uparrow$ | F |  | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Vol, veh/h | 14 | 0 | 79 | 5 | 0 | 4 | 5 | 31 | 170 | 8 | 4 | 2 | 141 | 28 |
| Future Vol, veh/h | 14 | 0 | 79 | 5 | 0 | 4 | 5 | 31 | 170 | 8 | 4 | 2 | 141 | 28 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control S | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | - | None | - | - | - | None |
| Storage Length | - | - | - | - | - | 0 | - | 100 | - | 100 | - | 100 | - | 125 |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | - | 0 | - | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | - | 0 | - | - | - | 0 | - |
| Peak Hour Factor | 43 | 43 | 43 | 46 | 46 | 46 | 92 | 81 | 81 | 81 | 92 | 65 | 65 | 65 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 3 | 3 | 3 | 2 | 0 | 0 | 0 |
| Mumt Flow | 33 | 0 | 184 | 11 | 0 | 9 | 5 | 38 | 210 | 10 | 4 | 3 | 217 | 43 |



| Approach | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| HCM Control Delay, s | 11.8 | 13.6 |  |  |
| HCM LOS | B | B |  |  |


| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1WBLn1WBLn2 | SBL | SBT | SBR |  |  |
| :--- | :---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- |
| Capacity (veh/h) | + | - | - | 741 | 312 | 835 | $\sim$ | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |




| Major/Minor | Minor2 | Minor1 |  |  |  | Major1 |  |  | Major2 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 818 | 819 | 328 | 955 | 819 | 329 | 332 | 0 | 0 | - | 333 | 0 | 0 |
| Stage 1 | 336 | 338 | - | 477 | 477 | - | - | - | - | - | - | - | - |
| Stage 2 | 482 | 481 |  | 478 | 342 | - |  | - | - | - | - | - | - |
| Critical Hdwy | 7.23 | 6.63 | 6.33 | 7.1 | 6.5 | 6.2 | 4.19 | - | - | - | 4.1 | - | - |
| Critical Hdwy Stg 1 | 6.23 | 5.63 | - | 6.1 | 5.5 | - | - | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | 6.23 | 5.63 | - | 6.1 | 5.5 | - | - | - | - | - | - | - | - |
| Follow-up Hdwy | 3.617 | 4.117 | 3.417 | 3.5 | 4 | 3.3 | 2.281 | - | - | - | 2.2 | - | - |
| Pot Cap-1 Maneuver | 282 | 298 | 689 | 240 | 312 | 717 | 1189 | - | - | - | 1238 | - | - |
| Stage 1 | 656 | 621 | - | 573 | 559 | - | - | - | - | - - | - | - | - |
| Stage 2 | 545 | 536 | - | 572 | 642 | - | - | - | - | - - | - | - | - |
| Platoon blocked, \% |  |  |  |  |  |  |  | - | - |  |  | - | - |
| Mov Cap-1 Maneuver | 264 | 280 | 689 | 134 | 293 | 717 | 1189 | - |  | $\sim-5$ | $\sim-5$ | - | - |
| Mov Cap-2 Maneuver | 264 | 280 | - | 134 | 293 | - | - | - | - | - - | - | - | - |
| Stage 1 | 615 | 621 | - | 537 | 524 | - | - | - | - | - | - | - | - |
| Stage 2 | 503 | 503 | - | 335 | 642 | - | - | - | - | - - | - | - | - |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 15.5 |  |  | 20.6 |  |  | 1.5 |  |  |  |  |  |  |
| HCM LOS | C |  |  | C |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvm |  | NBL | NBT | NBR | EBLn1 | BLn1 | SBL | SBT | SBR |  |  |  |  |
| Capacity (veh/h) |  | 1189 | - | - | 639 | 250 | + | - | - |  |  |  |  |
| HCM Lane V/C Ratio |  | 0.063 | - | - | 0.469 | 0.08 | - | - | - |  |  |  |  |
| HCM Control Delay (s) |  | 8.2 | - | - | 15.5 | 20.6 | - | - | - |  |  |  |  |
| HCM Lane LOS |  | A | - | - | C | C | - | - | - |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | 0.2 | - |  | 2.5 | 0.3 | - | - | - |  |  |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ : Volume exceeds cap | apacity | \$: Dela | lay exc | eeds 3 | 00s | : Com | putatio | Not D | fined | *: All | major volur | e | atoon |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 6.3 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \& |  |  | * |  |  | 4 |  |
| Traffic Vol, veh/h | 0 | 59 | 1 | 6 | 57 | 2 | 0 | 0 | 21 | 85 | 0 | 2 |
| Future Vol, veh/h | 0 | 59 | 1 | 6 | 57 | 2 | 0 | 0 | 21 | 85 | 0 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 33 | 33 | 33 | 65 | 65 | 65 | 48 | 48 | 48 | 45 | 45 | 45 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 3 | 3 | 3 | 0 | 0 | 0 | 25 | 25 | 25 |
| Mvmt Flow | 0 | 179 | 3 | 9 | 88 | 3 | 0 | 0 | 44 | 189 | 0 | 4 |



## APPENDIX C. SHORT-TERM FUTURE LEVEL OF SERVICE WORKSHEETS




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.3 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y |  |  | 4 | 4 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 16 | 100 | 124 | 113 | 207 | 97 |
| Future Vol, veh/h | 16 | 100 | 124 | 113 | 207 | 97 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | 100 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 64 | 64 | 60 | 60 |
| Heavy Vehicles, \% | 2 | 2 | 8 | 8 | 1 | 1 |
| Mvmt Flow | 17 | 109 | 194 | 177 | 345 | 162 |


| Major/Minor | Minor2 |  | Major1 |  | ajor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 910 | 345 | 507 | 0 | - | 0 |
| Stage 1 | 345 | - | - | - | - | - |
| Stage 2 | 565 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.18 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.272 | - | - | - |
| Pot Cap-1 Maneuver | 305 | 698 | 1028 | - | - | - |
| Stage 1 | 717 | - | - | - | - | - |
| Stage 2 | 569 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 247 | 698 | 1028 | - | - | - |
| Mov Cap-2 Maneuver | 375 | - | - | - | - | - |
| Stage 1 | 581 | - | - | - | - | - |
| Stage 2 | 569 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | NB |  | SB |  |
| HCM Control Delay, s | 12.2 |  | 4.9 |  | 0 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT EBLn1 |  | SBT | SBR |
| Capacity (veh/h) |  | 1028 | - | 624 | - | - |
| HCM Lane V/C Ratio |  | 0.188 | - | 0.202 | - | - |
| HCM Control Delay (s) |  | 9.3 | - | 12.2 | - | - |
| HCM Lane LOS |  | A | - | B | - | - |
| HCM 95th \%tile Q(veh) |  | 0.7 | - | 0.8 | - | - |




| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 6.1 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \& |  |  | \& |  |  | \$ |  |
| Traffic Vol, veh/h | 0 | 100 | 0 | 1 | 109 | 0 | 0 | 0 | 0 | 110 | 0 | 0 |
| Future Vol, veh/h | 0 | 100 | 0 | 1 | 109 | 0 | 0 | 0 | 0 | 110 | 0 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 48 | 48 | 48 | 44 | 44 | 44 | 92 | 92 | 92 | 51 | 51 | 51 |
| Heavy Vehicles, \% | 1 | 1 | 1 | 1 | 1 | 1 | 2 | 2 | 2 | 28 | 28 | 28 |
| Mvmt Flow | 0 | 208 | 0 | 2 | 248 | 0 | 0 | 0 | 0 | 216 | 0 | 0 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |





| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 6 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \& |  |  | $\uparrow$ |  |  | \& |  |
| Traffic Vol, veh/h | 0 | 59 | 1 | 6 | 57 | 0 | 0 | 0 | 21 | 79 | 0 | 2 |
| Future Vol, veh/h | 0 | 59 | 1 | 6 | 57 | 0 | 0 | 0 | 21 | 79 | 0 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 33 | 33 | 33 | 65 | 65 | 65 | 48 | 48 | 48 | 45 | 45 | 45 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 3 | 3 | 3 | 0 | 0 | 0 | 25 | 25 | 25 |
| Mvmt Flow | 0 | 179 | 3 | 9 | 88 | 0 | 0 | 0 | 44 | 176 | 0 | 4 |



## APPENDIX D. LONG-TERM FUTURE LEVEL OF SERVICE WORKSHEETS

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | $\uparrow$ | 「 | ${ }^{1}$ | 4 | F | ${ }^{*}$ | 4 | 「 |
| Traffic Vol, veh/h | 3 | 0 | 21 | 5 | 0 | 3 | 0 | 138 | 7 | 0 | 314 | 0 |
| Future Vol, veh/h | 3 | 0 | 21 | 5 | 0 | 3 | 0 | 138 | 7 | 0 | 314 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control S | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | 0 | 100 | - | - | 100 | - | 125 |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 42 | 42 | 42 | 36 | 36 | 36 | 60 | 60 | 60 | 72 | 72 | 72 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mumt Flow | 7 | 0 | 50 | 14 | 0 | 8 | 0 | 230 | 12 | 0 | 436 | 0 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.2 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y |  |  | 4 | 4 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 16 | 100 | 124 | 129 | 236 | 97 |
| Future Vol, veh/h | 16 | 100 | 124 | 129 | 236 | 97 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | 100 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 64 | 64 | 60 | 60 |
| Heavy Vehicles, \% | 2 | 2 | 8 | 8 | 1 | 1 |
| Mvmt Flow | 17 | 109 | 194 | 202 | 393 | 162 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 41.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | * |  | ${ }^{1}$ | F |  | ${ }^{4}$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 10 | 0 | 205 | 3 | 0 | 6 | 107 | 247 | 1 | 3 | 339 | 3 |
| Future Vol, veh/h | 10 | 0 | 205 | 3 | 0 | 6 | 107 | 247 | 1 | 3 | 339 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | 100 | - | - | 100 | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 47 | 47 | 47 | 75 | 75 | 75 | 50 | 50 | 50 | 57 | 57 | 57 |
| Heavy Vehicles, \% | 13 | 13 | 13 | 0 | 0 | 0 | 6 | 6 | 6 | 0 | 0 | 0 |
| Mvmt Flow | 21 | 0 | 436 | 4 | 0 | 8 | 214 | 494 | 2 | 5 | 595 | 5 |



| Approach | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| HCM Control Delay, s | 144.1 | $\$ 497.4$ | 3 | 0.1 |
| HCM LOS | F | F |  |  |


| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1WBLn1 | SBL | SBT | SBR |  |  |
| :--- | ---: | ---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Capacity (veh/h) | 958 | - | -381 | 15 | 1078 | - | - |  |
| HCM Lane V/C Ratio | 0.223 | - | -1.201 | 0.8 | 0.005 | - | - |  |
| HCM Control Delay (s) | 9.8 | - | $-144.1 \$ ~ 497.4$ | 8.4 | - | - |  |  |
| HCM Lane LOS | A | - | - | F | F | A | - | - |
| HCM 95th \%tile Q(veh) | 0.9 | - | - | 18.7 | 1.9 | 0 | - | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds 300s $\quad+$ : Computation Not Defined *: All major volume in platoon

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 6.1 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \& |  |  | \& |  |  | \$ |  |
| Traffic Vol, veh/h | 0 | 100 | 0 | 1 | 109 | 0 | 0 | 0 | 0 | 110 | 0 | 0 |
| Future Vol, veh/h | 0 | 100 | 0 | 1 | 109 | 0 | 0 | 0 | 0 | 110 | 0 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 48 | 48 | 48 | 44 | 44 | 44 | 92 | 92 | 92 | 51 | 51 | 51 |
| Heavy Vehicles, \% | 1 | 1 | 1 | 1 | 1 | 1 | 2 | 2 | 2 | 28 | 28 | 28 |
| Mvmt Flow | 0 | 208 | 0 | 2 | 248 | 0 | 0 | 0 | 0 | 216 | 0 | 0 |




HCMLOS B B

| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1WBLn1WBLn2 | SBL | SBT | SBR |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- |
| Capacity (veh/h) | 1258 | - | -724 | 392 | 785 | $\sim$ | - | - |  |
| HCM Lane V/C Ratio | - | - | -0.077 | 0.028 | 0.011 | $\sim$ | - | - |  |
| HCM Control Delay (s) | 0 | - | - | 10.4 | 14.4 | 9.6 | - | - | - |
| HCM Lane LOS | A | - | - | B | B | A | - | - | - |
| HCM 95th \%tile Q(veh) | 0 | - | - | 0.2 | 0.1 | 0 | $\sim$ | - | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |





| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 6 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \& |  |  | \$ |  |  | 4 |  |
| Traffic Vol, veh/h | 0 | 59 | 1 | 6 | 57 | 0 | 0 | 0 | 21 | 79 | 0 | 2 |
| Future Vol, veh/h | 0 | 59 | 1 | 6 | 57 | 0 | 0 | 0 | 21 | 79 | 0 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 33 | 33 | 33 | 65 | 65 | 65 | 48 | 48 | 48 | 45 | 45 | 45 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 3 | 3 | 3 | 0 | 0 | 0 | 25 | 25 | 25 |
| Mvmt Flow | 0 | 179 | 3 | 9 | 88 | 0 | 0 | 0 | 44 | 176 | 0 | 4 |



## V1_Traffic Impact Study Redlines.pdf Markup Summary

| Carlos (4) |  |  |
| :---: | :---: | :---: |
|  <br> Lewia Pilmer Muddile School Monumean co | Subject: Text Box <br> Page Label: 1 <br> Author: Carlos <br> Date: 3/14/2023 2:21:59 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Please add a signatures page after the cover page. Visit <br> https://planningdevelopment.elpasoco.com/plannin g-development-forms/\#1584029763943-19bc4c033586 for El Paso County standard signature blocks. |
|  | Subject: Text Box <br> Page Label: 1 <br> Author: Carlos <br> Date: 3/9/2023 2:03:47 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Please add "PCD File No. CDR-23-005" |
|  | Subject: Cloud+ <br> Page Label: 18 <br> Author: Carlos <br> Date: 3/21/2023 4:52:07 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Diagram shows buses driving through parking spaces for buses. Should the diagram follow the other route shown instead to prevent backing into woodmoor and reduce turning radius? |
|  | Subject: Highlight <br> Page Label: 18 <br> Author: Carlos <br> Date: 3/16/2023 5:10:44 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 |  |

## Daniel Torres (13)



Subject: Callout
Page Label: 4
Please indicate whether parents/visitors/students
Author: Daniel Torres etc. will be allowed to use the inbound only

Date: 3/21/2023 3:20:56 PM
Length: 0
Area: 0
Volume: 0 entrance during non peak hour


## Subject: Callout <br> Page Label: 9

Author: Daniel Torres
Date: 3/21/2023 2:18:51 PM
Length: 0
Area: 0
Volume: 0

|  | Subject: Text Box <br> Page Label: 10 <br> Author: Daniel Torres <br> Date: 3/21/2023 4:11:55 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | show ADT on all figures as indicated in ECM appendix B. 8 |
| :---: | :---: | :---: |
| $\qquad$ | Subject: Callout <br> Page Label: 12 <br> Author: Daniel Torres <br> Date: 3/21/2023 4:30:41 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Please identify possible solutions to the deficient turn movements. |
| $\qquad$ | Subject: Callout <br> Page Label: 13 <br> Author: Daniel Torres <br> Date: 3/21/2023 3:52:31 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | identify what the scaling factor is that is used |
|  | Subject: Callout <br> Page Label: 14 <br> Author: Daniel Torres <br> Date: 3/21/2023 4:10:19 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Please explain the large amount of exiting vehicles from the main school entrance (\#1) if the exit point for the students/visitors is at Willow Park Wy. (exit \#2) during peak hrs. Revise as necessary the peak hr traffic at the entrance and exit. |
|  | Subject: Highlight <br> Page Label: 14 <br> Author: Daniel Torres <br> Date: 3/21/2023 4:08:51 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 |  |
|  | Subject: Highlight <br> Page Label: 14 <br> Author: Daniel Torres <br> Date: 3/21/2023 4:08:54 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 |  |
| $x=2$ $2=2$ | Subject: Text Box <br> Page Label: 17 <br> Author: Daniel Torres <br> Date: 3/21/2023 4:38:16 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | staff recommends stating that volume thresholds for a right turn aux. lane are not met at this proposed access |



