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SUBSURFACE SOIL INVESTIGATION WINDINGWALK FILINGS 1 AND 2 AND STONEBRIDGE THE ENCLAVE FILINGS 4 AND 5 STAPLETON DRIVE AND EASTONVILLE ROAD EL PASO COUNTY, COLORADO

Prepared for:

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Attn: Mr. Raul Guzman

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Respectfully Submitted,

ENTECH ENGINEERING, INC.

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KAH/rm

Encl.

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SUBSURFACE SOIL INVESTIGATION WINDINGWALK FILINGS 1 AND 2 AND STONEBRIDGE THE ENCLAVE FILINGS 4 AND 5 STAPLETON DRIVE AND EASTONVILLE ROAD EL PASO COUNTY, COLORADO

1.0 INTRODUCTION

The project consists of the development of the site for the construction of single-family residences. Development is expected to include site grading, installation of subsurface utilities, roadways, and drainage structures. The site is Windingwalk Filings 1 and 2 and Stonebridge The Enclave Filings 4 and 5, north of Stapleton Drive and west of Eastonville Road in El Paso County, Colorado. The approximate location of the project site is shown on the Vicinity Location Map, Figure 1. The test boring locations are shown on Figure 2, the Test Boring Location Plan.

This report describes the subsurface investigation conducted for the site and provides recommendations for development design and construction. The Subsurface Soil Investigation included the drilling of fifty-five test borings across the site, collecting samples of soil, and conducting a geotechnical evaluation of the investigation findings. All drilling and subsurface investigation activities were performed by Entech Engineering, Inc. (Entech). The contents of this report, including the geotechnical evaluation and recommendations, are subject to the limitations and assumptions presented in Section 17.0.

Please add the size reptile the total area).

The project will consist of developing the site for single family residential structures. The planned lots are located in Windingwalk Filings 1 and 2 and Stonebridge The Enclave Filings 4 and 5, north of Stapleton Drive and west of Eastonville Road, just north of Falcon, Colorado in El Paso County. Investigation was performed at predetermined locations designated based on the roadway alignment and proposed grading on the site plan provided to Entech. At the time of drilling, the site was vacant and not developed. Enclave Scenic Way and Fairway Glen Circle have been graded prior to this investigation. The site has general gradual slope towards the south. Vegetation consisted of field grasses and weeds. Existing residences were located to the north and northeast of the site. A golf course exists west of the site. Stapleton Drive borders the southern boundary of the site.

3.0 SUBSURFACE EXPLORATIONS AND LABORATORY TESTING

Subsurface conditions on the site were explored by drilling fifty-five test borings at the approximate locations shown on Figure 2. The borings were drilled to depths of 20 to 30 feet below the existing ground surface (bgs). The drilling was performed using a truck-mounted, continuous flight auger-drilling rig supplied and operated by Entech. Boring logs descriptive of the subsurface conditions encountered during drilling are presented in Appendix A. At the conclusion and subsequent to drilling, observations for groundwater levels were made in each of the open boreholes.

Soil and bedrock samples were obtained from the borings utilizing the Standard Penetration Test (ASTM D-1586) using 2-inch O.D. split-barrel and California samplers. Results of the Standard Penetration Test (SPT) are included on the boring logs in terms of N-values expressed in blows per foot (bpf). Soil and bedrock samples recovered from the borings were visually classified and recorded on the boring logs. The soil and bedrock classifications were later verified utilizing laboratory testing and grouped by soil type. The soil and bedrock type numbers are included on the boring logs. It should be understood that the soil and bedrock descriptions shown on the boring logs may vary between boring location and sample depth. It should also be noted that the lines of stratigraphic separation shown on the boring logs

represent approximate boundaries between soil and bedrock types and the actual stratigraphic transitions may be more gradual or variable with location.

Water content testing (ASTM D-2216) was performed on the samples recovered from the borings, and the results are shown on the boring logs. Grain-Size Analysis (ASTM D-422) and Atterberg Limits testing (ASTM D-4318) were performed on selected samples to assist in classifying the materials encountered in the borings. Volume change testing was performed on selected samples using the Swell/Consolidation Test (ASTM D-4546) and the FHA Swell Test in order to evaluate potential expansion/compression characteristics of the soil and bedrock. Soluble sulfate testing was performed on select soil samples to evaluate the potential for below grade degradation of concrete due to sulfate attack. The Laboratory Testing Results are summarized on Table 1 and are presented in Appendix B.

4.0 SUBSURFACE CONDITIONS

Three soil types and three bedrock types were encountered in the test borings drilled for the subsurface investigation: Type 1A: silty to clayey sand fill (SM, SC), Type 1: slightly silty to silty sand and clayey sand (SM-SW, SM, SC), Type 2: sandy to very sandy clay (CL), Type 3: slightly silty to very silty and clayey to very clayey sandstone (SM-SW, SM, SC), Type 4: sandy to very sandy claystone (CL), and Type 5: clayey, sandy siltstone (ML). Soils were classified in accordance with the Unified Spices howethe Capproximate range of each State Highway and Transport Soil Office (MC) exhibitsing the laboratory testing results and the observations mathries of ved. Provide an exhibit showing

the approximate band (area) of the

4.1 Soil and Bedrock

<u>Soil Type 1A</u> classified as a silty to clayey sand fill (SM, SC, A-1-b). the sand fill was encountered in seven of the test borings, Test Boring Nos. 1, 2, 3, 36, 37, 38, and 39 at the existing ground surface and extending to depths ranging from 3 to 14 feet below the ground surface (bgs). The fill was located along Enclave Scenic Way and Fairway Glen Circle as noted in Figure 2. Reportedly the fill was observed and tested by others. Records of testing will be provided as required. Standard Penetration Testing resulted in SPT N-values ranging from 5 to 34 bpf, indicating loose to dense states. Water content and grain size testing resulted in water

different soil types.

contents ranging from 5 to 15 percent with approximately 15 to 32 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits Testing resulted in the silty sand fill being non-plastic. FHA Swell Testing resulted in a swell pressure of 240 psf, indicating low expansion potential.

Stockpiles of fill are located in the vicinity of six of the test borings, Test Boring Nos. 11, 12, 13, 14, 43, and 50. These test borings were drilled adjacent to and on the stockpiled fill. These fill piles will require complete removals prior to any additional fill planned for this portion of the site. Based on the soil type encountered in the single boring drilled in this stockpiled fill, it is likely that this stockpiled fill can be used within this development. The fill removals should be observed by Entech to determine if additional soil sampling or laboratory testing is required.

<u>Soil Type 1</u> classified as a slightly silty to silty sand and clayey sand (SM-SW, SM, SC, A-1-b, A-2-4, A-5, A-6). The sand was encountered in fifty-two of the test borings at depths ranging from the existing ground surface to 14 feet and extending to depths ranging from one foot to the termination of the borings (20 feet). Standard Penetration Testing conducted on the sand resulted in SPT N-values ranging from 8 to greater than 50 blows per foot (bpf), indicating the soil to be at loose to very dense states. Water content and grain size testing of soil samples resulted in a water content range of 2 to 23 percent with approximately 6 to 38 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits testing resulted in liquid limits ranging from 29 to no value and plastic indexes of 15 to non-plastic. Sulfate testing on samples of Soil Type 1 resulted 0.00 to 0.01 percent soluble sulfate by weight, which indicates a negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 2 classified as sandy to very sandy clay (CL, A-4, A-6). The clay was encountered in four of the test borings at depths ranging from the ground surface to 14 feet below the ground surface (bgs) and extending to depths ranging from 3 to 16 feet bgs. Standard Penetration Testing on the clay resulted in SPT N-values of 10 to 32 bpf, indicating firm to very stiff consistencies. Water content and grain size testing resulted in approximately 11 to 20 percent water content with approximately 52 to 53 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits testing resulted in liquid limits of 20 to 30 and plastic indexes of 10 to 14. A FHA Swell pressure of 1720 psf was measured on the clay, indicating moderate to high expansion potential. No swell (0.0 percent) was measured in the Swell/Consolidation Test

conducted on the clay. Sulfate testing resulted in less than 0.01 percent sulfate by weight indicating negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 3 classified as a slightly silty to very silty and clayey to very clayey sandstone (SM-SW, SM, SC, A-1-b, A-2-4, A-2-6, A-2-7, A-4, A-6). The sandstone was encountered in fifty-one of the test borings at depths ranging from 1 to 18 feet bgs and extending to depths ranging from 6 feet to 19 feet bgs and to the termination of the borings (20 to 30 feet). Standard Penetration Testing conducted on the sandstone resulted in SPT N-values from 37 to greater than 50 bpf, which indicates dense to very dense states. Water content and grain size testing resulted in approximately 4 to 21 percent water content with approximately 8 to 45 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits testing resulted in liquid limits of 41 to no value and plastic indexes of 18 to non-plastic. FHA Swell testing on the sandstone resulted in a swell pressure of 30 psf, indicating low expansion potential. Swell/Consolidation testing on the very clayey sandstone resulted in volume change of 2.1 percent indicating moderate to high expansion potential. Sulfate testing on the sandstone resulted in 0.00 to less than 0.01 percent sulfate by weight indicating the sandstone exhibits negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 4 classified as a sandy to very sandy claystone (CL, A-4, A-5, A-6). The claystone was encountered in thirteen of the test borings at depths ranging from 1 to 24 feet bgs and extending to depths ranging from 8 to 28 feet bgs or to the termination of the borings (20 to 25 feet). Standard Penetration Testing conducted on the claystone resulted in SPT N-values of 40 to greater than 50 bpf, which indicates very stiff to hard consistencies. Water content and grain size testing resulted in approximately 10 to 17 percent water content with approximately 57 to 66 percent of the soil size particles passing the No. 200 sieve. Atterberg limits testing on the claystone resulted in liquid limits of 21 to 29 and plastic indexes of 8 to 13. FHA Swell Testing resulted in swell pressures of 1970 to 2060 psf, indicting the claystone exhibits high expansion potential. Swell/Consolidation testing on the claystone resulted in a volume change of 1.7 percent, indicating moderate expansion potential. Sulfate testing on the claystone resulted in 0.00 to less than 0.01 percent sulfate by weight indicating the claystone exhibits negligible degradation to concrete due to sulfate attack.

<u>Soil Type 5</u> classified as a clayey, sandy siltstone (ML, A-6). The siltstone was encountered in two of the test borings at depths of 1 to 11 feet and extending to depths of 6 feet bgs to the

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termination of the boring (20 feet). Standard Penetration Testing on the siltstone resulted in

SPR N-values greater than 50 bpf, indicating hard consistencies. Water content and grain size testing resulted in water content of 9 to 19 percent with approximately 67 to 85 percent of the

soil size particles passing the No. 200 sieve. Atterberg Limits Testing resulted in a liquid limit of

40 and a plastic index of 14. Swell/Consolidation conducted on the siltstone resulted in a

volume change of 0.9 percent, indicating low to moderate expansion potential.

The test boring logs are included in Appendix A. Laboratory Test Results are included in

Appendix B and Summarized in Table 1. Bedrock depths are summarized in Table 2.

4.2 Groundwater

Depth to groundwater was measured in each of the borings at the conclusion of drilling and

subsequent to drilling. Groundwater was encountered in thirty-nine of the fifty-five test borings

at depths ranging from 2.5 to 20.5 feet. Groundwater may affect construction and utilities

construction on this site. Much of these areas are to be filled. The groundwater depths and

estimated cuts and fills are summarized in Table 2. Unstable conditions may be encountered

where excavations approach the groundwater level. Stabilization using shot rock or geogrids

may be necessary. Typically the sandstone will remain stable when water is encountered. It

should be noted that groundwater levels could change due to seasonal variations, changes in

land runoff characteristics and future development including nearby areas. Table 2 presents

depth to groundwater, estimated cut/fill and depth to bedrock.

5.0 DEVELOPMENT CONSIDERATIONS

The following discussion is based on the subsurface conditions encountered in the borings

drilled at the site. This investigation is for the site discussed in 2.0 Project and Site Description.

If subsurface conditions different from those described herein are encountered during

construction or if the project elements change from those described, Entech Engineering, Inc.

should be notified so that the evaluation and recommendations presented can be reviewed and

revised if necessary.

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Subsurface Soil Investigation Windingwalk Filings 1 and 2, and Stonebridge The Enclave Filings 4 and 5 El Paso County, Colorado

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Subsurface soil conditions encountered in the test borings drilled on the site generally consisted of sand fill, silty to clayey sand, and sandy clay overlying sandstone, claystone and siltstone bedrock. Bedrock was encountered at depths ranging from 1 to 16 feet bgs and greater than 20 feet in one of the test borings. The fill located along Enclave Scenic Way and Fairway Glen Circle is considered controlled, however records of fill placement should be provided to Entech for review to determine the extent of the testing in this area. Fill piles were encountered in the central portion of the site in the vicinity of Test Boring Nos. 11, 12, 13, 14, 43, and 50. All fill piles need to be completely removed. Complete removals of these undocumented fill piles will be required prior to developing this area or placing additional fill. Recommendations for site grading, fill preparation, and compaction efforts are described in Section 6.0. Consideration should be given to several conditions on this site in planning and excavating the development besides the fill pile mitigation described above including groundwater, expansive soils and bedrock materials.

5.1 Groundwater

Groundwater should be expected to impact the development. Table 2 presents depth to bedrock and groundwater. Subsequent to completion of overlot grading cuts per the grading plan presented to us, the measured water levels will be at 6 feet or shallower at locations of Test Boring Nos. 1, 5, 6, 8, 16, 19, 27 and 39. Groundwater measured as shallow as nine feet in planned fill areas is not expected to influence construction of fills. Unstable conditions should be expected where groundwater is shallow or close to excavated depths. Procedures and equipment to mitigate groundwater impact during and after construction should be anticipated. Pumps, cofferdams, wide area and localized drain systems and other procedures and equipment may be necessary. Shot rock and geotextiles may be appropriate for stabilizing excavations. An underdrain system can be considered for long term groundwater mitigation. Single figure, typical all.

Frequently, groundwater levels rise following development as result of increased irrigation and decreased potential area of evaporation. Provide an exhibit showing the general area where shallow ground water is

Expansive soils (clayey sand, clay, very clayey sandstone, claystone and clayey siltstone) are present on the site exhibiting expansion potential ranging from low to high. Mitigation of expansive soils will be required on portions of the lots.

5.3 Sandstone, Siltstone and Claystone

Sandstone, siltstone and claystone were encountered at depths as shallow as one to four feet. Bedrock was encountered at shallow depths across the majority of the site. Excavation of sandstone, siltstone and claystone should be expected to be moderate to difficult. Track type equipment likely will be needed to accomplish excavations particularly where harder materials or lenses are present. Upon completion of site grading per the plan provided to us, sandstone is expected to be exposed across the majority of the areas tested.

6.0 SITE GRADING

Shallow bedrock (less than 6 feet) was encountered in thirty-nine of the fifty-five test borings drilled on the site. Excavation of most bedrock materials on site is expected to be moderate to difficult with heavy duty earthmoving equipment. Claystone, siltstone and sandstone materials may require track equipment and ripping teeth. For conditions with no groundwater seepage, cut and fill slopes no steeper than 3 to 1 (horizontal to vertical) should be considered. If seepage occurs, then flatter slopes or a drain system should be considered. Recommendations may be subject to change depending upon particular field conditions.

6.1 Stripping

Debris, topsoil and organic materials should be stripped from the ground surface of areas to be filled. The fill in Enclave Scenic Way and Fairway Glen Circle are believed to be controlled and tested by others and appeared to be prepared to the approved Stonebridge Filing 1 grading plans. Any uncontrolled fill materials should be completely removed. Fill was encountered in central portion of the site that is considered uncontrolled. The materials may be used as fill pending approval if they are free of organic material and debris. Any soft or loose soils should be stabilized or removed to expose suitable material prior to placement of fill. Topsoil may be stored in stock piles and placed at the surface in landscape areas.

6.2 Fill Preparation

Surfaces which will receive fill should be scarified to depths of 6 inches, moisture conditioned to within 2 percent of optimum moisture, and compacted to minimum of 95 percent of Standard Proctor Dry Density (ASTM D 698). On-site natural soils and bedrock are anticipated to be used

as site grading fill. Bedrock must be processed and broken down where placed in the fill. Expansive materials used for fill should be placed at sufficient moisture content to mitigate potential swell. The fill quality will influence the performance of foundations, slabs-on-grade, and pavements. Fill settlement can be minimized by placing thin lifts at suitable moisture content and by verification of compaction with frequent density tests.

6.3 Compaction

Overlot grading fill consisting of granular soils should be placed in lifts to exceed 6 inches following compaction and compacted to at least 95 percent of the maximum dry density determined by Modified Proctor (ASTM D-1557). Clay materials should be placed in compacted lifts less than 6 inches thick compacted to at least 95 percent of maximum Standard Proctor (ASTM D 698) Dry Density. Fills below 10 feet in depth should be moisture conditioned as above and compacted to 98 percent of Standard Proctor Dry Density (ASTM D 698) for cohesive materials or 98 percent of maximum Modified Proctor Dry Density (ASTM D 1557) for granular materials. The soil materials should be placed at a moisture content conducive to adequate compaction, usually within ±2 percent of optimum moisture content. Fill placement and compaction should be observed and tested by Entech during construction to verify that adequate moisture and density has been achieved.

7.0 UNDERGROUND UTILITY CONSTRUCTION

Generally excavation is expected to be moderate to difficult utilizing heavy-duty trackhoes. Rock buckets and rock teeth will likely be required where excavations extend into very hard sandstone or cemented materials. Special procedures or equipment may be required to remove water and/or achieve stability in utility trenches where excavations approach or intercept groundwater.

Utilities including water and sewer lines are usually constructed beneath paved roads. Placement of fill and degree of compaction applied to trench backfill will influence performance of overlying structures including pavements. Fill placed into utility trenches should be compacted according to requirements of the local jurisdiction. Fill should be placed in horizontal lifts having compacted thickness of six inches or less and at a water content conducive adequate compaction, usually within ±2 percent of optimum water content. Typical compaction

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specifications would be similar to specifications in the Site Grading section. Mechanical methods should be used for fill placement, however, heavy equipment should be kept at a distance away from structures to avoid damage. No water flooding techniques of any type should be used for compaction or placement of utility trench backfill.

Trench backfill should be performed in accordance with El Paso County specifications and requirements. Excavations and excavation shoring/bracing should be performed in accordance with OSHA guidelines.

8.0 UNDERDRAIN SYSTEM

Depending on final site grading anticipated depths of excavations and structure foundations relative to groundwater occurrence, an underdrain system may be considered to be included as part of sewer system design and installation. The underdrain system drain pipe shall consist of smooth wall non perforated rigid PVC pipe placed at a minimum slope of 0.5 percent. Concrete or clay material fill may be strategically placed at the manhole locations to slow the water flow down the trench. The underdrain below sewer should be constructed with adequate depth to allow connection of residence foundation drain systems. Drain elements should be of appropriate slopes and sizes for anticipated flows. Maintenance of the underdrain system should be anticipated. Gravity outlet should be planned such that other developments and properties are not adversely affected.

9.0 PAVEMENT CONSIDERATIONS

Materials exposed at pavement subgrade elevations will be dependent upon native materials exposed at final overlot grading and the specific materials placed as fill at and near finish grade elevations after utility installation. The predominate materials are generally expected to be silty sand, sandstone, clayey sand, clay, claystone and siltstone. Materials anticipated at subgrade elevation generally would be rated as typically having good pavement support characteristics with some areas likely rated as having poor pavement support characteristics. AASHTO classifications of A-1-b, A-2-4, A-2-6, A-2-7, and A-4 were determined for the majority of the sandstone and upper granular soils. Based on depth to claystone and estimated cut, the

claystone may be exposed within the pavement subgrade. The claystone classifies as A-4, A-5, and A-6 using the AASHTO classification system which typically provide fair to poor pavement support and may be encountered on the site depending on the final grading plans. Thickness of asphalt pavement and aggregate base course sections may be 4 inches of asphalt overlying 6 to 10 inches of base course depending on specific subgrade materials and Roadway Classification of each particular street. For specific thickness determinations, a subsurface soil investigation and pavement design should be completed after completion of overlot grading and utility installation. Chemically treated subgrades may be used on this site and final recommendations can be provided in the pavement design.

10.0 ANTICIPATED RESIDENTIAL FOUNDATION SYSTEMS

Subsurface soil conditions consisted of granular materials with some areas of expansive clay claystone or clayey siltstone materials. We anticipate conventional spread footing foundation systems will be appropriate for residences constructed on the majority of the site. Where expansive materials are encountered at or near foundation grades, use of spread footings with overexcavation and replacement with non-expansive fill should be expected. Drilled pier foundations may be a suitable alternative where expansive soils are encountered. A Subsurface Soils Investigation report should be prepared after completion of overlot grading to address appropriate foundation systems for each lot. Perimeter below grade foundation drain systems should be anticipated. Overexcavation drains may also be required. Figures 3 and 4 present typical drain overexcavation and building perime Add 25 hallow groundwater was encountered at Test Boring Nos. 1, 5, 6, 8, 16, 19, 27 and 39. Temporary and permanent dewatering systems may be necessary at various foundation excavations. Shotrock and geotextiles may be appropriate for stabilizing excavations. An area wide subdrain may be considered for discharge of collected water.

11.0 RESIDENCE ON-GRADE FLOOR SLABS

On-grade floor slabs for the planned structures could be supported by on-site non-expansive soils or compacted, non-expansive, structural fill. Loose or expansive soils encountered at or near floor slab grade should be penetrated or overexcavated a distance below slab subgrade

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and replaced with a non-expansive structural fill to improve floor slab performance. If slab movement and cracks cannot be tolerated a structural floor system should be used. Evaluation of subgrade materials should be included within a Subsurface Soils Investigation for each

specific lot.

12.0 CONCRETE DEGRADATION DUE TO SULFATE ATTACK

Sulfate solubility testing was conducted on twenty-two samples recovered from the test borings to evaluate the potential for sulfate attack on concrete placed below surface grade. The test results indicated 0.00 to 0.01 percent soluble sulfate (by weight). The test results indicate the sulfate component of the in-place soils presents a negligible exposure threat to concrete placed below the site grade. Type II cement is recommended for the on-site soils. Additional testing

should be conducted following completion of overlot grading.

13.0 EXCAVATION STABILITY

Excavation walls must be properly sloped/benched or otherwise supported in order to maintain stable conditions. All excavation openings and work execution shall conform to OSHA standards

as in CFR 29, Part 1926.650-652 (Subport D).

14.0 SURFACE AND SUBSURFACE DRAINAGE

Surface drainage will influence performance of structures at the site including streets and residences. Drainage is recommended around each building perimeter at a minimum slope of 5 percent in the first 10 feet adjacent to exterior foundation walls and for unpaved areas, where possible. For paved areas and other impervious surfaces, a minimum slope of 2 percent is recommended. Drainage should be planned to avoid ponding of water. Collected water and irrigation should discharge well beyond foundation backfill zones. Surface runoff should be designed to avoid sheet flow and erosion. Slopes should be protected from erosion by materials such as mulch or appropriate plants or other methods. All fills and backfills should be properly compacted. Unprotected surfaces may be subject to undesirable, heavy erosion.

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15.0 WINTER CONSTRUCTION

In the event construction occurs during winter, concrete and soil materials should be protected from freezing conditions. Concrete should not be placed on frozen soil and once concrete has been placed, it should not be allowed to freeze. Similarly, once exposed, the soil subgrades should not be allowed to freeze. During grading operations and subgrade preparation, care should be taken to avoid burial of snow, ice or frozen material within the planned construction area.

16.0 CONSTRUCTION OBSERVATIONS

It is recommended that Entech observe and document the following activities during construction of the building foundations.

- Excavated subgrades and subgrade preparation.
- Placement/compaction of fill materials.
- Placement/compaction of utility bedding and trench backfill.

17.0 CLOSURE

The subsurface investigation, geotechnical evaluation and recommendations presented in this report are intended for use by Tech Contractors with application to the planned development of the single-family residential project site located at Windingwalk Filings 1 and 2 and Stonebridge The Enclave Filings 4 and 5, in El Paso County, Colorado. In conducting the preliminary subsurface investigation, laboratory testing, engineering evaluation and reporting, Entech Engineering, Inc. endeavored to work in accordance with generally accepted professional geotechnical and geologic practices and principles consistent with the level of care and skill ordinarily exercised by members of the geotechnical profession currently practicing in same locality and under similar conditions. No other warranty, expressed or implied is made. During final design and/or construction, if conditions are encountered which appear different from those described in this report, Entech Engineering, Inc. requests that it be notified so that the evaluation and recommendations presented herein can be reviewed and modified as appropriate.

If there are any questions regarding the information provided herein or if Entech Engineering, Inc. can be of further assistance, please do not hesitate to contact us.

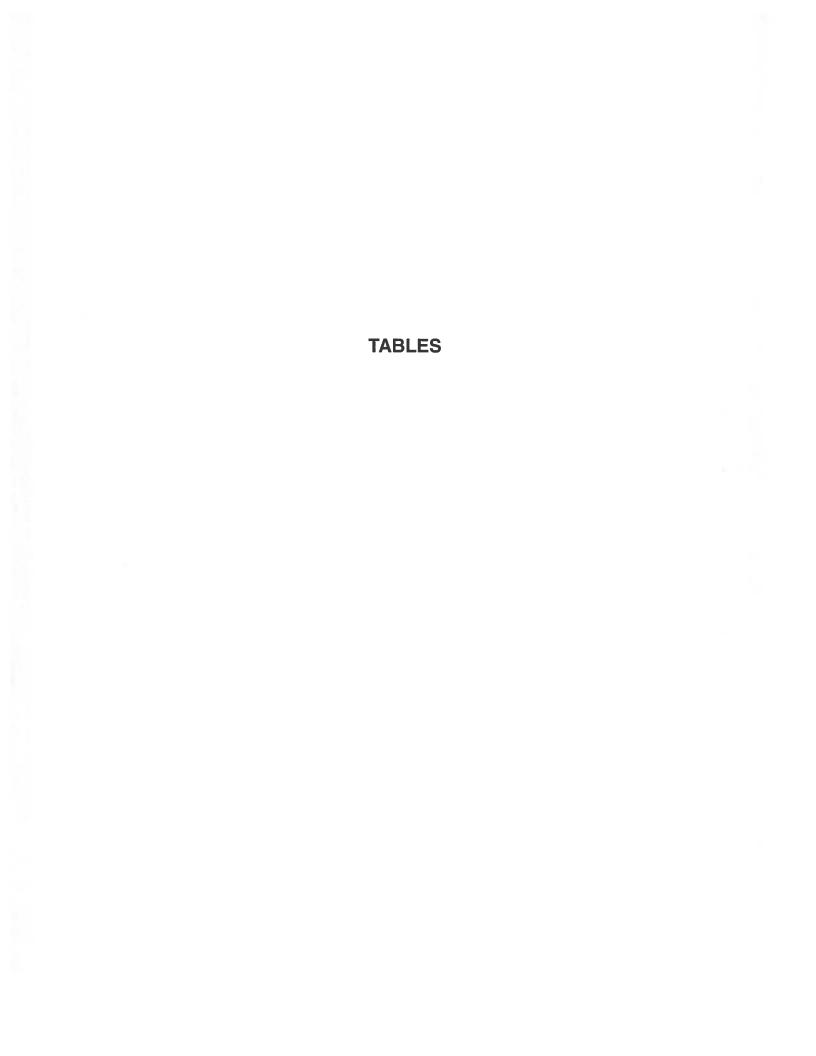


TABLE 1

SUMMARY OF LABORATORY TEST RESULTS

CLIENT PROJECT JOB NO.

TECH CONTRACTORS WINDINGWALK & STONEBRIDGE 171198

				_						_	_			_		_			_								_			_			_	_	_	_
SOIL DESCRIPTION	FILL, SAND, SILTY	FILL, SAND, CLAYEY	SAND, SLIGHTLY SILTY	SAND, CLAYEY	SAND, SLIGHTLY SILTY	SAND, CLAYEY	SAND, SILTY	SAND, SLIGHTLY SILTY	SAND, SLIGHTLY SILTY	CLAY, VERY SANDY	CLAY, SANDY	CLAY, VERY SANDY	SANDSTONE, SILTY	SANDSTONE, CLAYEY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, VERY CLAYEY	SANDSTONE, SILTY	SANDSTONE, VERY CLAYEY	SANDSTONE, SILTY	SANDSTONE, VERY CLAYEY	SANDSTONE, SILTY	SANDSTONE, SLIGHTLY SILTY	SANDSTONE, VERY SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY								
UNIFIED CLASS.	SM	SC	SM-SW	sc	SM-SW	sc	SM	SM-SW	SM	SM	SM	SM	SM-SW	ರ	ರ	75	SM	SC	SM	SM	SM	SM	sc	SM	SC	SM	SM	SM	SM	SM	SC	SM	SM-SW	SM	SM	SM
SWELL/ CONSOL (%)																0:0							2.1								2.1					
AASHTO CLASS.	A-1-b			A-5	A-1-b	A-6		A-1-b			A-2-4		A-1-b	A-4		9-Y	A-4	A-2-6	A-2-7		A-2-4		A-6									A-1-b	A-1-b	A-4		
FHA SWELL (PSF)		240													1720																					
SULFATE (WT %)					<0.01	<0.01		00.00			<0.01	0.01	<0.01	<0.01		<0.01	<0.01				<0.01		0.00	0.00								<0.01	<0.01	<0.01		
PLASTIC INDEX (%)	NP			6	NP	15		ΝP			_ dN		ΝP	10		14	ΝP	12	14		NP		17									NP	ΑN	ďN		
LIQUID LIMIT (%)	N			29	NV	28		N N			N N		N N	20		8	N N	36	41		NV		32									NV	2	N		
PASSING NO. 200 SIEVE (%)	15.3	31.8	11.7	35.5	6.1	38.4	12.6	11.0	26.9	13.7	28.2	17.1	8.7	51.6		53.2	38.7	29.5	28.7	16.9	27.1	25.2	47.2	15.1	38.2	24.4	28.7	24.3	13.2	18.5	43.5	15.6	11.8	40.4	19.3	23.4
DRY DENSITY (PCF)																101.2							111.8								108.3					
WATER (%)																22.2							14.3								20.1					
ОЕРТН (FT)	2-3	5	10	2-3	2-3	2-3	5	5	2-3	2	2-3	10	2-3	9	15	2-3	10	20	5	20	10	5	20	10	15	2-3	10	10	2	15	15	2	10	25	10	15
TEST BORING NO.	36	37	2	4	8	13	15	19	90	34	35	39	55	25	3	28	-	3	5	_7	6	10	11	12	14	16	18	20	21	22	23	29	31	32	33	38
SOIL	1A	1A	1	-	1	-	-	-	-	-	-	-	-	2	2	2	3	3	3	3	3	3	3	3	ဗ	3	3	3	3	3	3	3	8	3	3	3

											_									_
SOIL DESCRIPTION	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, VERY SILTY	SANDSTONE, SLIGHTLY SILTY	SANDSTONE, VERY CLAYEY	SANDSTONE, SILTY	SANDSTONE, SLIGHTLY SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, SLIGHTLY SILTY	SANDSTONE, SILTY	CLAYSTONE, SANDY	CLAYSTONE, SANDY	CLAYSTONE, SANDY	CLAYSTONE, VERY SANDY	CLAYSTONE, VERY SANDY	CLAYSTONE, SANDY	SILTSTONE, SANDY, CLAYEY	SILTSTONE, SANDY, CLAYEY
UNIFIED CLASS.	SM	SM	SM	SM-SW	SC	SM	SM-SW	SM	SM	SM	SM-SW	SM	ر ا	CL	CL	CL	CL	CL	ML	ML
SWELL/ CONSOL (%)														1.7						6.0
AASHTO CLASS.	A-1-b			A-1-b	A-6					4-1-P			9-Y	4-4				A-6	A-6	
FHA SWELL (PSF)						30									2060	1970				
SULFATE (WT %)			<0.01	00.00	<0.01					00.00			<0.01	<0.01				0.00		
PLASTIC INDEX (%)	NP			NP	18		-			NP			8	8				13	14	
LIQUID LIMIT (%)	N N			NV	33	ì				NN			21	29				27	40	
PASSING NO. 200 SIEVE (%)	12.0	22.2	45.3	8.4	41.5	15.0	11.3	15.1	27.7	18.3	9.7	20.2	66.3	64.7	65.2	56.5	60.3	63.3	85.4	66.5
DRY DENSITY (PCF)														117.2						111.1
WATER (%)														10.9						15.6
ОЕРТН (FT)	2-3	5	20	5	2-3	5	10	15	2-3	2	10	15	10	20	10	10	5	15	15	2-3
TEST BORING NO.	40	41	42	45	46	47	48	49	20	51	52	53	9	24	56	43	44	54	2	17
SOIL	င	က	9	3	3	3	3	3	3	3	3	3	4	4	4	4	4	4	5	5

TABLE 2: Summary of Estimated Cut/Fill, Depth to Bedrock, and Groundwater Depths

Client: Tech Contractors

Project: Windingwalk Filings 1 and 2, and Stonebridge The Enclave Filings 4 and 5

<u>Job No: 171198</u>

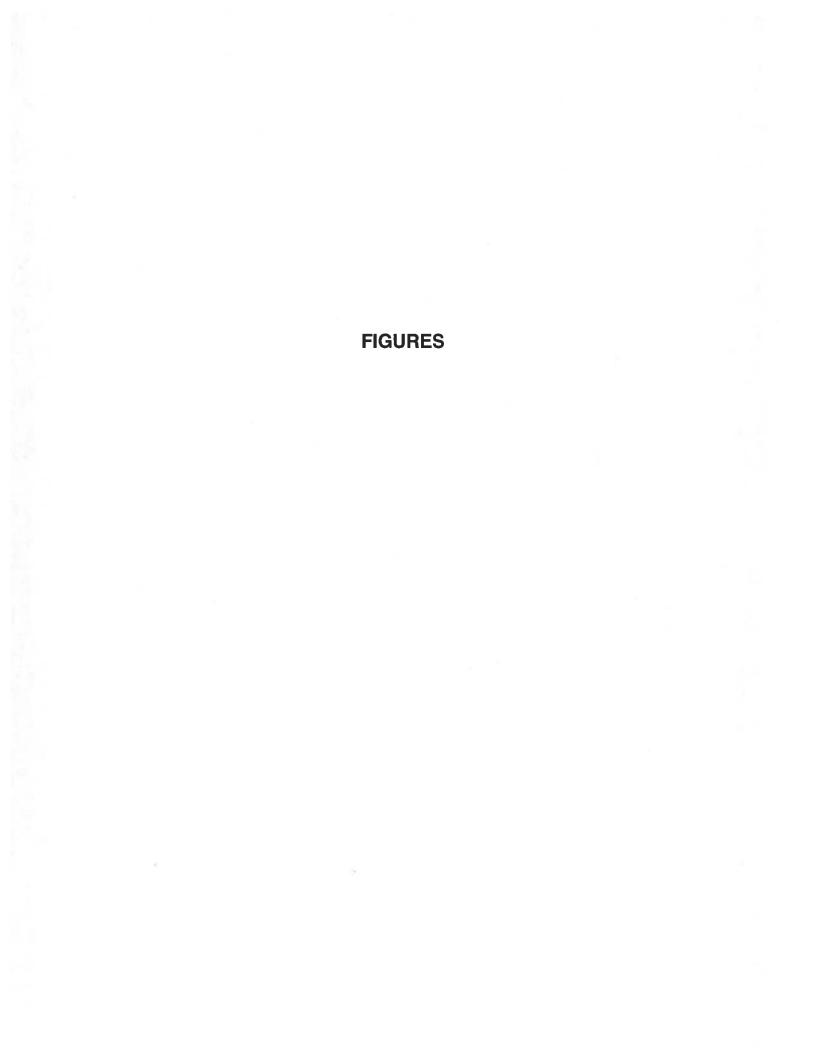
	F 11 1 10 1/21		
Toot Doning No	Estimated Cut/Fill	Depths to Bedrock	Depth to Groundwat
Test Boring No.	(ft.)	(ft.) ¹	(ft.) ¹
1	+ 0 - 2	8	2.5
2	+0-2	11	10.5
3	+ 0 - 2	16	12
4	+ 8 - 10	9*	4
5	+ 0 - 2	4	6
6	- 0 - 2	7	4
7	- 0 - 2	1	8.5
8	+0-2	11	6
9	+2-4	2	11.5
10	+0-2	2	9
11	- 2 - 4	1	15
12	- 0 - 2	1	13
13	-6-8	6	>25
14	- 10 - 12	2	20.5
15	+ 0 - 2	6	>20
16	- 2 - 4	1	7.5
17	+0-2	1	9
18	- 0 - 2	5	16
19	-0-2	8	6.5
20	+ 0 - 2	9	>20
21	+0-2	2	12.5
22	+ 2 - 4	2	14
23	+0-2	1	7.5
24	- 2 - 4	1	11
25	- 4 - 6	13	9
26	+ 0 - 2	4	7.5
27	+2-4	7	4
28	+0-2	3	11

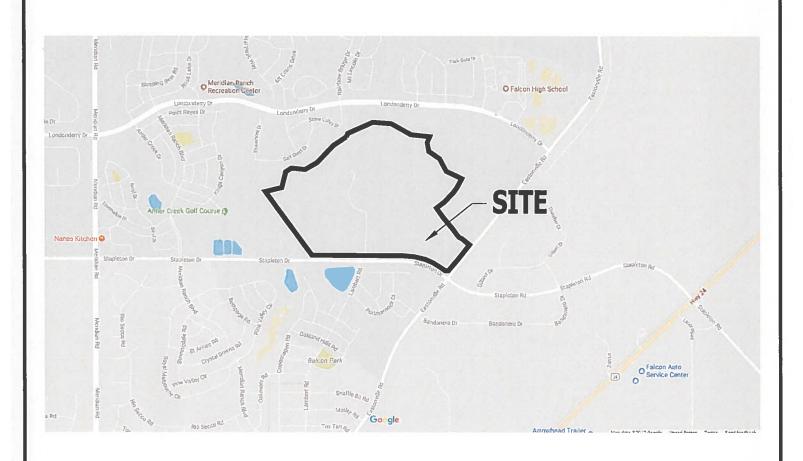
TABLE 2: Summary of Estimated Cut/Fill, Depth to Bedrock, and Groundwater Depths continued

<u>Client:</u> Tech Contractors <u>Project:</u> Winding Walk

Job No: 171198

- . .	Estimated Cut/Fill	Depths to Bedrock	Depth to Groundwate
Test Boring No.	(ft.)	(ft.) ¹	(ft.) ¹
29	+ 0 - 2	1	>20
30	+ 10 - 12	3	19
31	- 2 - 4	1	12.5
32	- 6 - 8	7	15.5
33	- 2 - 4	4	>20
34	+ 0 - 2	6	14.5
35	-2-4	6	15
36	- 0 - 2	>20	17.5
37	- 0 - 2	14	>20
38	- 0 - 2	6	>20
39	- 0 - 2	12	6
40	+ 2 - 4	1	15
41	+ 0 - 2	1	15.5
42	+ 2 - 4	4	18
43	- 4 - 6	2*	>20
44	+ 2 - 4	1	20
45	+ 2 - 4	1	18
46	- 4 - 6	1	>20
47	- 2 - 4	4	>20
48	+0-2	1	18
49	- 2 - 4	1	>20
50	+ 2 - 4	1	>20
51	- 2 - 4	1	>20
52	+ 0 - 2	1	>20
53	-0-2	1	>20
54	-0-2	14	18.5
55	+0-2	9	>20





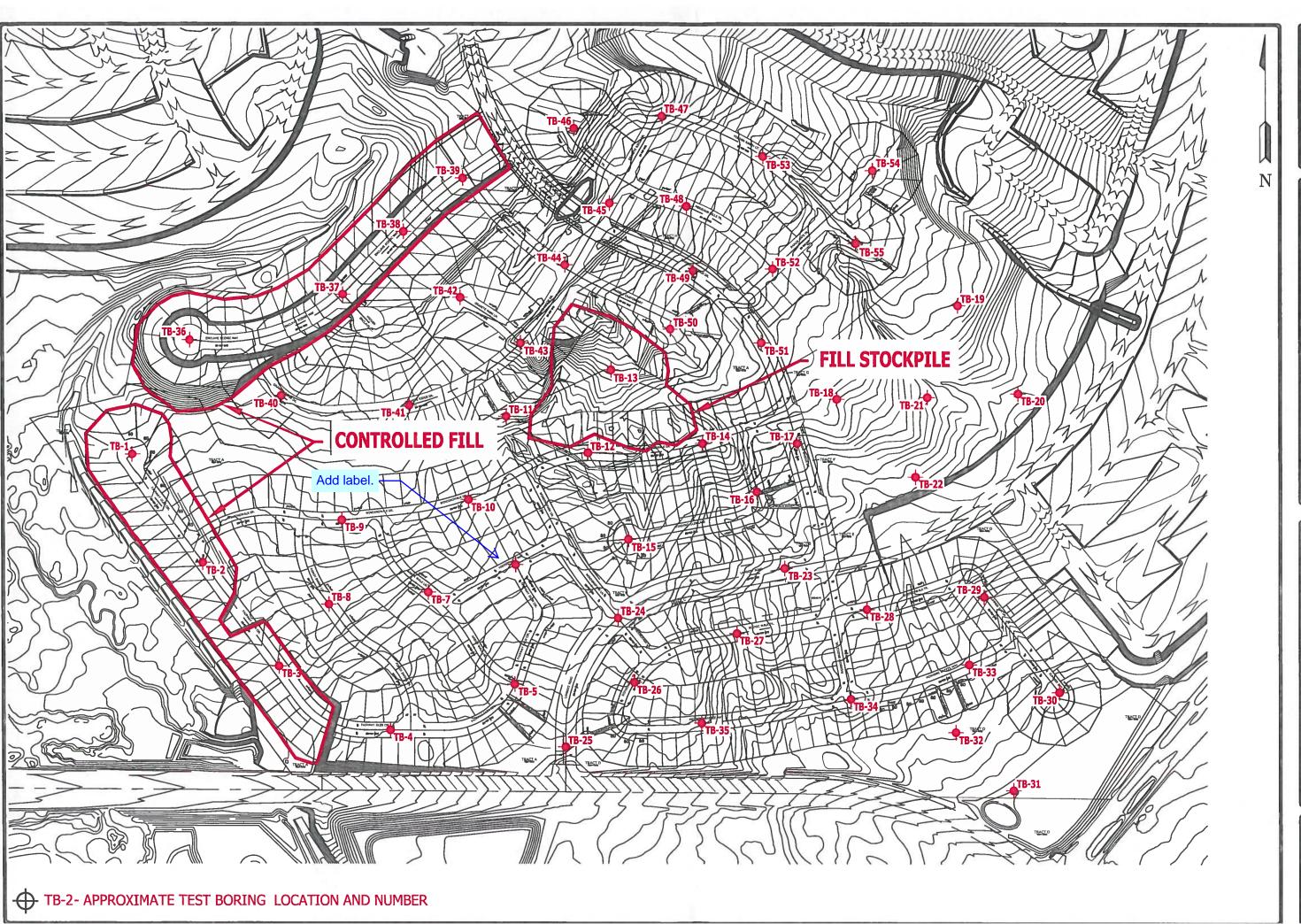


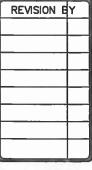
VICINITY LOCATION MAP
WINDINGWALK FILINGS 1 AND 2, AND
STONEBRIDGE THE ENCLAYE FILINGS 4 AND 5
EL PASO COUNTY, CO
FOR: TECH CONTRACTORS

DRAWN BY: TLC

DATE DRAWN: 11/08/17 DESIGNED BY: KAH CHECKED: KAH JOB NO.: 171198 FIG. NO.:

1

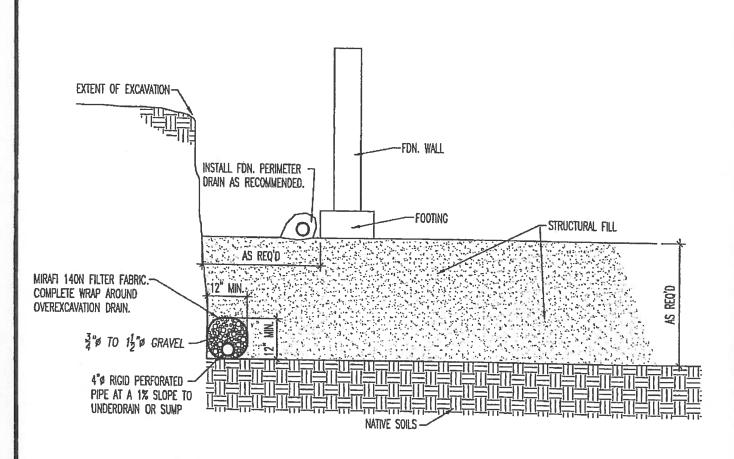




ENGINEERING, IN COLORADO SPRINGS, CO. 80807 (719) 531-55

TEST BORING LOCATION PLAN WINDINGWALK FILINGS 1 AND 2, AND STONEBRIDGE THE ENCLAVE FILINGS 4 AND EL PASO COUNTY, CO. FOR: TECH CONTRACTORS

DEAWN
TILC
CHECKED
KAH
PAIR
12/16/17
SCALE
AS SHOWN
JOB MO.
171198
FIGURE No.



OVEREXCAVATION DRAIN DETAIL

N.T.S.

NOTE:

EXTEND DRAIN TO SUMP AS REQ'D.



OVEREXCAVATION DRAIN DETAIL

DRAWN:

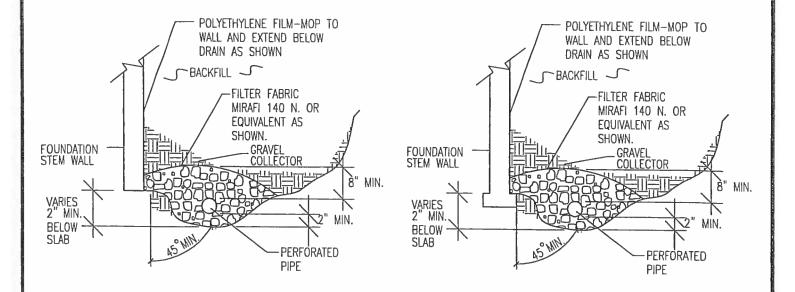
9/26/1-

DESIGNED BY: D. STECHAN

CHECKED:

JOB NO.: 171198 FIG. NO.:

3



NOTES:

- -GRAVEL SIZE IS RELATED TO DIAMETER OF PIPE PERFORATIONS-85% GRAVEL GREATER THAN 2x PERFORATION DIAMETER.
- -PIPE DIAMETER DEPENDS UPON EXPECTED SEEPAGE. 4-INCH DIAMETER IS MOST OFTEN USED.
- -ALL PIPE SHALL BE PERFORATED PLASTIC. THE DISCHARGE PORTION OF THE PIPE SHOULD BE NON-PERFORATED PIPE.
- -FLEXIBLE PIPE MAY BE USED UP TO 8 FEET IN DEPTH, IF SUCH PIPE IS DESIGNED TO WITHSTAND THE PRESSURES. RIGID PLASTIC PIPE WOULD OTHERWISE BE REQUIRED.
- -MINIMUM GRADE FOR DRAIN PIPE TO BE 1% OR 3 INCHES OF FALL IN 25 FEET.
- -DRAIN TO BE PROVIDED WITH A FREE GRAVITY OUTFALL, IF POSSIBLE. A SUMP AND PUMP MAY BE USED IF GRAVITY OUT FALL IS NOT AVAILABLE.

DRAWN:



PERIMETER	DRAIN DETAIL	

DESIGNED:

DS

DATE:

9/26/17

CHECKED:

JOB NO.: 171198

FIG NO.:

APPENDIX A: Test Boring Logs

TEST BORING NO. TEST BORING NO. DATE DRILLED 8/16/2017 DATE DRILLED 8/21/2017 Job# **TECH CONTRACTORS** 171198 CLIENT LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Watercontent Blows per Soil Type \equiv Soil Type Samples Symbol Symbol Depth WATER @ 2.5', 8/18/17 WATER @ 10.5', 9/2/17 FILL 0-3', SAND, SILTY, FINE FILL O-4', SAND, SILTY, FINE TO COARSE GRAINED, TAN, TO COARSE GRAINED, BROWN, MEDIUM DENSE, MOIST 24 10.9 1A MEDIUM DENSE, MOIST 22 8.3 1A SAND, CLAYEY, FINE TO 5 COARSE GRAINED, GREEN 18 116.4 1 SAND, SLIGHTLY SILTY, FINE 11 8.4 1 BROWN, MEDIUM DENSE, TO COARSE GRAINED, TAN, MOIST MEDIUM DENSE, MOIST SANDSTONE, SILTY, FINE TO COARSE GRAINED WITH FINE 10 50 14.1 3 10 19 7.1 1 GRAINED LENSES, GREEN BROWN, VERY DENSE, MOIST SILTSTONE, CLAYEY, SANDY, GREEN BROWN, HARD, MOIST 15 50 15.7 15]==:3 50 18.8 5 10" 10" ---F±3 <u>50</u> 13.7 3 20 <u>50</u> 5 14.1



DRAWN;	DATE:	CHECKED:	DATE: 1/9/17

TEST BORING LOG

TEST BORING NO. TEST BORING NO. DATE DRILLED 8/21/2017 DATE DRILLED 8/21/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE **REMARKS** REMARKS Watercontent Watercontent Blows per Blows per Soil Type Samples Soil Type Samples Symbol Depth (WATER @ 12', 92/17 WATER @ 4', 9/2/17 FILL O-5', SAND, CLAYEY, FINE SAND, CLAYEY, FINE TO TO COARSE GRAINED, TAN, MEDIUM GRAINED, GRAY MEDIUM DENSE, MOIST 10 11.6 1A BROWN, MEDIUM DENSE, MOIST 12 23.0 23 9.8 1A 5 23 16.7 SAND, SILTY TO SLIGHTLY SILTY, FINE TO COARSE GRAINED, DARK BROWN TO TAN, LOOSE, MOIST 10 8 3.5 10 WEATHERED TO FORMATIONAL 42 9.5 3 SANDSTONE, SILTY, FINE TO COARSE GRAINED, GRAY BROWN, DENSE TO VERY DENSE, MOIST CLAY, SANDY, DARK BROWN, 15 30 18.4 2 15 9.3 3 <u>50</u> VERY STIFF, MOIST 9" SANDSTONE, CLAYEY, FINE GRAINED, GREEN BROWN, VERY DENSE, MOIST 21.3 <u>50</u> 3 <u>50</u> 13.7



DRAWN:	DATE:	CHECKED	u PATEIN

TEST BORING LOG

TEST BORING NO. 5 TEST BORING NO. DATE DRILLED 8/21/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Watercontent Blows per Soil Type € Soil Type Samples Samples Symbol Symbol Depth (WATER @ 6', 92/17 WATER @ 4', 92/17 SAND, SILTY, FINE TO COARSE SAND, SILTY, FINE TO COARSE GRAINED, GREEN BROWN, GRAINED, GREEN BROWN, MEDIUM DENSE, MOIST 7.7 12 1 MEDIUM DENSE, MOIST 12 5.3 1 12.7 SANDSTONE, SILTY, FINE TO <u>50</u> 3 22 7.8 1 11" COARSE GRAINED, GRAY BROWN, VERY DENSE, MOIST CLAYSTONE, SANDY TO CLAYSTONE, SANDY, BROWN VERY SANDY, GRAY BROWN, TO BLUE GRAY, HARD, MOIST HARD, MOIST 4 10 16.9 <u>50</u> 10 <u>50</u> 14.9 11" 5" 15 17.4 15 <u>50</u> 4 <u>50</u> 12.9 5" <u>50</u> 13.9 <u>50</u> 7" 20 15.8



	TEST	BORING LOG	ì
DRAWN:	DATE:	CHECKED:	11/6/17

TEST BORING NO. 7 TEST BORING NO. 8 DATE DRILLED 8/21/2017 DATE DRILLED 8/25/2017 CLIENT Job# 171198 **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Watercontent Blows per Soil Type Soil Type Depth (ft) Samples Symbol Symbol WATER @ 8.5', 92/17 WATER @ 6', 92/17 <u> 71.</u> SAND, SILTY, TAN SAND, SLIGHTLY SILTY TO SANDSTONE, SILTY, FINE SILTY, FINE TO COARSE TO COARSE GRAINED, GREEN 50 4.2 3 GRAINED, TAN, MEDIUM 20 2.1 1 BROWN TO TAN, VERY DENSE, DENSE TO DENSE, MOIST MOIST TO WET <u>50</u> 8.7 3 25 10.1 1 7" 10 3 <u>50</u> 9.1 10 40 | 11.8 6" CLAYSTONE, SANDY, GRAY BROWN, HARD, MOIST 3 15 <u>50</u> 8.9 15 <u>50</u> 12.8 10" <u>50</u> 7" 20.6 3 WEATHERED ZONE 20 40 16.4



	TE	ST BORING LOG
DRAWN:	DATE;	CHECKED: USATE:

TEST BORING NO. TEST BORING NO. 10 DATE DRILLED 8/25/2017 DATE DRILLED 8/24/2017 Job# 171198 CLIENT TECH CONTRACTORS LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Natercontent % Blows per foot Blows per foot Watercontent Soil Type - Soil Type Samples Symbol Symbol WATER @ 11.5', 9/2/17 WATER @ 9', 9/2/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY COARSE GRAINED WITH FINE DENSE, MOIST <u>50</u> 7.1 GRAINED LENSES, TAN TO <u>50</u> 7.7 3 6" GRAY BROWN, VERY DENSE, 10" MOIST 10 <u>50</u> 8.8 3 10 3 <u>50</u> 13.2 10" 8" 15 9.4 3 <u>50</u> 15 50 11.0 6" CLAYSTONE, SANDY, GRAY <u>50</u> 15.9 <u>50</u> 7" 9.1 3 BROWN, HARD, MOIST



	TE	ST BORING LO	G
DRAWN:	DATE:	CHECKED:	DATE: 4/8/17

TEST BORING NO. TEST BORING NO. 11 12 DATE DRILLED 8/24/2017 DATE DRILLED 8/30/2017 Job# CLIENT 171198 **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Watercontent Blows per Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol WATER @ 15', 9/2/17 WATER @ 13', 9/2/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SILTY, FINE TO COARSE GRAINED, GRAY <u>50</u> 5.4 3 COARSE GRAINED, GRAY 50 9.4 3 10" BROWN TO TAN, VERY DENSE, BROWN, VERY DENSE, MOIST MOIST <u>50</u> 10.0 3 <u>50</u> 6.6 3 8" 7" 10 50 7.8 3 10 <u>50</u> 8.2 3 8" 15 15 <u>50</u> 11.2 3 <u>50</u> 9.7 3 6" SANDSTONE, CLAYEY TO VERY CLAYEY, FINE GRAINED, TAN, VERY DENSE, MOIST 20 <u>50</u> 11.4 3 20 <u>50</u> 3 10.1 <u>50</u> 11" 19.4 3



	TES	ST BORING LO	g
DRAWN:	DATE:	CHECKED:	DATE:

TEST BORING NO. 13 TEST BORING NO. 14 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent Watercontent Blows per Blows per Soil Type Samples Soil Type Symbol Symbol DRY TO 25', 8/29/17 WATER @ 20.5', 9/2/17 FILL O-6', SAND, CLAYEY TO SAND, SILTY, TAN VERY CLAYEY, FINE TO COARSE GRAINED, GREEN 5 11.6 1A SANDSTONE, CLAYEY TO BROWN, LOOSE TO MEDIUM VERY CLAYEY WITH SILTY DENSE, MOIST 18 15.1 1A LENSES, FINE TO COARSE 5 <u>50</u> 7.2 3 9" GRAINED, TAN, VERY DENSE, SANDSTONE, SILTY, FINE TO MOIST COARSE GRAINED, GREEN BROWN, VERY DENSE, MOIST 3 10 <u>50</u> 7.0 10 50 10.3 8" 15 <u>50</u> 7.3 3 15 <u>50</u> 8.5 3 6" 6" SANDSTONE, SILTY, FINE 20 3 GRAINED, TAN, VERY DENSE, <u>50</u> 9.6 20 10.9 3 <u>50</u> MOIST <u>50</u> 9.4 3 CLAYSTONE, SANDY, BLUE 25 <u>50</u> 13.3 GRAY, HARD, MOIST SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY <u>50</u> 12.8 3 DENSE, MOIST

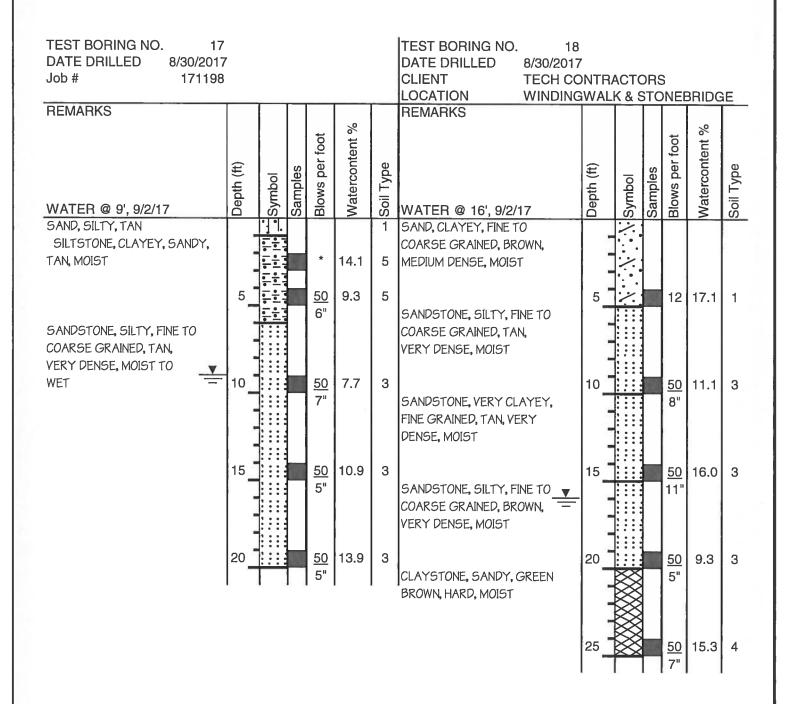


TEST BORING LOG				
DRAWN:	DATE:	CHECKED:	DATE:/17	

TEST BORING NO. 15 TEST BORING NO. 16 DATE DRILLED 8/21/2017 DATE DRILLED 8/25/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS **REMARKS** Watercontent % Blows per foot Blows per foot Watercontent Soil Type Depth (ft) Soil Type Samples Depth (ft) Samples Symbol Symbol DRY TO 20', 8/21/17 WATER @ 7.5', 9/2/17 SAND, SILTY, FINE TO COARSE SAND, SILTY, TAN GRAINED, TAN, MEDIUM SANDSTONE, SILTY, FINE DENSE, MOIST 27 5.9 1 TO COARSE GRAINED, TAN, 50 6.0 3 VERY DENSE, MOIST 5 24 8.6 1 5 50 3 5.4 7" SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY DENSE, MOIST 10 3 <u>50</u> 7.8 10 <u>50</u> 9.4 3 6" 5" 15 15 <u>50</u> 12.0 3 10.7 <u>50</u> 3 8" 8" <u>50</u> 7" 10.0 3 CLAYSTONE, VERY SANDY, 20 <u>50</u> 15.2 BLUE GRAY, HARD, MOIST



TEST BORING LOG					
DRAWN:	DATE:	CHECKED:	DATE:		





	TEST BORING LOG			
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17	

TEST BORING NO. 19 TEST BORING NO. 20 DATE DRILLED 8/30/2017 DATE DRILLED 8/30/2017 Job# 171198 **CLIENT TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS **REMARKS** Blows per foot Watercontent Watercontent Blows per Soil Type Soil Type Samples Depth (ft) Symbol Symbol WATER @ 6.5', 9/2/17 DRY TO 20', 9/2/17 SAND, SLIGHTLY SILTY, FINE SAND, SILTY, FINE TO COARSE TO COASE GRAINED, TAN, GRAINED, TAN, MEDIUM MEDIUM DENSE, MOIST DENSE TO DENSE, MOIST 27 3.1 1 22 8.2 1 30 3.9 1 SANDSTONE, SILTY, FINE TO 7.7 COARSE GRAINED, TAN, VERY 10 <u>50</u> SANDSTONE, SILTY, FINE TO 10 3 <u>50</u> 8.6 DENSE, MOIST COARSE GRAINED, TAN, 6" VERY DENSE, MOIST 5.7 3 15 <u>50</u> 15 <u>50</u> 7.3 3 3" 6" SANDSTONE, SILTY, FINE 3 <u>50</u> 9.4 <u>50</u> 3 20 6.3 GRAINED, TAN, VERY DENSE, MOIST



			-
DRAWN:	DATE:	CHECKED:	UB/17

TEST BORING LOG

TEST BORING NO. TEST BORING NO. 21 22 DATE DRILLED 8/30/2017 DATE DRILLED 8/30/2017 Job# CLIENT 171198 **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Watercontent Blows per Soil Type Samples Depth (ft) Soil Type Samples Symbol Symbol WATER @ 12.5', 9/2/17 WATER @ 14', 9/2/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY COARSE GRAINED, TAN, VERY 50 DENSE, MOIST 5 9.6 3 DENSE, MOIST TO WET 5 <u>50</u> 9.1 3 9" 3 10 <u>50</u> 7.2 10 <u>50</u> 8.4 3 9" 6" <u>50</u> 15 8.7 3 15 <u>50</u> 8.5 3 6" <u>50</u> 3 11.5 20 <u>50</u> 3 14.9



TEST BORING LOG				
DRAWN:	DATE:	CHECKED:	PATE:	

TEST BORING NO. 23 TEST BORING NO. 24 DATE DRILLED 8/30/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Watercontent Blows per Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol WATER @ 7.5', 9/2/17 WATER @ 11', 9/2/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, CLAYEY TO SANDSTONE, SILTY, FINE 8.5 VERY CLAYEY, FINE GRAINED, 3 TO COARSE GRAINED, TAN, 3 50 8.1 GREEN BROWN, VERY DENSE, VERY DENSE, MOIST 9" 3 MOIST <u>50</u> 8.2 9.2 3 <u>50</u> 6" 10 <u>50</u> 3 10 <u>50</u> 9.3 3 2" 8" 15 17.3 15 3 <u>50</u> 3 50 8.4 6" <u>50</u> 12.4 3 CLAYSTONE, SANDY, BLUE 20 13.5 <u>50</u> GRAY, HARD, MOIST SANDSTONE, CLAYEY, FINE TO 25 <u>50</u> 3 11.7 COARSE GRAINED, BLUE GRAY, VERY DENSE, MOIST



i	TE	ST BORING LOG	
DRAWN:	DATE:	CHECKED:	DATE: 1/8/17

TEST BORING NO. 25 TEST BORING NO. 26 DATE DRILLED 8/29/2017 DATE DRILLED 8/21/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Blows per foot Watercontent Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol WATER @ 9', 9/2/17 WATER @ 7.5', 9/2/17 SAND, SILTY, FINE TO COARSE SAND, SILTY, CLAYEY, FINE TO MEDIUM GRAINED, TAN, GRAINED, TAN, MEDIUM DENSE, 25 14.5 MOIST MEDIUM DENSE TO VERY 1 22 4.5 DENSE, MOIST 50 14.3 SANDSTONE, SILTY, FINE <u>50</u> 6.7 3 9" TO COARSE GRAINED, GREEN BROWN, VERY DENSE, MOIST CLAY, VERY SANDY, TAN, 10 32 10.7 2 10 CLAYSTONE, SANDY, DARK 50 15.3 VERY STIFF, MOIST BROWN, HARD, MOIST SANDSTONE, SILTY, FINE 15 9.2 TO COARSE GRAINED, GRAY 50 SANDSTONE, CLAYEY, FINE 15 11.0 3 <u>50</u> BROWN, VERY DENSE, MOIST 5" 5" GRAINED, GRAY BROWN, VERY DENSE, MOIST TO WET 20 <u>50</u> 13.5 3 <u>50</u> 18.5 3 6" 25 15.4 3 <u>50</u> <u>50</u> 12.8 3



	TE	ST BORING LO	G	
DRAWN:	DATE:	CHECKED	DATE:	

TEST BORING NO. 27 TEST BORING NO. 28 DATE DRILLED 8/21/2017 DATE DRILLED 8/30/2017 Job# **TECH CONTRACTORS** 171198 CLIENT LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Blows per foot Watercontent Soil Type Soil Type Depth (ft) Samples Samples Symbol Symbol WATER @ 4', 9/2/17 WATER @ 11', 9/2/17 SAND, CLAYEY, FINE TO CLAY, SANDY, TAN, STIFF, COARSE GRAINED, GREEN MOIST BROWN, MEDIUM DENSE, 14 10.5 21 17.8 2 MOIST SANDSTONE, SILTY, FINE TO CLAY, SANDY, BROWN, 10 119.5 COARSE GRAINED, GRAY <u>50</u> 6.6 3 FIRM, MOIST BROWN, VERY DENSE, MOIST 8" TO WET SANDSTONE, CLAYEY, FINE TO COARSE GRAINED, GRAY BROWN, VERY DENSE, MOIST 10 14.9 3 <u>50</u> 10 50 8.2 3 6" 8" 15 50 17.6 3 15 50 12.0 3 5" 10" <u>50</u> 12.5 20 <u>50</u> 3 8.5



	TE	ST BORING LO	og]
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TEST BORING NO. 29 TEST BORING NO. 30 DATE DRILLED 8/30/2017 DATE DRILLED 8/30/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE **REMARKS** REMARKS Watercontent % Blows per foot Blows per foot Watercontent Soil Type Soil Type Depth (ft) Samples Samples Symbol Symbol DRY TO 19.5', 9/2/17 WATER @ 19', 9/2/17 SAND, SILTY, TAN SAND, SILTY, FINE TO COARSE SANDSTONE, SILTY, FINE GRAINED, GREEN BROWN, TO COARSE GRAINED, GREEN 6.9 3 MOIST 8.7 BROWN, VERY DENSE, MOIST SANDSTONE, SILTY, FINE TO <u>50</u> 10.6 COARSE GRAINED, GREEN <u>50</u> 13.9 3 BROWN, VERY DENSE, MOIST 10" 10 8.6 3 10 <u>50</u> CLAYEY LENSES 50 15.4 3 15 <u>50</u> 8.8 3 15 <u>50</u> 5.9 3 6" * - BULK SAMPLE TAKEN <u>50</u> 8.3 3 * - BULK SAMPLE TAKEN <u>50</u> 9.8 3



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TEST BORING NO. 31 TEST BORING NO. 32 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Watercontent Blows per Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol WATER @ 12.5', 9/2/17 WATER @ 15.5', 9/2/17 SAND, SILTY, TAN SAND, SILTY, FINE TO MEDIUM SANDSTONE, SLIGHTLY GRAINED, TAN, MEDIUM SILTY TO SILTY, FINE TO 50 4.8 3 DENSE TO DENSE, MOIST 15 5.7 1 COARSE GRAINED, TAN, VERY DENSE, MOIST TO WET <u>50</u> 4.8 3 33 9.3 1 7" SANDSTONE, SILTY TO VERY SILTY, FINE TO MEDIUM 10 <u>50</u> 10.6 3 GRAINED, TAN, VERY DENSE, 10 <u>50</u> 11.7 11" MOIST TO WET **V** 15 50 20.5 3 15 <u>50</u> 11.5 3 10" 9" 20 <u>50</u> 14.6 3 20 <u>50</u> 17.3 3 8" <u>50</u> 16.6 3 25 <u>50</u> 14.9 3 6" 30 <u>50</u> 19.2 3



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TEST BORING NO. TEST BORING NO. 33 34 DATE DRILLED 8/30/2017 DATE DRILLED 8/30/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS **REMARKS** Watercontent Watercontent Blows per Blows per Soil Type Depth (ft) Soil Type Samples Samples Symbol DRY TO 19.5', 9/2/17 WATER @ 14.5', 9/2/17 ۲. SAND, CLAYEY, FINE TO COARSE SAND, SILTY, FINE GRAINED GRAINED, TAN, MEDIUM DENSE, WITH COARSE GRAINED ٠٠. MOIST 19 9.9 1 LENSES, TAN, MEDIUM DENSE, 8.8 1 MOIST SANDSTONE, SILTY, FINE TO 50 8.9 3 11.2 1 17 COARSE GRAINED, TAN, VERY DENSE, MOIST SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN TO GREEN BROWN, VERY DENSE, 10 8.8 3 <u>50</u> MOIST 10 3 50 10.6 9" 8" 3 15 6.2 <u>50</u> 15 <u>50</u> 9.1 3 6" * - BULK SAMPLE TAKEN 20 <u>50</u> 7.3 3 SANDSTONE, CLAYEY, FINE 20 11.9 3 <u>50</u> 6" GRAINED, GREEN BROWN, VERY DENSE, MOIST



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~ 11/2/17	DRAWN:	DATE:	CHECKED:	DATE: 11/8/17

TEST BORING NO. 35 TEST BORING NO. 36 DATE DRILLED 8/29/2017 DATE DRILLED 8/16/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Blows per foot Watercontent Soil Type Soil Type Depth (ft) Samples Symbol Symbol WATER @ 15', 9/2/17 WATER @ 17.5', 8/16/17 SAND, SILTY, FINE TO COARSE FILL O-14', SAND, SILTY, FINE GRAINED, BROWN TO TAN, TO COARSE GRAINED, BROWN, MEDIUM DENSE, MEDIUM DENSE TO DENSE, 15 8.7 1 18 11.9 1A MOIST MOIST 38 16.9 1 19 8.4 1A SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY DENSE, MOIST TO WET 10 9.3 3 10 23 <u>50</u> 9.4 1A 8" 15 50 9.2 3 SAND, SILTY, FINE TO COARSE 15 22 8.5 GRAINED, DARK BROWN TO GREEN BROWN, MEDIUM DENSE, MOIST TO WET 20 <u>50</u> 9.1 3 20 24 9.9 1 <u>50</u> 14.3 3



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TEST BORING LOG

TEST BORING NO. TEST BORING NO. 37 38 DATE DRILLED 8/16/2017 DATE DRILLED 8/16/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Watercontent Blows per Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol DRY TO 19', 8/18/17 DRY TO 19.5', 8/18/17 FILL O-8', SAND, SILTY TO FILL 0-6', SAND, SILTY, FINE CLAYEY, FINE TO COARSE TO COARSE GRAINED, BROWN, 22 GRAINED, BROWN, MEDIUM 8.3 1A DENSE TO MEDIUM DENSE, 34 6.3 1A DENSE, MOIST MOIST 28 10.7 1A 5 16 9.0 1A SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, SAND, SILTY, FINE TO COARSE VERY DENSE, MOIST TO GRAINED, BROWN, MEDIUM 10 16 11.6 1 WET 10 <u>50</u> 8.4 3 DENSE, MOIST 5" SANDSTONE, SILTY, FINE TO 50 12.0 3 15 <u>50</u> 9.8 3 COARSE GRAINED, BROWN, 5" VERY DENSE, MOIST <u>50</u> 5.8 3 20 <u>50</u> 14.8 3



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TEST BORING NO. 39 TEST BORING NO. 40 DATE DRILLED 8/16/2017 DATE DRILLED 8/16/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE **REMARKS REMARKS** Watercontent % Blows per foot Watercontent Blows per i Soil Type Depth (ft) Samples Soil Type Samples Symbol Symbol WATER @ 6', 8/18/17 WATER @ 15', 8/18/17 FILL O-7', SAND, CLAYEY TO SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SILTY, FINE TO COARSE GRAINED, BROWN, MEDIUM 21 10.8 1A COARSE GRAINED, BROWN TO 50 6.2 3 DENSE, MOIST GRAY BROWN, VERY DENSE, 10' 15 10.2 1A MOIST <u>50</u> 7.1 3 10' SAND, SILTY, FINE TO COARSE GRAINED, DARK BROWN, MEDIUM DENSE, MOIST 10 20 8.5 1 10 <u>50</u> 9.1 3 SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN. 15 VERY DENSE, MOIST 7.1 <u>50</u> 3 15 <u>50</u> 9.4 3 6" SANDSTONE, SILTY, FINE <u>50</u> 14.8 20 <u>50</u> 10.0 3 GRAINED, TAN, VERY DENSE, MOIST



TEST BORING LOG				
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TEST BORING NO. TEST BORING NO. 41 42 DATE DRILLED 8/16/2017 DATE DRILLED 8/16/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Watercontent Blows per Blows per Soil Type Soil Type Samples Depth (ft) Samples Symbol WATER @ 15.5', 8/18/17 WATER @ 18', 8/18/17 SAND, SILTY, TAN SAND, SILTY, FINE TO COARSE SANDSTONE, SILTY, FINE GRAINED, TAN, MEDIUM DENSE, TO COARSE GRAINED, BROWN, 6.0 MOIST <u>50</u> 3 9.7 26 9" VERY DENSE, MOIST TO WET 50 8.1 SANDSTONE, SILTY, FINE TO 50 8.9 3 COARSE GRAINED, BROWN, VERY DENSE, MOIST 10 10 <u>50</u> 10.5 3 <u>50</u> 9.1 3 15 8.5 3 <u>50</u> 15 <u>50</u> 12.7 3 SANDSTONE, VERY SILTY, <u>50</u> 13.9 FINE GRAINED, TAN, VERY <u>50</u> 8.8 DENSE, MOIST



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TEST BORING NO. TEST BORING NO. 43 44 DATE DRILLED 8/16/2017 DATE DRILLED 8/16/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Watercontent Blows per Blows per Soil Type Soil Type Depth (ft) Samples Samples Symbol Symbol DRY TO 18.5', 8/18/17 WATER @ 20', 8/18/17 SAND, SILTY, TAN SAND, SILTY, TAN CLAYSTONE, VERY SANDY, WEATHERED TO FORMATIONAL 37 8.8 3 BLUE GRAY, HARD, MOIST 4 <u>50</u> 16.1 SANDSTONE, SILTY, FINE 9" 3 TO COARSE GRAINED, GREEN <u>50</u> 7.8 50 10.0 BROWN, DENSE TO VERY 8" DENSE, MOIST CLAYSTONE, VERY SANDY, SANDSTONE, SILTY, FINE TO 10 GREEN BROWN, HARD, MOIST <u>50</u> 15.3 COARSE GRAINED, TAN TO 10 <u>50</u> 7.0 3 GREEN BROWN, VERY DENSE, SANDSTONE, SILTY, FINE TO MOIST COARSE GRAINED, TAN, VERY DENSE, MOIST 15 3 <u>50</u> 6.3 15 <u>50</u> 3 8.4 6" <u>50</u> 3 8.4 <u>50</u> 12.5 3



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TEST BORING NO. TEST BORING NO. 45 46 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Blows per foot Watercontent ' Soil Type Soil Type Samples Depth (ft) Samples Symbol Symbol WATER @ 18', 9/2/17 DRY TO 19', 9/2/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY TO SANDSTONE, VERY CLAYEY, SLIGHTLY SILTY, FINE TO 6.6 <u>50</u> 3 FINE GRAINED, BROWN, VERY 9.9 3 10" COARSE GRAINED, TAN, DENSE, MOIST VERY DENSE, MOIST <u>50</u> 5.7 3 5 11.9 3 <u>50</u> 9" 6" 10 8.0 3 <u>50</u> 10 SANDSTONE, SILTY, FINE TO 7.4 3 50 COARSE GRAINED, BROWN, 4" CLAYSTONE, SANDY, GREEN VERY DENSE, MOIST BROWN, HARD, MOIST 15 50 12.2 4 15 7.9 3 <u>50</u> 6" 6" SANDSTONE, SILTY, FINE TO ____ COARSE GRAINED, BROWN, VERY DENSE, MOIST 3 <u>50</u> 7.5 20 <u>50</u> 3 9.7



TEST BORING LOG				
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TEST BORING NO. 47 TEST BORING NO. 48 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Watercontent Blows per Soil Type Soil Type Samples Depth (ft) Samples Symbol Symbol DRY TO 19', 9/2/17 WATER @ 18', 9/2/17 SAND, SILTY, FINE TO COARSE SAND, SILTY, TAN GRAINED, GREEN BROWN, SANDSTONE, SLIGHTLY MEDIUM DENSE, MOIST 28 5.3 SILTY TO SILTY, FINE TO 50 6.7 3 COARSE GRAINED, TAN TO 10" SANDSTONE, SILTY, FINE TO 50 10.6 3 GREEN BROWN, VERY DENSE, 50 7.1 3 COARSE GRAINED, TAN TO 8" MOIST 6" GREEN BROWN, VERY DENSE, MOIST 10 <u>50</u> 5.1 3 10 <u>50</u> 6.2 3 6" 15 50 4.6 3 15 <u>50</u> 7.0 3 6" <u>50</u> 7.7 3 20 3 <u>50</u> 9.0



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TEST BORING NO. 49 TEST BORING NO. 50 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 **CLIENT TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Blows per foot Watercontent Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol DRY TO 20', 9/2/17 DRY TO 20', 8/29/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SILTY, FINE TO COARSE GRAINED WITH FINE <u>50</u> 7.8 3 COARSE GRAINED, TAN TO 3 <u>50</u> 4.8 11" GRAINED LENSES, TAN, VERY GREEN BROWN, VERY DENSE, 11' DENSE, MOIST 5 <u>50</u> 8.2 3 MOIST <u>50</u> 8.4 3 10" 10" 3 10 <u>50</u> 7.1 10 <u>50</u> 5.2 3 5" 6" 15 <u>50</u> 7.6 3 15 3 <u>50</u> 8.6 8" 6" <u>50</u> 7" 3 <u>50</u> 7" 12.2 20 8.8 3



TEST BORING LOG				
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TEST BORING NO. 51 TEST BORING NO. 52 8/29/2017 DATE DRILLED DATE DRILLED 8/29/2017 Job# 171198 **CLIENT TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Blows per foot Watercontent Soil Type Samples Soil Type Depth (ft) Symbol Samples Symbol DRY TO 18.5', 9/2/17 DRY TO 20', 8/29/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SLIGHTLY COARSE GRAINED, TAN TO 50 4.6 SILTY TO SILTY, FINE TO 10.1 GREEN BROWN, VERY DENSE, COARSE GRAINED, TAN TO MOIST <u>50</u> 6.2 GREEN BROWN, VERY DENSE, <u>50</u> 5.2 3 9" MOIST 7" 10 10 <u>50</u> 5.6 3 <u>50</u> 5.2 3 5" 15 <u>50</u> 8.5 3 15 <u>50</u> 5.5 6" <u>50</u> 7.6 3 * - BULK SAMPLE TAKEN 20 <u>50</u> 9.5 3



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TEST BORING NO. 53 TEST BORING NO. 54 DATE DRILLED 8/29/2017 DATE DRILLED 8/30/2017 Job# 171198 CLIENT **TECH CONTRACTORS** WINDINGWALK & STONEBRIDGE LOCATION REMARKS REMARKS Blows per foot Watercontent Blows per foot **Natercontent** Soil Type Samples Depth (ft) Soil Type Samples Symbol Symbol DRY TO 19.5', 9/2/17 WATER @ 18.5', 9/2/17 SAND, SILTY, TAN SAND, SILTY, FINE TO COARSE SANDSTONE, SILTY, FINE GRAINED, TAN, MEDIUM DENSE GRAINED, TAN, VERY DENSE, 20.9 3 TO DENSE, MOIST MOIST 10.0 3 <u>50</u> 26 4.3 1 10 <u>50</u> 12.3 3 10 43 9.4 1 6" SANDSTONE, SILTY, FINE TO COARSE GRAINED, GREEN BROWN, VERY DENSE, MOIST 15 <u>50</u> 7.3 3 15 CLAYSTONE, SANDY, TAN, 50 11.4 4 HARD, MOIST SANDSTONE, SILTY, FINE * - BULK SAMPLE TAKEN <u>50</u> 7.0 3 TO COARSE GRAINED, TAN, 20 <u>50</u> 8.3 3 VERY DENSE, MOIST



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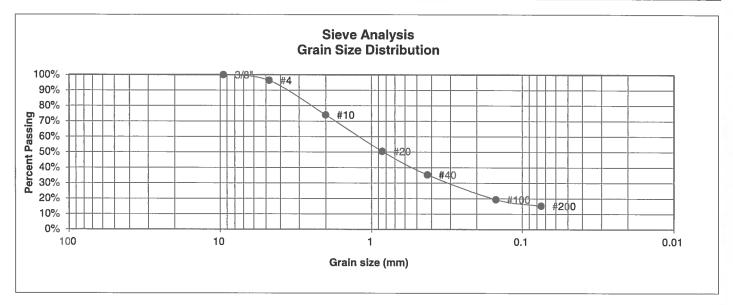
TEST BORING NO. 55 TEST BORING NO. DATE DRILLED 8/30/2017 DATE DRILLED Job# 171198 **CLIENT TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS **REMARKS** Watercontent % Watercontent % Blows per foot Blows per Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol DRY TO 20', 9/2/17 SAND, SLIGHTLY SILTY TO SILTY, FINE TO COARSE GRAINED, TAN, MEDIUM DENSE 12 2.7 1 TO DENSE, MOIST 5 40 6.9 1 SANDSTONE, SILTY, FINE TO 10 3 <u>50</u> 8.4 10 COARSE GRAINED, TAN, VERY 8' DENSE, MOIST 15 <u>50</u> 8.8 3 15 9" <u>50</u> 7" 3 7.0 20



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APPENDIX B: Laboratory Testing Results

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1A	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	36	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	96.3%	Swell
10	74.1%	Moisture at start
20 40	50.6% 35.4%	Moisture at finish Moisture increase
100 200	19.4% 15.3%	Initial dry density (pcf) Swell (psf)

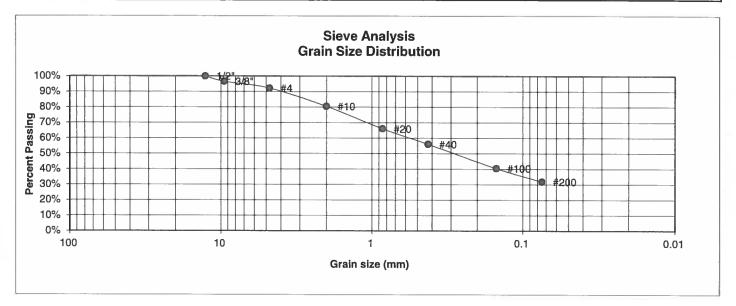


LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	h	DATE: 4/8/17	

JOB NO.: 171198 FIG NO.:

13-1

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1A	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	37	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



U.S.	Percent	Atterberg	
Sieve #	<u>Finer</u>	<u>Limits</u>	
3"		Plastic Limit	
1 1/2"		Liquid Limit	
3/4"		Plastic Index	
1/2"	100.0%		
3/8"	96.6%		
4	92.2%	<u>Swell</u>	
10	80.5%	Moisture at start	7.6%
20	66.0%	Moisture at finish	16.3%
40	56.1%	Moisture increase	8.8%
100	40.4%	Initial dry density (pcf)	108
200	31.8%	Swell (psf)	240

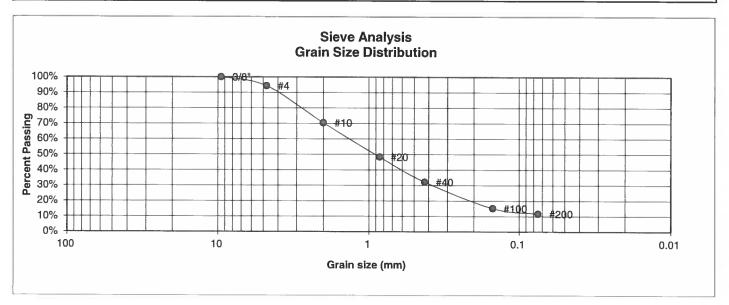


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JOB NO.: 171198 FIG NO.:

B-2

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	2	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



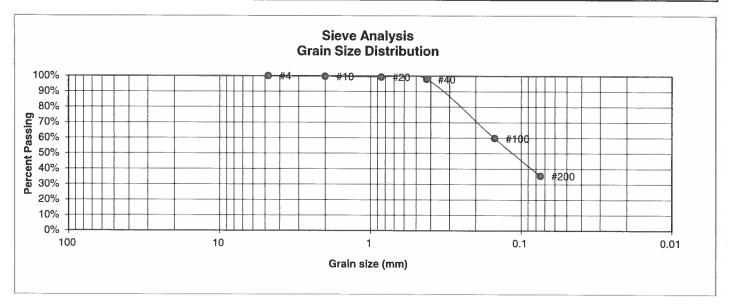
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	94.3%	<u>Swell</u>
10	70.4%	Moisture at start
20	48.3%	Moisture at finish
40	32.2%	Moisture increase
100	15.2%	Initial dry density (pcf)
200	11.7%	Swell (psf)



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UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	4	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



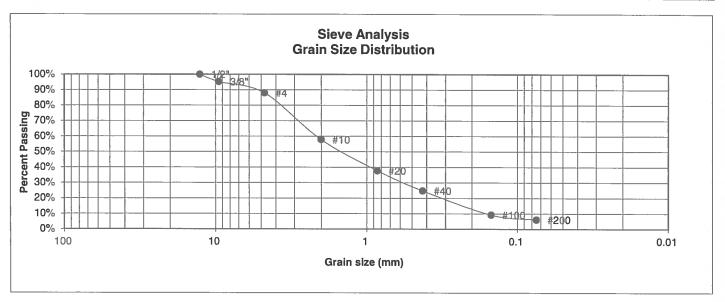
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Liquid Limit 2	0 9 9
4	100.0%	Swell	
10	99.7%	Moisture at start	
20	99.3%	Moisture at finish	
40	97.9%	Moisture increase	
100	59.8%	Initial dry density (pcf)	
200	35.5%	Swell (psf)	



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FIG NO.: B-4

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	8	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



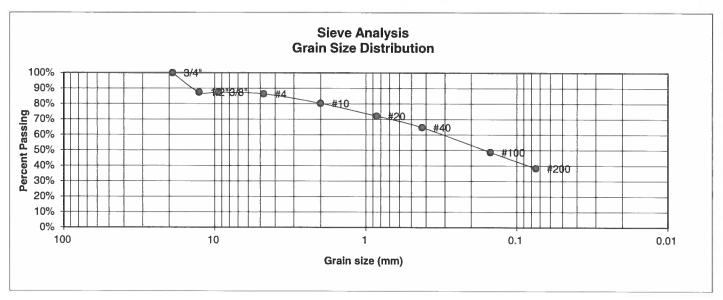
U.S. Sieve # 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
1/2"	100.0%	
3/8"	95.2%	
4	88.0%	<u>Swell</u>
10	57.8%	Moisture at start
20	37.8%	Moisture at finish
40	24.8%	Moisture increase
100	9.2%	Initial dry density (pcf)
200	6.1%	Swell (psf)



	LABOI RESUI	RATORY T LTS	EST	
DRAWN:	DATE	CHECKED:	n	DATE: 11/8/17

FIG NO.:

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	13	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

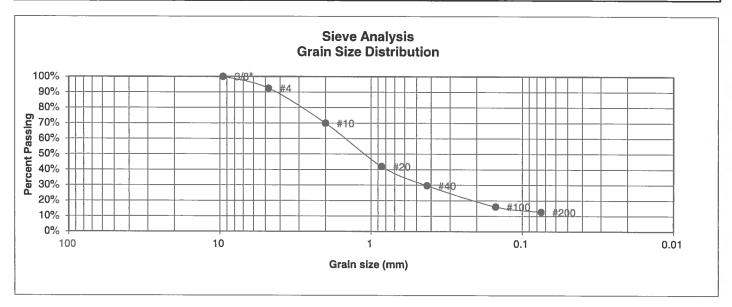


U.S.	Percent	Atterberg
Sieve #	<u>Finer</u>	<u>Limits</u>
3"		Plastic Limit 13
1 1/2"		Liquid Limit 28
3/4"	100.0%	Plastic Index 15
1/2"	87.4%	
3/8"	87.4%	
4	86.4%	<u>Swell</u>
10	80.3%	Moisture at start
20	72.1%	Moisture at finish
40	64.7%	Moisture increase
100	48.8%	Initial dry density (pcf)
200	38.4%	Swell (psf)



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UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	15	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



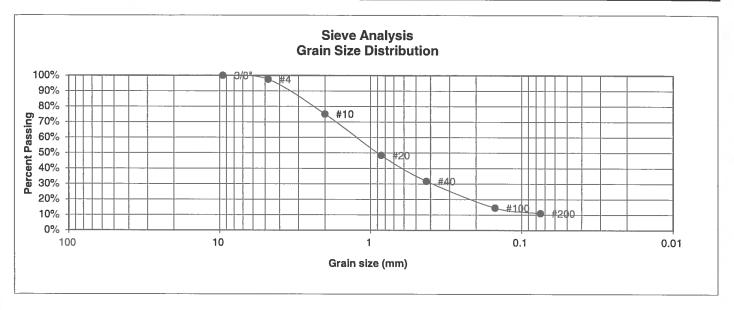
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	
4	92.4%	<u>Swell</u>
10	69.9%	Moisture at start
20	42.0%	Moisture at finish
40	29.6%	Moisture increase
100 200	16.1% 12.6%	Initial dry density (pcf) Swell (psf)



	LABOI RESUI	RATORY T	EST	
DRAWN:	DATE:	CHECKED:	a	DATE: 11/8/17

FIG NO.: 3-7

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	19	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

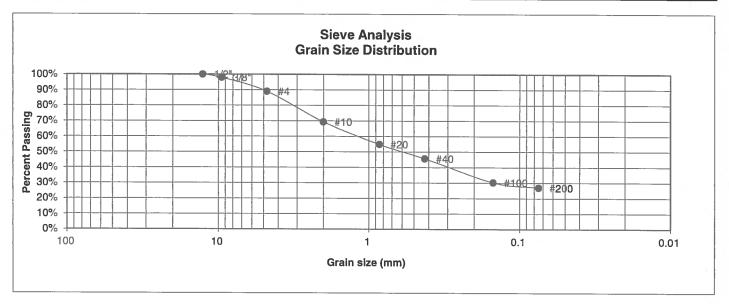


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP	
4	97.5%	<u>Swell</u>	
10	75.1%	Moisture at start	
20	48.5%	Moisture at finish	
40	31.8%	Moisture increase	
100	14.5%	Initial dry density (pcf)	
200	11.0%	Swell (psf)	



	LABO! RESU	RATORY TEST LTS	•
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	30	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



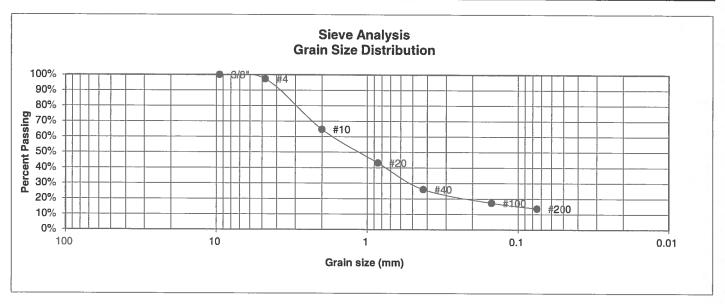
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0% 97.9%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4 10 20 40 100 200	89.0% 69.3% 54.8% 45.7% 30.3% 26.9%	Swell Moisture at start Moisture at finish Moisture increase Initial dry density (pcf) Swell (psf)



	RESULT	ATORY T	EST	
DRAWN:	DATE;	CHECKED:	A	DATE: 1/8/17



UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	34	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

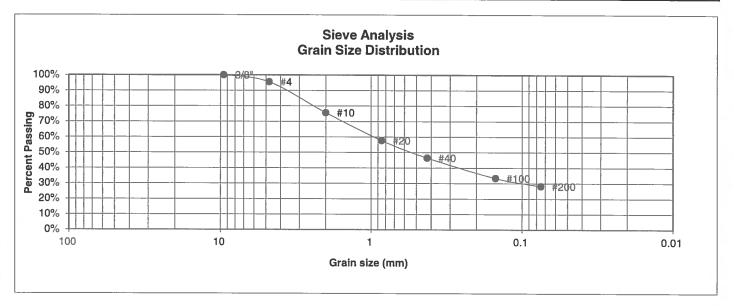


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	97.3%	<u>Swell</u>
10	64.7%	Moisture at start
20	43.1%	Moisture at finish
40	26.0%	Moisture increase
100	17.3%	Initial dry density (pcf)
200	13.7%	Swell (psf)



	LABOI RESU	RATORY T LTS	EST		
DRAWN:	DATE:	CHECKED	n	1/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	35	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

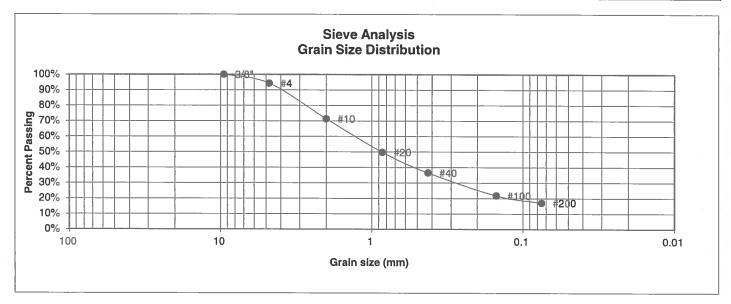


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
3/8"	100.0%	
4	95.5%	Swell
10	75.6%	Moisture at start
20	57.7%	Moisture at finish
40	46.5%	Moisture increase
100 200	33.4% 28.2%	Initial dry density (pcf) Swell (psf)



	LABOI RESU	RATORY T LTS	EST		
DRAWN:	DATE:	CHECKED:	h	1/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	39	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	94.3%	<u>Swell</u>
10	71.3%	Moisture at start
20	49.8%	Moisture at finish
40	36.5%	Moisture increase
100	21.8%	Initial dry density (pcf)
200	17.1%	Swell (psf)

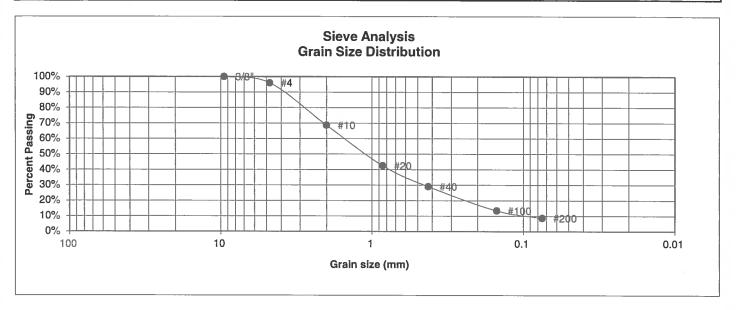


	LABOI RESU	RATORY TEST LTS	
DRAWN:	DATE:	CHECKED:	11/8/17

JOB NO.: 171198 FIG NO.:

B-12

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	55	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

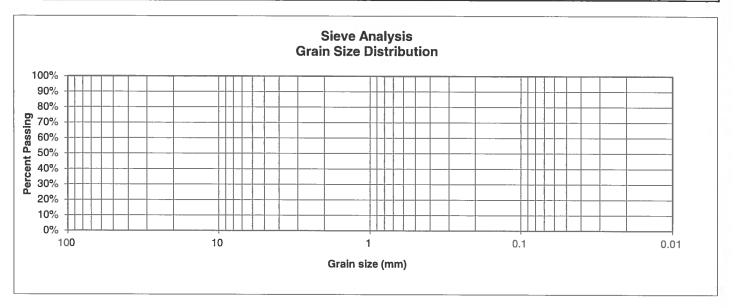


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	96.0%	<u>Swell</u>
10	68.6%	Moisture at start
20	42.5%	Moisture at finish
40	29.0%	Moisture increase
100	13.5%	Initial dry density (pcf)
200	8.7%	Swell (psf)



	LABOI RESU	RATORY TI LTS	EST	
DRAWN:	DATE:	CHECKED:	a	DATE

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	2	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	3	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL

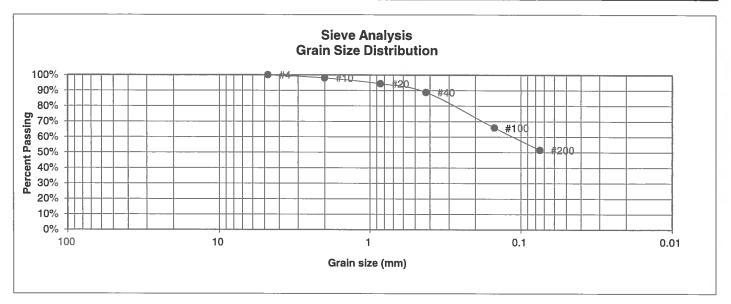


U.S.	Percent	Atterberg
Sieve #	<u>Finer</u>	<u>Limits</u>
3"		Plastic Limit
1 1/2"		Liquid Limit
3/4"		Plastic Index
1/2"		
3/8"		
4		<u>Swell</u>
10		Moisture at start 13.6%
20		Moisture at finish 23.9%
40		Moisture increase 10.3%
100		Initial dry density (pcf) 101
200		Swell (psf) 1720



LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	h	U/8/17	

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	2	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	25	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

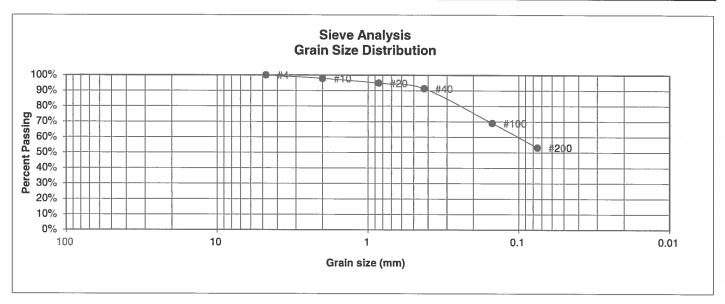


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 10 Liquid Limit 20 Plastic Index 10
4	100.0%	<u>Swell</u>
10	98.0%	Moisture at start
20	94.4%	Moisture at finish
40	88.8%	Moisture increase
100	66.0%	Initial dry density (pcf)
200	51.6%	Swell (psf)



	LABOI RESUI	RATORY T LTS	EST	
DRAWN:	DATE:	CHECKED:	a	DATE: 11/8/17

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	2	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	28	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit 16 Liquid Limit 30 Plastic Index 14
4	100.0%	<u>Swell</u>
10	97.9%	Moisture at start
20	94.9%	Moisture at finish
40	91.3%	Moisture increase
100	69.0%	Initial dry density (pcf)
200	53.2%	Swell (psf)

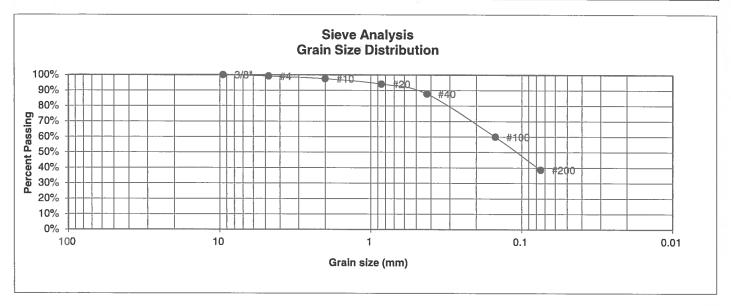


LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	h	DATE: 11/8/17	

JOB NO.: 171198 FIG NO.:

B-16

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	1	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



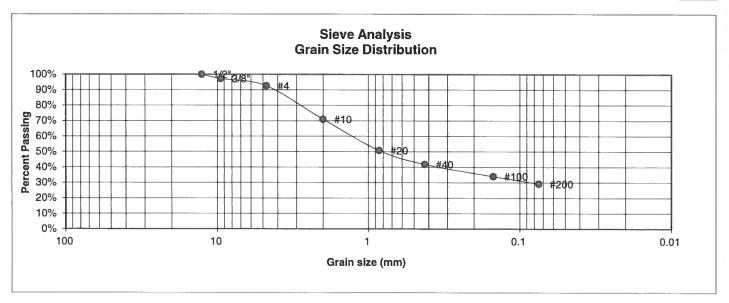
4 99.1% <u>Swell</u> 10 97.5% Moisture at start 20 94.1% Moisture at finish 40 87.8% Moisture increase 100 59.9% Initial dry density (pcf)	U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
20 94.1% Moisture at finish 40 87.8% Moisture increase			Swell
40 87.8% Moisture increase	10	97.5%	Moisture at start



LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED:	DATE: 4/8/17

JOB NO.: 171198 FIG NO.:

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	3	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL



U.S. Sieve # 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
1/2" 3/8"	100.0% 97.2%	
4	92.6%	<u>Swell</u>
10	71.0%	Moisture at start
20	50.8%	Moisture at finish
40	41.8%	Moisture increase
100	34.0%	Initial dry density (pcf)
200	29.2%	Swell (psf)

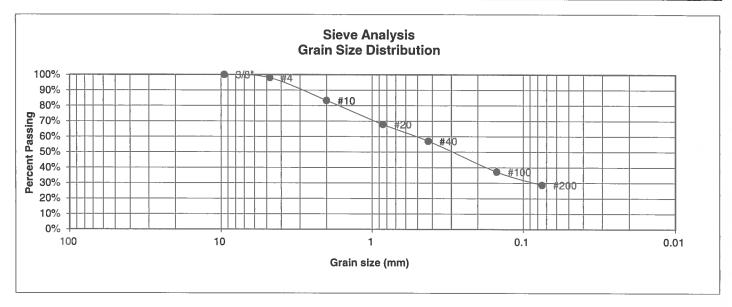


LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	DATE:	

24 36 12

> JOB NO.: 171198 FIG NO.: 13 -18

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	5	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



U.S. Sieve # 3" 1 1/2" 3/4" 1/2"	Percent Finer	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8" 4	100.0% 98.0%	Consti
10	83.4%	<u>Swell</u> Moisture at start
20 40	67.9% 57.1%	Moisture at finish Moisture increase
100 200	37.3% 28.7%	Initial dry density (pcf) Swell (psf)



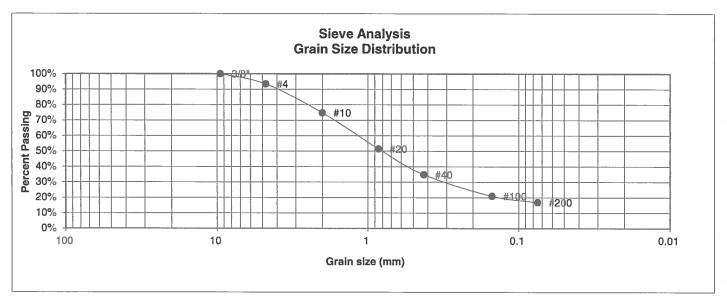
LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 4/8/17

27 41 14

> JOB NO.: 171198

FIG NO.: B-19

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	7	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL



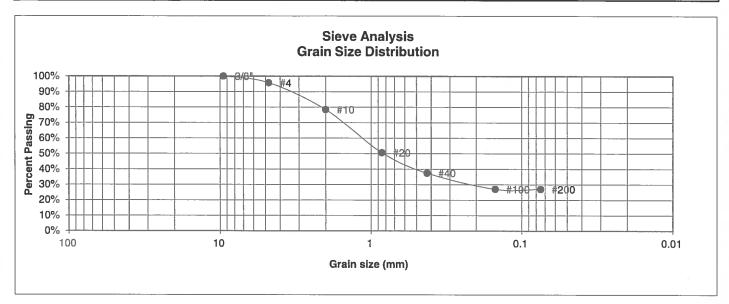
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	93.5%	Swell
10	74.8%	Moisture at start
20 40	51.5% 34.9%	Moisture at finish Moisture increase
100 200	20.9% 16.9%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 11/8/17

FIG NO.: B-20

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	9	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



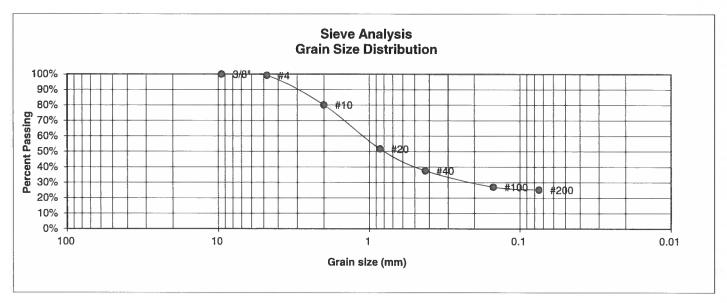
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	95.8%	Swell
10	78.4%	Moisture at start
20 40	50.7% 37.4%	Moisture at finish Moisture increase
100 200	27.1% 27.1%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 1/8/17

FIG NO.:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	10	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



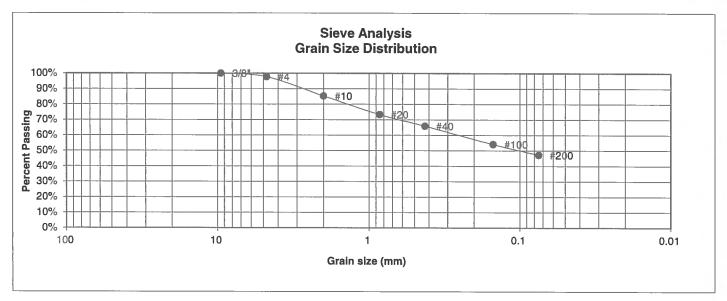
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	99.1% 80.1%	<u>Swell</u> Moisture at start
20	51.7%	Moisture at finish
40	37.6%	Moisture increase
100	27.1%	Initial dry density (pcf)
200	25.2%	Swell (psf)



	LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED: W	DATE: 11/8/17	

JOB NO.: 171198 FIG NO.: B-22

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	11	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL



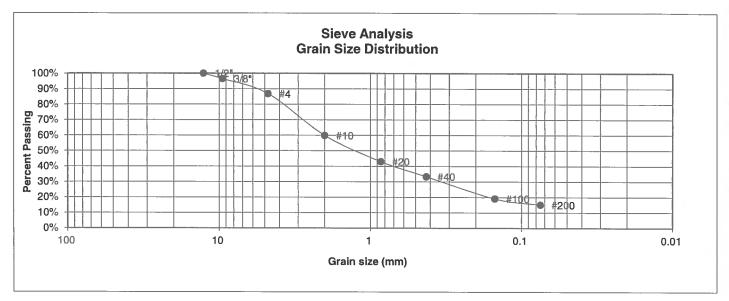
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 15 Liquid Limit 32 Plastic Index 17
3/8"	100.0%	
4	97.7%	Swell
10	85.3%	Moisture at start
20	73.3%	Moisture at finish
40	65.9%	Moisture increase
100 200	54.1% 47.2%	Initial dry density (pcf) Swell (psf)



	LABO! RESUI	RATORY TEST LTS	
DRAWN:	DATE:	CHECKED:	DATE:

FIG NO.: B-23

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	12	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



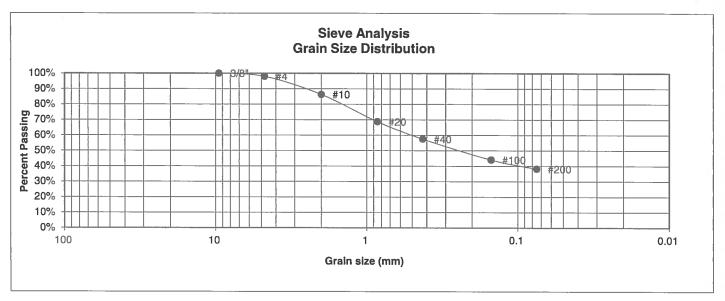
U.S. <u>Sieve #</u> 3"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit
1 1/2" 3/4"		Liquid Limit Plastic Index
1/2"	100.0%	Plastic Index
3/8"	96.5%	
4	86.9%	<u>Swell</u>
10	59.9%	Moisture at start
20	43.0%	Moisture at finish
40	33.3%	Moisture increase
100	19.1%	Initial dry density (pcf)
200	15.1%	Swell (psf)



	LABOI RESUI	RATORY T	EST	
DRAWN:	DATE:	CHECKED:	6	DATE: 17

JOB NO.: 171198 FIG NO.: B-24

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	14	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



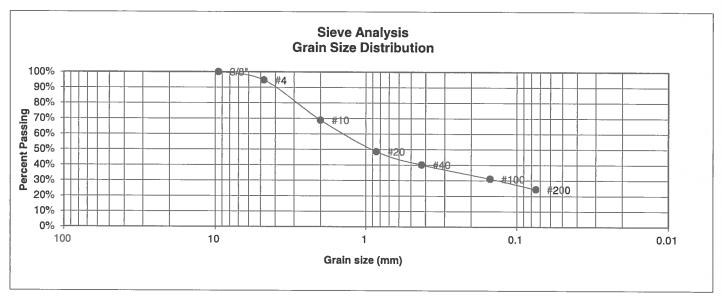
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	98.0%	<u>Swell</u>
10	86.3%	Moisture at start
20	68.7%	Moisture at finish
40	57.5%	Moisture increase
100	44.1%	Initial dry density (pcf)
200	38.2%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	4	DATE: 11/8/17

JOB NO.: 171198 FIG NO.: 3-25

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	16	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



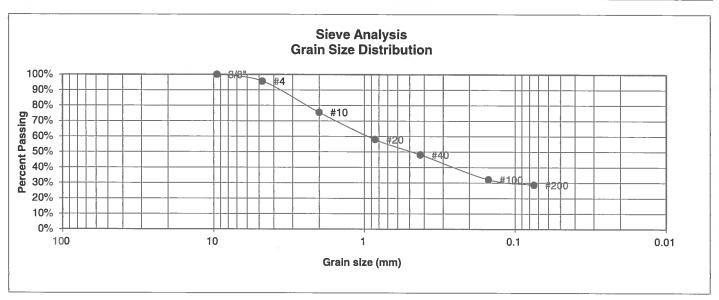
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	94.7% 68.9%	<u>Swell</u> Moisture at start
20	48.6%	Moisture at finish
40	40.1%	Moisture increase
100	31.1%	Initial dry density (pcf)
200	24.4%	Swell (psf)



LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17

JOB NO.: 171198 FIG NO.: 13-26

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	18	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



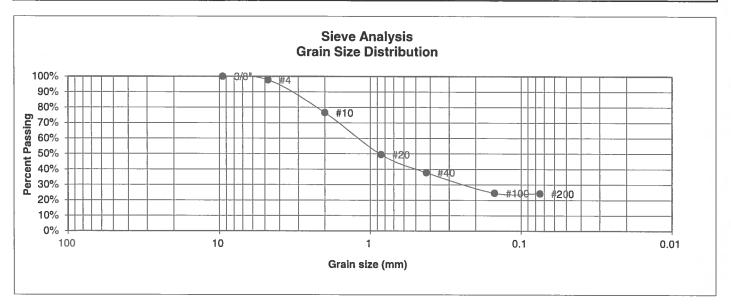
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	
4	95.7%	Swell
10	75.5%	Moisture at start
20	58.0%	Moisture at finish
40	48.2%	Moisture increase
100 200	32.1% 28.7%	Initial dry density (pcf) Swell (psf)
		" ,



LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	h.	DATE:	

FIG NO.: B-27

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	20	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	
4	97.8%	Swell
10	76.6%	Moisture at start
20	49.5%	Moisture at finish
40	37.8%	Moisture increase
100 200	24.6% 24.3%	Initial dry density (pcf) Swell (psf)

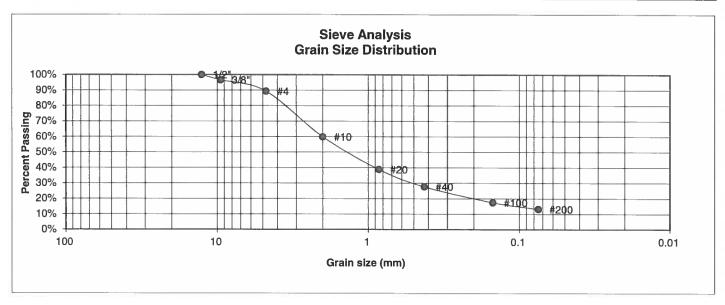


LABORATORY TEST RESULTS			
DRAWN:	DATE;	CHECKED:	U/8/17

ЈОВ NO.: 171198

FIG NO.:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	21	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent Finer	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
1/2" 3/8"	100.0% 96.6%	
4	89.3%	<u>Swell</u>
10	59.9%	Moisture at start
20	38.8%	Moisture at finish
40	27.7%	Moisture increase
100	17.5%	Initial dry density (pcf)
200	13.2%	Swell (psf)

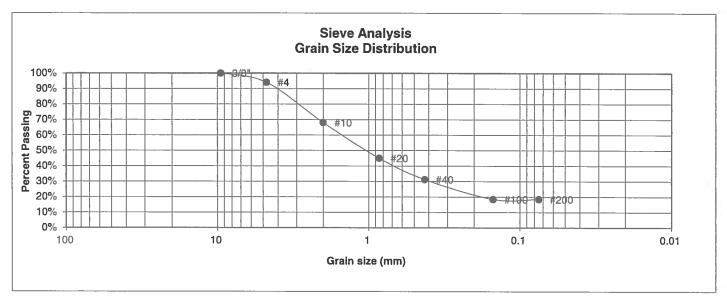


LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	h	UBIT 7	

JOB NO.: 171198 FIG NO.:

13-29

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	22	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



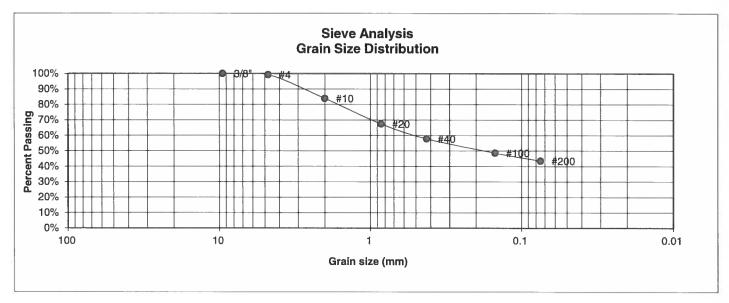
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	94.1%	<u>Swell</u>
10	68.0%	Moisture at start
20	45.0%	Moisture at finish
40	31.2%	Moisture increase
100	18.5%	Initial dry density (pcf)
200	18.5%	Swell (psf)



LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED:	DATE: 0/8/12

FIG NO.:

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	23	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	99.3%	<u>Swell</u>
10	84.0%	Moisture at start
20 40	67.5% 57.8%	Moisture at finish Moisture increase
100 200	48.6% 43.5%	Initial dry density (pcf) Swell (psf)

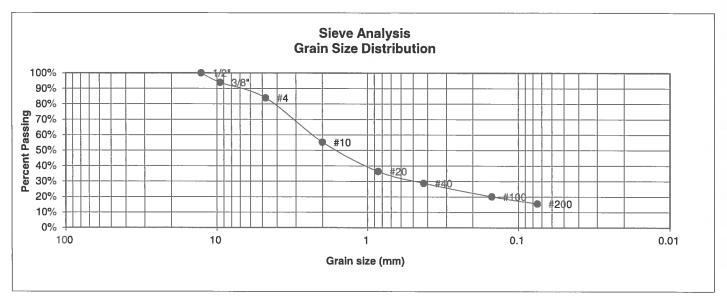


LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	L	DATE: 11/8/17	

FIG NO.:

13-31

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	29	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



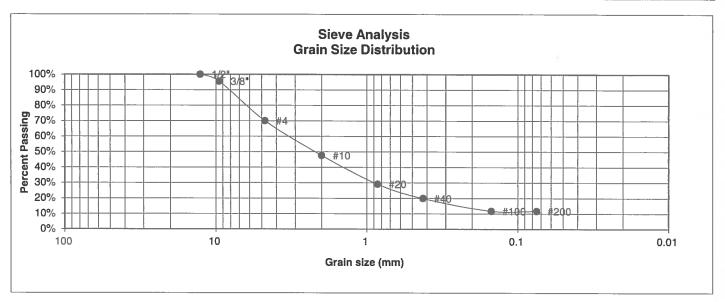
U.S. Sieve # 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
3/8"	93.9%	OII
4	83.9%	<u>Swell</u>
10	55.3%	Moisture at start
20	36.4%	Moisture at finish
40	28.8%	Moisture increase
100	20.1%	Initial dry density (pcf)
200	15.6%	Swell (psf)



LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	h	DATE:	

JOB NO.: 171198 FIG NO.:

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	31	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

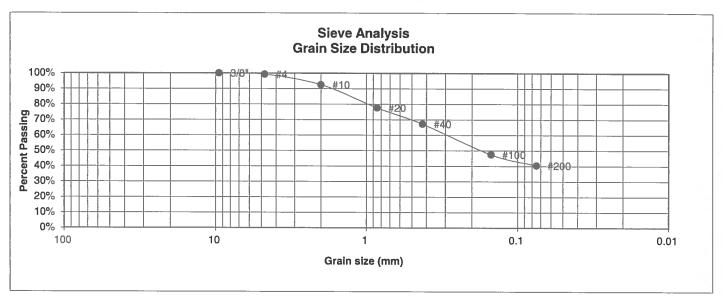


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent Finer 100.0% 95.6%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	70.0%	<u>Swell</u>
10	47.6%	Moisture at start
20	29.1%	Moisture at finish
40	19.9%	Moisture increase
100	11.8%	Initial dry density (pcf)
200	11.8%	Swell (psf)



	LABOI RESUI	RATORY TI LTS	EST	
DRAWN:	DATE:	CHECKED:	A	DATE/S/17

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	32	JOB NO.	171198
DEPTH (FT)	25	TEST BY	BL



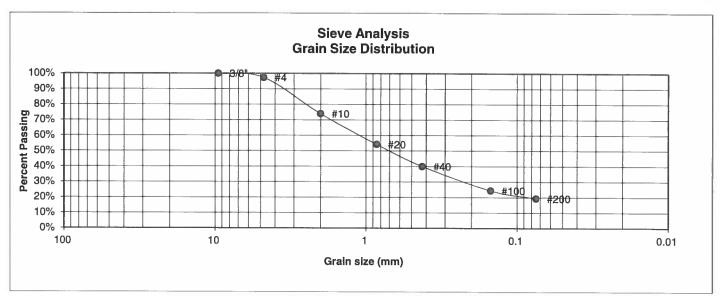
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	99.3%	Swell
10	92.5%	Moisture at start
20	77.7%	Moisture at finish
40	67.2%	Moisture increase
100	47.5%	Initial dry density (pcf)
200	40.4%	Swell (psf)



	LABOI RESUI	RATORY T LTS_	EST	
DRAWN:	DATE:	CHECKED:	a	DATE: 1/8/17

FIG NO.: B-34

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	33	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



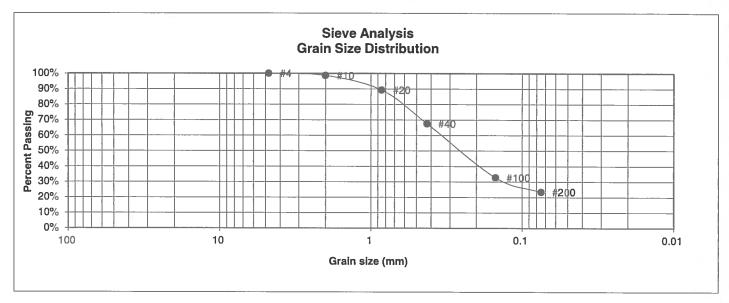
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	97.3%	<u>Swell</u>
10	73.9%	Moisture at start
20	54.2%	Moisture at finish
40	40.0%	Moisture increase
100	24.4%	Initial dry density (pcf)
200	19.3%	Swell (psf)



=	LABO RESU	RATORY T LTS	EST	
DRAWN:	DATE:	CHECKED:	a	DATE: 4/8/17

JOB NO.: 171198 FIG NO.: B-35

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	38	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	100.0%	<u>Swell</u>
10	98.7%	Moisture at start
20	89.3%	Moisture at finish
40	67.5%	Moisture increase
100	32.8%	Initial dry density (pcf)
200	23.4%	Swell (psf)

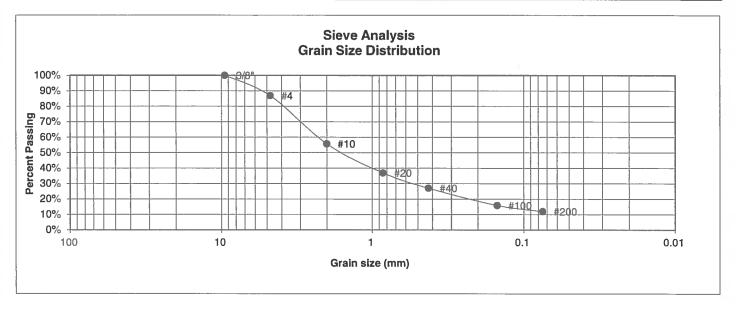


-	LABOI RESU	RATORY 1 LTS	EST		
DRAWN:	DATE:	CHECKED:	n	11/8/11	

FIG NO.:

13-36

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	40	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



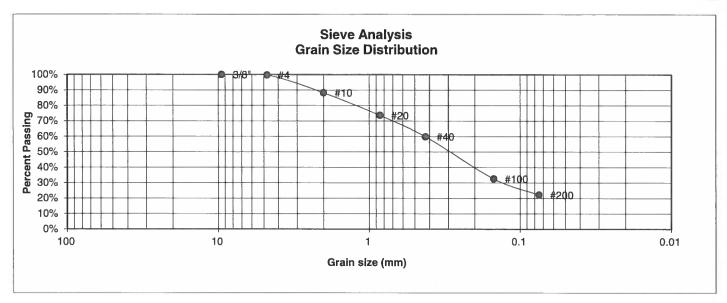
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	86.9% 55.7%	<u>Swell</u> Moisture at start
20 40 100 200	37.0% 27.0% 15.9% 12.0%	Moisture at finish Moisture increase Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	n	U/B/12

FIG NO.:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	41	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



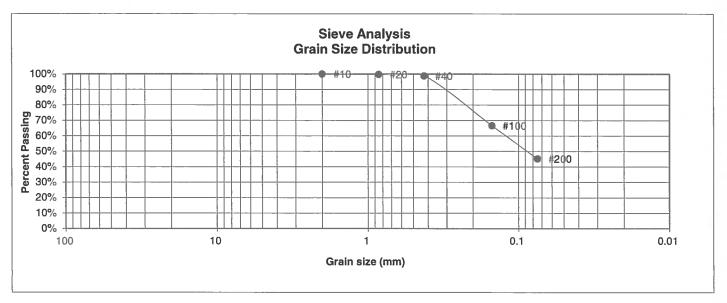
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	99.7%	<u>Swell</u>
10	88.3%	Moisture at start
20 40	73.6% 59.8%	Moisture at start Moisture at finish Moisture increase
100	32.6%	Initial dry density (pcf)
200	22.2%	Swell (psf)



LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED;	DATE:

JOB NO.: 171198 FIG NO.: 13-38

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	42	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL



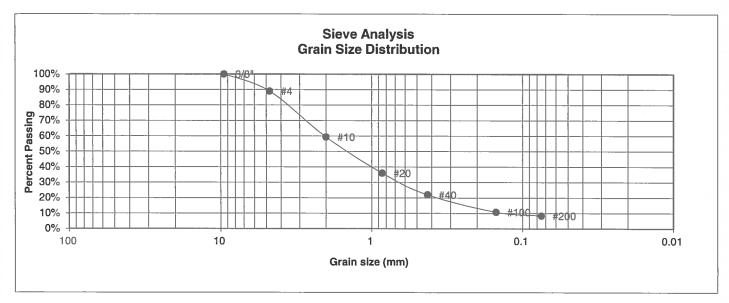
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4		Swell
10	100.0%	Moisture at start
20	99.7%	Moisture at finish
40	98.8%	Moisture increase
100 200	66.6% 45.3%	Initial dry density (pcf) Swell (psf)



	LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED: DATE:			

JOB NO.: 171198 FIG NO.: B-39

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	45	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



U.S. Sieve # 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
3/8" 4	100.0% 89.0%	Swall
10	59.3%	<u>Swell</u> Moisture at start
20 40	36.0% 21.9%	Moisture at finish Moisture increase
100 200	10.8% 8.4%	Initial dry density (pcf) Swell (psf)

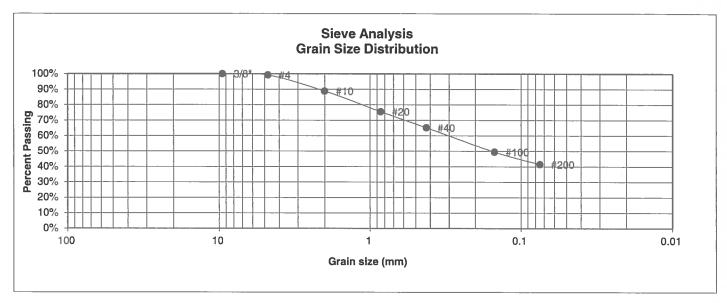


LABORATORY TEST RESULTS					
DRAWN: DATE: CHECKED: L DATE: 17					

JOB NO.: 171198 FIG NO.:

13-40

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	46	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



U.S. Sieve # 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	0 "
4	99.2%	Swell
10	88.8%	Moisture at start
20	75.5%	Moisture at finish
40	65.2%	Moisture increase
100	49.5%	Initial dry density (pcf)
200	41.5%	Swell (psf)

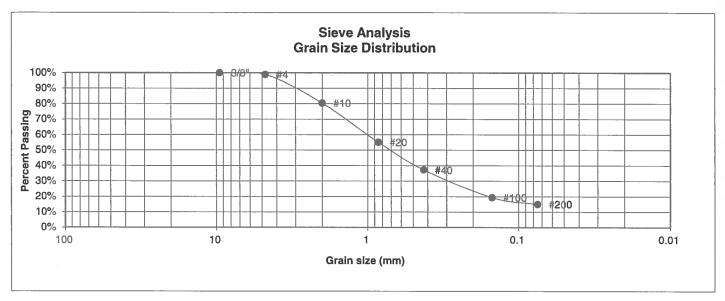


LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	a	DATE: 11/8/17

15 33 18

> JOB NO.: 171198 FIG NO.: 3-4/

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	47	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



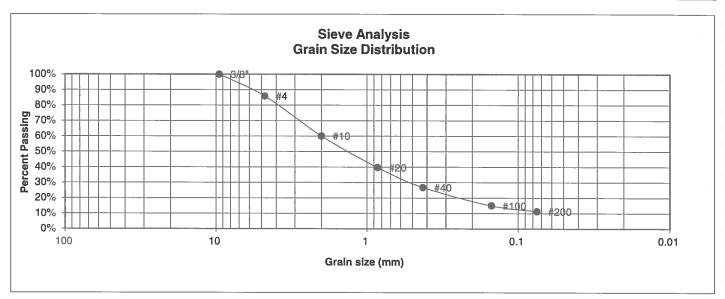
U.S.	Percent	Atterberg	
Sieve #	<u>Finer</u>	<u>Limits</u>	
3"		Plastic Limit	
1 1/2"		Liquid Limit	
3/4"		Plastic Index	
1/2"			
3/8"	100.0%		
4	98.9%	<u>Swell</u>	
10	80.4%	Moisture at start	9.0%
20	55.1%	Moisture at finish	20.0%
40	37.2%	Moisture increase	11.0%
100	19.4%	Initial dry density (pcf)	99
200	15.0%	Swell (psf)	30



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	DATE: 4/8/17	

JOB NO.: 171198 FIG NO.:

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	48	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



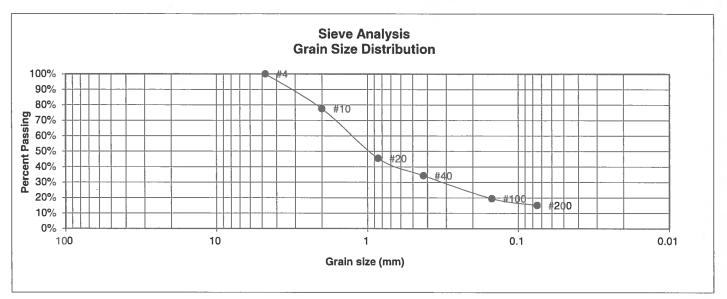
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	85.9%	<u>Swell</u>
10	60.0%	Moisture at start
20	39.7%	Moisture at finish
40	26.8%	Moisture increase
100	15.0%	Initial dry density (pcf)
200	11.3%	Swell (psf)



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DRAWN:	DATE:	CHECKED:	n	DATE:

JOB NO.: 171198 FIG NO.:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	49	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



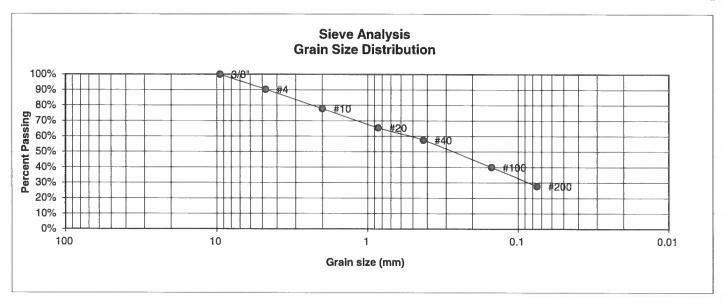
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	100.0%	Swell
10	77.6%	Moisture at start
20 40	45.5% 34.3%	Moisture at finish Moisture increase
100 200	19.5% 15.1%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	a	DATE: 11/8/17

JOB NO.: 171198 FIG NO.: B-44

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	50	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

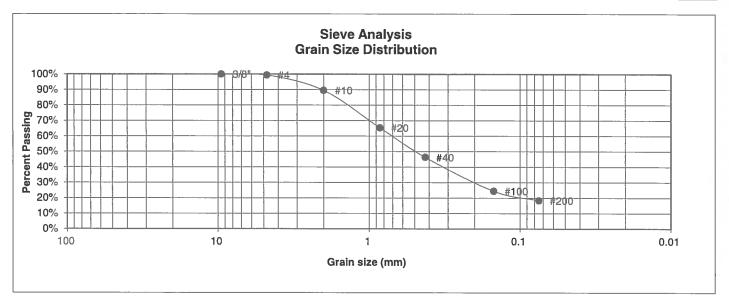


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	90.3%	<u>Swell</u>
10	77.8%	Moisture at start
20	65.4%	Moisture at finish
40	57.5%	Moisture increase
100	40.0%	Initial dry density (pcf)
200	27.7%	Swell (psf)



;	LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	n	DATE: 1/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	51	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



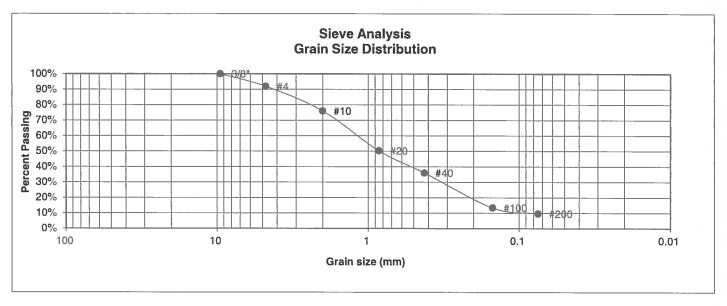
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	99.4%	Swell
10	89.5%	Moisture at start
20	65.4%	Moisture at finish
40	46.3%	Moisture increase
100 200	24.5% 18.3%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	en	DATE: 11/8/17

FIG NO.: 13-46

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	52	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



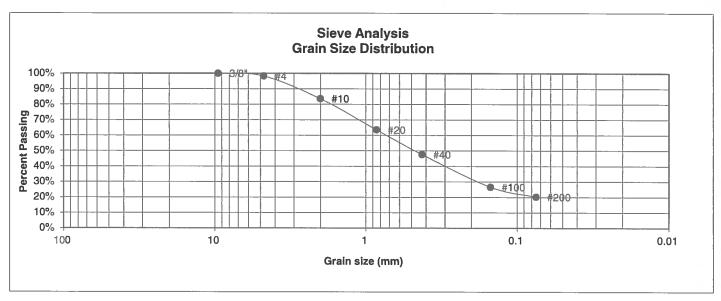
U.S. Percent Sieve # Finer 3" 1 1/2" 3/4" 1/2" 3/8" 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4 91.9%	<u>Swell</u>
10 76.1%	Moisture at start
20 50.3%	Moisture at finish
40 36.0%	Moisture increase
100 13.4%	Initial dry density (pcf)
200 9.7%	Swell (psf)



	LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	4_	DATE:	

JOB NO.: 171198 FIG NO.:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	53	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



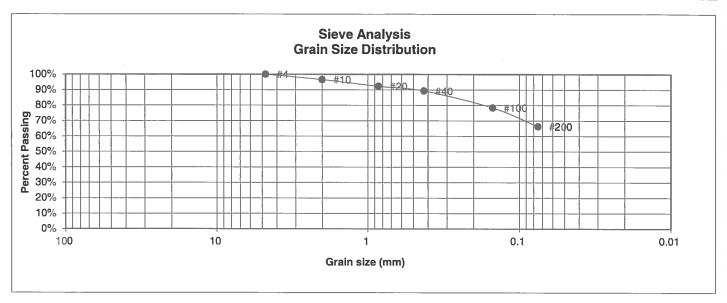
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent Finer	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8" 4	100.0% 98.2%	Swell
10	83.6%	Moisture at start
20 40	63.5% 47.6%	Moisture at finish Moisture increase
100 200	26.6% 20.2%	Initial dry density (pcf) Swell (psf)



ŧ	LABOI RESU	RATORY TES	Т
DRAWN:	DATE:	CHECKED:	DATE: (1/8/17

FIG NO.: 13-48

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	6	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	400.00	9 "
4	100.0%	Swell
10	96.6%	Moisture at start
20	92.3%	Moisture at finish
40	89.3%	Moisture increase
100 200	78.4% 66.3%	Initial dry density (pcf) Swell (psf)



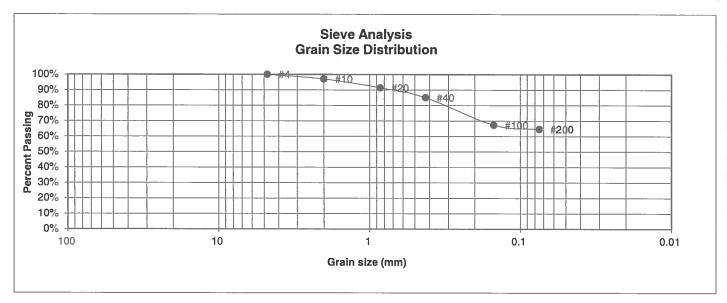
LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	1/8/17	

13 21 8

> JOB NO.: 171198

FIG NO.:

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	24	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL



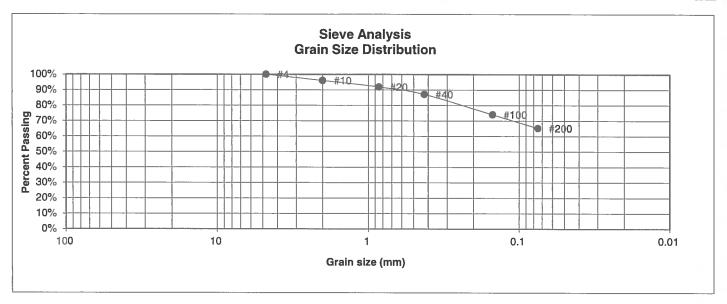
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 21 Liquid Limit 29 Plastic Index 8
4	100.0%	Swell
10	97.1%	Moisture at start
20	91.4%	Moisture at finish
40	85.1%	Moisture increase
100	67.3%	Initial dry density (pcf)
200	64.7%	Swell (psf)



LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	h	DATE: 1/9/17	

JOB NO.: 171198 FIG NO.:

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	26	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



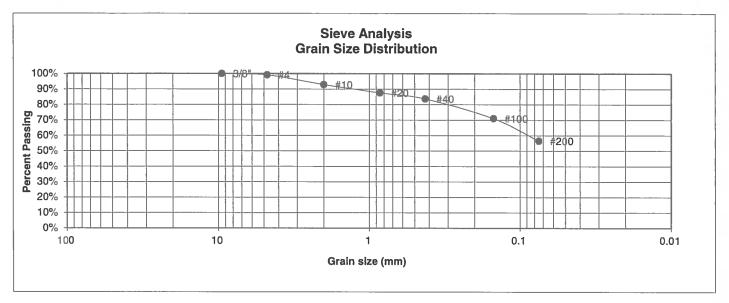
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index	
1/2" 3/8"			
4	100.0%	<u>Swell</u>	
10	96.1%	Moisture at start	14.4%
20	91.9%	Moisture at finish	23.5%
40	87.2%	Moisture increase	9.1%
100	74.0%	Initial dry density (pcf)	100
200	65.2%	Swell (psf)	2060



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 1/8/17

FIG NO.: B-51

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	43	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



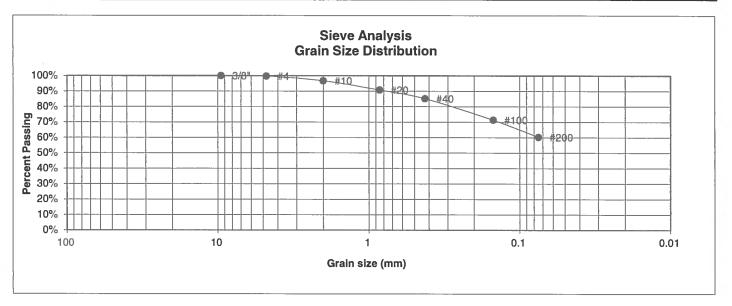
U.S.	Percent	Atterberg	
Sieve #	<u>Finer</u>	<u>Limits</u>	
3"		Plastic Limit	
1 1/2"		Liquid Limit	
3/4"		Plastic Index	
1/2"			
3/8"	100.0%		
4	99.2%	<u>Swell</u>	
10	92.9%	Moisture at start	12.1%
20	87.6%	Moisture at finish	21.6%
40	83.7%	Moisture increase	9.5%
100	71.0%	Initial dry density (pcf)	103
200	56.5%	Swell (psf)	1970



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 11/8/17

FIGNO .: 3-52

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	44	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
1/2" 3/8" 4 10	100.0% 99.7% 96.7%	<u>Swell</u> Moisture at start
20 40 100 200	90.8% 85.2% 71.3% 60.3%	Moisture at finish Moisture increase Initial dry density (pcf) Swell (psf)

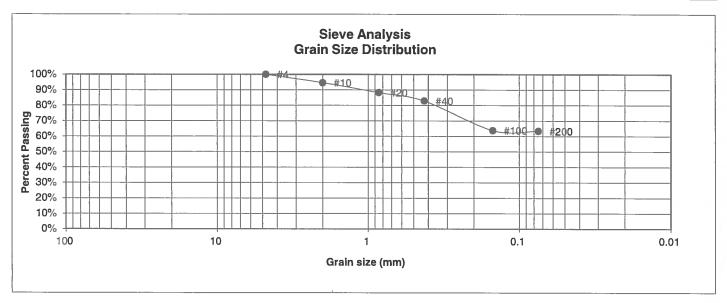


LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: U/8/17

FIG NO.:

13-53

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	54	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 14 Liquid Limit 27 Plastic Index 13
4	100.0%	Swell
10	94.6%	Moisture at start
20	88.2%	Moisture at finish
40	82.8%	Moisture increase
100 200	63.8% 63.3%	Initial dry density (pcf) Swell (psf)

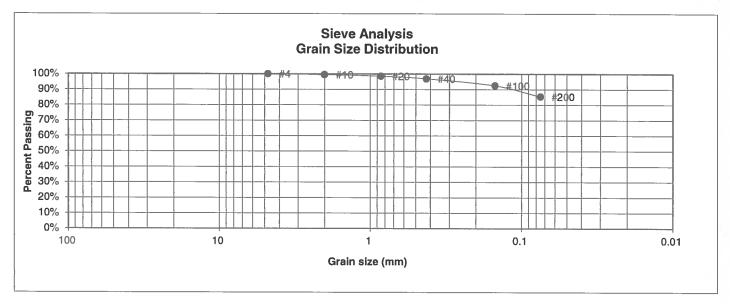


LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 117

FIG NO.:

B.54

UNIFIED CLASSIFICATION	ML	CLIENT	TECH CONTRACTORS
SOIL TYPE #	5	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	2	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



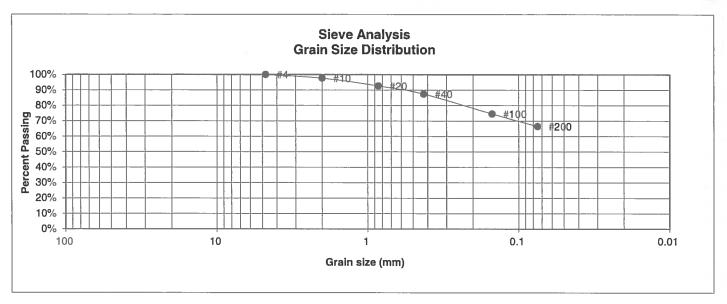
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit 26 Liquid Limit 40 Plastic Index 14
4	100.0%	Swell
10	99.5%	Moisture at start
20	98.6%	Moisture at finish
40	96.9%	Moisture increase
100	92.5%	Initial dry density (pcf)
200	85.4%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17	

FIG NO.:

UNIFIED CLASSIFICATION	ML	CLIENT	TECH CONTRACTORS
SOIL TYPE #	5	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	17	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	100.0%	<u>Swell</u>
10	97.7%	Moisture at start
20	92.7%	Moisture at finish
40	87.5%	Moisture increase
100 200	74.6% 66.5%	Initial dry density (pcf) Swell (psf)

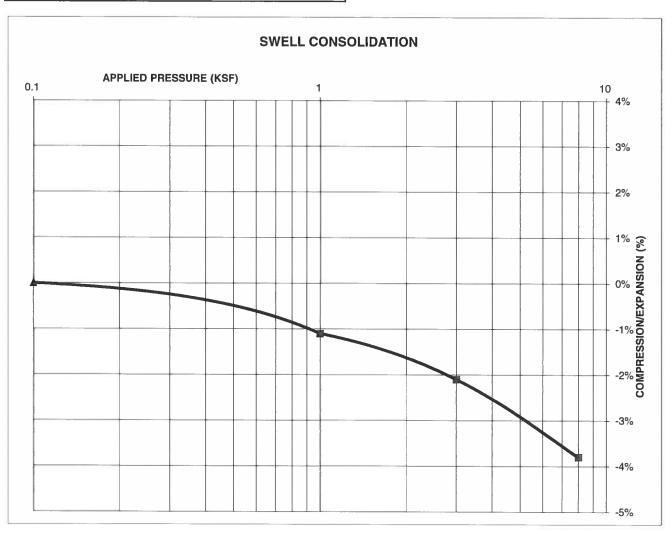


	LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 1/1/8/17	

FIG NO.:

TEST BORING # 28 DEPTH(ft) 2-3 DESCRIPTION CL SOIL TYPE 2 NATURAL UNIT DRY WEIGHT (PCF) 101 NATURAL MOISTURE CONTENT 22.2% SWELL/CONSOLIDATION (%) 0.0%

JOB NO. 171198 CLIENT **TECH CONTRACTORS** PROJECT WINDINGWALK & STONEBRIDGE





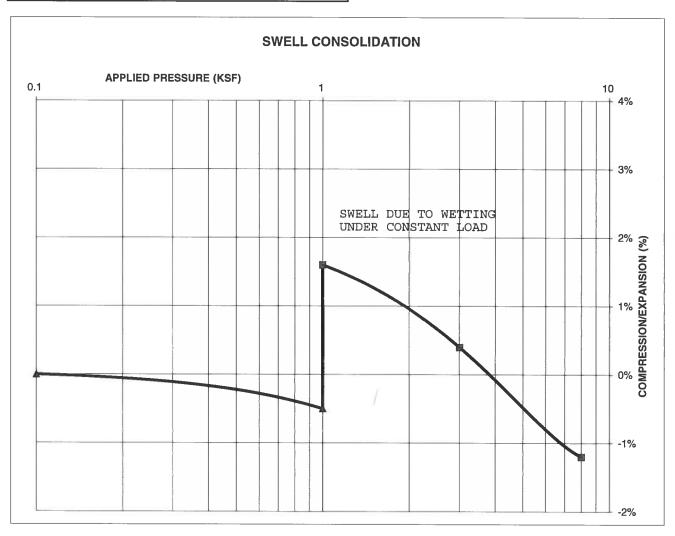
SWELL CONSOLIDATION				
TEST RESULTS				

DRAWN: DATE: CHECKED: 11/8/17 JOB NO.: 171198

FIG NO.: B-57

TEST BORING # 11 DEPTH(ft) 20
DESCRIPTION SC SOIL TYPE 3
NATURAL UNIT DRY WEIGHT (PCF) 112
NATURAL MOISTURE CONTENT 14.3%
SWELL/CONSOLIDATION (%) 2.1%

JOB NO. 171198
CLIENT TECH CONTRACTORS
PROJECT WINDINGWALK & STONEBRIDGE





SWELL CONSOLIDATION TEST RESULTS

DRAWN: DATE:

:

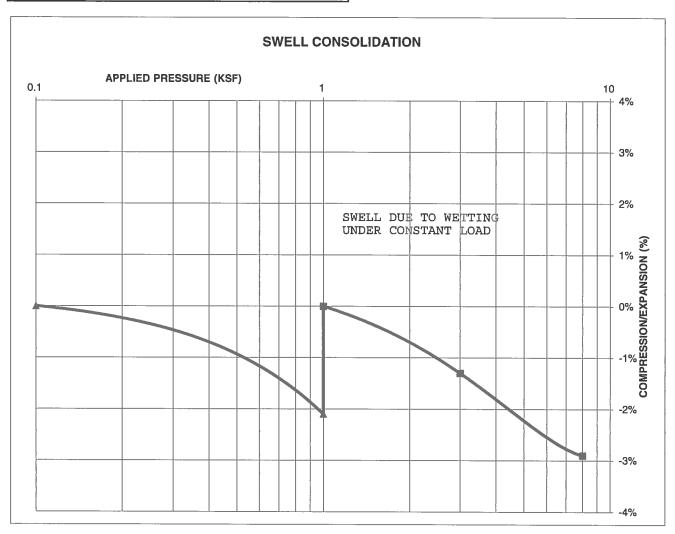
CHECKED: DATE: 11/8/17

JOB NO.: 171198

FIG NO.: B-58

TEST BORING # 23 DEPTH(ft) 15
DESCRIPTION SC SOIL TYPE 3
NATURAL UNIT DRY WEIGHT (PCF) 108
NATURAL MOISTURE CONTENT 20.1%
SWELL/CONSOLIDATION (%) 2.1%

JOB NO. 171198
CLIENT TECH CONTRACTORS
PROJECT WINDINGWALK & STONEBRIDGE



DRAWN:



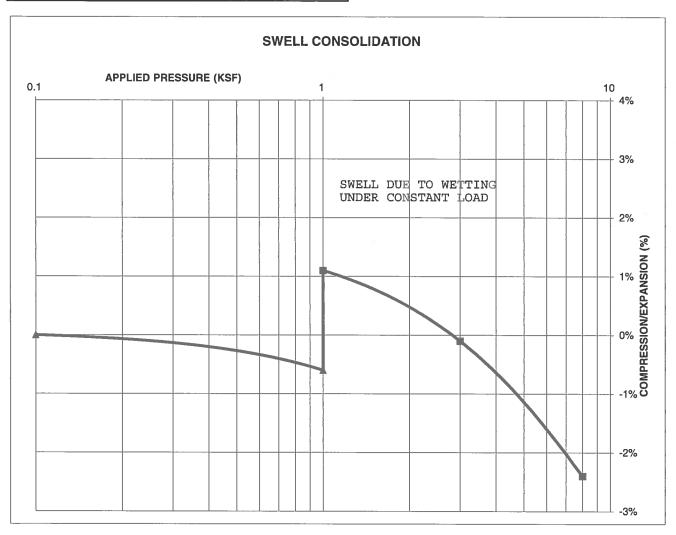
SWELL CONSOLIDATION TEST RESULTS				
	TILOULIU			
Ñ.	DATE:	CHECKED.	DA.	

JOB NO.: 171198

~ 4/6/17

TEST BORING # 24 DEPTH(ft) 20
DESCRIPTION CL SOIL TYPE 4
NATURAL UNIT DRY WEIGHT (PCF) 117
NATURAL MOISTURE CONTENT 10.9%
SWELL/CONSOLIDATION (%) 1.7%

JOB NO. 171198
CLIENT TECH CONTRACTORS
PROJECT WINDINGWALK & STONEBRIDGE





SWELL CONSOLIDATION			
TEST RESULTS			

DRAWN: DATE:

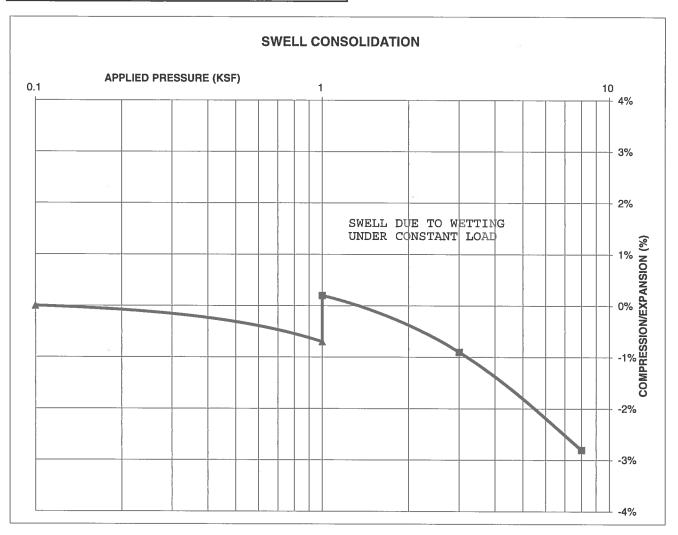
CHECKED: A PATE:

JOB NO.: 171198

FIG NO.:

TEST BORING # 17 DEPTH(ft) 2-3
DESCRIPTION ML SOIL TYPE 5
NATURAL UNIT DRY WEIGHT (PCF) 111
NATURAL MOISTURE CONTENT 15.6%
SWELL/CONSOLIDATION (%) 0.9%

JOB NO. 171198
CLIENT TECH CONTRACTORS
PROJECT WINDINGWALK & STONEBRIDGE





SWELL CONSOLIDATION TEST RESULTS

DRAWN: DATE:

CHECKED

U/8/17

JOB NO.: 171198

FIG NO.: B-61

	CLIENT	TECH CONTRACTORS	JOB NO.	171198
F	PROJECT	WINDINGWALK & STONEBRIDGE	DATE	9/6/2017
L	OCATION	WINDINGWALK & STONEBRIDGE	TEST BY	BL

BORING NUMBER	DEPTH, (ft)	SOIL TYPE NUMBER	UNIFIED CLASSIFICATION	WATER SOLUBLE SULFATE, (wt%)
TB-1	10	3	SM	<0.01
TB-39	10	1	SM	0.01
TB-42	20	3	SM	<0.01
TB-8	2-3	1	SM-SW	<0.01
TB-9	10	3	SM	<0.01
TB-11	20	3	SC	0.00
TB-6	10	4	CL	<0.01
TB-24	20	4	CL	<0.01
TB-25	10	2	CL	<0.01
TB-31	10	3	SM-SW	<0.01
TB-32	25	3	SM	<0.01
TB-35	2-3	1	SM	<0.01
TB-13	2-3	1	SC	<0.01
TB-45	5	3	SM-SW	0.00
TB-46	2-3	3	SC	<0.01
TB-51	5	3	SM	0.00
TB-12	10	3	SM	0.00
TB-29	5	3	SM	<0.01
TB-28	2-3	2	CL	<0.01
TB-19	5	1	SM-SW	0.00
TB-54	15	4	CL	0.00
TB-55	2-3	1	SM-SW	<0.01

QC BLANK PASS



LABORATORY TEST SULFATE RESULTS				
DRAWN:	DATE:	CHECKED:	11/8/17	

JOB NO.: 171198

FIG NO.: 13-62